# GENERAL NOTES

- 1. CODES USED: 2001 FLORIDA BUILDING CODE, ACI, NDS, APA AND ASCE-7-98. ALL LATEST EDITIONS USED.
- 2. ALL DESIGN, CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH APPLICABLE CODES AND AUTHORITIES HAVING JURISDICTION OVER THE WORK.
- 3. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS AT THE JOB SITE PRIOR TO COMMENCING CONSTRUCTION.
- 4. DETAILS FOUND WITHIN THESE DRAWINGS SHALL BE ASSUMED TO BE TYPICAL DETAILS FOR THIS JOB ONLY. DETAILS SHALL GOVERN CONSTRUCTION FOR THIS JOB UNLESS NOTED OTHERWISE ON THE PLANS.
- 5. SUBSURFACE SOIL CONDITIONS WERE NOT AVAILABLE AT THE TIME OF THIS DESIGN. THE OWNER SHALL PROVIDE TO THE CONTRACTOR A REPORT OF THE SUBSURFACE CONDITIONS. SOIL PREPARATIONS NOTED IN SAID REPORT SHALL BE FOLLOWED UNLESS MORE STRINGENT DESIGN IS SPECIFIED WITHIN THESE PLANS.

DESIGN CODE: The 2001 Florida Building Code (FBC)

OCCUPANCY: Residentail Group R-3 (One- and Two-Family Dwellings)

CONSTRUCTION: Type VI, Unprotected

BASIC WIND SPEED: 110 mph

WIND IMPORTANCE FACTOR: 1.0 (Category II Building)

WIND EXPOSURE: B (Greater than 1500 ft from CCL or mean high tide)

INTERNAL PRESSURE COEFFICIENT: ± 0.18 (Enclosed Building - Not in the Wind-Borne Debris Region)

MAXIMUM DESIGN WIND PRESSURES (psf):

Opening Size (sf)	Edge zones (3'-0" from outside corners)	Interior Zone
0-20	+26 and (-)35	+26 and (-)28
21-50	+25 and $(-)32$	+25 and $(-)27$
51-100	+23 and $(-)29$	+23 and $(-)25$
>100	+22 and $(-)27$	+22 and $(-)24$

DESIGN LIVE LOADS:

Roof: 20 psf Attics with Storage: 30 psf Attics without Storage: 10 psf

Floors/Stairs/BALCONY: 40 psf

CAST-IN-PLACE CONCRETE: shall have a compressive strength of 2500 psi at 28 days.

CONCRETE MASONRY UNITS: shall be hollow unit masonry in accordance with ASTM C 90 and shall have a minimum

net area compressive strength of 1900 psi when using Type M or S mortar (ASTM C 270).

In accordance with ACI 530, the 1900 psi block in combination with Type M or S mortar provides a design compressive

strength (f'm) of 1500 psi.

GROUT: The grout shall be in accordance with ASTM C 476 and shall have a maximum course aggregate size of 3/8 " placed at an 8" to 11" slump and have a minimum specified compressive strength of 2000 psi at 28 days when tested in accordance with ASTM C 1019.

REINFORCING STEEL: shall be ASTM A615, Grade 40

STRUCTURAL STEEL: shall be ASTM A36.

STEEL PIPE AND STRUCTURAL TUBING: shall be ASTM A500 (Grade B)

WELDED WIRE FABRIC (WWF): shall be ASTM A185.

ANCHOR BOLTS AND THREADED RODS: shall be in accordance with ASTM A 307 or ASTM F 1554 Grade 36.

WASHERS: shall be in accordance with ASTM F 436 Grade 36.

NUTS: shall be in accordance with ASTM A 563 Grade A Hex.

ANCHORING ADHESIVE: shall be one of the following products (DUAL CARTRIDGE INSTALLATION ONLY):

- \* Simpson Strong-tie Co., Product: Epoxy-Tie SET
- \* Simpson Strong-tie Co., Product: Acrylic-Tie AT

METAL CONNECTORS: All metal connectors which are exposed to exterior shall be galvanized, Z-MAX or stainless steel. All metal connectors which are exposed to pressure treated wood shall have a Z-max finish.

Roof Assemblies shall be in accordance with FBC chapter 15.

Masonry construction shall be in accordance with FBC chapter 21 and in accordance with the Specifications for Masonry Structures ACI 530.1—99. ACI 3.5 D limits the grout lift height to 5 ft and requires a 1—hour initial set time between lifts.

Steel Construction shall be in accordance with FBC chapter 22.

Wood framing shall be in accordance with FBC chapter 23 except as noted in these plans.

The "Fastening Schedule" in Table 2306.1 shall be used U.N.O. in these plans and as follows: The double top plate shall provide a 4 ft lap and shall be face—nailed with (8) 12d sinkers.

Footings and foundations shall be in accordance with FBC chapter 18. Soil compaction shall be in accordance with 1804.2 Typical anchor bolts are required at the following locations:

Spaced @ 6' o.c. at the exterior wall sill plate where the thru-bolt spacing is greater than 6' o.c.

At the exterior wall sill plates where the thru-bolt is more than 12" from the end of the plate, or at 36" o.c. where thru rods are not used. Anchor bolts are also required at the ends of stemwall at porches. Anchor bolts are not required at interior bearing wall sill plates. The nut at the double top plate shall be receive a final tightening after the roof system is installed.

Use 2x4 AND 2X6 studs for exterior walls WHERE NOTED and 2x4 studs for all interior walls U.N.O. in these plans. Space studs @ 16" o.c at all exterior walls, interior bearing walls and interior shear walls. Space studs @ 24" o.c. at interior non-bearing walls.

Use SPF stud grade (or better) for all walls U.N.O. in these plans. Use SYP #2 top plates and PT SYP #2 sill plates.

In general the thru-bolts serve as the continuous load path from the double top plate to the foundation.

Where "CONVENTIONAL STRAPPING" is shown in these plans, use:

Simpson SP2 w/(6) 10d nails each end for wall stud to top plate connections @ 32" O.C. Simpson CS20 strapping w/(7) 10dx1.5" nails each end for second story wall stud connections @ 32" O.C.

Simpson SP1 w/ (6) 10d nails to stud and (4) 10d nails to sill plate @ 16" O.C. and anchor bolts at 32" O.C. All conventional framing shall be SYP #2. Refer to truss shop drawings for truss details. Provide 1x4 purlin bracing above the bottom chord of roof trusses at the spacing indicated in the shop drawings. Fasten w/ (2) 6d nails at each truss. Provide solid blocking at roof sheathing joints in the first two rafter or truss spaces from gable ends.

Attach headers to posts w/(2) Simpson CS20 flat straps w/(7) 10dx1.5" nails to header and (7) 10dx1.5" nails to post. Notch headers under top plates and bear on jack studs or use Simpson HUC410 hanger w/(8) 16d to wall (8) 16d to header. Anchor posts to foundation w/(8) Simpson ABU post base w/(1) 5/8" threaded rod set 7" into footing and (12) 16d nails to post or use (1) Simpson HTT16 w/(1) 5/8" threaded rod set 7" into footing (18) 16d nails to post.

EXTERIOR WALL & INTERIOR SHEARWALL SHEATHING:

GENERAL AND RATED SHEATHING:

Fasten 16 thick sheathing w/8d common nails on 6"0.C. & supported edges & 12"0.C. & intermediate supports. Reduce nail spacing to 3"0.C. around all openings. Use 3" edge nailing in the lower top plate on all exterior walls.

SW-3 indicates 3" EDGE NAILING is also required at the bottom and side edges of the sheathing.

SW-2 indicates 2" EDGE NAILING is also required at the bottom and side edges of the sheathing.

Where stucco, brick veneer, or hardiplank lap siding, or interior shear wall indicated, underlay with 7/16 "Rated sheathing fastened w/8d nails. Unless noted as above, space nails at 6" o.c. edges, and 12" o.c. intermediate.

ROOF SHEATHING AND DIAPHRAGM ATTACHMENT

Use 15/32" APA Rated sheathing fastened w/ 8d nails 6" o.c edges, 6" o.c. intermediate.

SUBFLOORS:

Where subflooring is indicated in these plans use 3/4 "T&G plywood glued with a construction adhesive and fastened per Table 2306.1. FOUNDATION AND SLAB NOTES

- 1. 6X6 W1.4 X W1.4 WWF TO BE PLACED IN THE CENTER OF THE SLAB. WWF SHALL BE LAPPED 8". THE USE OF FIBERMESH SHALL BE ALLOWED IN LIEU OF WWF. MINIMUM FIBER LENGTH = 1/2".
- 2. SLAB THICKNESS IS 4", UNLESS NOTED OTHERWISE ON THE PLANS. SLAB SHOULD BE POURED OVER A 6 MIL. VAPOR BARRIER AND THE SOIL SHOULD BE TREATED WITH TERMITE POISON PRIOR TO POURING.
- 3. THE FILL BELOW THE FOUNDATION SHOULD BE FREE OF DEBRIS, ORGANIC MATERIAL, COHESIVE SOILS OR ANY OTHER DELETRIOUS MATERIAL. SOIL MUST BE COMPACTED TO 95% MODIFIED PROCTOR MAXIMUM DRY DENSITY FOR TWO FEET BELOW THE BOTTOM OF THE FOOTING.
- 4. VERTICAL AND HORIZONTAL REINFORCEMENT WILL BE LAPPED FOR 36 BAR DIAMETERS OR 24", WHICHEVER IS GREATER.
- 5. CORNER REINFORCEMENT SHALL BE LAPPED 30".
- 6. REINFORCEMENT SHALL HAVE THE FOLLOWING COVER REQUIREMENTS;
- 3" FOR CONCRETE CAST AND PERMANENTLY EXPOSED TO EARTH
- 2" FOR CONCRETE EXPOSED TO EARTH AND WEATHER

  1 1/2" FOR CONCRETE NOT EXPOSED TO WEATHER OR E
- 1 1/2" FOR CONCRETE NOT EXPOSED TO WEATHER OR EARTH FOR THE PRIMARY REINFORCEMENT.
- 7. ONLY DIMENSIONS FOUND ON THE FOUNDATION PLAN BY THE ARCHITECT SHOULD BE USED FOR FOUNDATION CONSTRUCTION. IF DIMENSIONS CAN NOT BE DETERMINED FROM FOUNDATION PLAN, CONTACT THE ENGINEER OF RECORD.
- 8. STEMWALL TO BE A MAXIMUM OF (6) COURSES TALL. CONTACT ENGINEER OF RECORD IF STEMWALL WILL EXCEED 6 COURSES IN HEIGHT.
- 9. WHERE THREADED RODS ARE EMBEDDED 12" INTO STEMWALLS, THE TOP TWO COURSES OF STEMWALL MUST BE FILLED.

#### CONVENTIONAL FRAMING NOTES:

- 1. ALL CONVENTIONAL FRAMING SHALL BE SYP. NO. 2 OR BETTER.
- 2. ALL RAFTERS SHALL BE 2X6's @ 16" O.C. U.N.O.
- 3. FASTEN ALL RAFT. TO PLATES WITH SIMP. H8 CLIPS w/ (5) 10d X 1.5" NAILS EACH END.
- 4. FASTEN ALL RAFTERS AT RIDGES WITH 2X6 COLLAR TIES.
- 5. FACE-NAIL COLLAR TIES TO RAFTERS WITH (5) .131 X 3" NAILS.
- 6. FASTEN ALL JACK RAFT. TO HIPS AND VALLEYS w/(4) .131 X 3" TOE-NAILS.
- 7. ALL RIDGES, HIPS AND VALLEYS SHALL BE ONE (1) NOM. SIZE LARGER THAN ADJOINING RAFTERS U.N.O.
- 8. FASTEN RAFTERS TO CEILING JOISTS WITH (4) .131 X 3" NAILS.
- 9. FASTEN RAFTERS AND CEILING JOISTS TO PLATES WITH (3) .131 X 3" TOE-NAILS.
- 10. FASTEN NAILERS/LEDGERS TO EA. STUD W/ (4) .131X3" NAILS.
- 11. ALL NAILERS SHOULD BE THE SAME SIZE OR (1) NOMIMNAL SIZE LARGER THAN ADJ. MEMBERS.
  12. FASTEN ALL 2X6 EXTERIOR STUDS TO PLATES W/ (6) 8D TOENAILS @ EA. END. TYP.

MAX. DESIGN WIND PRESSURE = -24.6 PSF AND +21.8 PSF FOR GARAGE DOOR



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### FLORIDA PRODUCT APPROVAL LIST

LEGEND:									
* Indicates bearing w	all below with stu	ds spaced at	16" o.c.						
** See Sheet DETAILS	for garage head	er tiedow n							
*** See Sheet DETAIL	S for gable end tr	uss tiedow n	and platform t	raming in atti	С				
Hurricane clips and	wist straps:								
H3 = (Prod. Code- FL	474.116) Simpson	H3 w / (4) 8d	nails each er	nd - Capacity	= 455 lb				
H8 = (Prod. Code- FL	1423.7) Simpson I	H8 w / (5) 10d	x1.5" nails ea	ch end - Cap	acity = 745 lb				
MTS = (Prod. Code- F									
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Flat straps:									
MSTA24 = (Prod. Cod	e- FL1901.57) Sir	npson MSTA2	24 w / (9) 10d	nails each er	nd - Capacity =	1640			
MSTA36 = (Prod. Cod					1 2				
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MSTC28 = (Prod. Cod							CONTOTOR		
MSTC40 = (Prod. Cod									
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Heavy girder tiedown	9								
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PHD2 = (Prod. Code-									) lle
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HTT16 = (Prod. Code-HTT22 = (Prod. Code-									
HDQ8 = (Prod. Code-									35 lb
THAI-2 = (Prod. Code	- FL474.414) SIMP	son THATZI	iverted neavy	girder hang	er w / (30) 100	nails into fac	e - Capacity	= 4135 lb	
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Face Mount Hangers									
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#### SYMBOL KEY

THRU—BOLT LOCATION

LTP4 = (Prod. Code- FL474.258) Simpson LTP4 Framing Anchor- Capacity = 670 lbs.

A35 = (Prod. Code- FL474.4) Simpson A35 Framing Anchor- Capacity = 450 lbs.

LTT20B = (Prod. Code- FL474.264) Simpson LTT20B Tension Tie w / 1/2" Anchor and (10)16d nails- Capacity = 1750lbs

\*\* CONVENTIONAL STRAPPING (SEE VERTICAL FRAMING ON SHT. S-1)

SW-3 SHEARWALL W/3" NAILING PATT.

SIMPSON HDQ8 HOLDDOWN LOCATION (SEE UPLIFT SCHEDULE)

// FASTEN (2) SIMPSON MSTA36 FLAT STRAPS

SIMPSON HTT22 HOLDDOWN

# TRUSS UPLIFT SCHEDULE:

USE MIN. (1) SIMPSON H8 EA. END OF EACH TRUSS TYP.

USE HOLDDOWNS WHERE SPECIFIED (SEE PLANS)

TAMARA G. BAKER, P.E.

SUITE 13 2207 837 TAMARA G. BAKER

BAKER ENGINEERING & CONSULTING, CERTIFICATE OF AUTHORIZATION # 262, 1628 SAN MARCO BLVD SUITE 13
JACKSONVILLE, FL 32207
PHONE: 904-398-9837

REMODELING/ADDITION TO EXISTING RESIDENCE FOR ED & DIANNE BISHOP WHITE LAKE CITY, FLORIDA

Check by:
TGB
Approved by:
TGB

GENERAL NOTE AND FLORIDA PRODUCT APPROVAL

REVISIONS

NO. DATE REVISED

05-98

S-1