

DATE 06/20/2006

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000024644

APPLICANT LINDA RODER PHONE 752-2281
ADDRESS 387 SW KEP CT LAKE CITY FL 32024
OWNER DAVID & JERELLE HAUCK PHONE 623-0575
ADDRESS 188 NW AMBLESIDE DR LAKE CITY FL 32055
CONTRACTOR SPARKS CONSTRUCTION/JOSH SPARK PHONE 623-0575
LOCATION OF PROPERTY 90 W, R ON LAKE JEFFERY, R ON BRIDGEWATER, L AMBLESIDE DR,
2ND LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 183800.00
HEATED FLOOR AREA 3676.00 TOTAL AREA 5897.00 HEIGHT 24.00 STORIES 2
FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 12/12 FLOOR SLAB
LAND USE & ZONING RSF-2 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-3S-16-02275-130 SUBDIVISION COBBLESTONE
LOT 30 BLOCK PHASE UNIT 2 TOTAL ACRES 2.15

000001119 CBC1252260
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
PERMIT 06-0465-N BK JH Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD, NOC ON FILE
ALTERNATIVE TERMITE TREATMENT ON FILE

Check # or Cash 3421

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 920.00 CERTIFICATION FEE \$ 29.48 SURCHARGE FEE \$ 29.48
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 1078.96
INSPECTORS OFFICE L. H. CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Hauck

Columbia County Building Permit Application

ek#3421
1078.96

Revised 9-23-04

For Office Use Only: Application # 0606-10 Date Received 6/2 By JW Permit # 1119/24644
Application Approved by - Zoning Official BLK Date 13.06.06 Plans Examiner OKJH Date 6-19-06
Flood Zone X Plot Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low-Den.
Comments _____

Applicants Name Linda Roder or Melanic Roder Phone 752-2281
Address 387 S.W. Kemp. Ct. Lake City FL 32024
Owners Name David & Jerelle Hauck Phone 623-0575
911 Address 188 NW Ambleside D Lake City FL 32055
Contractors Name Sparks Contractors - Josh Sparks Phone 623-0575
Address P.O. Box 1479 Lake City FL 32056
Fee Simple Owner Name & Address NA
Bonding Co. Name & Address NA
Architect/Engineer Name & Address Ben Sparks / Mark Disosway
Mortgage Lenders Name & Address Coast Bank of Florida
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
Property ID Number 24-35-16-02275-130 Estimated Cost of Construction 179,000
Subdivision Name Cobblestone Lot 30 Block _____ Unit 2 Phase _____
Driving Directions Hwy. 90 West, R on Lake Jeffrey, R on Bridgewater, L on Ambleside Dr. 2nd Lot on L

Type of Construction SFD Number of Existing Dwellings on Property 0
Total Acreage 2.15 Lot Size _____ Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 89' Side 60' Side 45' Rear 165'
Total Building Height 24' Number of Stories 2 Heated Floor Area 3676 Roof Pitch 12-12
Porch 1324 GARAGE 941 BONUS ROOM 532 GARAGE 941 TOTAL 5897

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

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Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA



Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

Sworn to (or affirmed) and subscribed before me

this _____ day of _____ 20____

Personally known _____ or Produced Identification _____

Contractor Signature

Contractors License Number CBC 1252260

Competency Card Number _____

NOTARY STAMP/SEAL

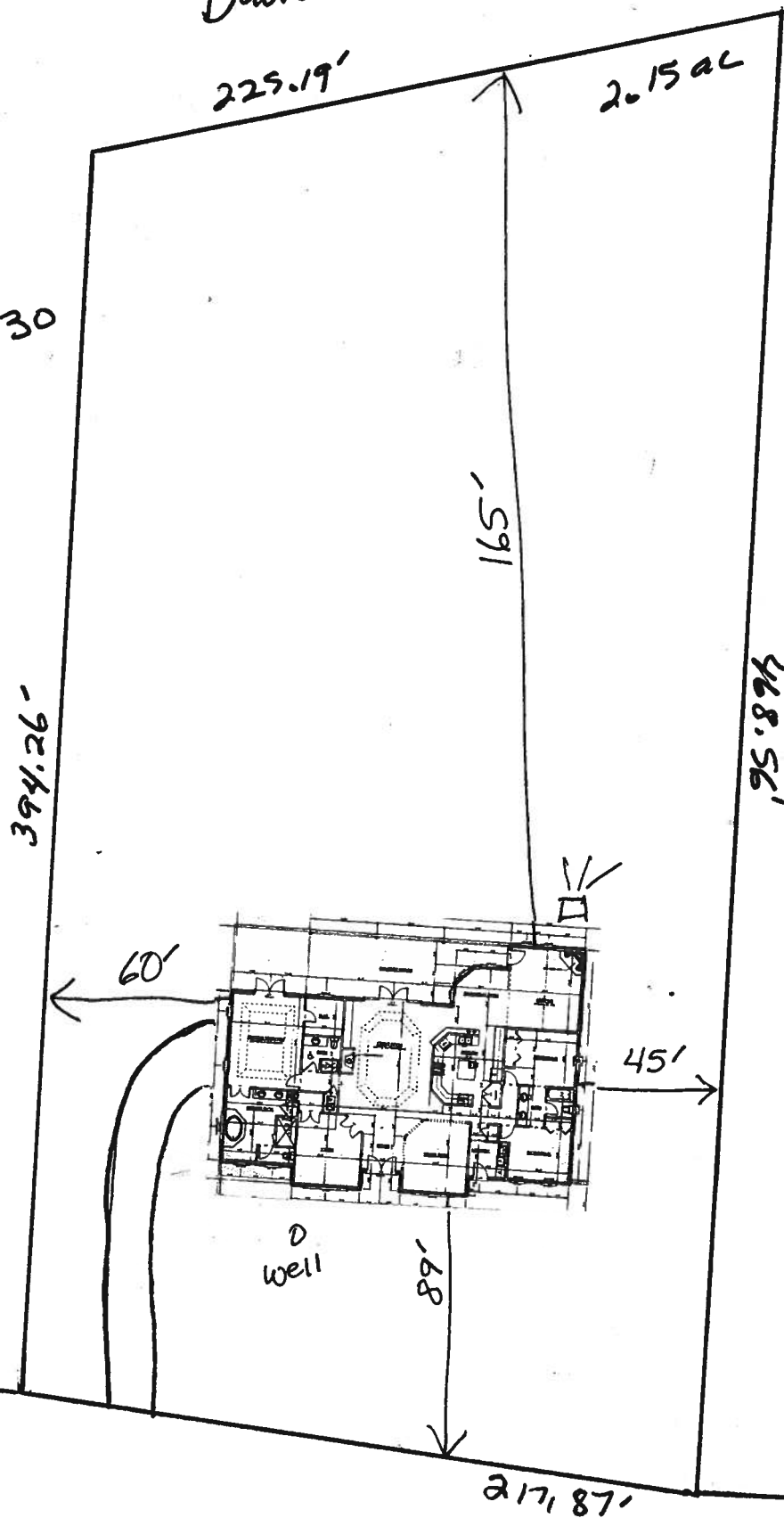
Notary Signature

Site Plan

David + Jenelle Hauck

Cobblestone
Lot 30

24-35-16-0275-130



NW Amble Side Dr

LOAN NUMBER: 7011012386

STATE OF FLORIDA, COUNTY OF Columbia ~~Sumner~~ *at JH*

NOTICE OF COMMENCEMENT

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. **Description of property:** LOT 30, COBBLESTONE, UNIT 2, ACCORDING TO THE MAP OR PLAT THEREOF, AS RECORDED IN PLAT BOOK 8, PAGE 3-6, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.
6021 SW STATE ROAD 247, LAKE CITY, FL 32024
2. **General description of improvements:** Single Family Residence
3. **Owner information:**
 - a. Name(s): DAVID HAUCK and JENNELLE HAUCK
 - b. Address: 724 SE ELOISE AVE., APT 101, LAKE CITY, FL 32025
 - c. Interest in property: Fee Simple
 - d. Name and address of fee simple titleholder (if other than Owner): N/A
4. **Contractor:** SPARKS CONSTRUCTION INC
PO BOX 1479, LAKE CITY, FL 32056
5. **Surety**
 - a. Name and address: None
 - b. The amount of bond: N/A
6. **Lender** COAST BANK OF FLORIDA - P. O. BOX 150, BRADENTON, FLORIDA 34206-0150
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7, Florida Statutes: N/A
8. In addition to himself, Owner designates ** to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
**COAST BANK OF FLORIDA - Attn: Wanda M. Farr - P. O. Box 150, Bradenton, Florida 34206-0150 (941-345-1473)
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____

David Hauck
Borrower: DAVID HAUCK

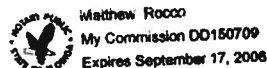
Jennelle Hauck
Borrower: JENNELLE HAUCK

Borrower: _____
STATE OF FL Columbia

Borrower: _____
COUNTY SS: _____

The forgoing instrument was acknowledge before me this 2nd day of MAY, 2006
by DAVID HAUCK and JENNELLE HAUCK who is personally known to me or has produced DL
as identification and who did not take an oath.
My Commission expires: _____

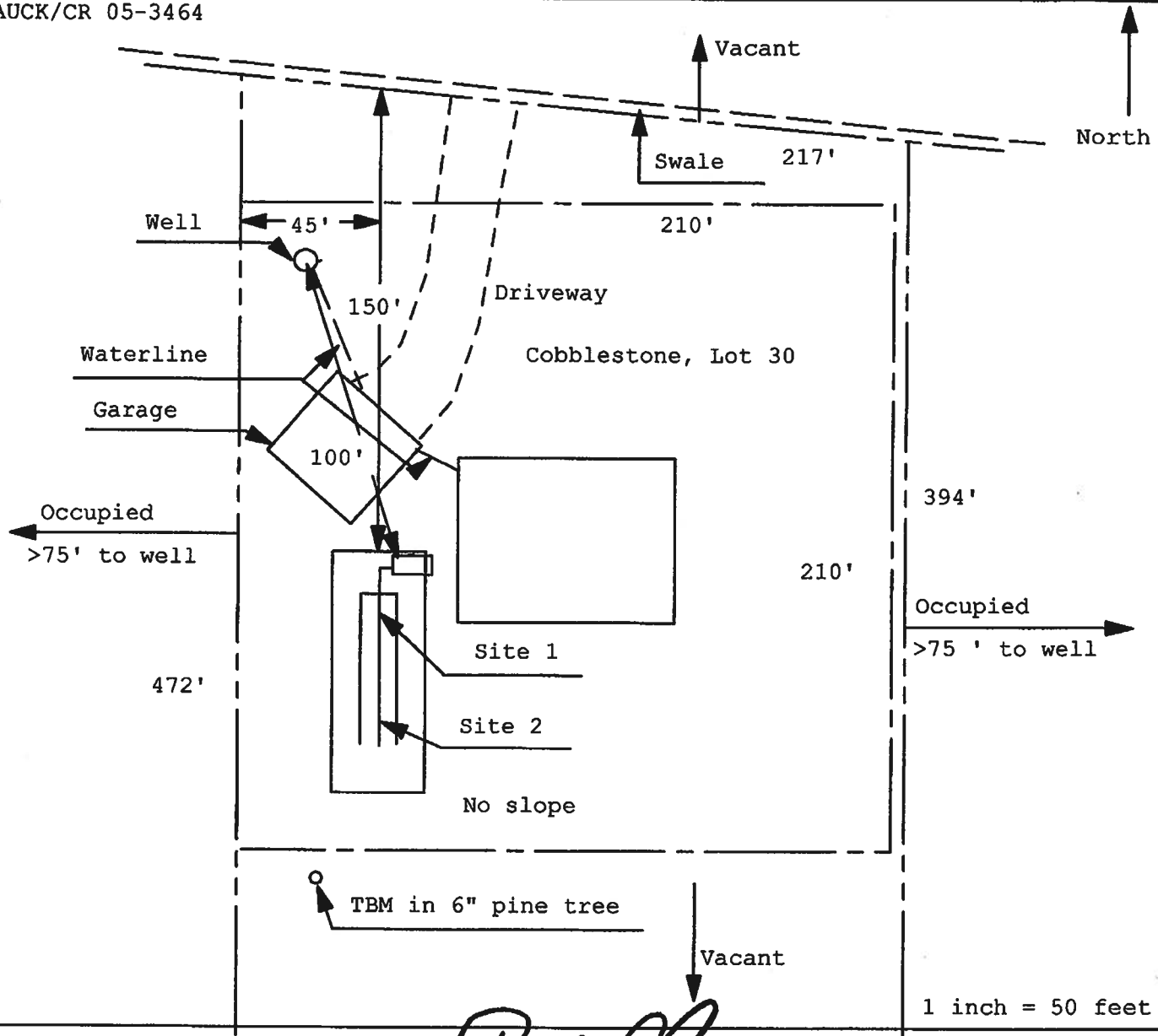
Notary Public *Matthew Rocco*



**Application for Onsite Sewage Disposal System
Construction Permit. Part II Site Plan**
Permit Application Number: 06-0465X1

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

HAUCK/CR 05-3464



Site Plan Submitted By Paul L. Lape Date 4/12/06
 Plan Approved ☒ Not Approved ☐ Date 5/12/06
 By M. J. Lape Columbia CPDU

Notes: _____

Corporate Warranty Deed

This Indenture, made, May 2, 2006 A.D.

Between

Sparks Contractors, Inc. whose post office address is: 162 SW Country Court, Lake City, Florida 32024 a corporation existing under the laws of the State of Florida, Grantor and David Hauck and his wife, Jennelle Hauck whose post office address is: 724 SE Eloise Ave Apt 101, Lake City, FL 32025, Grantee,

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

Lot 30, COBBLESTONE, Unit 2, according to the map or plat thereof, as recorded in Plat Book 8, Page 3-6, of the Public Records of Columbia County, Florida.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: R02275-130

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

Sparks Contractors, Inc.

Signed and Sealed in Our Presence:

By:

Jacob D. Sparks
Its: President

Witness Print Name:

Matthew D. Rocco

Witness Print Name:

State of
County of

Florida
Columbia

(Corporate Seal)

The foregoing instrument was acknowledged before me this 2nd day of May, 2006, by Jacob D. Sparks, the President of Sparks Contractors, Inc. A corporation existing under the laws of the State of Florida, on behalf of the corporation. He/She is personally known to me or has produced DL as identification.

I)

Notary Public

Notary Printed Name:

Matthew Rocco

My Commission Expires::

9/17/2006

Prepared by:

Matt Rocco, an employee of
Sierra Title, LLC,
619 SW Baya Drive, Suite 102
Lake City, Florida 32025

File Number: 06-0114

Inst:2006010853 Date:05/04/2006 Time:09:56

Doc Stamp-Deed : 560.00

DC, P. DeWitt Cason, Columbia County B:1082 P:1030



Matthew Rocco
My Commission DD160700
Expires September 17, 2006



Phone (386) 755-3611

Fax (386) 752-5381

Notice of Intent for Preventative Treatment for Termites

(As required by Florida Building Code (FBC) 104.2.6)

Aspen Pest Control, Inc.**(386) 755-3611****State License # - JB109476****State Certification # - JF104376****Hauck, Lot 30 Cobblestone, Lake City, FL**

Address of Treatment or Lot/Block of Treatment

Bora-Care Wood Treatment – 23% Disodium Octaborate Tetrahydrate

Method of Termite Prevention Treatment – Soil Barrier, Wood Treatment, Bait System, Other

Application onto Structural Wood

Description of Treatment

The above named structure will receive a complete treatment for the prevention of subterranean termites at the dried-in stage of construction. Treatment is done in accordance with the rules and laws established by the Florida Department of Agriculture and Consumer Services and according to EPA registered label directions as stated in Florida Building Code Section 1861.1.8.

Celia Oyler
Authorized Signature

4-6-06
Date

FROM :

FAX NO. : 386-755-7022

Sep. 17 2002 01:52PM P1

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL
OWNERS

PHONE (804) 782-1884

FAX (804) 785-7022

17000 NW 10th Street
LAKE CITY, FLORIDA 33555

904 NW Main Blvd.

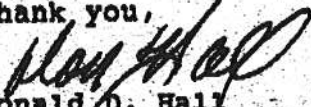
June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,


Donald D. Hall
DDH/jk

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: 512077SparksConstructionInc Address: City, State: , FL Owner: Climate Zone: North	Builder: Josh Sparks Permitting Office: Columbia Co. Permit Number: 24644 Jurisdiction Number: 21000
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1. New construction or existing New ☐ 2. Single family or multi-family Single family ☐ 3. Number of units, if multi-family 1 ☐ 4. Number of Bedrooms 3 ☐ 5. Is this a worst case? Yes ☐ 6. Conditioned floor area (ft²) 3100 ft² ☐ 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) <table style="width: 100%;"> <tr> <td style="width: 30%;">a. U-factor:</td> <td style="width: 30%;">Description</td> <td style="width: 40%;">Area</td> </tr> <tr> <td>(or Single or Double DEFAULT)</td> <td>7a. (Dble Default)</td> <td>481.0 ft²</td> </tr> <tr> <td colspan="3">b. SHGC:</td> </tr> <tr> <td>(or Clear or Tint DEFAULT)</td> <td>7b. (Clear)</td> <td>481.0 ft²</td> </tr> </table> 8. Floor types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Slab-On-Grade Edge Insulation</td> <td style="width: 40%;">R=0.0, 263.0(p) ft</td> <td style="width: 30%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 9. Wall types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Frame, Wood, Exterior</td> <td style="width: 40%;">R=13.0, 2029.0 ft²</td> <td style="width: 30%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> <tr> <td>d. N/A</td> <td></td> <td></td> </tr> <tr> <td>e. N/A</td> <td></td> <td></td> </tr> </table> 10. Ceiling types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Under Attic</td> <td style="width: 40%;">R=30.0, 3200.0 ft²</td> <td style="width: 30%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 11. Ducts <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Sup: Unc. Ret: Unc. AH: Interior</td> <td style="width: 40%;">Sup. R=6.0, 220.0 ft</td> <td style="width: 30%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> </table>	a. U-factor:	Description	Area	(or Single or Double DEFAULT)	7a. (Dble Default)	481.0 ft²	b. SHGC:			(or Clear or Tint DEFAULT)	7b. (Clear)	481.0 ft²	a. Slab-On-Grade Edge Insulation	R=0.0, 263.0(p) ft		b. N/A			c. N/A			a. Frame, Wood, Exterior	R=13.0, 2029.0 ft²		b. N/A			c. N/A			d. N/A			e. N/A			a. Under Attic	R=30.0, 3200.0 ft²		b. N/A			c. N/A			a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 220.0 ft		b. N/A			12. Cooling systems <table style="width: 100%;"> <tr> <td style="width: 50%;">a. Central Unit</td> <td style="width: 50%;">Cap: 61.0 kBtu/hr</td> </tr> <tr> <td></td> <td>SEER: 11.00</td> </tr> <tr> <td>b. N/A</td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> </tr> </table> 13. Heating systems <table style="width: 100%;"> <tr> <td style="width: 50%;">a. Electric Heat Pump</td> <td style="width: 50%;">Cap: 61.0 kBtu/hr</td> </tr> <tr> <td></td> <td>HSPF: 7.30</td> </tr> <tr> <td>b. N/A</td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> </tr> </table> 14. Hot water systems <table style="width: 100%;"> <tr> <td style="width: 50%;">a. Electric Resistance</td> <td style="width: 50%;">Cap: 40.0 gallons</td> </tr> <tr> <td></td> <td>EF: 0.93</td> </tr> <tr> <td>b. N/A</td> <td></td> </tr> <tr> <td>c. Conservation credits</td> <td></td> </tr> <tr> <td colspan="2">(HR-Heat recovery, Solar</td> </tr> <tr> <td colspan="2">DHP-Dedicated heat pump)</td> </tr> </table> 15. HVAC credits <table style="width: 100%;"> <tr> <td style="width: 50%;">(CF-Ceiling fan, CV-Cross ventilation,</td> <td style="width: 50%;"></td> </tr> <tr> <td>HF-Whole house fan,</td> <td></td> </tr> <tr> <td>PT-Programmable Thermostat,</td> <td></td> </tr> <tr> <td>MZ-C-Multizone cooling,</td> <td></td> </tr> <tr> <td>MZ-H-Multizone heating)</td> <td></td> </tr> </table>	a. Central Unit	Cap: 61.0 kBtu/hr		SEER: 11.00	b. N/A		c. N/A		a. Electric Heat Pump	Cap: 61.0 kBtu/hr		HSPF: 7.30	b. N/A		c. N/A		a. Electric Resistance	Cap: 40.0 gallons		EF: 0.93	b. N/A		c. Conservation credits		(HR-Heat recovery, Solar		DHP-Dedicated heat pump)		(CF-Ceiling fan, CV-Cross ventilation,		HF-Whole house fan,		PT-Programmable Thermostat,		MZ-C-Multizone cooling,		MZ-H-Multizone heating)	
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Glass/Floor Area: 0.16

Total as-built points: 38956

Total base points: 40106

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Ben Sparks
DATE: 5-22-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Justin Pater
DATE: 6-20-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	3100.0	20.04	11182.3	Double, Clear	S	1.5	6.5	20.0	35.87	0.88	629.0
				Double, Clear	S	13.0	6.5	15.0	35.87	0.45	242.7
				Double, Clear	S	13.0	6.5	40.0	35.87	0.45	647.2
				Double, Clear	E	30.0	7.0	18.0	42.06	0.36	270.2
				Double, Clear	SE	8.0	7.0	24.0	42.75	0.47	480.2
				Double, Clear	S	4.0	7.0	24.0	35.87	0.62	537.5
				Double, Clear	S	1.5	0.0	36.0	35.87	0.43	557.7
				Double, Clear	W	1.5	0.0	36.0	38.52	0.37	519.5
				Double, Clear	W	1.5	0.0	8.0	38.52	0.37	115.4
				Double, Clear	N	8.0	7.0	72.0	19.20	0.69	951.7
				Double, Clear	N	8.0	7.0	108.0	19.20	0.69	1427.5
				Double, Clear	S	12.0	8.0	20.0	35.87	0.47	339.4
				Double, Clear	E	1.5	0.0	24.0	42.06	0.36	360.2
				Double, Clear	S	1.5	0.0	36.0	35.87	0.43	557.7
				As-Built Total:				481.0	7636.0		
WALL TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0		2029.0	1.50		3043.5	
Exterior	2029.0	1.70	3449.3								
Base Total:				2029.0		3449.3					
				As-Built Total:		2029.0		3043.5			
DOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	0.0	0.00	0.0	Exterior Insulated			80.0	4.10		328.0	
Exterior	120.0	4.10	492.0	Exterior Insulated			40.0	4.10		164.0	
Base Total:				120.0		492.0					
				As-Built Total:		120.0		492.0			
CEILING TYPES Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	3100.0	1.73	5363.0	Under Attic	30.0		3200.0	1.73 X 1.00		5536.0	
Base Total:				3100.0		5363.0					
				As-Built Total:		3200.0		5536.0			
FLOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	263.0(p)	-37.0	-9731.0	Slab-On-Grade Edge Insulation	0.0		263.0(p)	-41.20		-10835.6	
Raised	0.0	0.00	0.0								
Base Total:				-9731.0		263.0		-10835.6			
				As-Built Total:		263.0		-10835.6			

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,

PERMIT #:

BASE				AS-BUILT					
INFILTRATION Area X BSPM = Points				Area X SPM = Points					
3100.0	10.21	31651.0		3100.0	10.21	31651.0			
Summer Base Points: 42406.6				Summer As-Built Points: 37522.9					
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
42406.6	0.4266	18090.7		(sys 1: Central Unit 61000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 37523 1.00 (1.09 x 1.147 x 0.91) 0.310 1.000 13245.6 37522.9 1.00 1.138 0.310 1.000 13245.6					

(sys 1: Central Unit 61000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ormt Len Hgt		Area X WPM X WOF = Points				
.18	3100.0	12.74	7108.9	Double, Clear	S	1.5	6.5	20.0	13.30	1.09	291.0
				Double, Clear	S	13.0	6.5	15.0	13.30	3.51	699.2
				Double, Clear	S	13.0	6.5	40.0	13.30	3.51	1864.5
				Double, Clear	E	30.0	7.0	18.0	18.79	1.51	509.7
				Double, Clear	SE	8.0	7.0	24.0	14.71	2.07	730.1
				Double, Clear	S	4.0	7.0	24.0	13.30	1.82	581.8
				Double, Clear	S	1.5	0.0	36.0	13.30	3.66	1752.1
				Double, Clear	W	1.5	0.0	36.0	20.73	1.24	923.6
				Double, Clear	W	1.5	0.0	8.0	20.73	1.24	205.3
				Double, Clear	N	8.0	7.0	72.0	24.58	1.02	1805.0
				Double, Clear	N	8.0	7.0	108.0	24.58	1.02	2707.5
				Double, Clear	S	12.0	8.0	20.0	13.30	3.28	871.0
				Double, Clear	E	1.5	0.0	24.0	18.79	1.51	679.6
				Double, Clear	S	1.5	0.0	36.0	13.30	3.66	1752.1
				As-Built Total:		481.0			15372.5		
WALL TYPES				Area X BWPM = Points		Type		R-Value		Area X WPM = Points	
Adjacent		0.0	0.00	0.0	Frame, Wood, Exterior		13.0	2029.0	3.40	6898.6	
Exterior		2029.0	3.70	7507.3							
Base Total:		2029.0	7507.3		As-Built Total:		2029.0		6898.6		
DOOR TYPES				Area X BWPM = Points		Type		Area X WPM = Points			
Adjacent		0.0	0.00	0.0	Exterior Insulated		80.0	8.40	672.0		
Exterior		120.0	8.40	1008.0	Exterior Insulated		40.0	8.40	336.0		
Base Total:		120.0	1008.0		As-Built Total:		120.0		1008.0		
CEILING TYPES				Area X BWPM = Points		Type		R-Value		Area X WPM X WCM = Points	
Under Attic		3100.0	2.05	6355.0	Under Attic		30.0	3200.0	2.05 X 1.00	6560.0	
Base Total:		3100.0	6355.0		As-Built Total:		3200.0		6560.0		
FLOOR TYPES				Area X BWPM = Points		Type		R-Value		Area X WPM = Points	
Slab		263.0(p)	8.9	2340.7	Slab-On-Grade Edge Insulation		0.0	263.0(p)	18.80	4944.4	
Raised		0.0	0.00	0.0							
Base Total:		2340.7		As-Built Total:		263.0		4944.4			

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,

PERMIT #:

BASE				AS-BUILT			
INFILTRATION Area X BWPM = Points				Area X WPM = Points			
3100.0 -0.59 -1829.0				3100.0 -0.59 -1829.0			
Winter Base Points:		22490.9		Winter As-Built Points:		32954.5	
Total Winter X Points	System = Multiplier	Heating Points		Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points			
				(System - Points)	(DM x DSM x AHU)		
				(sys 1: Electric Heat Pump 61000 btuh ,EFF(7.3) Ducts:Unc(S),Unc(R),Int(AH),R6.0			
				32954.5 1.000 (1.069 x 1.169 x 0.93) 0.467 1.000 17890.4			
22490.9	0.6274	14110.8		32954.5	1.00	1.162	0.467 1.000 17890.4

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,

PERMIT #:

BASE					AS-BUILT						
WATER HEATING											
Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	= Total Multiplier
3		2635.00		7905.0	40.0	0.93	3		1.00	2606.67	1.00 7820.0
					As-Built Total:						7820.0

CODE COMPLIANCE STATUS											
BASE						AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
18091		14111		7905	40106	13246		17890		7820	38956

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.8

The higher the score, the more efficient the home.

, , , FL,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 61.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 11.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	3100 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 61.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 481.0 ft ²		HSPF: 7.30
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 481.0 ft ²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 263.0(p) ft	a. Electric Resistance	Cap: 40.0 gallons
b. N/A			EF: 0.93
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Exterior	R=13.0, 2029.0 ft ²	(HR-Heat recovery, Solar	
b. N/A		DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 3200.0 ft ²	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 220.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLR2PB v4.1)

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001119

DATE 06/20/2006 PARCEL ID # 24-3S-16-02275-130

APPLICANT LINDA RODER PHONE 752-2281

ADDRESS 387 SW KEP CT LAKE CITY FL 32024

OWNER DAVID & JERELLE HAUCK PHONE 623-0575

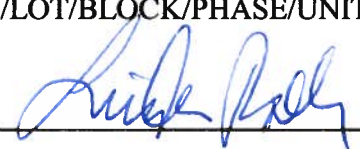
ADDRESS 188 NW AMBLESIDE DR LAKE CITY FL 32055

CONTRACTOR SPARKS CONSTRUCTION/JOSH SPARKS PHONE 623-0575

LOCATION OF PROPERTY 90 W, R ON LAKE JEFFERY, R ON BRIDGEWATER, L AMBLESIDE DR,

2ND LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT COBBLESTONE 30 2

SIGNATURE 

INSTALLATION REQUIREMENTS

☒ X

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

☐

Culvert installation shall conform to the approved site plan standards.

☐

Department of Transportation Permit installation approved standards.

☐

Other _____

**ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.**

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





P.O. Box 1625, Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5, Jacksonville, FL 32257

Fax • (904) 262-4047

DATE TESTED: 7-13-06

ASTM METHOD: ✓ (D-2922) Nuclear (D-2937) Drive Cylinder Sandcone

CLIENT: Sparkz Construction

EARTHWORK CONTRACTOR: S.A.C.

SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: C. Day

MODIFIED (ASTM D-1557):

STANDARD (ASTM D-698):

REMARKS:

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other



From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: **0606-10**

Contractor: Sparks Construction Owner David Hauck lot 30 Unit 2 Cobblestone Subdivision.

On the date of June 13, 2006 application 0606-10 and plans for construction of a single family dwelling with an attached garage and bonus room area were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0606-10 when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. The area above the attached garage plans shows a bonus room area.

Therefore the plans must indicate compliance with section R309.1 Opening protection.

The openings between the garage and bonus room stairwell shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.

Additional separation required per section R309.2. The garage shall be separated from the bonus room by not less than 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than 1/2-inch (12.7 mm) gypsum board or equivalent. The stairwell which provides egress to the bonus room shall also have the same protection as required in section R309.2.

2. The bonus room dormer flooring is shown as 3/4" T&G flooring please indicate that the 3/4" T&G flooring will also be used as flooring onto the floor truss system.

3. Truss spacing is provided for placement of two walk out live dormers, show the floor joist framing material and attachment method to the floor joist which will attach to the truss flooring system.

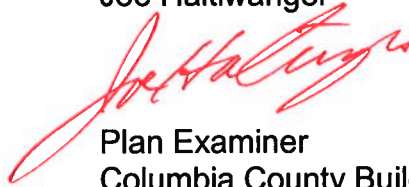
4. The total rise and run of the stair system that will be employed to gain egress to the bonus room for the garage area shall be stated on the plans along with a stair design which will show compliance with the following section of the Florida Residential Codes section R311.5 Stairways width stairways shall not be less

than 36 inches (914 mm) in clear width at all points above the permitted handrail height and below the required headroom height. Handrails shall not project more than 4.5 inches (114 mm) on either side of the stairway and the minimum clear width of the stairway at and below the handrail height, including treads and landings, shall not be less than 31.5 inches (787 mm) where a handrail is installed on one side and 27 inches (698 mm) where handrails are provided on both sides. Section R311.5.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2036 mm) measured vertically from the sloped plane adjoining the tread nosing or from the floor surface of the landing or platform. R311.5.3 Stair treads and risers. R311.5.3.1 Riser height. The maximum riser height shall be $7\frac{3}{4}$ inches (196 mm). The riser shall be measured vertically between leading edges of the adjacent treads. The greatest riser height within any flight of stairs shall not exceed the smallest by more than $\frac{3}{8}$ inch (9.5 mm). R311.5.3.2 Tread depth. The minimum tread depth, exclusive of nosing, shall be not less than 9 inches (229 mm). Treads and risers of stairs shall be permitted to be so proportioned that the sum of two risers and a tread, exclusive of projection of nosing, is not less than 24 inches (610 mm) nor more than 25 inches (635 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. The greatest tread depth within any flight of stairs shall not exceed the smallest by more than $\frac{3}{8}$ inch (9.5 mm). Winder treads shall have a minimum tread depth of 10 inches (254 mm) measured as above at a point 12 inches (305) mm from the side where the treads are

narrower. Winder treads shall have a minimum tread depth of 6 inches (152 mm) at any point. Within any flight of stairs, the greatest winder tread depth at the 12 inch (305 mm) walk line shall not exceed the smallest by more than 3/8 inch (9.5 mm). R311.5.3.3 Profile. The radius of curvature at the leading edge of the tread shall be no greater than 9/16 inch (14.3 mm). Every tread less than 10 inches (254 mm) wide shall have a nosing or effective projection of approximately 1 inch (25 mm) over the level immediately below that tread. Beveling of nosing shall not exceed 1/2 inch (12.7 mm). Risers shall be vertical or sloped from the underside of the leading edge of the tread above at an angle not more than 30 (0.51 rad) degrees from the vertical. Open risers are permitted, provided that the opening between treads does not permit the passage of a 4-inch diameter (102 mm) sphere. Exceptions: 1. A nosing is not required where the tread depth is a minimum of 11 inches (279 mm). : 2. The opening between adjacent treads is not limited on stairs with a total rise of 30 inches (762 mm) or less. R311.5.4 Landings for stairways. There shall be a floor or landing at the top and bottom of each stairway. Exception: A door at the top of a stair shall be permitted to open directly at a stair, provided the door does not swing over the stair. A flight of stairs shall not have a vertical rise greater than 12 feet (3658 mm) between floor levels or landings. The width of each landing shall not be less than the stairway served. Every landing shall have a minimum dimension of 36 inches (914 mm) measured in the direction of travel. R311.5.5 Stairway walking surface. The walking surface of treads and landings of stairways shall be sloped no steeper than one unit vertical in 48 inches horizontal (2-percent slope).

5. The bonus room area shows one 3'0" X 5' 0" window this window shall meet the requirements of an FBC-2004 Section R310.1.1 Minimum opening area: All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m2). Verify on the plans that the window in the bonus room will meet the requirements of FBC-2004 Section R310.1.1

Joe Haltiwanger

A red ink signature of Joe Haltiwanger, written in a cursive style, positioned over the printed name and title.

Plan Examiner
Columbia County Building Department

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529



Phone Number 386-758-1163
Fax Number 386-754-7088

FAX TRANSMITTAL FORM

To:
Name: Linda Roder
CC: Building permit application **0606-10**
Phone: 752-2281
Fax: 752-2282

From:
Date Sent: 06/13/06

Number of Pages: **Five**

Message: Reference to a building permit application Number: **0606-10**

Contractor: *Sparks Construction Owner David Hauck lot 30 Unit 2 Cobblestone Subdivision.*

The review of the party to whom it is addressed. It may contain proprietary and/or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you have received this transmission in error, please immediately telephone the sender above to arrange for its return.

Residential System Sizing Calculation

Summary

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

, FL

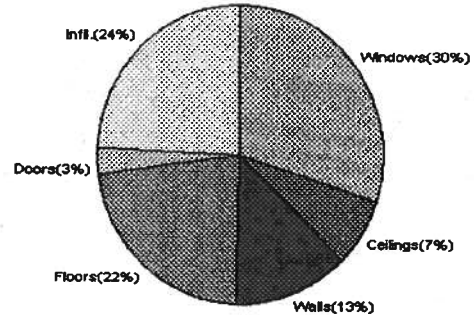
5/22/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)			
Winter design temperature	33 F	Summer design temperature	92 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	37 F	Summer temperature difference	17 F
Total heating load calculation	51092 Btuh	Total cooling load calculation	43143 Btuh
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	119.4 61000	Sensible (SHR = 0.75)	126.1 45750
Heat Pump + Auxiliary(0.0kW)	119.4 61000	Latent	222.2 15250
		Total (Electric Heat Pump)	141.4 61000

WINTER CALCULATIONS

Winter Heating Load (for 3100 sqft)

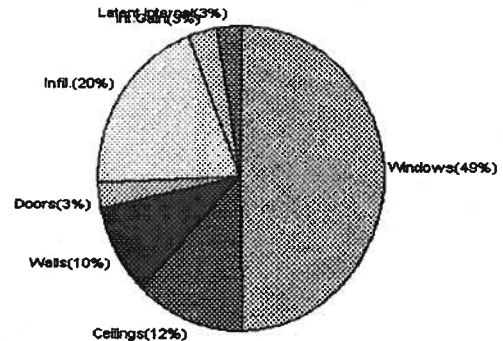
Load component			Load	
Window total	481 sqft		15483	Btuh
Wall total	2029 sqft		6663	Btuh
Door total	120 sqft		1554	Btuh
Ceiling total	3200 sqft		3771	Btuh
Floor total	263 sqft		11483	Btuh
Infiltration	300 cfm		12138	Btuh
Duct loss			0	Btuh
Subtotal			51092	Btuh
Ventilation	0 cfm		0	Btuh
TOTAL HEAT LOSS			51092	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 3100 sqft)

Load component			Load	
Window total	481 sqft		21306	Btuh
Wall total	2029 sqft		4232	Btuh
Door total	120 sqft		1176	Btuh
Ceiling total	3200 sqft		5299	Btuh
Floor total			0	Btuh
Infiltration	155 cfm		2885	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0 cfm		0	Btuh
Total sensible gain			36278	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			5665	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			6865	Btuh
TOTAL HEAT GAIN			43143	Btuh



For Florida residences only

EnergyGauge® System Sizing

PREPARED BY: *Ben Sparks*

DATE: 5-22-06

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F
This calculation is for Worst Case. The house has been rotated 315 degrees.

5/22/2006

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
2	2, Clear, Metal, 0.87	NW	15.0		32.2	483 Btuh
3	2, Clear, Metal, 0.87	NW	40.0		32.2	1288 Btuh
4	2, Clear, Metal, 0.87	SW	18.0		32.2	579 Btuh
5	2, Clear, Metal, 0.87	W	24.0		32.2	773 Btuh
6	2, Clear, Metal, 0.87	NW	24.0		32.2	773 Btuh
7	2, Clear, Metal, 0.87	NW	36.0		32.2	1159 Btuh
8	2, Clear, Metal, 0.87	NE	36.0		32.2	1159 Btuh
9	2, Clear, Metal, 0.87	NE	8.0		32.2	258 Btuh
10	2, Clear, Metal, 0.87	SE	72.0		32.2	2318 Btuh
11	2, Clear, Metal, 0.87	SE	108.0		32.2	3477 Btuh
12	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
13	2, Clear, Metal, 0.87	SW	24.0		32.2	773 Btuh
14	2, Clear, Metal, 0.87	NW	36.0		32.2	1159 Btuh
Window Total			481 (sqft)			15483 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	2029		3.3	6663 Btuh
Wall Total			2029			6663 Btuh
Doors	Type		Area	X	HTM=	Load
1	Insulated - Exterior		40		12.9	518 Btuh
2	Insulated - Exterior		80		12.9	1036 Btuh
Door Total			120			1554 Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic(D/Shin)	30.0	3200		1.2	3771 Btuh
Ceiling Total			3200			3771 Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Slab On Grade	0	263.0 ft(p)		43.7	11483 Btuh
Floor Total			263			11483 Btuh
Zone Envelope Subtotal:						38954 Btuh
Infiltration	Type	ACH X	Zone Volume		CFM=	
	Natural	0.58	31000		299.7	12138 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)				(DLM of 0.00)	0 Btuh
Zone #1	Sensible Zone Subtotal					51092 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

, FL

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

5/22/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	51092 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	51092 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F
This calculation is for Worst Case. The house has been rotated 315 degrees.

5/22/2006

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
2	2, Clear, Metal, 0.87	NW	15.0		32.2	483 Btuh
3	2, Clear, Metal, 0.87	NW	40.0		32.2	1288 Btuh
4	2, Clear, Metal, 0.87	SW	18.0		32.2	579 Btuh
5	2, Clear, Metal, 0.87	W	24.0		32.2	773 Btuh
6	2, Clear, Metal, 0.87	NW	24.0		32.2	773 Btuh
7	2, Clear, Metal, 0.87	NW	36.0		32.2	1159 Btuh
8	2, Clear, Metal, 0.87	NE	36.0		32.2	1159 Btuh
9	2, Clear, Metal, 0.87	NE	8.0		32.2	258 Btuh
10	2, Clear, Metal, 0.87	SE	72.0		32.2	2318 Btuh
11	2, Clear, Metal, 0.87	SE	108.0		32.2	3477 Btuh
12	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
13	2, Clear, Metal, 0.87	SW	24.0		32.2	773 Btuh
14	2, Clear, Metal, 0.87	NW	36.0		32.2	1159 Btuh
Window Total			481(sqft)			15483 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	2029		3.3	6663 Btuh
Wall Total			2029			6663 Btuh
Doors	Type		Area	X	HTM=	Load
1	Insulated - Exterior		40		12.9	518 Btuh
2	Insulated - Exterior		80		12.9	1036 Btuh
Door Total			120			1554 Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic(D/Shin)	30.0	3200		1.2	3771 Btuh
Ceiling Total			3200			3771 Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Slab On Grade	0	263.0 ft(p)		43.7	11483 Btuh
Floor Total			263			11483 Btuh
Zone Envelope Subtotal:						38954 Btuh
Infiltration	Type	ACH X	Zone Volume		CFM=	
	Natural	0.58	31000		299.7	12138 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)				(DLM of 0.00)	0 Btuh
Zone #1	Sensible Zone Subtotal					51092 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

, FL

5/22/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	51092 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	51092 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F
This calculation is for Worst Case. The house has been rotated 315 degrees.

5/22/2006

Component Loads for Whole House

Window	Type*	Ornt	Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS		Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	20.0	0.0	20.0	29	60	1201	Btuh
2	2, Clear, 0.87, None,N,N	NW	13ft.	6.5ft.	15.0	0.0	15.0	29	60	901	Btuh
3	2, Clear, 0.87, None,N,N	NW	13ft.	6.5ft.	40.0	0.0	40.0	29	60	2401	Btuh
4	2, Clear, 0.87, None,N,N	SW	30ft.	7ft.	18.0	18.0	0.0	29	63	521	Btuh
5	2, Clear, 0.87, None,N,N	W	8ft.	7ft.	24.0	22.6	1.4	29	80	768	Btuh
6	2, Clear, 0.87, None,N,N	NW	4ft.	7ft.	24.0	0.0	24.0	29	60	1441	Btuh
7	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	36.0	0.0	36.0	29	60	2161	Btuh
8	2, Clear, 0.87, None,N,N	NE	1.5ft.	0ft.	36.0	0.0	36.0	29	60	2161	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft.	0ft.	8.0	0.0	8.0	29	60	480	Btuh
10	2, Clear, 0.87, None,N,N	SE	8ft.	7ft.	72.0	72.0	0.0	29	63	2085	Btuh
11	2, Clear, 0.87, None,N,N	SE	8ft.	7ft.	108.0	108.0	0.0	29	63	3128	Btuh
12	2, Clear, 0.87, None,N,N	NW	12ft.	8ft.	20.0	0.0	20.0	29	60	1201	Btuh
13	2, Clear, 0.87, None,N,N	SW	1.5ft.	0ft.	24.0	24.0	0.0	29	63	695	Btuh
14	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	36.0	0.0	36.0	29	60	2161	Btuh
Window Total					481 (sqft)					21306 Btuh	
Walls	Type	R-Value/U-Value		Area(sqft)		HTM		Load			
1	Frame - Wood - Ext	13.0/0.09		2029.0		2.1		4232 Btuh			
Wall Total					2029 (sqft)				4232 Btuh		
Doors	Type			Area (sqft)		HTM		Load			
1	Insulated - Exterior			40.0		9.8		392 Btuh			
2	Insulated - Exterior			80.0		9.8		784 Btuh			
Door Total					120 (sqft)				1176 Btuh		
Ceilings	Type/Color/Surface	R-Value		Area(sqft)		HTM		Load			
1	Vented Attic/DarkShingle	30.0		3200.0		1.7		5299 Btuh			
Ceiling Total					3200 (sqft)				5299 Btuh		
Floors	Type	R-Value		Size		HTM		Load			
1	Slab On Grade	0.0		263 (ft(p))		0.0		0 Btuh			
Floor Total					263.0 (sqft)				0 Btuh		
Zone Envelope Subtotal:									32014 Btuh		
Infiltration	Type	ACH		Volume(cuft)		CFM=		Load			
	SensibleNatural	0.30		31000		155.0		2885 Btuh			
Internal gain	Occupants		Btuh/occupant		Appliance		Load				
	6		X 230 +		0		1380 Btuh				
Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)						DGM = 0.00		0.0 Btuh		
Sensible Zone Load									36278 Btuh		

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

, FL

5/22/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	36278 Btuh
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	36278 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	36278 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	5665 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	6865 Btuh
	TOTAL GAIN	43143 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F
This calculation is for Worst Case. The house has been rotated 315 degrees.

5/22/2006

Component Loads for Zone #1: Main

Window	Type*	Ornt	Overhang		Window Area(sqft)			HTM		Load
	Pn/SHGC/U/InSh/ExSh/IS		Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
2	2, Clear, 0.87, None,N,N	NW	13ft.	6.5ft.	15.0	0.0	15.0	29	60	901 Btuh
3	2, Clear, 0.87, None,N,N	NW	13ft.	6.5ft.	40.0	0.0	40.0	29	60	2401 Btuh
4	2, Clear, 0.87, None,N,N	SW	30ft.	7ft.	18.0	18.0	0.0	29	63	521 Btuh
5	2, Clear, 0.87, None,N,N	W	8ft.	7ft.	24.0	22.6	1.4	29	80	768 Btuh
6	2, Clear, 0.87, None,N,N	NW	4ft.	7ft.	24.0	0.0	24.0	29	60	1441 Btuh
7	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	36.0	0.0	36.0	29	60	2161 Btuh
8	2, Clear, 0.87, None,N,N	NE	1.5ft.	0ft.	36.0	0.0	36.0	29	60	2161 Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft.	0ft.	8.0	0.0	8.0	29	60	480 Btuh
10	2, Clear, 0.87, None,N,N	SE	8ft.	7ft.	72.0	72.0	0.0	29	63	2085 Btuh
11	2, Clear, 0.87, None,N,N	SE	8ft.	7ft.	108.0	108.0	0.0	29	63	3128 Btuh
12	2, Clear, 0.87, None,N,N	NW	12ft.	8ft.	20.0	0.0	20.0	29	60	1201 Btuh
13	2, Clear, 0.87, None,N,N	SW	1.5ft.	0ft.	24.0	24.0	0.0	29	63	695 Btuh
14	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	36.0	0.0	36.0	29	60	2161 Btuh
	Window Total				481 (sqft)					21306 Btuh
Walls	Type	R-Value/U-Value		Area(sqft)			HTM		Load	
1	Frame - Wood - Ext	13.0/0.09		2029.0			2.1		4232 Btuh	
	Wall Total				2029 (sqft)					4232 Btuh
Doors	Type			Area (sqft)			HTM		Load	
1	Insulated - Exterior			40.0			9.8		392 Btuh	
2	Insulated - Exterior			80.0			9.8		784 Btuh	
	Door Total				120 (sqft)					1176 Btuh
Ceilings	Type/Color/Surface	R-Value		Area(sqft)			HTM		Load	
1	Vented Attic/DarkShingle	30.0		3200.0			1.7		5299 Btuh	
	Ceiling Total				3200 (sqft)					5299 Btuh
Floors	Type	R-Value		Size			HTM		Load	
1	Slab On Grade	0.0		263 (ft(p))			0.0		0 Btuh	
	Floor Total				263.0 (sqft)					0 Btuh
	Zone Envelope Subtotal:									32014 Btuh
Infiltration	Type	ACH		Volume(cuft)			CFM=		Load	
	SensibleNatural	0.30		31000			155.0		2885 Btuh	
Internal gain	Occupants		Btuh/occupant			Appliance		Load		
	6		X 230 +			0		1380 Btuh		
Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)							DGM = 0.00		0.0 Btuh
	Sensible Zone Load									36278 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

, FL

Project Title:
512077SparksConstructionInc

Class 3 Rating
Registration No. 0
Climate: North

5/22/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	36278 Btuh
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	36278 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	36278 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	5665 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	6865 Btuh
	TOTAL GAIN	43143 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)



For Florida residences only

Residential Window Diversity

MidSummer

Project Title:
512077SparksConstructionInc

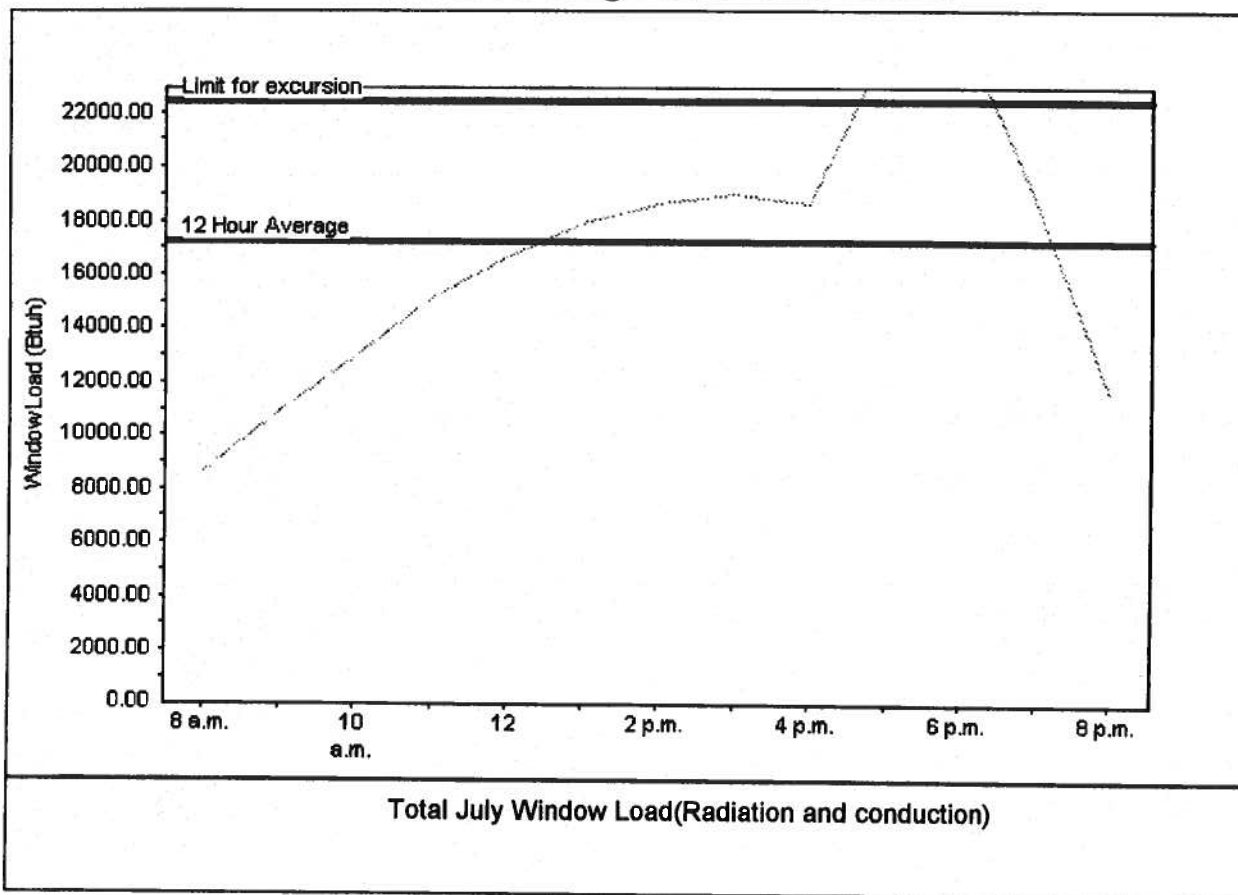
Class 3 Rating
Registration No. 0
Climate: North

5/22/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	17241 Btu
Summer setpoint	75 F	Peak window load for July	24974 Btu
Summer temperature difference	17 F	Excursion limit(130% of Ave.)	22413 Btu
Latitude	29 North	Window excursion (July)	2562 Btu/h

WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY: *Y3en Sparks*
DATE: *5-22-06*

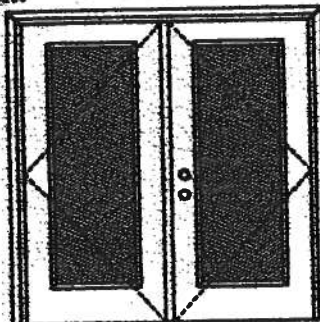
EnergyGauge® FLR2PB v4.1



XX**Glazed Outswing Unit**

DUP-WL-JH0162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'6".

Double Door
Maximum unit size = 6'0" x 6'6"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed -- see MID-WL-MA0002-02.

APPROVED DOOR STYLES:**1/4 GLASS:**

100 Series



135, 136 Series



138 Series



680 Series



822 Series

1/2 GLASS:

105 Series*



106, 100 Series*



120 Series*



200 Series*



12 RL, 20 RL, 24 RL Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 6-panel with screen; Eyebrow 5-panel; Eyebrow 6-panel with screen.

Johnson
EntrySystems

March 23, 2002
Current listing program of product improvements makes specifications, design and product detail subject to change without notice.

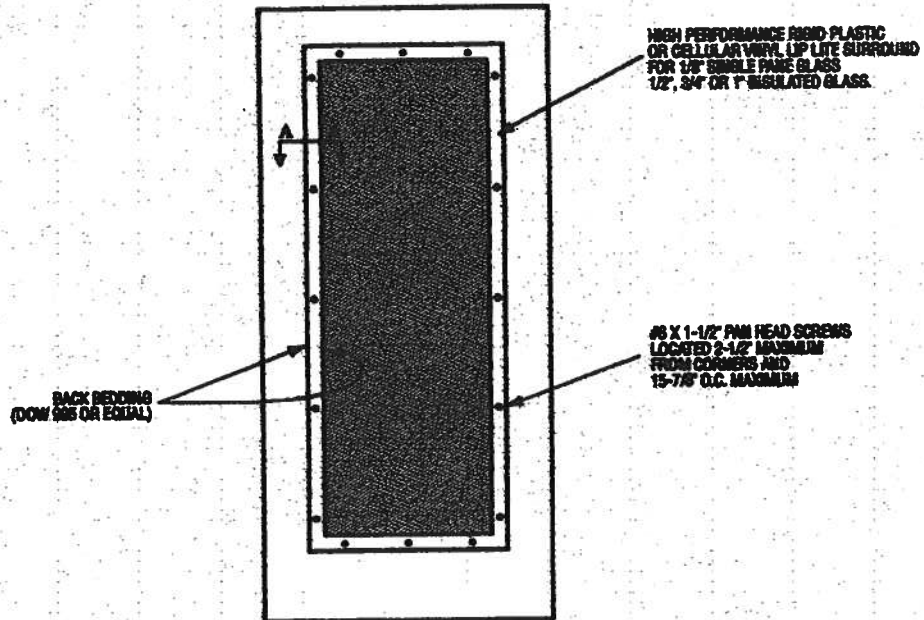


Exclusively from

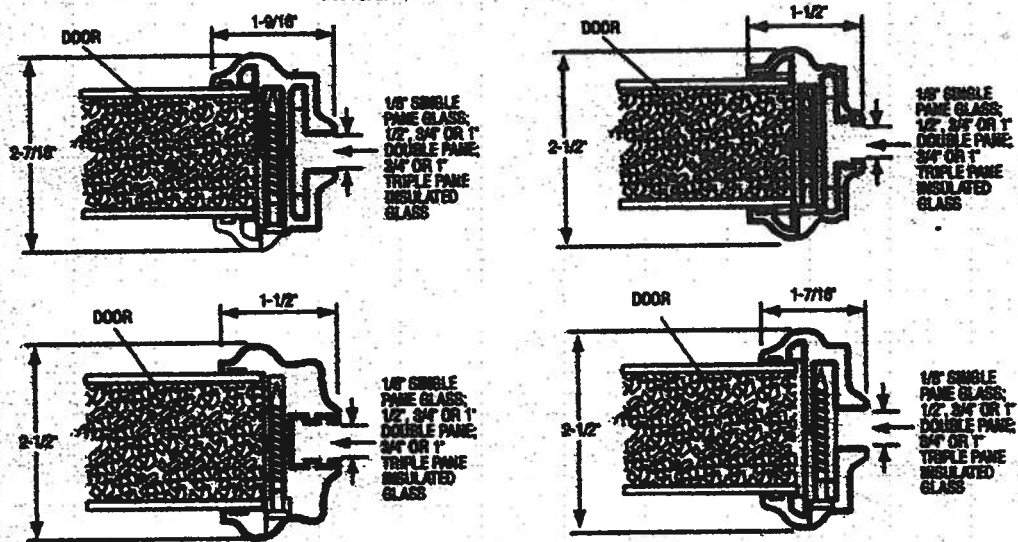
Masonite
Masonite International Corporation

MAD-WL-MAG011-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A TYPICAL RIGID PLASTIC LIP LITE SURROUND



March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation

XX

Glazed Outswing Unit

COP-VL-JHD162-02

WOOD-EDGE STEEL DOORS**APPROVED DOOR STYLES:
3/4 GLASS:**

404 Series



418 Series



450 Series

FULL GLASS:

100 Series

114, 120, 122
Series

182 Series



140 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1884-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Johnson
EntrySystems

March 29, 2002
Our continuing program of product improvement meets specifications, design and product detail subject to change without notice.

PREMDORE
Premium Quality Doors

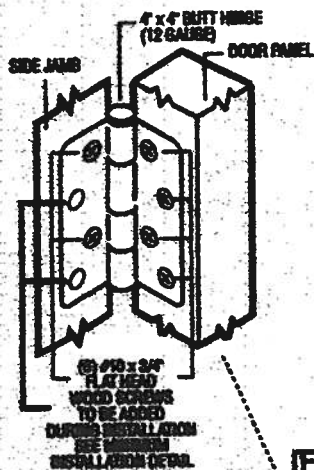
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Masonite International Corporation

XX
Unit

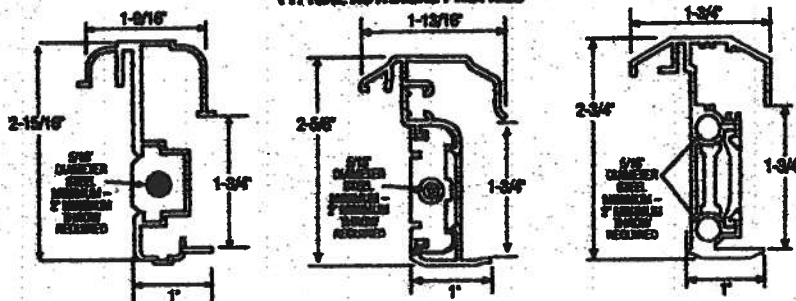
MAD-WL-MAS012-02

OUTSWING UNITS WITH DOUBLE DOOR

TYPICAL HINGE ATTACHMENT



TYPICAL ASTRAL PROFILES

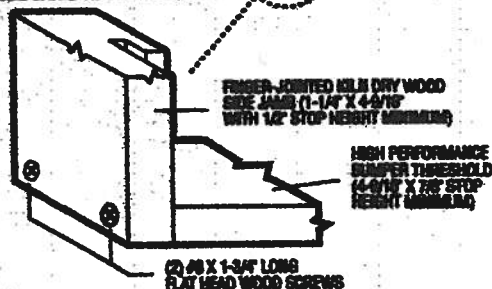


ALUMINUM EXTRUDED ASTRAL (1/8" MINIMUM WALL THICKNESS) WITH ADDED REINFORCEMENT INSERTS AT TOP EXTENSION BOLT, BOTTOM EXTENSION BOLT AND CYLINDRICAL DEADBOLT LATCHING LOCATIONS. ATTACH WITH #6 X 1" PIN HEAD SCREWS - LOCATE 1" FROM EACH END MINIMUM AND 22" O.C. MAXIMUM.

TYPICAL HEADER & SIDE JAMB ATTACHMENT



TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT



March 25, 2022
Our continuing program of product improvement makes specifications,
drawings and product detail subject to change without notice.



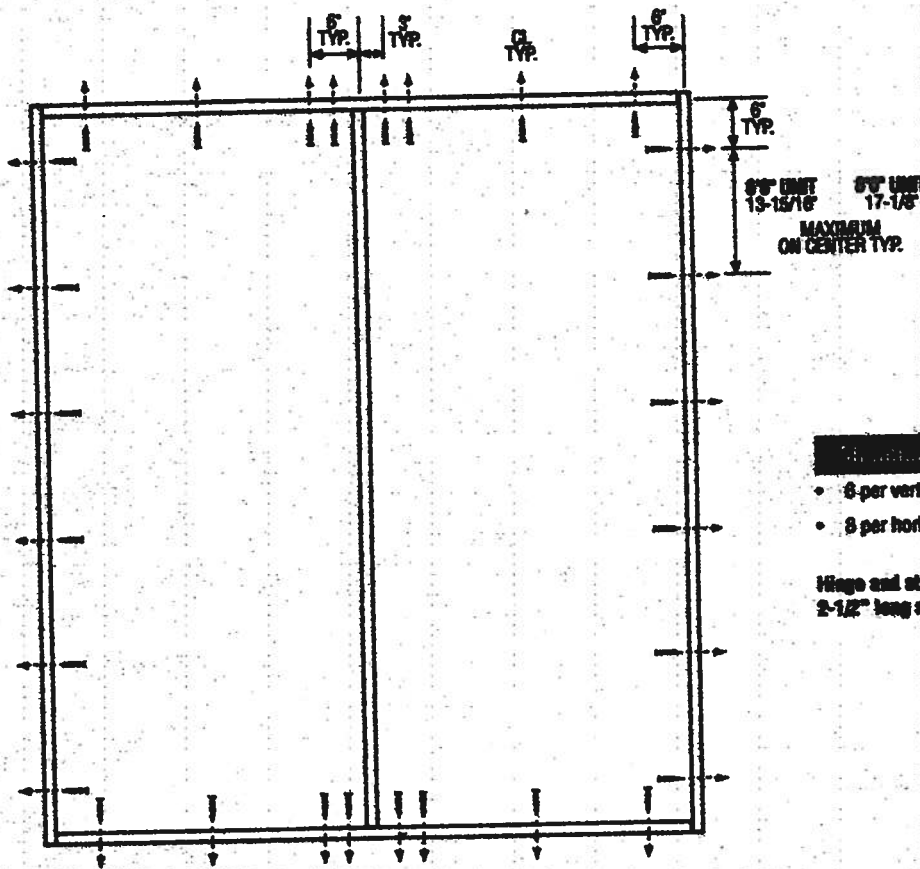
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Masonite International Corporation

XX
Unit

IND-VTL-MAG002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Code Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 22, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from

Masonite International Corporation



FEB - 4 REC'D

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

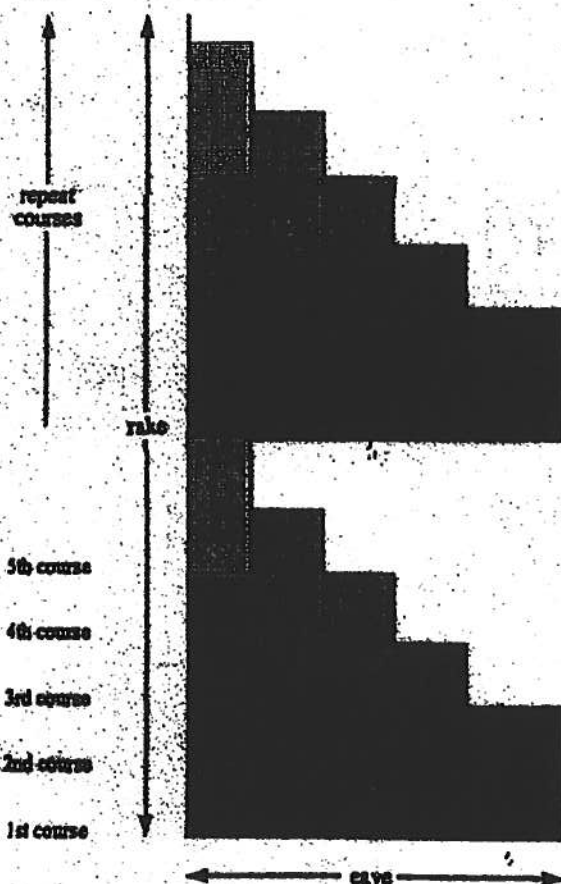
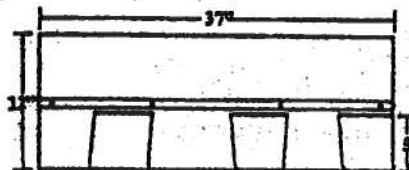
Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

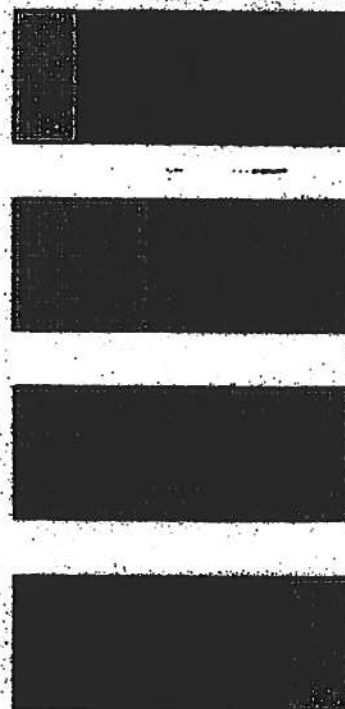


Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"



The 4 cuts in the first 10 courses:



In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.



Application Instructions for

- Glass-Seal
- Elite Glass-Seal®
- Glass-Seal AR
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER. IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 5/8 in. thick and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be at least 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.

FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.

WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These

conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

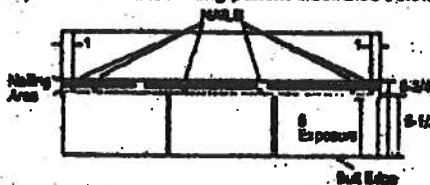
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagrams and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below.)



2) Mansard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fasteners per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

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Central District	220 West 4th St., Joplin, MO 64801	800-641-4691
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07/01

TAMKO

ROOFING PRODUCTS

(CONTINUED FROM Pg. 2)

- Glass-Seal
- Glass-Seal AR

- Elite Glass-Seal®
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a 3.25 piece and applied to shingles with a 6 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

5. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

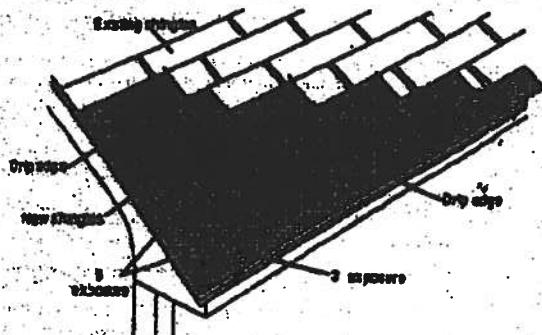
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nailing pattern described below is the preferred method for re-roofing over square tab shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Successive Courses: According to the off-set application method you choose to use, remove the appropriate length from the



take end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingles used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.

6. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 60 lb. roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

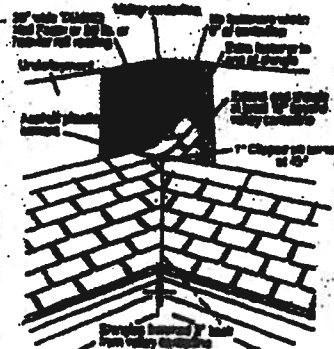
Note: For a nester installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

CAUTION: Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

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07/01



(CONTINUED from Pg. 3)

• Glass-Seal
• Glass-Seal AR

• Elite Glass-Seal®
• Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

FOR ALTERNATE VALLEY APPLICATION METHODS, PLEASE CONTACT TAMKO'S TECHNICAL SERVICES DEPARTMENT.

12. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener 5-1/2 in. back from the exposed end and 1 in. up from the edge. Do not nail directly into the sealant.

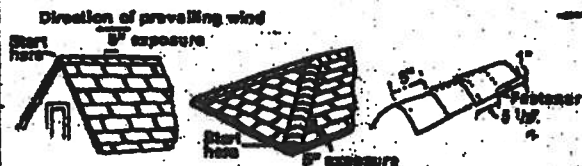
TAMKO recommends the use of TAMKO Hip & Ridge shingle products. Where matching colors are available, it is acceptable to use TAMKO's Glass-Seal or Elite Glass-Seal shingles cut down to 12 in. pieces.

NOTE: AR type shingle products should be used as Hip & Ridge on Glass-Seal AR and Elite Glass-Seal AR shingles.

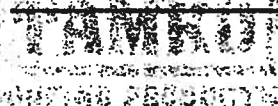
Fasteners should be 1/4 in. longer than the one used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHEN BENDING SHINGLES IN COOL WEATHER.

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.



THIS PRODUCT IS COVERED BY A LIMITED WARRANTY. THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.



SHINGLES ARE NOT TO BE USED FOR ROOFING ON ROADSIDE BUILDINGS OR OTHER STRUCTURES WHERE THEY ARE EXPOSED TO IMPACT FROM TRAFFIC OR OTHER VEHICLES.

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IMPORTANT - READ CAREFULLY BEFORE OPENING BUNDLE

In this paragraph "You" and "Your" refer to the installer of the shingles and the owner of the building on which these shingles will be installed. This is a legally binding agreement between You and TAMKO Roofing Products, Inc. ("TAMKO"). By opening this bundle You agree: (a) to install the shingles strictly in accordance with the instructions printed on this wrapper; or (b) that shingles which are not installed strictly in accordance with the instructions printed on this wrapper are sold "AS IS" and are not covered by the limited warranty that is also printed on this wrapper, or any other warranty, including, but not limited to (except where prohibited by law) implied warranties of MERCHANTABILITY and FITNESS FOR USE.

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07/01

I

**AAMA/NWDA 101/1.9.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

**SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window**

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft ²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.


Mark A. Hess, Technician

MAH:mb


1 APRIL 2002



II

Architectural Testing

AAMA/NWWDA 101/LS-2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethtown, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/LS-2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

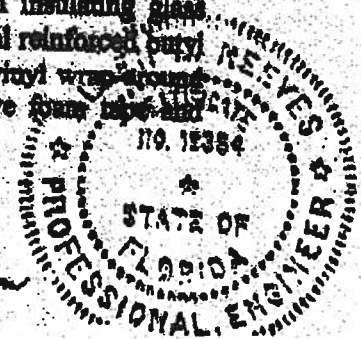
Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced vinyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. R...
1 APRIL 2002



III

Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

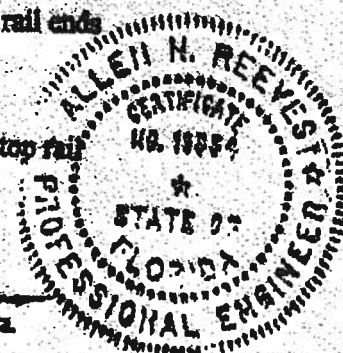
Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen N. Reeves
1 APRIL 2002



IV

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft ²	0.3 cfm/ft ² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/LS-2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.42" 0.43"	0.26" max. 0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.02" 0.02"	0.18" max. 0.18" max.
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Allen H. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

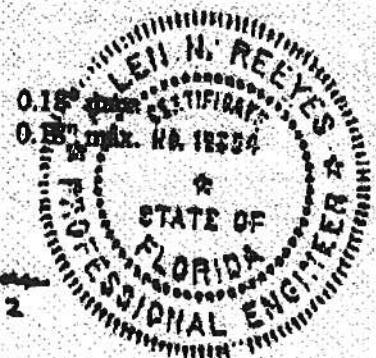
Optional Performance

4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"	0.26" max.
	@ 47.2 psf (negative)	0.46"	0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
@ 67.5 psf (positive)	0.05"	
@ 70.8 psf (negative)	0.05"	

Allen H. Reeves
1 APRIL 2002



VI

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:



Mark A. Heas
Technician

MAH:nlb
01-41134.01



Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002



Florida Building Code Online



Florida Building Code Information System

FLORIDA BUILDING CODE

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Select the organization type, status, or name to find an organization

Organization Product Manufacturer



Manufact Building

Approved Status: (All)

Organization Name: General American Door - Product Manufacturer

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Result List for Organizations

Displaying 1-1 of 1

Name	City	Contact	Phone	Type	Expires	Status
General American Door	Montgomery	James Campbell	6810597000	Product Manufacturer	01/01/2079	Approved
Org Code: PDM	System ID: 3365	Site Link: www.gadco.com				

Displaying 1-1 of 1

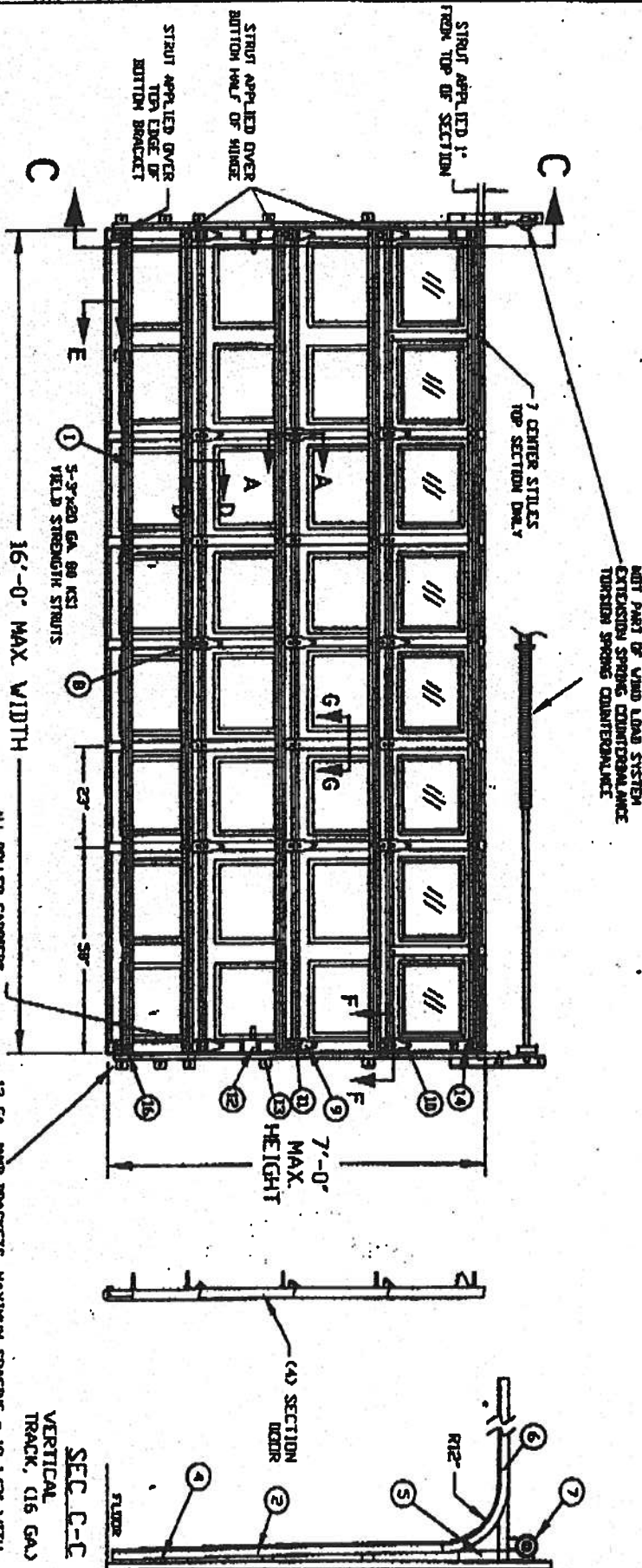
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http://www.floridabuilding.org/Common/c_org_reg_SRCH.asp

6/21/2004

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF SECTION AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. WINDOW SECTION HEIGHT: 21"
3. SECTION HEIGHTS OF 21" AND 19.5" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. VIBRODORS MAY BE INSTALLED IN THE TOP SECTION, AS TESTED WITH LOW ESS GLASS OR EQUIVALENT OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MAXIMUM LENGTH OF ROLLER STICH IS 34" OR AS TESTED
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOW.
7. STRUTS SECURED AT ALL LOCATIONS WITH TIE SCREWS.
8. QUANTITY OF TIE SCREWS CAN BE Q1, Q2 OR Q3 AS TESTED.
9. DROP IN TYPE OF INSULATION IS OPTIONAL.

**INSIDE ELEVATION**

16'-0" MAX WIDTH

ALL ROLLER CARRIERS AND HINGES ARE 14 GA.

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLIR, 2ND BRACKET NEAR THE HORIZONTAL & OF THE BOTTOM SECTION, AND 3RD BRACKET NEAR THE TOP OF THE BOTTOM SECTION.

SEC C-C

VERTICAL TRACK, (16 GA)

DESIGN LOAD +200 PSF & -200 PSF
TEST LOAD +300 PSF & -300 PSF

GENERAL AMERICAN DOOR COMPANY
5000 BASELINE ROAD
MIDLAND, TX 79701

DESIGN: JES
REVISED: (A) 11-10-00
APPROVED BY: [Signature]

16" X 7" MAX. RAISED PANEL STEEL DOOR - VIBRODOR 200 PSF

PAGE 1 OF 2
V1320-1

The seal on this drawing only illustrates the product(s) and configuration(s) of the door as tested.



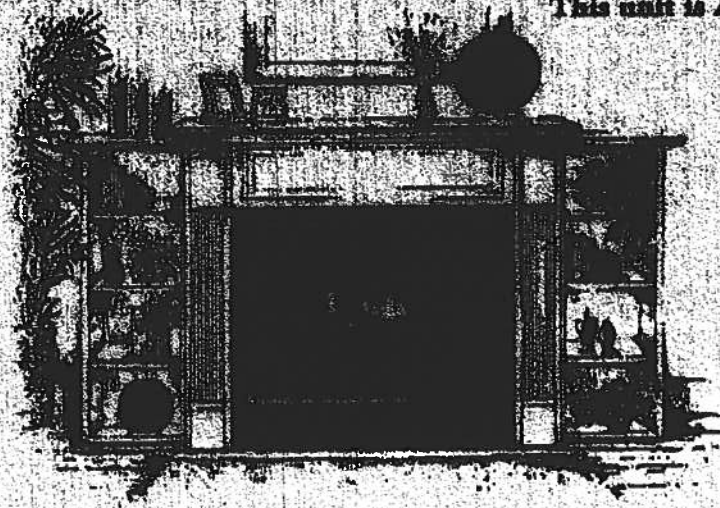
GADDO DOORS					
SERIES 7400, EXTERIOR STEEL = .017 MIN US TESTED					
SERIES 7405, EXTERIOR STEEL = .017 MIN A					
SERIES 7524, EXTERIOR STEEL = .024 MIN A					
TESTED WITH VIBRODORS					
MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL SETTING SPECIFICATIONS	STRUTS		VERTICAL TRACK
16'	7'	23°	3"	5	2 IN.

REV.	DATE	BY	REVISION
A	11-10-00	DR	SEE FIELD, EN1

AL0560

VENT-FREE

This unit is A.C.A. certified as a heater with 99% heat efficiency
 No chimney or flue system required
 Wide selection of factory installed options offered

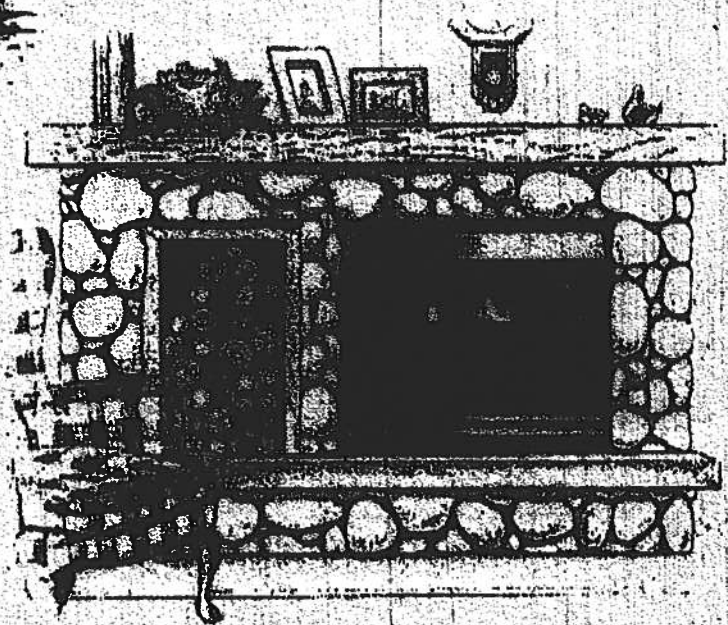


VF-4000

- 14,000 - 25,000 Btu/hr with manual control valve
- 19,500 - 25,000 Btu/hr with millivolt control valve
- Fully assembled and ready to install
- Attractive wood surrounds available
- 15" x 30" fixed or operable screen opening

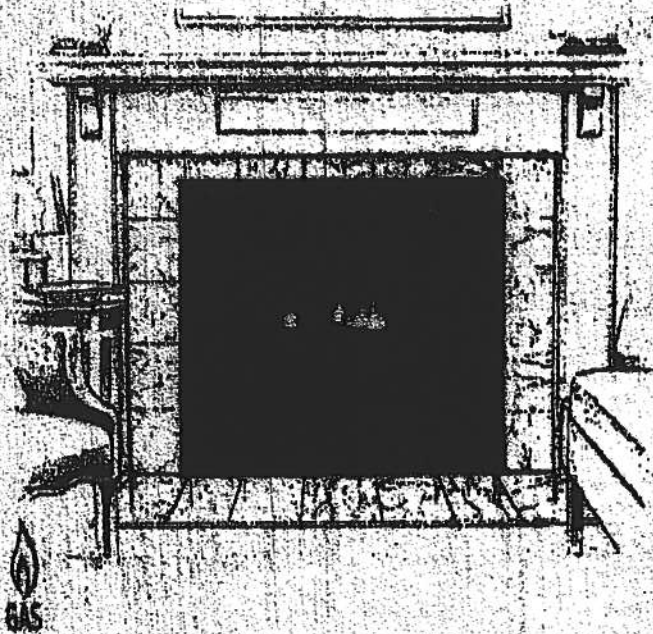
VF-5000

- 25,000 Btu/hr millivolt variable heat output
- 15" X 30" glass or screen viewing area
- Clean burning, safe and easy to install
- Realistic charred oak logs with glowing embers



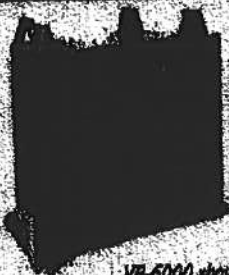
VF-6000

- 32,000 Btu/hr millivolt variable heat output
- Beautiful 20" X 34" glass or screen viewing area
- Will operate during a power failure
- Designed for large rooms



SUPERIOR

VF-1000/5000/6000



VF-6000 electric

Controls hidden in access compartment



Optional RAS-1100 Blower

Optional enter knobs, arches, glass panel and fire mesh screen

Controls hidden in access compartment



Optional RAS-1100 Blower

Milliken controls with photo function operate during a power failure

VF-4000

VF-5000/6000

Fixed or operable heavy duty mesh screen



Refractory tan brick panels



Gas box liner kit



Square brass trim kit



Brass Louver kit (For VF-4 only)



Screen panel kit (For VF-5 & VF-6 only)



Arch kit (For VF-5 & VF-6 only)



Glass door kit (For VF-5 & VF-6 only)



Brass hood (For VF-5 & VF-6 only)

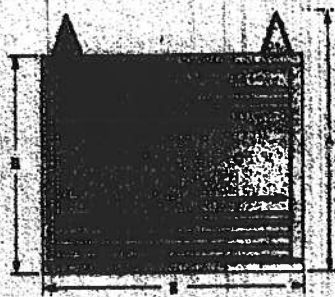


Wall switch or optional wireless remote available (For VF-4000, VF-5 & VF-6)

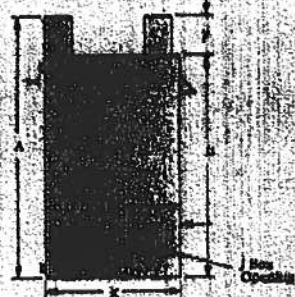


Wall thermostat (For VF-4000, VF-5 & VF-6)

Front View



Left Side View



Top View



Vene-Free Product Dimensions

	VF-4000/5000C	VF-6000C
W	11-1/2"	15-1/2"
H	30"	36"
D	5-1/2"	5-1/2"
L	3-3/4"	3-3/4"
T	3"	3"
E	27"	28-1/2"

Stn Chart

Model	Neutral	Propane
VF-4000-neutral	14,000 - 25,000	14,000 - 25,000
VF-4000/5000 dual-fuel	19,500 - 25,000	19,500 - 25,000
VF-6000	25,000 - 32,000	25,000 - 32,000

Framing Dimension

Model	Width	Height	Depth
VF-4000/5000	37"	37-1/4"	15-1/2"
VF-6000	41"	43-3/4"	19-1/8"

NOTE: Diagrams and illustrations are not to scale. Product designs, materials, dimensions, specifications, colors and prices subject to change or discontinuation without notice. Built to ANSI Z39.1 L2 standard and approved by A.S.A. (Report # 12870017).

Consult your distributor for local fireplace code information.



SUPERIOR

www.LennoxHearthProducts.com

SURROUNDS

The Charleston Poplar Surround is hand crafted using a combination of solid Poplar and Poplar veneer. Using the unique wood type of Poplar allows you the option to paint or stain this elegantly detailed surround. The surround is constructed using easy to assemble cam locks, and available in corner and wall units.



Distributed by:

P/N 40444 REV. 2/01

May 01 2003 07:51AM P2

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FAX NO.: +386 758 4735

FROM: LAKE CITY INDUSTRIES

c. Crawl space (if applicable)

☒☐**b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiteicide or alternative method)
11. Slab on grade
 - a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

☐☐**c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)****Floor Framing System:**

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout**Electrical layout including:**

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment

HVAC information

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)**Gas System Type (LP or Natural) Location and BTU demand of equipment****Disclosure Statement for Owner Builders****Notice Of Commencement****Private Potable Water**

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☒☐☐☐

Notice of Treatment

Applicator: **Florida Pest Control & Chemical Co. (www.flapest.com)**

Address: 53658 Baya Ln.

City Lake City Phone 255-0043

Site Location: Subdivision 188 NW Ambleside

Lot # 30 Block# Permit # 24644

Address Cobblestone off Lake Jeffery

<u>Product used</u>	<u>Active Ingredient</u>	<u>% Concentration</u>
---------------------	--------------------------	------------------------

☒ Premise	Imidacloprid	0.1%
---	--------------	------

☐ Termidor	Fipronil	0.12%
-----------------------------------	----------	-------

☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%
------------------------------------	----------------------------------	-------

Type treatment:

☒ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

Main Body only
of Garage
1x Driveway

941

124

20 gal

3000

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

Date

Time

Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05



Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1SV6487-Z0103103143

Truss Fabricator: Anderson Truss Company
Job Identification: 6-063--Sparks Construction Hauck -- Lot #30 Cobblestone , **
Truss Count: 54
Model Code: Florida Building Code 2004
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.24.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed



Seal Date: 03/03/2006

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

-Truss Design Engineer-
Arthur R. Fisher

Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

Details: A11015EE-GBLLETIN-BRCLBSUB-A11030EE-PIGBACKB-

#	Ref	Description	Drawing#	Date
1	89671--A1-GE		06062008	03/03/06
2	89672--A2		06062009	03/03/06
3	89673--A3		06062010	03/03/06
4	89674--A4		06062011	03/03/06
5	89675--A5		06062012	03/03/06
6	89676--A6-GE		06062013	03/03/06
7	89677--A7		06062014	03/03/06
8	89678--A8		06062015	03/03/06
9	89679--B1-GE		06062016	03/03/06
10	89680--B2		06062017	03/03/06
11	89681--B3		06062018	03/03/06
12	89682--C1-GE		06062019	03/03/06
13	89683--C2		06062020	03/03/06
14	89684--C3		06062021	03/03/06
15	89685--DMG		06062022	03/03/06
16	89686--D1-GE		06062023	03/03/06
17	89687--D2		06062001	03/03/06
18	89688--D3		06062024	03/03/06
19	89689--D4		06062025	03/03/06
20	89690--FG		06062026	03/03/06
21	89691--HJK		06062027	03/03/06
22	89692--EJK		06062028	03/03/06
23	89693--J5K		06062029	03/03/06
24	89694--J5		06062030	03/03/06
25	89695--J3		06062031	03/03/06
26	89696--J1		06062032	03/03/06
27	89697--K1-GE		06062033	03/03/06
28	89698--K2		06062002	03/03/06
29	89699--K3G		06062034	03/03/06
30	89700--K4		06062035	03/03/06
31	89701--K5		06062036	03/03/06
32	89702--K6		06062037	03/03/06
33	89703--K7		06062038	03/03/06
34	89704--K8G		06062039	03/03/06
35	89705--KK9		06062003	03/03/06
36	89706--KK10		06062040	03/03/06

#	Ref	Description	Drawing#	Date
37	89707--KK11		06062041	03/03/06
38	89708--KK12		06062004	03/03/06
39	89709--KK13		06062042	03/03/06
40	89710--KK14		06062043	03/03/06
41	89711--O1		06062005	03/03/06
42	89712--AP1-GE		06062044	03/03/06
43	89713--AP2		06062045	03/03/06
44	89714--BP1-GE		06062046	03/03/06
45	89715--BP2		06062047	03/03/06
46	89716--RP1-GE		06062048	03/03/06
47	89717--RP2		06062049	03/03/06
48	89718--R1-GE		06062050	03/03/06
49	89719--R2		06062051	03/03/06
50	89720--R3G		06062052	03/03/06
51	89721--R4-GE		06062053	03/03/06
52	89722--S1-GE		06062054	03/03/06
53	89723--S2		06062006	03/03/06
54	89724--S3		06062007	03/03/06





623-0575 Cell
Josh

Alpine Engineering

Top chord 2x6 SP #1 Dense :T1 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

- SPECIAL LOADS
- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
 - TC - From 61 PLF at -1.50 to 61 PLF at 20.37
 - TC - From 64 PLF at 20.37 to 64 PLF at 26.67
 - TC - From 61 PLF at 26.67 to 61 PLF at 40.09
 - TC - From 194 PLF at 40.09 to 199 PLF at 40.67
 - TC - From 203 PLF at 40.67 to 293 PLF at 49.92
 - BC - From 4 PLF at -1.50 to 4 PLF at 0.00
 - BC - From 20 PLF at 0.00 to 20 PLF at 49.92
 - TC - 418 LB Conc. Load at 49.92

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

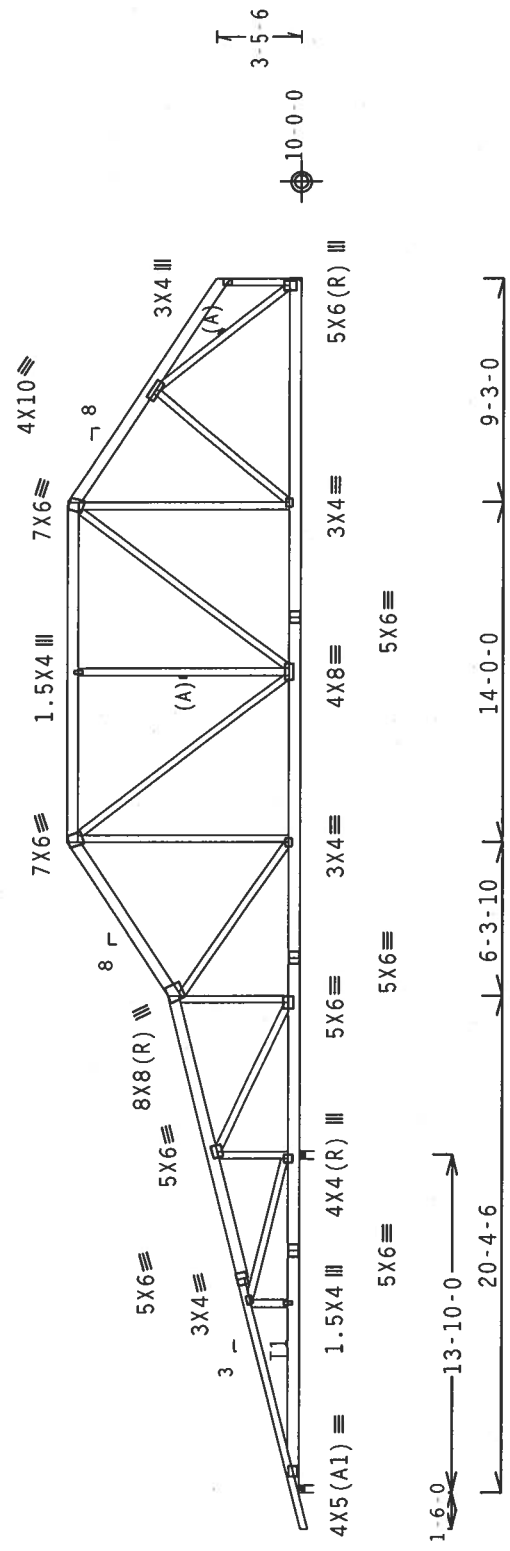
Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



49-11-1 Over 3 Supports
R-426 U=180 W=3.5" R-2562 U=274 W=4"

R-3385 U=362 H=Simpson HGUS26
w/ (8) 10d Common, 0.148"x3.0" nails in Truss
w/ (20) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

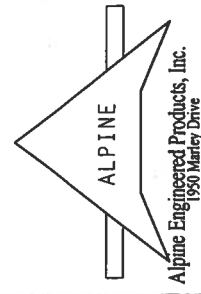
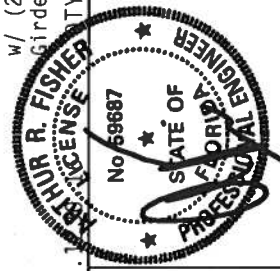
Scale = .125"/Ft.

PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE DESIGNER ACCEPTS NO LIABILITY FOR THE TRUSS COMPONENTS. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (NATIONAL DESIGN SPEC. IN ACCEPTANCE OF THE AIA) AND THE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W-H/S/K) ASTM A653 GRADE 40/50 (W, K/H S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THE CIRCULAR SEAL AND USE OF THIS DOCUMENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE USER.

TC LL	20.0 PSF	REF	R487 -- 89672
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062009
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87261
DUR.FAC.	1.25		



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Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

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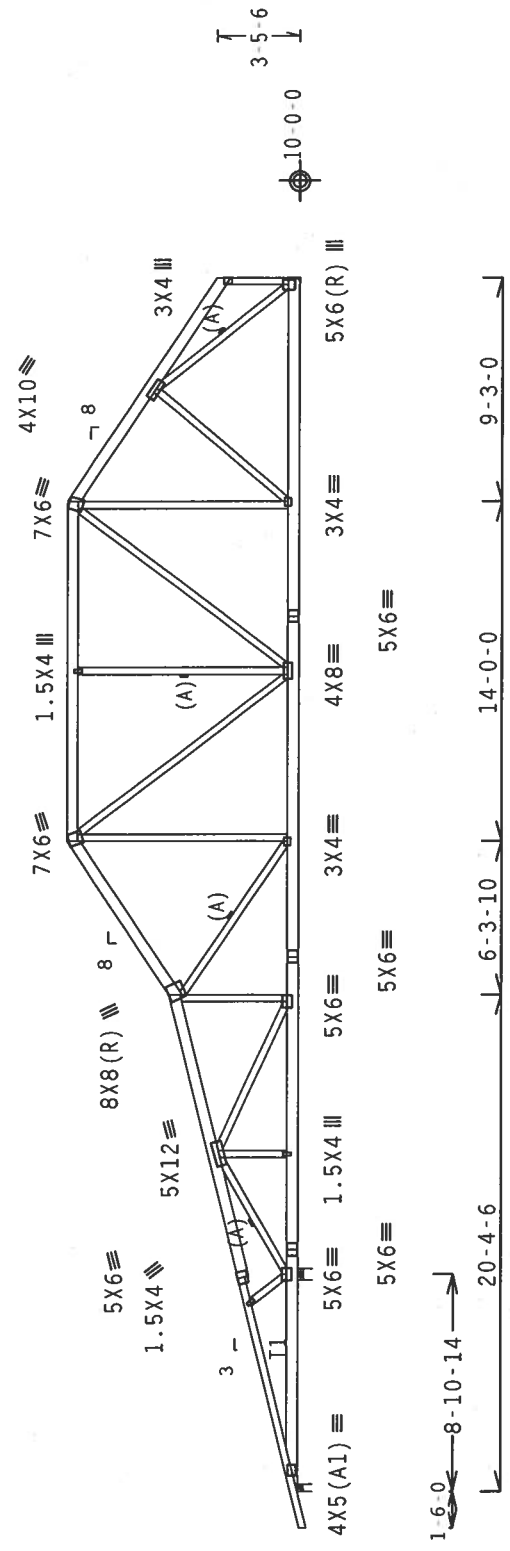
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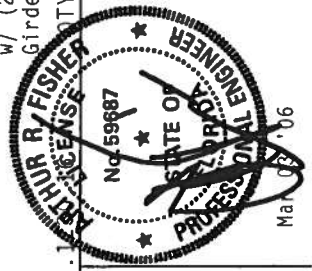
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49-11-1 Over 3 Supports

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Cq/RT=1.00(1.25)/10(0) 7.24.1

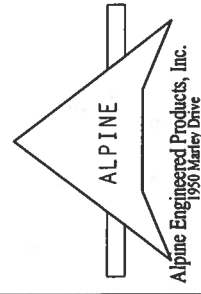
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PLT TYP. Wave



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IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE DESIGN REQUIREMENTS MAY BE THE RESULT OF THE INSTALLATION CONTRACTOR'S NEGLIGENCE. THE DESIGN CONTRACTOR AND THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS. THE TRUSS SHALL BE MADE OF 20/18/16GA (40 V/24) ASTM A453 GRADE 40/50 (40 V/24) GALV STEEL. THE TRUSS SHALL BE MADE OF 20/18/16GA (40 V/24) ASTM A453 GRADE 40/50 (40 V/24) GALV STEEL. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS TO THE DESIGN REQUIREMENTS.



TC LL	20.0 PSF	REF	R487--	89673
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062010
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87271	
DUR.FAC.	1.25			

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Webs 2x4 SP #3

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 - BC - From 4 PLF at 1.50 to 4 PLF at 0.00
 - BC - From 20 PLF at 0.00 to 20 PLF at 49.92

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

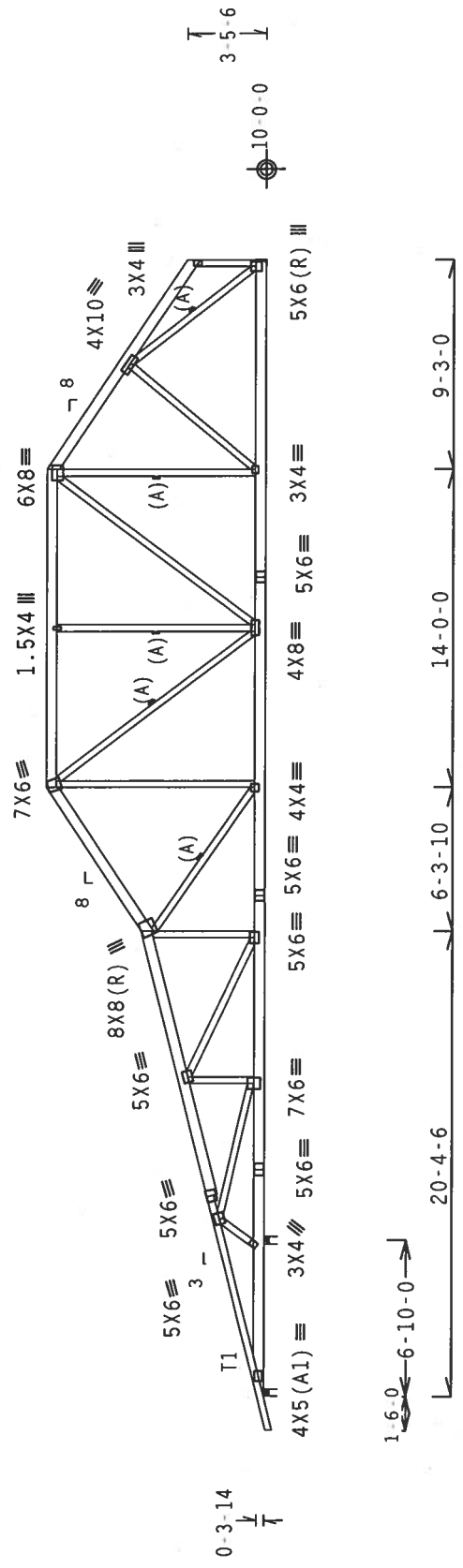
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



49-11-1 Over 3 Supports

R-81 U-180 W=3.5"

R-2359 U-252 W=4"

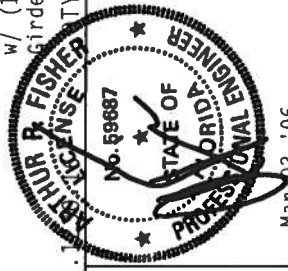
R-1734 U-186 H-Simpson HUS26

w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2)2X6 min. So.Pine

Scale = .125"/Ft.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

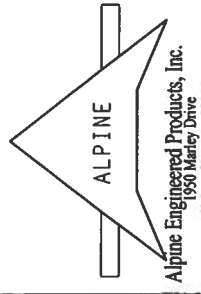
PLT TYP. Wave



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1.03 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 HARRISON, MI 48031-1571) AND WCA (WOOD CONSTRUCTION ASSOCIATION, 6300 ENTERPRISE LN, HARRISON, MI 48031-1571) FOR CLARIFICATION. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFPA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (W. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

TC LL	20.0 PSF	REF	R487 -- 89674
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062011
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87282
DUR.FAC.	1.25		



	Top chord	2x6	SP	#1	Dense	:T1	2x4	SP	#2	Dense:
Bot	chord	2x6	SP	#1	Dense					
	Webbs	2x4	SP	#3						

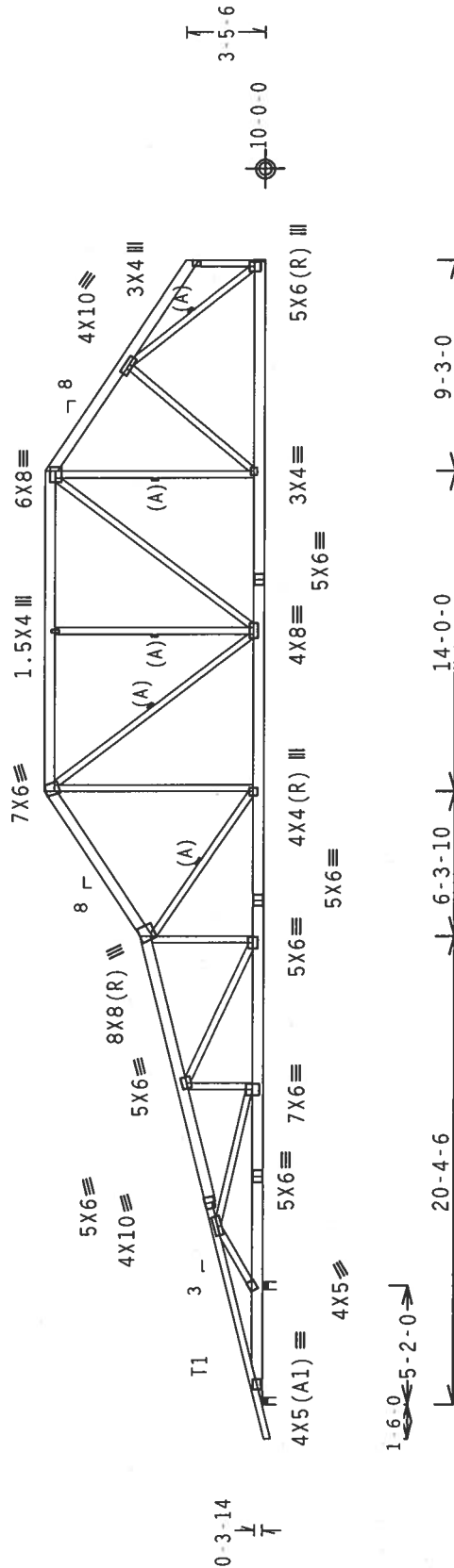
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

SPECIAL LOADS

			(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC	From	61 PLF at -1.50 to	61 PLF at 20.37
TC	From	64 PLF at 20.37 to	64 PLF at 26.67
TC	From	61 PLF at 26.67 to	61 PLF at 40.67
TC	From	64 PLF at 40.67 to	64 PLF at 49.92
BC	From	4 PLF at -1.50 to	4 PLF at 0.00
BC	From	20 PLF at 0.00 to	20 PLF at 49.92

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



-49-11-1 Over 3 Supports

R=-34 U=180 W=3.5"

R-1800 U-193 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

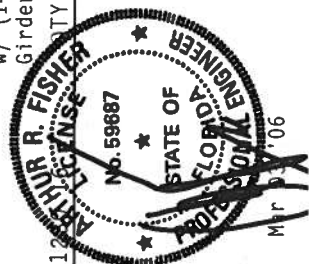
7.24.128

Scale = .125"/Ft.

WARNING RUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. *ATTENTION* BECAUSE OF THE COMPLEX NATURE OF THIS PLATING SYSTEM, IT IS RECOMMENDED THAT YOU CONTACT ONE OF THE FOLLOWING FOR ASSISTANCE. IF YOU ARE NOT SURE OF THE PROPER PROCEDURE, STOP! DO NOT PROCEED UNTIL YOU HAVE BEEN ADVISED BY AN AUTHORIZED PERSONNEL FROM THE ABOVE LISTED SOURCES.

D'ONOFIO DR., SUITE 200, MADISON, WI 53719, AND WCA WOOD TRUSS CO., SUITE 100, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED.

TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

[illegible]

TC LL	20.0	PSF	REF R487-- 89675
TC DL	10.0	PSF	DATE 03/03/06
BC DL	10.0	PSF	DRW HCURS487 06062012
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 87289
DUR.FAC.	1.25		

Top chord 2x6 SP #1 Dense :T1 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W17 2x4 SP #2 Dense:

- SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 61 PLF at -1.50 to 61 PLF at 20.37
TC - From 64 PLF at 20.37 to 64 PLF at 26.67
TC - From 61 PLF at 26.67 to 61 PLF at 40.67
TC - From 64 PLF at 40.67 to 64 PLF at 49.92
BC - From 4 PLF at -1.50 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 49.92

WARNING: Furnish a copy of this DWG to the installation contractor.
Special care must be taken during handling, shipping and installation of
trusses. See "WARNING" note below.

+ MEMBER TO BE Laterally Braced For Horizontal Wind Loads.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

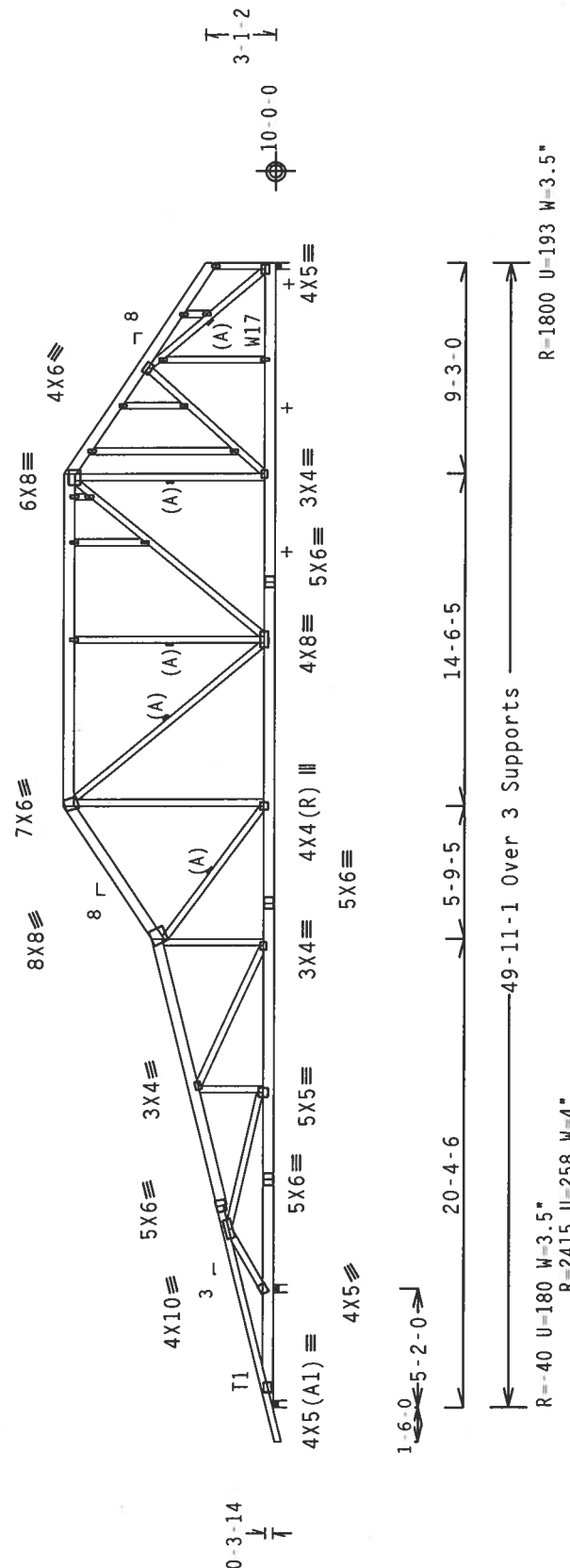
Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC
@ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND
CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS.
SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END.
ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

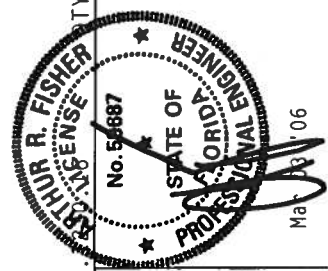
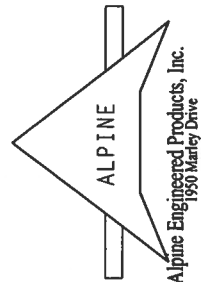
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.

PLT TYP. Wave

Scale = .125" / Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583
D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN,
MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED,
ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED
PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THIS DESIGN. ANY FAILURE TO BUILD THE
TRUSS IN CONFORMANCE WITH THE MANUFACTURING INSTRUCTIONS AND THE TRUSS
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI (ALPINE
CONNECTOR PLATES ARE MADE OF 20/18/16GA (H-H/S/K) ASTM A653 GRADE 40/60 (H, K/H/S) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE



TC LL	20.0 PSF	REF	R487--	89676
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062013
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87365	
DUR.FAC.	1.25			

Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

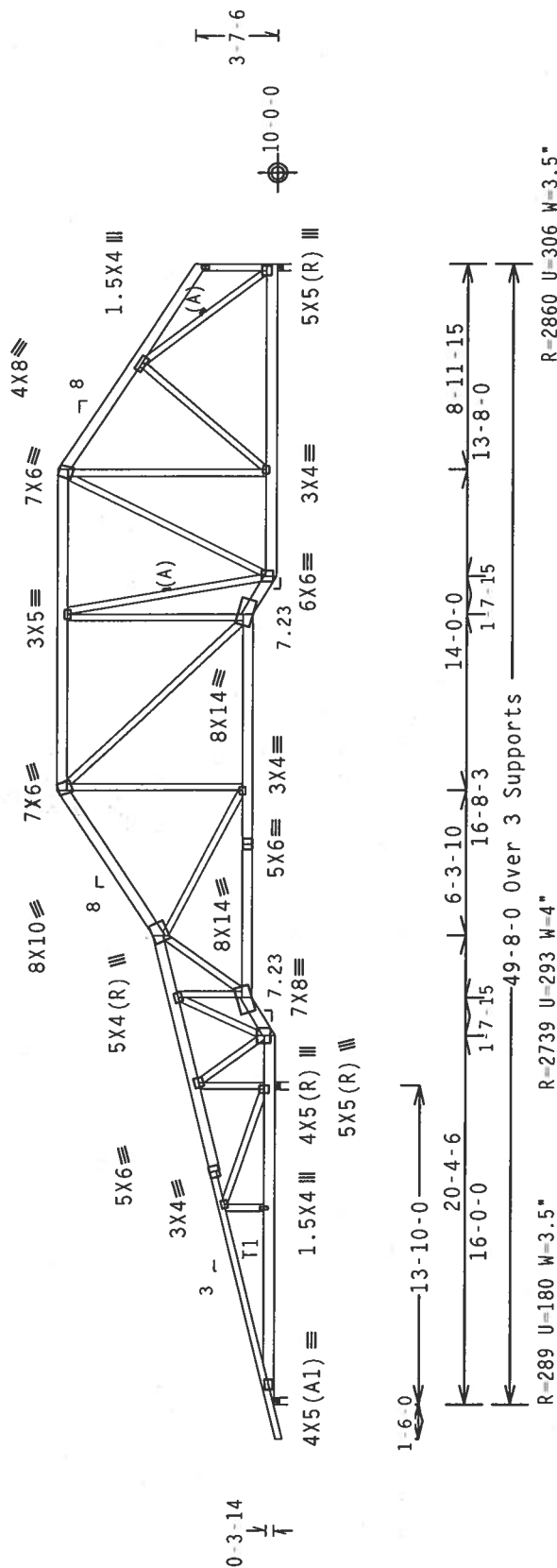
Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

7.24.12

El 1-141-1-181-

Scale = 125"/Ft

EE D487 -- 80678

DATE 03/03/00

DATE: 03/03/06

IRW HCUSR487 06062015

IC-ENG JB/AF

EQN- 87408

100

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE ALPINE ENGINEER DESIGN IS SUBJECT TO ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE TRUSSER. THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., B. BEAMAN AND TRUSS CONNECTOR PLATES ARE MADE OF 2018/1864 (M.W./THS) ASTM A653 GRADE 40/60 (M. V.H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAWINGS 160A-Z. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 43 OF TPII 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



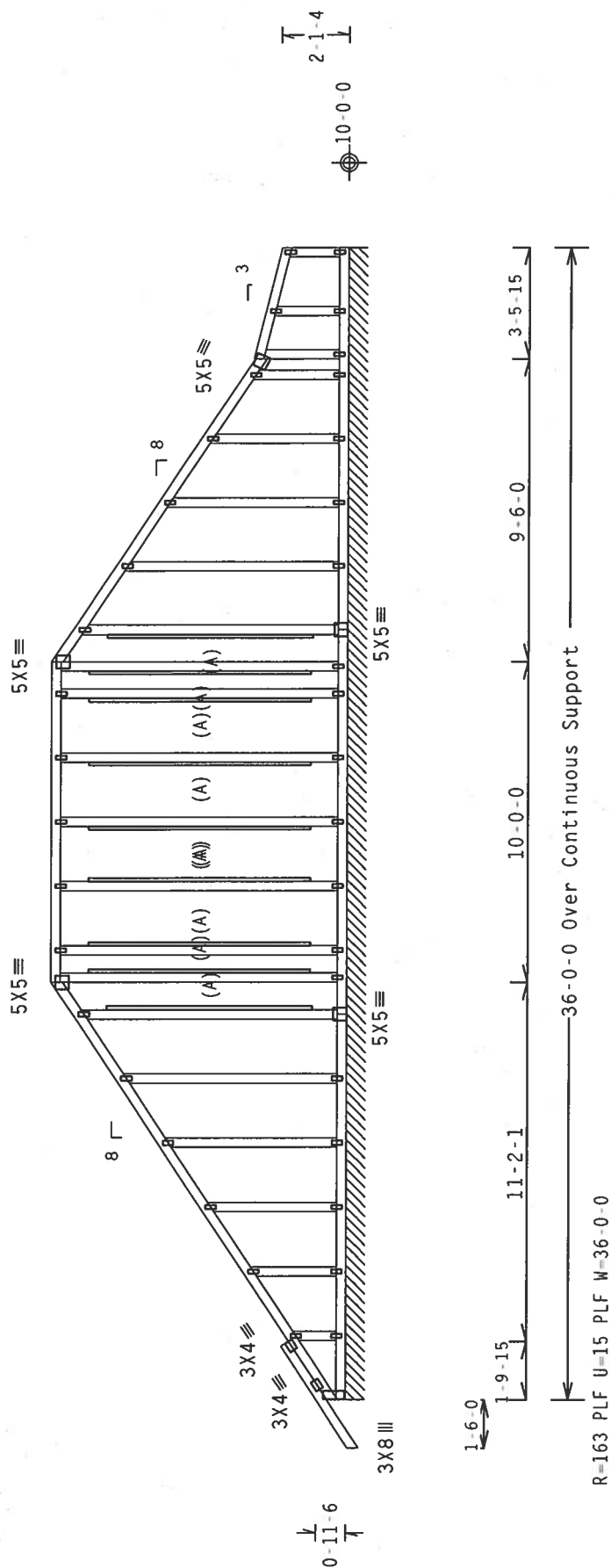
ALPINE
Alpine Engineered Products, Inc.
1950 Meriden Drive

	Top chord	2x4	SP	#2	Dense
	Bot chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers.
Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or
notched.

See DWGS A11015EE0405 & GBULLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

FL/-14/-1-1R/-

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BCSP 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 W. O'HANLON DR., SUITE 200, MADISON, WI 53719, AND WCA WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE DR., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE PROVISIONS OF THIS DESIGN MAY BE CAUSAL TO THE TRUSSING CONTRACTOR'S NON-CONFORMANCE WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY ALPINE ENGINEERING) AND THE ALPINE DESIGNER'S DESIGN. ALL TRUSSING MATERIALS SHALL BE PROVIDED BY THE TRUSSING CONTRACTOR. ALL CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H-S/GA) ASTM A663 ON THIS DESIGN (W/ K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, WHEN OTHERWISE LOCATED ON THE TRUSSING PER DRAWINGS 160A-Z. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.



Alpine Engineered Products, Inc.
1950 Marley Drive

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

: Lt Slider 2x6 SP #1 Dense: BLOCK LENGTH = 2.078'

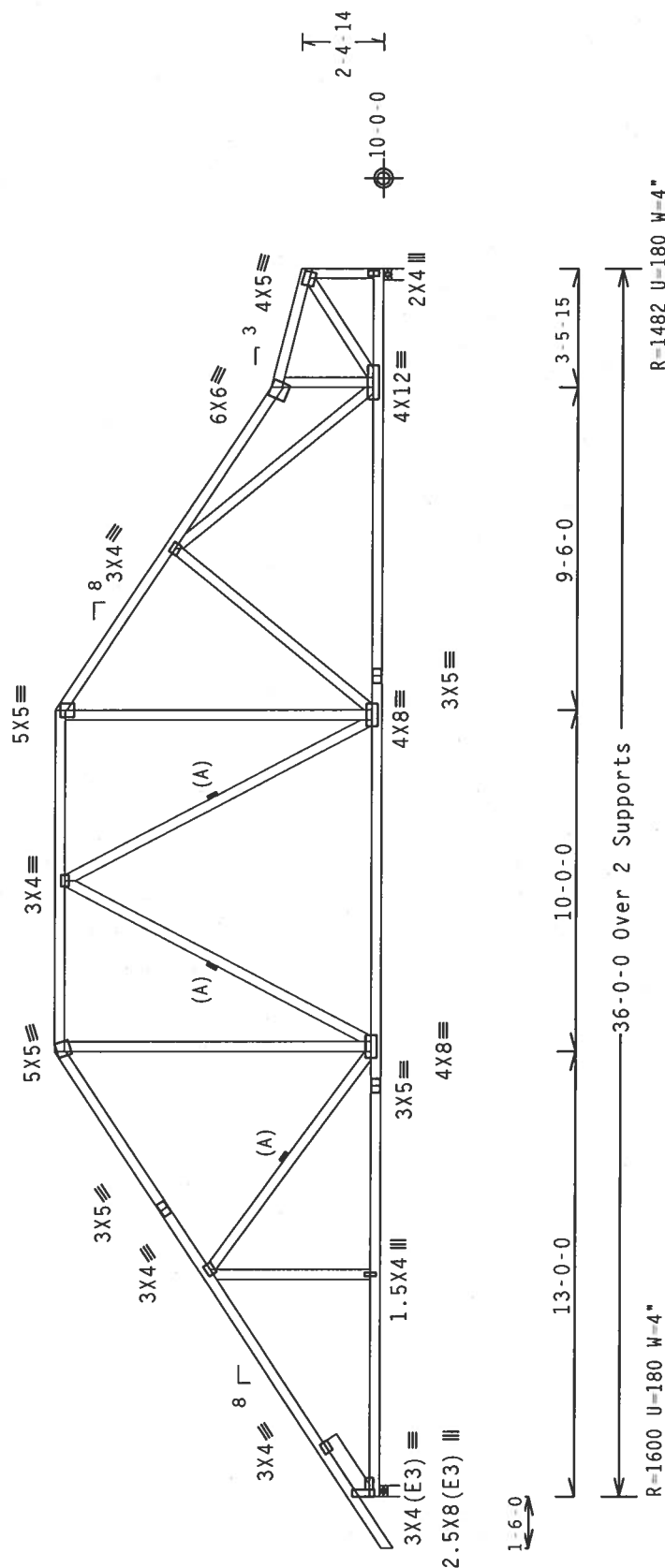
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC
Ca/RT=1.00(1.25)/10(0) 7.2

PLT TYP. Wave

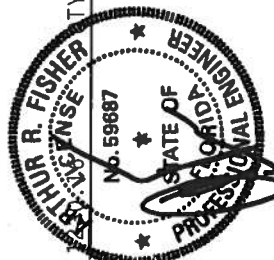
7.24.

FL1-141-1-1R1-

Scale = 1875"/Ft

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE NATIONAL BUREAU OF STANDARDS, 4800 RILEY DRIVE, SUITE 200, MADISON, WI 53719, AND NRC 4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE DRIVE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY DEVIATION FROM THIS DESIGN, OR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH RESPONSIBLE PRACTICES, SHALL BE THE RESPONSIBILITY OF THE DESIGNER. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIAA) AND THE ALPINE DESIGN CRITERIA. THE TRUSS IS DESIGNED FOR A DEAD LOAD OF 10 PSF AND A LIVE LOAD OF 20 PSF. THE ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA. (W/H/S/C) ASTM A653 GRADE 40/60 (W. K/H/S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF 1P11-2002 SEC.3. A SEAL ON THIS ANNEX SHALL BE THE SUFFICIENT INDICATION OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN, SHOWING THE SUITABILITY OF THE DESIGN FOR THE TRUSS COMPONENT.



Mar 03 '06

Alpine Engineered Products, Inc.
1950 Marley Drive

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

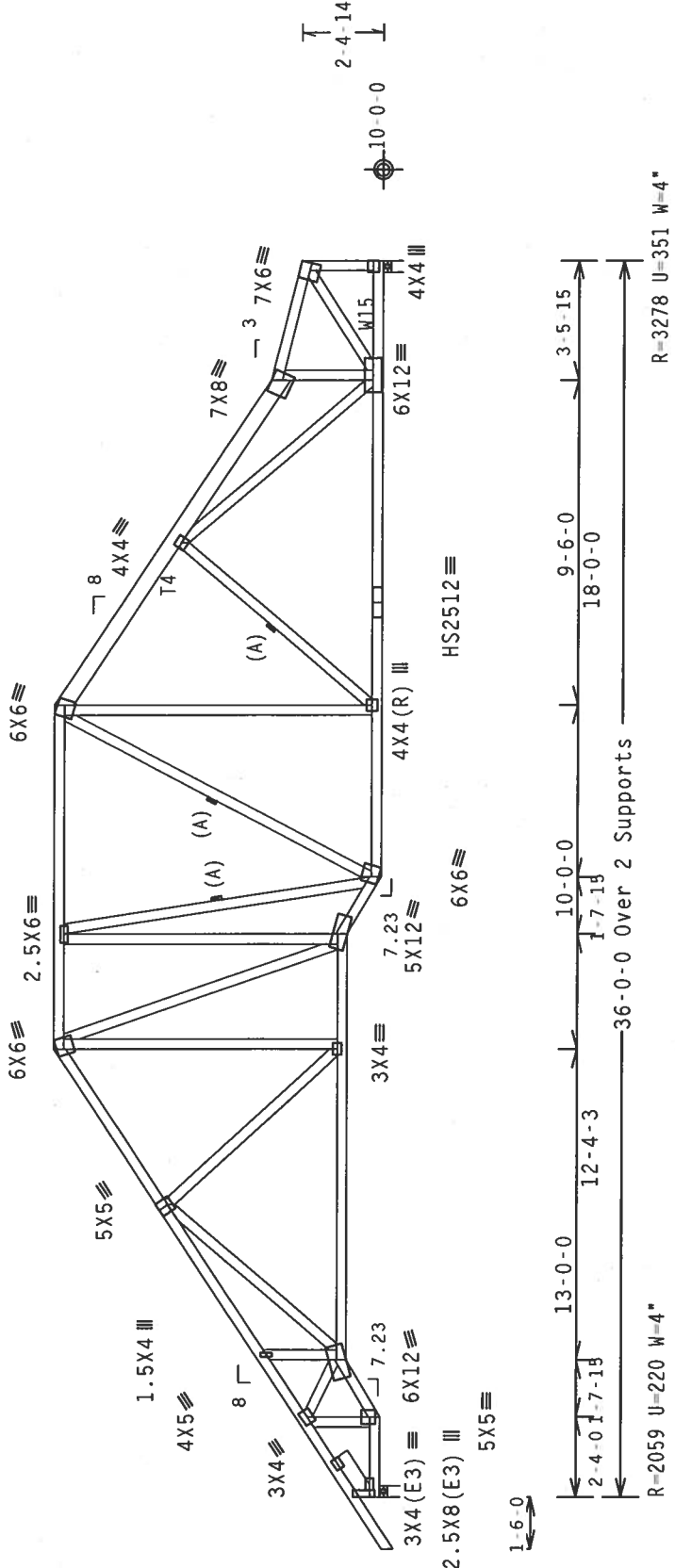
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x4 SP #2 Dense : 14 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense
Webs 2x4 Sp #3 : W15 2x4 SP #2 Dense:
: Lt Slider 2x6 SP #1 Dense: BLOCK LENGTH = 1.500'

SPECIAL LOADS

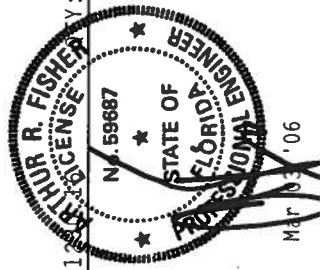
TC	From	64 PLF at 1.50 to	64 PLF at 13.00 to	(LUMBER DUR.FAC. = 1.25 / PLATE DUR.FAC. = 1.25)
TC	From	61 PLF at 13.00 to	61 PLF at 22.42 to	
TC	From	194 PLF at 22.42 to	199 PLF at 23.00 to	
TC	From	293 PLF at 23.00 to	293 PLF at 32.50 to	
TC	From	61 PLF at 32.50 to	61 PLF at 36.00 to	
BC	From	5 PLF at 1.50 to	5 PLF at 0.00 to	
BC	From	20 PLF at 0.00 to	20 PLF at 2.33 to	
BC	From	23 PLF at 2.33 to	23 PLF at 3.99 to	
BC	From	20 PLF at 3.99 to	20 PLF at 16.34 to	
BC	From	23 PLF at 16.34 to	23 PLF at 18.00 to	
BC	From	20 PLF at 18.00 to	20 PLF at 36.00 to	
TC	-	418 LB Conc. Load at 32.50		



Design Crit: TPI-2002(STD)/FBC

Scale = .1875"/Ft.

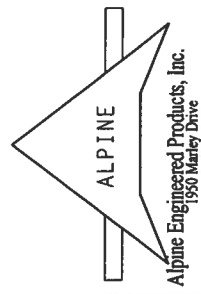
TC LL	20.0 PSF	REF	R487--	89681
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062018
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87218	
DUR.FAC.	1.25			



Wave	Design Crit: TPI-2002(STD)/FBC
	$C_q/RT=1.00(1.25)/10(0)$
	7.

***WARNING:** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI, TRUSS PLATE, A INSTITUTION FOR THE TRUSS INDUSTRY, 10000 W. 10TH AVE., SUITE 200, MADISON, WI 53719, AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, TRUSS COMPANY, INC., WHO HAS RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN. THE TRUSS COMPANY SHALL BE RESPONSIBLE FOR ANY FAILURE OF THE TRUSS IN CONFORMANCE WITH TPI. OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ASEA) AND ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (M/H/S/K) ASTM A653 GRADE 40/60 (F. Y/H.S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 43 OF TPI 2002 SEC. 3. THE TRUSS COMPANY SHALL BE RESPONSIBLE FOR PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN. SHOW THE SUFFICIENCY OF THE TRUSS COMPANY'S DESIGN.



	SP	#2	Dense	:T3	2x6	SP	#1	Dense:
top chord	2x4	SP	#2	Dense				
bot chord	2x4	SP	#2	Dense				
webs	2x4	SP	#3					

11110 mph wind, 15.68 ft mean hgt., ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

and verticals not exposed to wind pressure.

See DWGS A11030EE0405 & GBULLETIN0405 for more requirements.

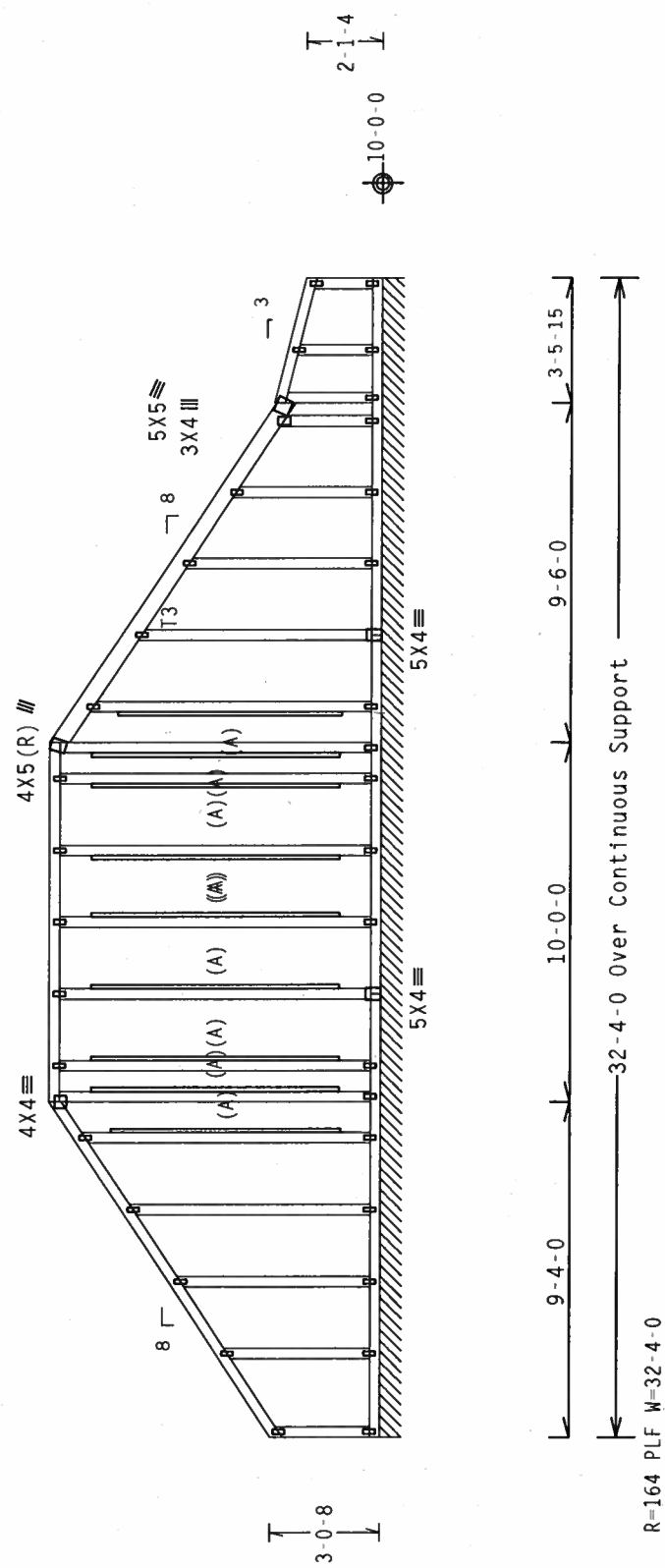
1x4 SP #3 or better "L" brace. 80% length of web member. Attach with 2d Box or Gun (0.113"x2.5",min.)nails@ 6" OC.

deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord out lookers. Cladding load shall not exceed 10.00 PSF. Top chord not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Fasten rated sheathing to one face of this frame.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLOT TYP. Wave

7.24.1

FL/-/4/-/-/R/-/-

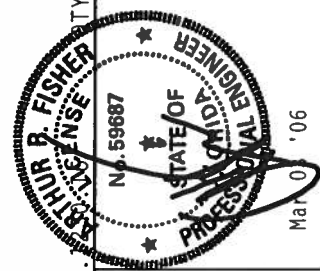
Scale = .1875"/Ft.



Alpine Engineered Products, Inc.
1950 Marley Drive

*WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSI 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TP1 (TRUSS PLATE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

* IMPORTANT * FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE PRODUCTS IN ACCORDANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. FOR WOOD JOINTS AND CONNECTOR PLATES ARE MADE OF 20/18/166GA (H-V/S/45) ASTM A653 GRADE 40/50 (M. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP11 2002 SEC.3. A SEAL ON THIS DRAWING SHALL BE REQUIRED TO VERIFY THE RESPONSIBILITY OF THE CONTRACTOR TO SOLIDIFY THE TRUSS COMPONENT. THE CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY THE SUITABILITY AND FITNESS OF THIS COMPONENT FOR THE DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY THE SUITABILITY AND FITNESS OF THIS COMPONENT FOR THE DESIGN.



TC LL	20.0 PSF	REF R487-- 89682
TC DL	10.0 PSF	DATE 03/03/06
BC DL	10.0 PSF	DRW HCUSR487 06062019
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 87236
DUR.FAC.	1.25	

Top chord 2x4 SP #2 Dense :T3 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W11 2x4 SP #2 Dense:

110 mph wind, 16.01 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

SPECIAL LOADS

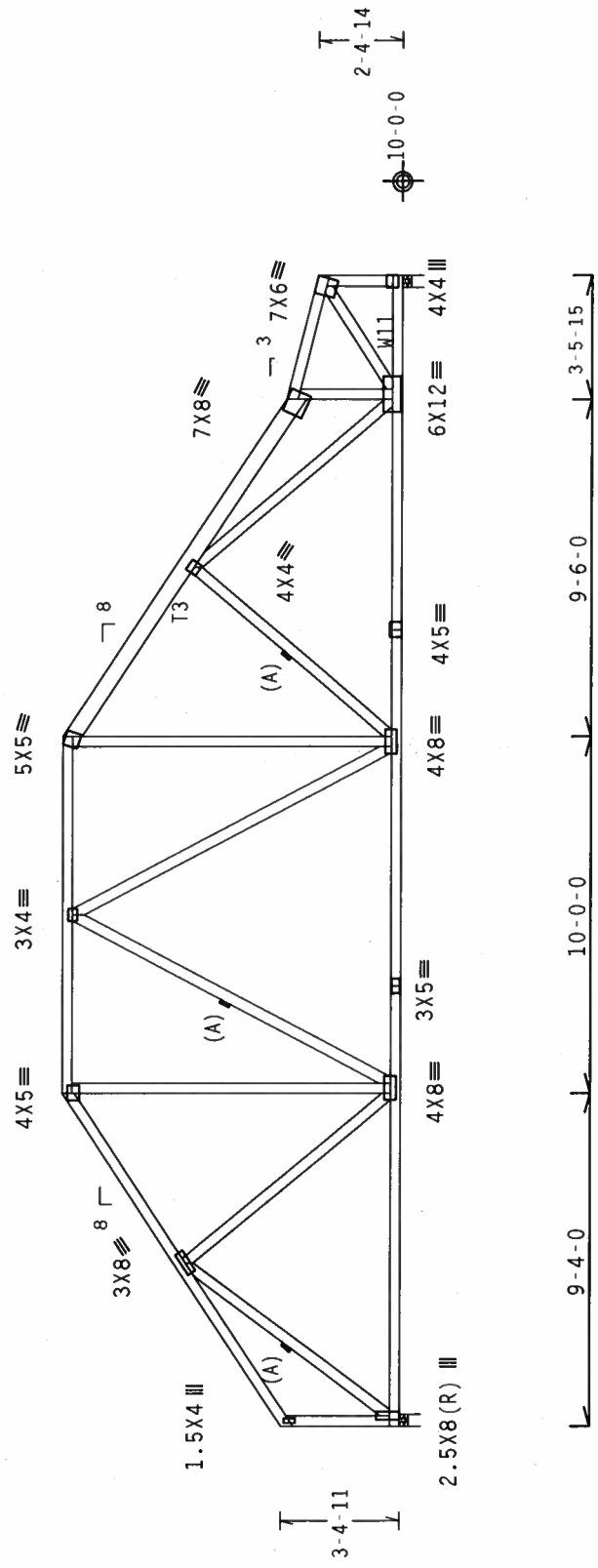
	(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)	
TC - From	64 PLF at 0.00 to	64 PLF at 9.33
TC - From	61 PLF at 9.33 to	61 PLF at 18.76
TC - From	194 PLF at 18.76 to	199 PLF at 19.34
TC - From	203 PLF at 19.34 to	293 PLF at 28.84
TC - From	61 PLF at 28.84 to	61 PLF at 32.33
BC - From	20 PLF at 0.00 to	20 PLF at 32.33
TC -	418 LB Conc. Load at 28.84	

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1841 U=209 W=4"

R=3074 U=349 W=4"

-32-4-0 Over 2 Supports

PLT TYP. Wave

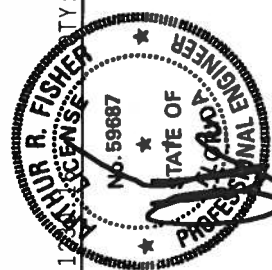
Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0) \quad 7.24.12$$

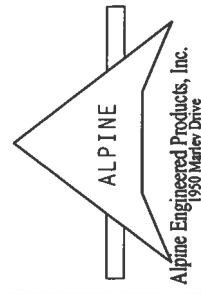
FL/-/4/-/-/R/-

Scale = .1875"/Ft.

TC LL	20.0	PSF	REF	R487 --	89683
TC DL	10.0	PSF	DATE	03/03/06	
BC DL	10.0	PSF	DRW	HCUSR487	06062020
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	87230	
DUR.FAC.	1.25				



Mar 03 '06



Top chord 2x6 SP #1 Dense :T1, T6 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 6.50 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

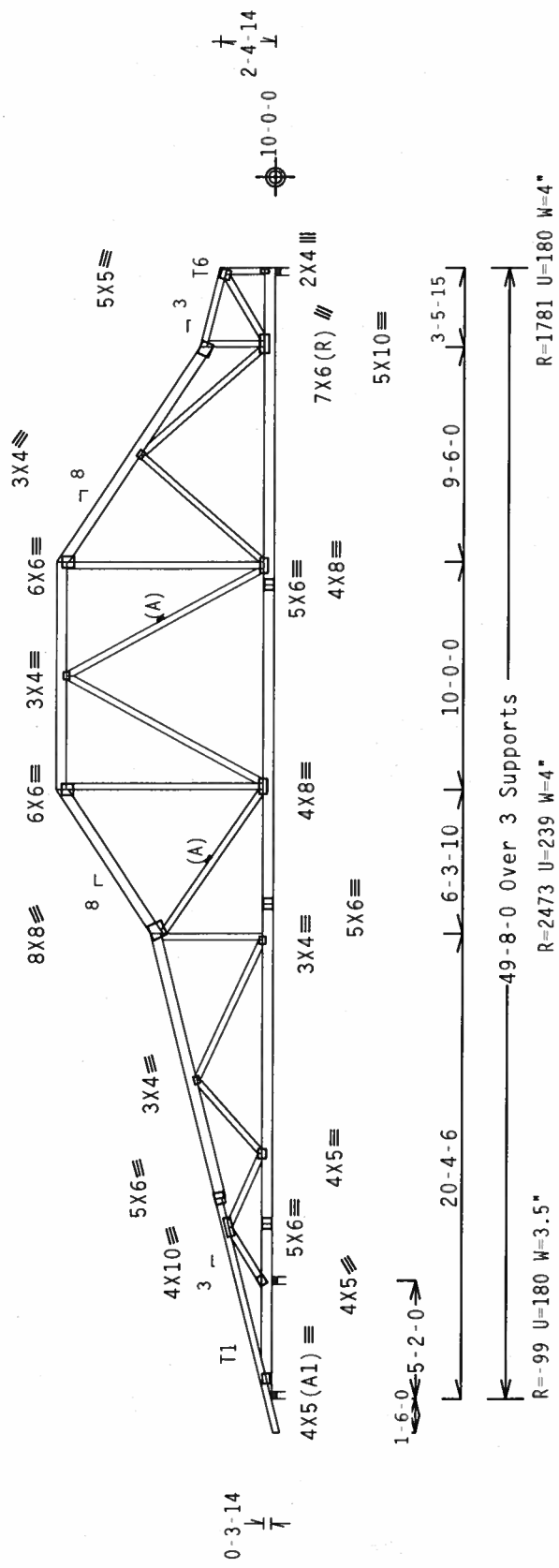
(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor.
Special care must be taken during handling, shipping and installation of
trusses. See "WARNING" note below.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

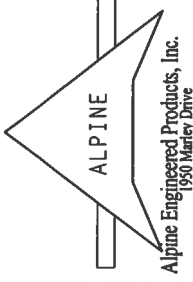
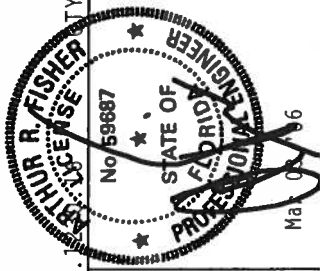
Cq/RT=1.00(1.25)/10(0) 7.24

Scale = .125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BC31 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583
D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN,
MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED,
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE
TRUSSES CONFORMING WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR STEEL TRUSSES AND
CONNECTOR PLATES ARE MADE OF 20/18/16GA (A-H/S/K) ASTM A653 GRADE 40/60 (IN. E/M-S) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN. FURNISH YOUR PROFESSIONAL SEAL AND SIGNATURE FOR THE TRUSS COMPONENT DESIGN.

TC LL	20.0 PSF	REF	R487--	89684
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062021
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87248	
DUR.FAC.	1.25			



Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W2 2x4 SP #2 Dense:

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W2 2x4 SP #2 Dense:

(A) Continuous lateral bracing equally spaced on member.

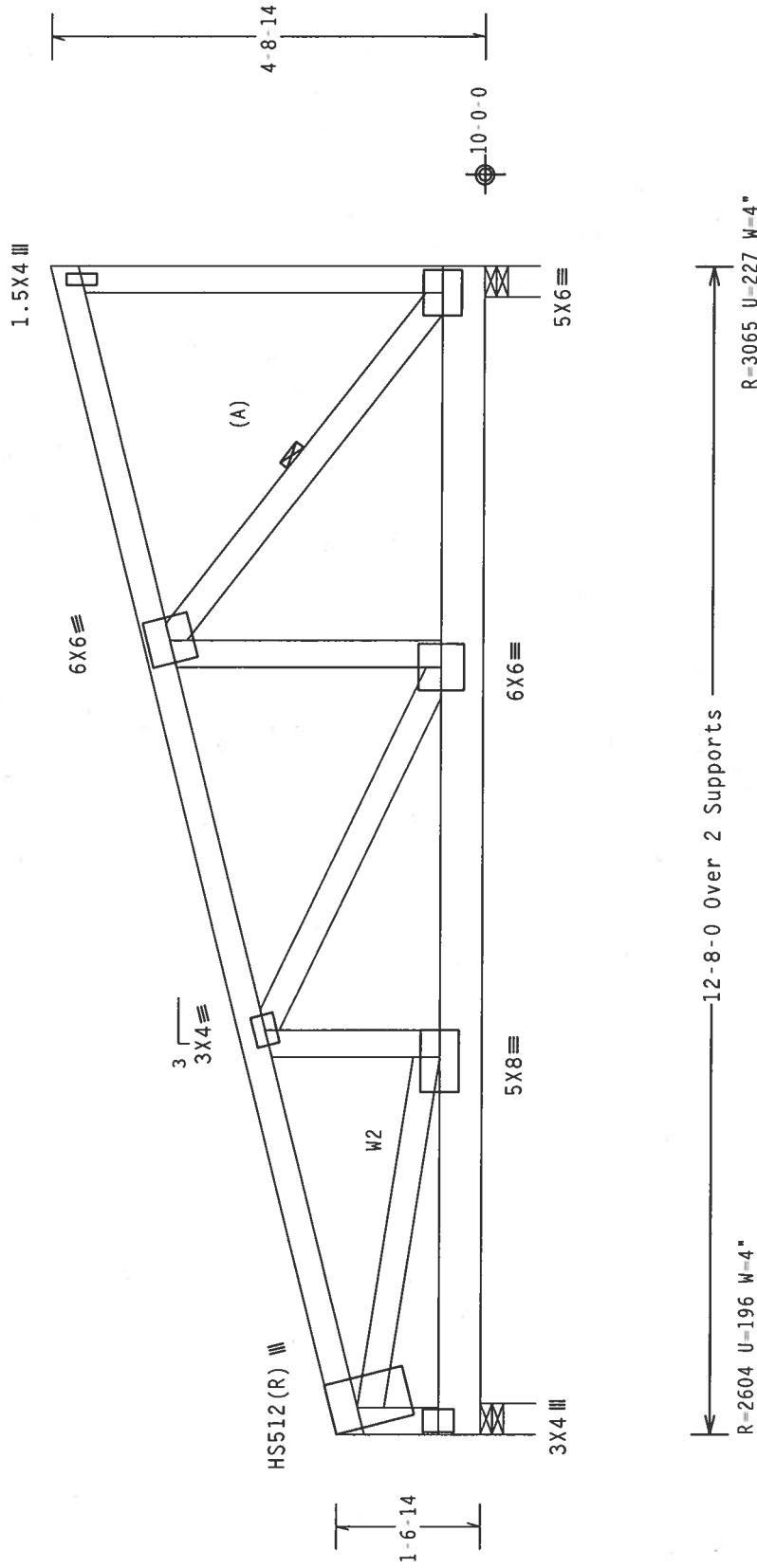
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS

		(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC	From 61 PLF at 0.00 to 61 PLF at 12.67	
BC	From 20 PLF at 0.00 to 20 PLF at 12.67	
BC	662 LB Conc. Load at 1.06, 3.06, 5.06, 7.06	
BC	667 LB Conc. Load at 9.06, 11.06, 12.27	

Right end vertical not exposed to wind pressure.

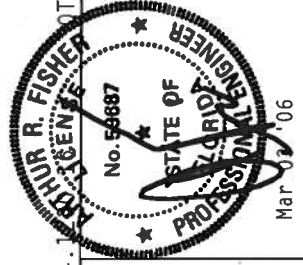
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



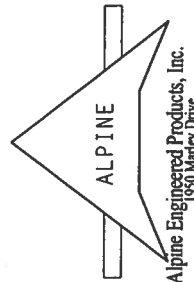
PLT TYP. 20 Gauge HS, Wave

FL/-/4/-/-/R/-/

TC LL	20.0	PSF	REF	R487 --	89685
TC DL	10.0	PSF	DATE	03/03/06	
BC DL	10.0	PSF	DRW	HCUSR487	06062022
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	87151	
DUR.FAC.	1.25				



Mar 23 '06



Alpine Engineered Products, Inc.
1950 Marlow Drive

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI TRUSS BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE AMERICAN TRUSS MANUFACTURERS ASSOCIATION (ATMA), 583 WEST OXFORD RD., SUITE 200, MADISON, WI 53719, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 1000 W. WISCONSIN AVE., SUITE 100, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TOP CHORD CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS STRUCTURE IN ACCORDANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.), AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION), AND AASHTO (AMERICAN ASSOCIATION OF HIGHWAY ENGINEERS AND TRANSPORTATION BUILDING OFFICIALS). CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/12X16) ASTM A453 GRADE 40/50 (W. K/H 5.5) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND UNDERGIRTS LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A.2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 4 OF TP11-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf.

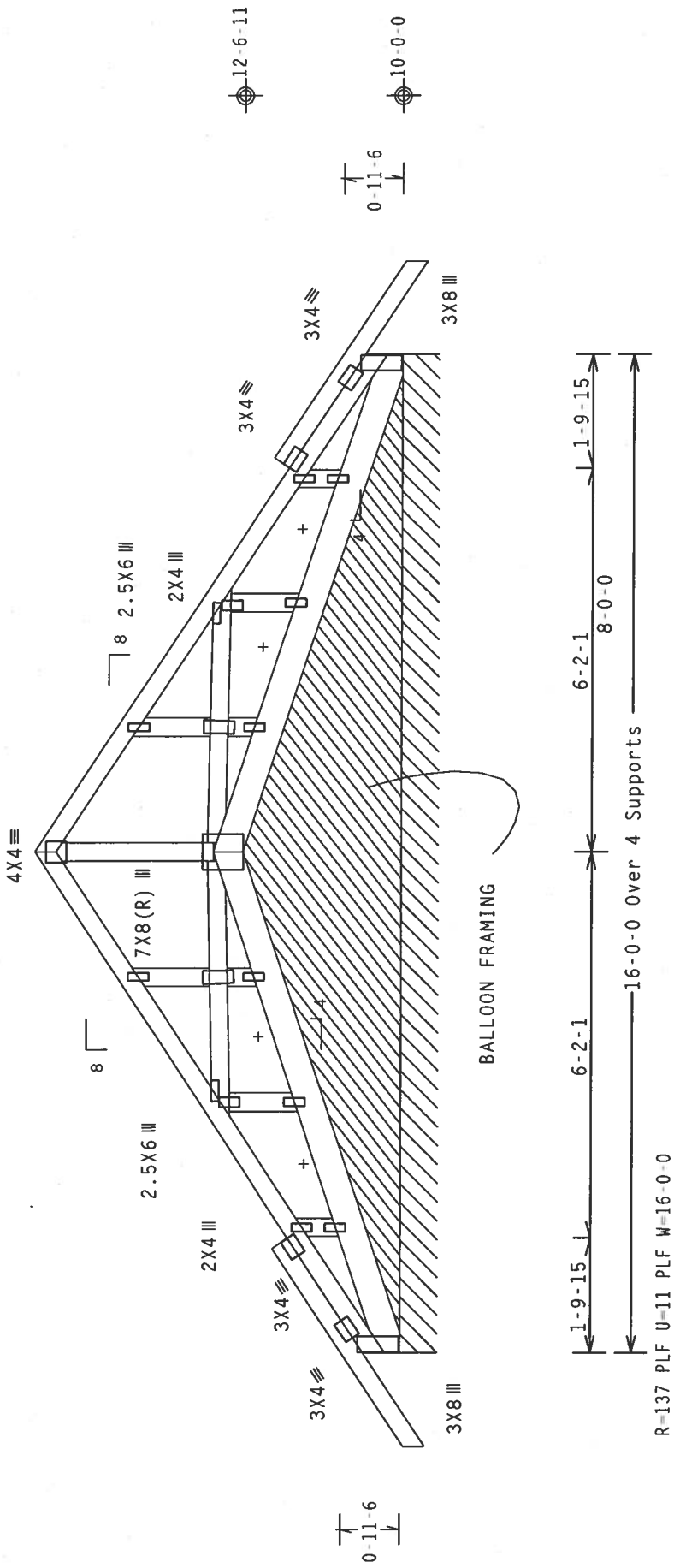
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Shim all supports to solid bearing.

(**) Plate relocated as shown.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24.12

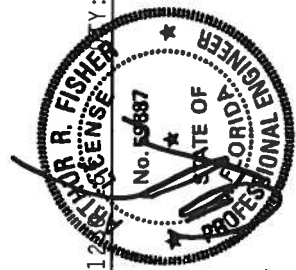
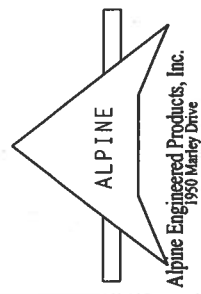
FL/-4/-/R/-

Scale = .375"/Ft.

TC LL	20.0 PSF	REF	R487--	89686
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062023
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87144	REV
DUR.FAC.	1.25			

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRIO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

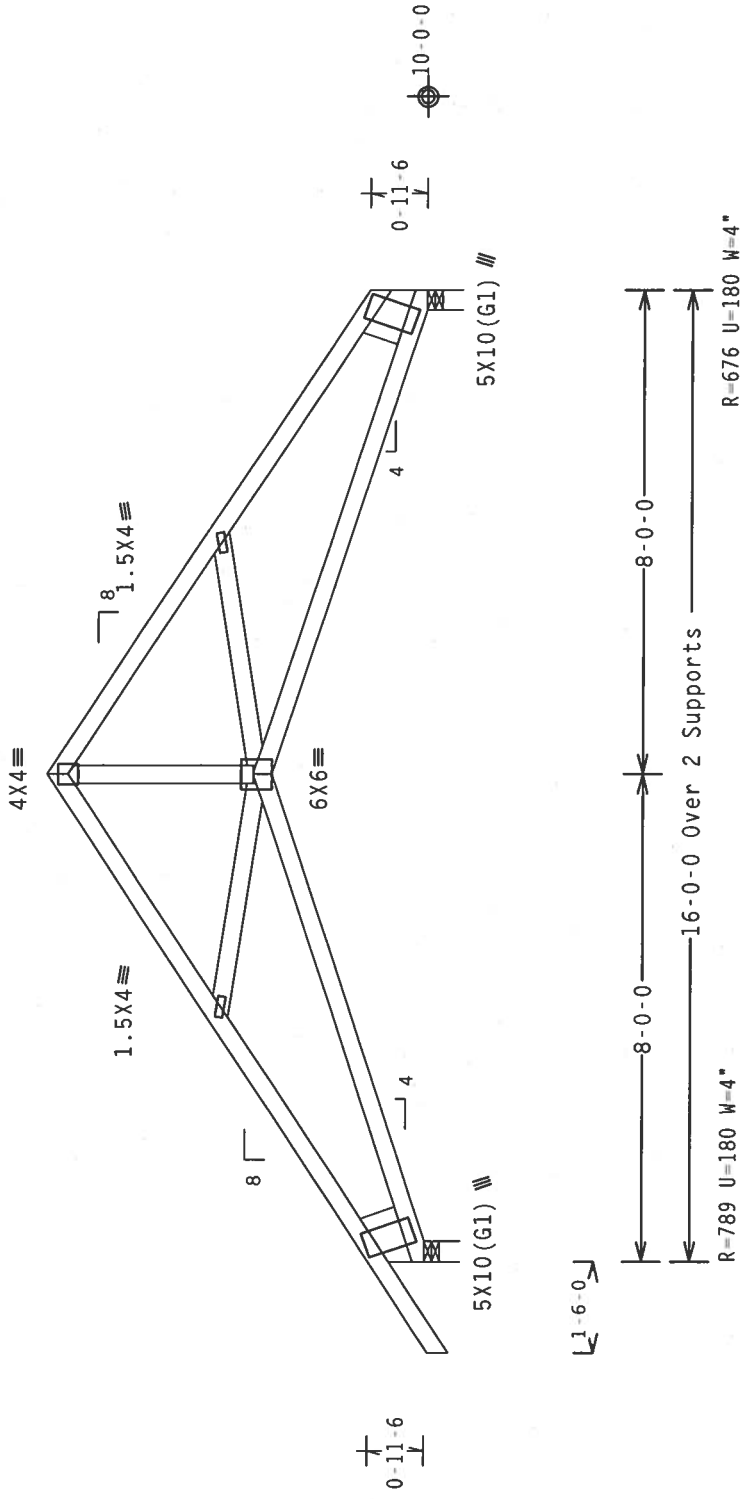
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF THE BUILDING OR THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AFPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M-H/S/K) ASTM A653 GRADE 40/60 (IN. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE LIABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.



Mar 03 '06

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3
 :Lt Stubby Wedge 2x8 SP #1 Dense:
 :Rt Stubby Wedge 2x8 SP #1 Dense:
 In lieu of structural panels or rigid ceiling use purlins to brace TC @
 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
 anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
 psf.
 Calculated horizontal deflection is 0.10" due to live load and 0.17"
 due to dead load.
 Deflection meets L/360 live and L/240 total load. Creep increase
 factor for dead load is 1.50.

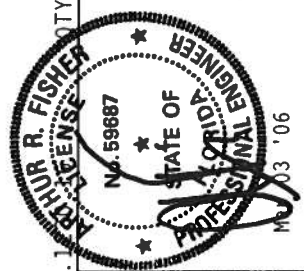


PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0)

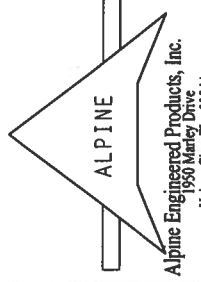
QUANTITY: 2 FL/-/4/-/R/-

Scale = .3125" / Ft.
REF R487 -- 89687
DATE 03/03/06
DRW HCUSR487 06062001
HC-ENG JB/AF *
SEQN- 87121
DUR.FAC. 1.25



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 8651 T-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 MADISON, NJ 07093), AND TCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LN., MADISON, NJ 07093) FOR SAFETY PRACTICES. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES IN CONFORMANCE WITH TPI. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AFAPA AND TPI. CONNECTOR PLATES ARE MADE OF 2018/16GA (M-H/5/2) ASTM A653 GRADE 40/60 (M. K/H-5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W4 2x4 SP #2 Dense:
:Lt Stubby Wedge 2x8 SP #1 Dense:

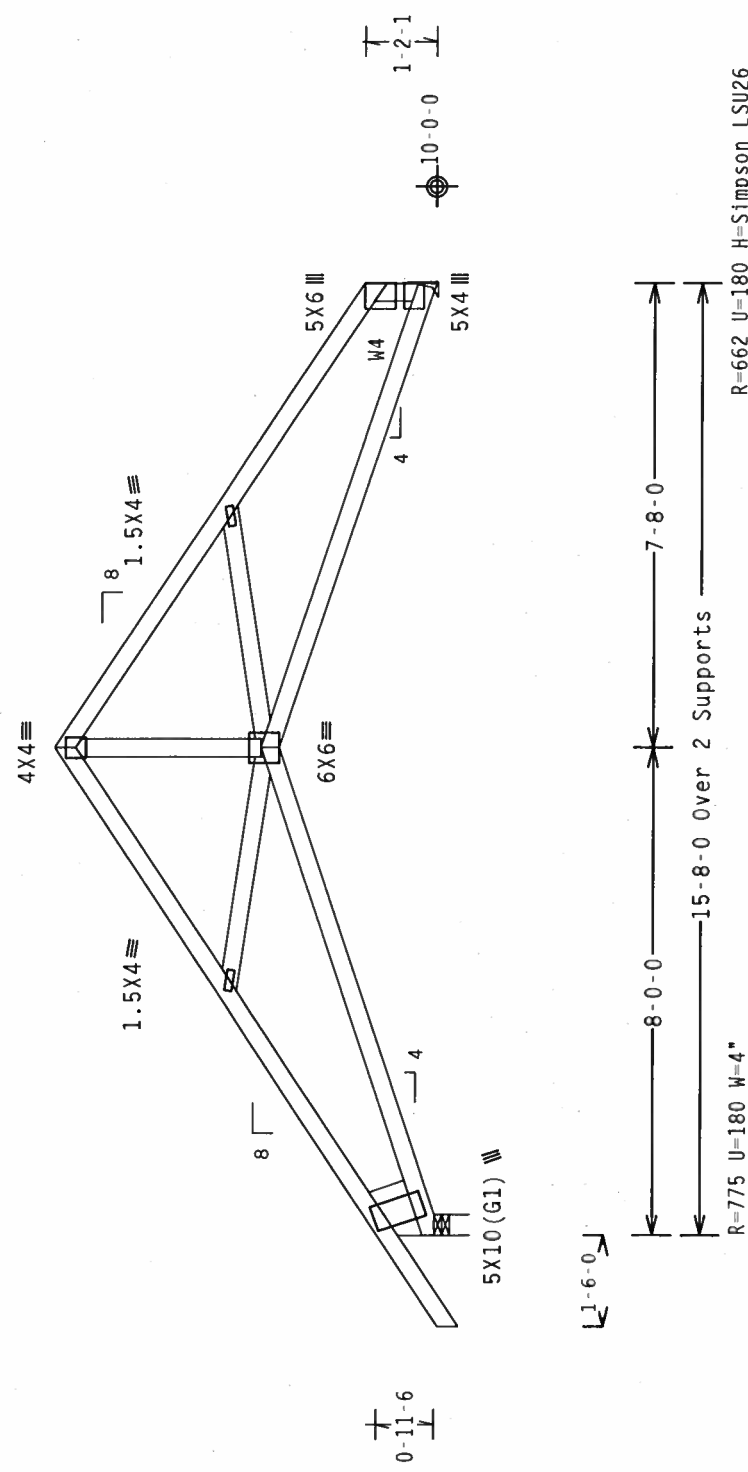
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.33" due to live load and 0.54" due to dead load.

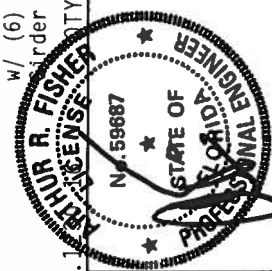
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



R-662 U=180 H-Simpson LSU26
w/ (5) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2x6 min. So.Pine

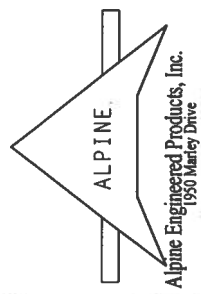
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

PLT TYP. Wave	FL/-4/-/-R/-	Scale = .3125"/Ft.
TC LL	20.0 PSF	REF R487-- 89688
TC DL	10.0 PSF	DATE 03/03/06
BC DL	10.0 PSF	DRW HCUR487 06062024
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 87131
DUR.FAC.	1.25	



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'DONNELL DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., SUITE 100, FORT WORTH, TX 76116) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO FOLLOW THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE DESIGN IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. AND SHALL NOT BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT PERMISSION IN WRITING FROM ALPINE ENGINEERED PRODUCTS, INC. DESIGN CONFORMS WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2010/160A (M/N/S/K) ASTM A653 GRADE 40/60 (M. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W4 2x4 SP #2 Dense:
:Lt Stubb'd Wedge 2x8 SP #1 Dense:

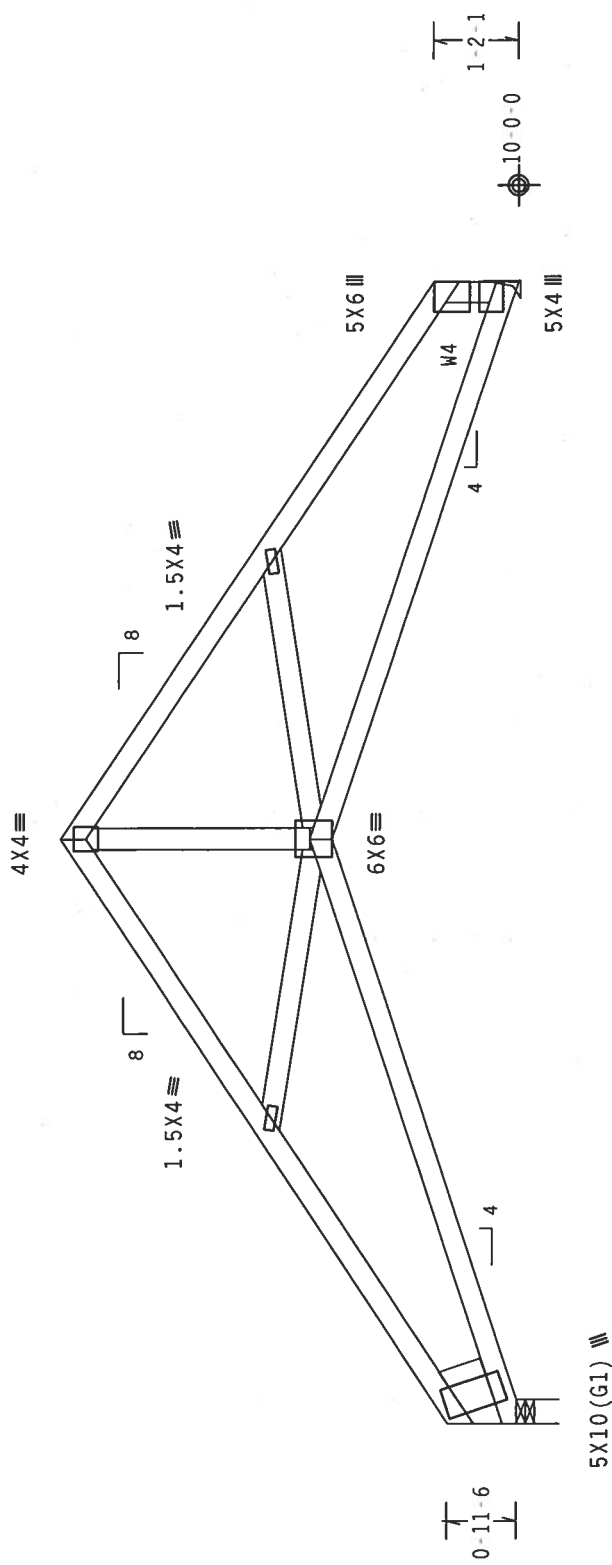
110 mph wind, 15,00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.33" due to live load and 0.55" due to dead load.

In lieu of structural panels or rigid ceiling use purlins to brace
TC @ 24" OC, 8C @ 24" OC.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



8-0-0 7-8-0

15.8-0 Over 2 Supports

R=667 U=180 W=4"

R=667 U=180 H=Simpson LSU26
w/ (5) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

$Cq/RT=1.00(1.25)/10(0)$ 7.24.13

FL/-/4/-/-/R/-

Scale = .375"/Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BCS 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TP1 TRUSS PLATE INSTITUTE, 693 D'ONOFIO DR., SUITE 200, MADISON, WI 53719, AND MCA WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID SHELLING.

REF R487-- 89689

DATE 03/03/06

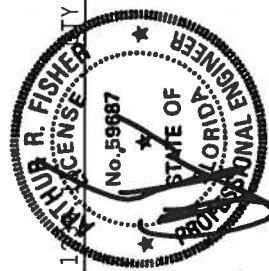
DATE 05/05/00
CPIW HC11SP487 0606202E

ENCLOSURE 10/18/2000

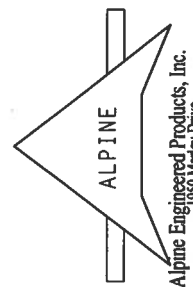
TC-ENG JD/AF
SEAN 07130

DUR.FAC.	1.25
LOI.LD.	40.0

Mar 03 '06



****IMPORTANT*****TURN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
THIS DESIGN IS THE PROPERTY OF THE ALPINE ENGINEERING FIRM. ANY FAILURE TO BUILD THE TRUSS IN ACCORDANCE WITH THIS DESIGN WILL BE THE RESPONSIBILITY OF THE INSTALLER. THE DESIGN CONFORMS WITH TPI - OR FABRICATING NATIONAL SHIPPING CONTAINER DESIGN SPECIFICATION AND ALL APPLICABLE PROVISIONS OF NDS "NATIONAL DESIGN SPEC." BY NEHA AND TPI. CONNECTOR PLATES ARE MADE OF 20X18/16GA (W/1/2X1/8) ASTM A563 GRADE 40/60 (A /K/H/S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPII-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR TRUSS COMPONENT.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

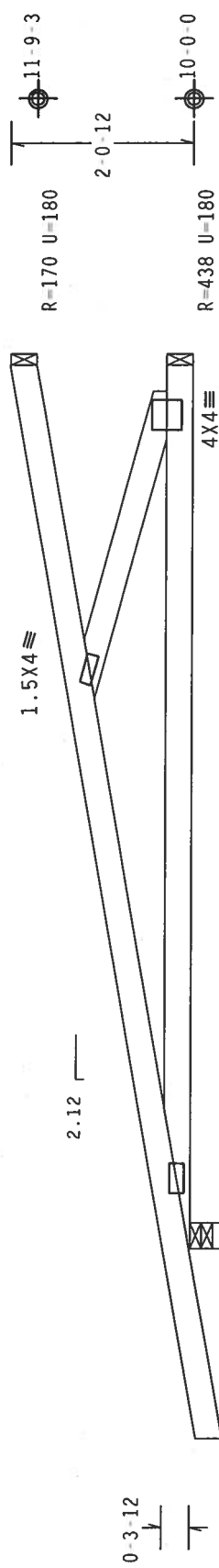
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-1-7
9-10-13 Over 3 Supports
R-450 U-180 W-3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.12

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487 -- 89691
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062027
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87105
DUR.FAC.	1.25		

ALPINE

Alpine Engineered Products, Inc.
1550 Marney Drive

PROFESSIONAL ENGINEER

R. FISHER
No. 59887
STATE OF OHIO
Professional Seal

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

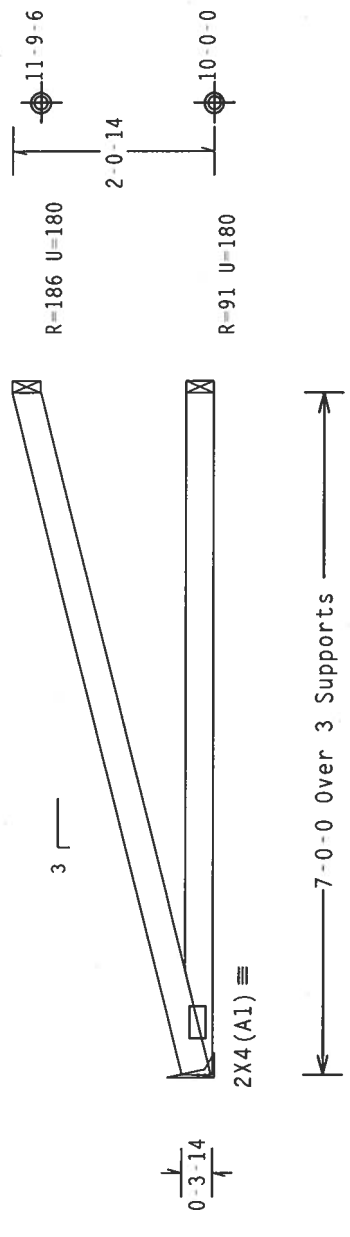
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

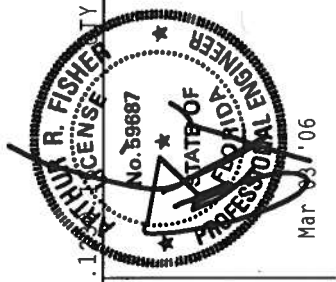
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



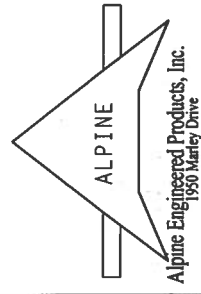
R=287 U=180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

PLT TYP. Wave	Scale = .5" / Ft.
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
REF	R487 -- 89692
DATE	03/03/06
DRW	HCUSR487 06062028
HC-ENG	JB/AF
SEQN-	87089



ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THE DESIGN OR CONSTRUCTION OF THE TRUSS IN CONFORMANCE WITH TPI-2002(STD)/FBC. THE TRUSS SHALL BE FABRICATED, SHIPPED, INSTALLED AND BRACED IN CONFORMANCE WITH TPI-2002(STD)/FBC. THE TRUSS SHALL BE MADE OF 20/18/16GA (W-H/S/K) ASTM A653 GRADE 40/60 (W, K/H, S) GALV. STEEL. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W-H/S/K) ASTM A653 GRADE 40/60 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT END AND BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

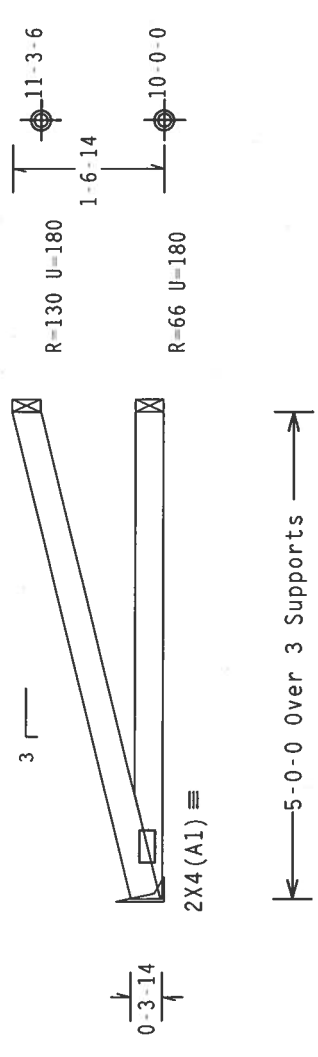
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



R=207 U-180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

Scale = 5" / Ft.

TC LL	20.0 PSF	REF	R487-- 89693
TC DL	10.0 PSF <td>DATE</td> <td>03/03/06</td>	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062029
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87157
DUR.FAC.	1.25		

ALPINE
Alpine Engineered Products, Inc.
1550 Marley Drive

ARTHUR R. FISHER
No 59887
STATE OF
PROFESSIONAL ENGINEER
MA 03 '06

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 H. OGDON DR., SUITE 200, MADISON, WI 53719), AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719), FOR ADDITIONAL INFORMATION. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF ANY FABRICATED TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AEP/A AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M-N/S/K) ASTM A653 GRADE 40/60 (M, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

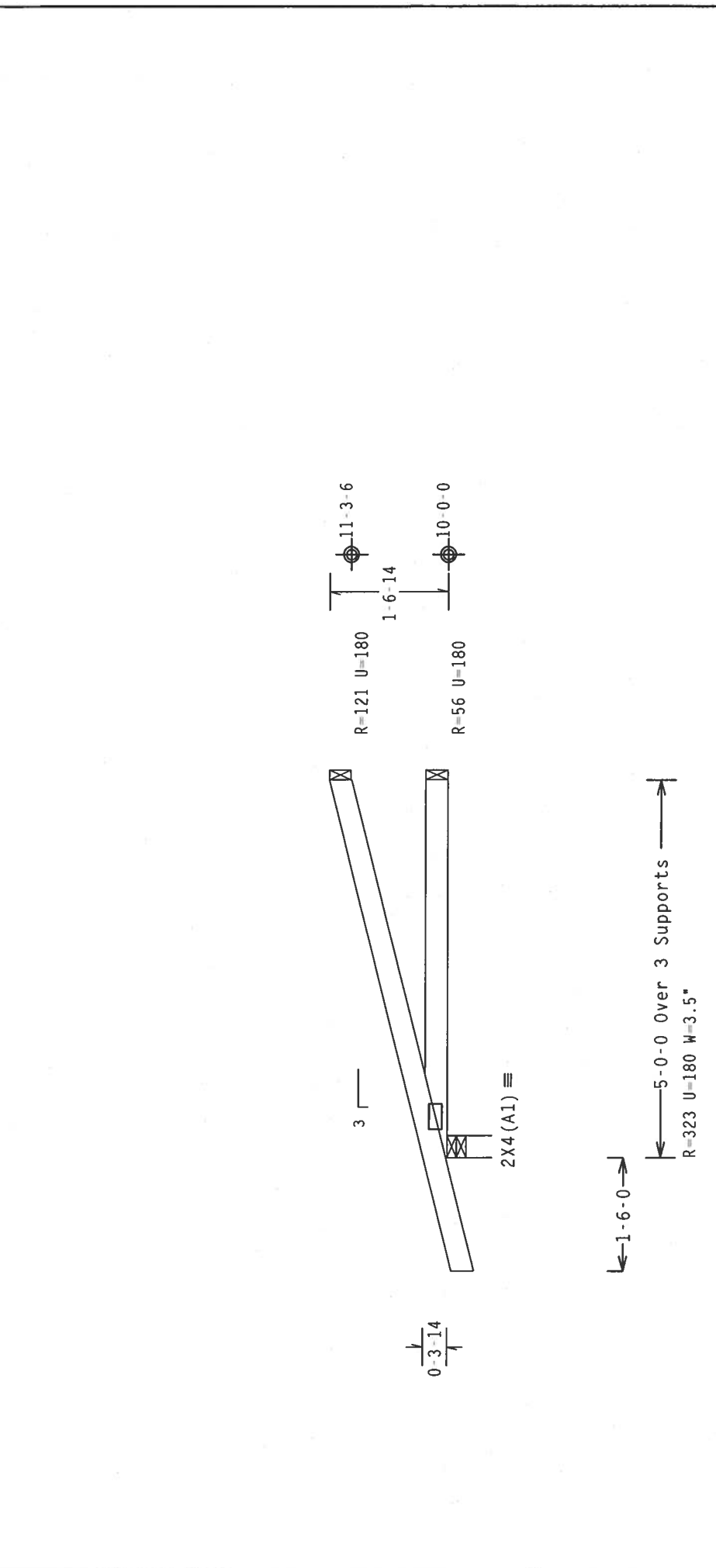
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

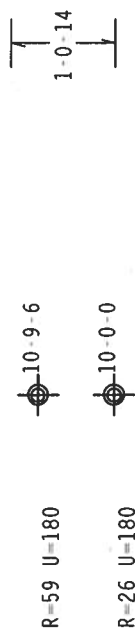
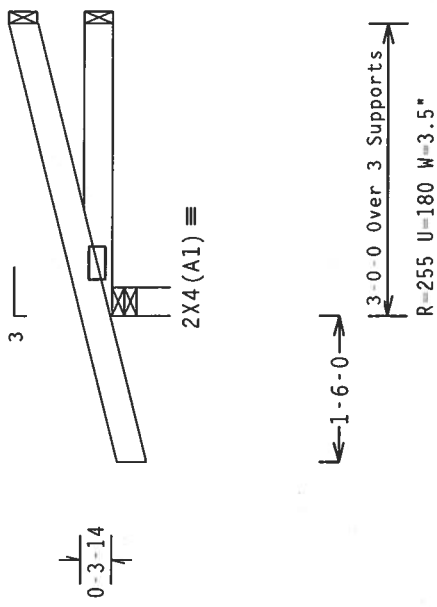
Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487 -- 89694
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062030
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87072
DUR.FAC.	1.25		

ARTHUR R. FISHER
Professional Engineer
No. 59887
STATE OF FLORIDA
Professional Engineer
May 03 '06

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Alpine Engineered Products, Inc.
1950 Marney Drive

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487-- 89695
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062031
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87075
DUR.FAC.	1.25		

03/03/06

Ma 03 '06

Professional Engineer
No. 59687
State of Arizona

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'DRISCOLL DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, YOU OR YOUR SUBS CONTRACTORS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE TRUSS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AEPAL AND TPI. CONNECTOR PLATES ARE MADE OF 20/10/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (IN. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE QUALITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

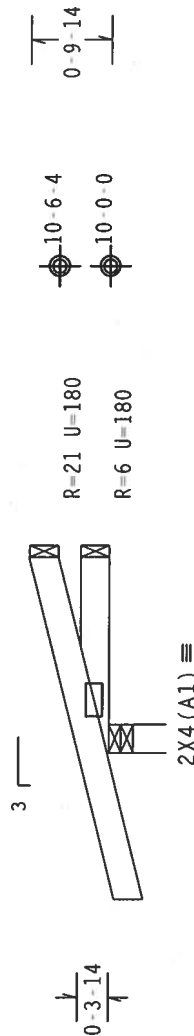
Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



$\leftarrow 1-6-0 \rightarrow$
 $\leftarrow 2-0-0 \rightarrow$ Over 3 Supports
 $R=231 \quad U=180 \quad W=3.5"$

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

$$Cq/RT = 1.00(1.25)/10(0)$$

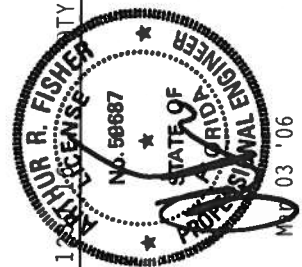
TY. 4

E11-141-1-181-

Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THESE TRUSSES ARE NOT TO BE USED AS A COMPONENT OF A BUILDING FRAMEWORK. PUBLISHED BY TPI (TRUSS PACE) INSTITUTE, 1863 S. DUNFORD RD., SUITE 100, WILMINGTON, MISSISSIPPI 39204. FOR MORE INFORMATION, CONTACT TPI, 6300 ENTERPRISE LN., MADISON, WI 53719. FOR SAFETY, ALL TRUSSES MUST BE PROPERLY BRACED TO PREVENT UNDESIRABLE TORSION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: SUBMIT A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS COMPONENTS. THE TRUSS DESIGN CONFORMS WITH TPI OR FABRICATING, WELDING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (NATIONAL DESIGN SPEC. BY AISC) AND ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (.015/.018/.016) ASTM A653 GRADE 40/60 (M. K/H/S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMNH AS OF TPII-2000 SEC.3. A SEAL ON THIS DESIGN SHOWS THE DATE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN, THE DESIGNER ASSUMES THE RESPONSIBILITY FOR THE TRUSS DESIGN.



TC LL	20.0	PSF	REF	R487--	89696
TC DL	10.0	PSF	DATE	03/03/06	
BC DL	10.0	PSF	DRW	HCUSR487	06062032
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	87083	
DUR.FAC.	1.25				

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

:Stack Chord SC1 2x4 SP #2 Dense:

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

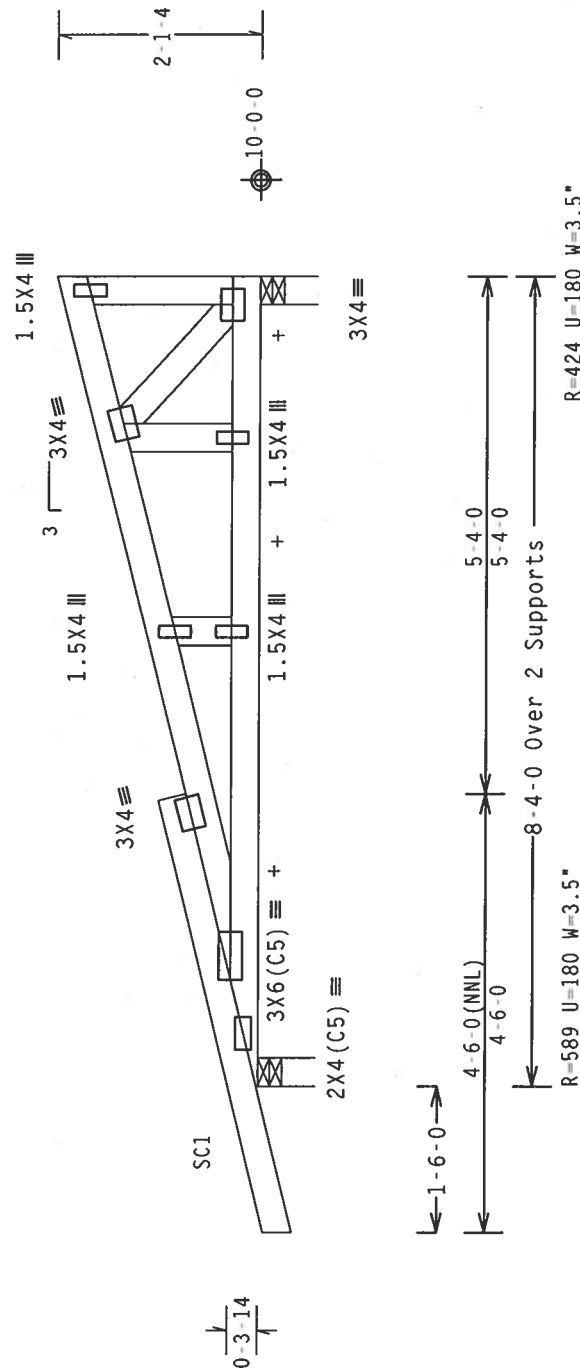
Right end vertical not exposed to wind pressure.

See DWGS A11015EE0405 & G8LLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

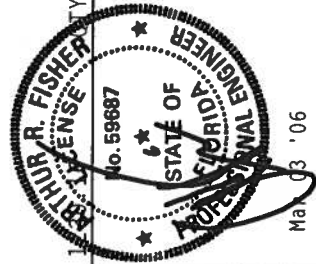
7.24.1

FL/-/4/-/R/-

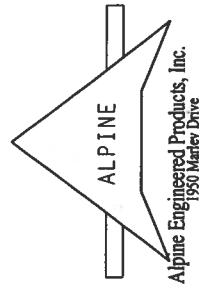
Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 MADISON, WISCONSIN, 53718) AND NFCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WISCONSIN, 53718) FOR PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR NFCA REQUIREMENTS SHALL BE THE RESPONSIBILITY OF THE INSTALLER. CONNECTOR PLATES ARE MADE OF 2018/186A (N-H/S/K) ASTM A653 GRADE 40/60 (IN. K/H-SI) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE QUALITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



TC LL	20.0 PSF	REF	R487 --	89697
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062033
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT. LD.	40.0 PSF	SEQN-	87032	
DUR. FAC.	1.25			



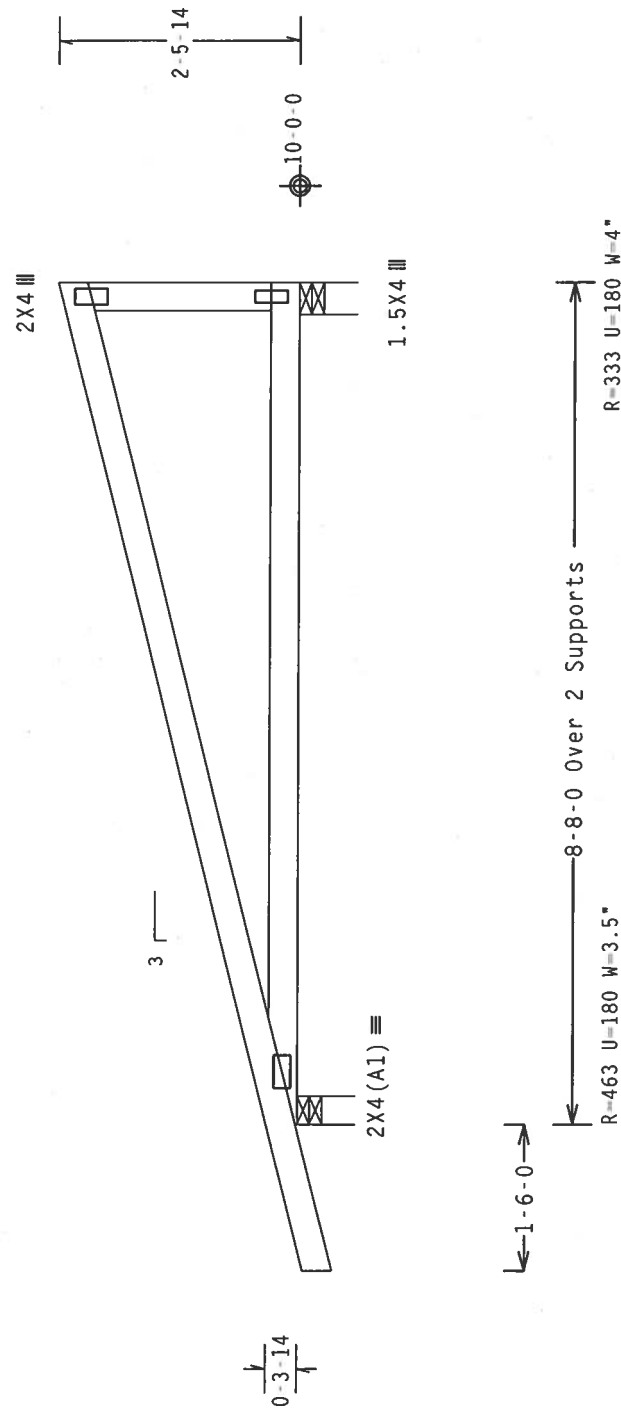
Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: $\text{TPI-2002(STD)}/\text{FBC}$
 $\text{Ca}/\text{RT}=1.00(1.25)/$

Scale = .5"/Ft.

TC LL	20.0	PSF	REF R487 -- 89698
TC DL	10.0	PSF	DATE 03/03/06
BC DL	10.0	PSF	DRW HCUR487 06062002
BC LL	0.0	PSF	HC-ENG JB/AF *
TOT.LD.	40.0	PSF	SEQN- 87017
DUR.FAC.	1.25		



Alpine Engineered Products, Inc.
1950 Marley Drive

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

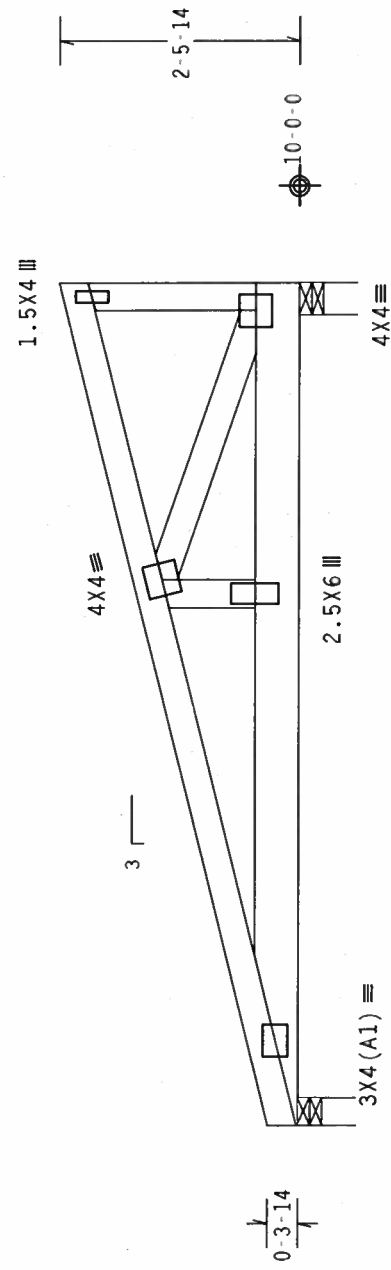
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 61 PLF at 0.00 to 61 PLF at 8.67
BC - From 20 PLF at 0.00 to 20 PLF at 8.67
BC - 207 LB Conc. Load at 1.09
BC - 287 LB Conc. Load at 3.06, 5.06, 7.06

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



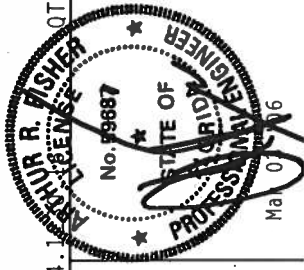
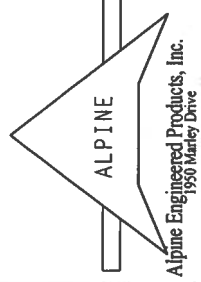
8-8-0 Over 2 Supports
R=904 U=180 W=3.5
R=864 U=180 W=4

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 MADISON DR., SUITE 200, MADISON, WI 53719), AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LN., MADISON, WI 53719) FOR TRUSS DESIGN, FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS NATIONAL DESIGN SPEC. BY AFPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M-H/S/K) ASTM A653 GRADE 40/60 (M, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



QTY: 2	FL / - / 4 / - / - / R / -	Scale = .5" / Ft.
TC LL	20.0 PSF	REF R487 - - 89699
TC DL	10.0 PSF	DATE 03/03/06
BC DL	10.0 PSF	DRW HCUSR487 06062034
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN - 87164
DUR.FAC.	1.25	

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

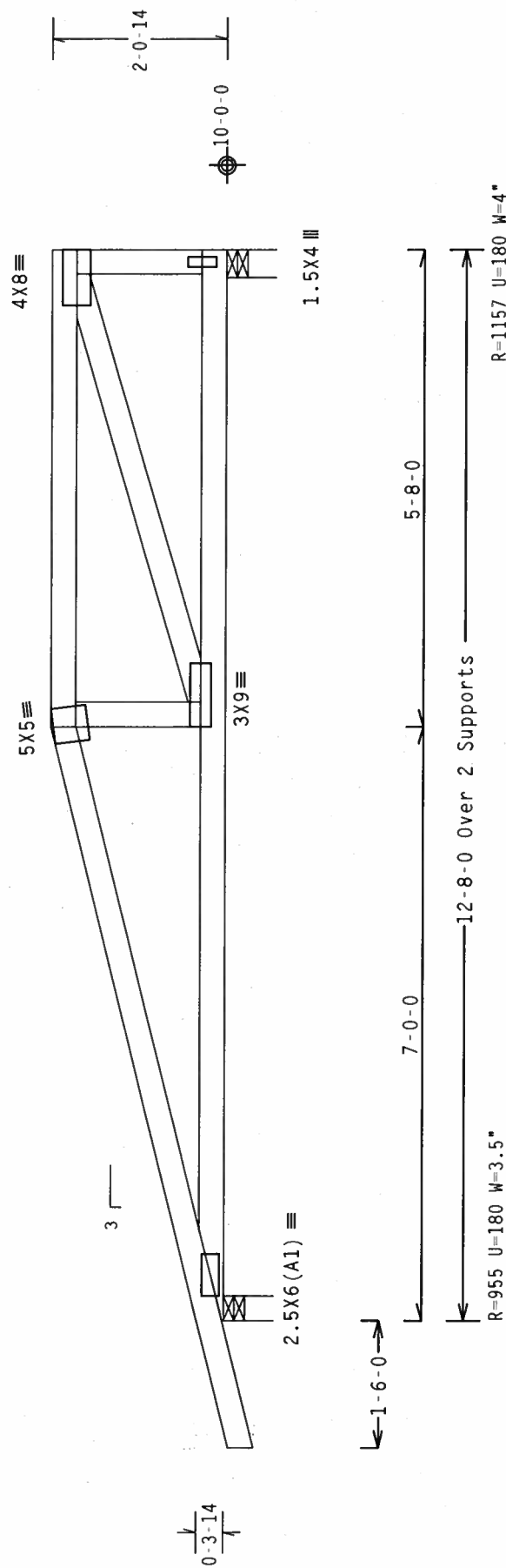
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.



PLT TYP. Wave

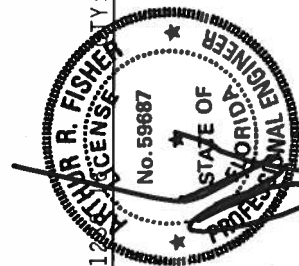
Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

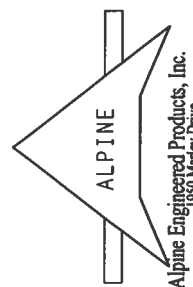
ITY:2 FL/-/4/-/-/R/-

Scale = 5"/Ft.

TC LL	20.0 PSF	REF R487 -- 89700
TC DL	10.0 PSF	DATE 03/03/06
BC DL	10.0 PSF	DRW HCUSR487 06062035
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 87068
DUR.FAC.	1.25	



Mar 06



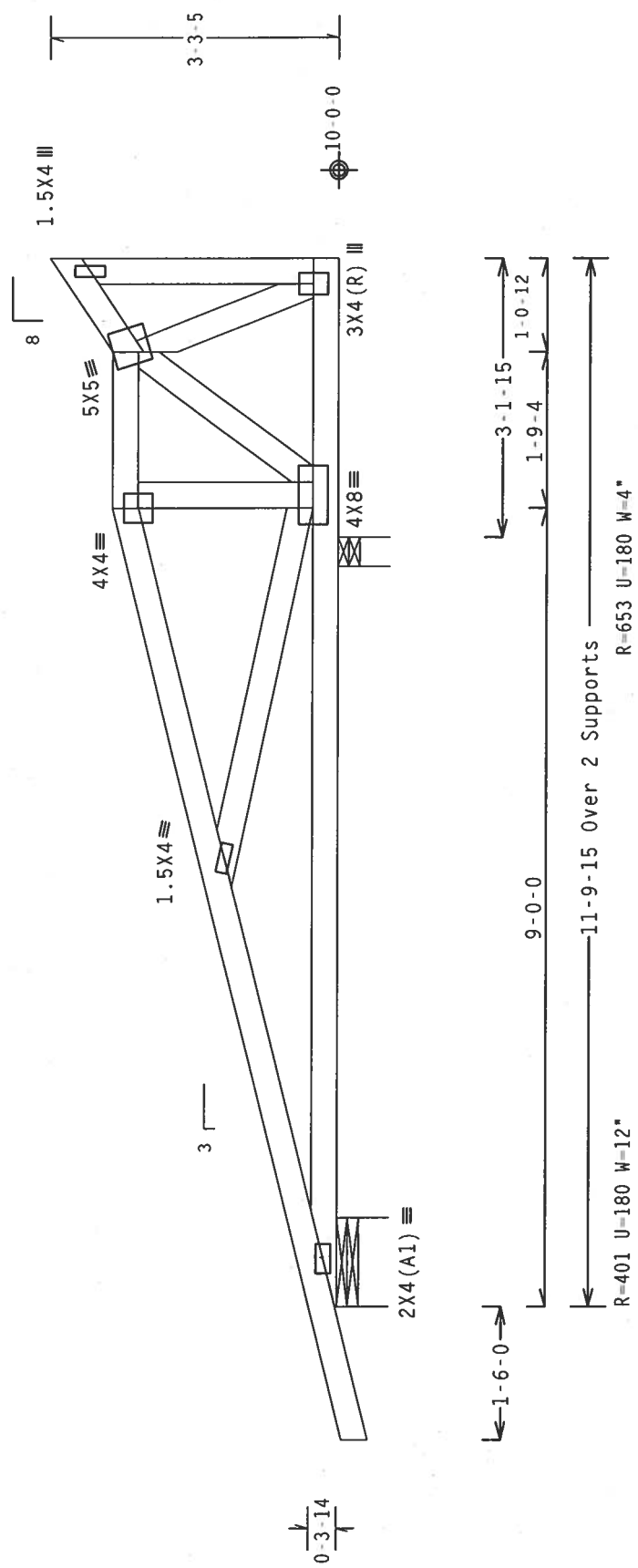
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R487 -- 89701
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062036
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87064
DUR.FAC.	1.25		

ARTHUR R. FISHER
No. 159687
STATE OF FLORIDA
Professional Engineer
March 2006

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGNER'S RESPONSIBILITY IS TO PROVIDE THE TRUSS WITH APPLICABLE PROVISIONS OF WDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. APPLICABLE TO EACH FACE OF TRUSS AND UNLESS OTHERWISE SPECIFIED, ALL TRUSSES SHALL BE GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE SPECIFIED, ALL TRUSSES SHALL BE GALV. STEEL. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC 3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

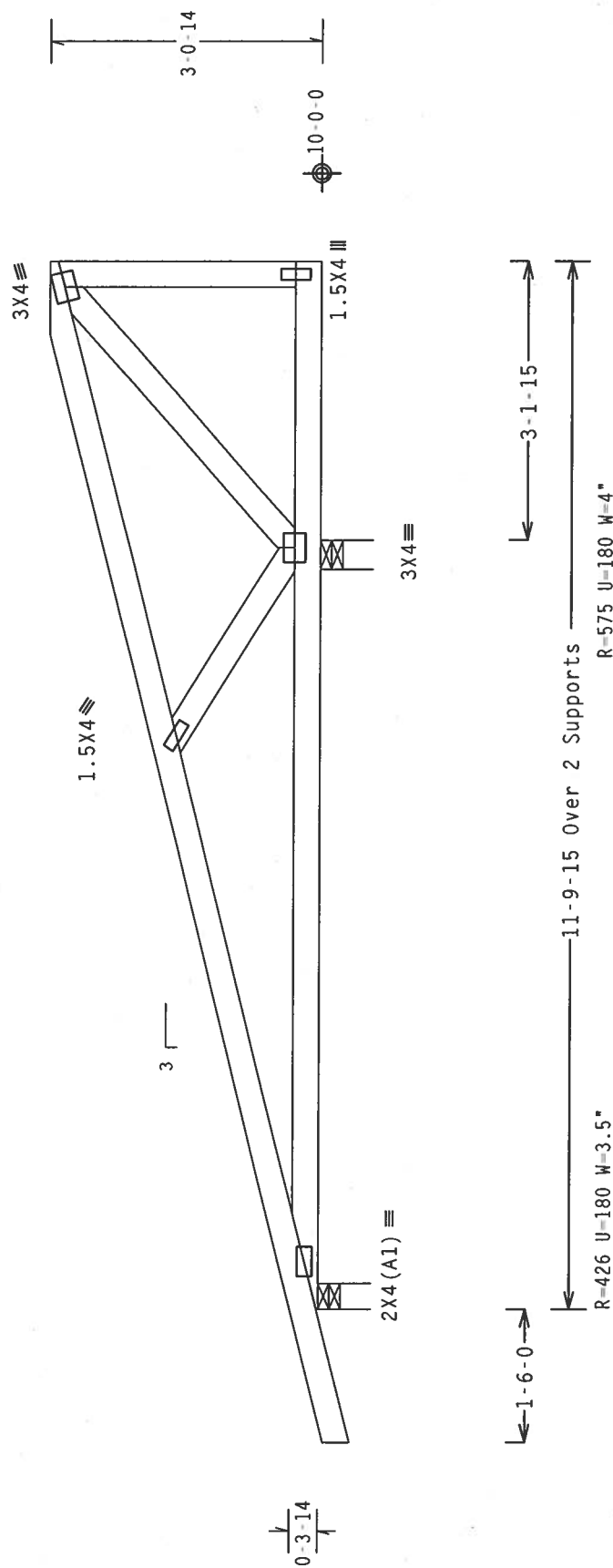
	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

12

TY:2

FL/-/4/-/-/R/-

Scale = 5"/Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BCSP 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 O'DONOFIO DR., SUITE 200, MADISON, WI 53719, AND MTCA WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE AVE., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

**** IMPORTANT ****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES WITH APPLICABLE PROVISIONS OF ASTM A633 GRADE 40/50 (W. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002-SEC.3. A SEAL ON THIS



TC LL	20.0 PSF	REF R487 --	89702
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062037
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87054
DUR.FAC.	1.25		

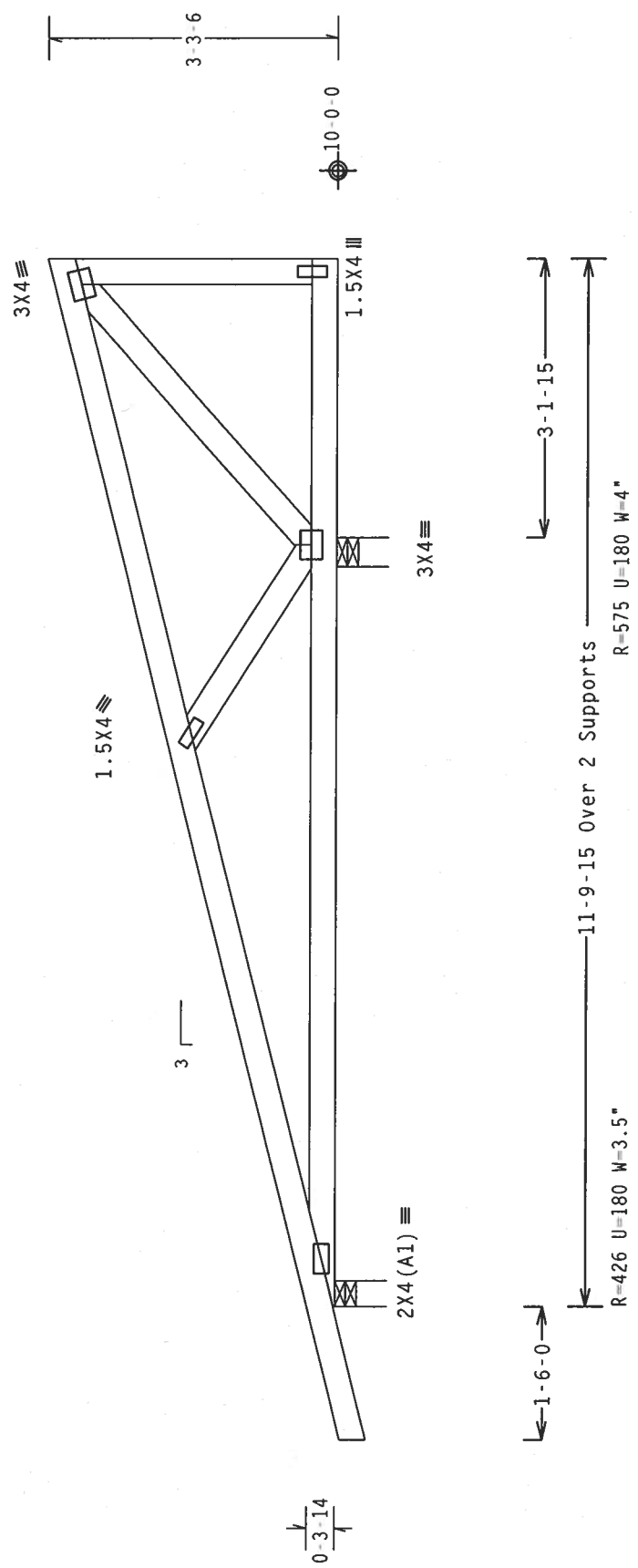
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Right end vertical not exposed to wind pressure.

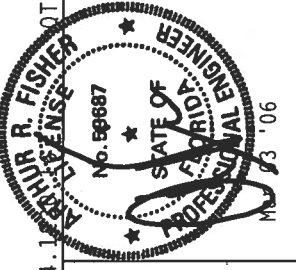
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

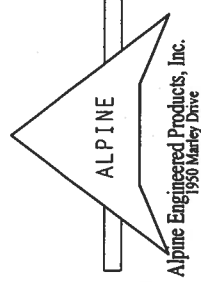
Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487--	89703
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062038
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87060	
DUR.FAC.	1.25			



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'AMORIO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., SUITE 100, WILSONVILLE, OR 97150) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR FABRICATING OF TRUSSES. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/A/S/X) ASTM A653 GRADE 40/60 (N/A/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE COUNTERSINK AND USE OF THIS ENGINEER'S SEAL AND BUILDING OF THE TRUSS COMPONENT OF THE DESIGN SHOWN.



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x6	SP	#1	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

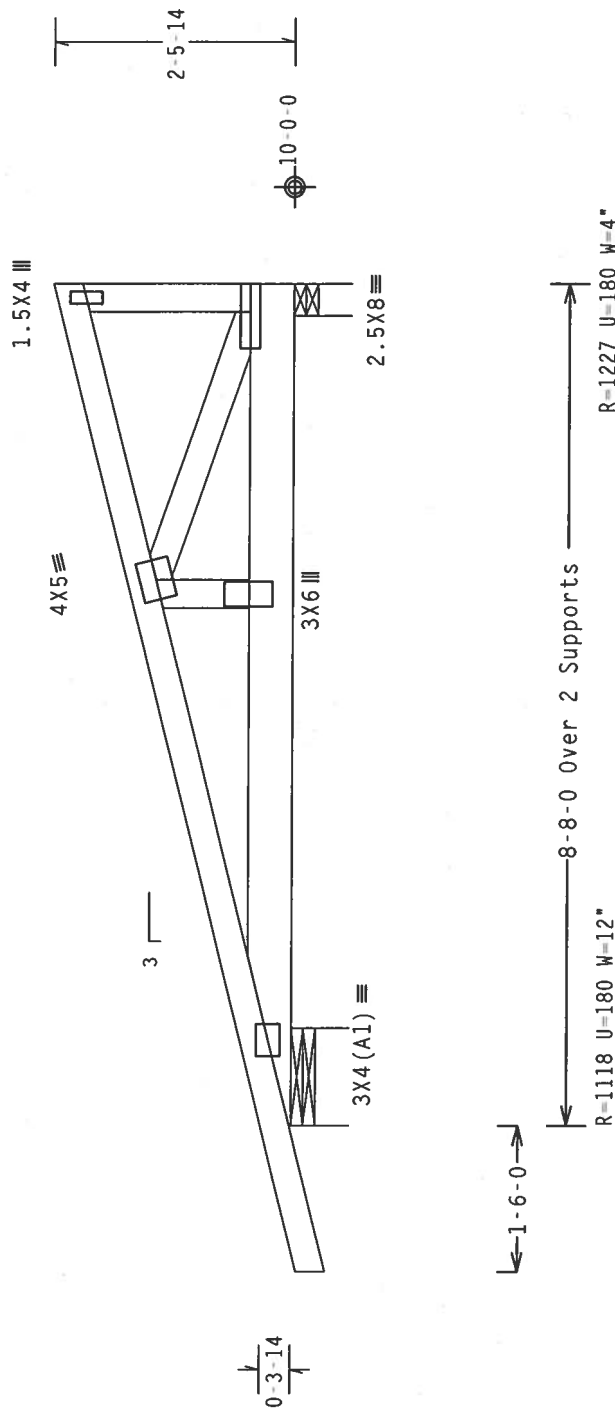
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

SPECIAL LOADS

		(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC	From 61 PLF at -1.50 to	61 PLF at 8.67
BC	From 4 PLF at -1.50 to	4 PLF at 0.00
BC	From 20 PLF at 0.00 to	20 PLF at 8.67
BC	387 LB Conc. Load at 2.06,	4.06, 6.06, 8.06

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

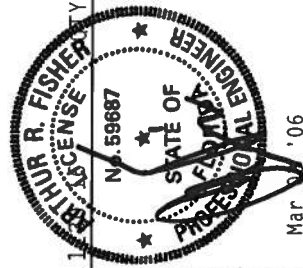
$$Cq/RT = 1.00(1.25)/10(0)$$

7.24 14

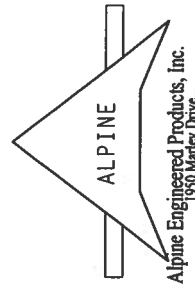
YOUTH CENTER
LICENSE
SHEPHERD
V. 2

$$\text{scale} = 5"/\text{r} +$$

TC LL	20.0	PSF	REF R487 -- 89704
TC DL	10.0	PSF	DATE 03/03/06
BC DL	10.0	PSF	DRW HCUSR487 06062039
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 87186
DUR.FAC.	1.25		



Mar 90,

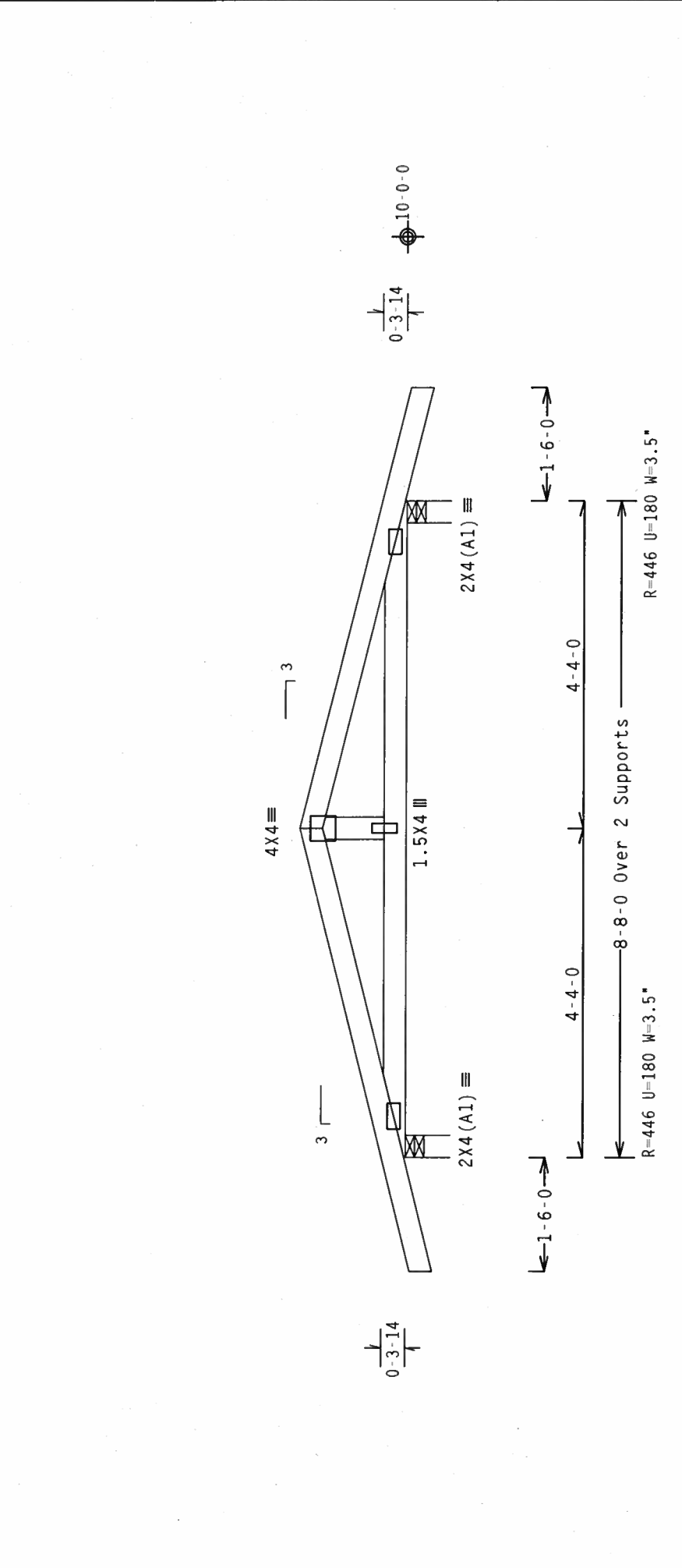


Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487-- 89705
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062003
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	87293
DUR.FAC.	1.25		

ALPINE
Alpine Engineered Products, Inc.
1550 Marney Drive

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1 03 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORPHEO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF THE TRUSS OR BUILDING. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND BUILDING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE TRUSS AND BUILDING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE TRUSS AND BUILDING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE TRUSS AND BUILDING.

PROFESSIONAL ENGINEER
STATE OF ILLINOIS
No. 59887
MAR 03 '06

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

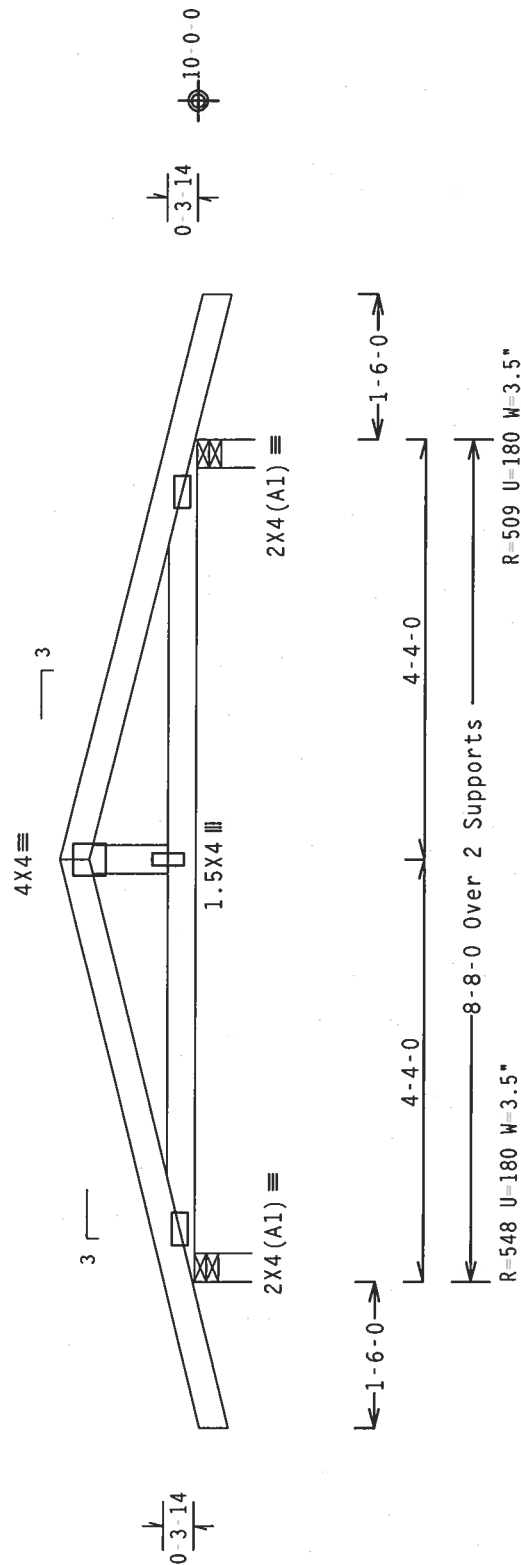
110 mph wind, 15.00 ft mean hgt. ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

SPECIAL LOADS

	(LUMBER DUR.FAC. = 1.25 / PLATE DUR.FAC. = 1.25)		
TC	From	61 PLF at 1.50 to	61 PLF at 10.17
BC	From	20 PLF at 0.00 to	20 PLF at 8.67
BC	From	4 PLF at 0.67 to	4 PLF at 10.17
BC	-	140 LB Conc. Load at	2.75
BC	-	42 LB Conc. Load at	5.58
BC	-	11 LB Conc. Load at	7.00

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



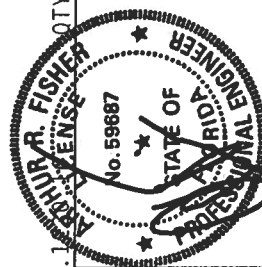
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

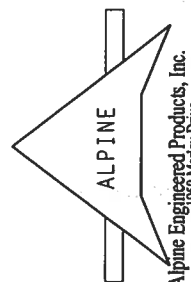
 $Cq/RT=1.00(1.25)/10(0)$ 7.24.14

ATTENTION: FISHF

Scale = .5"/Ft.



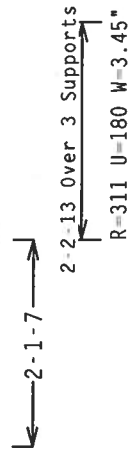
Mar-03 '06



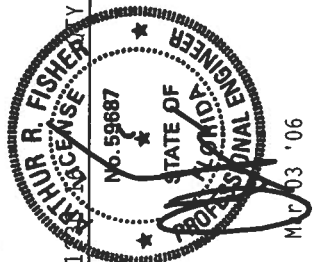
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. FOR ADDITIONAL SAFETY INFORMATION, PUBLISHED BY TPI CORP., PLATE INSTITUTE, 983 S. DODD AVE., CHICAGO, ILL. 60607 OR TPI CORP., HANCOCK, N.Y. 14456. IF YOU ARE AN ENGINEER, ARCHITECT, CONTRACTOR, OR SUBCONTRACTOR, ADVISE YOUR CLIENTS OF THIS WARNING. IF YOU ARE A DESIGNER, ADVISOR, OR MANAGER, ADVISE YOUR CLIENTS OF THIS WARNING. IF YOU ARE A FIELD PERSONNEL, ADVISE YOUR SUPERVISORS OF THIS WARNING. IF YOU ARE A FIELD PERSONNEL, ADVISE YOUR SUPERVISORS OF THIS WARNING.

****IMPORTANT****—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE STRUCTURE IN ACCORDANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS "NATIONAL DESIGN SPEC. BY AIA/AFPA AND TYPICAL DESIGN CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/MS/16GA) ASTM A653 GRADE 40/60 (N/ K/H/S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, OTHERWIS/SE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP11 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

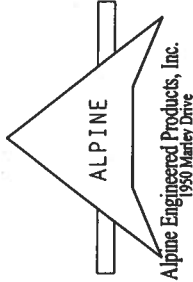
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



FL/-/4/-/R/-		Scale = 5"/Ft.	
TC LL	20.0 PSF	REF	R487 -- 89707
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062041
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN -	87300
DUR.FAC.	1.25		



***IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERS AND CONSULTANTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AFAPA) AND TP1. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/18GA. (M./H./S) ASTM A653 GRADE 40/60 (M./H./S) GALV. STEEL. APPLY PLATES TO EACH PLATE OF TRUSS AND, UNLESS OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP11-2002-SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



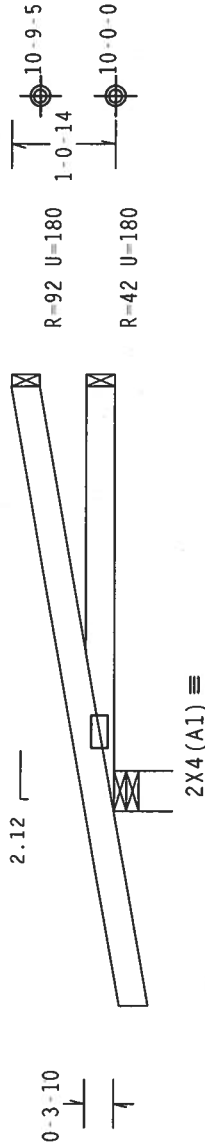
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



←-1-11-15→

←-4-4-6 Over 3 Supports→
R-345 U=180 W=4.949"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.24.125

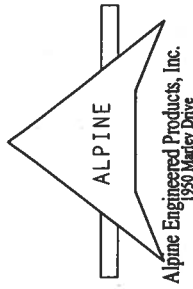
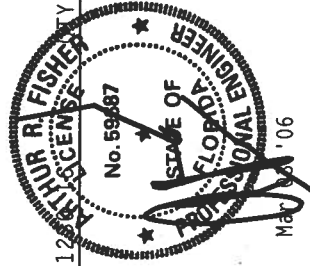
PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583
O'DONOHIO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN,
MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED,
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED
PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE
TRUSS TO THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE DESIGNER'S
DESIGN CONFORMS WITH APPLICABLE PROVISIONS AND SPECIFICATIONS OF THE TPI TRUSS PLATE INSTITUTE.
CONNECTOR PLATES ARE MADE OF 2018/166A (W-1/5/15) ASTM A553 GRADE 40/50 (W-1/5/15) GALV STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION OF THE TRUSS COMPONENT.

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487 -- 89708
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062004
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87308
DUR.FAC.	1.25		



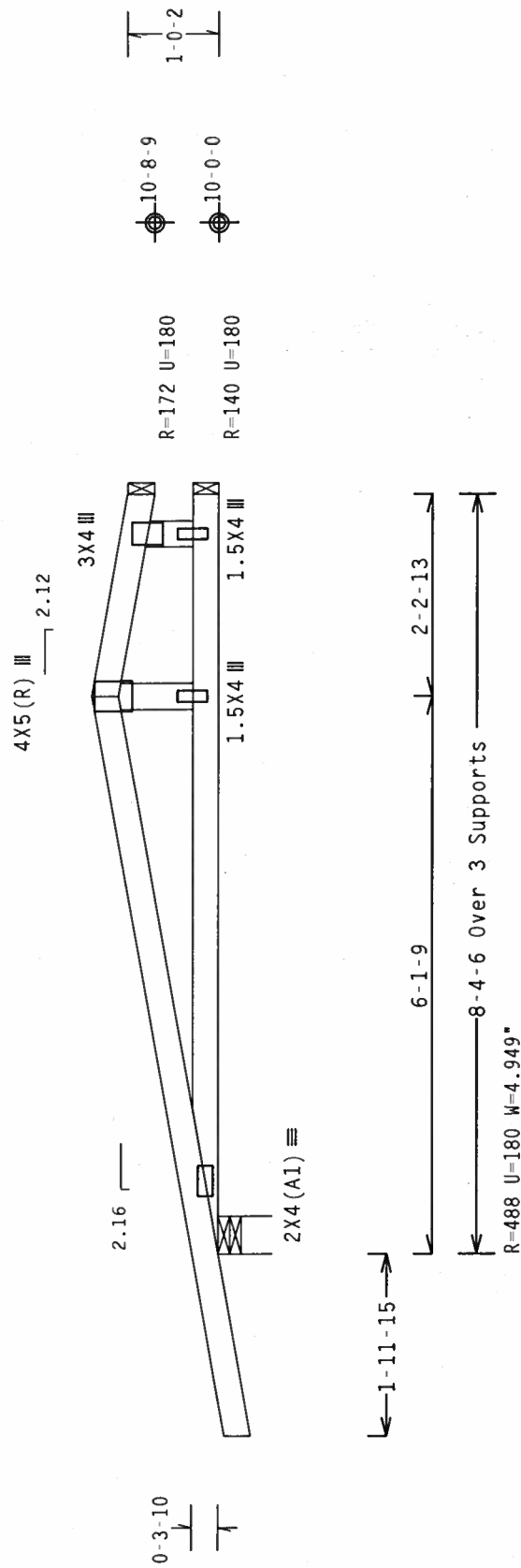
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

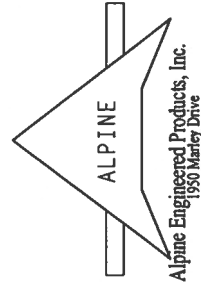
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



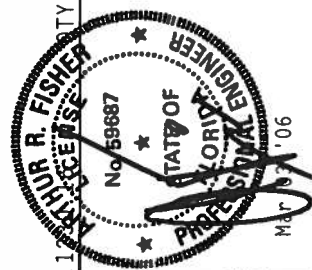
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

PLT TYP. Wave



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53719) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE DESIGNING OF THE TRUSS CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (M H/S) ASTM A653 GRADE 40/50 (M, K/H S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN CHORD. THE SUITABILITY AND USE OF THIS COMPONENT END AND BUILT-UP IS THE RESPONSIBILITY OF THE DESIGNER.



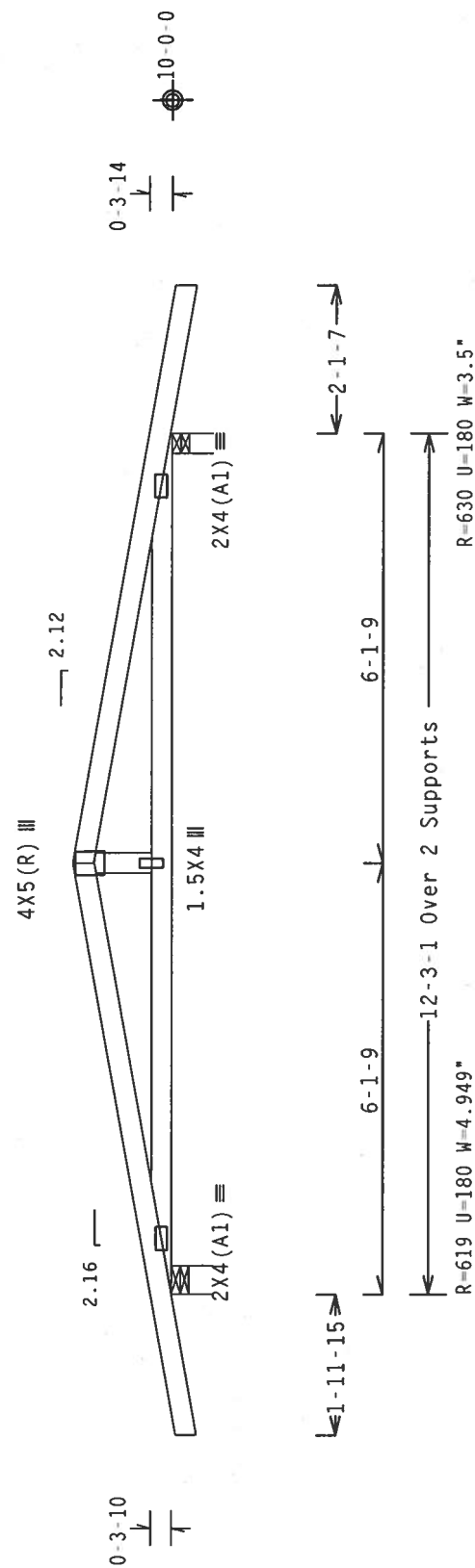
TC LL	20.0 PSF	FL/-4/-/-R/-	Scale = .5"/Ft.
TC DL	10.0 PSF	REF	R487-- 89709
BC DL	10.0 PSF	DATE	03/03/06
BC LL	0.0 PSF	DRW	HCUSR487 06062042
TOT.LD.	40.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	87317

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.

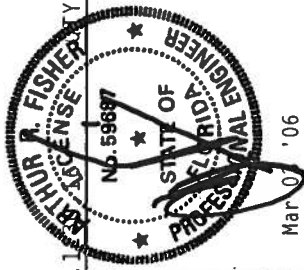
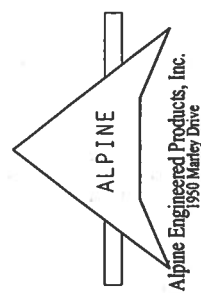


PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 O'DONOHIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, CHORD MEMBERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** UNLESS A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONS OR PROPERTY. THE TRUSS IS DESIGNED TO BE INSTALLED AND BRACED IN CONFORMANCE WITH TPI OR FABRICATING HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (N/A/S/K) ASTM A653 GRADE 40/50 (N/A/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THE LIABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



TC LL	20.0 PSF	REF	R487--	89710
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062043
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN	-	87323
DUR.FAC.	1.25			

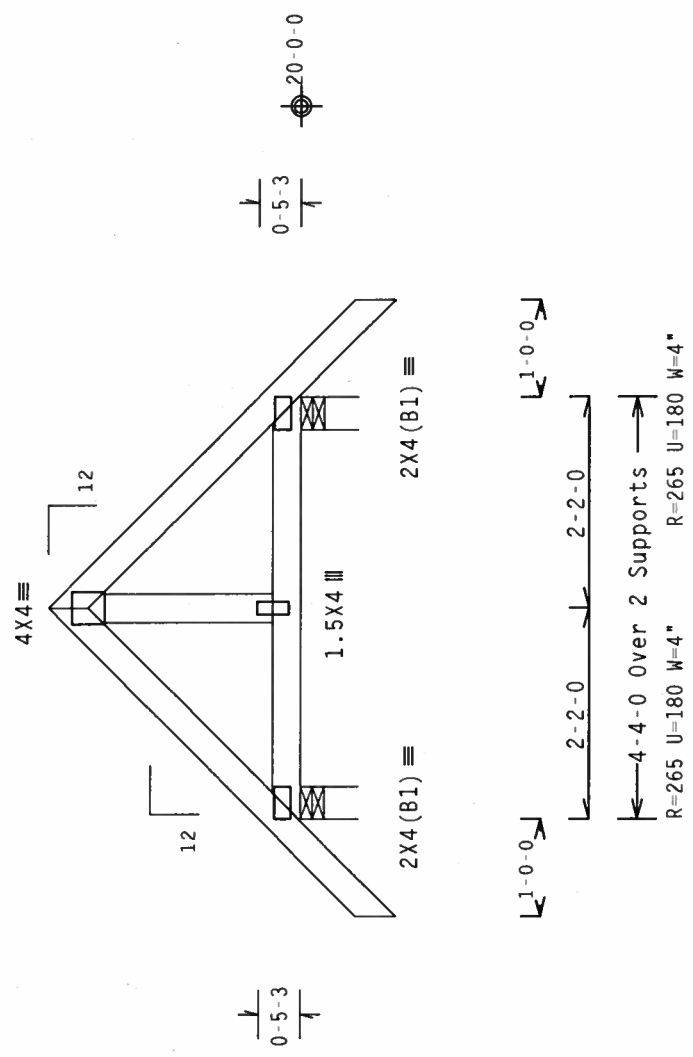
Scale = .375" / Ft.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

110 mph wind, 21.02 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.

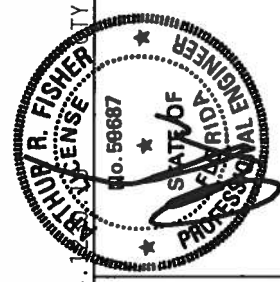
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

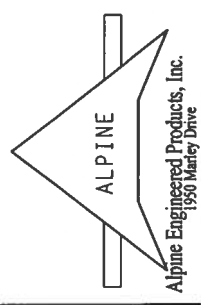
Scale = .5" / Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1.03 BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'DONOHIO DR., SUITE 200, MADISON, WI 53717) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, SUITE 100, FORT WORTH, TX 76116) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, YOURS OR OURS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (W.H/2/K) ASTM A653 GRADE 40/60 (W. K/H.5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

TC LL	20.0 PSF	REF	R487-- 89711
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062005
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	87011
DUR.FAC.	1.25		



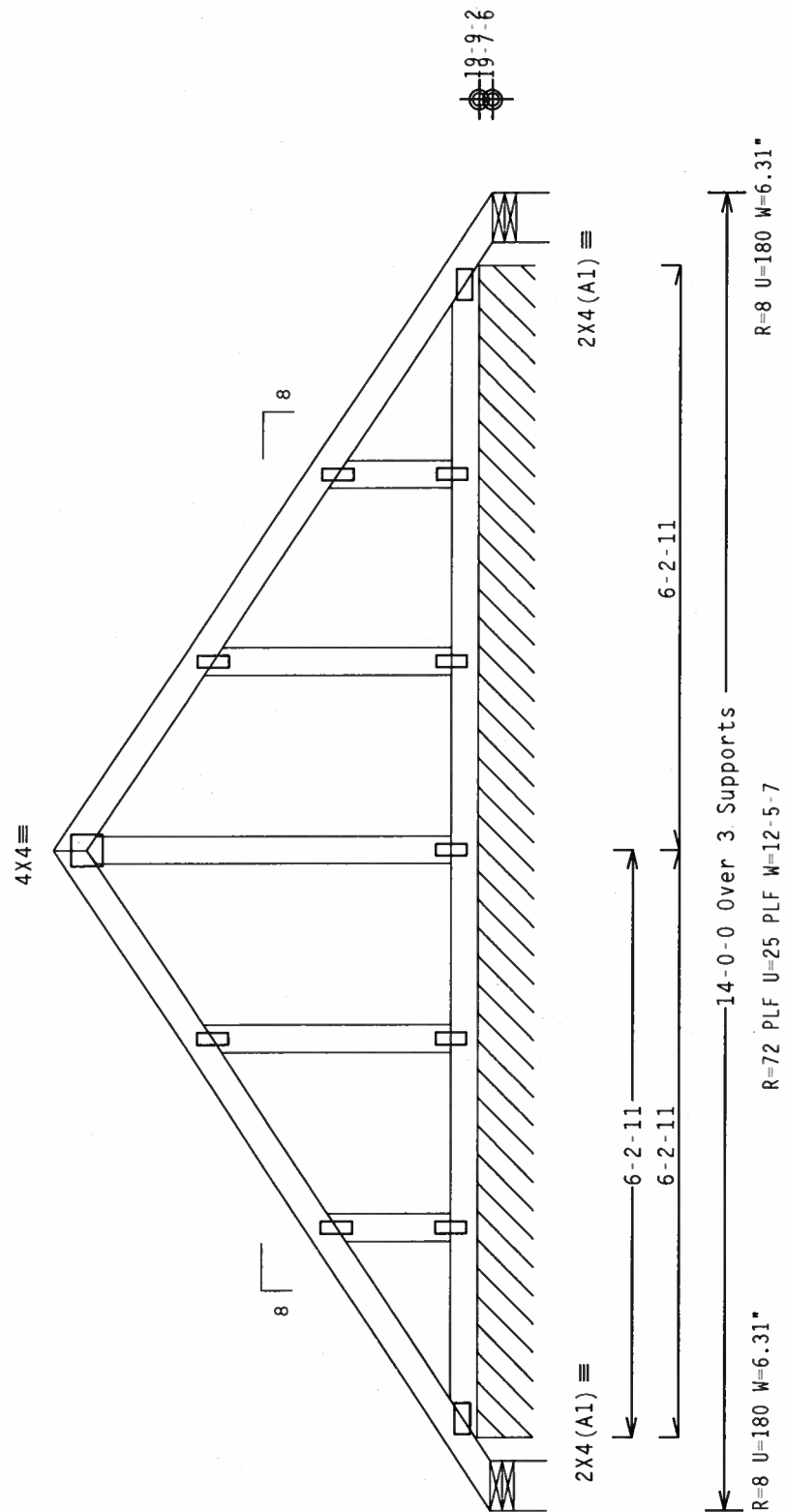
Top chord	2x4	SP #2	Dense
Bot chord	2x4	SP #2	Dense
Webbs	2x4	SP #3	

110 mph wind, 21.95 ft mean hgt, ASEE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

design c/c. 1P1-2002(SIU)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24.10

PROPERTY: 2 FL/-/4/-/R/-

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487 --	89712
TC DL	10.0 PSF	DATE	03/03/06
BC DL	2.0 PSF	DRW HCUSR487	06062044
BC LL	0.0 PSF	HC-ENG JB/AF	
TOT.LD.	32.0 PSF	SEQN-	87117
DUR.FAC.	1.25		

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 983 D. OROFOLD DR., SUITE 200, MADISON, WI 53719) AND WICKA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., FORT SMITH, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACP&A) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/716GA (W/H/S/K) A578 A653 GRADE 40/60 (W. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC-3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELLY FOR THE TRUSS COMPONENT

ALPINE

Alpine Engineered Products, Inc.
1950 Maple Drive

NO 59687

STATE OF

PROFESSIONAL ENGINEER

Ma 3/06

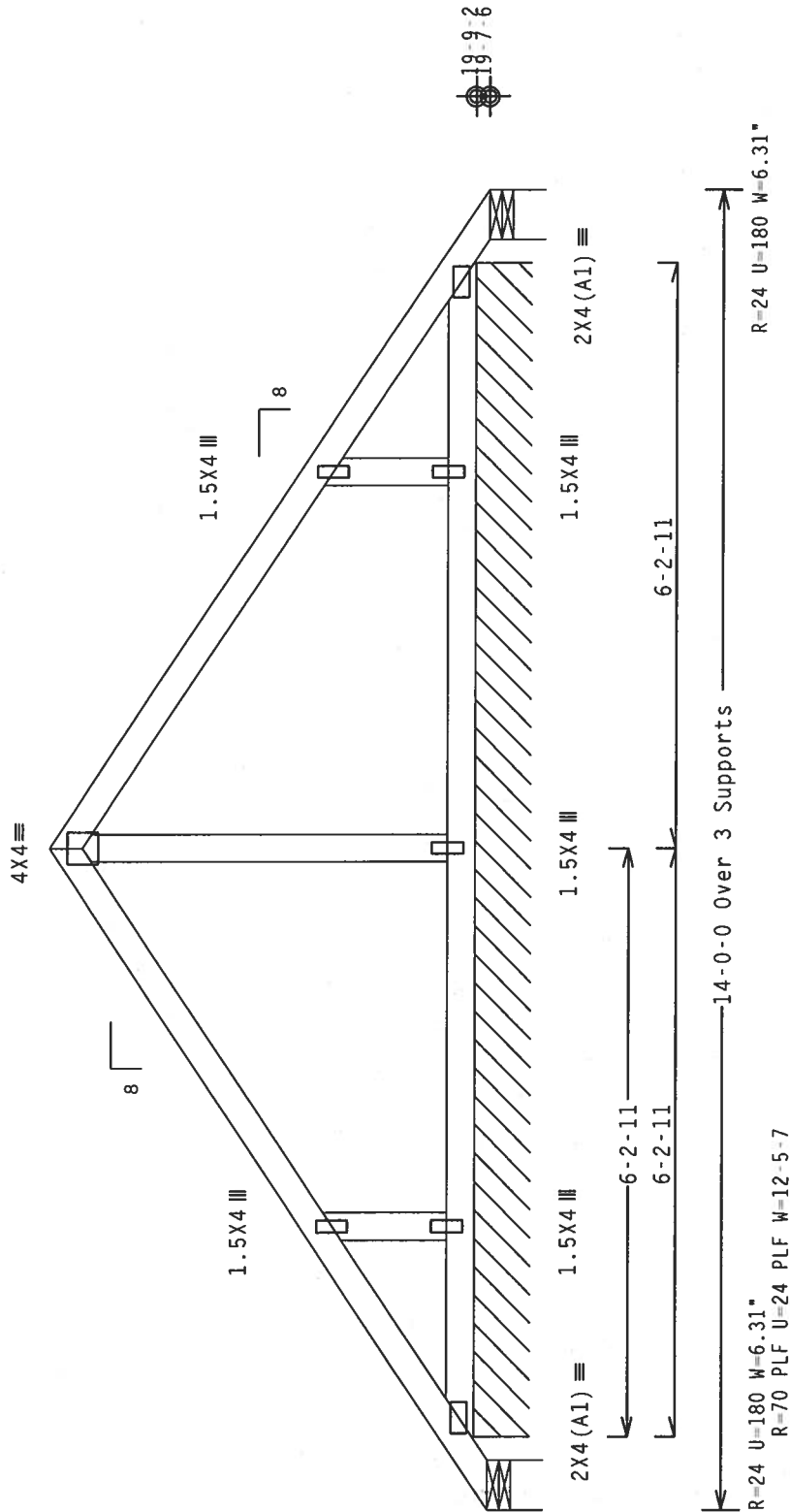
Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 21.95 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACK80405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED



PLT TYP. Wave

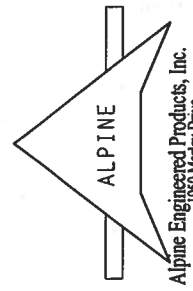
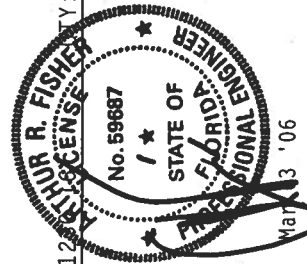
Design Crit: TPI-2002(STD)/FBC

PI-200Z(S10)/FBC
Cg/RT=1.00(1.25)/10(0) 7 24 12

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	2.0 PSF
BC LL	0.0 PSF
TOT.LD.	32.0 PSF
DUR.FAC.	1.25



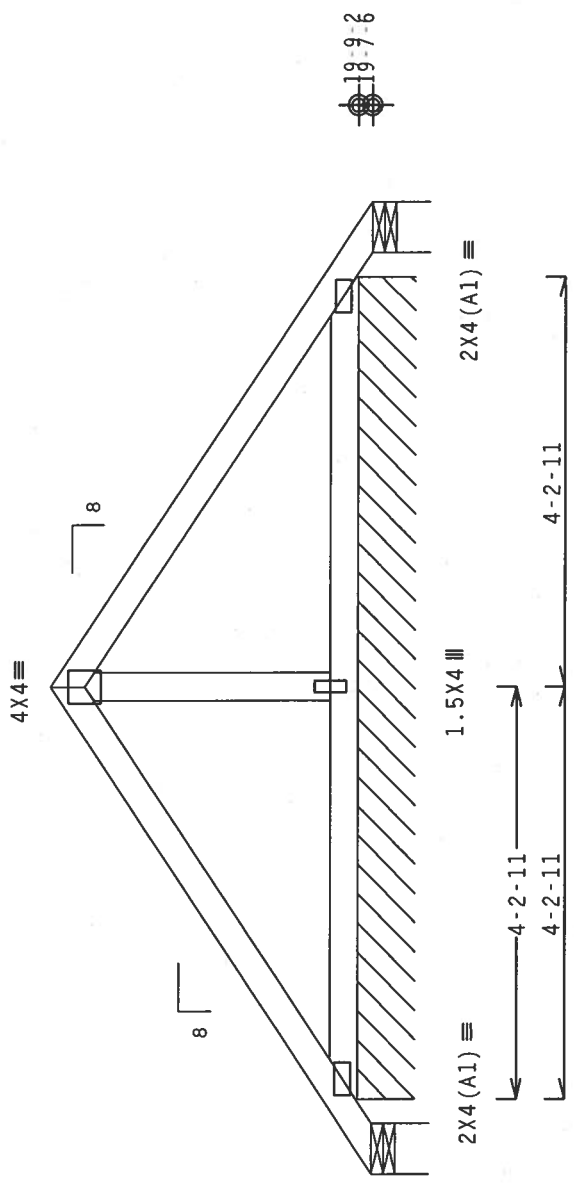
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 21.28 ft mean hgt. ASCE 7-02, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=62 U=180 W=6.31"
R=91 PLF U=35 PLF W=8-5-7
R=62 U=180 W=6.31"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.11

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487 -- 89715
TC DL	10.0 PSF	DATE	03/03/06
BC DL	2.0 PSF	DRW	HCUSR487 06062047
BC LL	0.0 PSF	HC-ENG	J8/AF
TOT.LD.	32.0 PSF	SEQN-	86992
DUR.FAC.	1.25		

ARTHUR R. FISHER
LICENSE
No. 69887
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
Mar 07 2006

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** UNLESS A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE INSTALLING OF TRUSSES SHALL CONFORM WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. IN ALL APPLICABLE AREAS. CONNECTOR PLATES ARE MADE OF 2017B16GA (W 1/2X1/8) ASTM A653 GRADE 40/50 (W 1/2X1/8) GALT. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THE ENTIRETY AND USE OF THIS DOCUMENT FOR ANY PURPOSE IS THE RESPONSIBILITY OF THE USER.

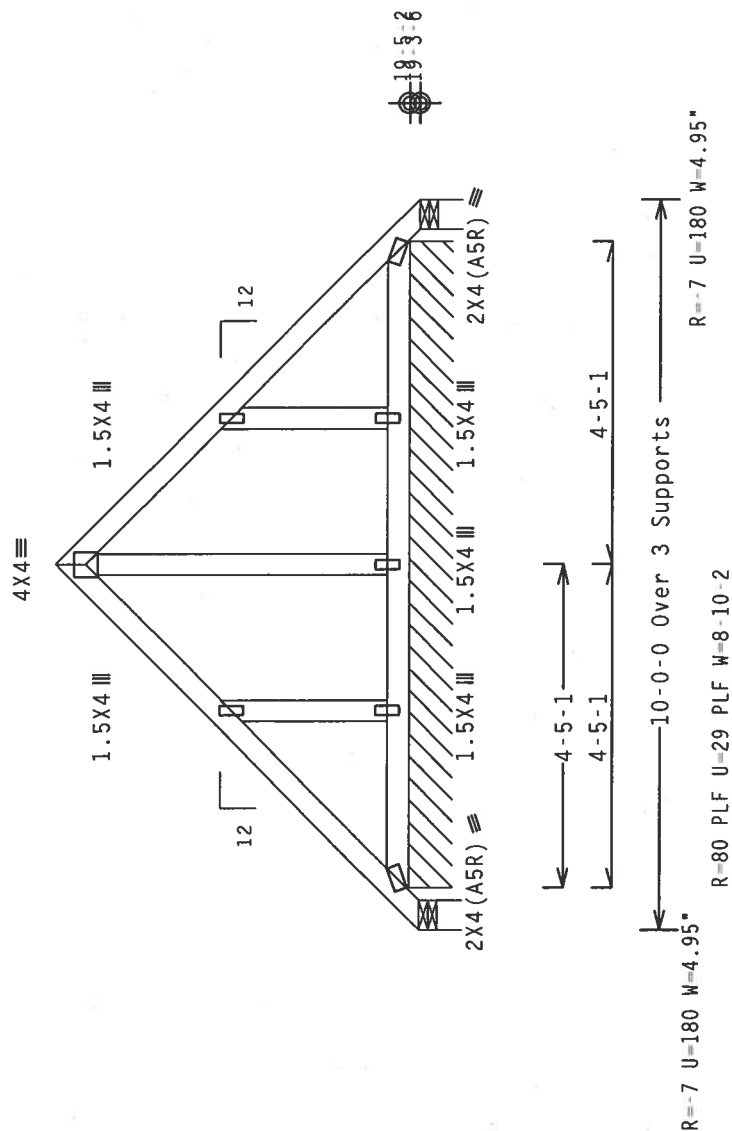
Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

1110 mph wind, 21.78 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED



Design Crit: TPI-2002(STD)/FBC

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI BUILDING INFORMATION SYSTEMS, BCSI TRUSS PRODUCTS INSTITUTE, 583 D'ONOFIO RD., SUITE 200, MADISON, WI 53719, AND WTC4000 TRUSS COMPANY, 1000 W. MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
THIS DESIGN IS BASED ON THE ASSUMPTION THAT THERE WILL BE NO DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE DESIGN IN CONFORMANCE WITH THE RESPONSE SPECIFICATIONS OF THE ALPINE ENGINEERING COMPANY SHALL BE THE RESPONSIBILITY OF THE ALPINE DESIGNER.
THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&AP) AND TPI
DESIGN CONNECTOR PLATES ARE MADE OF 20/18/16GA (.019/.020/.025") ASTM A663 OR 40760 (M. K/H-S) GALV. STEEL.
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.
DRAWING INDICATES ACCEPTANCE CRITERIA FOR ALL LOW RISE SMALL MEMBER SPANS UP TO 11' - 0".
THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.

QTY: 2 FL / - / 4 / - / - / R / -

Scale = .375"/Ft.

TC LL	20.0 PSF
-------	----------

REF R487-- 89716

TC DL 10.0 PSF

DATE 03/03/06

BC DI 2.0 PSE

ORW HC11SR487 06062048

350	0	11	28
351	0	11	28

11000K40, 00002040
JC-ENG 1B/AE

00 77	0.0	131
TOT 10	33 0	135

IC LING 00/AF
SEON 07A10

101. LD: 32.0 PSF

SEQU- 8/418



Max '06

PLT TYP. Wave



Alpine Engineered Products, Inc.
1950 Marley Drive

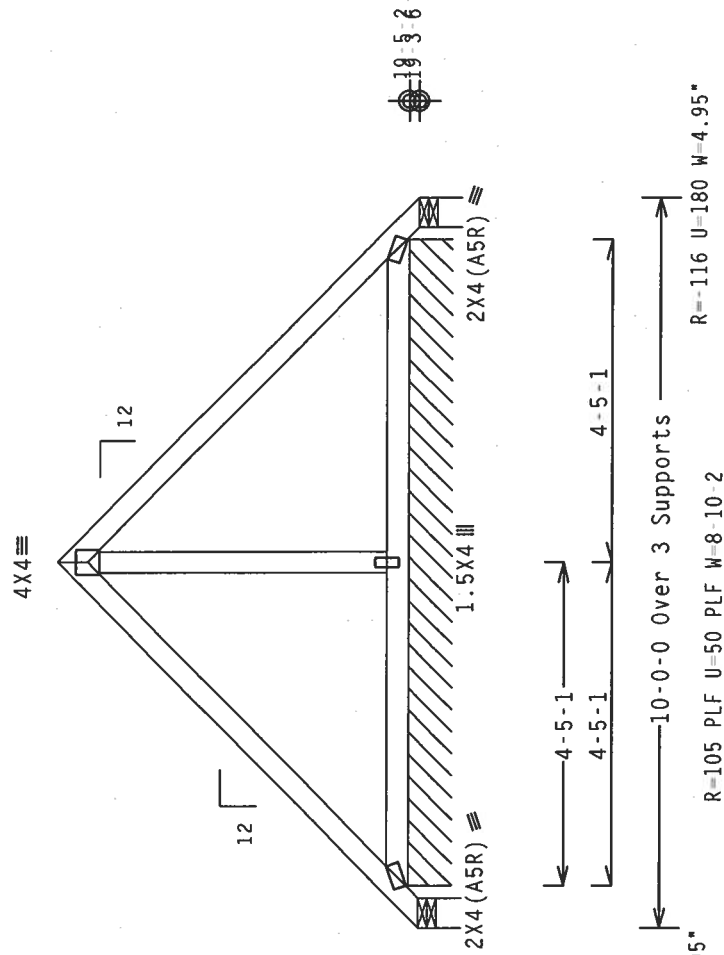
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.78 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2
psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.

REFER TO DWG PIGBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED



PLT TYP. Wave

Alpine Engineered Products, Inc.
1950 Mandy Drive

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12

Scale = .375"/Ft.

TC LL	20.0 PSF	REF	R487--	89717
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	2.0 PSF	DRW	HCUSR487	06062049
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	32.0 PSF	SEQN-	87413	
DUR.FAC.	1.25			

106

STATE OF CALIFORNIA
Professional Engineer
No. 59887
HUBERT B. FISHER
LICENSE

106

ALPINE ENGINEERED PRODUCTS, INC.
1950 MANDY DRIVE
DALLAS, TEXAS 75244
TEL: 972-412-1000
FAX: 972-412-1001
WWW.ALPINEENGINEERING.COM

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

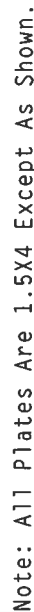
****IMPORTANT**** *URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&SP) AND TPI. THE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H./K) ASTM A653 GRADE 40/50 (W. K/M.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

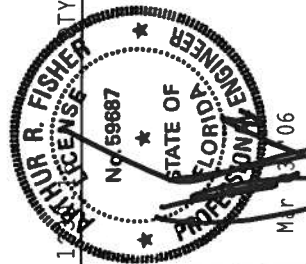
PLT TYP. Wave

$$Cq/RT = 1.00(1.25)/10(0)$$

511-1A1-1-1B1-

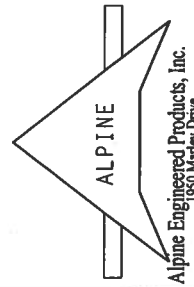
Scale = .1875"/Ft.

TC LL	20.0	PSF	REF	R487--	89718
TC DL	10.0	PSF	DATE	03/03/06	
BC DL	10.0	PSF	DRW	HCUSR487	06062050
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	87538	REV
DUR.FAC.	1.25				



*****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS 1-608 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY IPTI (TRUSS PULPIT INSTITUTE, 583 W. 10TH AVE., MINNEAPOLIS, MN 55402) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 530 N. DEARBORN AVE., MADISON, WI 53716) FOR SAFETY GUIDELINES. TRUSSES ARE NOT TO BE USED FOR ANY PURPOSES UNLESS OTHERWISE SPECIFIED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND BRACING OF TRUSSES. THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MODS NATIONAL DESIGN SPEC. IN CONNECTION WITH THE DESIGN OF THE TRUSS, THE FOLLOWING ASSUMPTIONS ARE MADE:
CONNECTION PLATES ARE MADE OF 20X18/16GA. U.S. #12W/16GA. ASTM A563 GRADE 40/50 (W. 7.5/5.5) GALV. STEEL.
PLATES TO EACH FACE OF TRUSS AND UNDER/HORIZEWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 16GA. Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 43 OF TP11-2002 SEC.3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
ALPINE ENGINEERED



Top chord 2x4 SP #2 Dense
Bot chord 2x10 SP SS :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

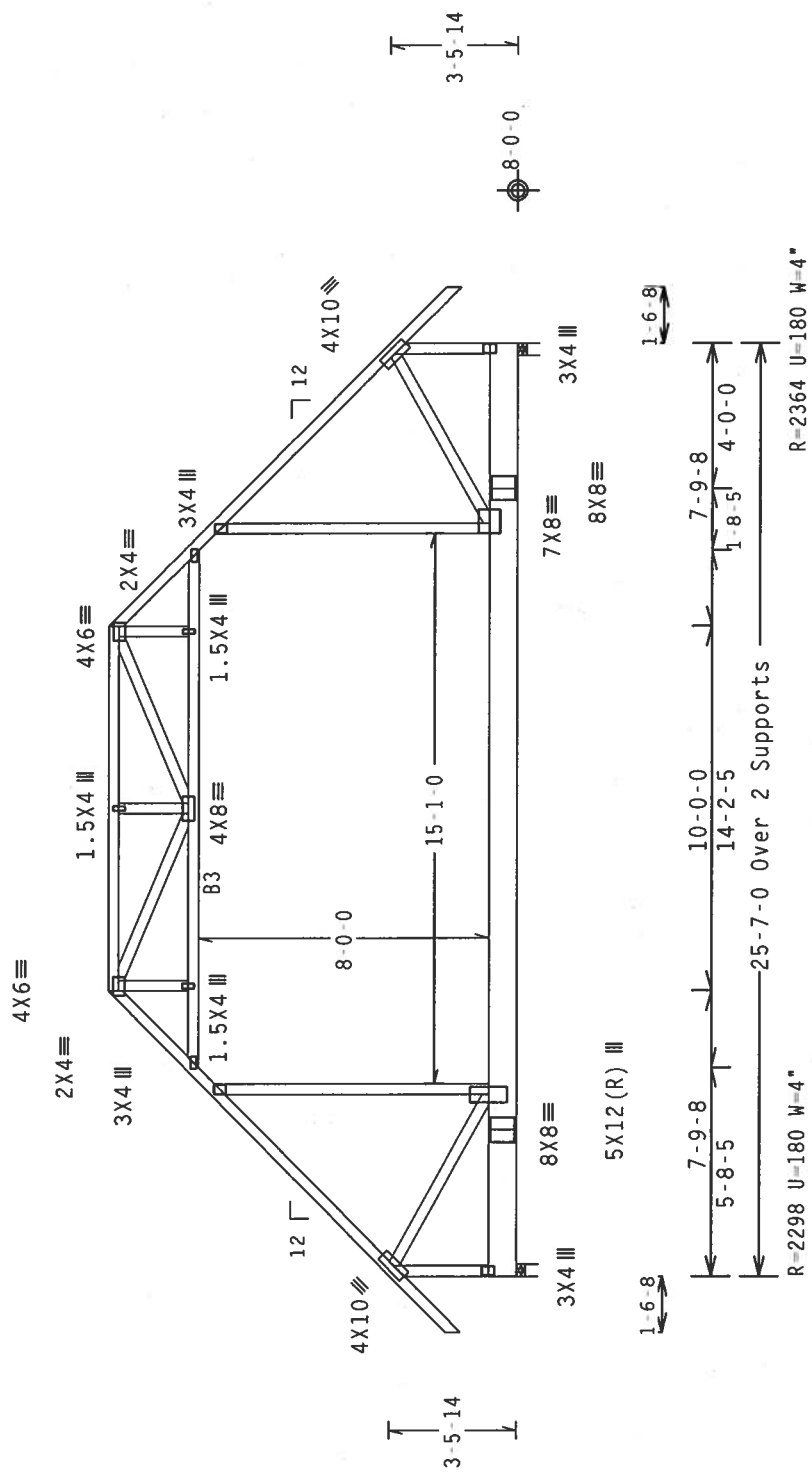
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-3-0 to 20-7-8.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



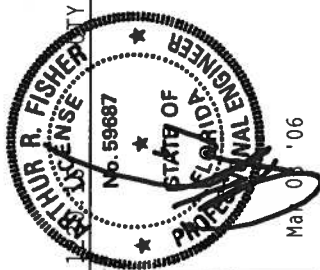
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

1
ARTHUR R. FISHER
LICENSE
EXPIRY: 11 FL / - 4 / - / R / -

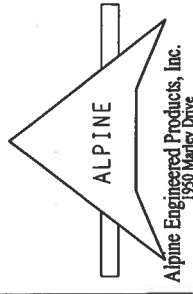
Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487 -- 89719
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062051
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87432
DUR.FAC.	1.25		



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PACE INSTITUTE, 583 D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE FOLLOWING LISTING OF MATERIALS AND DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BUREAU OF ALPHABETICALLY ORDERED CONNECTOR PLATES ARE MADE OF 2018/16GA (M/TH) ASTM A563 GRADE 40/60 (W. E/W/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THE DESIGN, POSITION PER DRAWINGS 16DA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



Top chord 2x4 SP #2 Dense

Bot chord 2x10 SP SS :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.) nails)
 Top Chord: 1 Row @ 9.25" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails
 in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

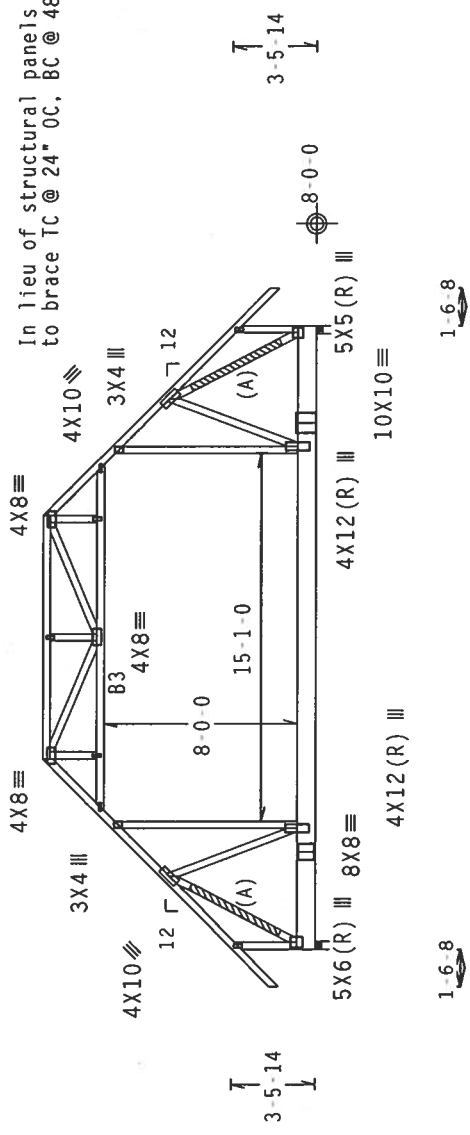
End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Calculated horizontal deflection is 0.12" due to live load and 0.19" due to dead load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 48" OC.



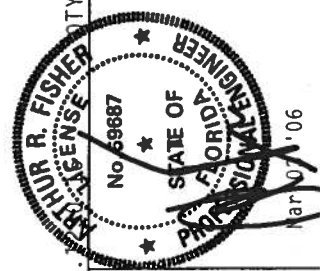
Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

 $Cq/RT=1.00(1.25)/10(0) \quad 7.24.$

TC LL	20.0	PSF	REF	R487 --	89720
TC DL	10.0	PSF	DATE	03/03/06	
BC DL	10.0	PSF	DRW	HCUSR487	06062052
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	87568	
DUR.FAC.	1.25				



****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1. OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY A/E/P/A) AND TP1. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1816GA. (M. H/2/S) ASTM A653 GRADE 40/60 (M. K/H/S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND (M. H/2/S) OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP1. 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Top chord 2x4 SP #2 Dense
Bot chord 2x10 SP SS : B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

- SPECIAL LOADS
- (LUMBER DUR. FAC. = 1.25 / PLATE DUR. FAC. = 1.25)
 - TC - From 174 PLF at 5.25 to 174 PLF at 5.11
 - TC - From 129 PLF at 6.11 to 129 PLF at 7.79
 - TC - From 129 PLF at 7.79 to 129 PLF at 19.48
 - TC - From 174 PLF at 19.48 to 174 PLF at 20.33
 - TC - From 129 PLF at 20.33 to 129 PLF at 25.58
 - TC - From 129 PLF at 25.58 to 129 PLF at 27.13
 - PLT - From 32 PLF at 6.36 to 32 PLF at 19.22
 - BC - From 9 PLF at 1.50 to 9 PLF at 0.00
 - BC - From 32 PLF at 0.00 to 32 PLF at 5.25
 - BC - From 190 PLF at 5.25 to 190 PLF at 20.33
 - BC - From 32 PLF at 20.33 to 32 PLF at 25.58
 - BC - From 9 PLF at 25.58 to 9 PLF at 27.13
 - BC - 226 LB Conc. Load at 5.25, 20.33

Collar-tie braced with continuous lateral bracing at 24" OC.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25" min.)_nails)
Top Chord: 1 Row @12.00" O.C.
Bot Chord: 1 Row @12.00" O.C.
Webs : 1 Row @ 4" O.C.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

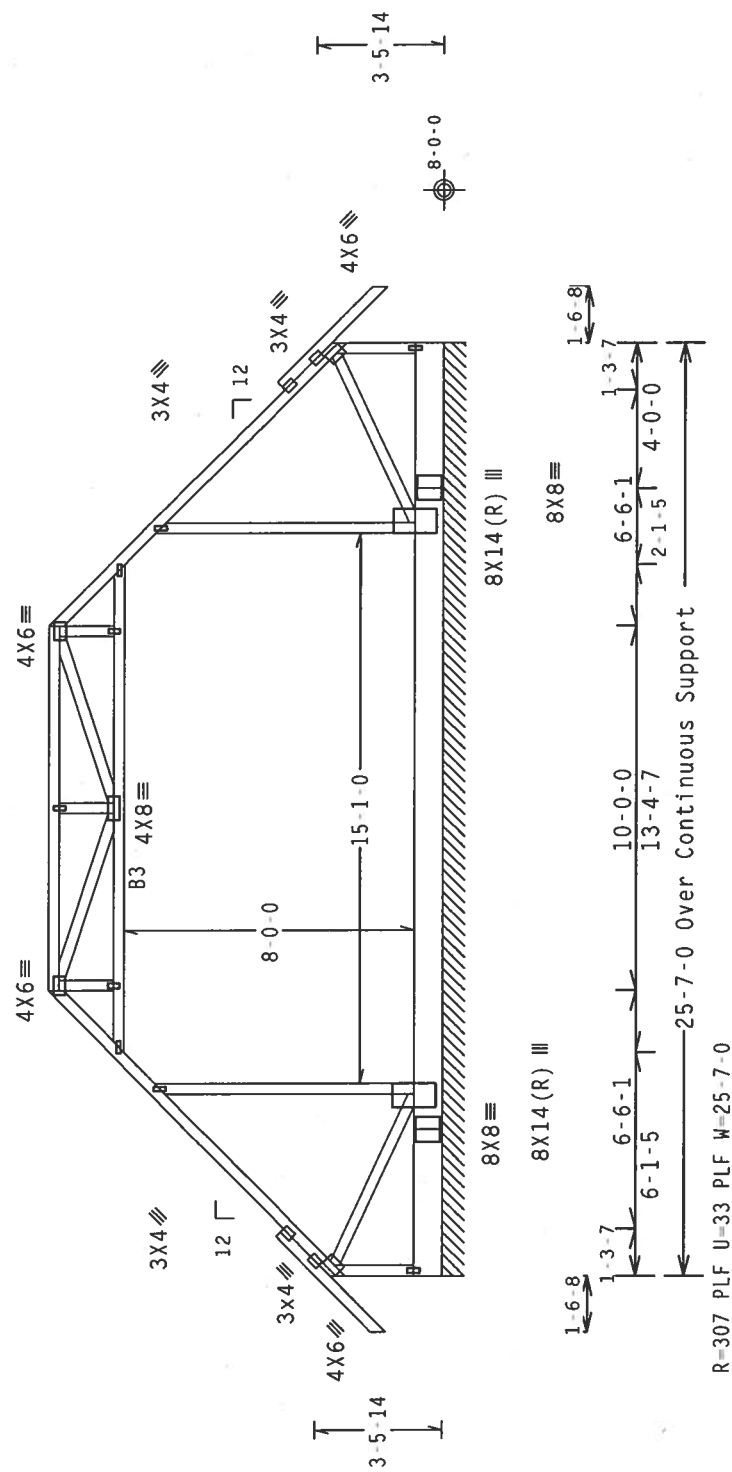
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Trusses to be spaced at 38.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 48" OC.



Note: All Plates Are 1.5X4 Except As Shown.

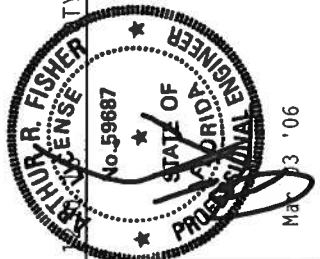
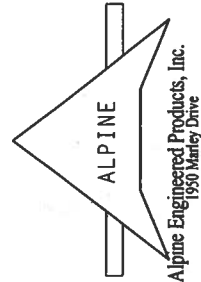
Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.24.18

PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN ACCORDANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL CONFORM WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFPA) AND TPI. APPLY CONNECTOR PLATES ARE MADE OF 2018/16GA (W.H./K) ASTM A653 GRADE 40/50 (N. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



TC LL	20.0 PSF	REF	R487--	89721
TC DL	10.0 PSF	DATE	03/03/06	
BC DL	10.0 PSF	DRW	HCUSR487	06062053
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	87546	REV
DUR.FAC.	1.25			

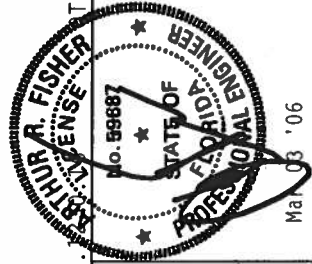
Scale = .1875"/Ft.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

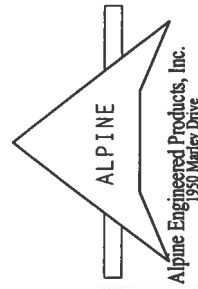
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Scale = .5"/Ft.

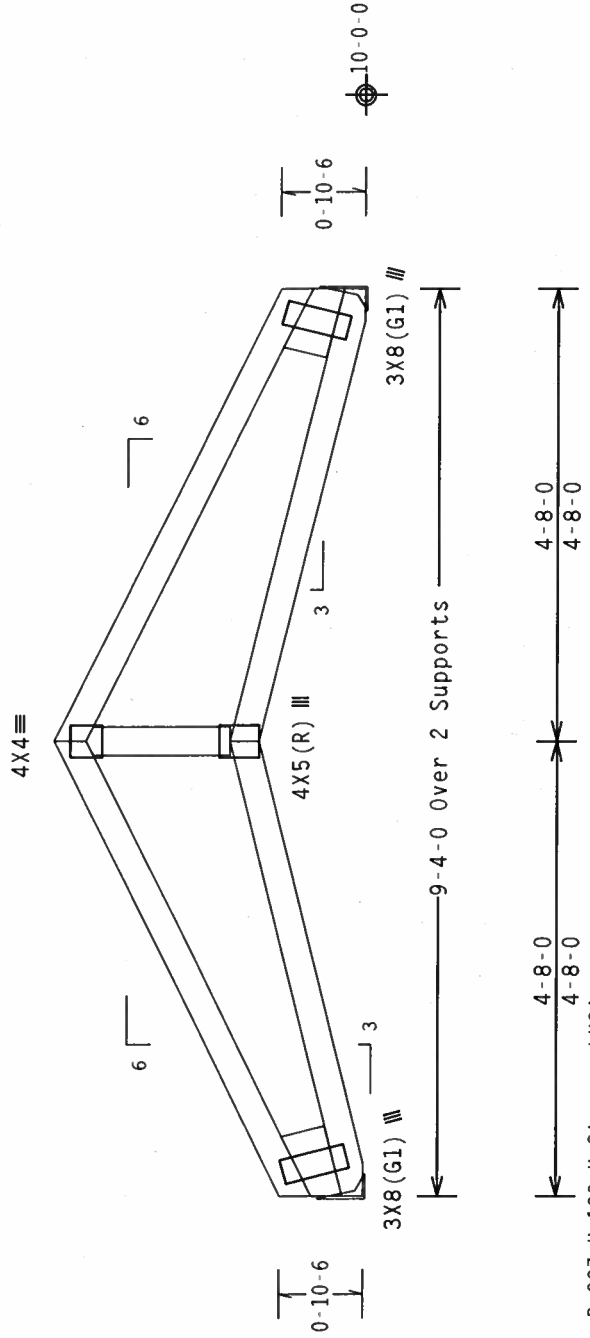
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TC DL	10.0	PSF	DATE	03/03/06	
BC DL	10.0	PSF	DRW	HCUSR487	06062054
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	87557	REV
DUR.FAC.	1.25				

Mar 03 '06



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubby Wedge 2x6 SP #1 Dense:
:Rt Stubby Wedge 2x6 SP #1 Dense:
In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.
Deflection meets L/360 live and L/240 total load. Creep increase factor
for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg. Located
anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0
psf.
H = recommended connection based on manufacturer tested capacities and
calculations. Conditions may exist that require different connections
than indicated. Refer to manufacturer publication for additional
information.



R=387 U=180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X4 min. So.Pine

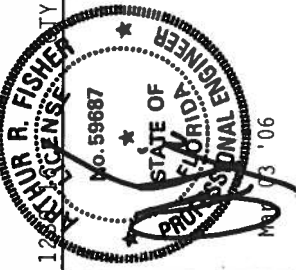
R=387 U=180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X4 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

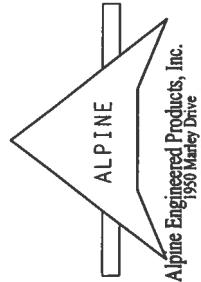
PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCST 1-03 BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583
O'CONNOR DR., SUITE 200, WAUWATON, WI 53190) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN,
WILSONVILLE, OR 97137) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED,
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED
PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THE DESIGN SPECIFICATIONS. ANY FAILURE
OR DAMAGE TO THE TRUSS OR TO THE BUILDING SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.
DESIGN CONFORMS WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
CONNECTOR PLATES ARE MADE OF 2018/166A (W.N/2/K) ASTM A553 GRADE 40/60 (W. K/H.5) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 11 2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

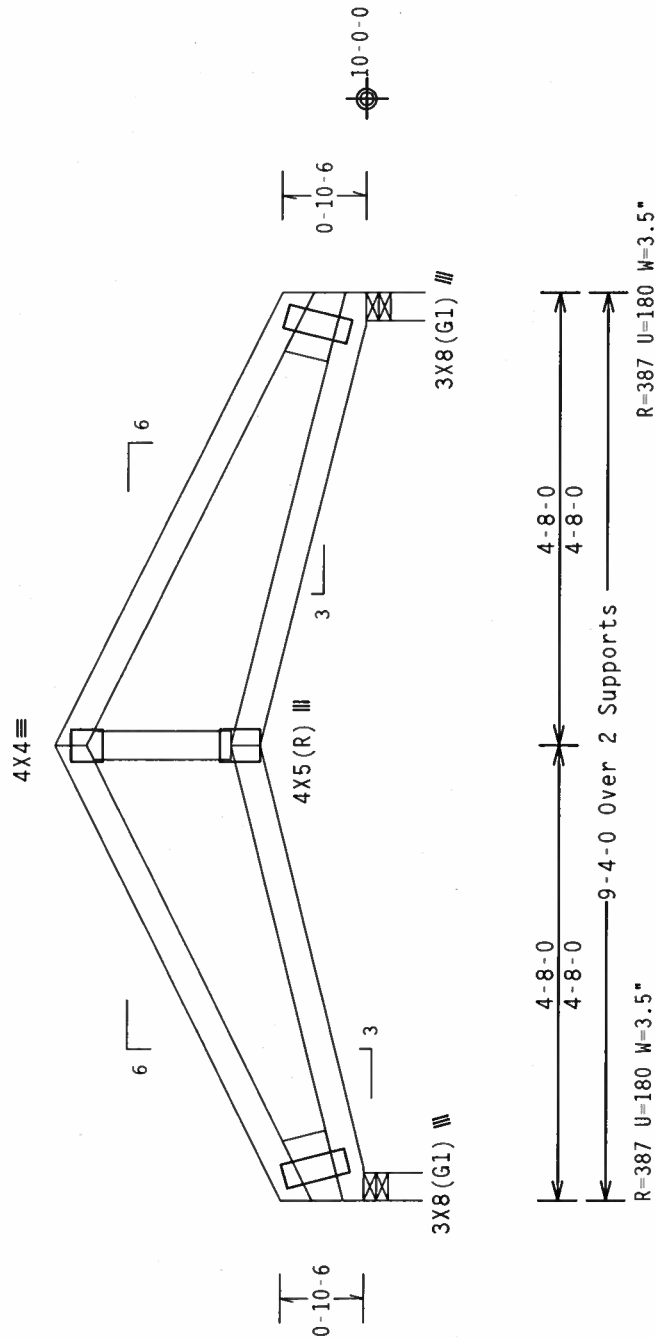


TC LL	20.0 PSF	FL/-/4/-/R/-	Scale = .5"/Ft.
TC DL	10.0 PSF	REF	R487-- 89723
BC DL	10.0 PSF	DATE	03/03/06
BC LL	0.0 PSF	DRW	HCUSR487 06062006
TOT.LD.	40.0 PSF	HC-ENG	JB/AF *
DUR.FAC.	1.25	SEQN-	87171



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbbed Wedge 2x6 SP #1 Dense:
:Rt Stubbbed Wedge 2x6 SP #1 Dense:
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

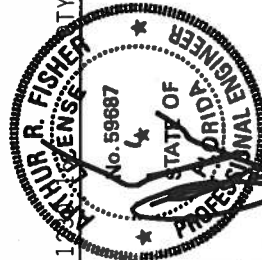
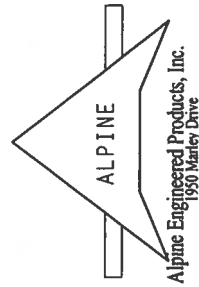
7.24.1

FL/-4/-/-R/-

Scale = .5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 89724
TC DL	10.0 PSF	DATE	03/03/06
BC DL	10.0 PSF	DRW	HCUSR487 06062007
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	87179
DUR.FAC.	1.25		

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

2X4 GABLE VERTICAL SPACING		BRACE GRADE	BRACE		(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
MAX GABLE VERTICAL LENGTH	12" O.C.	SPF	3' 10"	6' 8"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	3' 9"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	3' 9"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	4' 3"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"	14' 0"
24" O.C.	SP	#2	4' 2"	6' 8"	7' 2"	7' 11"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 0"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#1 / #2	4' 5"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#3	4' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	4' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	9' 1"	9' 1"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#3	4' 6"	7' 6"	9' 1"	9' 1"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#1 / #2	4' 11"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	HF	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	5' 0"	8' 5"	9' 1"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

MAX GABLE VERTICAL LENGTH

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH

DOUGLAS FIR-LARCH		SOUTHERN PINE	
#3	STUD	#3	STUD
STANDARD	STANDARD	STANDARD	STANDARD

GROUP B:

HEM-FIR		DOUGLAS FIR-LARCH	
#1 & BTR	#1	#1	#2

SOUTHERN PINE

SOUTHERN PINE		DOUGLAS FIR-LARCH	
#1	#2	#1	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

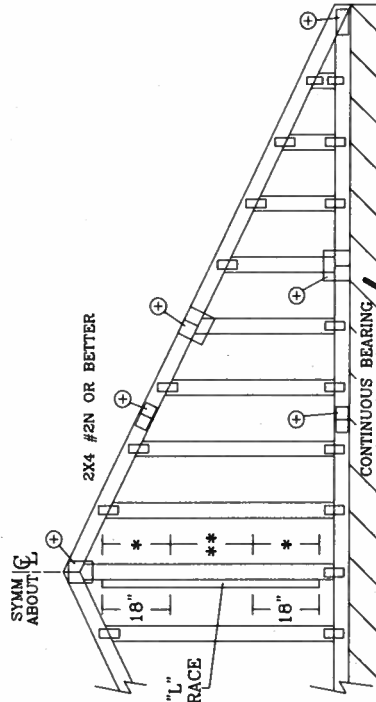
IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

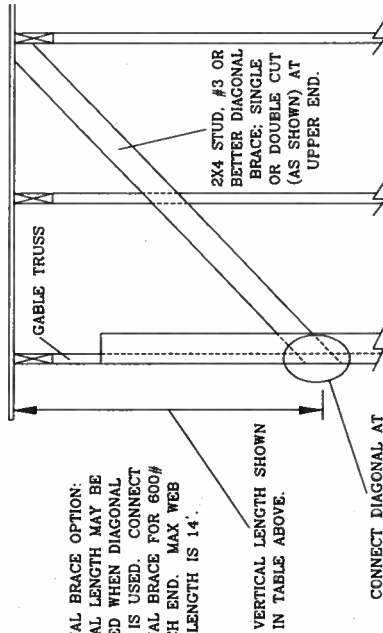
IN 16" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH	NO SPLICE		
		1X4 OR 2X3	
		2X4	
		2.5X4	
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.			



SYMM. ABOUT C



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

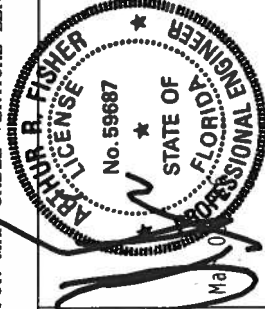
REFER TO CHART ABOVE FOR MAX. GABLE VERTICAL LENGTH.

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ALPINE ENGINEERED PRODUCTS, INC.
POMPAHO BEACH, FLORIDA

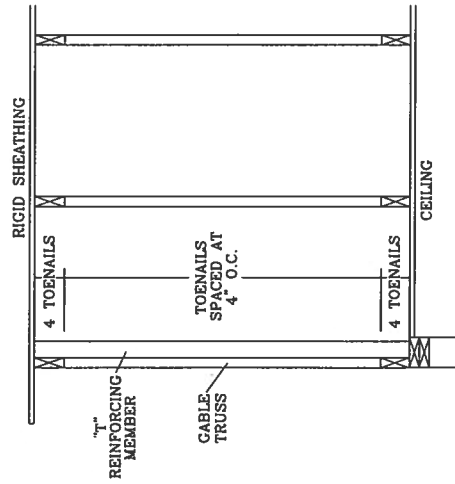
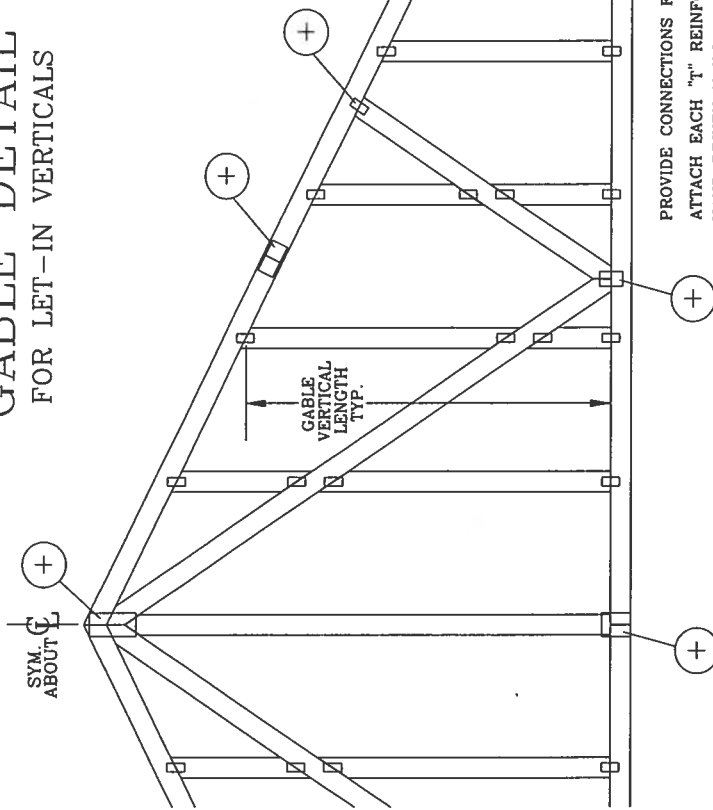


REF	ASCE7-02-GAB1015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	

MAX. TOT. LD. 60 PSF

TRUSS BRACING 04-05

GABLE DETAIL FOR LET-IN VERTICALS



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH HAND DRIVEN NAILS:

10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS (4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:

8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS

A10015ENI103, A09015ENI103, A08015ENI103, A07015ENI103, A11030ENI103, A10030ENI103, A09030ENI103, A08030ENI103, A07030ENI103

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015ECI103, A12015ECI103, A11015ECI103, A10015ECI103, A08515ECI103, A13030ECI103, A12030ECI103, A11030ECI103, A10030ECI103, A08530ECI103

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015EE0405, A12015EE0405, A11015EE0405, A10015EE0405, A08515EE0405, A13030EE0405, A12030EE0405, A11030EE0405, A10030EE0405, A08530EE0405

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

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GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
GREATER THAN 4' 0"	1X4 OR 2X3	2X8
LESS THAN 4' 0" BUT GREATER THAN 1' 6"	2X4	2X8
GREATER THAN 1' 6"	2.5X4	2.5X8

* REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

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EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

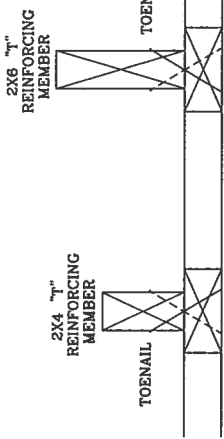
EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8

EXAMPLE: 2X4 2X4 2X8



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "L" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

REPLACES DRAWINGS GAB98117 876,719 & HC26294035

REF	LET-IN VERT
DATE	04/14/05
DRWG	GBLETTIN0405
-ENG	DLJ/KAR

MAX TOT. LD.	60 PSF
DUR. FAC.	ANY

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

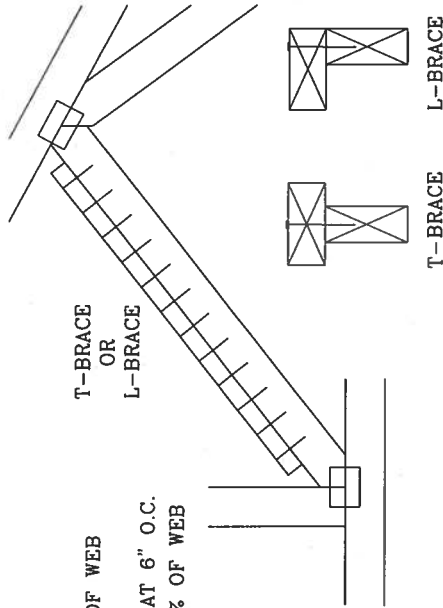
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

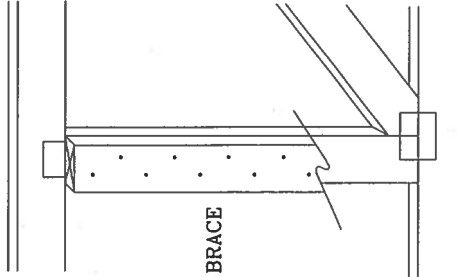
T-BRACING
OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB
NARROW FACE
ATTACH WITH 16d NAILS AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB
MEMBER LENGTH

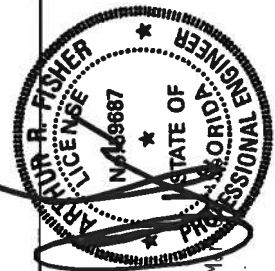


SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d OR .128"x3" GUN
NAILS AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579.640



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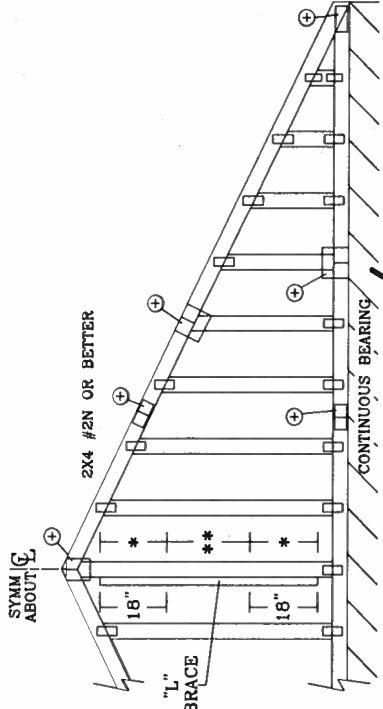
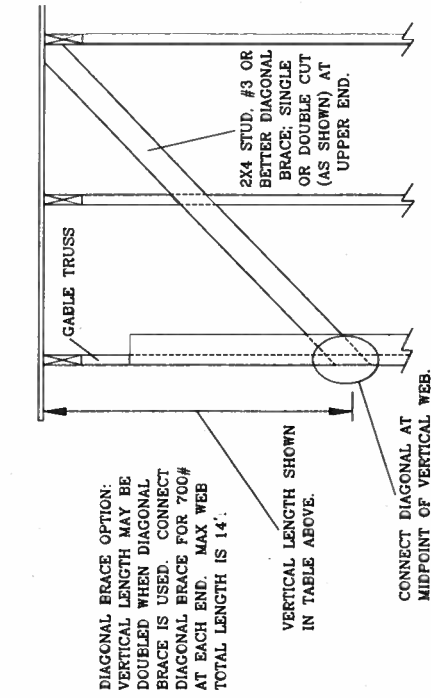
TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCLBSUB1103
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

ASCE 7-02: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		2X4 GABLE VERTICAL SPACING	BRACE GRADE	NO BRACES		(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
						GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	STUD	3' 8"	6' 4"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
				3' 7"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
				3' 7"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	#1	3' 7"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
				4' 0"	6' 4"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
				3' 11"	6' 4"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
	DFL	#3	STUD	3' 9"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
				3' 9"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
				3' 8"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
		#1 / #2	STANDARD	4' 2"	7' 3"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
				4' 1"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
				4' 1"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
16" O.C.	SPF	#1	STUD	4' 1"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
				4' 7"	7' 3"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 6"	7' 3"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#3	STANDARD	4' 4"	6' 10"	8' 7"	8' 7"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 4"	6' 9"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 2"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
	DFL	#1 / #2	STUD	4' 7"	8' 0"	9' 5"	9' 5"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 6"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 6"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	#1	4' 6"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
				5' 1"	8' 0"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 11"	8' 0"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	#3	STUD	4' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 7"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"
	DFL	#3	STANDARD	4' 7"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"
				4' 7"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"
				4' 7"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	HEM-FIR
#1 / #2 STANDARD	#2 STUD
#3	#3 STANDARD

DOUGLAS FIR-LARCH

#3	SOUTHERN PINE
STUD	#3
STANDARD	STANDARD

GROUP B:

HEM-FIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
	#2

SOUTHERN PINE

#1	
#2	

GABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS L/240.
- PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
- GABLE END SUPPORTS LOAD FROM 4' 0"
- OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH "L" BRACE WITH 10d NAILS.
- * FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
- ** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
- "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

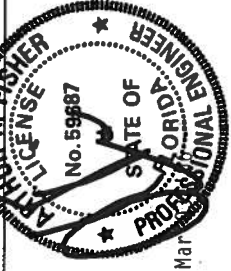
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

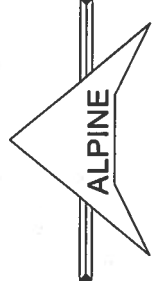
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

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REF	ASCE7-02-CAB11030
DATE	04/14/05
DRWG	A11030EE0405
-ENG	
MAX. TOT. LD. 60 PSF	
MAX. SPACING 24"	



ALPINE ENGINEERED PRODUCTS, INC.
POMPAHO BEACH, FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG,

LOCATED ANYWHERE IN ROOF, CAT II, EXP C,

WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC

ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

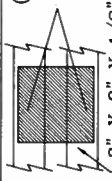
WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE

PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

PIGGYBACK DETAIL

(4) 6d BOX (0.099" X 2" MIN) NAILS.

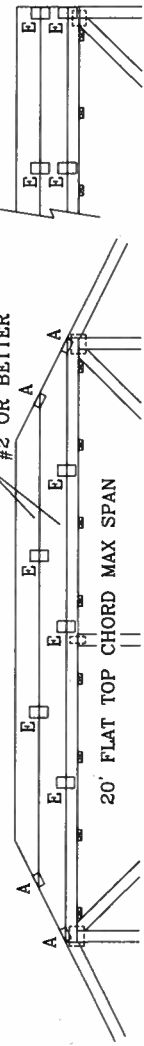


8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES. ATTACH WITH (8) 6d BOX (0.099" X 2" MIN) NAILS PER GUSSET.

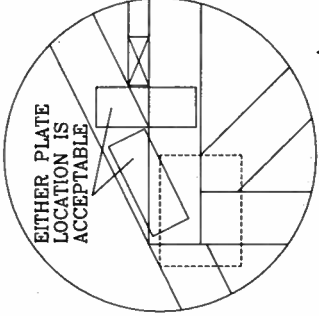
(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

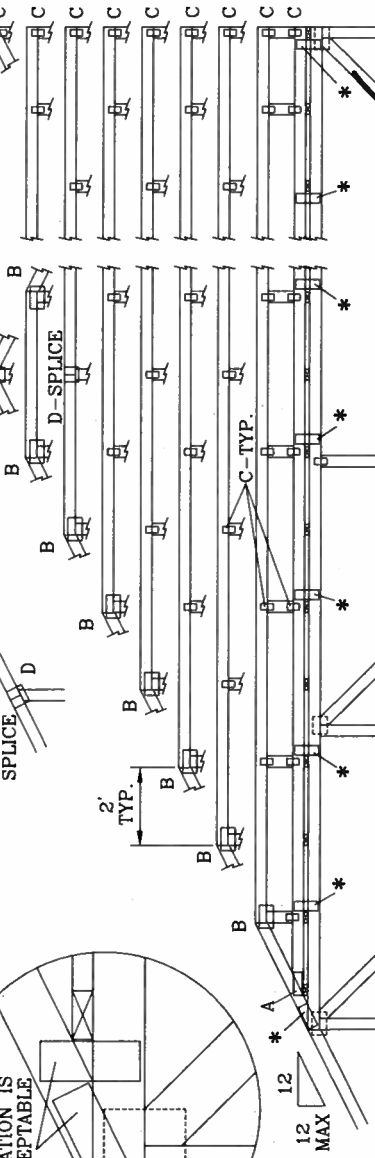
MAX SIZE OF 2X12 #2 OR BETTER



EITHER PLATE LOCATION IS ACCEPTABLE



OPTIONAL SPLICE



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

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ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

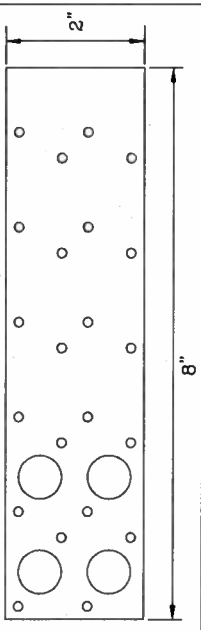
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB LENGTH	WEB BRACING CHART	
	NO BRACING	REQUIRED BRACING
0' TO 7'9"		
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113" X 2.5" MIN) NAILS AT 4" OC.	
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135" X 3.5" MIN) NAILS AT 4" OC.	

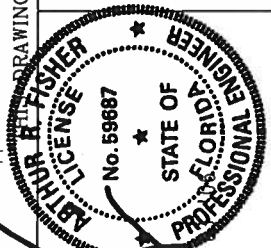
* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

MAX LOADING	REF	PIGGYBACK
55 PSF AT 1.33 DUR. FAC.	DATE	04/14/05
50 PSF AT 1.25 DUR. FAC.	DRWG	PIGGYBACK0405
47 PSF AT 1.15 DUR. FAC.	-ENG	DLJ/KAR



COLUMBIA COUNTY OFFICE OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-3S-16-02275-130

Building permit No. 000024644

Use Classification SFD, UTILITY

Fire: 50.22

Permit Holder SPARKS CONSTRUCTION/JOSH SPARK

Waste: 150.75

Owner of Building DAVID & JERELLE HAUCK

Total: 200.97

Location: 188 NW AMBLESIDE DR(COBBLESTONE, LOT 30)

Date: 01/22/2007



A handwritten signature in black ink, appearing to read "L. J. ...", is written over a horizontal line.

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)