

TYPICAL 2 STORY HEADER STRAPING DETAIL

-1/2" ANCHOR 48" OC MAX

(TYP.) HEADER

ONE STORY WOOD FRAME w/ RODS

IN ALL OPENINGS OVER 48"

OPTION: 1 (FLUSH HEADER)

(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6

5'-3" 7'-9" 7'-8" 11'-4"

4'-4" 6'-6" 6'-5" 9'-6"

4'-0" 6'-0" 5'-11" 8'-9" DIVIDED B3Y (H/10)

WIND SPEED

90-100 MPH

110-120 MPH

ANCHOR TABLE

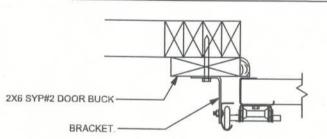
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
	,	HEAVY GIRDER TIEDOWNS*	1, 100	14-104	TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		O/O AD
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AP
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	- 127		12 100		1/2" AB

2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG

SCREWS w/ 1" WASHER LAG SCREWS MAY BI COUNTERSUNK. HORIZONTAL JAMBS DO NOT RANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

OOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS		
8' - 10'	24" OC	5" OC	5" OC		
11' - 15'	18" OC	4" OC	4" OC		
16' - 18'	16" OC	3" OC	3" OC		



(TYP.) GARAGE DOOR BUCK INSTALLATION

MAX GIRDER AND	GIRDERS &		READER	BUILD	ING	WIDTH /	RAF	TER-TR	USS	SPAN (F	T)
HEADERS SPANS	HEADERS		SPANS	12		20		28		36	
	SUPPORTING		(FT-IN)	SPAN	NJ	SPAN	NJ	SPAN	NJ.	SPAN	N.
FOR EXTERIOR	ROOF &		2-2x4	4-2	1	3-6	1	3-2	1	2-10	1
BEARING WALLS	CEILING		2-2x6	6-1	1	5-5	1	4-8	1	4-2	1
MAX SPANS FOR #2 GRADE	(30 LB GROUND		2-2x8	7-8	2	6-10	1	5-11	2	5-4	2
DOUGLAS FIR-LARCH.	SNOW LOAD)		2-2×10	9-4	2	8-5	2	7-3	2	6-6	2
HEM-FER, SOUTHERN PINE.			2-2x12	10-10	2	9-9	2	8-5	2	7-6	2
& SPRUCE-PINE-FIR			3-2x8	9-7	1	8-4	1	7-5	1	6-8	1
AND REQUIRED NUMBER		_	3-2x10	11-9	2	10-6	1	9-1	2	8-2	2
OF JACK STUDS			3-2x12	13-7	2	12-2	2	10-7	2	9-5	2
BASED ON:	ROOF, CEILING		2-2x6	5-1	2	4-6	1	4-0	1	3-7	2
RC TABLE R502.5(1)	& ONE CENTER-		2-2x8	6-5	2	5-9	2	5-0	2	4-6	2
WFCM TABLE 3.22A-F	BEARING FLOOR		2-2x10	7-10	2	7-0	2	6-2	2	5-6	2
			2-2x12	9-1	2	8-1	2	7-1	2	6-5	2
NOTES:			3-2x8	8-0	1	7-2	1	6-3	2	5-8	2
- NJ = NUMBER OF JACK		SIZES	3-2x10	9-10	2	8-9	2	7-8	2	6-11	2
STUDS REQUIRED TO			3-2×12	11-4	2	10-2	2	8-11	2	8-0	2
SUPPORT EACH END.	ROOF, CEILING		2-2x6	4-9	2	3-11	1	3-5	2	3-0	2
BUILDING WIDTH IS	& ONE CLEAR- SPAN FLOOR	HEADER	2-2x8	6-0	2	5-0	2	4-4	2	3-10	2
MEASURED		EA	2-2×10	7-4	2	6-1	2	5-3	2	4-8	2
PERPENDICULAR TO THE		工	2-2x12	8-6	3	7-1	2	6-1	3	5-5	3
RIDGE. - FOR WIDTHS BETWEEN			3-2x8	7-6	2	6-3	2	5-5	2	4-10	2
THOSE SHOWN, SPANS			3-2x10	9-2	2	7-7	2	6-7	2	5-11	2
MAY BE INTERPOLATED.			3-2×12	10-8	2	8-10	2	7-8	2	6-10	2
- SPANS ARE BASED ON	ROOF, CEILING		2-2x8	5-7	2	4-8	2	4-2	2	3-9	2
UNIFORM LOADS ON	& TWO CENTER-		2-2×10	6-10	2	5-9	2	5-1	2	4-7	3
HEADER.	BEARING FLOOR		2-2x12	7-11	2	6-8	2	5-10	3	5-3	3
- AN EQUIVALENT			3-2x10	8-6	3	7-3	2	6-4	2	5-8	2
NUMBER OF FULL HEIGHT			3-2x12	9-11	3	8-5	2	7-4	2	6-7	2
STUDS ARE PERMITTED	ROOF, CEILING		2-2x8	4-8	2	3-10	2	3-4	2	3-0	3
TO REPLACE JACK STUDS,	& TWO CLEAR-		2-2x10	5-9	3	4-9	2	4-1	3	3-8	3
WHEN ADEQUATE	SPAN FLOOR		2-2x12	6-8	3	5-6	3	4-9	3	4-3	3
GRAVITY CONNECTIONS ARE PROVIDED.			3-2x10	7-2	2	5-11	2	5-1	2	4-7	3
ANE PROVIDED.			3-2x12	8-4	3	6-10	2	5-11	3	5-4	3

LOAD BEARING HEADER SIZING METHODS (BY BUILDER)

1. LOOKUP HEADER SIZE FROM HEADER SPAN TABLES OF THE REFERENCED CODE.

2. USE SUPPLIER PUBISHED DATA OR SOUTHERN PINE SPAN TABLES.

3. FOR ENGINEERED LUMBER BEAMS HAVE SUPPLIERS ENGINEER SIZE BEAM.

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JACK STUDS AND KING STUDS (BY BUILDER)

1. LOOKUP REQUIRED JACK STUDS FROM HEADER SPAN TABLES OF THE REFERENCED CODE.

2. USE ONE JACK STUD FOR EVERY 2000 LB VERTICAL LOAD.

3. TOTAL KING STUDS = OPENING WIDTH / STUD SPACING (60" OPENING / 16" OC = 4 = 2 KINGS EA. SIDE)

HEADER UPLIFT CONNECTIONS (BY BUILDER)

1. CALCULATE THE UPLIFT AT EACH END OF THE HEADER BY SUMMING THE MOMENTS OF ALL TRUSS /
RAFTER UPLIFTS AND DIVIDING BY THE LENGTH OF THE HEADER.

SELECT HEADER CONNECTIONS FROM DETAIL OR MFG. CATALOG TO CONNECT HEADER TO STUD (TOP CONNECTION) AND STUD TO FOUNDATION (BOTTOM CONNECTION).

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

TO 10'-6" STUD HEIGHT
TO 11'-7" STUD HEIGHT
TO 16'-10" STUD HEIGHT
TO 18'-7" STUD HEIGHT

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0
			1000000

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3"

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

DESIGN PRESSURES.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION 01.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF YSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

IVIE	CLOSED SIMPLE DIAPHRAGM BUILDINGS WI AN ROOF HEIGHT NOT EXCEEDING LEAST HO UPPER HALF OF HILL OR ESCARPMENT 60F	ORIZONTAI	DIME	NSION	OR 60	FT. N
SLC	PE AND UNOBSTRUCTED UPWIND FOR 50x I	HEIGHT OF	1 MIL	E WHIC	HEVE	RISL
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRI	CANE ZON	E			
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS F	REGION				
1.)	BASIC WIND SPEED = 110 MPH					
2.)	WIND EXPOSURE = C					
3.)	WIND IMPORTANCE FACTOR = 1.0					
4.)	BUILDING CATEGORY = II					
5.)	ROOF ANGLE = 10-45 DEGREES					-
6.)	MEAN ROOF HEIGHT = <30 FT					
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (I	ENCLOSED	BUILD	ING)		
8.)	COMPONENTS AND CLADDING DESIGN WIN	ID PRESSU	RES (1	ABLE F	R301.2	(2))
		Zone	Effective Wind Area (ff			
	*	Zone	спес	tive w	ing Ar	ea (n
	**************************************	Zone		tive w	_	ea (n 100
		1 1	1	77.5		100
		1 2	27.8	0		100
		1 2 2 O'hg	27.8 27.8	-30.5 -35.7 -56.8	25.3	
	2 2 2 2 2 2 5	1 2 2 O'hg 3	27.8 27.8	-30.5 -35.7 -56.8 -35.7	25.3	-25 -30. -56 -30.
		1 2 2 O'hg	27.8 27.8 27.8	-30.5 -35.7 -56.8	25.3 25.3	-25 -30 -56

Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 27.3 16x7 Garage Door 25.9 -29.4 DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY

5 30.5 -40.7 25.9 -31.6

Doors & Windows 30.5 -40.7

REVISIONS

SOFTPLAN

SCOPE OF WORK: This seal is for structural engineering for an addition to a single family residence. The structural work was partially complete at the time of this engineering. Portions of the structure including connections and nailing patterns were already covered. Therefore this engineering is

1) as built documentation based on discussion with the builders and personal observations, engineering analysis of existing member andconnections based on available information and 3) specification of future construction required to provide wind load stability to the completed structure. To the best of my knowledge this engineering is as accurate as possible without dissembling finished work. It is also quite apparent that the owner's intent was to construct this addition in full compliance with the structural requirement of the building code and not allow deficiencies in member sizes, material, or workmanship WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service This document is not to be reproduced, alteror copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2007 & 2009 supplements to the best of my knowledge. LIMITATION: This design is valid for one building, at specified location. P.E. 53915

Ganskop Jeff Addition ADDRESS: Southwood Acres Columbia County, Florida Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE: March 11, 2010 DRAWN BY: STRUCTURAL BY: Evan Beamsley Evan Beamsley

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DRAWING NUMBER

FINALS DATE:

S-1 OF 7 SHEETS