

GENERAL NOTES

DESIGN CRITERIA

1. THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE LOADING CRITERIA OF ASCE 7-16 AND THE FLORIDA BUILDING CODE, 1TH EDITION 2020.
2. DESIGN LOADS FOR THE STRUCTURE AND ITS COMPONENTS SHALL BE AS FOLLOWS:
SLOPED ROOF
DEAD LOAD: TC = 25 PSF, BC = 10 PSF
LIVE LOAD: TC = 20 PSF
3. WIND CRITERIA FOR THE STRUCTURE SHALL BE AS FOLLOWS:
WIND DESIGN METHOD = SIMPLIFIED METHOD
ULTIMATE WIND DESIGN SPEED = 120 MPH (3 SECOND WIND GUST).
NOMINAL DESIGN WIND SPEED = 93 MPH.
RISK CATEGORY OF STRUCTURE = II.
WIND EXPOSURE CATEGORY = C.
WIND ENCLOSURE COEFFICIENT = ± 0.18 (ENCLOSED).
WIND PRESSURES FOR COMPONENT & CLADDING = REFER TO §16.
4. DEFLECTION LIMITS SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS BASED ON MATERIALS SPECIFIED BY THE ARCHITECT. MATERIALS THAT REQUIRE DEFLECTION CRITERIA EXCEEDING CODE LIMITS PER THE MANUFACTURER'S AND/OR ARCHITECT'S SPECIFICATIONS SHALL MEET THE CRITERIA SPECIFIED BY THE MANUFACTURER AND/OR THE ARCHITECT.
5. CONTRACTOR SHALL INFORM AND SUBMIT TO THE ENGINEER OF RECORD LOADING INFORMATION FOR ALL EQUIPMENT NOT SHOWN, OR DIFFERING FROM THOSE SHOWN, ON THE CONTRACT DRAWINGS.
6. DELEGATE ENGINEERS SHALL ACCOUNT FOR SELF WEIGHT OF STRUCTURE IN DESIGN AS REQUIRED.

SHOP DRAWINGS AND SUBMITTALS

1. SHOP DRAWINGS AND SUBMITTALS FOR ALL STRUCTURAL FRAMING, ELEMENTS, COMPONENTS, AND SYSTEMS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED AND APPROVED BY THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION.
2. SPECIAL ENGINEERING COMPONENTS SUCH AS PRE-ENGINEERED WOOD TRUSSES AND COLD-FORMED STEEL SYSTEMS SHALL BE PROVIDED BY A DELEGATE REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA WHO SHALL BE THE ENGINEER OF RECORD OF THESE SPECIAL ENGINEERING COMPONENTS PER THE FLORIDA ADMINISTRATIVE CODE SECTION 61G15-30.006. SIGNED AND SEALED DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW OF GENERAL CONFORMANCE WITH THE DESIGN CRITERIA SPECIFIED BY THE ENGINEER OF RECORD.
3. SHOP DRAWINGS AND SUBMITTALS SHALL BE REVIEWED BY THE ENGINEER OF RECORD FOR OVERALL COMPLIANCE WITH CONTRACT DOCUMENTS. COMMENTS AND/OR CORRECTIONS MARKED ON SHOP DRAWINGS AND SUBMITTALS SHALL NOT ALLEVIATE THE CONTRACTOR FROM CONFORMANCE WITH THE CONTRACT DOCUMENTS.
4. CONTRACT DOCUMENTS SHALL PRESIDE OVER SHOP DRAWINGS AND SUBMITTALS UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD.
5. SHOP DRAWINGS SHALL BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION TO THE ENGINEER OF RECORD. REVIEW STAMP SHALL BE DATED, INITIALED AND INDICATING REVIEW DISPOSITION.
6. CONTRACTOR SHALL BE RESPONSIBLE FOR CONFORMING DIMENSIONS, ELEVATIONS, QUANTITIES, FOR FABRICATION SELECTION PROCESSES, FOR CONSTRUCTION METHODS, FOR SUBCONTRACTORS COORDINATION AND FOR EXECUTION OF THE WORK IN A SAFELY MANNER ACCORDING TO THE LATEST CODES AND STANDARDS.
7. CONTRACTOR SHALL ALLOW FOR (10) TEN BUSINESS DAYS FOR EACH SUBMITTAL PRIOR TO REVIEW AND/OR APPROVAL FROM THE ENGINEER OF RECORD.

SITE WORK

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING EXPLORATION REPORT PROVIDED BY CAL-TECH TESTING, INC, PROJECT No. 21-00911-01, DATED JANUARY 11, 2022.
2. AN ALLOWABLE DESIGN SOIL BEARING PRESSURE OF 2,000 PSF HAVE BEEN USED BASED ON THE AFOREMENTIONED REFERENCED GEOTECHNICAL REPORT.
3. THE GEOTECHNICAL ENGINEER SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY OF ANY DISCREPANCIES BETWEEN THE RECOMMENDED DESIGN SOIL BEARING PRESSURE AND THE ACTUAL SOIL CONDITIONS. CHANGES NECESSARY TO THE CONTRACT DOCUMENTS DUE TO SPECIFIC INFORMATION IN THE GEOTECHNICAL REPORT SHALL BE CONSIDERED ADDITIONAL SERVICES.
4. CONTRACTOR SHALL REVIEW THE GEOTECHNICAL REPORT AND MAKE ALL SITE PREPARATION IN ACCORDANCE TO THIS DOCUMENT.
5. FILL MATERIAL SHALL BE CLEAN SOIL, FREE OF DEBRIS AND ORGANIC MATERIAL. FILL MATERIAL SHALL BE PLACED IN ONE FOOT LIFTS AND COMPACTED TO A CRITERIA NOT LESS THAN 95% OF ITS MODIFIED PROCTOR DENSITY.
6. CONTRACTOR SHALL COORDINATE ALL BELOW GRADE PIPING ROUTING WITH THE FOUNDATIONS. NO PIPING THROUGH THE FOUNDATIONS SHALL BE ALLOWED. REFER TO CIVIL DRAWINGS FOR MINIMUM PIPING CLEARANCE BELOW THE FOUNDATIONS.
7. FOUNDATION WALLS RETAINING SOIL SHALL BE BRACED UNTIL FLOOR SLABS ARE IN PLACE AND HAVE REACHED THEIR SPECIFIED COMPRESSIVE STRENGTH.

CAST IN PLACE CONCRETE

1. CAST IN PLACE CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF THE FOLLOWING CODES AND STANDARDS (LATEST EDITIONS):
- a. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-08).
- b. DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315).

- c. SPECIFICATION FOR HOT WEATHER CONCRETING (ACI 308).
- d. SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301).
- e. CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
2. CONCRETE SHALL BE NORMAL WEIGHT UNLESS NOTED OTHERWISE AND SHALL COMPLY WITH ASTM C194 AND THE FOLLOWING:
- a. PORTLAND CEMENT SHALL CONFORM TO ASTM C150.
- b. AGGREGATES (¾" MAX.) SHALL CONFORM TO ASTM C33.
- c. AIR ENTRAINING AGENT SHALL CONFORM TO ASTM C260.
- d. WATER REDUCING AGENT SHALL CONFORM TO ASTM C494.
- e. FLYASH (20% MAX.) CLASS F SHALL CONFORM TO ASTM C618-18.
- f. WATER SHALL BE POTABLE.
- g. REQUIRED SLUMP RANGE = 3" TO 5".
3. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:
- | | | |
|---------------|------------|------------------|
| SLAB-ON-GRADE | = 3000 PSI | W/C RATIO = 0.55 |
| MASONRY GROUT | = 3000 PSI | W/C RATIO = 0.58 |
| FOOTINGS | = 3000 PSI | W/C RATIO = 0.52 |
| COLUMNS | = 4000 PSI | W/C RATIO = 0.45 |
| WALLS | = 4000 PSI | W/C RATIO = 0.45 |
| BEAMS | = 4000 PSI | W/C RATIO = 0.45 |
| TIE COLUMNS | = 4000 PSI | W/C RATIO = 0.45 |
| TIE BEAMS | = 4000 PSI | W/C RATIO = 0.45 |

4. REINFORCING STEEL BARS USED SHALL BE DEFORMED BARS GRADE 60 KSI AND SHALL CONFORM TO ASTM A615. REINFORCEMENT SHALL BE BENT AND PLACED IN ACCORDANCE WITH CRSI REQUIREMENTS.
5. WELDED WIRE FABRIC USED SHALL CONFORM TO ASTM A1064 AND SHALL BE FURNISHED IN FLAT SHEETS, NOT ROLLS. MINIMUM MESH LAP SHALL BE (2) PANELS.
6. VAPOR RETARDER USED SHALL BE 10 MIL POLYETHYLENE. MINIMUM LAP AT JOINTS SHALL BE 6".
7. REINFORCING BAR COVER SHALL BE AS FOLLOWS:
- | | |
|-------------------------|--------------------------------|
| FOOTINGS | = 3" |
| SLAB ON GRADE | = 3" (BOTTOM) 1" (TOP) |
| COLUMNS AND TIE COLUMNS | = 2" (EXTERIOR) 1 ½" (TYPICAL) |
| SHEAR WALLS | = 2" (EXTERIOR) 1 ½" (TYPICAL) |
| BEAMS AND TIE BEAMS | = 1 ½" |

8. CONTRACTOR SHALL INSTALL, IN ACCORDANCE WITH CRSI REQUIREMENTS, TIES, SPACERS, CHAIRS, BOLSTERS, ETC NECESSARY TO HOLD REINFORCEMENT SECURELY DURING CONCRETE PLACEMENT. USE PLASTIC TIPS ON ALL EXPOSED SURFACES.
9. CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS, SLEEVES, RECESSES, EMBEDS, ACCESSORIES, ETC PRIOR TO CONCRETE PLACEMENT. NOTIFY ENGINEER OF RECORD OF ANY CONFLICTS WITH REBAR LOCATIONS. NO OPENING OR SLEEVE SHALL BE ALLOWED IN BEAMS OR COLUMNS UNLESS APPROVED BY THE ENGINEER OF RECORD.
10. ALL PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO CONCRETE PLACEMENT.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER DESIGN, CONSTRUCTION, SHORING, RESHORING, AND BACKSHORING OF CONCRETE FORMS. DESIGN SHALL BE PERFORMED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA.
12. CONCRETE FORMS SHALL BE COATED WITH COMMERCIAL COMPOUNDS THAT WILL NOT BOND, STAIN, OR ADVERSELY AFFECT THE CONCRETE SURFACES. FORMS SHALL BE UET BEFORE CONCRETE PLACEMENT. CONCRETE FORMS SHALL REMAIN IN PLACE UNTIL TESTING SAMPLES INDICATE THAT CONCRETE HAS ATTAINED A MINIMUM OF 61% OF THE SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS.
13. REPAIR AND PATCH DEFECTIVE AREAS AND HONEYCOMBS IMMEDIATELY AFTER REMOVAL OF FORMS WITH A HIGH STRENGTH CEMENT MORTAR. NOTIFY THE ENGINEER OF RECORD FOR EVALUATION OF EXPOSED REINFORCING IN DEFECTIVE AREAS.
14. CONCRETE SHALL BE CURED IN ACCORDANCE WITH ONE OF THE FOLLOWING OPTIONS:
- a. APPLY A LIQUID MEMBRANE FORMING A CHEMICAL CURING COMPOUND CONFORMING TO ASTM C309.
- b. PROVIDE CONTINUOUS MOISTURE TO CONCRETE IN ACCORDANCE WITH ACI 301.
16. CONCRETE SHALL BE CONSOLIDATED IN PLACE USING INTERNAL VIBRATORS.
17. CONCRETE FINISHES (COLOR, CHAMFERING, ETC) SHALL BE PER ARCHITECTURAL DRAWINGS.
18. THE FOLLOWING TESTS SHALL BE PERFORMED ON SITE BY A QUALIFIED TESTING LABORATORY AND COPY OF RESULTS SENT TO OWNER, ARCHITECT, ENGINEER OF RECORD, AND CONTRACTOR:
- a. COMPRESSIVE STRENGTH TESTS IN ACCORDANCE WITH ASTM C39. FOUR LAB-CURED AND FOUR FIELD-CURED CYLINDERS FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF OF EACH CLASS OF CONCRETE PLACED PER DAY. ONE CYLINDER SHALL BE TESTED AT 1 DAYS AND TWO CYLINDERS AT 28 DAYS. THE FINAL CYLINDER SHALL BE RESERVED AND TESTED, IF NECESSARY, WITH PRIOR APPROVAL FROM THE ENGINEER OF RECORD.
- b. SLUMP TESTS IN ACCORDANCE WITH ASTM C143.
19. CAST IN PLACE CONCRETE SUBMITTALS:
- a. CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO CONCRETE PLACEMENT. SUBMITTALS SHALL BEAR MIX NUMBER, EXACT LOCATION WHERE MIX WILL BE PLACED IN THE STRUCTURE, AND BACKUP DATA FROM RECENT FIELD AND LAB CYLINDER TESTS.
- b. DETAILED SHOP DRAWINGS OF REINFORCING BARS INCLUDING NUMBER, SIZE, LOCATION, BAR LISTS AND BEND DIAGRAMS.

CONCRETE MASONRY UNITS

1. CONCRETE MASONRY UNITS SHALL COMPLY WITH THE REQUIREMENTS OF THE FOLLOWING CODES AND STANDARDS (LATEST EDITIONS):
- a. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-08 AND TMS 402-08).
- b. SPECIFICATION FOR MASONRY STRUCTURES (ACI 530J-08 AND TMS 602-08).

2. CONCRETE MASONRY UNITS SHALL COMPLY WITH ASTM C90 AND SHALL BE HOLLOW LOAD BEARING CONCRETE MASONRY UNITS WITH THE FOLLOWING MINIMUM PROPERTIES:
- a. NORMAL WEIGHT, TYPE II.
- b. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS = 1300 PSI
- c. NET AREA COMPRESSIVE STRENGTH OF MASONRY (Fm) = 1500 PSI
3. MORTAR SHALL BE TYPE M OR S AND SHALL COMPLY WITH ASTM C270.
4. GROUT USED IN ALL FILLED CELLS AND MASONRY BEAMS SHALL BE COARSE GROUT AND SHALL COMPLY WITH ASTM C476 WITH THE FOLLOWING MINIMUM PROPERTIES:
- a. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 3,000 PSI.
- b. MAXIMUM AGGREGATE SIZE SHALL BE ¾".
- c. REQUIRED SLUMP RANGE = 8" TO 11".
5. PROVIDE 4" x 4" CLEANOUT HOLES IN THE BOTTOM COURSE OF MASONRY FOR EACH GROUT POUR WHEN THE GROUT FOUR HEIGHT EXCEEDS 5 FT. REMOVE DEBRIS FROM CLEANOUT HOLES PRIOR TO CAPPING. TOP OF GROUT POUR SHALL BE TERMINATED ½" BELOW TOP OF MASONRY UNITS TO DEVELOP SHEAR KEY FOR CONTINUED GROUTING.
6. CONSOLIDATE GROUT AT THE TIME OF PLACEMENT BY USE OF A LOW SPEED MECHANICAL VIBRATOR. RECONSOLIDATE BY A LOW SPEED MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED.
7. MASONRY UNITS CONSTRUCTION SHALL BE PLACED IN RUNNING BOND WITH ¾" FACE SHELL BEDDING MORTAR JOINTS VERTICALLY AND HORIZONTALLY.
8. REINFORCING STEEL BARS USED SHALL BE DEFORMED BARS GRADE 60 KSI AND SHALL CONFORM TO ASTM A615.
9. HORIZONTAL REINFORCEMENT SHALL BE LADDER TYPE MASONRY WALL JOINT REINFORCEMENT TYPE DUR-O-WAL (OR EQUIVALENT) AND SHALL COMPLY WITH ASTM A82. HORIZONTAL REINFORCEMENT SHALL BE 9 GAUGE WIRE MINIMUM PLACED AT 16" o.c. MINIMUM LAP LENGTH AT SPLICE SHALL BE 6".
10. VERTICAL REINFORCEMENT BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM WITH ¼" MINIMUM CLEARANCE TO THE MASONRY WALLS AND ONE BAR DIAMETER BETWEEN BARS. VERTICAL REINFORCEMENT SHALL EXTEND FROM FOUNDATION TO CONCRETE BEAM OR TIE-BEAM.
11. PROVIDE MASONRY CONTROL JOINTS AT LOCATIONS INDICATED ON ARCHITECTURAL DRAWINGS WITH MAXIMUM SPACING OF 25'-0".
12. MASONRY CONSTRUCTION SHALL BE INSPECTED IN ACCORDANCE WITH ALL APPLICABLE CODE PROVISIONS.
13. CONCRETE MASONRY UNITS SUBMITTALS:
- a. TYPE OF CONCRETE MASONRY UNITS AND COMPRESSION STRENGTH TEST RESULTS.
- b. MASONRY GROUT MIX DESIGN SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO GROUT PLACEMENT. SUBMITTALS SHALL BEAR MIX NUMBER, EXACT LOCATION WHERE MIX WILL BE PLACED IN THE STRUCTURE, AND BACKUP DATA FROM RECENT FIELD AND LAB CYLINDER TESTS.
- c. DETAILED SHOP DRAWINGS OF REINFORCING BARS INCLUDING NUMBER, SIZE, LOCATION, BAR LISTS AND BEND DIAGRAMS.

STRUCTURAL WOOD

1. STRUCTURAL WOOD WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE FOLLOWING CODES AND STANDARDS (LATEST EDITIONS):
- a. NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION.
- b. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC).
2. DIMENSIONED LUMBER SHALL BE DRESSED S4S, AND SHALL BEAR THE GRADE STAMP OF THE SUPPLIER'S ASSOCIATION.
3. ALL STRUCTURAL WOOD SHALL BE SOUND AND FREE FROM WARP.
4. ALL STRUCTURAL WOOD SHALL BE SOUTHERN PINE, GRADE No. 2 OR BETTER WITH THE FOLLOWING MINIMUM PROPERTIES:
- Fb = 1350 PSI
Fv = 175 PSI
Fc = 1600 PSI
E = 1,400,000 PSI
MAX. MOISTURE CONTENT = 19%
5. ALL EXPOSED STRUCTURAL WOOD OR IN CONTACT WITH EARTH, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.
6. PLYWOOD SHEATHING SHALL BE DFFA-CD RATED STRUCTURAL I OR II, CDX EXPOSURE I, MIN. (4) PLY WITH EXTERIOR GLUE. ALL ROOF SHEATHING SHALL BE INSTALLED WITH PLYCLIPS.
7. APA SHEATHING RATING SHALL BE AS FOLLOWS (OR BETTER):
- FLOORS = 32/16
WALLS = 32/16
ROOF = 40/20
8. INSTALL BRIDGING IN ALL FLOORS AND ROOF JOISTS @ 8'-0" o.c. MAX. INSTALL BLOCKING IN ALL WALL STUDS AT MID-HEIGHT.
9. PROVIDE A SINGLE PLATE AT BOTTOM AND A DOUBLE PLATE AT TOP OF ALL STUD WALLS.
10. STUDS SHALL BE DOUBLED AT ALL ANGLES AND AROUND ALL OPENINGS. STUDS SHALL BE TRIPLED AT ALL CORNERS.

PRE-ENGINEERED WOOD TRUSSES

1. PRE-ENGINEERED WOOD TRUSSES SHALL COMPLY WITH THE REQUIREMENTS OF THE FOLLOWING CODES AND STANDARDS (LATEST EDITIONS):
- a. NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION.
- b. DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSS PUBLISHED BY THE TRUSS PLATE INSTITUTE (AN81/TPI).
- c. COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES (HIB).
- d. AMERICAN FOREST AND PAPER ASSOCIATION (APFA).

- e. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC).
- f. ALL REQUIREMENTS OF THE TRUSS PLATE INSTITUTE (TPI).
2. DESIGN, FABRICATION, FIELD STORAGE, HANDLING, AND INSTALLATION OF PRE-ENGINEERED TRUSSES SHALL BE THE JOINT RESPONSIBILITY OF THE TRUSS MANUFACTURER AND THE DELEGATE REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA HIRED BY THE TRUSS MANUFACTURER TO GENERATE SIGNED AND SEALED DOCUMENTS FOR THE DESIGN, DETAILING, AND PREPARATION OF TRUSS LAYOUTS.
3. STRUCTURAL WOOD OF THE TRUSS MEMBERS SHALL BE SOUTHERN PINE, GRADE No. 2 OR BETTER WITH THE FOLLOWING MINIMUM PROPERTIES:
- Fb = 1350 PSI
Fv = 175 PSI
Fc = 1600 PSI
E = 1,400,000 PSI
MAX. MOISTURE CONTENT = 19%
4. MAXIMUM DEFLECTION PERMITTED, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:
LIVE LOAD = L/360
TOTAL LOAD = L/240
5. TRUSS ERECTOR SHALL BE RESPONSIBLE FOR THE TEMPORARY BRACING OF THE TRUSSES.
6. THE DELEGATE ENGINEER SHALL PROVIDE ALL CONNECTIONS, HANGERS, HEADER BEAMS, SIZE AND LOCATION OF ALL REQUIRED TOP AND BOTTOM CHORD BRACING IN ORDER TO COMPLETE THE ROOF AND FLOOR FRAMING.
7. CONTRACTOR SHALL PROVIDE APPROVED HURRICANE CLIPS AND STRAPS TO RESIST UPLIFT FORCES SHOWN ON THE TRUSS SHOP DRAWINGS.
8. ALL HARDWARE SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS INCLUDING PROPER SIZE, NUMBER, LOCATION AND INSTALLATION OF THE REQUIRED HARDWARE.
9. ALL CONNECTORS, TIE-DOWNS, STRAPS, ETC. SHALL BE HOT DIPPED GALVANIZED.

POST-INSTALLED ANCHORS

1. POST-INSTALLED ANCHORS SHALL COMPLY WITH THE REQUIREMENTS OF THE FOLLOWING CODES AND STANDARDS (LATEST EDITIONS):
- a. QUALIFICATION OF POST-INSTALLED MECHANICAL ANCHORS IN CONCRETE & COMMENTARY (ACI 308.2).
- b. INTERNATIONAL CODE COUNCIL EVALUATION SERVICE (ICC-ES).
2. THE SPECIFIED PRODUCTS ARE THE DESIGN BASIS FOR THIS PROJECT. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW MAY BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD (EOR) FOR REVIEW. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A CODE REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION. SUBSTITUTION REQUESTS SHALL INCLUDE CALCULATIONS THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT. CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE FOR PRODUCT INSTALLATION TRAINING AND A LETTER SHALL BE SUBMITTED TO THE EOR INDICATING TRAINING HAS TAKEN PLACE. SPECIAL INSPECTIONS ARE REQUIRED PER THE IBC AND ICC-ES REPORTS.
3. FOR ANCHORING INTO CONCRETE
- a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI308.2 AND ICC-ES AC308.2. PRE-APPROVED ANCHORS INCLUDE:
- i. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2719).
- ii. SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-3037).
- b. ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI308.4 AND ICC-ES AC308. PRE-APPROVED ANCHORS INCLUDE:
- i. SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508).
4. FOR ANCHORING INTO SOLID-GROUTED CONCRETE MASONRY
- a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR ICC-ES AC106. PRE-APPROVED ANCHORS INCLUDE:
- i. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056).
- ii. SIMPSON STRONG-TIE "WEDGE-ALL" (ICC-ES ESR-1936).
- b. ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC038. PRE-APPROVED ANCHORS INCLUDE:
- i. SIMPSON STRONG-TIE "SET" (ICC-ES ESR-1172).
5. FOR ANCHORING INTO HOLLOW CONCRETE MASONRY
- a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC106. PRE-APPROVED ANCHORS INCLUDE:
- i. SIMPSON STRONG-TIE "TITEN-HD".
- b. ADHESIVE ANCHORS WITH SCREEN TUBES SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC038. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER. PRE-APPROVED ANCHORS WITH SCREEN TUBES INCLUDE:
- i. SIMPSON STRONG-TIE "SET" (ICC-ES ESR-1172).

FLORIDA APPROVAL NUMBERS

1. THE FOLLOWING TABLE REPRESENTS THE FLORIDA APPROVAL NUMBERS FOR THE ANCHORS SHOWN ON THE STRUCTURAL DRAWINGS:

PRODUCT	FL APPROVAL #
A23	10446.2
A35	114781/10446.4
HU528	10655.96
HGA10	11470.4
HGU510	11460
HTS191	10456.33
HTS16	10456.22
H8	11470.3
TITEN ANCHORS	2355.1

2. CONTRACTOR TO VERIFY AND SUBMIT TO THE AUTHORITY HAVING JURISDICTION (A41) ANY MISSING FL APPROVAL NUMBERS FOR PRODUCTS NOT LISTED AND/OR SUBSTITUTED IN THIS TABLE.

Mouchir
Chenouda

Digitally signed by Mouchir
Chenouda
DN: c=US, o=Creative Structures
Engineering LLC,
ou=A01410C0000016F1AF4EA91
000016C8, cn=Mouchir
Chenouda
Date: 2022.04.14 09:29:00 -04'00'

REVISIONS	BY

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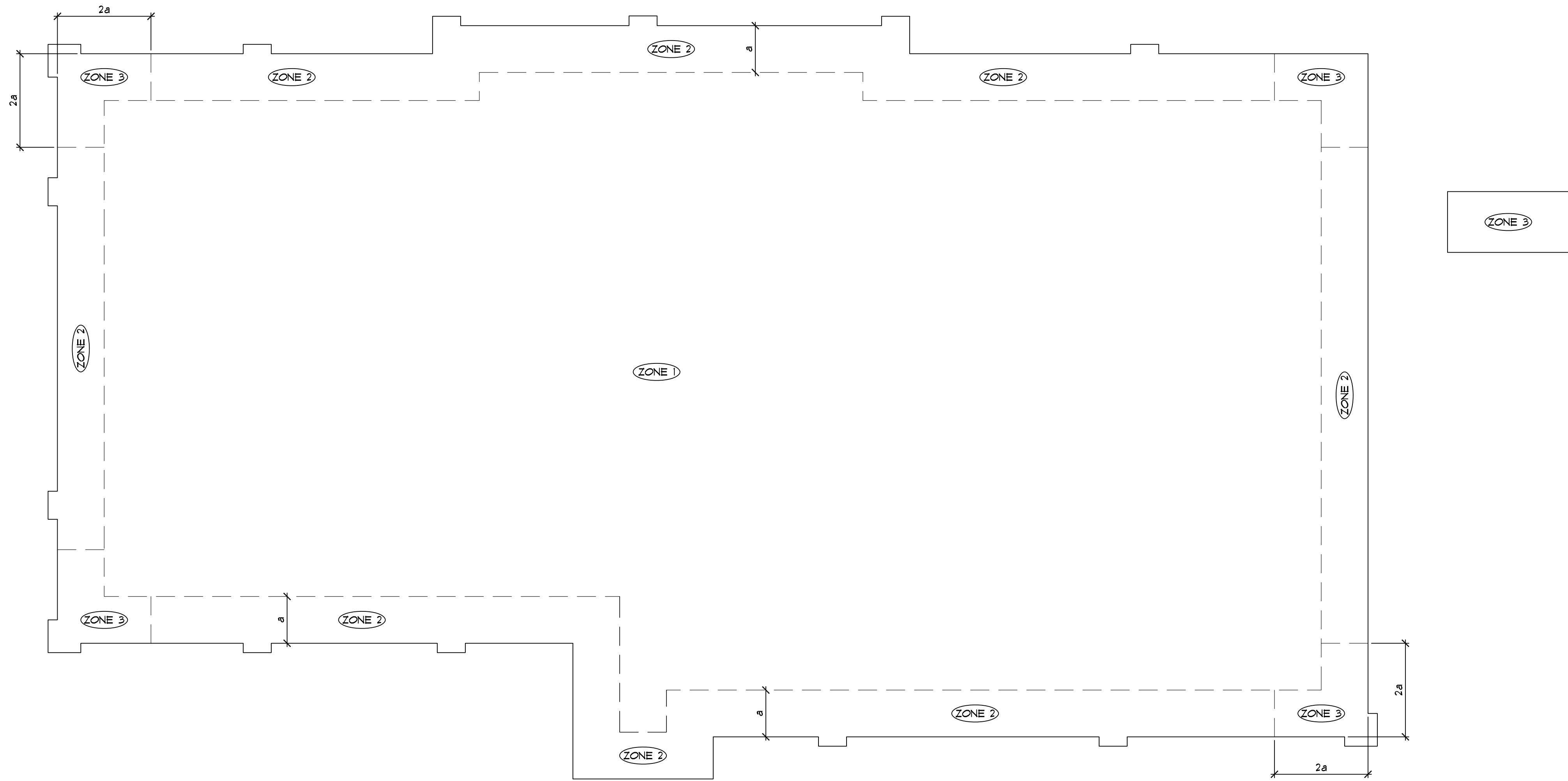
STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 69180
★ MOUCHIR CHENOUDA ★
PE # 69180

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RG # 28855

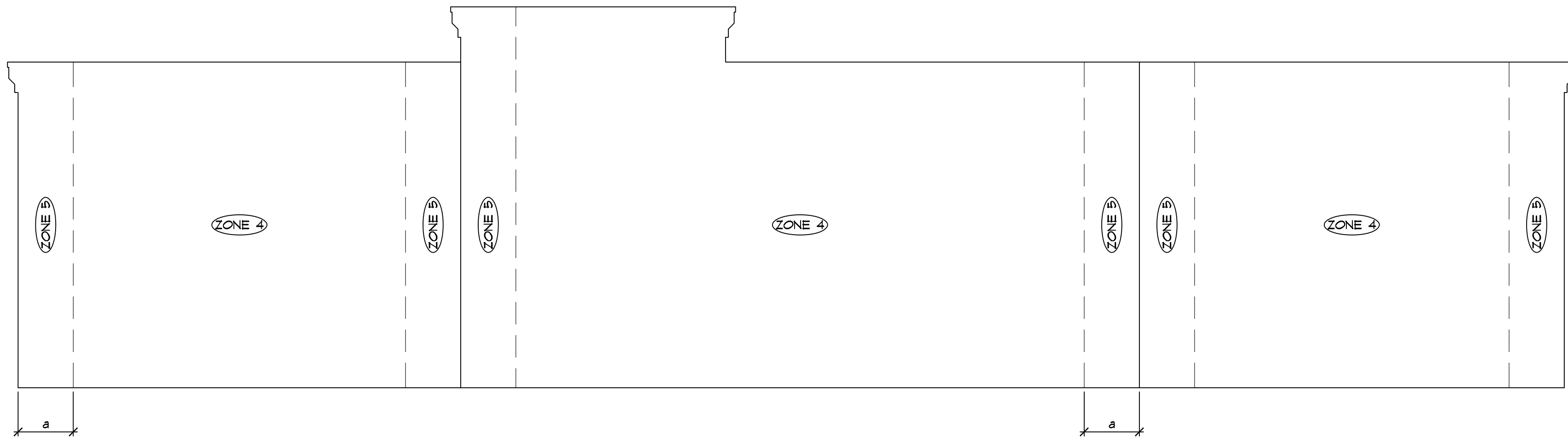
New Free Standing
US Hwy 90
Lake City, Florida
Columbia County

Date: 04. 13. 22
Scale: AS NOTED
Project Mgr: MSC
Drawn:
Job: 21-244
Sheet
S1a

NOTE:
THE SHOWN SCHEMATICS ARE ONLY
REPRESENTATIVE OF THE STRUCTURE.
SEE PLANS FOR LAYOUT AND DIMENSIONS.



END ZONE WIDTH (a) = 5.4 FT.



END ZONE WIDTH (a) = 5.4 FT.

COMPONENTS & CLADDING WIND PRESSURES

ZONE	TRIBUTARY AREA (SQ. FT.)	POSITIVE PRESSURE (PSF)	NEGATIVE PRESSURE (PSF)
ZONE 1 (ROOF)	10	16.00	-50.69
	20	16.00	-47.35
	50	16.00	-42.93
	100	16.00	-39.58
ZONE 2 (ROOF)	10	29.12	-66.87
	20	27.83	-62.57
	50	26.13	-56.89
	100	24.84	-52.59
ZONE 3 (ROOF)	10	29.12	-66.87
	20	27.83	-62.57
	50	26.13	-56.89
	100	24.84	-52.59
ZONE 2 (OVERHANG)	10	-	-78.02
	20	-	-72.28
	50	-	-64.70
	100	-	-58.37
ZONE 3 (OVERHANG)	10	-	-102.28
	20	-	-92.25
	50	-	-78.99
	100	-	-68.95
ZONE 4 (WALLS)	10	29.12	-31.55
	20	27.83	-30.26
	50	26.13	-28.55
	100	24.84	-27.26
	500	21.84	-24.27
ZONE 5 (WALLS)	10	29.12	-38.83
	20	27.83	-36.25
	50	26.13	-32.84
	100	24.84	-30.26
	500	21.84	-24.27

ROOF FASTENING SCHEDULE

ZONE	NAIL SIZE	NAIL SPACING	
		AT PANEL EDGES	AT INTERMEDIATE SUPPORTS
ZONE 1	10d	3" o.c.	8" o.c.
ZONE 2	10d	3" o.c.	6" o.c.
ZONE 3	10d	3" o.c.	6" o.c.

FOR ROOF SHEATHING PANELS LESS THAN 24" IN WIDTH, PROVIDE, PER APA TECHNICAL NOTE #215A, THE FOLLOWING:

FOR PANEL WIDTHS GREATER THAN 16" BUT LESS THAN 24" USE DOUBLE PLY CLIPS ON THE SIDE ADJACENT TO THE FULL WIDTH PANEL - NONE REQUIRED AT RIDGE OR HIP.

FOR PANEL WIDTHS GREATER THAN 12" BUT LESS THAN 16" USE LUMBER BLOCKING ONLY (2x4) FLATWISE OR EDGEWISE ON THE SIDE ADJACENT TO THE FULL WIDTH PANEL - NONE REQUIRED AT RIDGE OR HIP.

FOR PANEL WIDTHS LESS THAN 12" USE LUMBER BLOCKING ONLY (2x4) FLATWISE OR EDGEWISE ON BOTH PANEL EDGES.

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S1b

FOUNDATION PLAN NOTES

1. COORDINATE ALL DIMENSIONS w/ ARCH. DWGS. ALL MASONRY DIMENSIONS SHOWN ARE NOMINAL.

2. CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS RELATED TO THE BUILDING PRIOR FABRICATION AND CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY SHORING AND BRACING.

3. CONTRACTOR SHALL INFORM ENGINEER OF RECORD OF ANY DISCREPANCIES IN THE FIELD FROM THE ASSUMPTIONS STATED ON THESE DRAWINGS.

4. CENTER ALL FOOTINGS BELOW COLUMNS, WALLS AND PANELS UNO. REFER TO FOOTING SCHEDULE FOR SIZE AND REINFORCEMENT. EXTEND WALL FOOTINGS FOR BUMP-OUTS 8'-8" MIN. BEYOND SUPPORTING CMU KNEE WALL.

5. SLAB ON GRADE TO BE 4" THICK CONCRETE SLAB w/ 6 x 6 x W14 x W14 WJWF PLACED AT 1" MAX. FROM TOP OF SLAB AND SUPPORTED WITH CHAIRS AT SPACING NOT TO EXCEED 3'-0" ON 10 MIL. VAPOR RETARDER OVER TERMITES TREATED (AS PER FBC) AND WELL COMPACTED SOIL UNO.

6. UNLESS SHOWN OTHERWISE ON THE FOUNDATION PLAN, 1/2" EXPANSION JOINTS SHALL BE PROVIDED IN THE SLAB ON GRADE EVERY 100'-120' SQ. FT. MAX. REFER TO DETAILS 1/53a AND 2/53a.

7. TOP OF SLAB ELEVATION = ± 0'-0" UNO.

8. STEP FOOTINGS WITH GRADE AS REQUIRED TO MAINTAIN MIN. SOIL COVERAGE OVER ALL EXTERIOR FOOTINGS. REFER TO CIVIL DWGS. FOR FINISH GRADE. CONTRACTOR TO FIELD VERIFY FOOTINGS ELEVATIONS. REFER TO 9/53a FOR STEPPED FOOTING DETAIL.

9. REFER TO CMU WALL LEGEND SCHEDULE FOR WALL THICKNESS AND REINFORCEMENT. ALL CMU WALLS BELOW
- GRADE SHALL BE GROUTED SOLID PRIOR TO BACKFILLING AND COMPACTION.

10. PROVIDE FOUNDATION CORNER BARS AT ALL CORNERS, INTERSECTIONS AND EACH SIDE OF OPENINGS AND CMU CONTROL JOINTS. REFER TO DETAILS 11/53a AND 19/53a FOR MORE INFORMATION.

11. PROVIDE (2) #4 x 4'-0" AT MID DEPTH OF SLAB ON GRADE AT DOOR AND STOREFRONT ENTRY CORNERS (TYP.). REFER TO DETAIL 10/53a FOR MORE INFORMATION.

12. REFER TO ARCH. DWGS. FOR SIZE AND LOCATION OF MASONRY WALL OPENINGS (M.O.). REFER TO DETAIL 11/53a FOR ADD'L REINFORCEMENT INFORMATION AT OPENINGS.

13. REFER TO ARCH. DWGS. FOR LOCATIONS OF CMU CONTROL JOINTS. CMU CONTROL JOINTS SHALL BE LOCATED AT 25'-0" o.c. (MAX.) AND 4'-0" o.c. (MIN.) FROM OPENING EDGE. REFER TO DETAILS 11/53a AND 12/53a FOR MORE INFORMATION.

14. WALL SHEATHING SHALL BE 5/8" APA RATED CDX PLYWOOD. FASTEN WALL SHEATHING TO WOOD STUDS w/ 8d NAILS SPACED AT 4" o.c. (MIN.) AT PANEL EDGES AND 8" o.c. (MIN.) AT PANEL FIELD.

15. COORDINATE FLOOR FINISHES, WATERPROOFING REQUIREMENTS, ETC w/ ARCH. DWGS.

16. REFER TO ARCH. / PLUMBING DWGS. FOR FIXTURES AND DRAINS LOCATIONS AND INFORMATION.

17. REFER TO ARCH. / CIVIL DWGS. FOR EXTERIOR SLAB AND SIDEWALK INFORMATION.

18. REFER TO DETAIL 3/53c FOR PIPE/CONDUIT PENETRATIONS AND LOUVER OPENINGS. COORD EXACT LOCATIONS w/ ARCH, MEP, AND INVOLVED TRADES.

FOOTING SCHEDULE				
MARK	SIZE	DEPTH	REINFORCEMENT	REMARKS
F-1	2'-0" x CONT.	2'-0"	(3) #5 CONT. BOTTOM BAR w/ (1) #5 @ 24" o.c. TRANS. BAR	WALL FOOTING
F-2	2'-8" x CONT.	2'-0"	(4) #5 CONT. BOTTOM BAR w/ (1) #5 @ 16" o.c. TRANS. BAR	WALL FOOTING

CMU WALL LEGEND SCHEDULE		
LEGEND	VERTICAL REINFORCEMENT	REMARKS
	8" CMU WALL w/ #5 VERTICAL @ 32" o.c. MAX.	SEE PLAN, NOTES AND DETAILS FOR LOCATION
	8" CMU WALL w/ #5 VERTICAL @ 40" o.c. MAX.	SEE PLAN, NOTES AND DETAILS FOR LOCATION
	8" CMU WALL. MATCH REINF. w/ ADJACENT WALL	CMU WALL BELOW & ABOVE OPENING

- NOTES:
1. CMU WALL CONSTRUCTION SHALL BE RUNNING-BOND CONSTRUCTION AND VERTICALLY REINFORCED ACCORDING TO THE SCHEDULE.

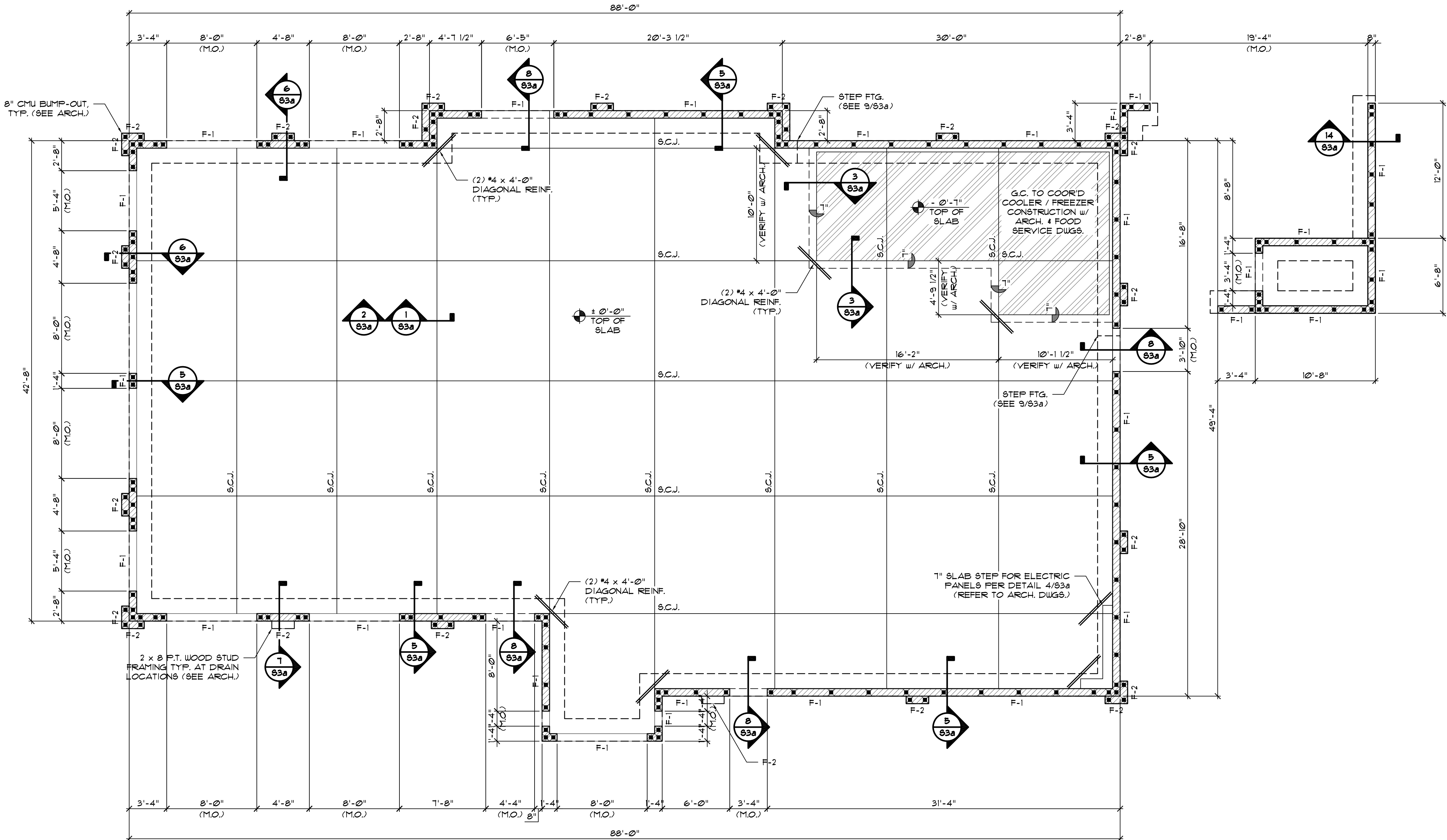
2. REINFORCEMENT BARS SHALL BE PLACED IN GROUT-FILLED CELLS. SPACING OF VERTICAL REINFORCEMENT BARS SHALL NOT EXCEED 4'-0" o.c.

3. HORIZONTAL CMU WALL REINFORCEMENT SHALL BE 9 GA. SPACED AT 16" o.c.

4. VERTICAL REINFORCEMENT BELOW AND ABOVE OPENINGS SHALL MATCH ADJACENT WALLS VERTICAL REINFORCEMENT.

5. CMU WALL REINFORCEMENT SHALL BE DOUELED INTO FOOTINGS AND TIE BEAMS / LINTELS.

6. ALL CMU WALL BELOW GRADE SHALL BE FULLY GROUTED.



FOUNDATION PLAN
SCALE: 3/8" = 1'-0"

REVISIONS

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S2a

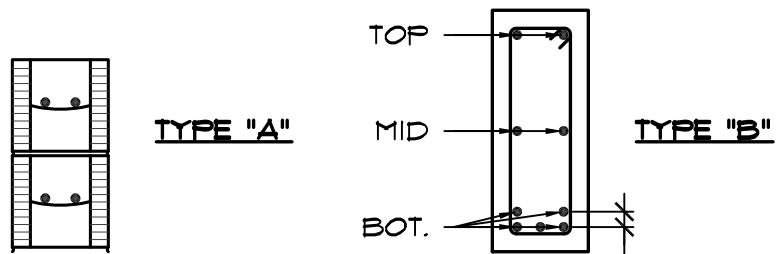
ROOF FRAMING PLAN NOTES

- COORDINATE ALL DIMENSIONS w/ ARCH. DWGS. AND FOUNDATION PLAN.
- CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS RELATED TO THE BUILDING PRIOR FABRICATION AND CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY SHORING AND BRACING.
- CONTRACTOR SHALL INFORM ENGINEER OF RECORD OF ANY DISCREPANCIES IN THE FIELD FROM THE ASSUMPTIONS STATED ON THESE DRAWINGS.
- COORDINATE ROOF SLOPES, TRUSS PROFILES, FINISHES, DRAINAGE AND WATERPROOFING w/ ARCH. DWGS.
- CONTRACTOR TO COORDINATE TOP OF ROOF TRUSSES ELEVATION AND SLOPES w/ TRUSS MANUFACTURER DWGS. (VERIFY w/ ARCH. DWGS.).
- (CB-X) AND (TB-X) DENOTE CONCRETE BEAM AND TIE-BEAM TYPE RESPECTIVELY. REFER TO CONCRETE BEAM / TIE-BEAM SCHEDULE FOR SIZE AND REINFORCEMENT. BEAMS SHALL BE EXTENDED TO BEAR ON THEIR FULL ENTIRE SUPPORT WHETHER CONCRETE COLUMN OR CMU FILLED CELLS / PILASTER COLUMN.
- TOP OF TIE-BEAM / CMU WALL = TRUSS BEARING (42" DEEP + 30" DEEP w/ 12" HEEL) = 10'-8" UNO. VERIFY w/ ARCH. DWGS.
- (L-X) DENOTES LINTEL TYPE. REFER TO DETAIL 4/93c FOR LINTEL SCHEDULE SHOWING SIZE AND REINF. PROVIDE (1) K.O. COURSE w/ (1) #5 FOR 8" CMU WALL BELOW OPENINGS. IN CASE OF (TB-X) AND (L-X) REINF OVERLAP, HIGHER REINF IN BOTH SHALL PREVAIL IN EACH K.O. COURSE.
- STUD WALL HATCH DENOTES WALL HEIGHT. REFER TO CMU WALL ELEVATION SCHEDULE FOR WALL LEGEND ELEVATIONS.
- PROVIDE BEAM CORNER BARS AT ALL CORNERS AND INTERSECTIONS. REFER TO DETAIL 13/93a FOR MORE INFO.
- PRE-ENGINEERED WOOD ROOF TRUSSES SHALL BE DESIGNED FOR THE DEAD, LIVE, AND WIND LOADS SPECIFIED AND ALL COMBINATIONS OF THESE LOADS. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA FOR REVIEW PRIOR TO TRUSS FABRICATION.
- THE TRUSS ENGINEER SHALL DESIGN THE ROOF TRUSSES FOR MAIN WIND FORCE RESISTING SYSTEM AS WELL AS FOR COMPONENTS AND CLADDING. REFER TO NET WIND UPLIFT PLAN ON SHEET 91b.
- ROOF SHEATHING SHALL BE 3/4" APA RATED CDX PLYWOOD. FASTEN ROOF SHEATHING TO ROOF TRUSSES w/ 10d NAILS AS SHOWN IN SHEET 91b.
- PERMANENT TRUSS BRACING AND BRIDGING SHALL BE PROVIDED IN ACCORDANCE WITH "TRUSS PLATE INSTITUTE" IN ORDER TO PROVIDE ADEQUATE LATERAL WIND STABILITY FOR THE TRUSS SYSTEM. THE TRUSS ENGINEER SHALL INCLUDE THESE CALCULATIONS AS PART OF THE SHOP DRAWINGS SUBMITTAL. REFER TO DETAILS 6/93c, 9/93b AND 6/93b FOR MORE INFORMATION.
- COORDINATE w/ ARCHITECTURAL AND MECHANICAL ROOF FRAMING PLANS FOR ADDITIONAL DEAD LOADS ON TOP OR BOTTOM CHORDS OF TRUSSES DUE TO MECHANICAL EQUIPMENTS, WHERE APPLICABLE. COORDINATE SIZE, WEIGHT AND LOCATION OF ROOF TOP UNITS (RTUs) AND MECHANICAL EQUIPMENT w/ MECH. AND ARCH. DWGS. REFER TO DETAIL 7/93b FOR RTU / CURB / TRUSS ATTACHMENT. CONTRACTOR TO COORDINATE RTU CURB ATTACHMENT WITH RTU MANUF. PRE-ENGINEERED WOOD TRUSS DELEGATE ENGINEER, MECH. AND ARCH. DWGS.

- POINT LOADINGS WHICH ARE GENERATED BY SPECIAL TRUSS CONFIGURATIONS HAVING TRIBUTARY WIDTHS GREATER THAN 2'-0" (HIP TRUSSES, VALLEY TRUSSES, TRANSFER TRUSSES, ETC.) SHALL BE ACCOUNTED FOR IN TRUSS DESIGN. MAGNITUDE OF SUCH POINT LOADS SHALL BE CLEARLY NOTED ON SHOP DRAWINGS. TRUSSES SHALL ACCEPT POINT LOADS FROM ADJACENT TRUSSES AT TRUSS PANEL POINTS ONLY.
- TRUSS SHOP DRAWINGS SHALL BE REVIEWED AND COORDINATED w/ ARCHITECTURAL AND MECHANICAL ROOF FRAMING PLANS FOR ADDITIONAL DEAD LOADS ON TOP OR BOTTOM CHORDS OF TRUSSES DUE TO MECHANICAL EQUIPMENTS, WHERE APPLICABLE.
- PRE-ENGINEERED WOOD TRUSS MANUFACTURER SHALL PROVIDE ALL CONNECTIONS FOR COMPLETE TRUSS PACKAGE INCLUDING BRIDGING, BRACING, WEB STIFFENERS, ETC.
- REFER TO TIE-DOWN CONNECTOR SCHEDULE FOR CONNECTOR DESIGNATIONS AS MANUFACTURED BY "SIMPSON STRONG-TIE". EQUIVALENT PRODUCTS WILL BE ACCEPTED w/ PRIOR APPROVAL OF THE ENGINEER OF RECORD PROVIDED THEY ARE OF EQUAL OR GREATER CAPACITY. MOISTURE BARRIER SHALL BE INSTALLED PER MANUFACTURER RECOMMENDATIONS.
- THE CONTRACTOR SHALL VERIFY THAT THE TIE-DOWNS LISTED IN THE SCHEDULE MEET THE UPLIFT REACTIONS SHOWN ON THE TRUSS SHOP DRAWINGS.
- UNLESS NOTED OTHERWISE ON THE PLANS OR NOTES, ALL NAILING, AT A MINIMUM, SHALL BE IN ACCORDANCE TO THE APPLICABLE BUILDING CODE.
- REFER TO NET WIND UPLIFT PLAN ON SHEET 91b.

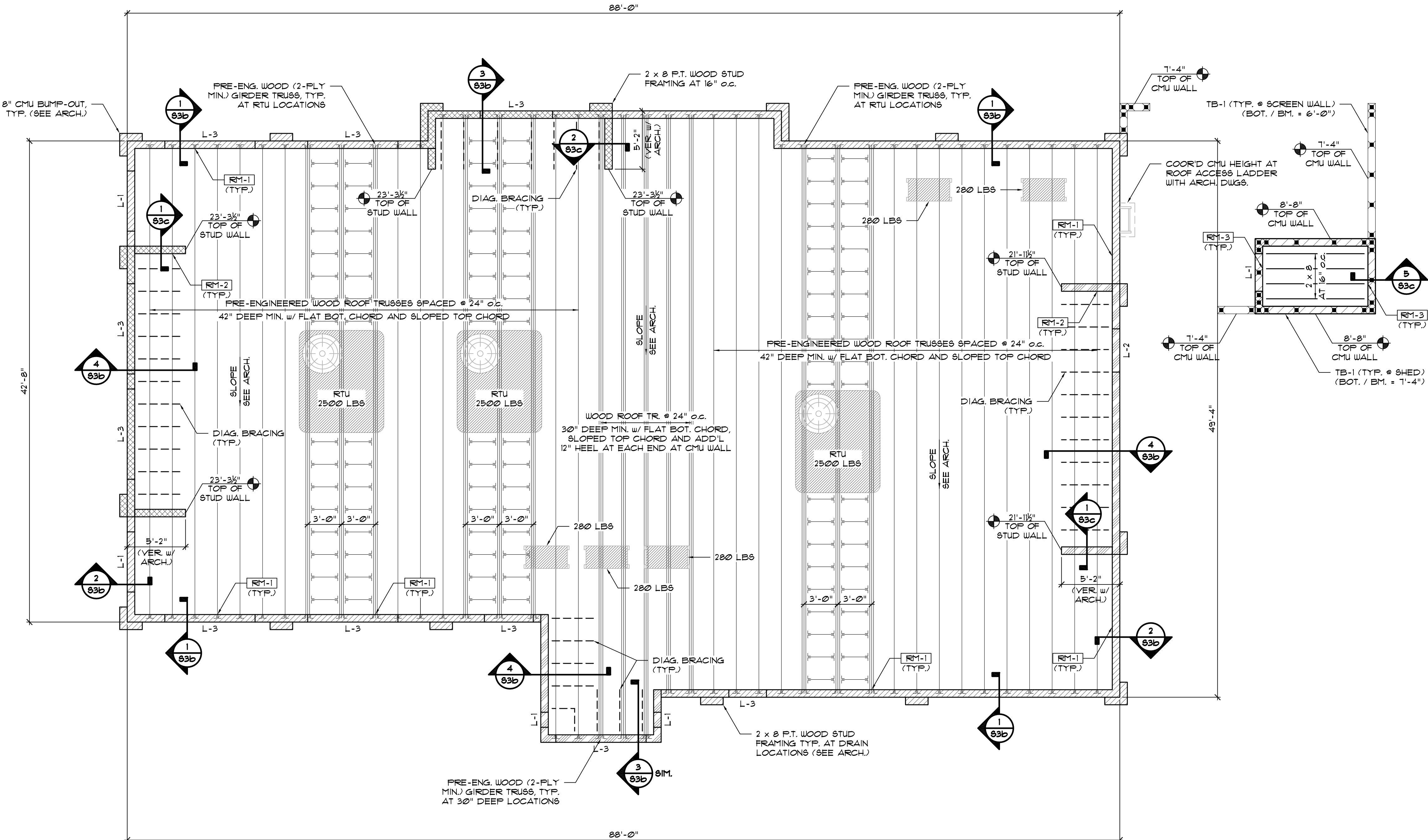
STUD WALL ELEVATION SCHEDULE		
LEGEND	STUD WALL ELEVATION	REMARKS
	TOP OF STUD WALL = 19'-1 1/2"	BUMP-OUT TO MATCH
	TOP OF STUD WALL = 21'-1 1/2"	BUMP-OUT TO MATCH
	TOP OF STUD WALL = 23'-3 1/2"	BUMP-OUT TO MATCH

CONCRETE BEAM / TIE-BEAM SCHEDULE						
MARK	WIDTH	DEPTH	REINFORCEMENT			REMARKS
			TOP	MID	BOTTOM	
TB-1	8"	16"	(2) #5	-	(2) #5	(2) K.O. BLOCK GROUT FULLY



TRUSS TIE-DOWN CONNECTOR SCHEDULE (SIMPSON STRONG-TIE)			
MARK	TYPE	REMARKS	UPLIFT CAPACITY
RM-1	H10S, HGA10 4 MTS12	TYPICAL ROOF TRUSS (1-PLY) CONNECTOR TO CONC. BEAM AND P.T. IE	2915 LBS
	LGT-2/3, HGA10 4 MTS12	TYPICAL ROOF TRUSS (MULTI-PLY) CONNECTOR TO CONC. BEAM AND P.T. IE	4000 LBS
RM-2	MTS12	TYPICAL BRACE CONNECTOR TO STUD WALL	2330 LBS
	HTS16	TYPICAL ROOF TRUSS CONNECTOR TO BRACE AND STUD WALL	1235 LBS
RM-3	H8	TYPICAL BOTTOM CHORD ROOF TRUSS CONNECTOR TO STUD WALL	1235 LBS
	HU28	TYPICAL P.T. WOOD RAFTER CONNECTOR TO CMU WALL	160 LBS

ROOF FRAMING PLAN SHOWN IS CONCEPTUAL. FINAL LAYOUT AND DESIGN SHALL BE PROVIDED BY THE ROOF TRUSS SYSTEM ENGINEER INCLUDING ALL BRACING, BRIDGING, HANGERS, CONNECTIONS, TIE-DOWNS, ETC. A COMPLETE ROOF TRUSS SYSTEM DWGS. AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA AND SUBMITTED FOR REVIEW.



1 ROOF FRAMING PLAN
SCALE: 3/8" = 1'-0"

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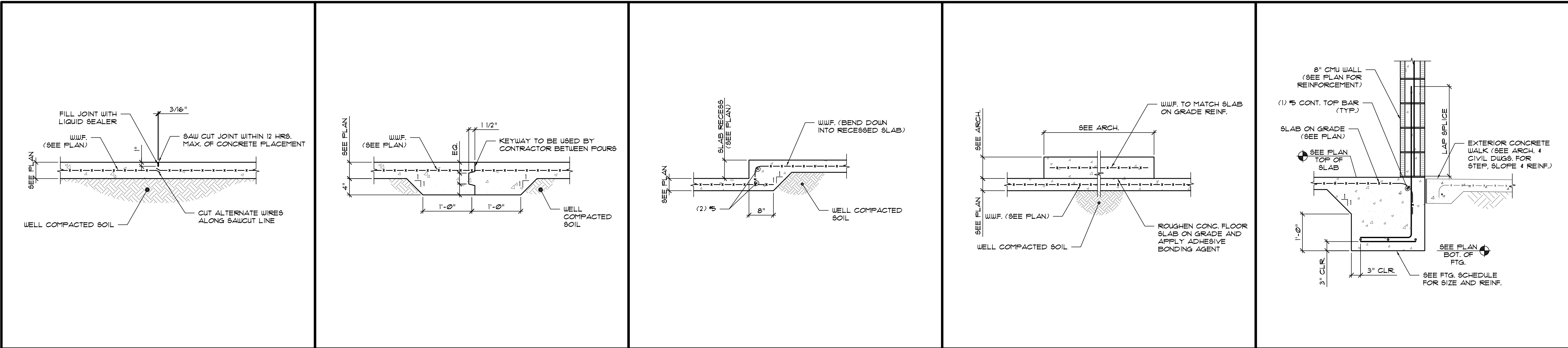
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S2b



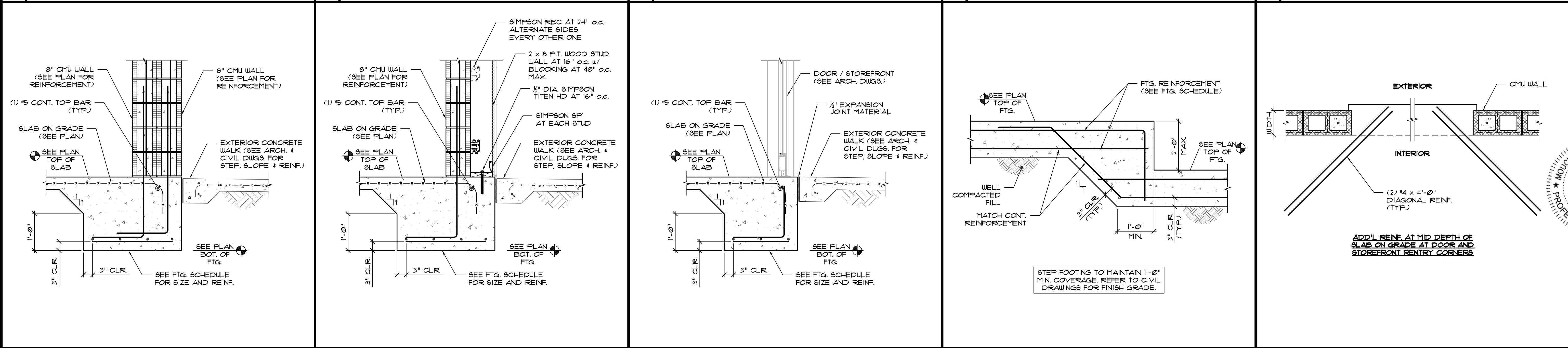
1 DETAIL SCALE: 1" = 1'-0"

2 DETAIL SCALE: 1" = 1'-0"

3 DETAIL SCALE: 3/4" = 1'-0"

4 DETAIL SCALE: 3/4" = 1'-0"

5 DETAIL SCALE: 3/4" = 1'-0"



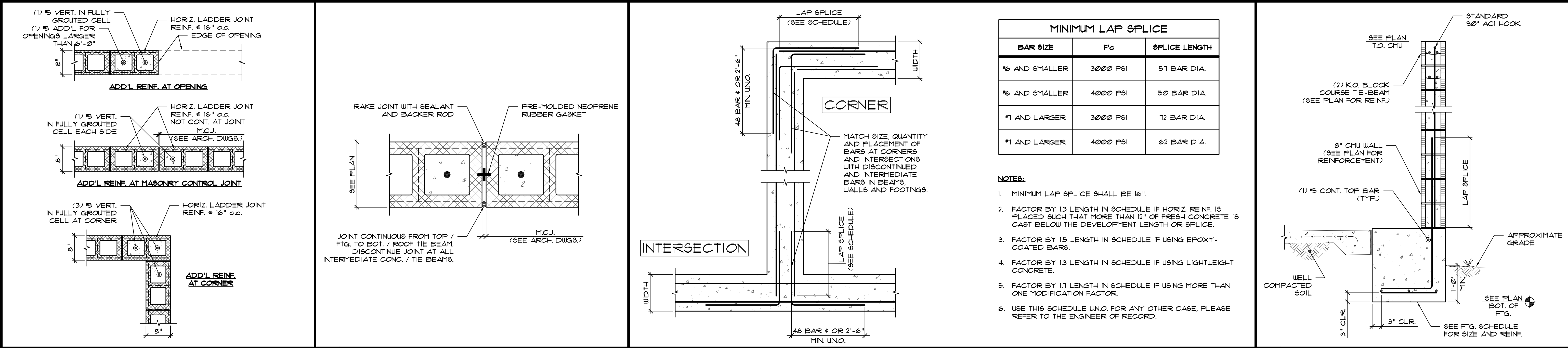
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7 DETAIL SCALE: 3/4" = 1'-0"

8 DETAIL SCALE: 3/4" = 1'-0"

9 DETAIL SCALE: 3/4" = 1'-0"

10 DETAIL SCALE: 3/4" = 1'-0"



11 DETAIL SCALE: 3/4" = 1'-0"

12 DETAIL SCALE: 2" = 1'-0"

13 DETAIL SCALE: N.T.S.

14 DETAIL SCALE: 3/4" = 1'-0"

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1 **DETAIL**
SCALE: $\frac{3}{4}" = 1'-0"$



2 DETAIL
SCALE: $\frac{3}{4}" = 1'-0"$



3 DETAIL
SCALE: $\frac{3}{4}" = 1'-0"$



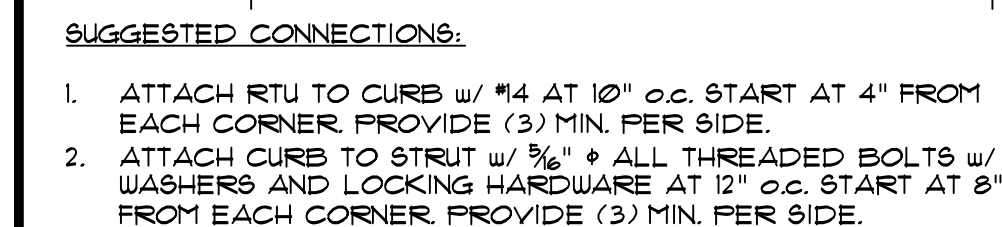
4 DETAIL
SCALE: $\frac{3}{4}" = 1'-0"$



5 DETAIL
SCALE: N.T.S.



6 DETAIL
SCALE: N.T.S.



7 DETAIL
SCALE: N.T.S.

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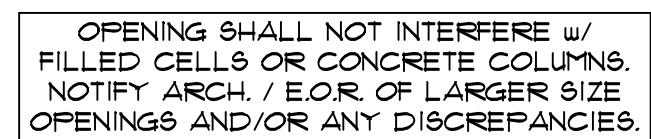
DETAIL
SCALE: $\frac{3}{4}" = 1'-0"$



DETAIL
SCALE: N.T.S.

NOTES:

-
- Diagram illustrating the components and dimensions of a Type "B" arch lintel:
- K.O. BLOCKS FULLY GROUTED (SEE SCHEDULE FOR REIN.)**: Points to the top and bottom blocks of the arch.
 - 8" PRECAST / PRESTRESSED LINTEL w/ (1) 5 FULLY GROUTED**: Points to the central precast arch section.
 - SEE ARCH. BOT. OF LINTEL**: Points to the bottom of the arch section.
 - DEPTH**: Dimension indicating the vertical height of the arch section.
 - WIDTH**: Dimension indicating the horizontal width of the arch section.
 - TYPE "B"**: Label for the arch lintel type.



DETAIL
SCALE: $\frac{3}{4}" = 1'-0"$



DETAIL
SCALE: N.T.S.

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