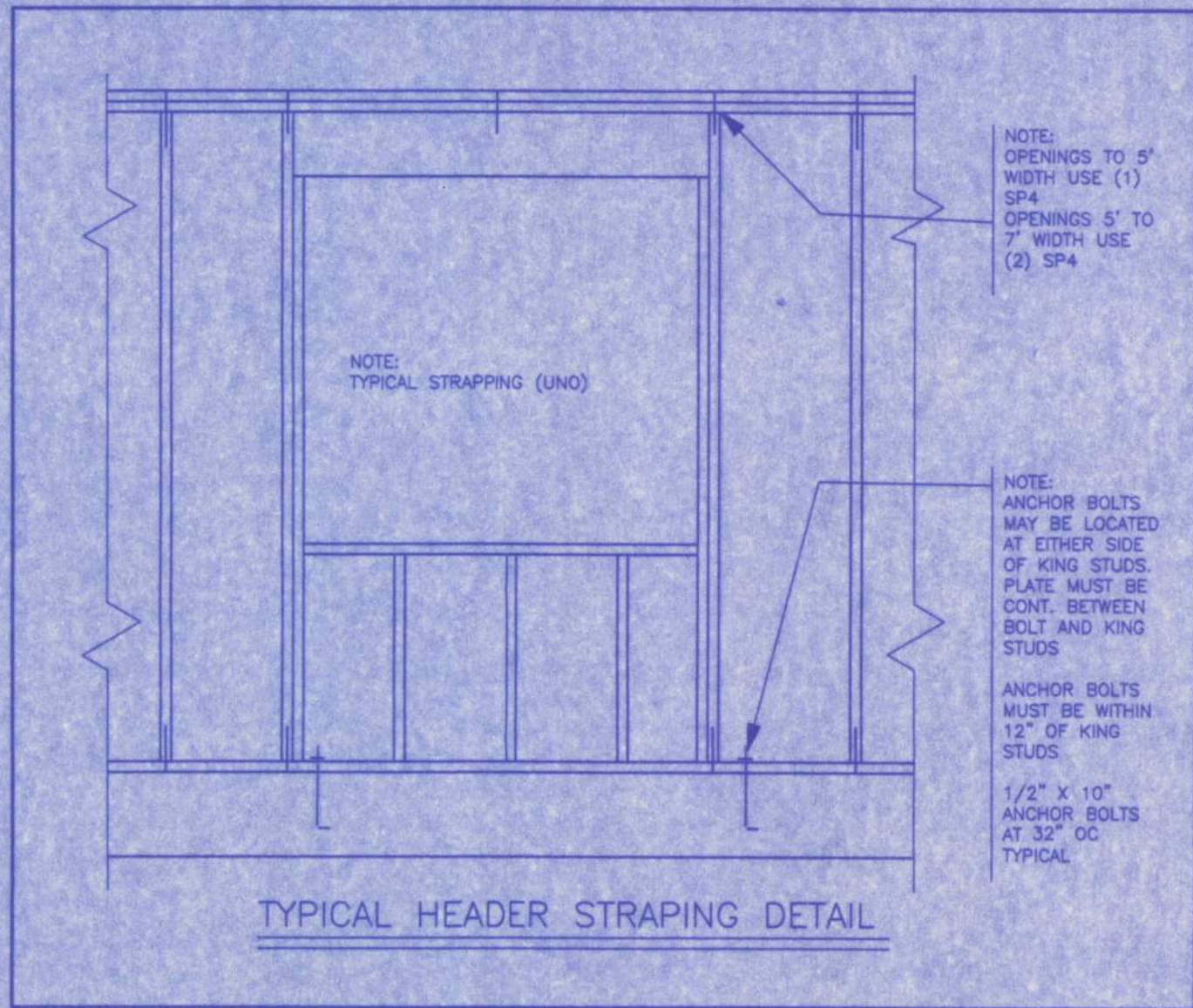
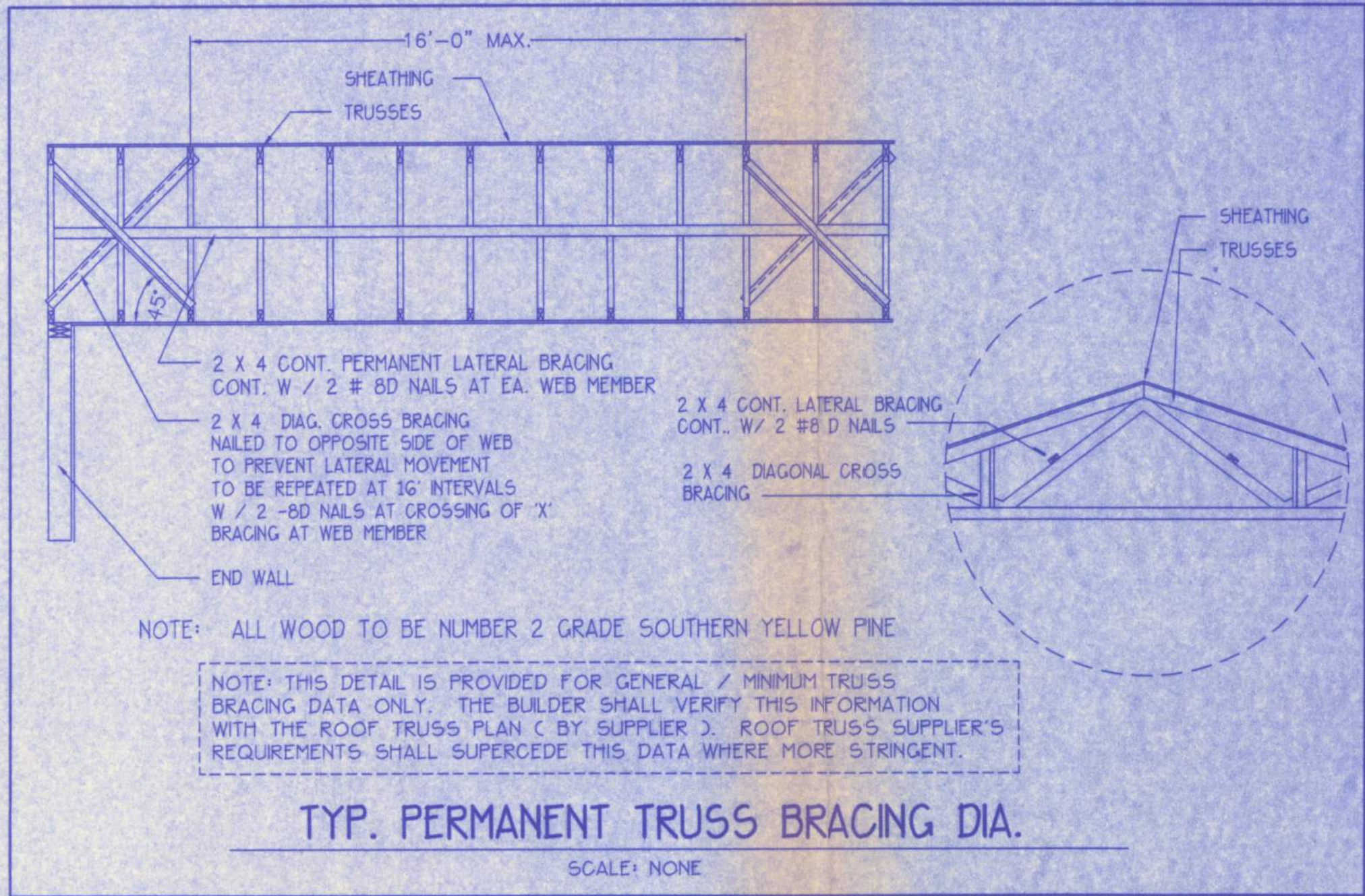




ENGINEER'S NOTES:

- The report establishes the minimum requirements for wind load stability. It is the owner/builder's responsibility to provide materials and construction techniques which comply with SBC requirements for the stated wind velocity. It is the builder's responsibility to provide a continuous load path from trusses to foundation.
- Since truss engineering was not complete at the time of this analysis, it is the builder's responsibility to select uplift connections based on truss engineering uplift and provide footings for interior bearing walls. Builder is to furnish truss engineering to wind load engineer for review of truss reactions on the building structure. Strap 2x6 rafters with min uplift connection 415lb each end, 2x6 rafters 700lb each end.
- Site analysis and preparation information is not part of this plan and is responsibility of the owner. All foundations and footings are designed for stable soil conditions with 1000 psf bearing capacity. It is the owner's / builder's responsibility to verify soil and clean fill are compacted to provide 1000 psf minimum bearing capacity or to request foundation design based on actual site conditions.
- Manufacturers and product number for connectors, anchors, and reinforcement are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads.
- Anchor bolts - A-307. Minimum embedment: 7" in concrete or reinforced bond beam, 15" in grouted CMU.
- Concrete - Minimum compressive strength $f_c = 2500$ psi.
- Rebar - Grade 40 deformed bars. $f_y = 36ksi$. All laps 40#Db (25' for #5) unless otherwise specified.
- Nails - All nails are common nails unless otherwise specified or accepted by SBC test reports as having equal structural values.

WALL STUD TABLE

STUDS ARE SPRUCE/PINE/FIR AT 16 IN. O.C.

1 - 2 X 4	TO 10 FT. WALL HEIGHT
2 - 2 X 4	TO 12.5 FT. WALL HEIGHT
3 - 2 X 4	TO 16.5 FT. WALL HEIGHT
1 - 2 X 6	TO 16 FT. WALL HEIGHT
2 - 2 X 6	TO 21 FT. WALL HEIGHT

TOP PLATE IS 2 - 2 X 4 (OR 2 X 6) SPRUCE/PINE/FIR.

BOTTOM PLATE IS 2 X 4 (OR 2 X 6) PRESS. TREATED BEARING ON FOUNDATION WALL.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2001, SECTION 1606 IS BASED ON REACTIONS, UPLIFTS AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQ. FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.		TOP CONNECTOR *		BOTTOM CONNECTOR *
< 415	H2.5	10-8d	415	NO SPECIAL CONNECTOR REQ.
< 750	H16	6-10d, 1-1/2"	750	NO SPECIAL CONNECTOR REQ.
< 905	H10	16-8d, 1-1/2"	905	NO SPECIAL CONNECTOR REQ.
< 1250	H16	10-10d, 1-1/2"	1250	SPH4 W/10-10d, 1-1/2" + 1/2" AB
< 1245	HTS20	24-10d, 1-1/2"	1245	LTT19 W/8-16d + 1/2" AB
< 2490	2-HTS20	24-10d, 1-1/2"	2490	HDBA-2.5", 5/8" AB

UPLIFT GREATER THAN 2500 LBS. REQUIRES ENGINEERING APPROVAL

* MODEL NUMBERS SHOWN ARE FOR SIMPSON CONNECTORS. THESE ARE LISTED AS EXAMPLES * NOT FOR ENDORSEMENT. OTHER MANUFACTURER'S CONNECTORS WITH SUFFICIENT LOAD CAPACITY MAY BE SUBSTITUTED. FOLLOW MANUFACT. INSTRUCTIONS.

DESIGN DATA

WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2
(FOR ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED AND GABLE-SHAPED ROOFS HAVING A MEAN ROOF HEIGHT NOT EXCEEDING THE LEAST HORIZONTAL DIMENSION OF THE BUILDING OR 60 FT. NOT SITED ON THE UPPER HALF OF A HILL OR ESCARPMENT 50 FT. IN EXPOSURE B, 30 FT. IN EXPOSURE C AND >10% SLOPE AND UNOBSTRUCTED UPWARD FOR 50' HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1) BASIC WIND SPEED = 110 MPH
2) WIND IMPORTANCE FACTOR = 1
3) BUILDING CATEGORY = 0
4) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
5) COMPONENTS AND CLADDING DESIGN WIND PRESSURES: (DOORS & WINDOWS) +21.5 / -23.1 PSF GARAGE DOOR (N/A) 9x7 +18.3 / -22.2 PSF, 16x7, +18.5 / -20.9
DESIGN LOADS
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
NO SNOW LOAD
WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2001 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMIT'S A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. SELECT UPLIFT CONNECTIONS AND PROVIDE FOOTINGS BASED ON TRUSS ENGINEERING REACTIONS. FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS.

WINDLOAD ENGINEER: Huey Hawkins, PE No. 39665 140 NW Ridgewood Avenue, Lake City, FL 32055 - 386-755-6166

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code 2001, Section 1606 wind loads, to the best of my knowledge.

SIGNATURE

DATE: 12/13/2005

A-4

FILE: 04-046	HUEY HAWKINS	SHEET: 4 OF 5
DATE: 11-20-04		CAD FILE: 04046
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
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