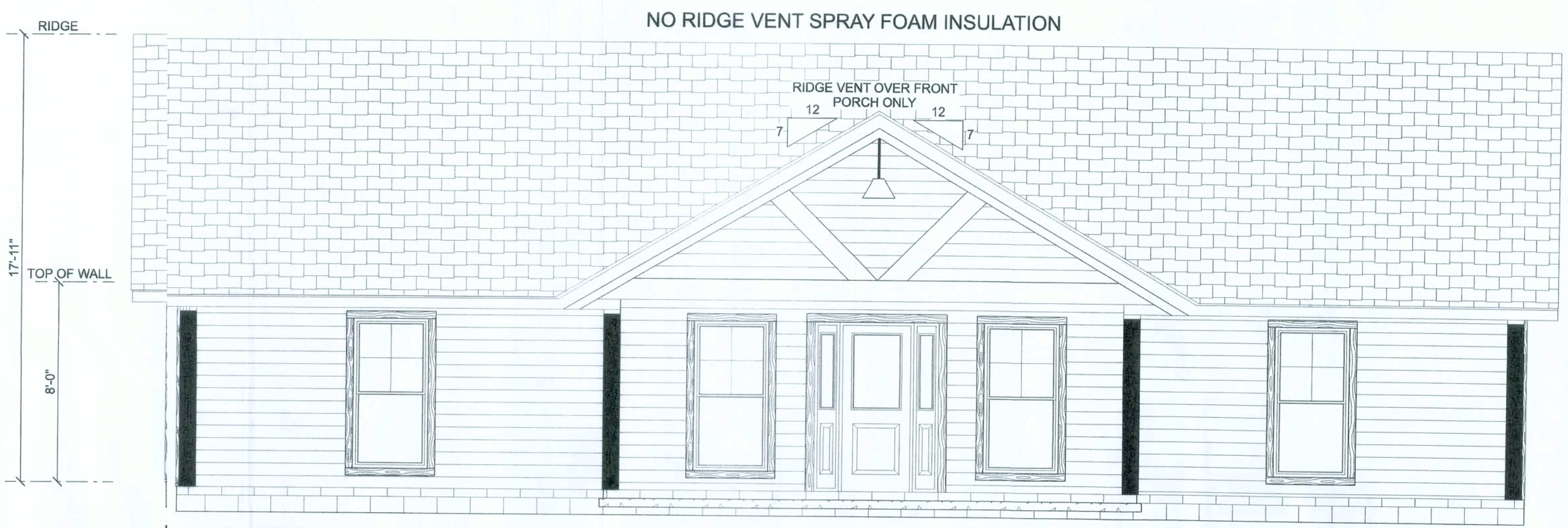


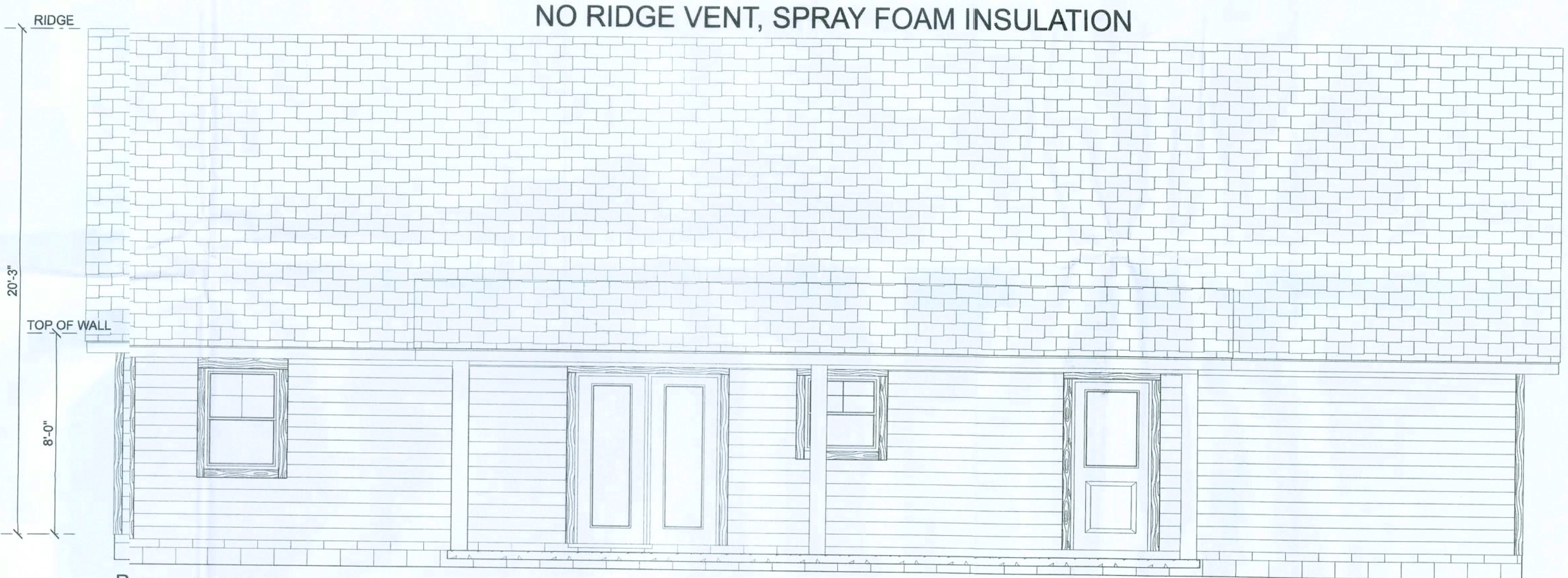
TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1" = 1'-0"



LEFT ELEVATION



FRONT ELEVATION



REAR ELEVATION



RIGHT ELEVATION

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



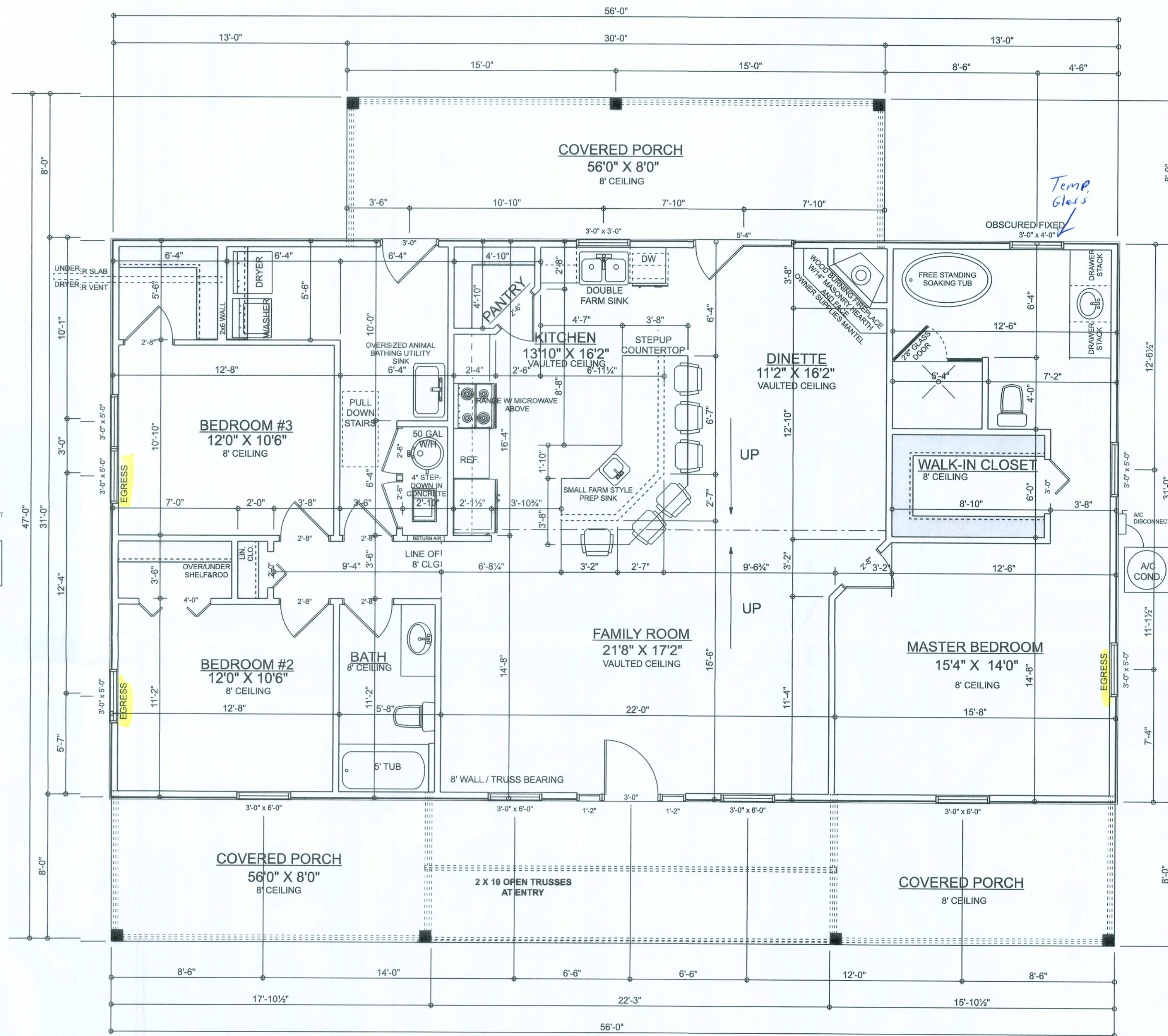
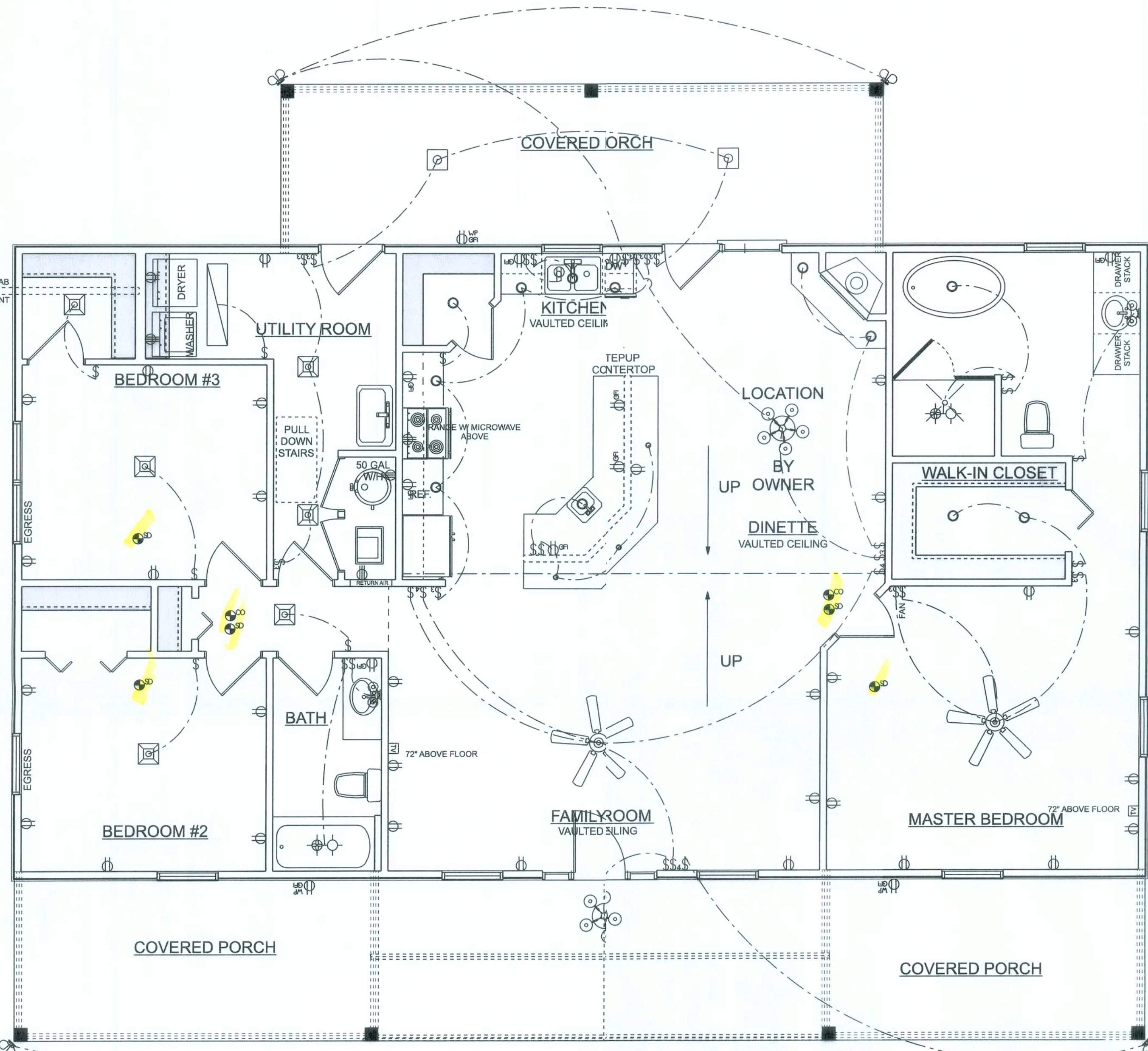
Erkinger Construction Group
Jason G Oliver
Residence

ADDRESS:
1379 SWCR 240
Lake City FL 32024
PARCEL ID#:
09-55-17-0171-001
Columbia County, FL

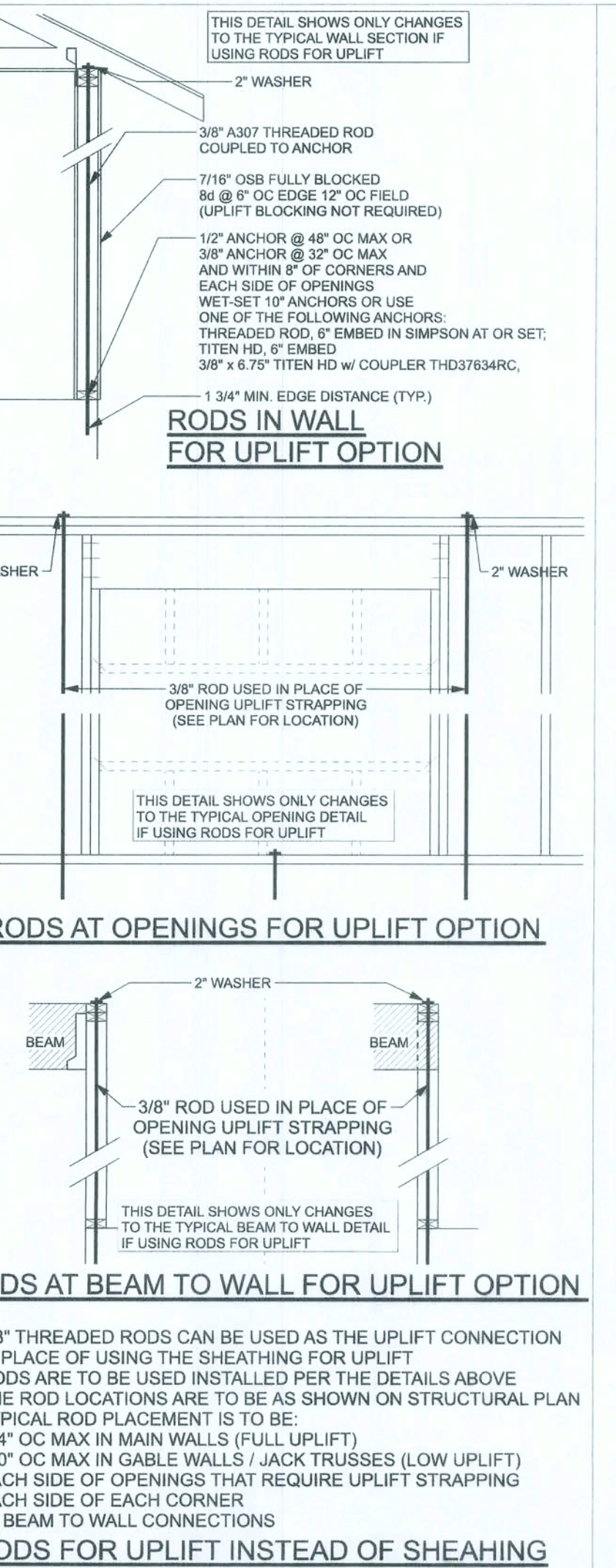
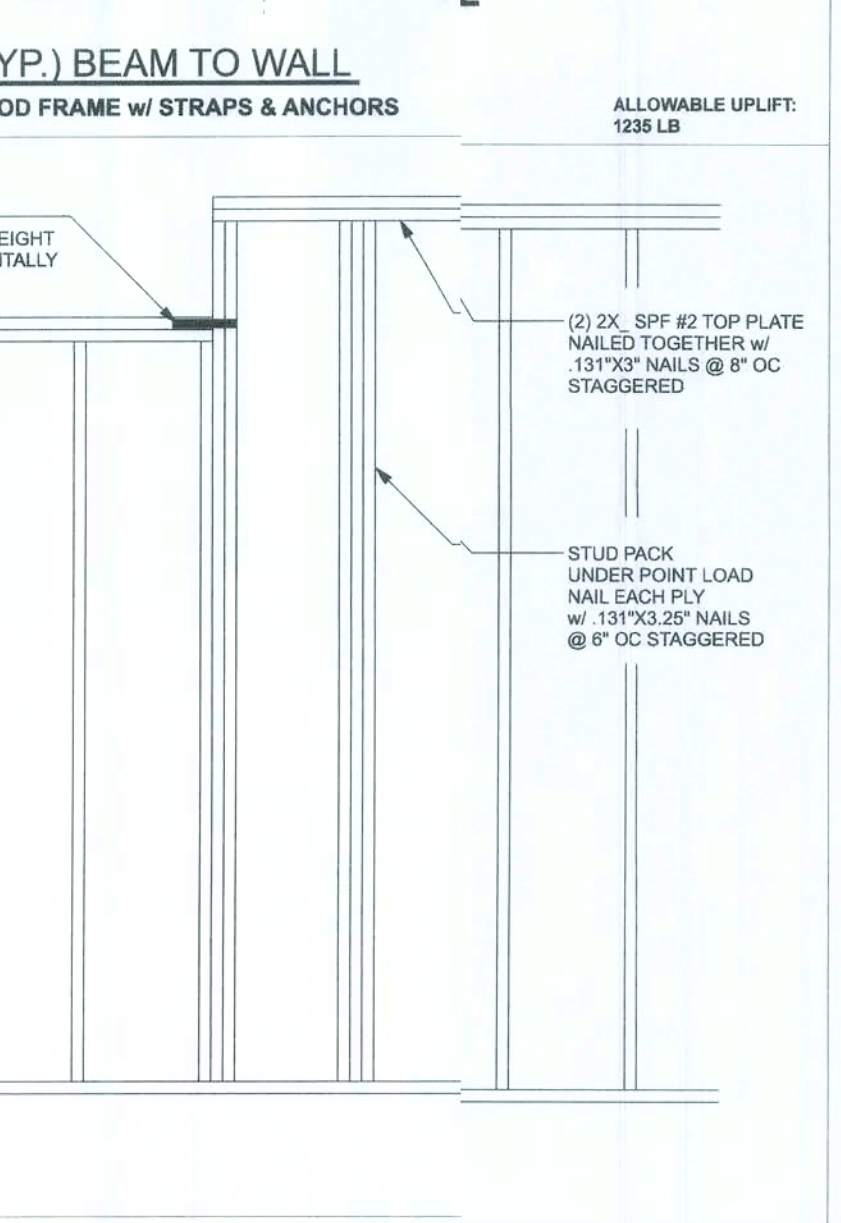
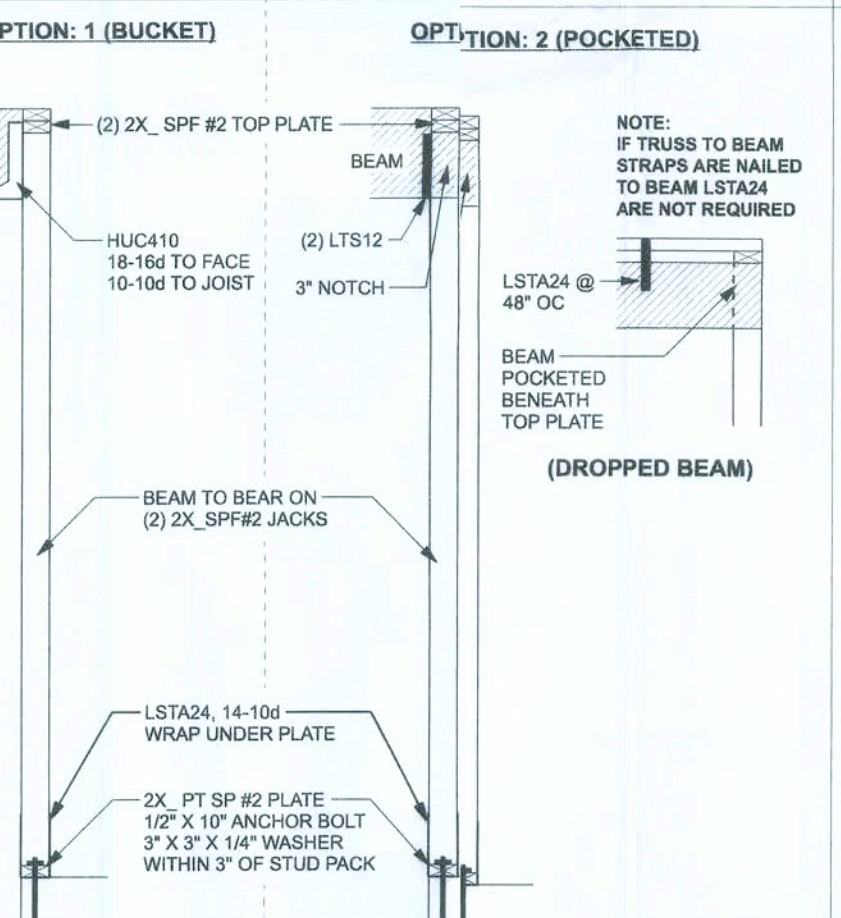
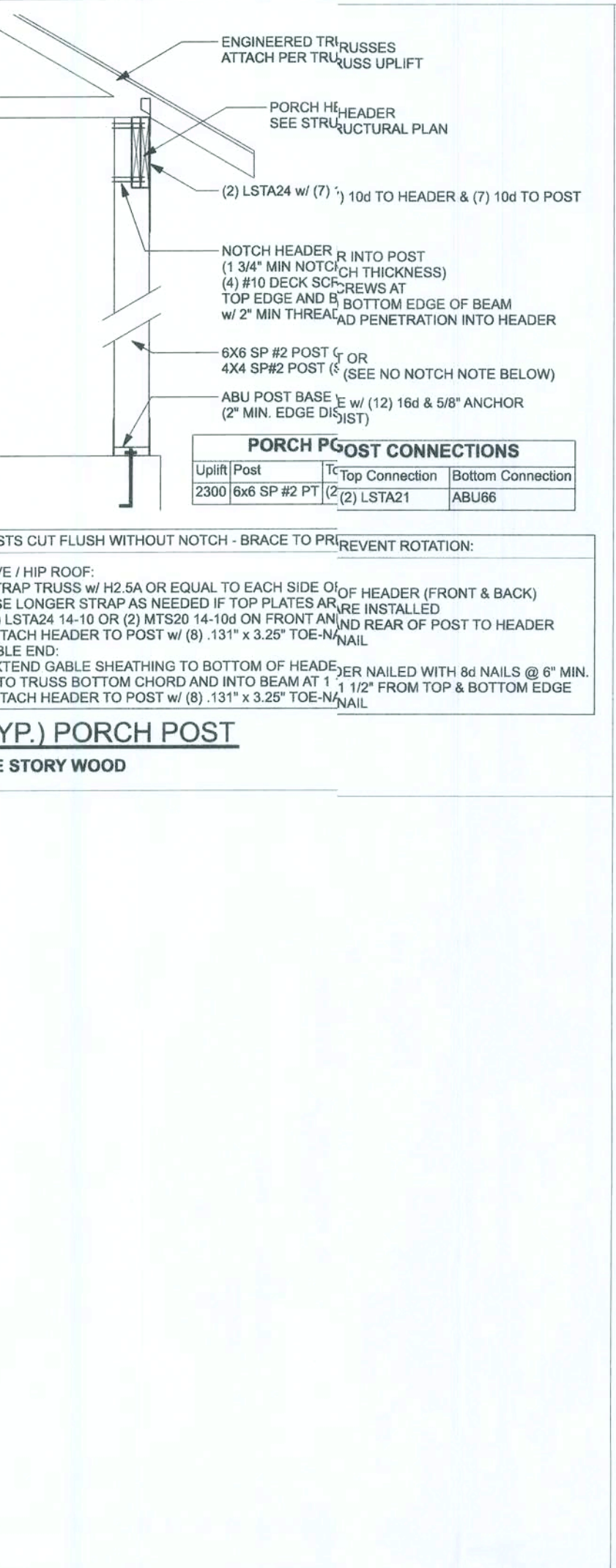
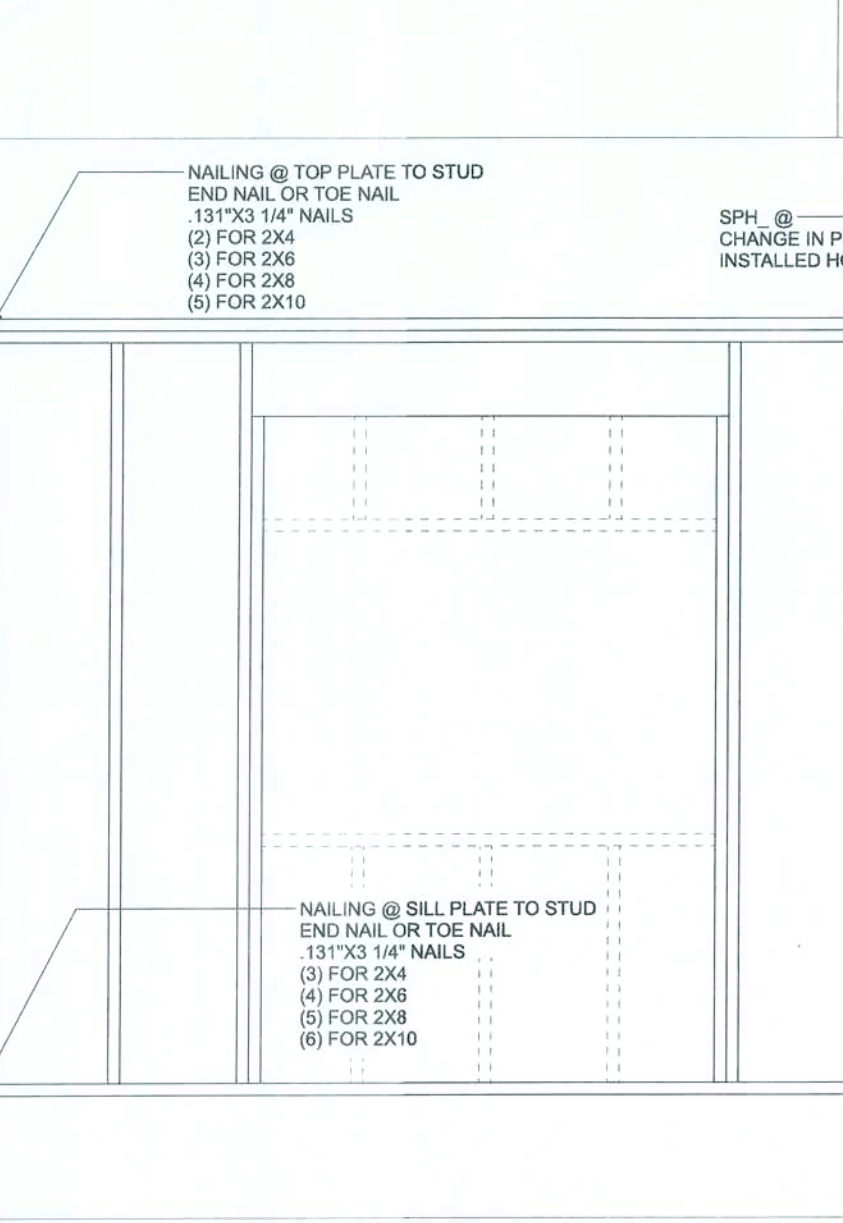
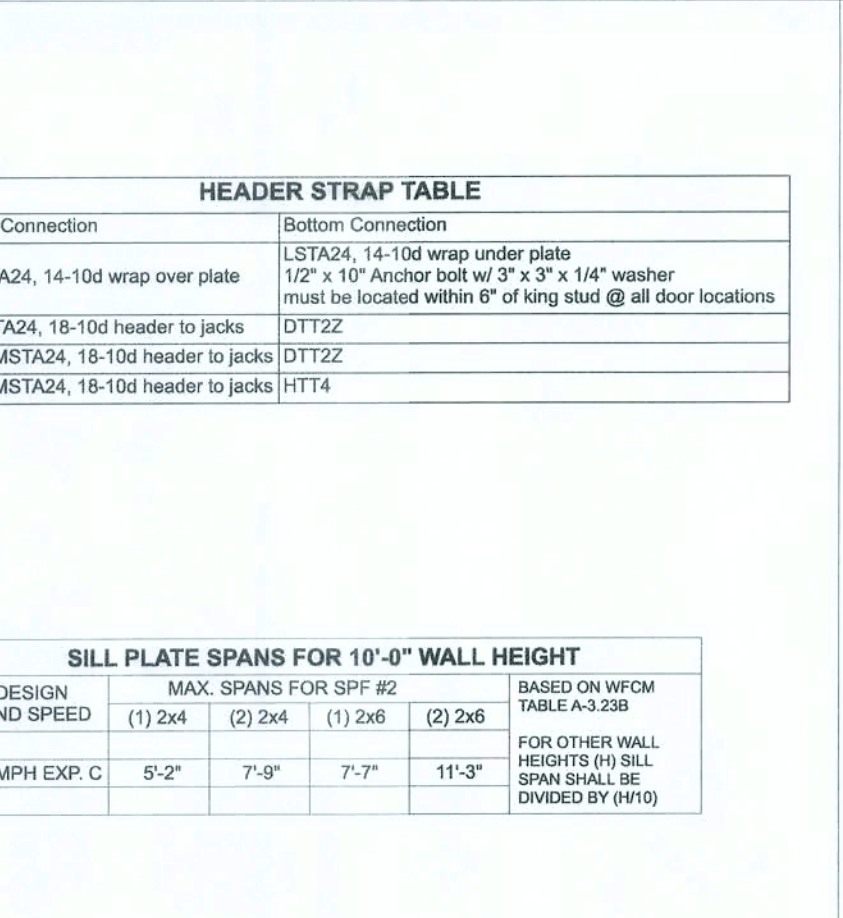
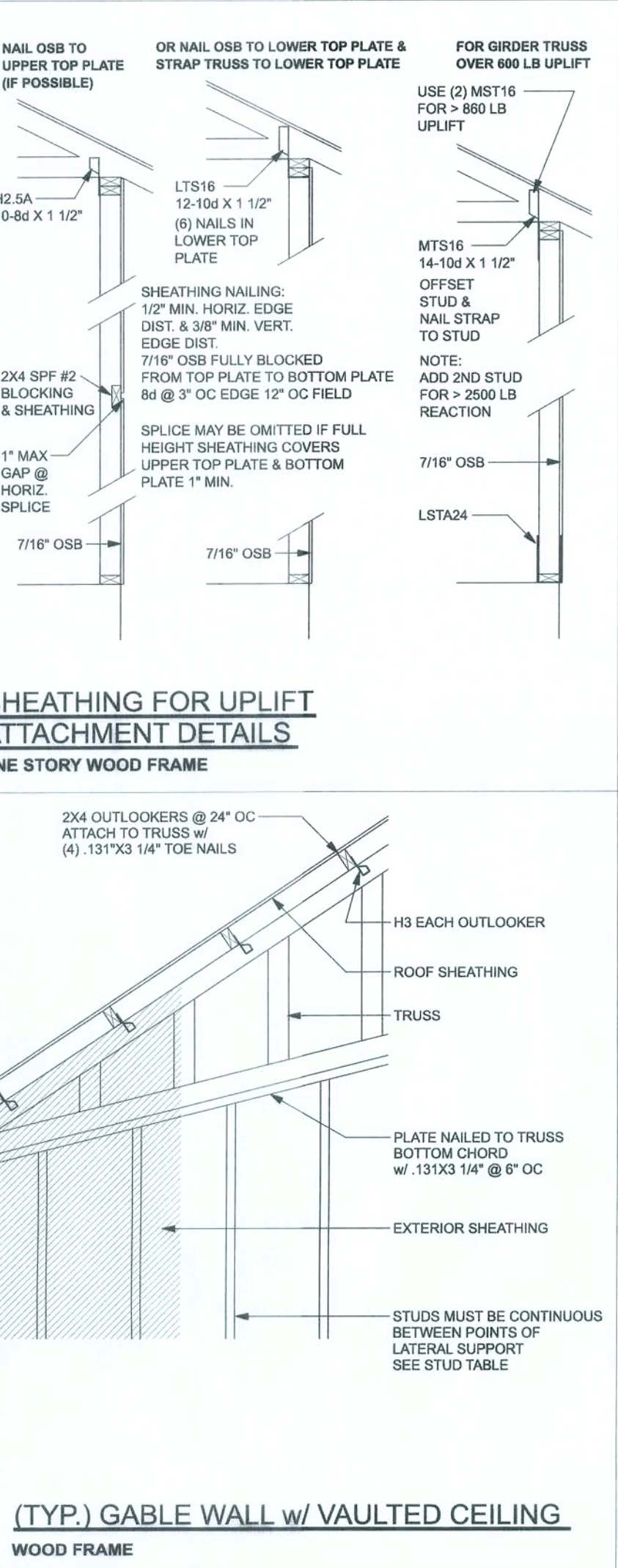
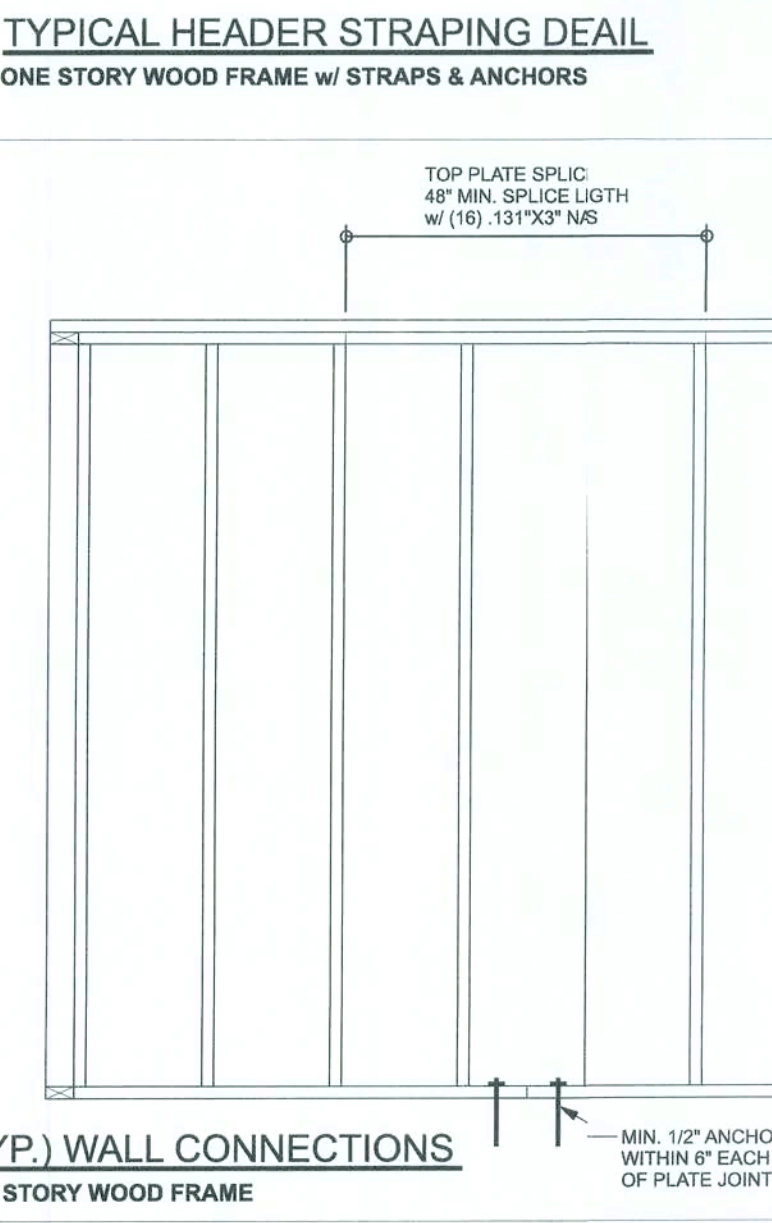
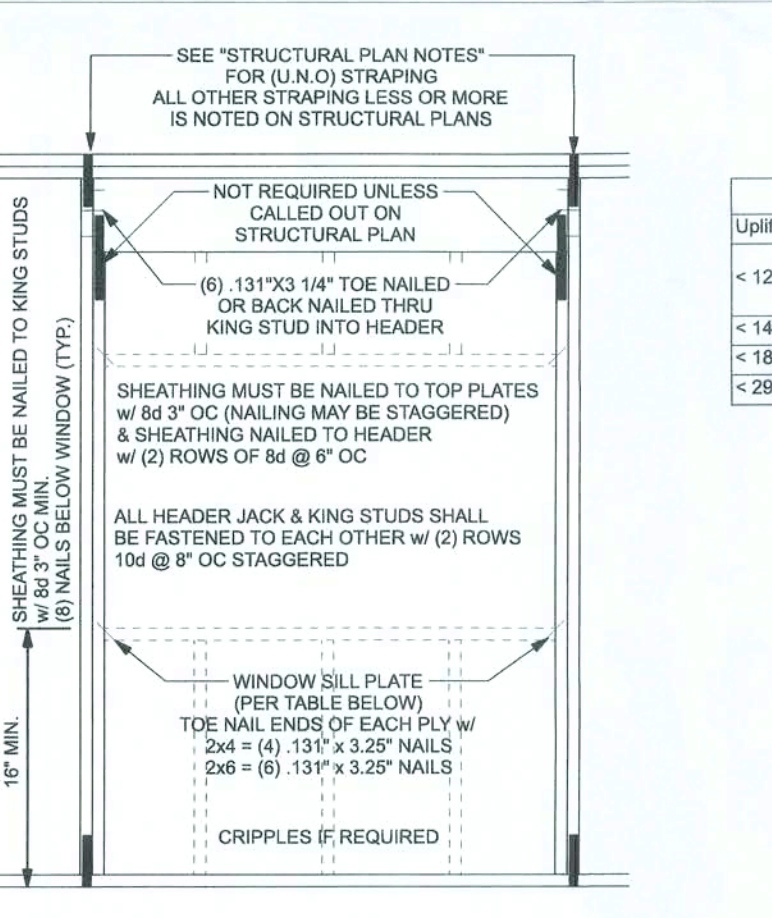
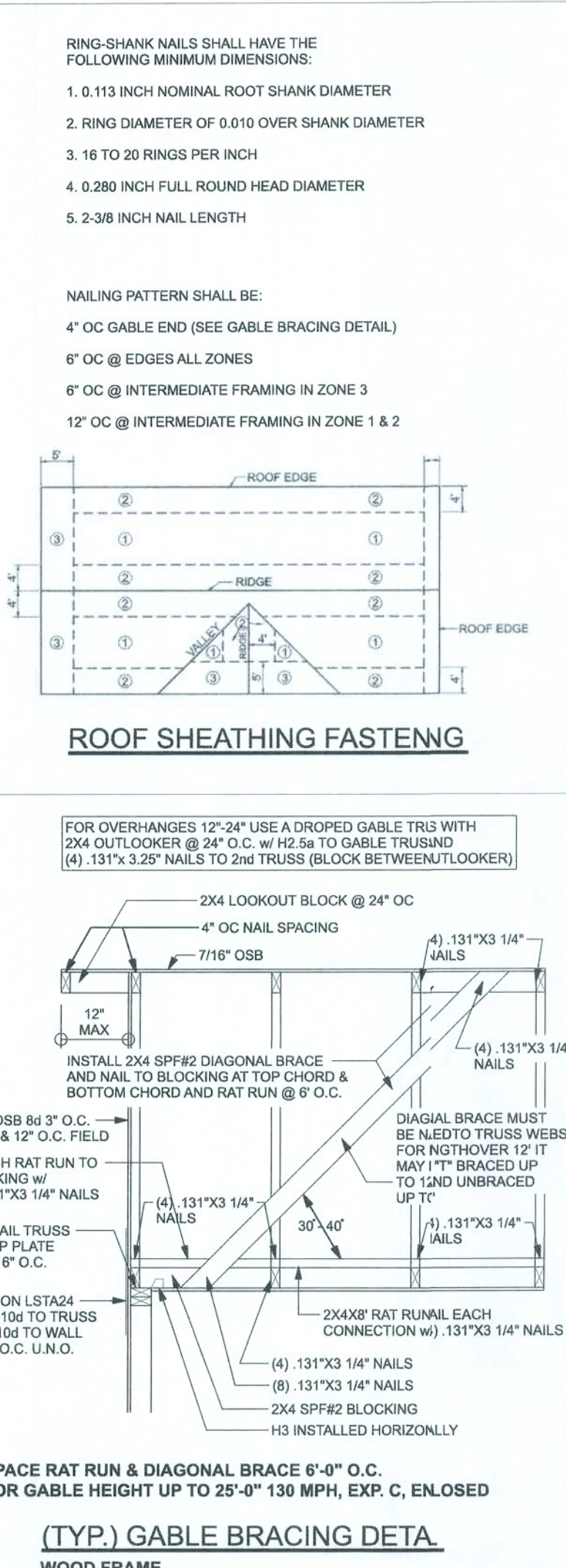
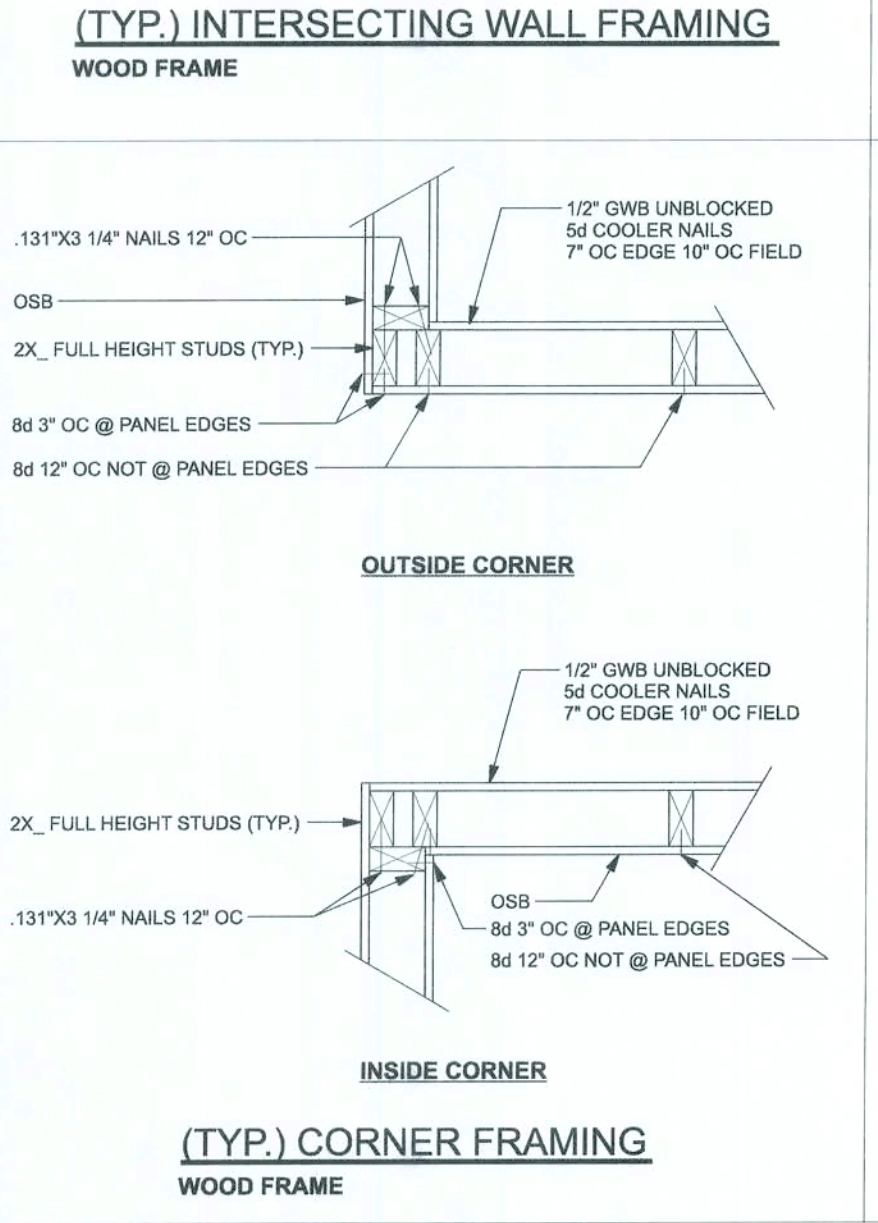
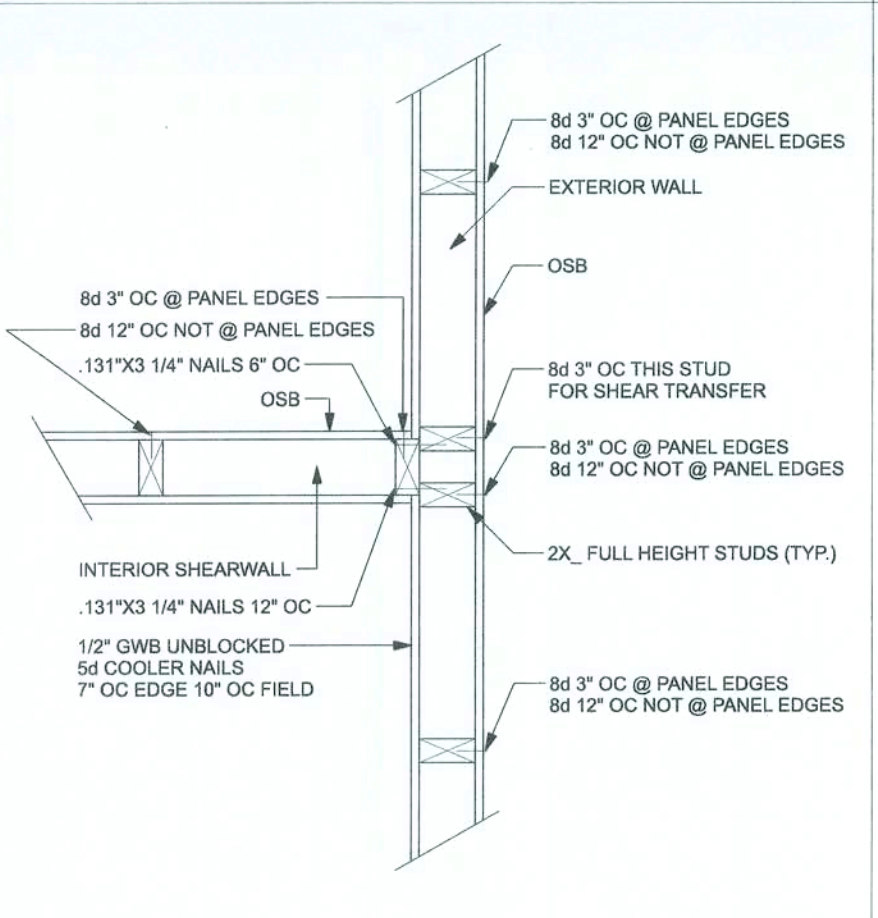
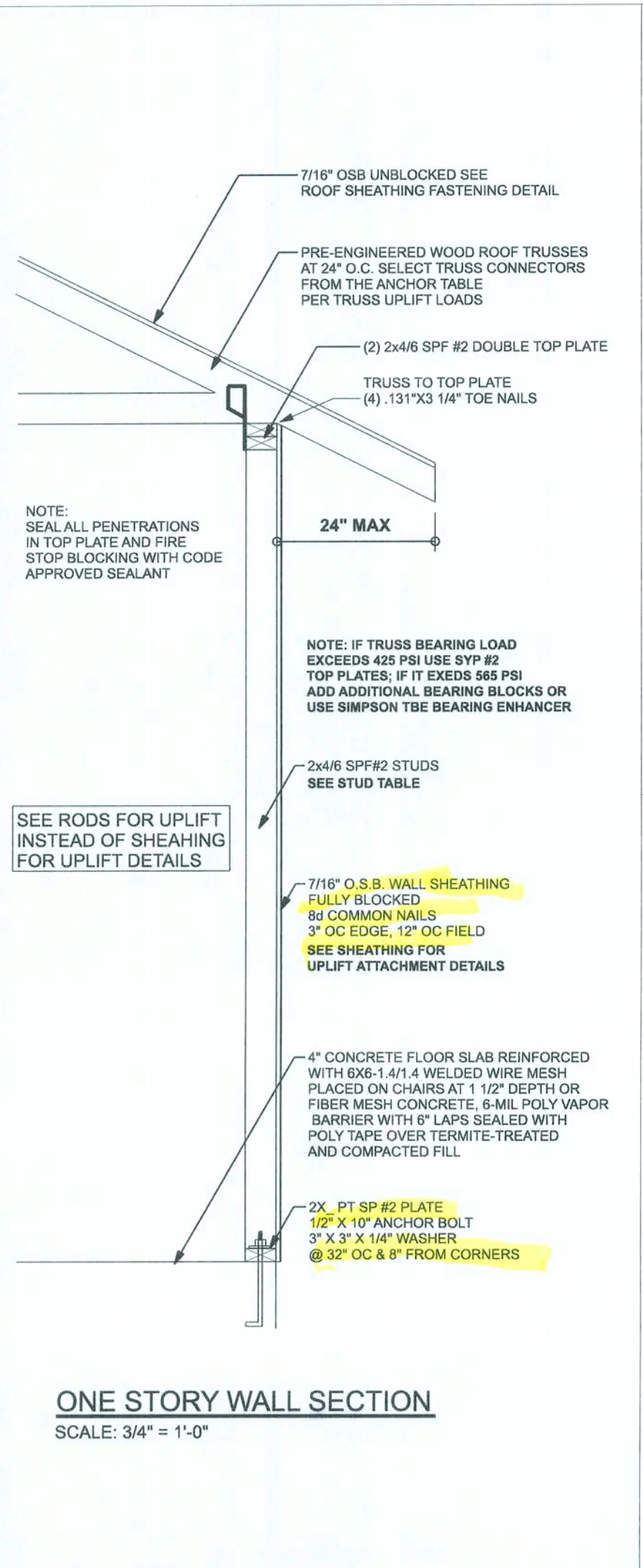
PRINTED DATE:
Wednesday, December 23, 2020

DRAWN BY:
Matthew A. Erkinger Sr.

DRAWING NUMBER
#1
OF 2 SHEETS



AREA SUMMARY	
LIVING AREA	1,758 S. F.
PORCH AREA	678 S. F.
TOTAL AREA	2,436 S. F.



CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8x1 1/2"	4-8x1 1/2"
575	485	H2.5A	5-8x1 1/2"	5-8x1 1/2"
1340	1015	H10A	9-10x1 1/2"	9-10x1 1/2"
720	620	LTS12-20	6-10x1 1/2"	6-10x1 1/2"
1000	860	MTS12-30	7-10x1 1/2"	7-10x1 1/2"
1430	1245	HTS20-30	12-10x1 1/2"	12-10x1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	655	SP2	9-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
1625	1600	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy

GRADE & SPECIES TABLE			
	SP #2	Fb	E
2x6	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PAL	PARALAM	2900	2.0

DESIGN CRITERIA & LOADS:		
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)	
CODE FOR DESIGN LOADS	ASCE 7-10	
WINDLOADS		
BASIC WIND SPEED (ASCE 7-10, SS GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1	
RISK CATEGORY	II	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7.45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
FLOOR LOADING		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY		
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE	
COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)		
EFFECTIVE WIND AREA (Ft2)	ZONE 4 INTERIOR	ZONE 5 END 4 FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.8(Vult) -46.2(Vult)	+42.8(Vult) -57.0(Vult)

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCL. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S TRUSS MANUFACTURER AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, Fy = 60 KSI, WELDED WIRE REINFORCEMENT FABRIC (WWM) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 5'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. TRUSSES TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 60 KSI, ALL LAP SPLICES 40" DB (20" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCL REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCL, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCL REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MARK DISCOWAY P.E.
163 SW Mdtown Place
Suite 103
Lake City, Florida 32025
386.54.5419
disoswaydesign@gmail.com

MARK DISCOWAY P.E.
163 SW Mdtown Place
Suite 103
Lake City, Florida 32025
386.54.5419
disoswaydesign@gmail.com

ERKINGER CONSTRUCTION GROUP

Mark Discoway P.E.
Residence

PROJECT ADDRESS:
1379 SW CR 240, Lake City, FL 32024
PARCEL: 08-08-17-001
Colman County, FL

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway P.E. or resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY P.E. 53915

Mark Discoway P.E.
163 SW Mdtown Place
Suite 103
Lake City, Florida 32025
386.54.5419
disoswaydesign@gmail.com

JOB NUMBER:
201391

S-1
OF 25 SHEETS



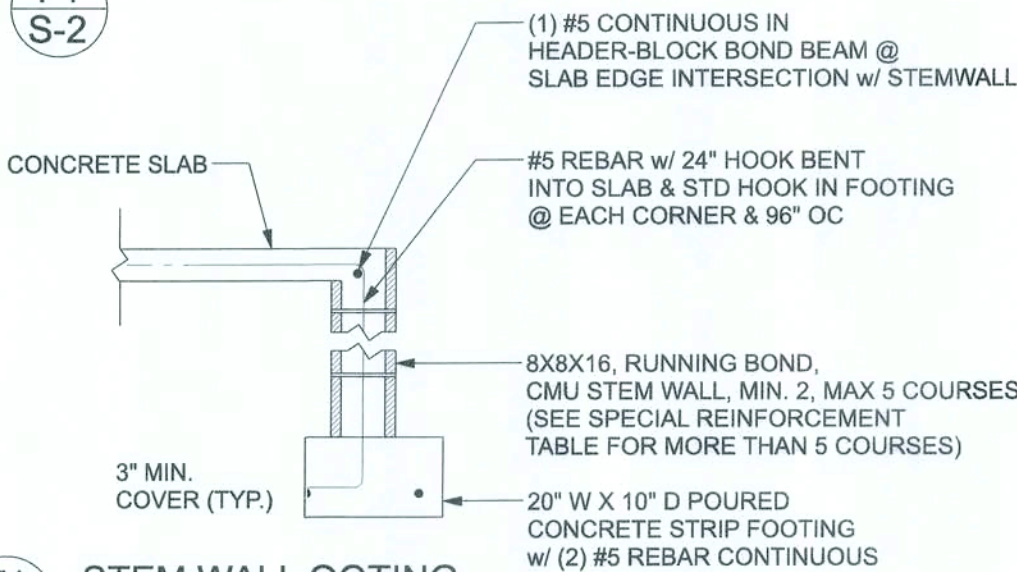
SCALE: 1/4" = 1'-0"

SCALE: 1/4" = 1'-0"

FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, TERRACES, ETC. CONTRACTOR SHALL BE RESPONSIBLE FOR MARK DISOWAY. PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN - 3	THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ Ø6X-14 WELDED BARRIER MESH PLACED ON CHAIRS @ 1" DEPTH OR FIBER REINFORCED CONCRETE 6-MIL POLY VAPO BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER THERMITE TREATED & COMPACTED FILL.

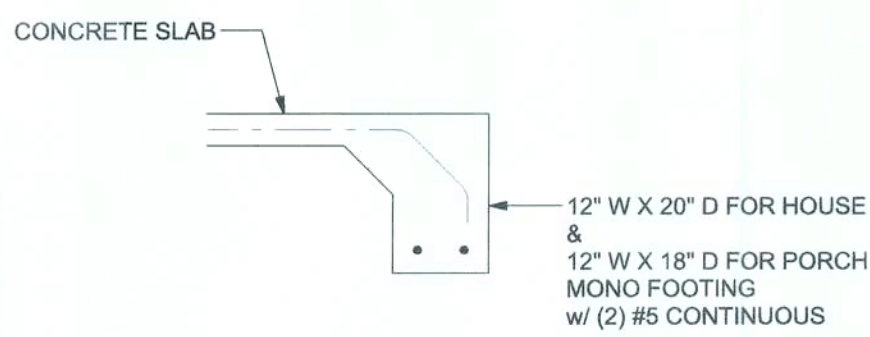
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

1	ACI308.2-02 Section 4.14A	Compressive strength	8" block bearing walls $f'_m = 1500$ psi
2	CMU	ASTM C 292, Type N	ASTM C 292, Type N
2.1	Corner	ASTM C 476, admittances require approval	
2.2	ASTM standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block	
2.3	Clay brick, brick standard	ASTM C 216-02, Grade SW, Type FBS, S-15 12"x12"x12"	
2.4	Reinforcing bars, #3 - #11	A575-61, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25' for #5)	
2.5	Coating for corrosion protection	Anchors, steel mesh for rebar completely embedded in mortar or grout, ASTM A525, Class 900, 0.60 cp/rft or 304SS	
2.6	Coating for corrosion protection	Joint reinforcement in walls exposed to weathering, steel wire, anchors, steel mesh not completely embedded in mortar or grout, ASTM A193, Class 82, 1.50 cp/rft or 304SS	
3.1	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.	
3.2	Movement joints	Contractor assumes responsibility for type and location of movement joints. If not detailed on project drawings.	


$$\begin{array}{r} -2 \end{array}$$

SCALE: 1/2" = 1'-0"

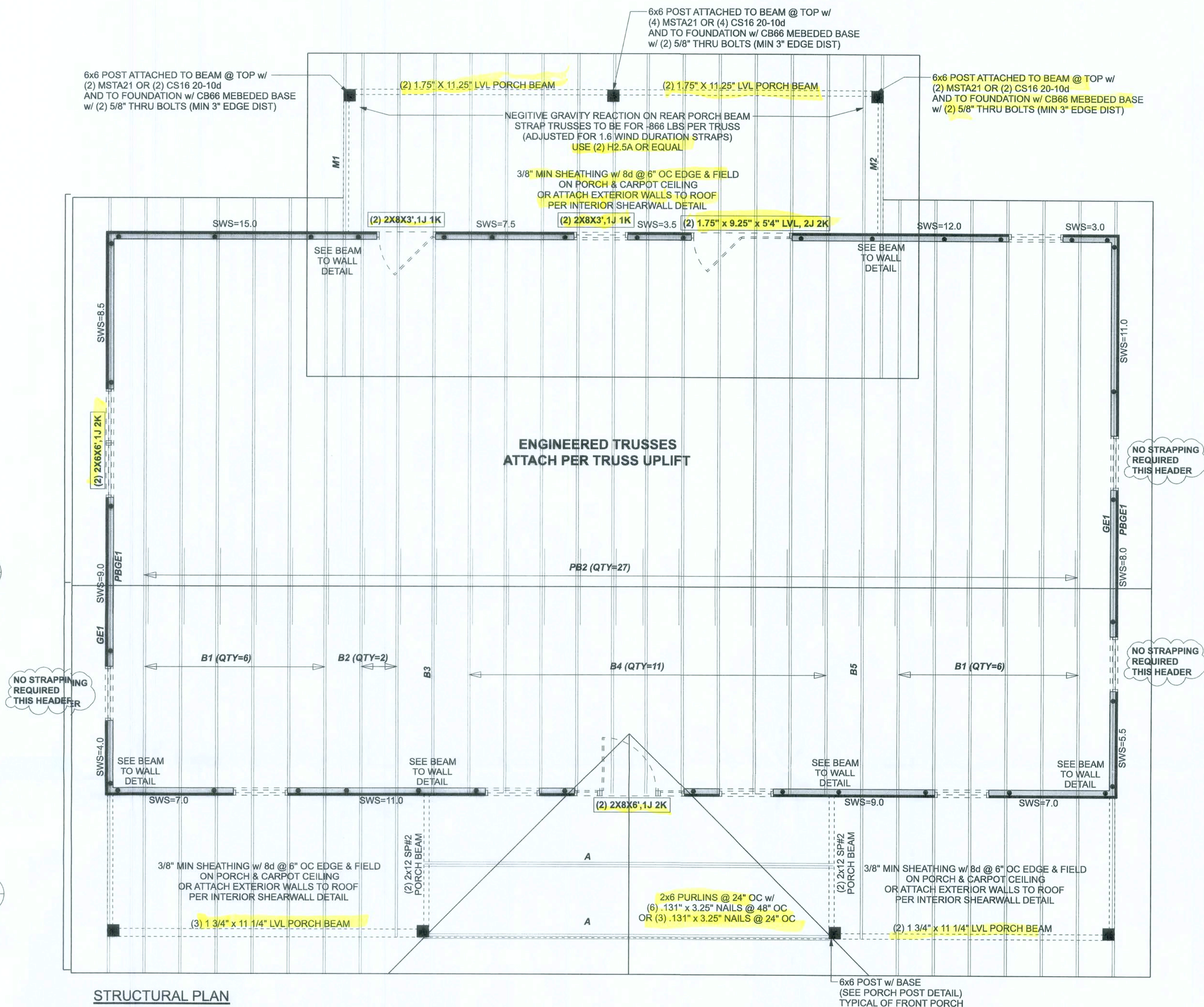
TALL STEM WALL TABLE:								
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & 1 rebar with 6" hook in the footing and bent 24" into the reinforced slab at top. The theoretical steel is to be placed toward the tension side of the CMU wall (away from the spigot, within 2" of the exterior side of the wall). If the wall is over 8' high, two Durcrete layer reinforcement is required vertically or a horizontal bond beam with 1#5 continuous and height. For higher parts of the wall 12" CMU may be used with reinforcement as show in the table below.								
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	96	80	96	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	



S-2

S-2 SCALE: 1/2" = 1'-0"

**BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF
12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL
PER FBC 2017-RES. SECTION R403.1.4**



SCALE: 1/4" = 1'-0"

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)

- | | |
|------|--|
| SN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.) |
| SN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) |
| SN-3 | ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL
WRAP UNDER BOTTOM PLATE & OVER TOP PLATE
12" x 10" ANCHOR BOLT w/ 3" x 3" x 1/2" WASHER
MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
(OR SEE RED HOLD DOWN OPTIONS) |
| SN-4 | USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD |
| SN-5 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS |
| SN-6 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSCI-03, BCSCI-B1, PCSCI-B2, & BCSCI-B3, BCSCI-B1, BCSCI-B2, & BCSCI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS BRACKET |

● = OPTIONAL ROD PLACEMENT
SEE ROD STRAPPING OPTION DETAIL ON S-1

(2) 2X10X0', 1J 1K

HEADER/BEAM CALL-OUT (U.N.O.)

- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	18216 LBF	29700 LBF
REQUIRED	17705 LBF	14226 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. SEMINOLE TRUSSES, INC
JOB # B52399A 12/14/2020

ERKINGER
CONSTRUCTION GROUP

Jason G. Oliver
Residence

PROJECT ADDRESS:
1379 SW CR 240, Lake City, FL 32024
PARCEL ID#: 09-5S-17-09171-001
Columbia County, FL

DIMENSIONS:
Stated dimensions suprcede scales
dimensions. Refer all gestions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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CERTIFICATION: I herby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSVAY P.E. 53915

MARK DISOSVAY P.E. 53915



Mark Disosway P.E.
163 SW Miltown Place
Suite 103
Lake City, Florida 32025
386.714.5419
disoswaydesign@gmail.com

JOB NUMBER:
201391

S-2
OF 2 SHEETS

OF 2 SHEETS