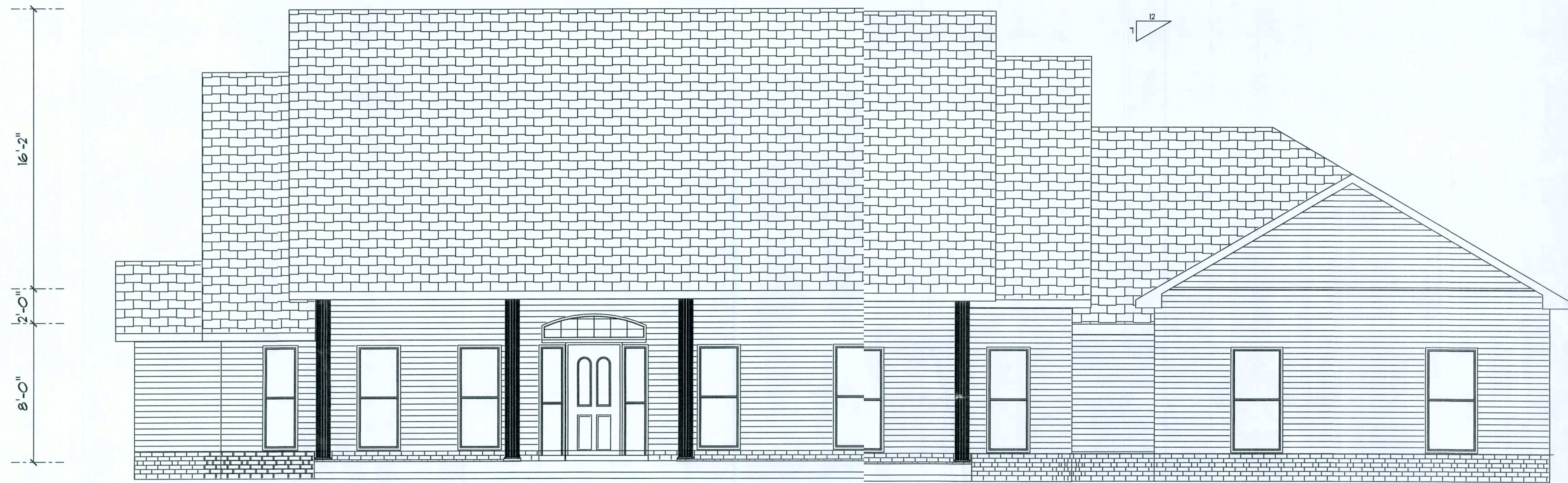


REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



FRONT ELEVATION

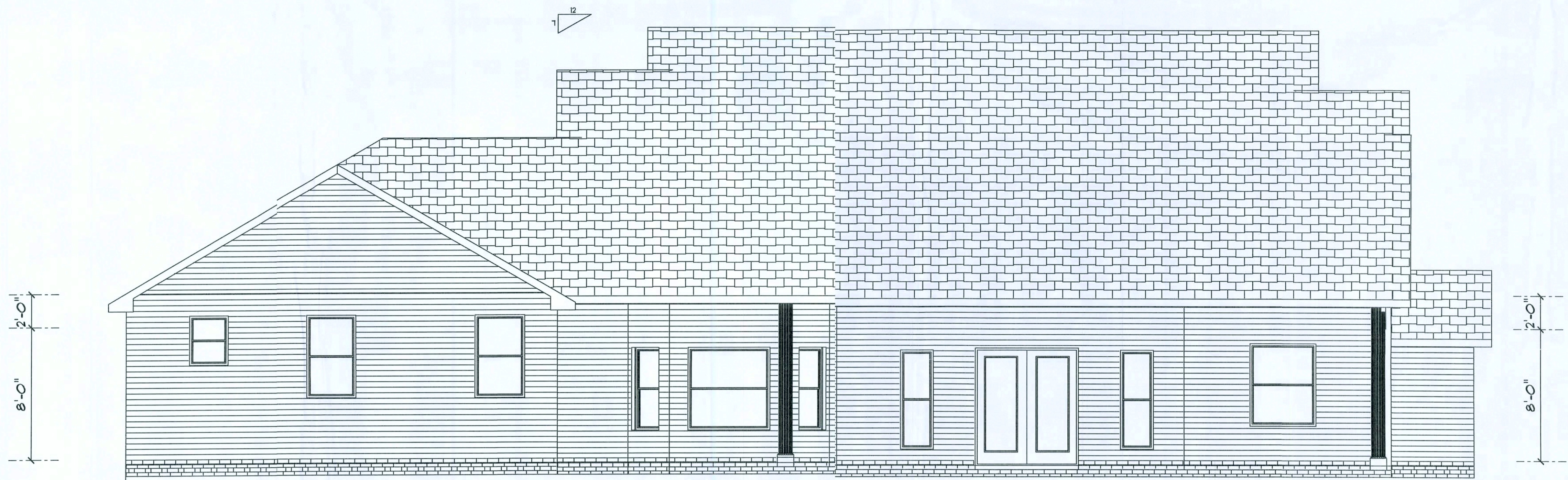
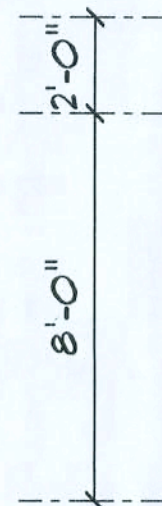
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
4187 S.F. / 300 x 50% = 6.9 S.F. RIDGE VENT AREA REQUIRED
62.7 FEET OF RIDGE VENT REQUIRED

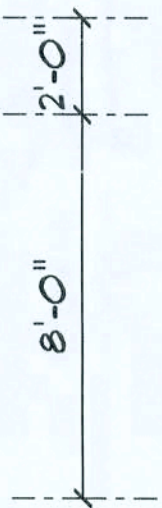
SOFFIT VENT
4187 S.F. / 300 x 50% = 6.9 S.F. SOFFIT VENT AREA REQUIRED
230 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



REAR ELEVATION



Lipscomb Eagle
Development

Devare Residence

ADDRESS:
Lot 8 Hills of Huntsville S/D
Columbia County, Florida

PRINTED DATE:
January 30, 2008

DRAWN BY:

STRUCTURAL BY:

FINALS DATE:
30 / Jan / 08

JOBNUMBER:
801261

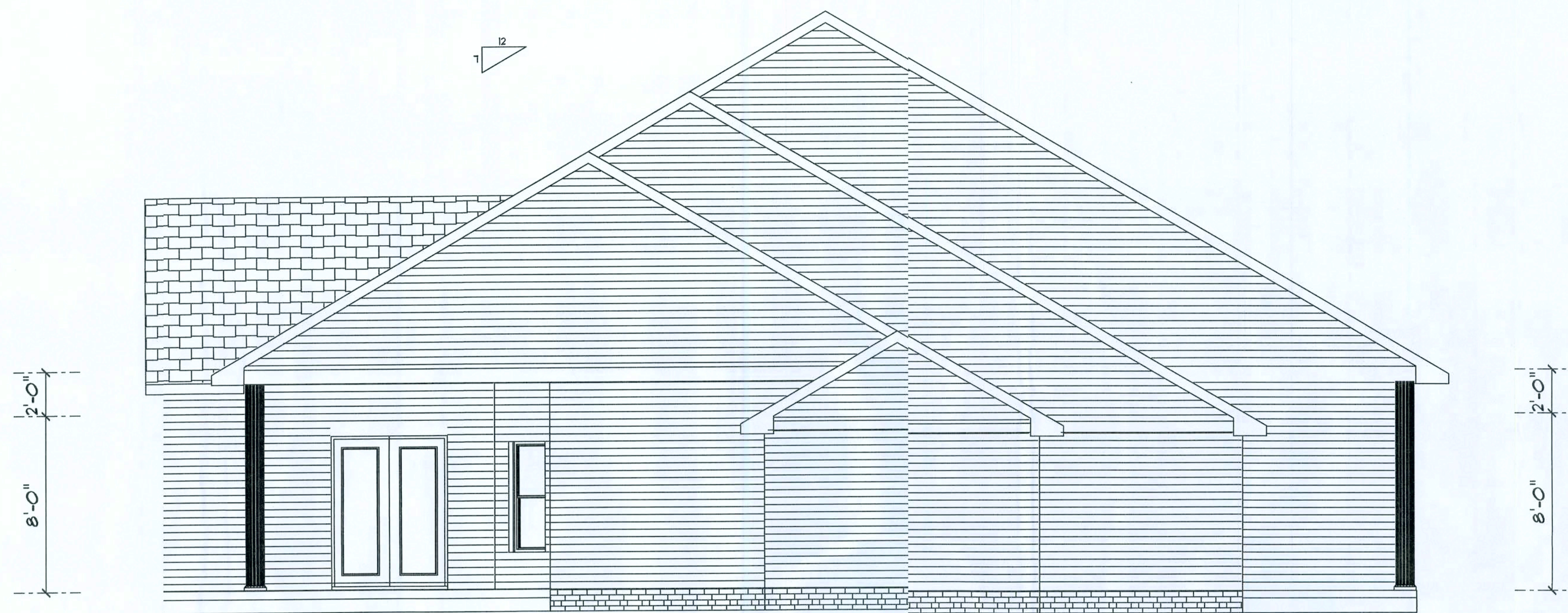
DRAWING NUMBER

1

OF 4 SHEETS

REVISIONS

SCFPLAN
ARCHITECTURAL DESIGN SOFTWARE



LEFT ELEVATION



RIGHT ELEVATION

Lipcomb Eagle
Development

Devane Residence

ADDRESS:
Lot 8 Hills of Huntsville S/D
Columbia County, Florida

PRINTED DATE:
January 30, 2008

DRAWN BY: STRUCTURAL BY:

FINALS DATE:
30 / Jan / 18

JOB NUMBER:
801261

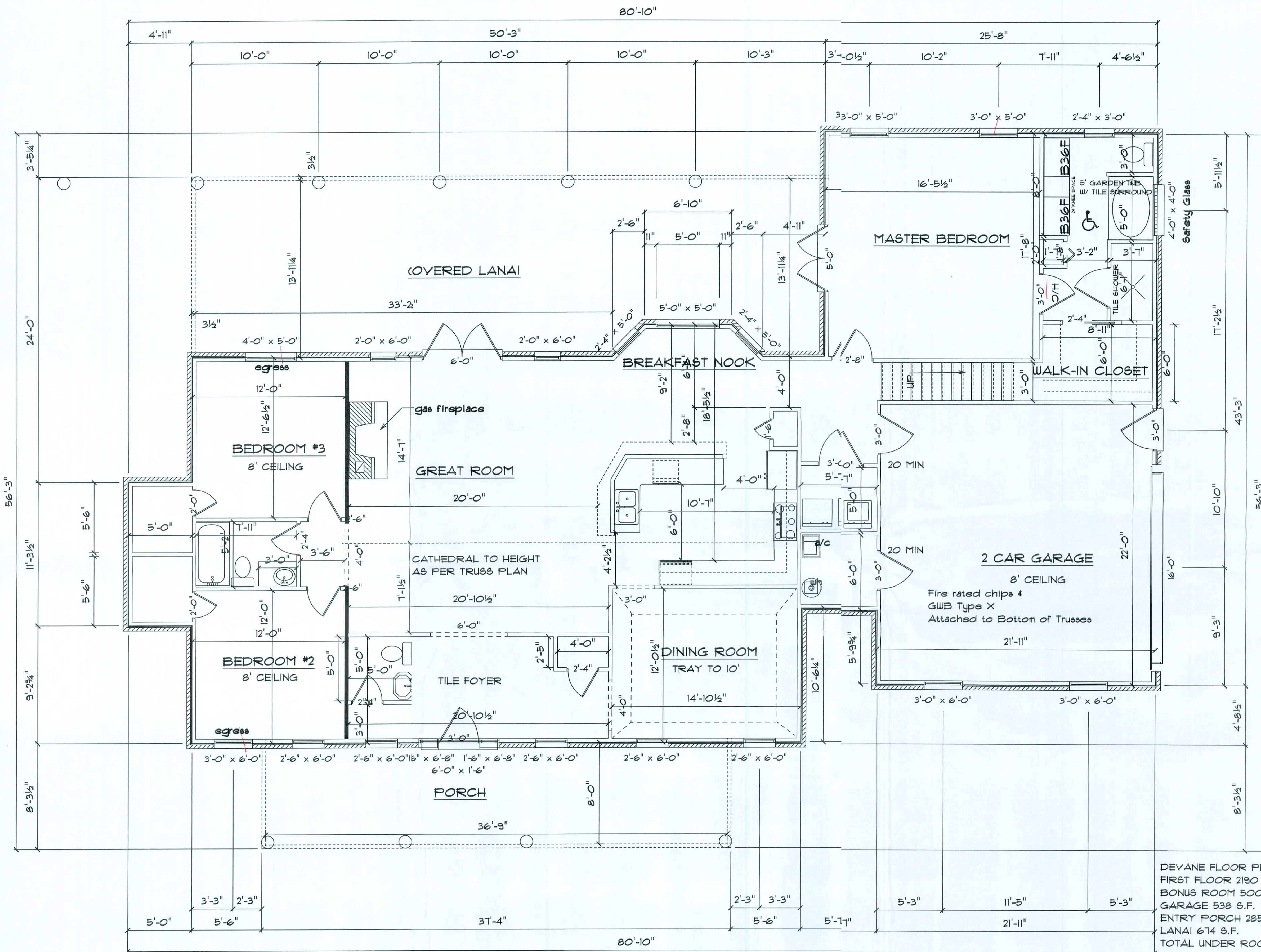
DRAWING NUMBER

2

OF 4 SHEETS

Notes:

1. Staircase run not to exceed 9"
Rise not to exceed 7"
2. Garage will be separated from all habitable rooms by not less than 5/8" Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than 1/2" gypsum board or equivalent.
3. The second story bonus area is not intended to be used as a bedroom.
4. The type and thickness of the sub flooring for the bonus room area to be 3/4" plywood. Nailing #8 every 4" on edge and every 8" in the field.



DEVANE FLOOR PLAN
FIRST FLOOR 2190 S.F.
BONUS ROOM 500 S.F.
GARAGE 538 S.F.
ENTRY PORCH 285 S.F.
LANAI 674 S.F.
TOTAL UNDER ROOF 4187 S.F.

FLOOR PLAN

SCALE: 1/4" = 1'-0"

- Garage fire separations shall comply with the following:
1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
 2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
 3. A separation is not required between a Group R-3 and U carport provided the carport entirely open on two or more sides and there are not enclosed areas above.

Lipscomb Eagle
Development

Devane Residence

ADDRESS:
Lot 8 Hills of Funtsville S/D
Columbia County, Florida

PRINTED DATE:
January 31, 2008

DRAWN BY: STRUCTURAL BY:

FINALS DATE:
30 / Jan / 08

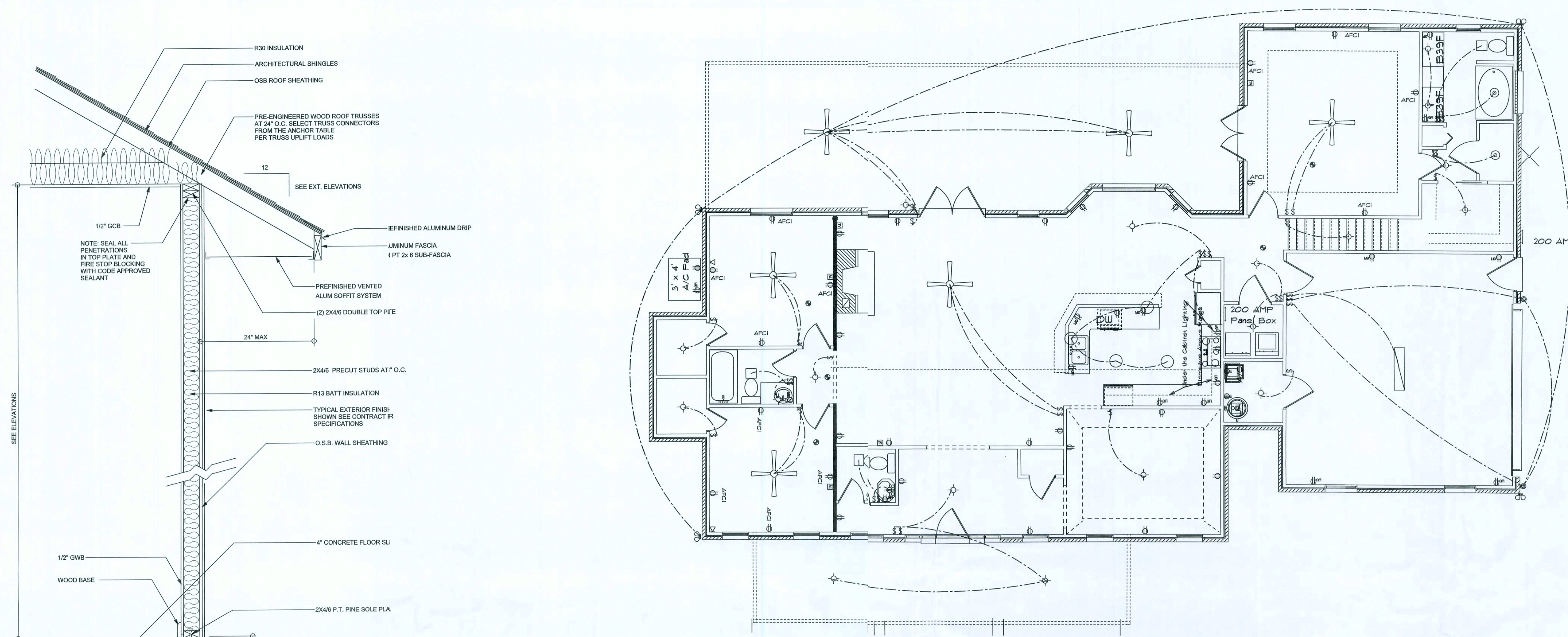
JOB NUMBER:
801261

DRAWING NUMBER

3

OF 4 SHEETS

REVISIONS	

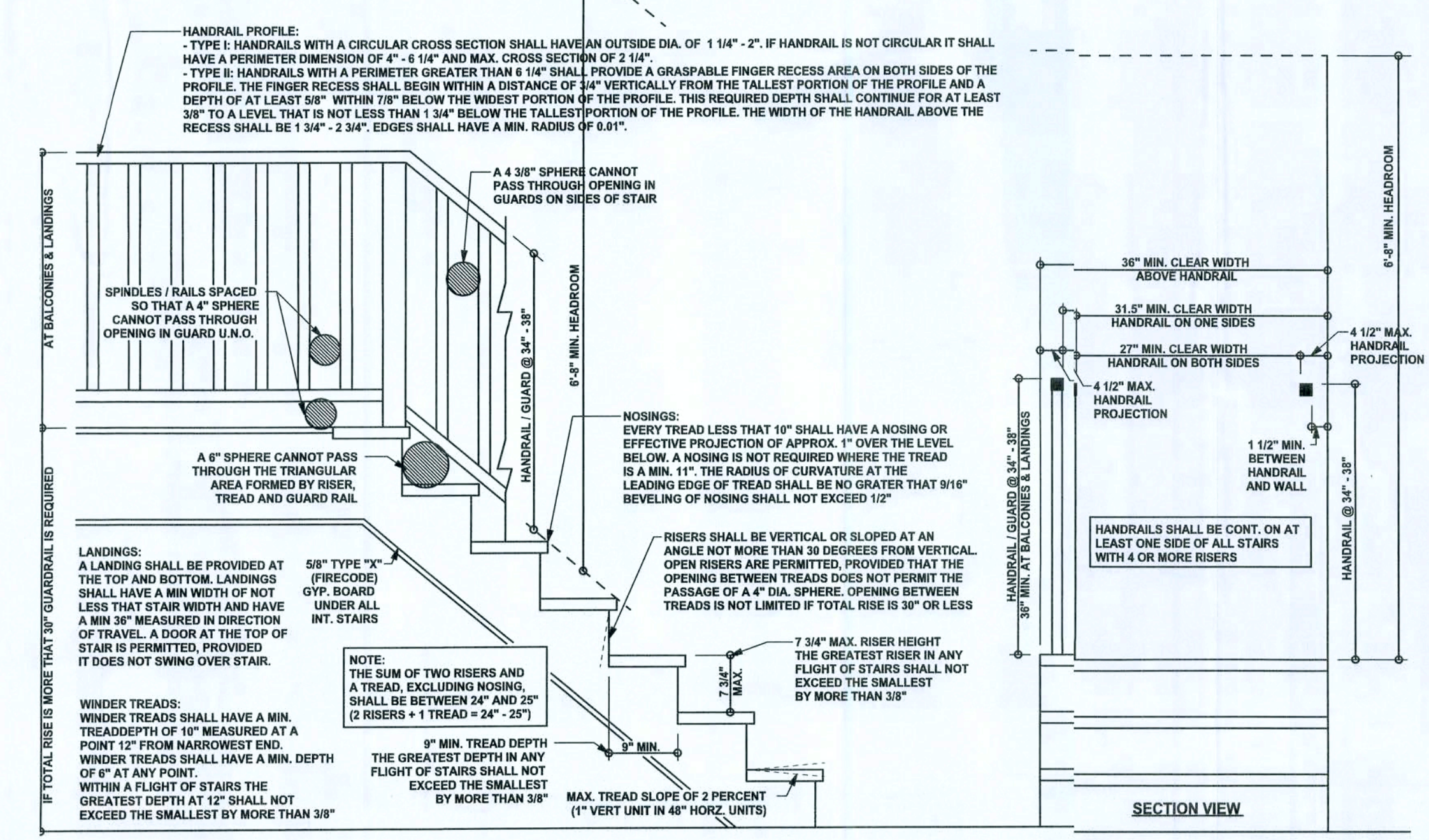


ELECTRICAL PLAN

This electrical/mechanical plan is intended to show schematically the layout of the electrical components and the HVAC system. Final locations and sizing of the components shall be the responsibility of a licensed electrical/mechanical contractor. All work shall be performed in accordance with all applicable codes.

TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

SCALE: 1" = 1'-0"



TYPICAL STAIR AND GUARDRAIL REQUIREMENTS

SCALE: 3/4" = 1'-0"

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

ELECTRICAL LEGEND

	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

Lipscomb Eagle
Development

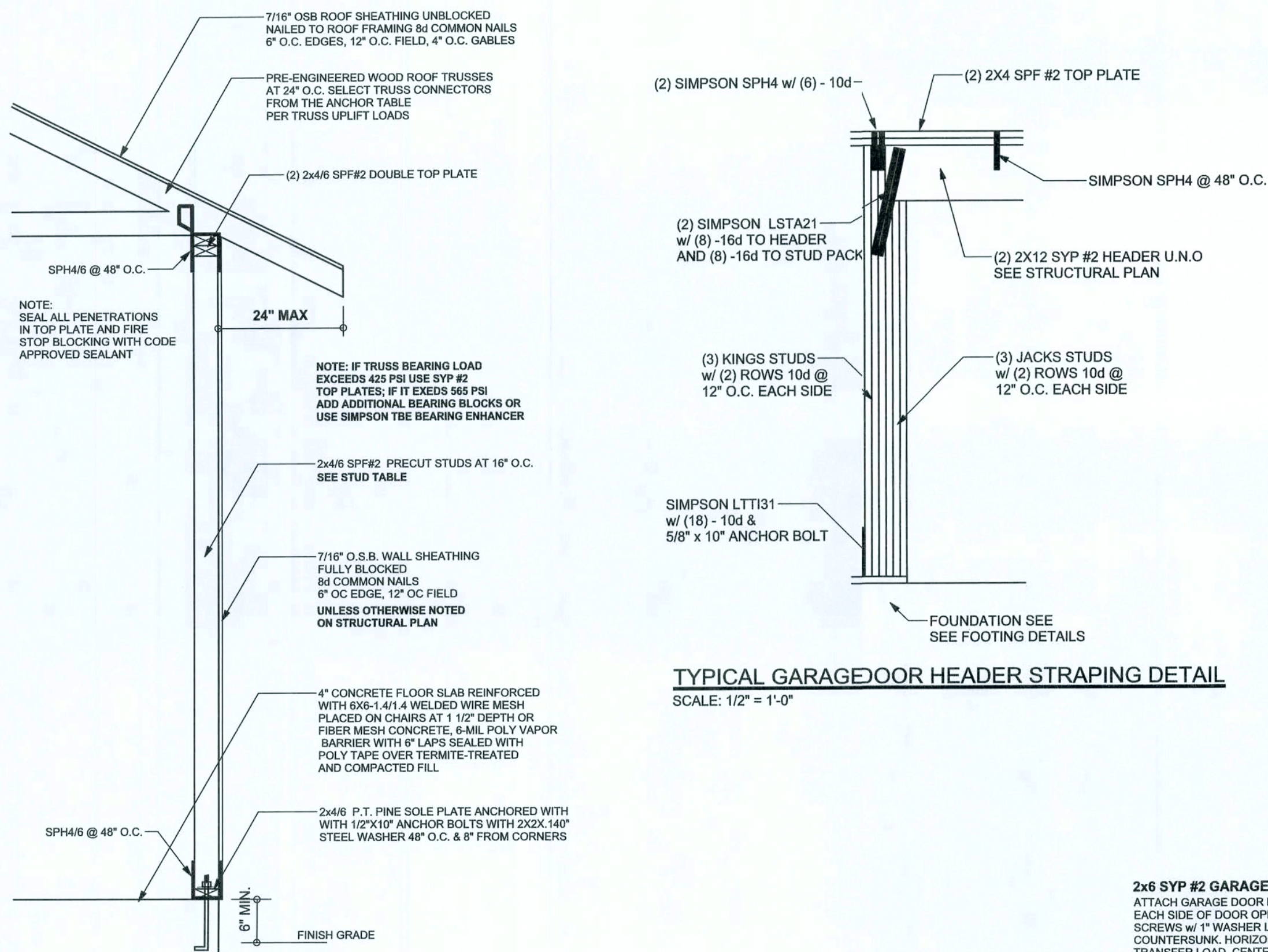
Devare Residence

ADDRESS:
Lot 8 Hill of Huntsville S/D
Columbia County, Florida

PRINTED DATE:
January 30, 2008
DRAWN BY: STRUCTURAL BY:

FINALS DATE
30 / Jan / 08

JOBNUMBER:
301261
DRAWING NUMBER



TYPICAL GARAGE DOOR HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

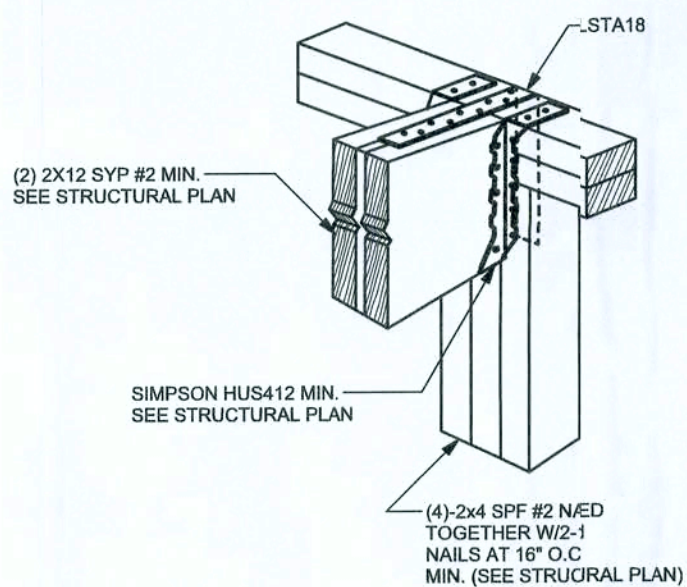
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

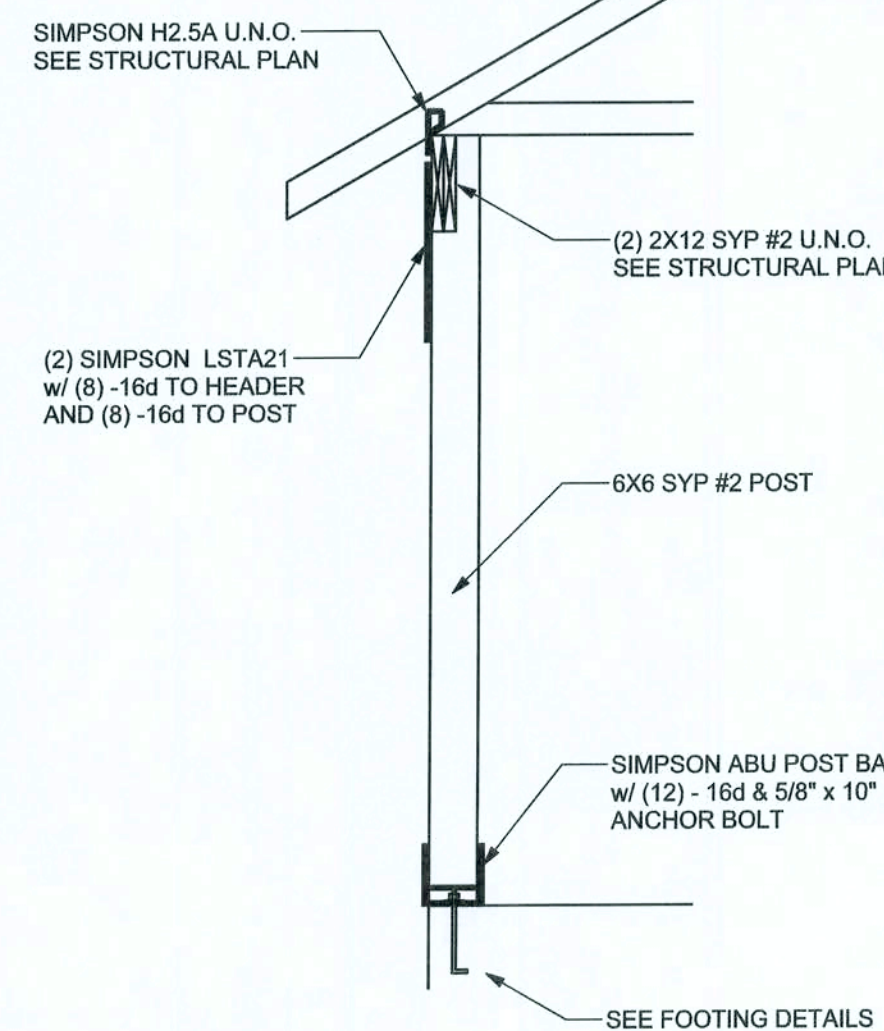
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLE OF FRAMING STUDS LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

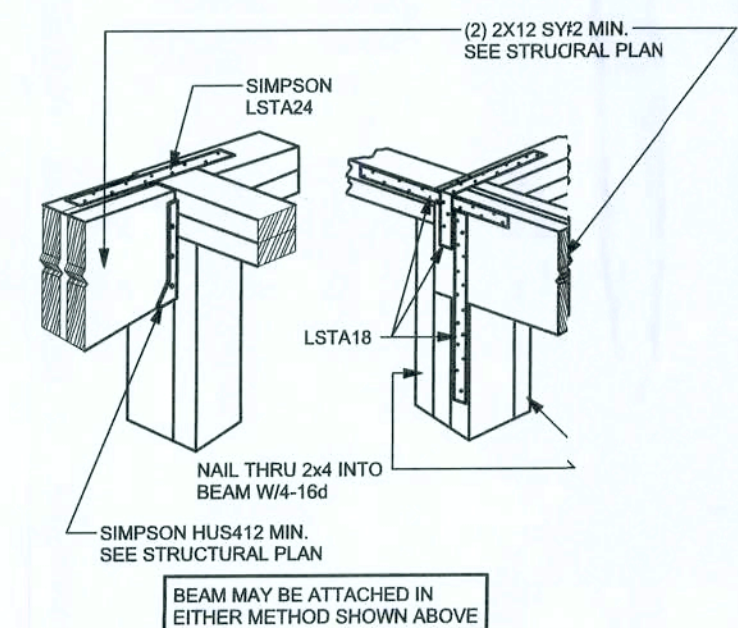
INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



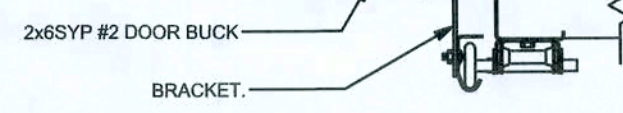
BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.



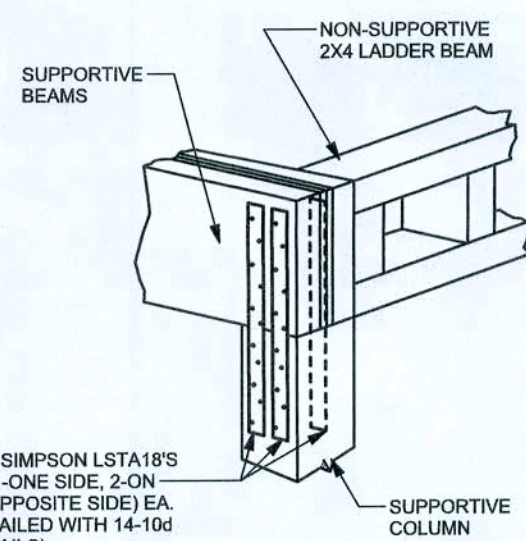
TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"



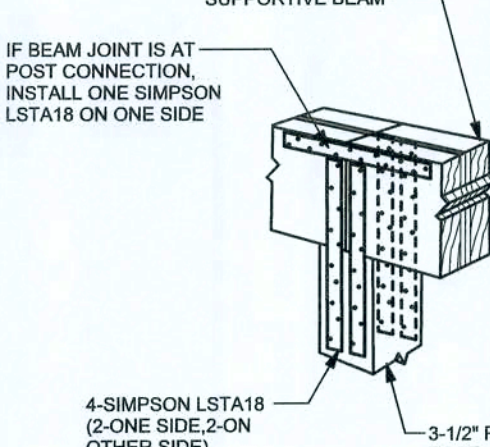
BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.



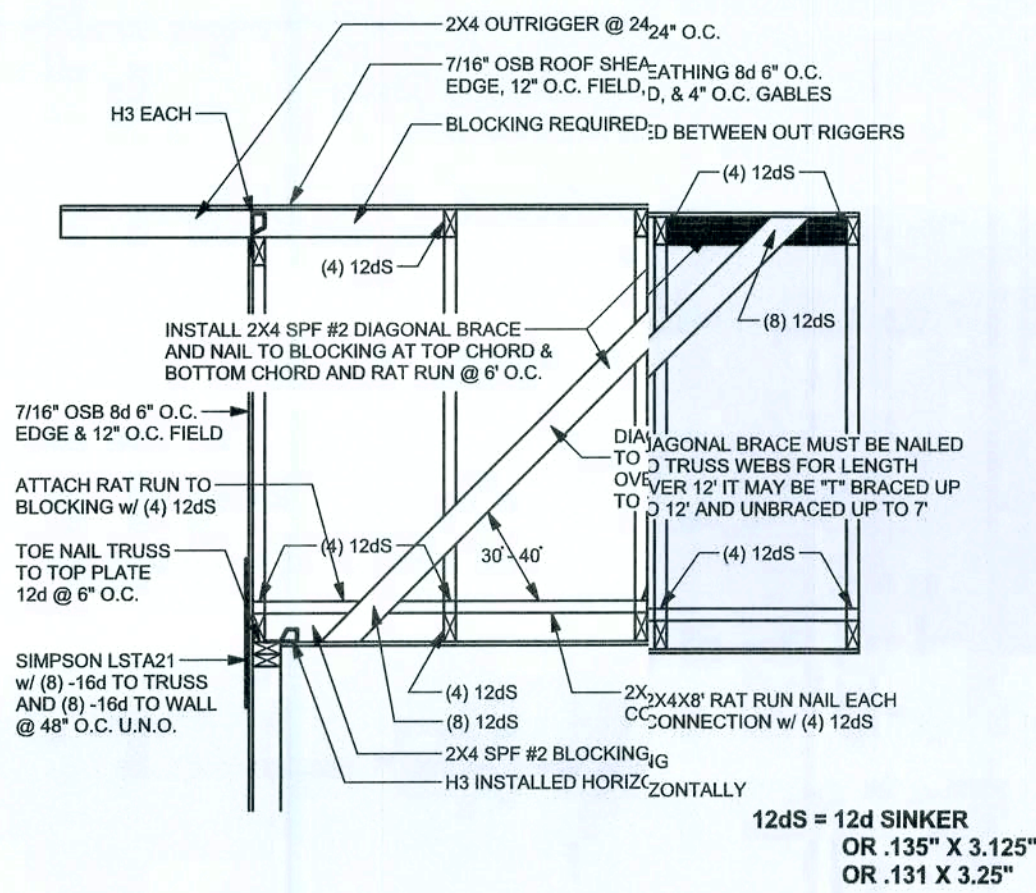
GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM
SCALE: N.T.S.



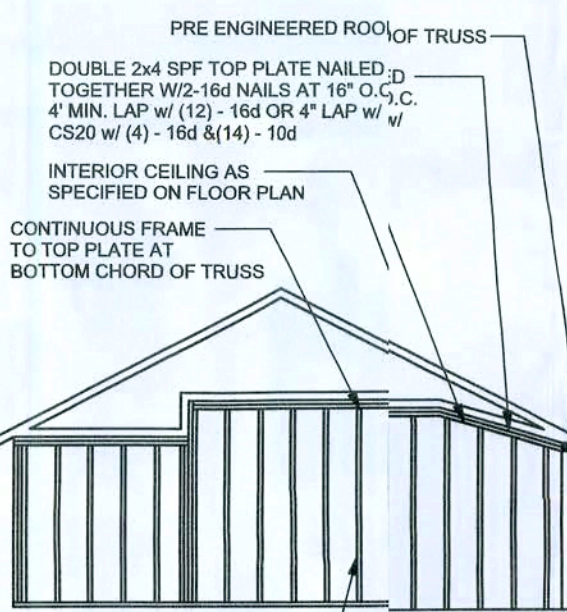
SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.



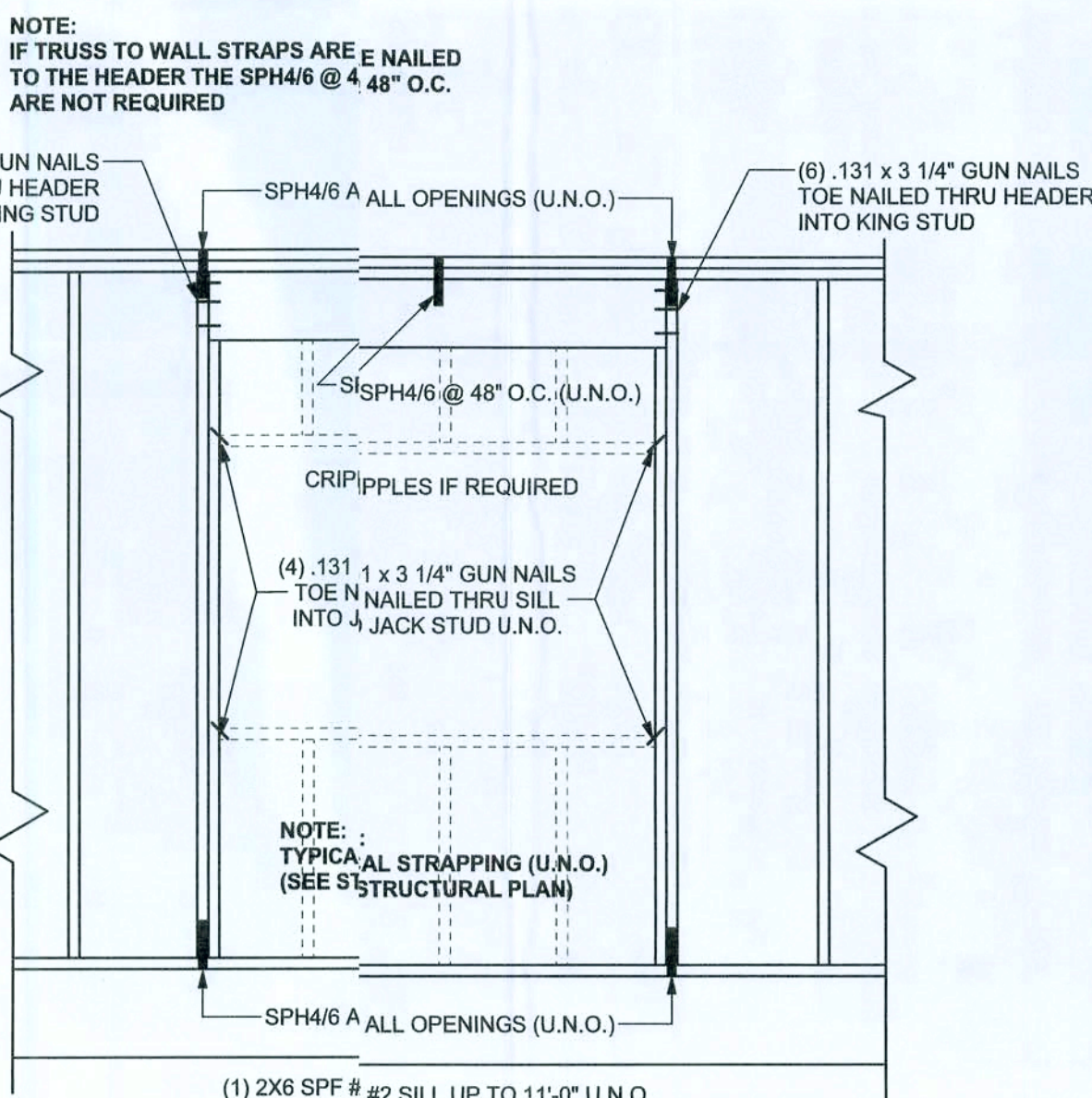
TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

		Fb (F _b)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.



TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004 TRUSS ENGINEERING. TRUSSES SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTORS BASED ON TRUSS ENGINEERING UPLIFT AND REACTION FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS TO LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 8" x 6" W14, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A188, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 10 INCH TO 3 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPICES 40" DB (20" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2.4ksi, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (1.51) 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1A/SEI 610MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F _m = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 960, 0.80 oz/ft ² or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG-T		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 1240	< 1065	SPH4			6-10d, 1 1/2"
< 885	< 760	SPH4			10-10d, 1 1/2"
< 1240	< 1065	SPH6			6-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*				TO STUDS	TO FOUNDATION
< 1350	< 1305	LT119	8-16d		1/2" AB
< 2310	< 2310	LT101	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HT116	18-16d		5/8" AB
< 1400	< 1400	PAWD42	16-16d		
< 3335	< 3335	HPWD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = < 30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	
1	19.9 - 21.8	18.1 - 21.8
2	19.9 - 25.5	18.1 - 21.8
2 Ohg	-40.6	-40.6
3	19.9 - 25.5	18.1 - 21.8
3 Ohg	-68.3	-42.4
4	21.8 - 23.6	18.5 - 20.4
5	21.8 - 29.1	18.5 - 22.6

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	30 PSF (SLEEPING ROOMS)	30 PSF (ATTICS WITH STORAGE)	10 PSF (ATTICS WITHOUT STORAGE, <12)
ROOF	20 PSF (FLAT OR <4:12)	16 PSF (4:12 TO <12:12)	12 PSF (12:12 AND GREATER)	STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF				
NOT IN FLOOD ZONE (BUILDER TO VERIFY)				

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Dicosway, P.E. No. 53816, P.O. Box 68, Lake City, FL 32065, 386-754-5411

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Dicosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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PRINTED DATE:
January 30, 2008

FINALS DATE:
30 / Jan / 08

JOB NUMBER:
81261

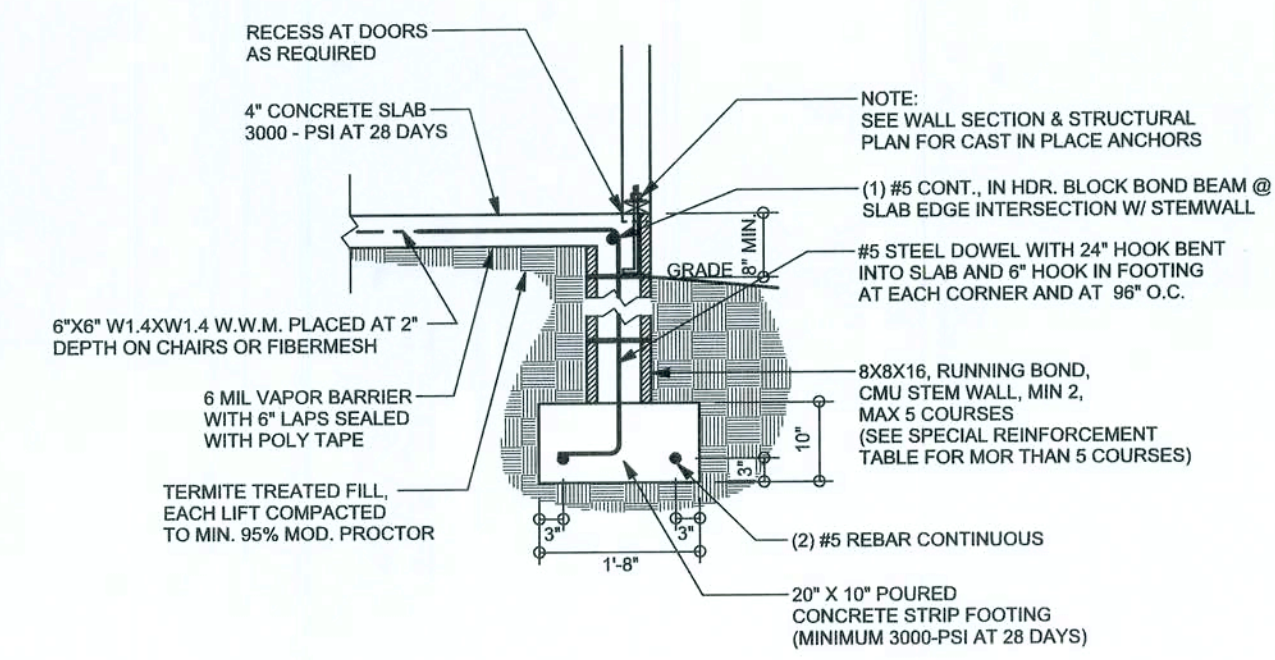
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S-1

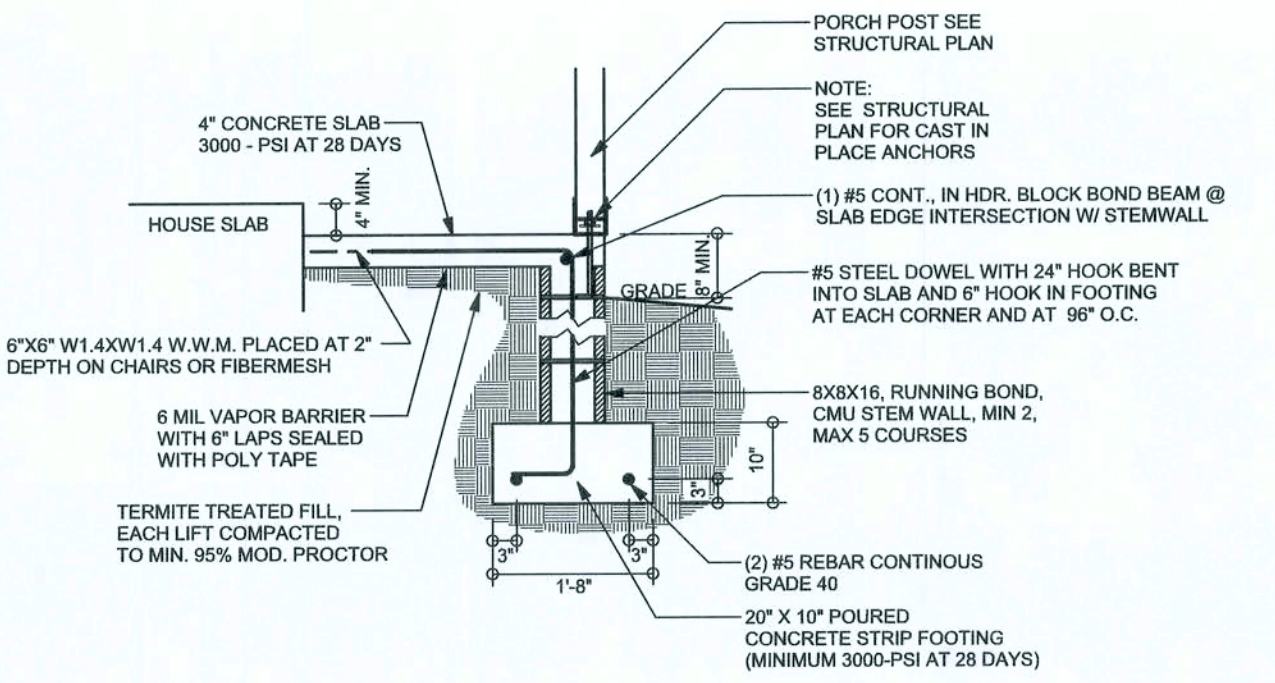
OF: SHEETS

REVISIONS	

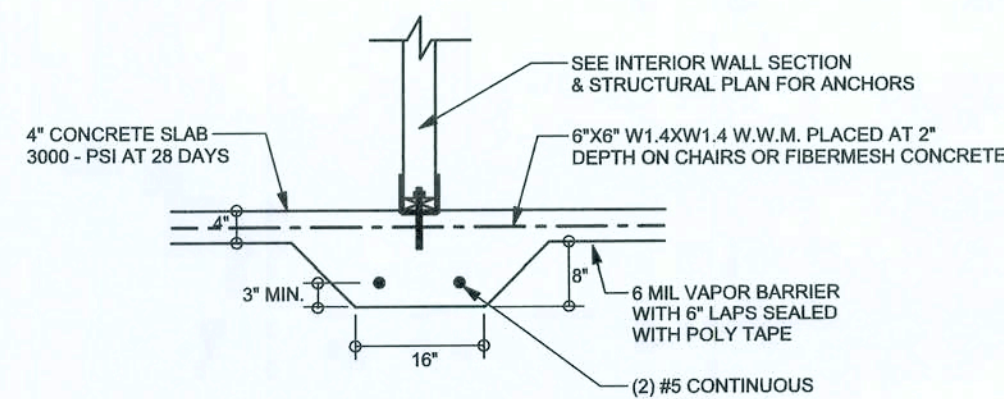
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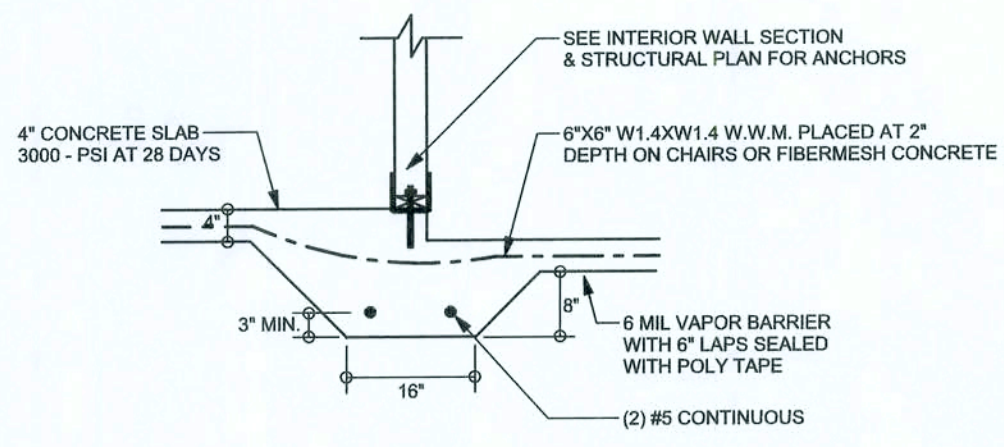
F9
S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



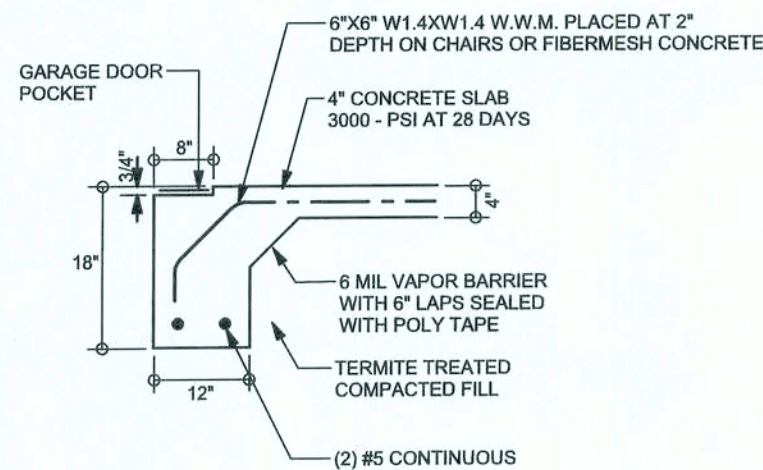
F12
S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F2
S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3
S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

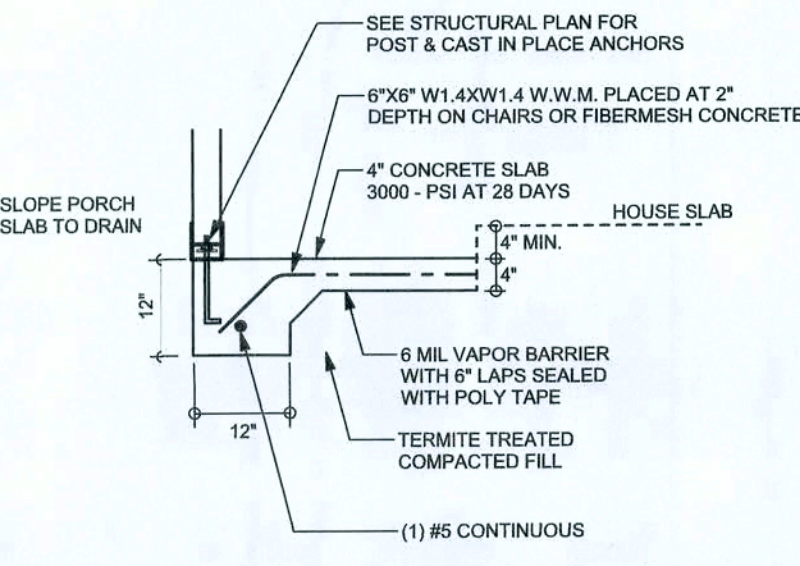


F4
S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

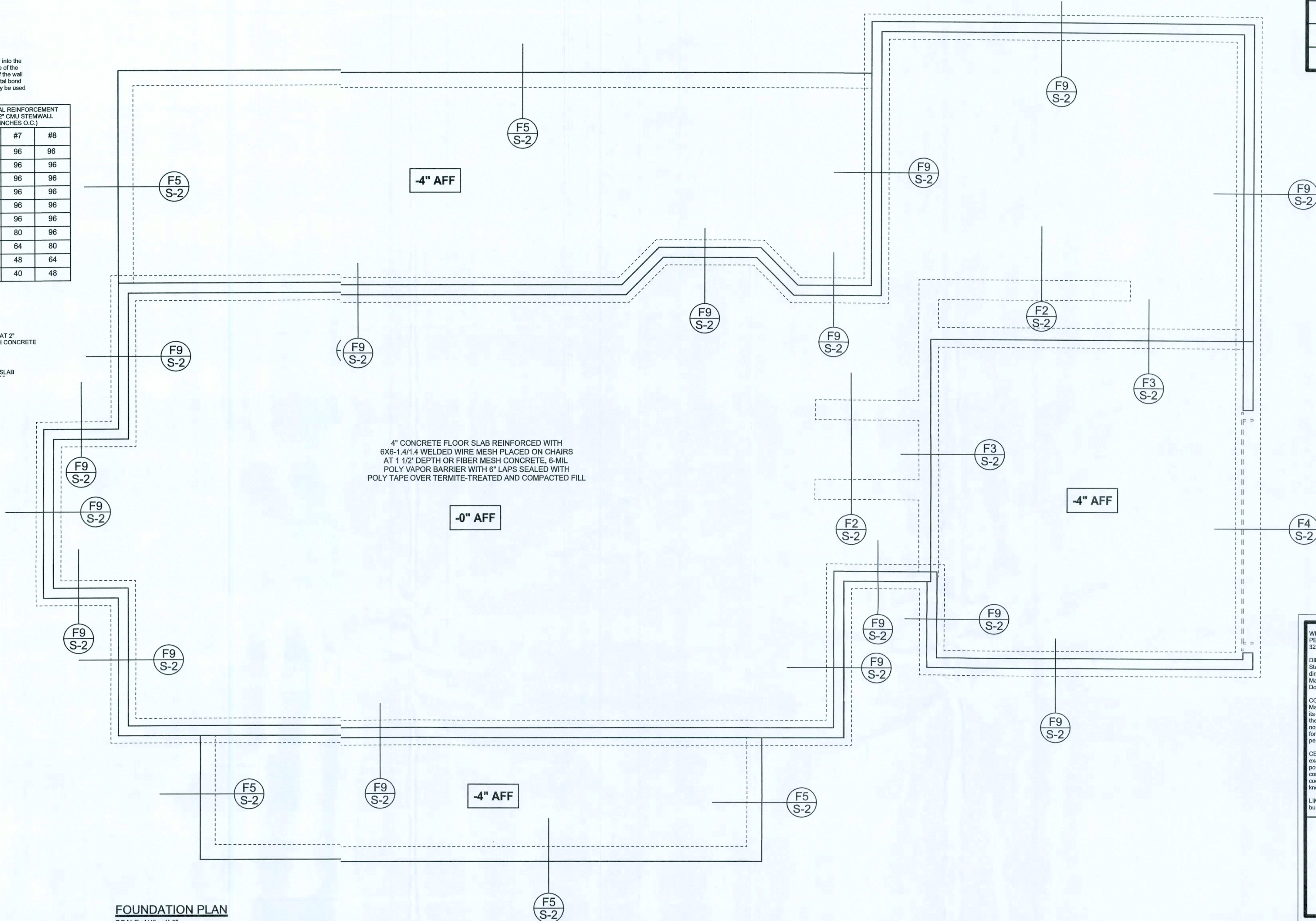
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab on top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add wall ladder reinforcement at 18" OC vertically or a horizontal bond beam with 16S circous at mid height. For higher parts of the wall 12" CMU may be used with reinforcing shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED JOINT FILL EIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F5
S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,
PE No. 33915, OS 868, Lake City, FL
32056, 386-75-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F501.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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P.E. 33915

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January 30, 2008

STRUCTURAL BY:

FINALS DATE:
30 / Jan / 08

JO3 NUMBER:
801261

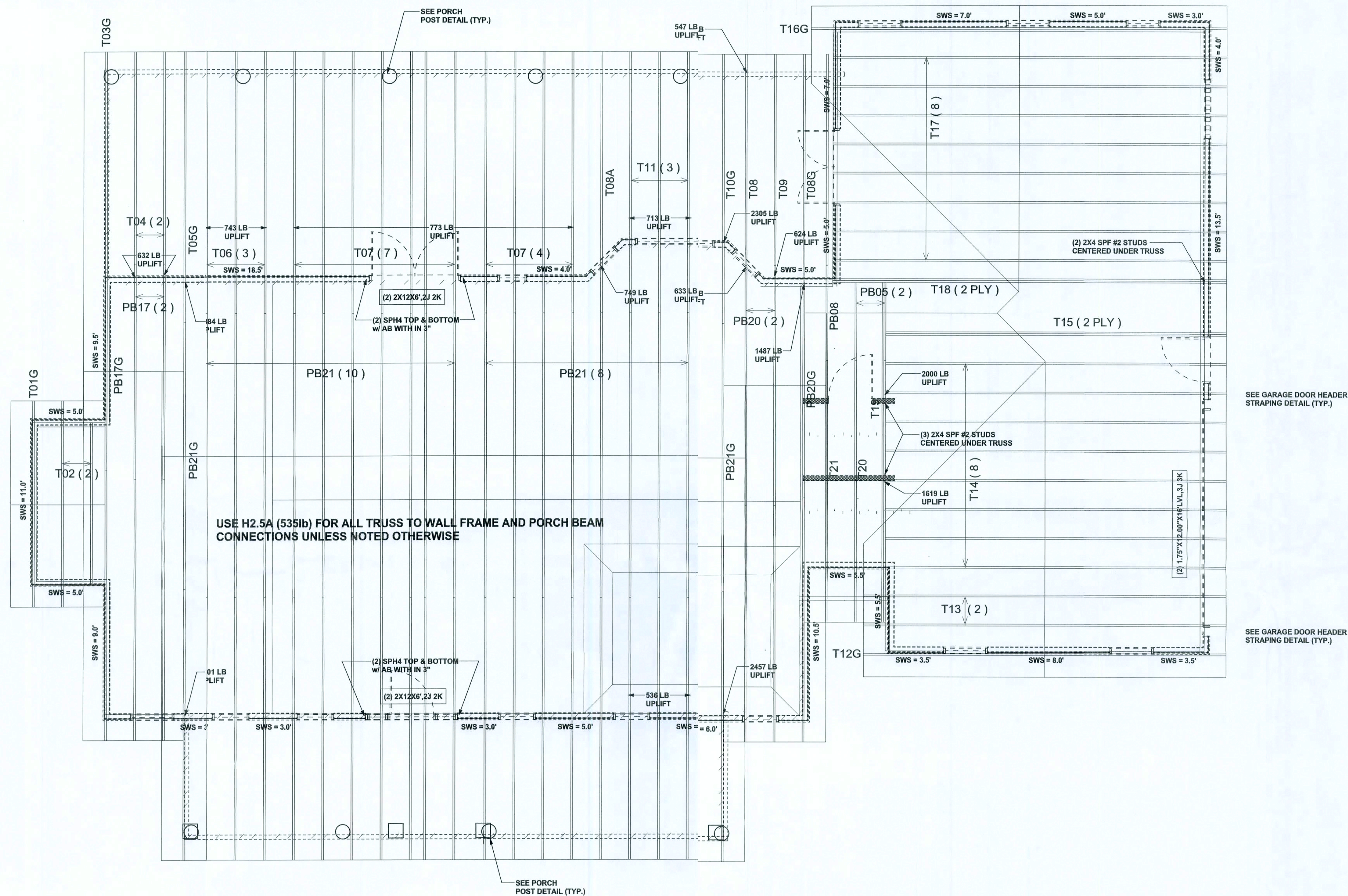
DRAWING NUMBER

S-2

OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLN SCALE: 1/4" = 1'-0"

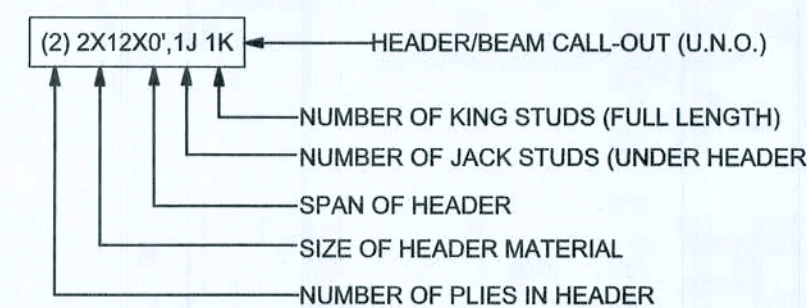
STRUCTURAL PLN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2 & BCSI-B3. BCSI-B1, BCSI-B2 & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	39.5'	74.0'
LONGITUDINAL	32.8'	93.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L266613

WINDLOAD ENGINEER: Mark Discoway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
32056, 386-754-5411

DIMENSIONS: Stated dimensions are scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1901.2.1, Florida building code residential 200, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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30 / Jan / 08

JOB NUMBER:
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DRAWING NUMBER

S-3

OF SHEETS