

AB ACT	ANCHOR BOLT	. 1 3 1	
	ACTUAL	JST JT	JOIST JOINT
ADDN	ADDITIONAL	k	KIP
ADH ADJ	ADHESIVE ADJACENT	L-	ANGLE
ADJ AFF	ABOVE FINISHED FLOOR	L— LB	POUND
AHU	AIR HANDLING UNIT	LG	LONG
ALUM ALT	ALUMINUM ALTERNATE	LL LLH	LIVE LOAD LONG LEG HOIZONTAL
ANCH	ANCHORAGE, ANCHOR	LLV	LONG LEG VEITICAL
APPD ARCH	APPROVED ARCHITECT,(URAL)	LONG LP	LONGITUDINAL LOW POINT
		LSL	LONG SLOTTEI HOLES
B- B/	CONC. BEAM BOTTOM OF	LT	LIGHT LIGHTWEIGHT
В/В	BACK TO BACK	LTWT	
BIT BLDG	BITUMINOUS, BITUMASTIC BUILDING	MAS MAX	MASONRY MAXIMUM
ВМ	BEAM	MB	MACHINE BOL
BOT BRDG	BOTTOM BRIDGING	MECH MET	MECHANICAL METAL
BRG	BEARING	MEZZ	MEZZANINE
BLK BTWN	BLOCK BETWEEN	MFR MID	MANUFACTURR MIDDLE
BSMT	BASEMENT	MIN	MINIMUM
С	CHANNEL	MISC MJ	MISCELLANEOIS MASONRY JOIT
CANT	CANTILEVER	MO	
CEN	CENTER, ED CENTER TO CENTER	N	
CHAM	CENTER TO CENTER CHAMFER	NIC	
CIRC	CIRCULAR	NO	NUMBER
	CONTROL JOINT CLEAR	NS NTS	
CMU	CONCRETE MASONRY UNITS		
COL	COLUMN		OUT TO OUT ON CENTER
CONN	CONNECTION	OD	OUTSIDE DIAMTER
CONST	CONSTRUCTION	OF OPNG	
CONTR	CONTRACTOR	OPP	OPPOSITE
COORD	COORDINATE, TION	OSL OVS	
	DEPTH, DISTANCE	PAF PL	
DBA DBL		PLF	POUNDS PER INEAL FOOT
DBS	DOWEL BAR SPLICER	PROJ PSF	
DET DIA	DETAIL DIAMETER	PSI	POUNDS PER SQUARE INCH
DIAG	DIAGONAL	PT	PRESSURE TRATED
	DIMENSION DEAD LOAD	PVC	POLYVINYL CHORIDE
DN	DOWN	R/W RAD	REINFORCED VTH
DSGN DWGS	DESIGN DRAWINGS	REF	REFERENCE
DWL	DOWEL	REINF	REINFORCE, MNT
E	EAST	REQ	REQUIRE, D, MENT
EA	EACH	RET REV	RETURN REVISION
EB EE	EXPANSION BOLT EACH END	RP	RADIUS POINT
EF	EACH FACE	RT RTU	RIGHT ROOF TOP UNT
EJ EL	EXPANSION JOINT ELEVATION, ELEVATED		
ELEV	ELEVATOR	S SA	SOUTH SLAB BOLSTE
ENGR EQ	ENGINEER EQUAL	SB	SLEEVE ANCHIR
EQUIP	EQUIPMENT	SCHED SECT	
ES	EACH SIDE	SF-	STEP FOOTING
EW EXP		SIM	
EXIST	EXISTING	SPEC SP	SPECIFICATIONS SPACING,ES
EXT	EXTERIOR	SQ	SQUARE
F-	FOOTING MARK	SSL SS	
FC FF	FILLED CELL FINISHED FLOOR	STD	STANDARD
FIN	FINISH	STIFF STL	
FLG	FLANGE	STRL	
FLR FDN	FLOOR FOUNDATION	SYMM	SYMMETRICAL
FRMG	FRAMING	T	TIES
FS FT	FARSIDE FOOT, FEET	t	THICKNESS
FTG	FOOTING	T/ TB-	TOP OF TIE BEAM
FV	FIELD VERIFY	TC	TIE COLUMN
GA	GAUGE	T&B TEMP	TOP AND BOTOM TEMPORARY, EMPERATURE
GALV	GALVANIZED	TERM	TERMINATE
GL	GLU LAM	TOL TRAN	TOLERANCE TRANSVERSE
HC HK	HIGH CHAIR HOOK	TS	TUBE STEEL
HK HORIZ	HORIZONTAL	TYP	TYPICAL
HP	HIGH POINT	UNO	UNLESS NOTE OTHERWISE
HSA HSB		U/S VERT	UNDERSIDE
HT	HEIGHT	VERT	VERTICAL
ID	INSIDE DIAMETER	W W/	WIDTH, WEST, MIDE FLANGE WITH
F	INSIDE FACE	W/O	WITHOUT
N NCL	INCH INCLUDE, ING	WF-	WALL FOOTING
NSUL	INSULATE, D, ION	WP WS	WORKING POIN WATERSTOP
NT	INTERIOR	11.00	

## STANDARDS

ACI AMERICAN CONCRETE INSTITUTE

AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION

AITC AMERICAN INSTITUTE OF TIMBER CONSTRUCTION

ANSI AMERICAN NATIONAL STANDARDS INSTITUTE

ASTM AMERICAN SOCIETY OF TESTING AND MATERIALS

AWS AMERICAN WELDING SOCIETY

CRSI CONCRETE REINFORCING STEEL INSTITUTE

NFPA NATIONAL FOREST PRODUCTS ASSOCIATION

PCI PRESTRESSED CONCRETE INSTITUTE

POST TENSIONING INSTITUTE

UNDERWRITER'S LABORATORY

# SYMBOLS

&: AND

AT

CENTERLINE

DIAMETER

ELEVATION

NUMBER, POUNDS

CONSTRUCTION JT.

SAW CUT JOINT

PL PLATE

## STRUCTURAL DESIGN CRITERIA

D-1 CODE - FLORIDA BUILDING CODE 2004 EDITION.

D-2 DESIGN LIVE LOADS:

ROOF 20 PSF -

D-3 DESIGN WIND SPEED:

IMPORTANCE FACTOR:

BUILDING CATEGORY:

EXPOSURE:

INTERNAL PRESURE COEFICIENT:

COMPONENTS AND CLADDING PRESSURE:

(SEE SHT. S-0.01)

D-4 SEISMIC: ZONE O

D-5 ALLOWABLE SOIL BEARING PRESSURE 3000 PSF PER REPORT BY "CAL-TECH TESTING", DATED APRIL 26, 2006

## GENERAL NOTES

- G-1 THE GENERAL CONTRACTOR SHALL REVIEW ALL PROJECT DOCUMETNTS, PRIOR TO FABRICATION AND START OF CONSTRUCTION. REPORT ANY DISCREPANCIES TO ARCHITECT OR STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH WORK.
- THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY
  TO PROTECT THE STRUCTURE AND WORKERS DURING CONSTRUCTION,
  INCLUDING ADEQUATE BRACING OF STRUCTURAL MEMBERS, WALLS AND
  OTHER NON-STRUCTURAL ELEMENTS TO WITHSTAND CONSTRUCTION,
  LOADS INCLUDING WIND.
- G-3 GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM ALL DAMAGE DURING CONSTRUCTION.
- G-4 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISEE REDUCED IN SIZE OR STRENGTH WITHOUT PRIOR APPROVAL IN WRITING FROM THE STRUCTURAL ENGINEER.
- G-5 THE GENERAL CONTRACTOR SHALL COORDINATE STRUCTURAL AND) OTHER DRAWINGS WHICH ARE PART OF THE CONTRACT DOCUMENTS FOR ANCHORED, EMBEDDED OR SUPPORTED ITEMS WHICH MAY AFFECT. THE STRUCTURAL DRAWINGS.
- G-6 ALL DETAILS AND SECTIONS ON THE DRAWINGS ARE INTENDED ITO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR? SITUATION ELSEWHERE ON THE PROJECT EXCEPT WHERE A SEPAKRATE DETAIL IS SHOWN.
- THE INTENTION OF THE PLANS AND SPECIFICATIONS IS TO PRODUIDE ALL NECESSARY DETAILS TO CONSTRUCT A COMPLETE STRUCTURGE, WHEN SPECIFIC INFORMATION IS MISSING OR IS IN CONFLICT;, THE CONTRACTOR SHALL USE A SIMILAR DETAIL AND/OR THE MORE (COSTLY ITEM OF CONFLICT. THE CONFLICTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER.
- G-8 REPRODUCTION OF THE CONTRACT DOCUMENTS USED FOR THE PREPARATION AND SUBMITTAL OF SHOP DRAWINGS WILL NOT BE: PERMITTED. REPRODUCED DRAWINGS WILL BE RETURNED UNCHECKED FOR RESUBMITTAL.
- THE GENERAL CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS, COMPLETENESS FAND SHALL ANSWER ALL CONTRACTOR RELATED QUESTIONS. GENERALL CONTRACTOR SHALL STAMP AND INITIAL ALL SHEETS PRIOR TO SUBMITTING SHOP DRAWINGS TO ARCHITECT/ENGINEER FOR REVIIEW. NON-COMPLIANCE WITH THIS REQUIREMENT WILL RESULT IN REJECTION OF SUBMITTAL.
- G-10 ALL SHOP DRAWINGS SHALL CONSIST OF 4 BLUE LINES, ALL ONTHER SETS OF BLUE LINES WILL BE DISCARDED.

## SHALLOW FOUNDATIONS

- SF-1 SOIL TO BE STRIPPED, COMPACTED AND TESTED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE SOILS ENGINEER AND PROJECT SPECIFICATIONS.
- SF-2 CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, UNLESS OTHERWISE SHOWN ON PLANS. MAXIMUM MISPLACEMENT OR ECCENTRICITY 2". TOLERANCE FOR MISLOCATION OF COLUMN DOWELS OR ANCHOR BOLTS TO BE PER ACI OR AISC STANDARDS.
- SF-3 HORIZONTAL JOINTS IN FOOTINGS WILL NOT BE PERMITTED.
- SF-4 WHERE VERTICAL CONSTRUCTION JOINTS OCCUR IN CONTINUOUS FOOTINGS, PROVIDE A MINIMUM CONTINUOUS 2x4 KEYWAY ACROSS JOINT FOR EACH 12" OF DEPTH.
- SF-5 CONTRACTOR TO NOTIFY ARCHITECT/ENGINEER IF SOIL CONDITIONS ARE UNCOVERED THAT PREVENT THE REQUIRED SOIL BEARING PRESSURE FROM BEING OBTAINED.
- SF-6 COORDINATE PLUMBING LINE WITH FOOTING LOCATIONS FOR INTERFERENCE. INDIVIDUAL FOOTINGS SHOULD BE LOWERED WITH THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. CONTINUOUS WALL FOOTINGS SHOULD BE STEPPED AS DETAILED ON THE DRAWINGS.
- SF-7 EXCAVATING UNDER OR NEAR IN PLACE FOOTINGS/FOUNDATIONS WHICH DISTURBS THE COMPACTED SOIL BENEATH THE FOOTINGS/FOUNDATIONS WILL NOT BE PERMITTED.
- SF-8 REINFORCING SHALL BE SUPPORTED ON PRECAST PADS. DOWELS FOR COLUMNS AND FILLED CELLS SHALL BE SECURED IN PLACE PRIOR TO POURING CONCRETE. USE TEMPLATES FOR SETTING COLUMN DOWELS AND ANCHOR BOLTS.

## REINFORCED CONCRETE

- RC-1 ALL CONCRETE DESIGN AND PLACEMENT SHALL BE IN STRICT ACCORDANCE WITH THE ACI "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318.
- RC-2 STRUCTURAL CONCRETE SHALL CONFORM TO ACI 301 SPECIFICATIONS AND SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.
- RC-3 USE REGULAR WEIGHT CONCRETE.
- RC-4 STRUCTURAL CONCRETE SHALL CONFORM TO ACI 301 AND HAVE THE FOLLOWING SLUMPS & AGGREGATE REQUIREMENTS:

LOCATION	SLUMP	MAX. AGGREGATE
FOOTINGS	4" ± 1"	ASTM #57
COLUMNS	5" ± 1"	ASTM #57
BEAMS AND SLABS	5" ± 1"	ASTM #57
FILLED CELLS	8" ± 1"	ASTM #89 PEAROCK

FLYASH MAYBE SUBSTITUTED FOR CEMENT AT A MAXIMUM OF 20% (PERCENT) OF THE TOTAL CEMENT CONTENT. DO NOT USE FLYASH IN FILLED CELL CONCRETE.

- RC-5 THE USE OF JITTERBUGS TO CONSOLIDATE CONCRETE WILL NOT BE PERMITTED.
- RC-6 ALL PUMPED CONCRETE WITH #57 AGGREGATE IS TO CONTAIN A HIGH RANGE WATER REDUCING AGENT. MINIMUM SIZE OF DISCHARGE TO BE 4" I.D. SEE SPECIFICATIONS FOR REQUIRED MIX DESIGN. A 2" I.D. DISCHARGE MAY BE USED WITH #89 AGGREGATE.
- RC-7 ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND INSTALLED IN ACCORDANCE WITH ACI 318 AND ACI DETAILING MANUAL, ACI-315 LATEST EDITION.
- RC-8 REINFORCING STEEL SHALL BE NEW DEFORMED BARS, FREE FROM RUST, SCALE AND DIL, CONFORMING TO ASTM A-615, GRADE 60, WITH MINIMUM YIELD STRENGTH = 60,000 PSI.
- RC-9 ALL SLABS ON GRADE SHALL BE REINFORCED WITH POLYPROPYLENE FIBERS IN ACCORDANCE WITH MANUFACTURER'S WRITTEN RECOMMENDATIONS AND SPECIFICATIONS.
- RC-10 PROVIDE THE FOLLOWING CONCRETE COVERAGES OVER REINFORCING

FOOTINGS BOTTOMS & UNFORMED EDGES 3" CLR

BEAMS & COLS. TIES 1-1/2" CLR

- RC-11 SHOP DRAWINGS FOR PLACEMENT SHALL BE SUBMITTED FOR REVIEW PRIOR TO REBAR FABRICATION.
- RC-12 NO REINFORCING BARS SHALL BE CUT TO ACCOMODATE THE INSTALLATION OF ANCHORS, EMBEDS OR OTHER ITEMS.
- RC-13 USE THE STRUCTURAL DRAWINGS(INCLUDING REVISIONS AND ADDENDA) IN CONJUNCTION WITH REVIEWED SHOP DRAWINGS FOR PLACEMENT OF REINFORCING.
- RC-14 AT CHANGES IN DIRECTION OF BLOCK BEAMS & STRIP FOOTINGS, PROVIDE CORNER BARS OF SAME SIZE AND QUANTITY(U. N. O. ) AS HORIZONTAL STEEL. REFER TO TYPICAL DETAIL.
- RC-15 ALL EMBEDDED ITEMS SHALL BE SECURELY TIED IN PLACE PRIOR TO CONCRETE PLACEMENT.
- RC-16 EXPANSION BOLTS SHALL BE HILTI KWIK BOLTS OR APPROVED EQUIVALENT, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

#### MASONRY

- M-1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI/ASCE 530) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ASCE 530) EXCEPT AS AMENDED BELOW.
- M-2 STRUCTURE HAS BEEN DESIGNED AS A BEARING WALL STRUCTURE
- M-3 USE TYPE "S" MORTAR WITH MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI.
- M-4 MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI ON NET SECTION, TO PROVIDE NET AREA COMPRESSIVE STRENGTH OF MASONRY (F'm) OF 1500 PSI.
- M-5 PROVIDE FILLED CELLS AS SHOWN ON PLANS. IN ADDITION, PROVIDE FILLED CELLS ADJACENT TO ALL OPENINGS AND AT ANCHORAGE OF CONNECTIONS.
- M-6 PROVIDE FULL MORTAR BEDDING AROUND ALL FILLED CELLS WITH VERTICAL REINFORCING.
- M-7 REINFORCING FOR FILLED CELLS SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE THE FOLLOWING LAP SPLICES FOR REINFORCING:

#5 BARS 38

- M-8 ALL FILLED CELLS AND COLUMNS SHALL BE POURED AT LEAST TWO(2) HOURS PRIOR TO POURING BEAMS AND LINTELS.
- -9 REINFORCE WALL WITH LADDER TYPE REINFORCEMENT (DUR-O-WALL"
  OR EQUAL) IN BED JOINTS AT 16" O.C. MEASURED VERTICALLY.
  LAP SPLICE ALL HORIZONTAL WALL REINFORCING 6". PROVIDE
  PREFABRICATED "TEE" OR CORNER SECTIONS AT ALL INTERSECTING
  WALLS.
- M-10 REFER TO TYPICAL WALL SECTIONS FOR MAXIMUM CONSTRUCTION
  HEIGHT OF MASONRY WALLS. PROVIDE CLEAN-OUT HOLES AT BASE OF
  FILLED CELL WHEN THE CONCRETE POUR EXCEEDS 5 FEET IN HEIGHT.
- M-11 CONCRETE FOR FILLED CELLS SHALL BE VIBRATED DURING PLACEMENT USING A "PENCIL" TYPE VIBRATOR.
- M-12 UNLESS NOTED OTHERWISE, PROVIDE THE FOLLOWING LINTELS OVER ALL MASONRY WALL OPENINGS:
  - 1. OPENING LESS THAN OR EQUAL TO 6'-O": 8" PRECAST CONCRETE
    "U" LINTEL R/W 2-#5 FILLED SOLID WITH CONCRETE.
    EXTEND REINFORCING 8" PAST OPENING FACE.
  - 2. OPENING GREATER THAN 6'-0" & LESS THAN 10'-0": 8" PRECAST CONC. "U" LINTEL & 8" KNOCK-OUT COURSE R/W 2-#5 EA. COURSE FILLED SOLID W/CONCRETE. EXTEND REINFORCING 8" PAST OPENING.

WOOD

- WD-1 ALL WOOD CONSTRUCTION AND CONNECTIONS SHALL CONFORM TO AITC "AMERICAN INSTITUTE OF TIMBER CONSTRUCTION" MANUAL, 4TH EDITION, AND THE "NATIONAL DESIGN SPECIFICATIONS" FOR WOOD CONSTRUCTION, 1991 EDITION, AND STANDARD BUILDING CODE, CHAPTER 17.
- WD-2 ALL MEMBER SIZES ARE TO BE SHOWN ON DRAWINGS AND PROVIDE THE FOLLOWING MINIMUM PROPERTIES:

MEMBER	SPECIES	<u>FbKSI</u>	E(KSI)
STSIOL	SOUTHERN PINE	1. 5	1,600
STUDS	SOUTHERN PINE	1. 5	1,600
DTHER	SOUTHERN PINE	1. 25	1,600

- WD-3 ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.
- WD-4 ALL BOLTS FOR BOLTED CONNECTIONS SHALL CONFORM TO A307. USE WASHERS BETWEEN WOOD AND ALL BOLT HEAD AND NUTS.
- WD-5 ALL METAL WOOD CONNECTORS SHALL BE AS MANUFACTURED BY SIMPSON STRONG TIE CO. OR APPROVED EQUAL AND SHALL BE GALVANIZED. THE NAILING PATTERN SPECIFIED IN THE LATEST SIMPSON CATALOG SHALL BE USED.
- WD-6 WHERE BEAMS OR TRUSSES ARE FORMED OF 2 OR MORE MEMBERS THEY SHALL BE FULL LENGTH AND FASTENED TOGETHER PER CHAPTER 17 OF THE STANDARD BUILDING CODE.
- WD-7 ROOF PLYWOOD SHALL BE 5/8" CDX WITH 10d NAILS @ 6" O.C. MAX AT ALL SUPPORTED EDGES. SPACE NAILS 12" O.C. ALONG INTERMEDIATE FRAMING MEMBERS. ROOF PLYWOOD SHALL HAVE STAGGERED END JOINTS.

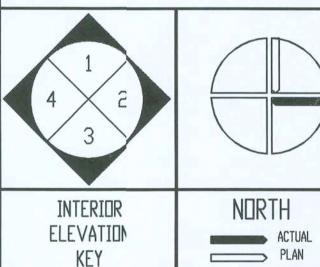


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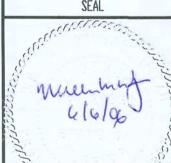
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STRUCTURAL
GENERAL
NOTES
SEAL ARCHITECT/CONSULTANT



MITZO
E NGINEERING, LLC
STRUCTURAL CONSULTANTS
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MATLAND, FLORIDA 32761
PHONE: (407) 644—1500
FAX: (407) 644—1562

SHEET NUMBER

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#### STRUCTURAL STEEL

- S-1 FABRICATION AND ERECTION IF STRUCTURAL STEEL SHALL COFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION", NINTH EDITION AND THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILLINGS", 1989 EDITION.
- S-2 ALL STRUCTURAL STEEL TO CONFORM TO ASTM A-572 WITH MILIMUM YIELD STRENGTH OF 50 KSI. MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A-36 WITH A MINIMUM YIELD STRENGTH OF 36 KSI.
- S-3 STRUCTURAL TUBES SHALL CONFORM TO ASTM A-441 WITH MINMUM YIELD STRNEGTH OF 46 KSI.
- S-4 ALL HIGH-STRENGTH BOLTS SHALL MEET THE REQUIREMENTS OF THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 IR A490 BOLTS".
- S-5 UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4" DIAMEER A325 AND SHALL BE BEARING TYPE CONNECTIONS.
- S-6 ALL BOLTS CAST IN CONCRETE SHALL CONFORM TO ASTM A-36 OR A-307.
- S-7 ALL SHOP AND FIELD WELDING SHALL BE DONE BY CURRENTLY CERTIFIED WELDERS IN ACCORDANCE WITH AWS D1. 1 "STRUCTURAL WELDING CODE" LATEST EDITION.
- S-8 USE E70XX ELECTRODES FOR ALL WELDING, UNLESS NOTED OTHERWISE. (GRIND SMOOTH ALL EXPOSED WELDS.)
- S-9 SUBMIT STRUCTURAL STEEL SHOP DRAWINGS FOR REVIEW PRIR TO FABRICATION. CLEARLY SHOW ALL PIECE MARKS, CONNECTIONS, AND ERECTION DRAWINGS. ANY SPLICES NOT SHOWN ON CONTRACT DRAWINGS ARE TO BE CLEARLY NOTED FOR APPROVAL.

## PRE-ENGINEERED WOOD TRUSSES

WT-1 DESIGN OF METAL CONNECTED ROOF TRUSSES TO COMPLY WITH

APPLICABLE BUILDING CODE (SEE NOTE D-1/SO, OO)

NFPA'S "NATIONAL DESIGN SPECIFICATION FOR STRSS GRADED LUMBER AND ITS FASTENINGS".

TRUSS PLATE INSTITUTE'S "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED FLOOR TRUSSES".

- WT-2 PRE-ENGINEERED PRE-FABRICATED WOOD TRUSSES AND THEIR CONNECTIONS TO EACH OTHER SHALL BE DESIGNED FOR THE LIADS INDICATED IN NOTE WT-8 BY A PROFESSIONAL ENGINEER REGSTERED IN THE STATE OF FLORIDA.
- WT-3 SIGNED AND SEALED SHOP DRAWINGS SHOWING TRUSS CONFIGURATION WITH MEMBER SIZES AND CONNECTIONS, TRUSS LAYOUT WITH 'IECE MARKS, REQUIRED TRUSS TO TRUSS CONNECTIONS, DESIGN LODS, DURATION FACTORS AND ERECTION DETAILS MUST BE SUBMITTD FOR REVIEW PRIOR TO FABRICATION. IF REQ., SUBMIT COPIES TO THE BUILDING DEPARTMENT AT TIME OF PERMITTING.
- WT-4 PRE-FABRICATED WOOD TRUSSES SHALL BE FABRICATED FROM SOUTHERN PINE(SPIB) KILN DRIED #2 OR BETTER FOR CHORD AND #3 GRADE OR BETTER FOR WEBS.
- WT-5 NO WANE, SKIPS OR OTHER DEFECTS SHALL OCCUR IN THE PLATE CONTACT AREA OR SCARFED AREA OF WEB MEMBERS. PLATES SHALL BE CONNECTED WITH ONE REQUIRED EACH SIDE OF TRUSS.
- WT-6 NUMBER OF PANELS AND DIRECTION OF WEB MEMBERS TO SUIT CONTINUOUS OR SIMPLE SPAN TRUSS DESIGN REQUIREMENTS. SEE STRUCTURAL AND ARCHITECTURAL DRAWINGS FOR DUTLINE SHAYE AND ANY SPECIAL CONDITIONS/LOCATIONS OF PANEL POINTS.
- WT-7 PERMANENT TRUSS BRACING OR BRIDGING MEMBERS SHALL BE 'x4 MINIMUM SOUTHERN PINE WITH MINIMUM LOCATIONS AS NOTEDIN WT-10. ADDITIONAL BRACING REQUIRED TO STRENGTHEN TRUS COMPONENTS SHOULD BE NOTED ON THE ERECTION DRAWINGS IN ACCORDANCE WITH TRUSS MANUFACTURER'S RECOMMENDATIONS.
- WT-8 TRUSS DESIGN LOADS AS FOLLOWS:

SLOPED TOP CHORD TRUSSES, (COORD, W/ARCH'L, : 12

24 PS TOP CHORD DEAD LOAD LIVE LOAD 16 PS 10 PS BOTTOM CHORD DEAD LOAD TOTAL LOAD-DURATION FACTOR 1, 25

MECHANICAL UNITS-SEE PLANS FOR LOCATION AND LIADS

FABRICATOR TO DESIGN TRUSSES AND SUPPLY ADDITONAL BRIDGING AS REQUIRED TO RESIST NET UPLIFT FORES AS SHOWN ON WIND PRESSURE DIAGRAM

PROVIDE DESIGN CALCULATIONS, SIGNED AND BEARING THE EMBOSSED SEAL OF AN ENGINEER REGISTERED IN TH STATE OF FLORIDA, VERIFYING DESIGN FOR UPLIFT STRESES.

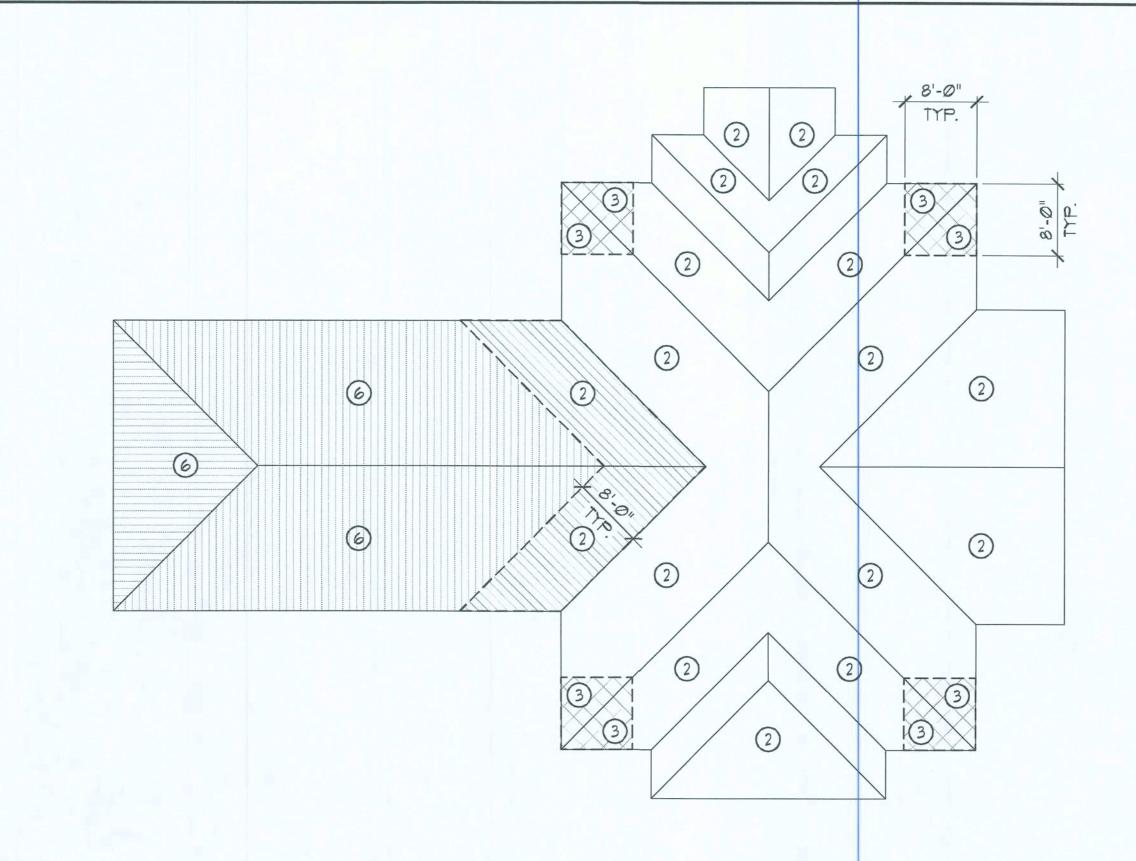
- WT-9 HANDLING, ERECTION AND BRACING OF WOOD TRUSSES SHALL BE IN ACCURDANCE WITH "HANDLING AND ERECTING WOOD TRUSSES" (HET-80) AND "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS" BY THE TRUSS PLATE INSTITUTE, LATEST EDITIONS.
- WT-10 MINIMUM PERMANENT BRIDGING CRITERIA FOR PRE-ENGINEERED TRUSSES:

PROVIDE 2x4 CONTINUOUS HORIZONTAL BRIDGING AT TOP AND BOTTOM CHORDS AND INTERMITTENT CROSS BRIDGING AT 10' SPACING AT THE FOLLOWING MINIMUM LOCATIONS

- A. AT RIDGES
- B. AT 10'-10" D. C. HORIZONTALLY-AT PANEL POINT LOCATIONS; C. AT ALL DEEP(>18") BEARING ENDS

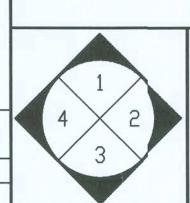
PROVIDE CONTINUOUS 2x4 @ 48" D.C. PERPENDICULAR TO TRUSSES AT TOP CHORD WHERE ROOF PLYWOOD IS NOT RIGIDLY ATTACHED TO TOP CHORD OF TRUSS

PROVIDE CONTINUOUS 2x4 @ 48" O.C. AT BOTTOM CHORD WHERE A RIGID CEILING IS NOT FIRMLY ATTACHED TO THE BOTTOM CHORD, 2



NET ROOF UPLIFT							
TRIBUTARY AREA (ft <sup>2</sup> )	INTERIOR ZONE (psf)	EDGE ZONE (psf)	CORNER ZONE () (psf)	CANOPY ZONE (psf)			
10	-2Ø	-42.1	-42.1	-40.6			
2Ø	-19.4	-38.2	-382	-40.6			
50	-18.6	-33	-33	-40.6			
≥100	-18.1	-29.1	-29.1	-40.6			

NET	OVERHANG UPLIFT				
	ITARY A (ft²)	EDGE ZONE (psf)	CORNER ZONE (3) (psf)		
1	0	-40.6	-68.3		
2	Ø	-40.6	-605		
5	0	-40.6	-502		
≥10	00	-40.6	-42.4		



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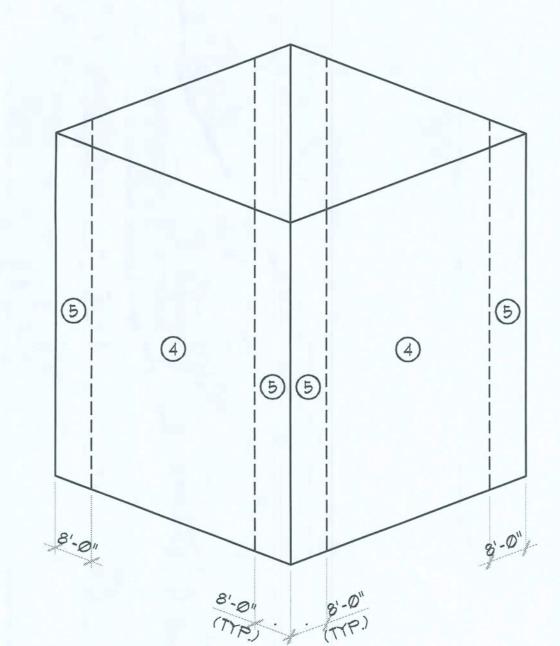
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ARCHITECT/CONSULTANT SEAL ENGINEERING, LLC STRUCTURAL CONSULTANTS

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ROOF WIND PRESSURE DIAGRAM

3/32" = 1'-0"

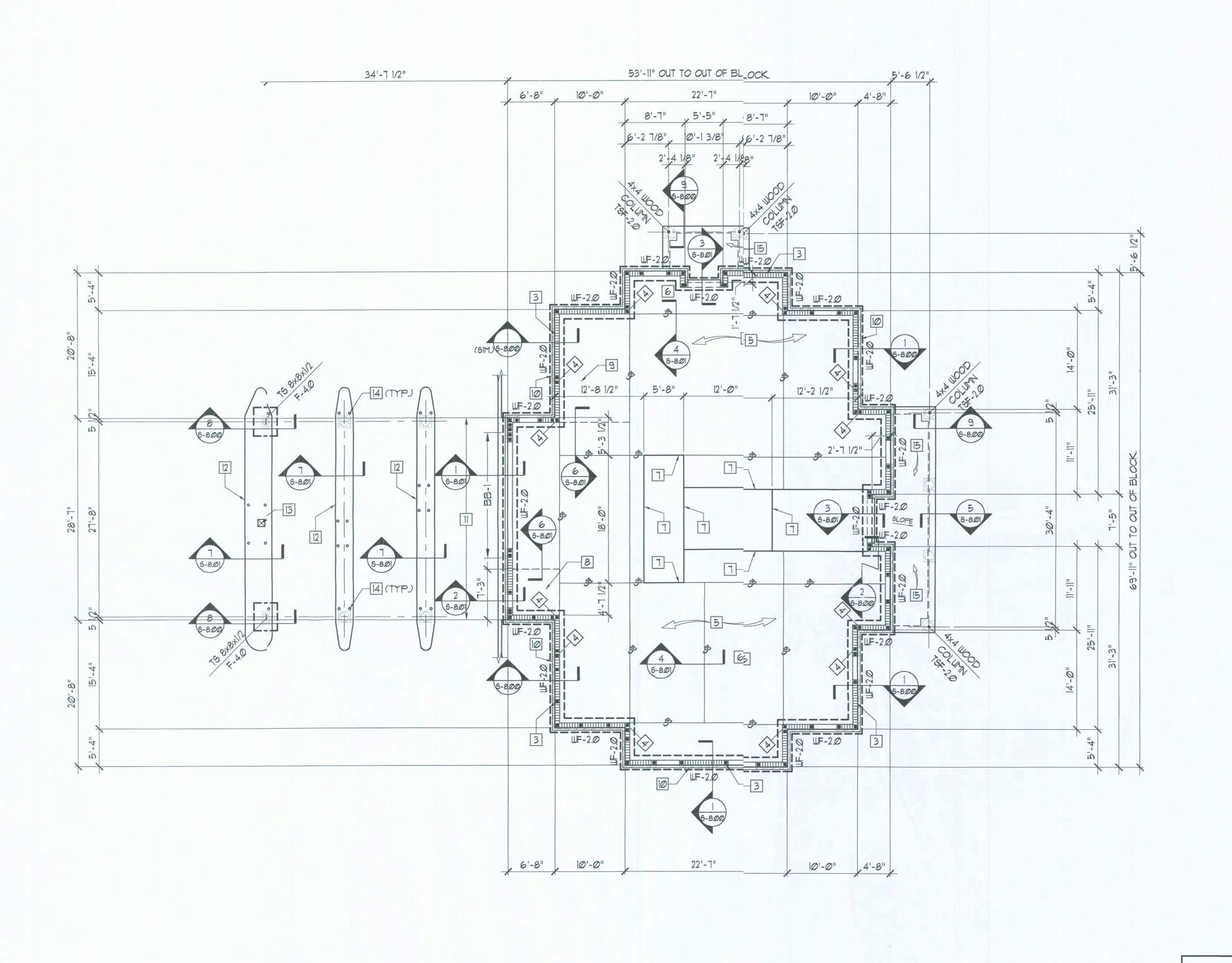


TRIBUTARY AREA (ft <sup>2</sup> )	INTERIOR ZONE (4) (psf		sf)	CORNER ZONE (5) (psf)	
7,1,42,1,1,1,7	POSITIVE	NEC	:ATIVE	POSITIVE	NEGATIVE
10	22	-	23.6	22	-29.1
20	21	-	22.6	21	-27.2
50	20		21.3	20	-24.6
> 100	19	-	20.4	19	-22.6

WALL PRESSURE (PSF)

COMPONENTS AND CLADDING

WALL PRESSURE / COMPONENTS AND CLADDING DIAGRAM NOT TO SCALE



- 2 PROVIDE CORNER BARS WHERE ALL FOOTINGS CHANGE DIRECTION AND AT FOOTING INTERSECTIONS. SEE SECTION 6/5-8.00 FOR FURTHER INFORMATION.
- W/(1)-#5 (VERT.) BAR AND MATCHING DOWEL AT FOOTING, IN SOLID GROUT FILLED CELLS AT ALL CORNERS, INTERSECTIONS, AND ADJACENT MASONRY OPENINGS (COORD. SIZE, LOCATIONS AND ELEVATIONS THERE OF WITH ARCH'L. DRAWINGS), ENDS OF WALLS AND BETWEEN AT 48" O.C. (MAX.) SEE PLAN FOR ADDITIONAL LOCATIONS. EXTEND VERTICAL REINFIG. BARS THRU TO UPPER MOST MASONRY BOND BEAM (SPLICE / LAP AS REQUIRED) - SEE SHEET 5-3.00) AND TERMINATE BARS WITH STANDARD 90° HOOK. (SEE SEC. 2/5-8,02 FOR FURTHER INFORMATION)
- BAR AND MATCHING DOWEL AT FOOTING, IN SOLID GROUT FILLED CELLS, (TYP. EACH CELL) LOCATE SUCH THAT DOUBLE FILLED CELLS ALIGN WITH PRE-ENGINEERED WOOD GIRDER TRUSS ABOVE (SEE SHEET S-3,00 AND COORDINATE EXACT LOCATIONS WITH PRE-ENGINEERED WOOD TRUSS MANUFACTURER / SUPPLIER'S APPROVED SHOP DRAWINGS)
- 5 SLAB ON GRADE CONSTRUCTION: 4" POLYPROPYLENE FIBER REINFORCED CONCRETE SLAB ON GRADE ON VAPOR BARRIER (SEE ARCH'L. DRAWINGS) OVER COMPACTED SUBGRADE. COORD. ANY AND ALL SLAB SLOPES, DEPRESSIONS AND LIMITS THERE OF WITH ARCH'L. DRAWINGS. (FOR ACTUAL TOP OF SLAB ELEVATIONS SEE ARCH'L. AND / OR CIVIL DRAWINGS)
- 6 SLAB CONTROL JOINTS (SEE PLAN FOR SPACING) AND CONTROL JOINT DETAIL 4/6-8.01 FOR FURTHER INFORMATION AND COORD. W/ARCH'L. DRAWINGS FOR ADDITIONAL LOCATIONS.
- 1 PROVIDE 1/4" (DEEP) SLAB CUT / JOINTS.
- 8 8" CONCRETE SLAB (SEE SEC. 6/5-8.0) FOR REINFORCING) UNDER "ATM" MACHINE. GENERAL CONTRACTOR TO COORDINATE EXTENT / LIMITS THERE OF WITH ARCH'L. DRAWINGS.
- "SAFE KEEPING ROOM". GENERAL CONTRACTOR TO COORDINATE EXTENT / LIMITS THERE OF WITH ARCH'L. DRAWINGS.
- EXTERIOR VENEER: COORDINATE LIMITS / EXTENTS WITH ARCH'L.
- 11 PROVIDE 12"(WIDE)x1'-4"(DEEP MIN.) CONCRETE CURB. SEE SEC. 1/5-8.01 FOR FURTHER INFORMATION. COORD. EXTENT / LIMITS OF CURB WITH ARCH'L. AND / OR CIVIL DRAWINGS.
- 12 GENERAL CONTRACTOR TO COORDINATE EXTENT / LIMITS OF
- 13 CUTOUT FOR CONDUIT IN DRIVE THRU ISLAND, COORDINATE EXACT SIZE AND LOCATION WITH ARCH'L. DRAWINGS.
- 14 BOLLARD: SEE ARCH'L. DRAWINGS FOR TYPICAL INFORMATION AND LOCATION. (TYP.)
- 15 EXTERIOR SLAB ON GRADE: 4" POLYPROPYLENE FIBER REINFORCED CONCRETE SLAB ON GRADE ON VAPOR BARRIER (SEE ARCH'L. DRAWINGS) OVER COMPACTED SUBGRADE. COORD. LIMITS / EXTENTS THERE OF, AND TOP OF SLAB SLOPES WITH ARCH'L. DRAWINGS. (FOR ACTUAL TOP OF SLAB ELEVATIONS SEE ARCH'L. AND / OR CIVIL DRAWINGS)

FOOTING SCHEDULE

BOTTOM

(5)-#5's EACH WAY

(3)-#5's EACH WAY

(3)-5's CONT.

#5's @ 24"O.C. TRAVERSE

REINFORCEMENT REINFORCEMENT

SIZE

WIDTH x LENGTH x DEPTH

4'-0"x4'-0"x12"

2'-Ø"x2'-Ø"x12"

2'-Ø"xCONT.x12"

MARK

F-40

TSF-2.0

WF-2.0

1	STEP FOOTINGS	(AS REQUIRED) SEE DETAIL 1/5-8.00 FOR
	ADDITIONAL INF	ORMATION. GENERAL CONTRACTOR TO
	COORDINATE 5	EP FOOTING LOCATIONS.

3 IIIIIII INDICATES 8" MASONRY BEARING WALLS REINFORCED

4 PROVIDE DOUBLE FILLED CELLS REINFORCED WITH (1)-#6 (VERT.)

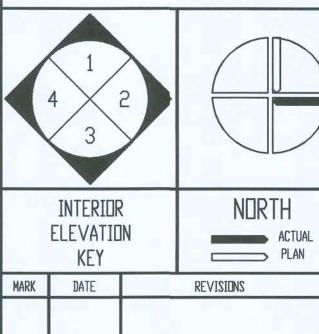
9 8" CONCRETE SLAB (SEE SEC. 6/5-8.0) FOR REINFORCING) UNDER

- DRIVE THRU ISLANDS WITH ARCH'L. AND / OR CIVIL DRAWINGS.

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GENERAL NOTES



06003.00 DRAWN: 02/15/06 CHECKED: W.M.M. SCALE: AS NOTED FILE: 2006075111

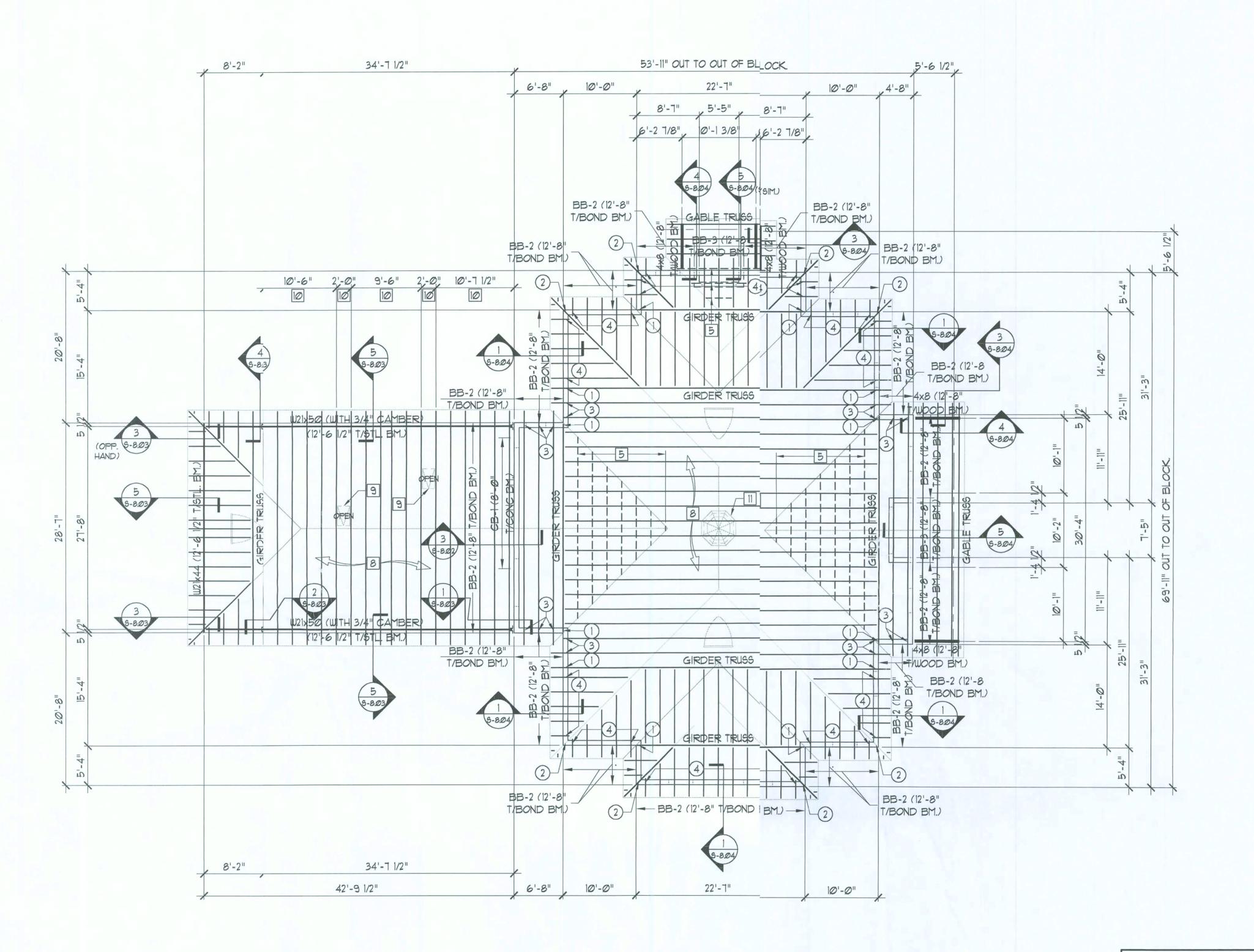
FOUNDATION AND SLAB ON GRADE PLAN

Mulen must elelou

ARCHITECT/CONSULTANT INGINEERING, LLC STRUCTURAL CONSULTANTS
670 NORTH ORLANDO AVE., SUITE 103A
MATILAND, FLORIDA 32751
PHONE: (407) 644-1500
FAX: (407) 644-1562 SHEET NUMBER WILLIAM IN: MITZO PROFESSIONAL ENGINEED FLORIDA REGISTRATION NUMBER: 2169

FOUNDATION AND SLAB ON GRADE FOLAN 1/8"=1'-0" 0'-0" (REF.) T/SLAB (TYPP, UN.O.)

-1'-4" (REF.)) TIFTG (TYP., UN.O.)



ROOF FRAMING PLAN

1/8"=1'-0"

1 PROVIDE MASONRY LINTEL OVER ALL OPENINGS IN MASONRY WALLS (COORDINATE EXACT SIZE, LOCATIONS AND ELEVATIONS WITH ARCH'L. DRAWINGS) SEE GENERAL NOTES FOR LINTEL TYPE SIZES AND REINFORCING. PROVIDE TEMPORARY SHORING DURING CONSTRUCTION IF LINTEL SPAN IS GREATER THAN 6'-0" (SIX FEET.

PROVIDE CORNER BARS WHERE ALL BOND BEAMS (BB- ) CHANGE DIRECTION AND AT ALL BOND BEAM (BB-INTERSECTIONS. SEE SEC. 6/5-8.00 (SIM.) FOR FURTHER INFORMATION.

3 ALL PRE-ENGINEERED WOOD TRUSSES AND MISCELLANEOUS 2x FRAMING SPACED AT 24"O.C. (MAX.) COORD. EXACT LOCATIONS WITH PRE-ENGINEERED WOOD MANUFACTURER / SUPPLIER'S APPROVED SHOP DRAWINGS.

- 4 PROVIDE 2x (TALL) WOOD BLOCKING BETWEEN EACH TRUSS WHERE PLYWOOD ROOF SHEATHING CHANGES DIRECTION (RIDGE LINES, ROOF EDGE, HIPS AND VALLIES, ETC.)
- 5 OVER BUILT FRAMING: PRE-ENGINEERED WOOD TRUSSES OR 2x FRAMING SPACED AT 24"O.C. (MAX.) COORD. EXACT LOCATIONS WITH PRE-ENGINEERED WOOD MANUFACTURER / SUPPLIER'S APPROVED SHOP DRAWINGS.
- 6 SEE GENERAL NOTES: (SHT. S-0.01) FOR CONNECTION INFORMATION OF PRE-ENGINEERED WOOD TRUSSES, GIRDER TRUSSES AND MISCELLANEOUS 2x FRAMING, ETC.
- 7 PROVIDE HEAVY GIRDER TIEDOWN: HGT-2 (FOR 2 PLY) (IF ONE PLY - PROVIDE 3/4" (WOOD) SHIM ON EACH SIDE) OR HGT-3 (FOR 3 PLY) WITH ANCHOR BOLTS AS RECOMMENDED BY SIMPSON COMP. AT EACH PRE-ENGINEERED WOOD GIRDER TRUSS (OR AN APPROVED EQUAL SUBSTITUTE) COORDINATE SIZE (NUMBER OF PLY'S) WITH PRE-ENGINEERED WOOD TRUSS MANUFACTURER / SUPPLIER'S APPROVED SHOP DRAWINGS.
- 8 ROOF CONSTRUCTION: (ROOFING FINISH SEE ARCH'L. DRAWINGS) OVER 5/8" CDX EXTERIOR GRADE PLYWOOD SHEATHING NAILED WITH 10d NAILS AT 6"O.C. (MAX.) AT SUPPORTED EDGES, AND 10d NAILS AT 12"O.C. (MAX.) AT INTEREDIATE SUPPORTS (FIELD). PROVIDE ONE PLYWOOD CLIP PER SPAN BETWEEN SHEET EDGES ON PRE-ENGINEERED WOOD TRUSSES.
- 9 ACCESS HATCH COORDINATE EXACT SIZE AND LOCATION WITH ARCH'L. DRAWINGS.
- 10 HOLD TRUSS LOCATION DIMENSIONS (AS SHOWN) FOR LOCATION OF ACCESS HATCH.
- "COPULA" SEE ARCH'L. DRAWINGS FOR SIZE AND LOCATION. (SEE SEC. 6/5-8.03 FOR ANCHOR / ATTACHMENT INFORMATION)

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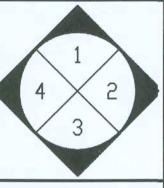
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GENERAL NOTES



NORTH INTERIOR **ELEVATION** ACTUAL

PLAN MARK DATE REVISIONS

	COMM. NO.	06003.00	DRAWN:	S.L.
	DATE:	02/15/06	CHECKED	W.M.W
	SCALE:	AS NOTED		
	FILE:	2006075-34		

ROOF FRAMING PLAN ARCHITECT/CONSULTANT

SEAL weenment celelo6

WILLIAM IT MITZO PROFESSIONAL ENGINEEF FLORIDA REGISTRATION NUMBER: 2169:

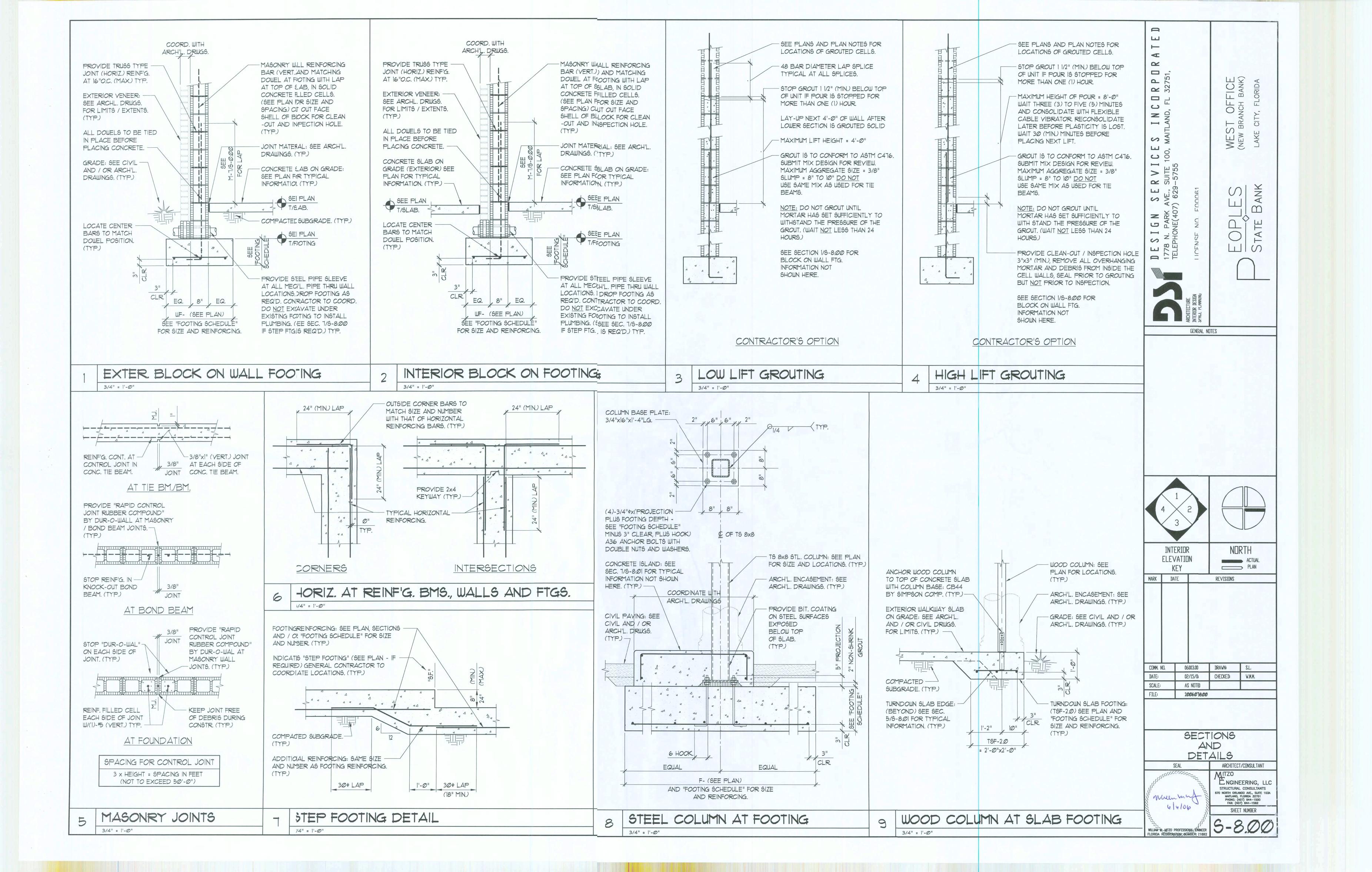
L'NGINEERING, LLC STRUCTURAL CONSULTANTS 670 NORTH ORLANDO AVE., SUITE 103A MATLAND, FLORIDA 32751 PHONE: (407) 644-1500 FAX: (407) 644-1562 SHEET NUMBER

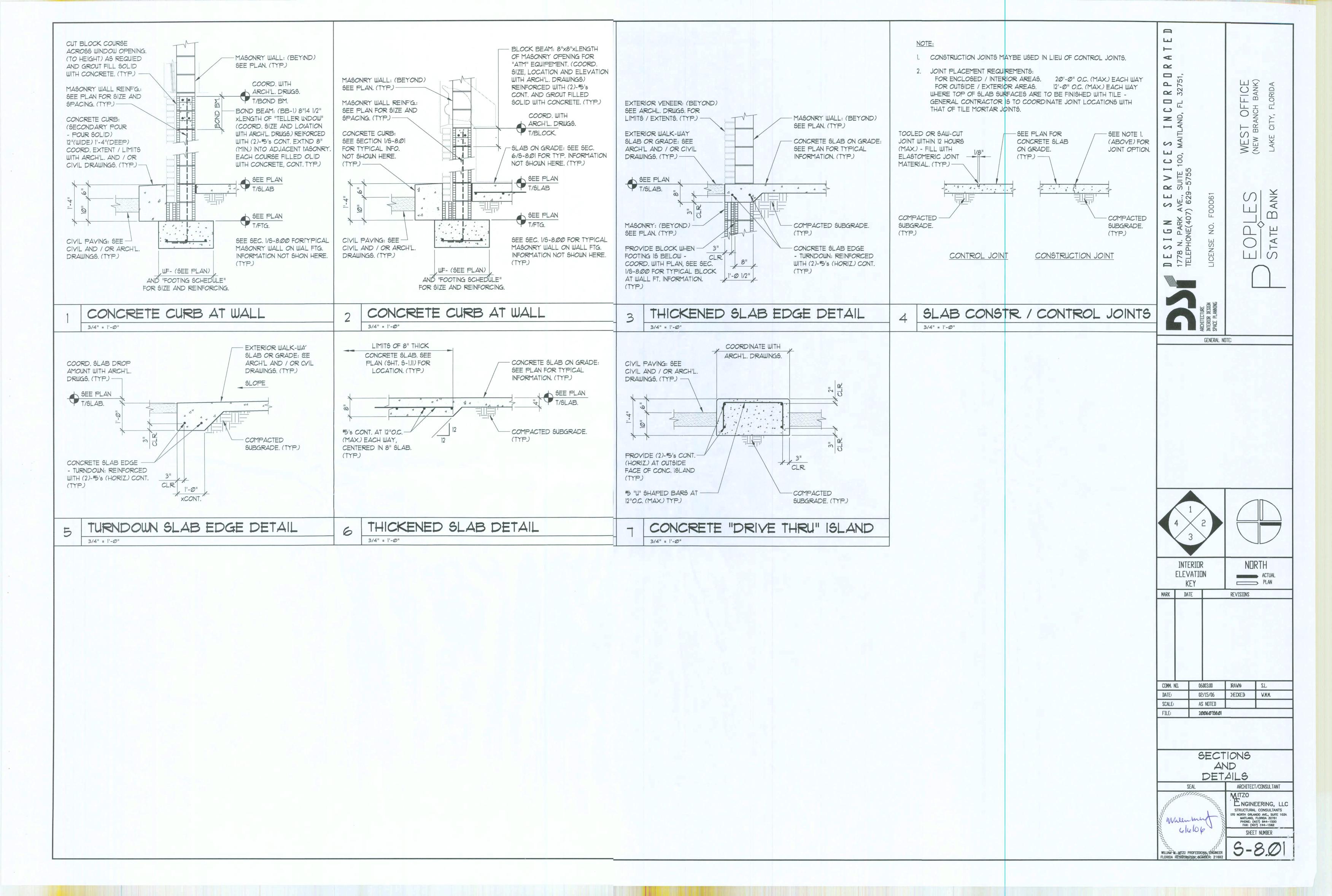
FRAMING ANCHOR SCHEDULE (AT MASONRY ONLY)

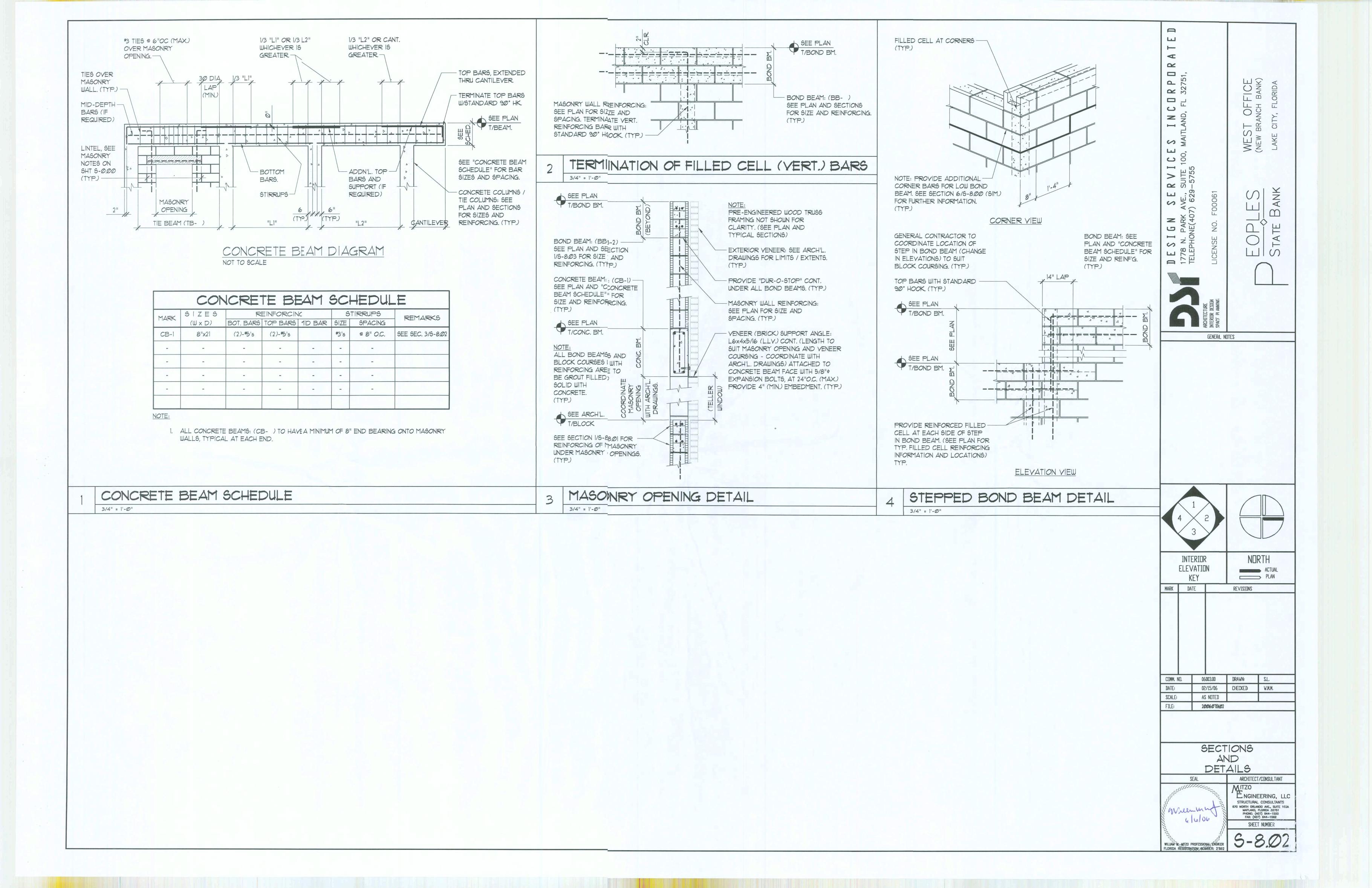
MARK:	TRUSS TYPES:	HANGER TYPE:		MANUE	DESCRIPTION	DEMADES
MARK:	TRUSS TIPES:	TOP	BOTTOM	MANUF.	DESCRIPTION	REMARKS
	GIRDER TRUSSES	-	HGT-2	SIMPSON	HEAVY GIRDER TIE DOWN	SEE NOTE 7
2	HIP TRUSSES	-	HETA2Ø	SIMPSON	EMBEDDED TRUSS ANCHOR	
3	FULL TRUSSES	-	HETA20	SIMPSON	EMBEDDED TRUSS ANCHOR	
4	JACK TRUSSES	-	HETA2Ø	SIMPSON	EMBEDDED TRUSS ANCHOR	

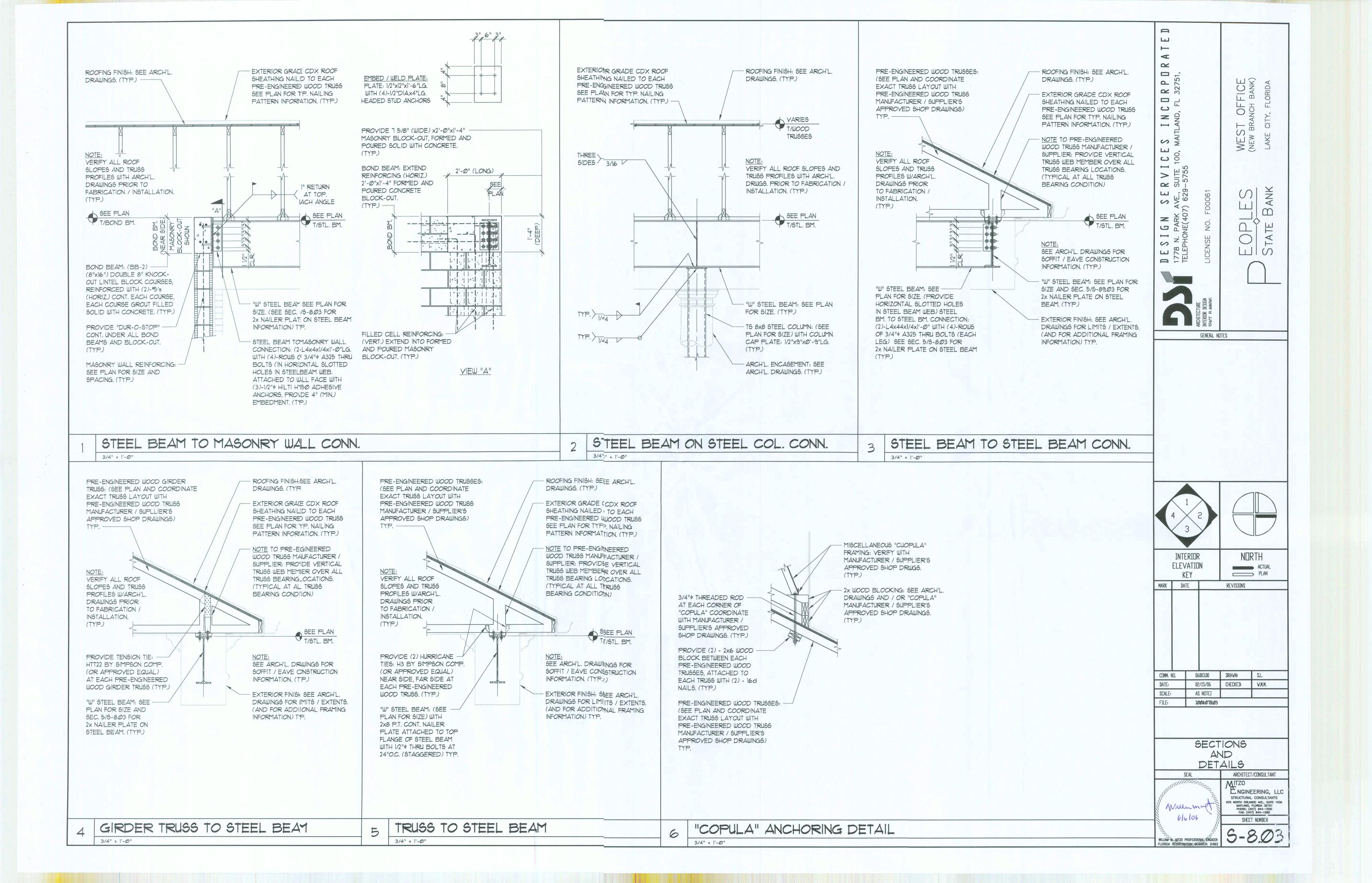
1. SEE SECTIONS FOR ADDITIONAL CONNECTOR CALLOUTS AND COORDINATE / YERIFY WITH APPROVED SHOP DRAWINGS.

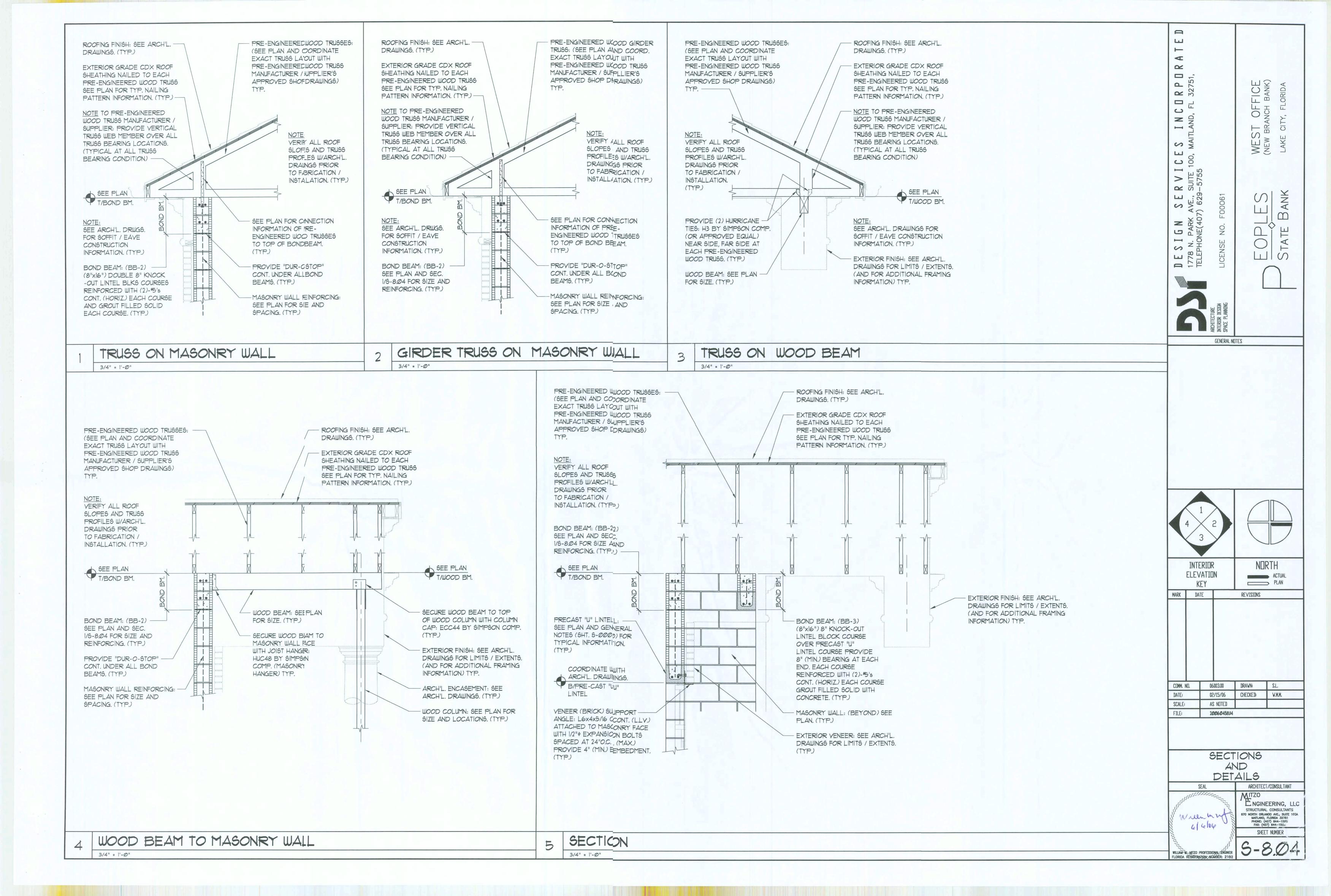
2. ALL "SIMPSON COMPANY" HANGERS, ETC. (OR APPROVED EQUAL) SHALL BE USED / INSTALLED AS RECOMMENDED BY THE MANUFACTURE'S REQUIREMENTS.

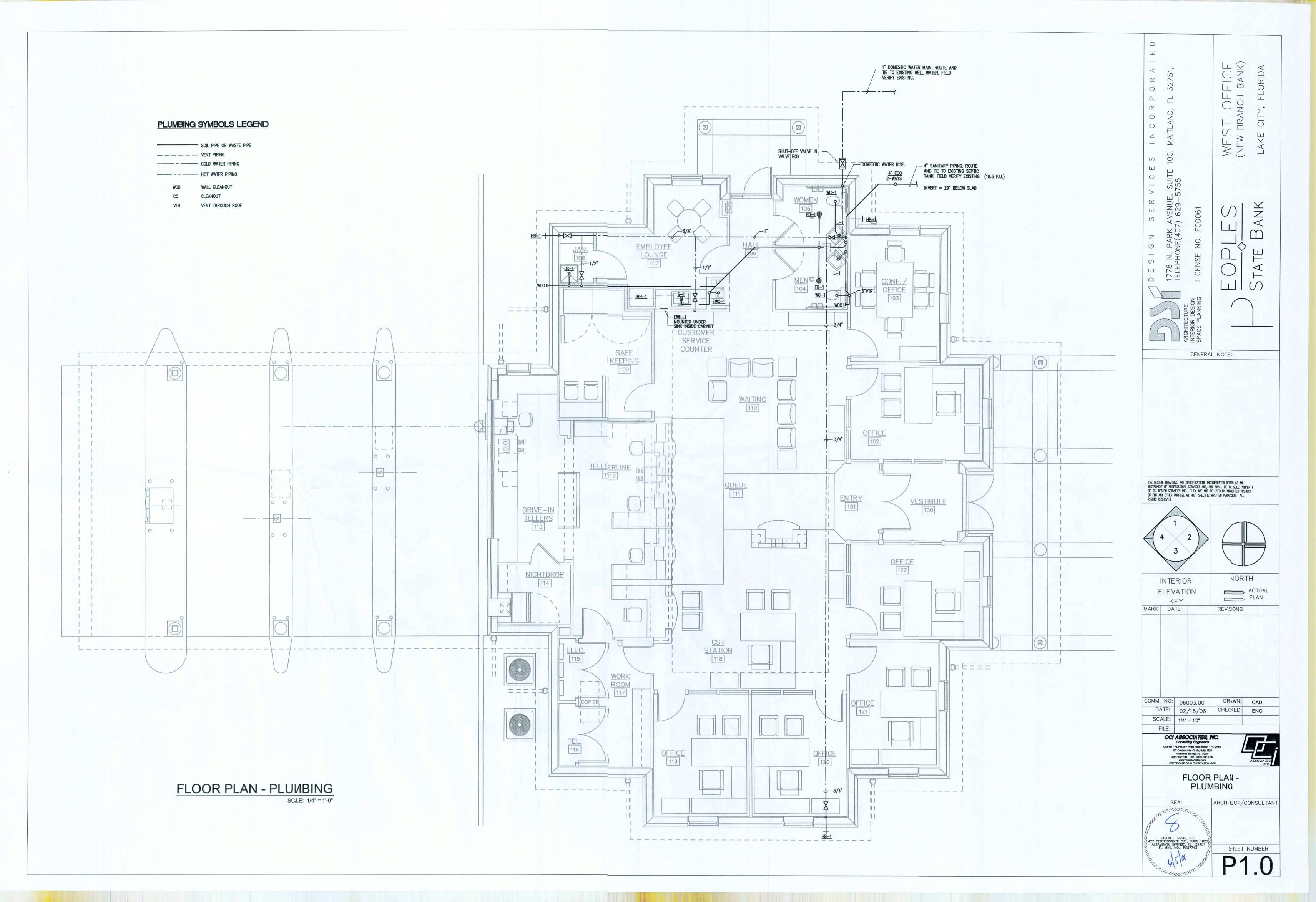




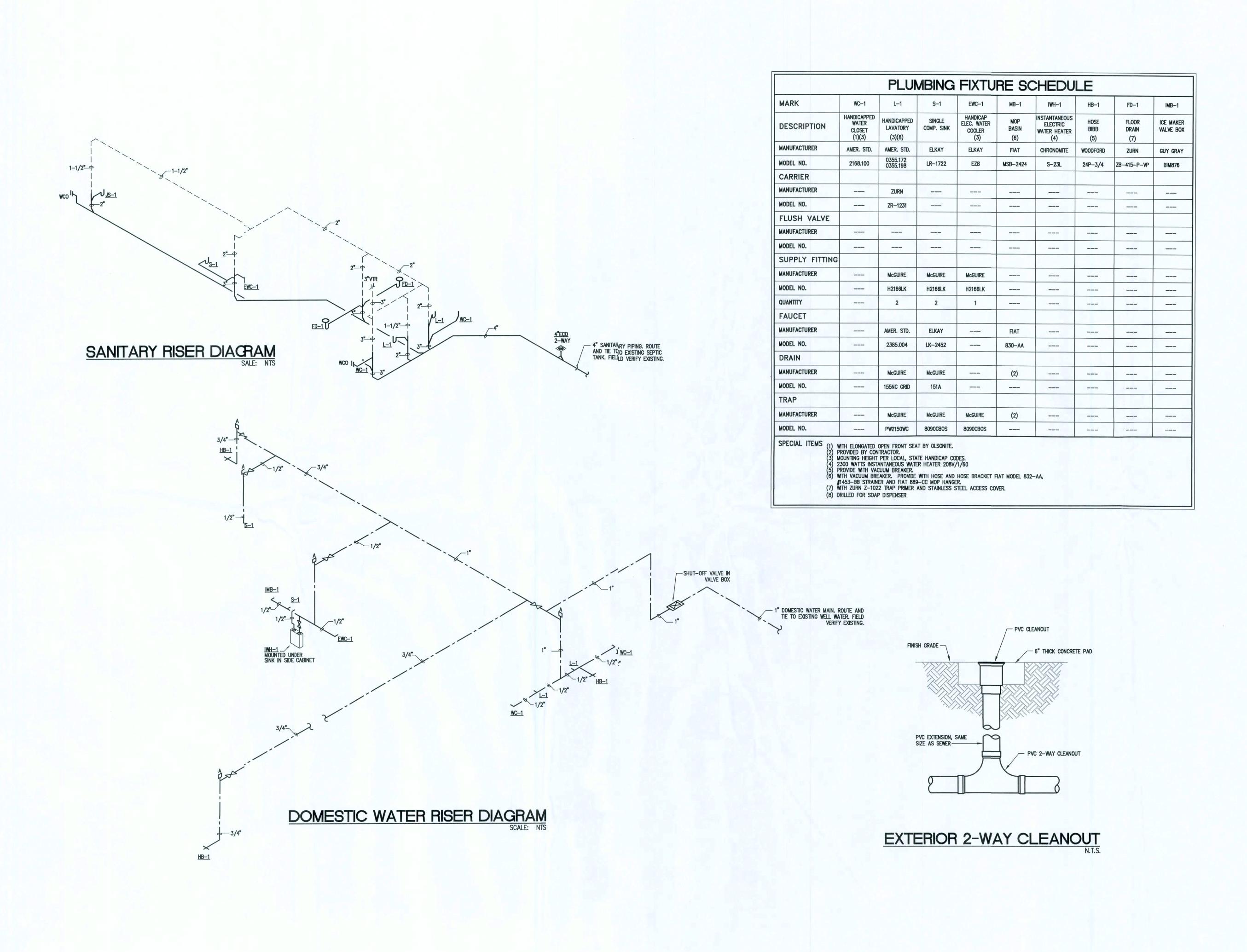






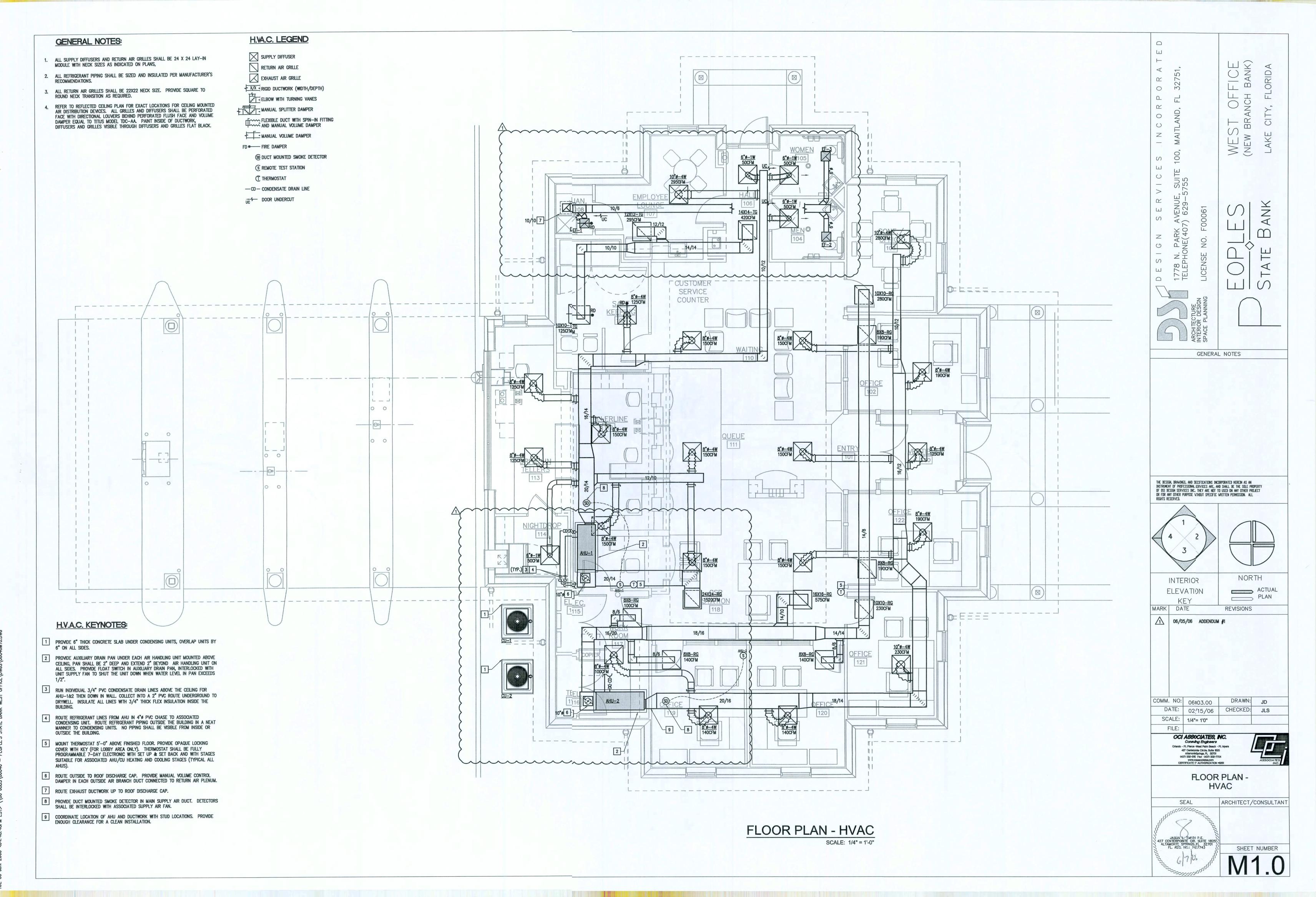


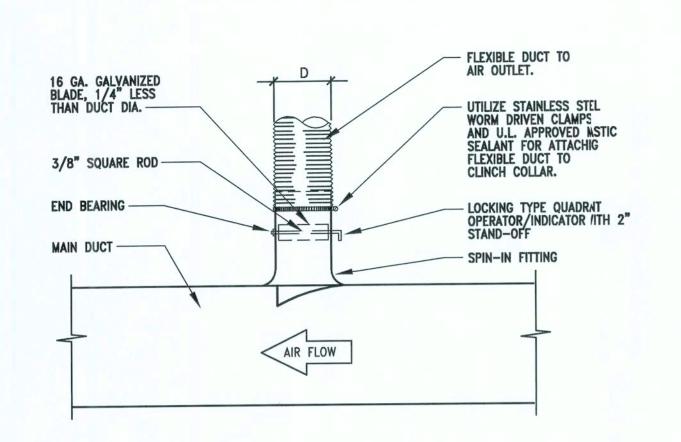
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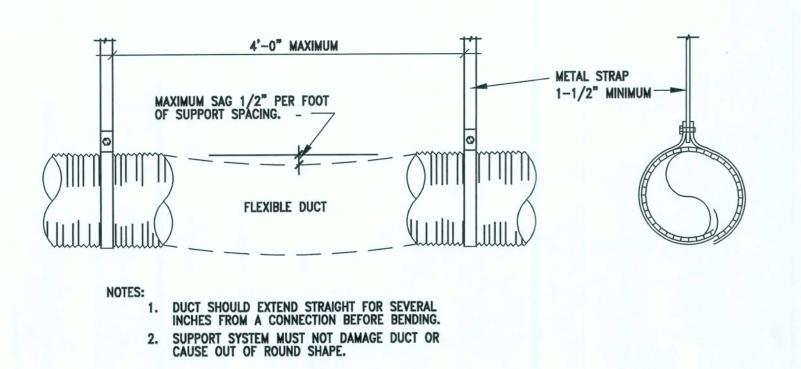
0 R X  $\triangleleft$ S D E 9 GENERAL NOTES THE DESIGN, DRAWINGS, AND SPECIFICATIONS INCORPORATED HEREIN AS AN INSTRUMENT OF PROFESSIONAL SERVICES RE, AND SHALL BE THE SOLE PROPERTY OF DSI DESIGN SERVICES INC.. THEY ARE NOT TO USED ON ANY OTHER PROJECT OR FOR ANY OTHER PURPOSE WITHOUT SPEIFIC WRITTEN PERMISSION. ALL RIGHTS RESERVED. ACTUAL PLAN ELEVATION MARK DATE REVISIONS COMM. NO: 06003.00 DRAWN: CAD DATE: 02/15/06 CHECKED: ENG SCALE: 1/4" = 1'0" OCI ASSOCIATES, NC.
Consulting Engineers

Orlando - Ft. Plerce - West Palm Bach - Ft. Myers
427 Centerpointe Circle, Site 1825
Altamonte Springs, Ft. 2701
(407) 332-510 Fext (407) 32-7704
www.oclassociates.om
CERTIFICATE OF AUTHORIZ/TION +6261 SCHEDULE, DETAILS AND RISERS - PLUMBING ARCHITECT/CONSULTANT SHEET NUMBER

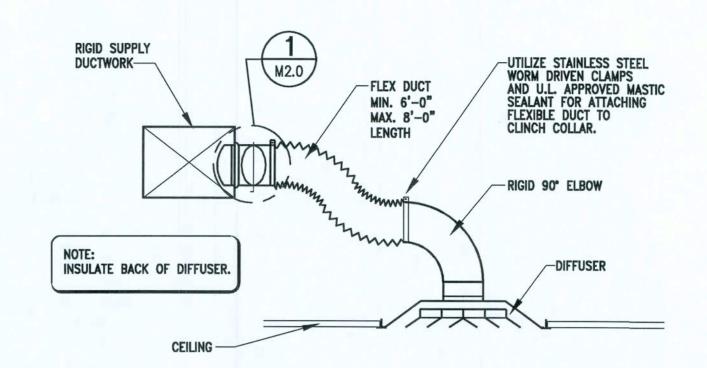




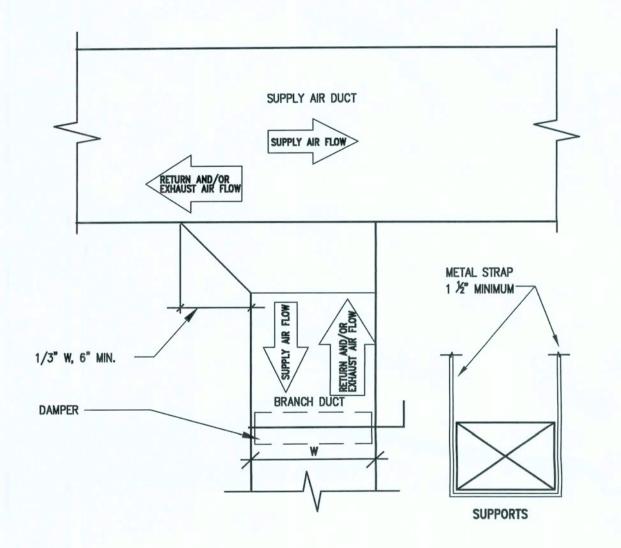
SPIN-IN BRANCH CONNECTION TO SINGLE AIR OUTLET



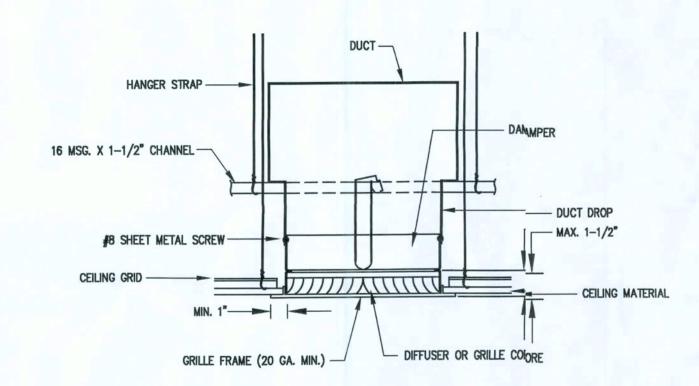
2 FLEX DUCT SUPPORT



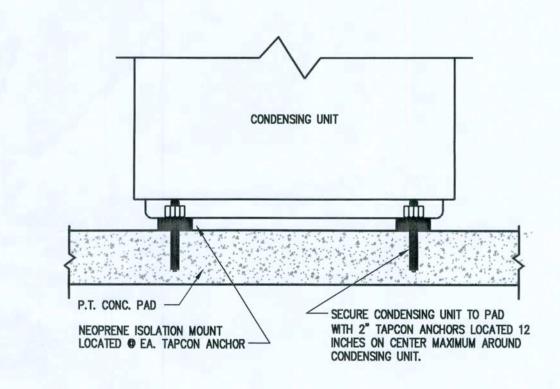
DIFFUSER AND FLEX DUCT DETAIL

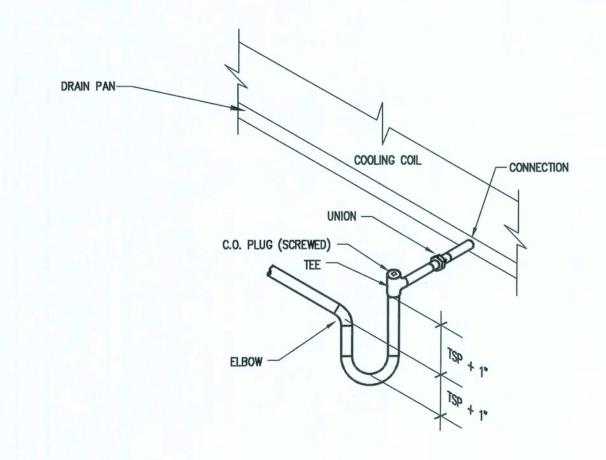


DUCT WITH SPLITTER
DAMPER AT SINGLE BRANCH

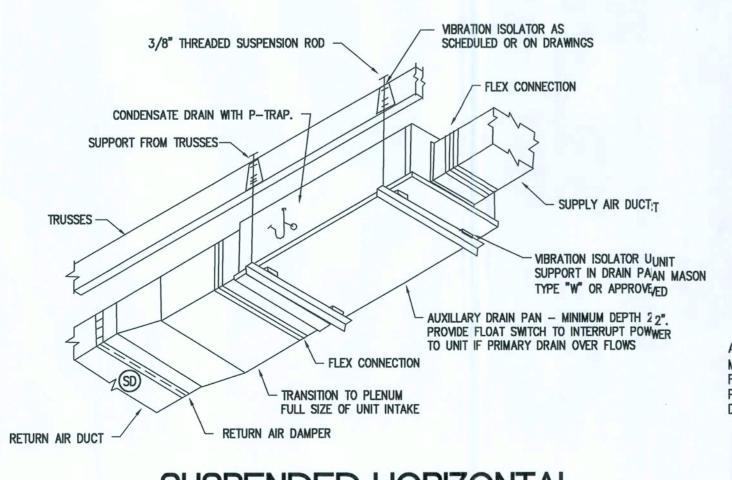


5 DIFFUSER/GRILLE RADIANT DAMPER DETAIL
NTS 6 CONDENSING UNIT AT PAD DETAIL
NTS NTS



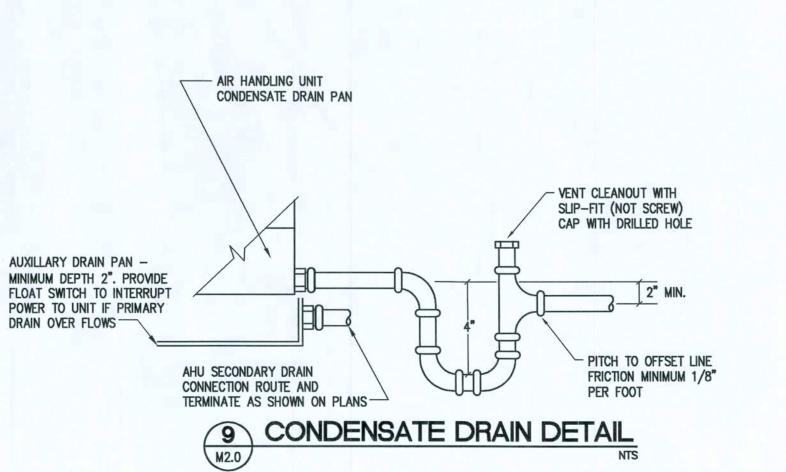


7 CONDENSATE DRAIN DETAIL NTS



SUSPENDED HORIZONTAL

8 AIR HANDLING UNIT DETAIL



ST OI BRANCI ANK 1778 TELEP GENERA\_ NOTES THE DESIGN, DRAVINGS, AND SPECIFICATIONS INORPORATED HEREIN AS AN INSTRUMENT OF PROFESSIONAL SERVICES ARE, AD SHALL BE THE SOLE PROPERTY OF DSI DESIGN SERVICES INC. THEY ARE NOT 1 USED ON ANY OTHER PROJECT OR FOR ANY OTHER PURPOSE VITHOUT SPECIFIC /RITTEN PERMISSION. ALL NORTH INTERIOR ACTUAL ELEVATION PLAN KEY MARK DATE **REVISIONS** COMM. NO: 06003.00 DATE: 02/15/06 CHECKED: SCALE: N.T.S. FILE: OCI ASSOCIATES, NC. Consulting Engineers Orlando - Ft. Pierce - Weet Palm Beach - t. Myers 427 Centerpointe Circle, Sulte 183 Attamonte Springs, Ft. 32701 (407) 332-510 Fax: (407) 332-7)4 www.oolassoolates.com CERTIFICATE OF AUTHORIZATION 8261 DETAILS - HVAC

SHEET NUMBER

ARCHITECT/CONSULTANT

SEAL