



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2125064 - GIEBEIG - FEAGLE RES.

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Giebeig Const. Project Name: Feagle Res. Model: Custom

Subdivision: N/A

Lot/Block: N/A Address: 277 SE Rodney Dicks Dr., N/A

State: FL City: Columbia Cty

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

State: City:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

Loading Conditions):

Design Code: FBC2017/TPI2014

Wind Code: ASCE 7-10

Roof Load: 37.0 psf

Design Program: MiTek 20/20 8.2

Wind Speed: 130 mph Floor Load: N/A psf

This package includes 1 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Seal# No

Truss Name Date

T19136700 T14R

1/14/20

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Lee, Julius

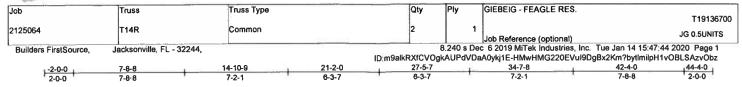
My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

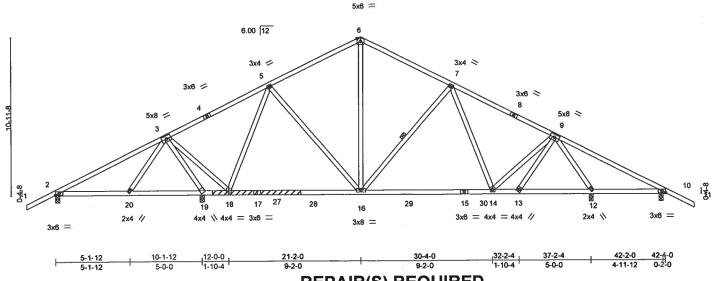


Julius Lee PE No.34869 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 14,2020



Scale = 1:76.8



REPAIR(S) REQUIRED

(See Notes Below)

"OADING (psf) FCLL 20.0 FCDL 7.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25	CSI. TC 0.58 BC 0.79	Vert(LL) 0.06 12-26	l/defi L/d >935 240 >999 180	PLATES GRIP MT20 244/190
BCLL 0.0 *	Rep Stress Incr YES Code FBC2017/TPI2014	WB 0.90 Matrix-MS	Horz(CT) 0.03 12	n/a n/a	Weight: 255 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS

2x4 SP No.3

BRACING-TOP CHORD

WEBS

BOT CHORD

Structural wood sheathing directly applied or 5-5-11 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 18-19,10-12. 1 Row at midpt

REACTIONS. All bearings 0-3-8.

Max Horz 2=-157(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) except 2=-153(LC 8), 19=-310(LC 12),

12=-266(LC 13), 10=-121(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 10 except 2=505(LC 23), 19=1347(LC 1), 12=1310(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-477/436, 3-5=-451/254, 5-6=-777/591, 6-7=-777/592, 7-9=-969/621, 9-10=-50/370

2-20=-193/378, 19-20=-91/295, 18-19=-669/524, 16-18=-61/607, 14-16=-200/807, **BOT CHORD**

13-14=-177/514, 12-13=-216/523, 10-12=-255/220 3-20=-293/272, 3-19=-1599/992, 3-18=-481/1298, 5-18=-715/462, 6-16=-269/395,

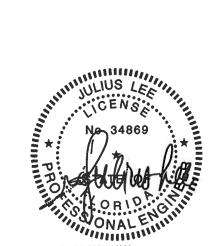
7-16=-346/354, 9-14=-14/514, 9-12=-1365/714

NOTES-

WEBS

1) Repair Condition: Missing or damaged plate(s) on both side(s) of truss at joint(s) 17.

- 2) Apply 72" long 2x4 SP No.2 scab to front side(s) of truss centered on damage at joint 17 with 2 row(s) of 10d (0.131"x3") nails spaced 2" o.c. from front face. Minimum 0-3-0 end distance.
- 3) N/A
- 4) Unbalanced roof live loads have been considered for this design.
 5) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 2, 310 lb uplift at joint 19, 266 lb uplift at joint 12 and 121 lb uplift at joint 10.



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A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

