Project Information for:		L212216			
Builder:		JOHN NORRIS - DEMPSEY		Date:	9/28/2006
Lot:		N/A		Start Number:	**1111**
Subdivision:		162 NW DON HART WAY		SEI Ref:	L212216
County or City:		COLUMBIA COUNTY			
Truss Page Count:		38			

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORRIS, JOHN DAVID RG 0066597
Address: 351 NW CORWIN GLN
LAKE CITY, FL. 32025 Designer: **113**

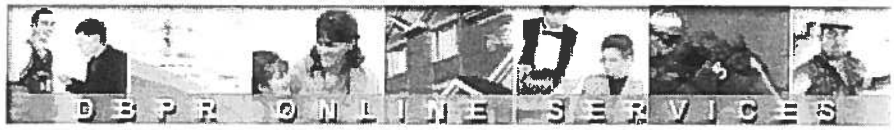
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0928061111	9/28/2006				
2	CJ3	0928061112	9/28/2006				
3	CJ3T	0928061113	9/28/2006				
4	CJ5	0928061114	9/28/2006				
5	CJ5T	0928061115	9/28/2006				
6	EJ7	0928061116	9/28/2006				
7	EJ7A	0928061117	9/28/2006				
8	EJ7T	0928061118	9/28/2006				
9	HJ9	0928061119	9/28/2006				
10	HJ9T	0928061120	9/28/2006				
11	T01	0928061121	9/28/2006				
12	T01A	0928061122	9/28/2006				
13	T01G	0928061123	9/28/2006				
14	T02	0928061124	9/28/2006				
15	T03	0928061125	9/28/2006				
16	T04	0928061126	9/28/2006				
17	T04A	0928061127	9/28/2006				
18	T04G	0928061128	9/28/2006				
19	T05	0928061129	9/28/2006				
20	T06	0928061130	9/28/2006				
21	T07	0928061131	9/28/2006				
22	T08	0928061132	9/28/2006				
23	T09	0928061133	9/28/2006				
24	T10	0928061134	9/28/2006				
25	T11	0928061135	9/28/2006				
26	T12	0928061136	9/28/2006				
27	T13	0928061137	9/28/2006				
28	T14	0928061138	9/28/2006				
29	T15	0928061139	9/28/2006				
30	T17	0928061140	9/28/2006				
31	T18	0928061141	9/28/2006				
32	T19	0928061142	9/28/2006				
33	T20	0928061143	9/28/2006				
34	T21	0928061144	9/28/2006				
35	T22	0928061145	9/28/2006				
36	T23	0928061146	9/28/2006				
37	T24	0928061147	9/28/2006				
38	T25	0928061148	9/28/2006				

SEP 2 8 2006



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Licensee Details

Licensee Information

Name:	**NORRIS, JOHN DAVID (Primary Name)** **INDIVIDUAL (Alternate Name)**
Main Address:	**351 NW CORWIN GLN** **LAKE CITY, Florida 32055**
Lic. Location:	**WOODGLEN DRIVE** **LAKE CITY, FL 32055** **Columbia**

License Information

License Type:	**Registered General Contractor**
Rank:	**Reg General**
License Number:	**RG0066597**
Status:	**Current, Active**
Licensure Date:	**06/20/1996**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
No Qualified Business License Required	02/20/2004

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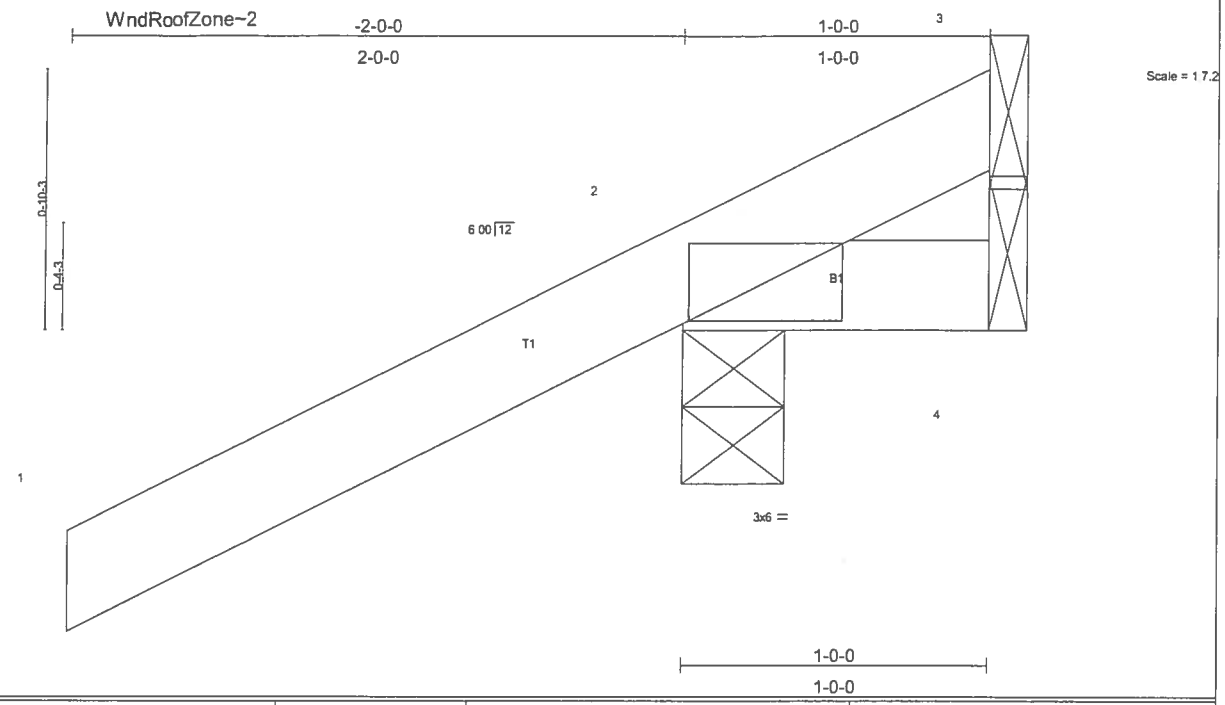
SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061111

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	CJ1	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:18:53 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-287(load case 5), 4=-9(load case 3), 3=-91(load case 1)
Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

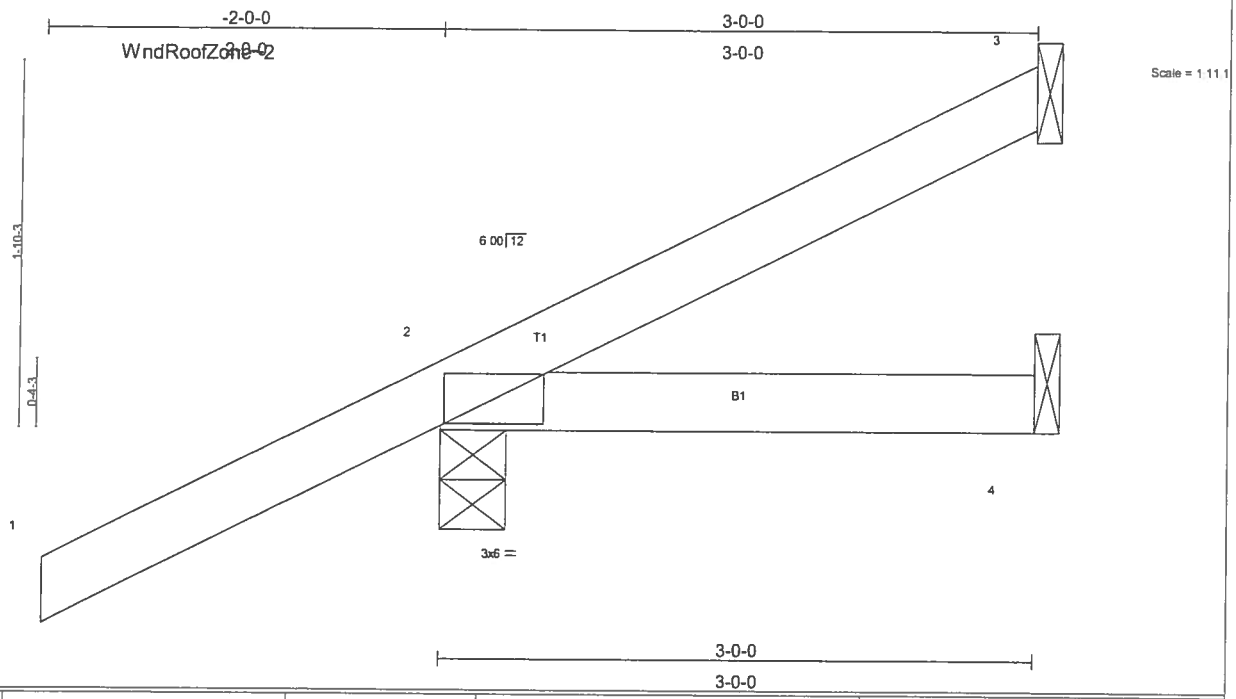
LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061112

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:18:54 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-27(load case 6), 2=-240(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

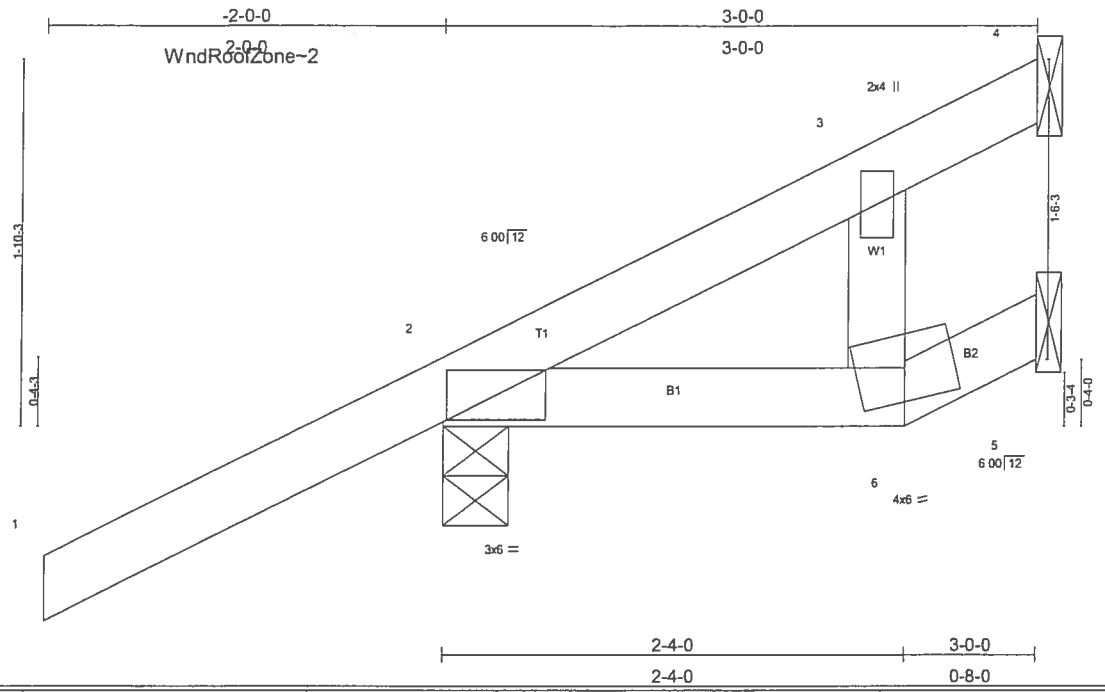
LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061113

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	CJ3T	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:18:55 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=62/Mechanical, 2=279/0-4-0, 5=9/Mechanical
Max Horz 2=132(load case 5)
Max Uplift4=-21(load case 6), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-59/3, 3-4=-16/26
BOT CHORD 2-6=-5/0, 5-6=-3/3
WEBS 3-6=0/42

JOINT STRESS INDEX
2 = 0.13, 3 = 0.03 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 4 and 205 lb uplift at joint 2.

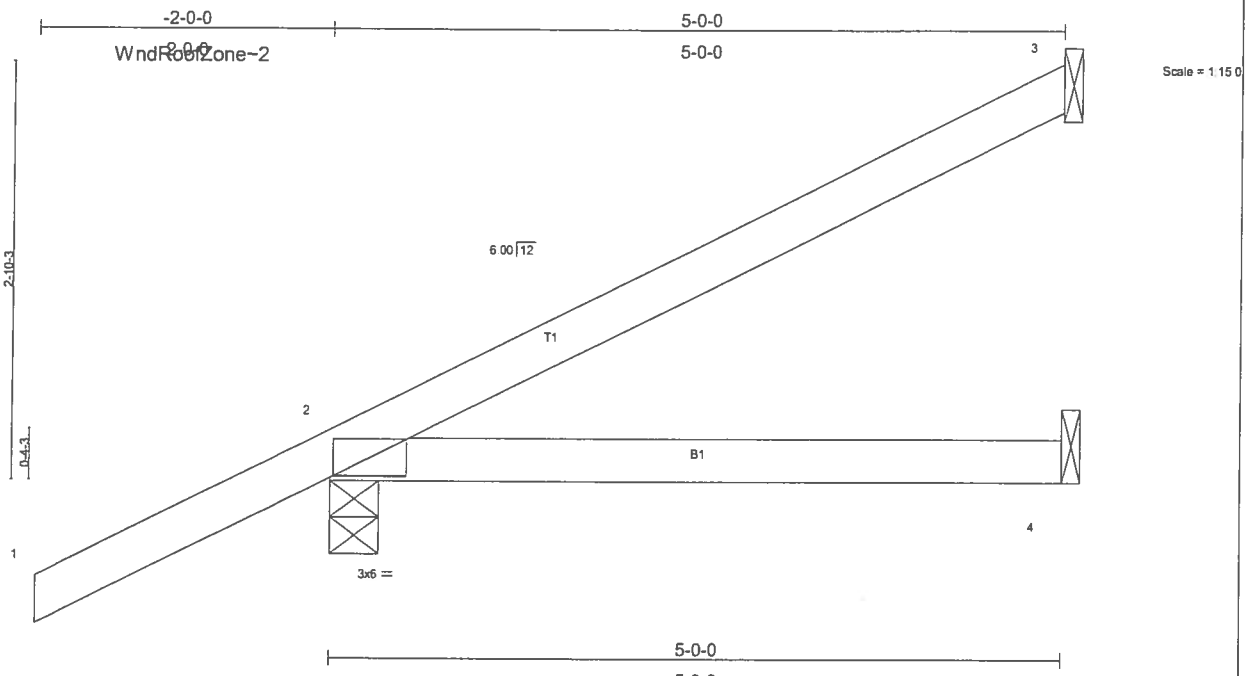
LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061114

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	CJ5	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:18:55 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.09	2-4	>671	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-86(load case 5), 2=-261(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061115

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	CJ5T	SPECIAL	2	1	Job Reference (optional)

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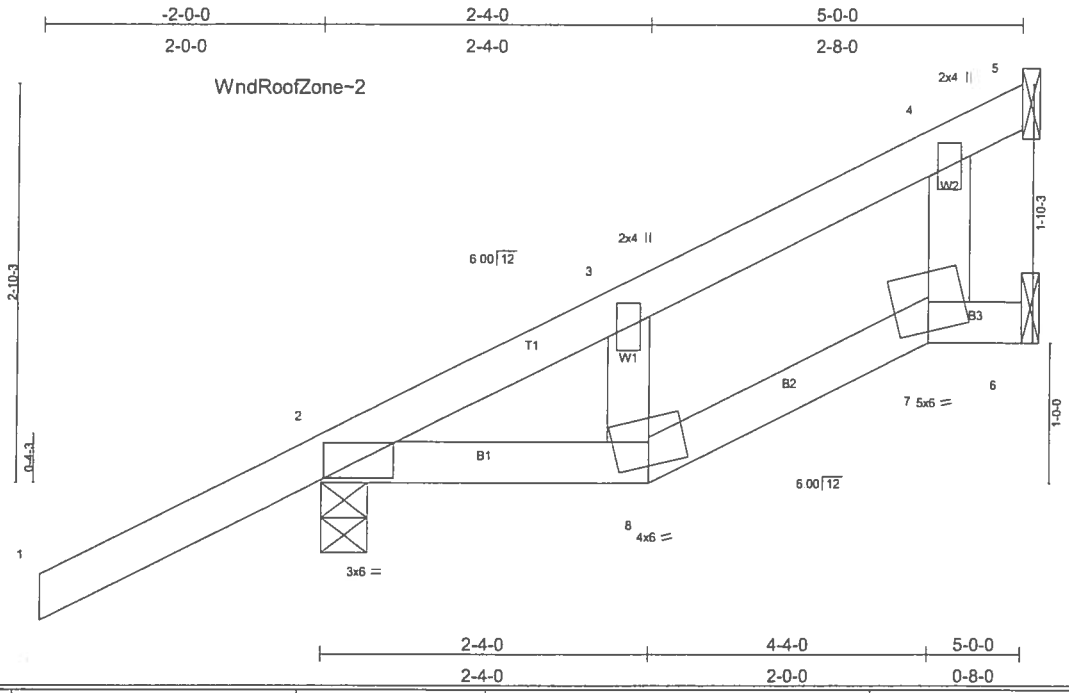


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.06	8	>910	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.09	8	>618	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=165/Mechanical, 2=344/0-4-0, 6=9/Mechanical
Max Horz 2=178(load case 5)
Max Uplift5=-74(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-92/0, 3-4=-74/34, 4-5=-62/73
BOT CHORD 2-8=-3/0, 7-8=-5/16, 6-7=-0/0
WEBS 3-8=0/61, 4-7=0/42

JOINT STRESS INDEX
2 = 0.15, 3 = 0.04, 4 = 0.03, 7 = 0.01 and 8 = 0.03

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 5 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061116

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	EJ7	MONO TRUSS	24	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:18:57 2006 Page 1

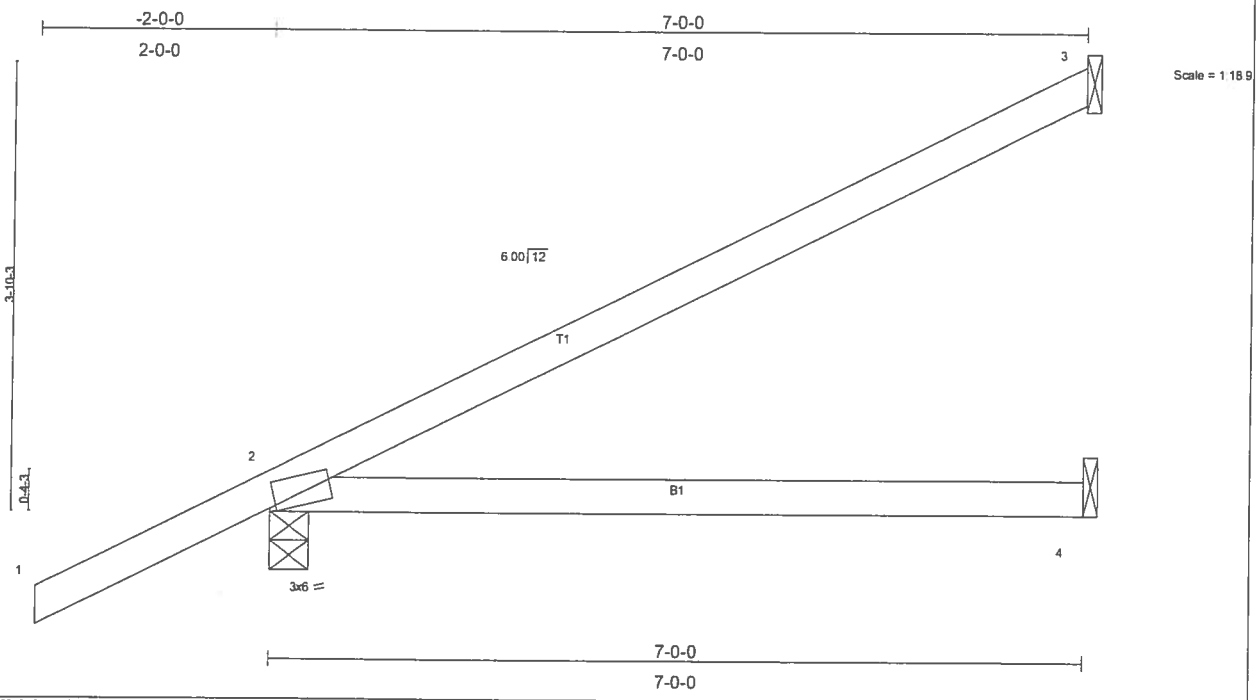


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.26	2-4	>309	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.21	2-4	>379	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-296(load case 5), 4=-67(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.76

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 296 lb uplift at joint 2 and 67 lb uplift at joint 4.

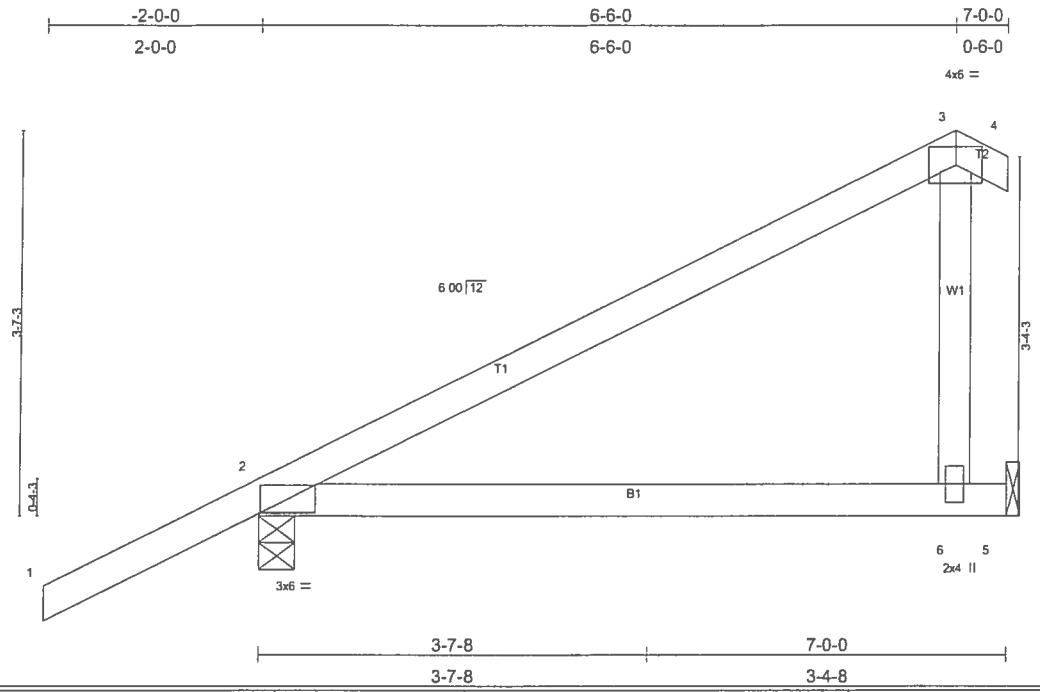
LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061117

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	EJ7A	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:18:57 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.14	2-6	>589	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.23	2-6	>355	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=420/0-4-0, 5=269/Mechanical, 5=269/Mechanical
Max Horz 2=201(load case 5)
Max Uplift2=-222(load case 5), 5=-119(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-86/56, 3-4=0/12
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-183/141

JOINT STRESS INDEX
2 = 0.48, 3 = 0.07 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 2 and 119 lb uplift at joint 5.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061118

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	EJ7T	SPECIAL	4	1	Job Reference (optional)

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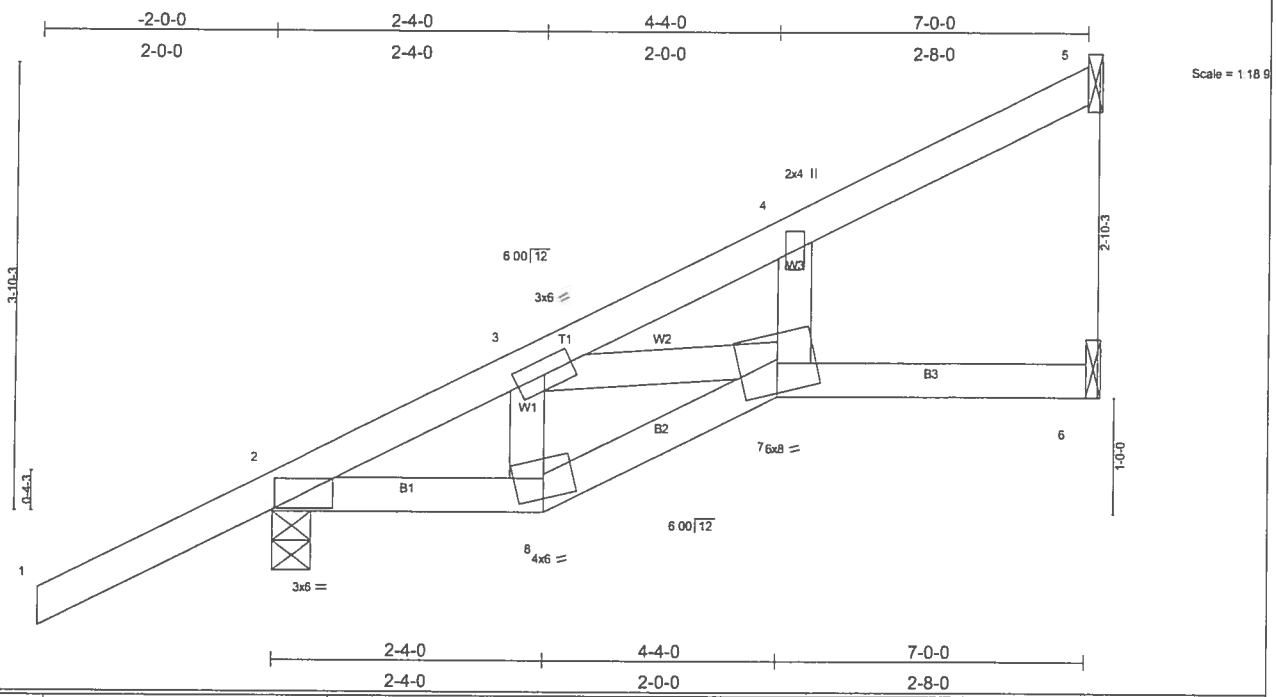

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.09	7	>943	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.14	7	>587	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=144/Mechanical, 2=420/0-4-0, 6=121/Mechanical
Max Horz 2=224(load case 5)
Max Uplift5=-91(load case 5), 2=-211(load case 5), 6=-35(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-443/29, 3-4=-84/0, 4-5=-49/59
BOT CHORD 2-8=-126/362, 7-8=-111/361, 6-7=-0/0
WEBS 3-8=-1/48, 3-7=-355/126, 4-7=0/61

JOINT STRESS INDEX
2 = 0.57, 3 = 0.09, 4 = 0.04, 7 = 0.74 and 8 = 0.29

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 5, 211 lb uplift at joint 2 and 35 lb uplift at joint 6.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061119

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	HJ9	MONO TRUSS	5	1	Job Reference (optional)

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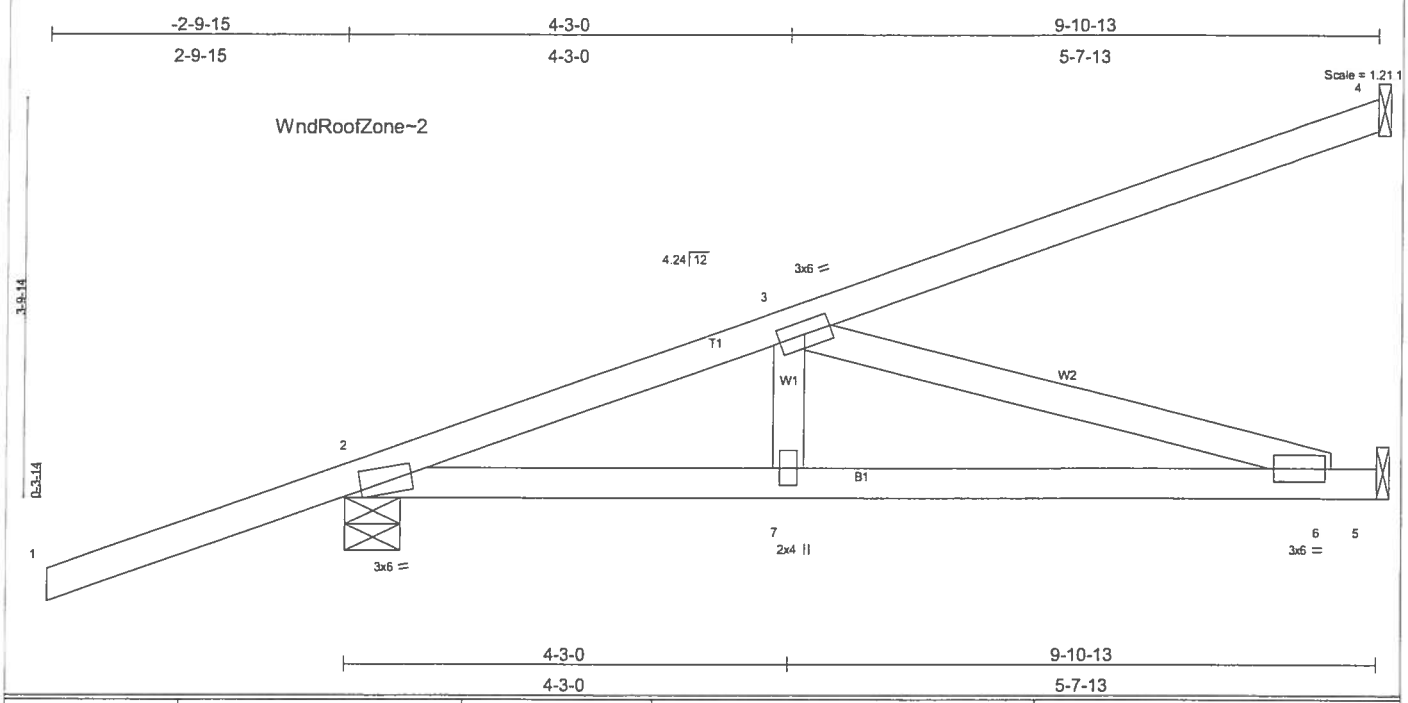

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=537/0-6-7, 5=372/Mechanical
Max Horz 2=270(load case 2)
Max Uplift4=-233(load case 2), 2=-404(load case 2), 5=-180(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-877/359, 3-4=-105/66
BOT CHORD 2-7=-530/810, 6-7=-530/810, 5-6=0/0
WEBS 3-7=-94/187, 3-6=-844/553

JOINT STRESS INDEX
2 = 0.78, 3 = 0.23, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 404 lb uplift at joint 2 and 180 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061120

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	HJ9T	SPECIAL	1	1	Job Reference (optional)

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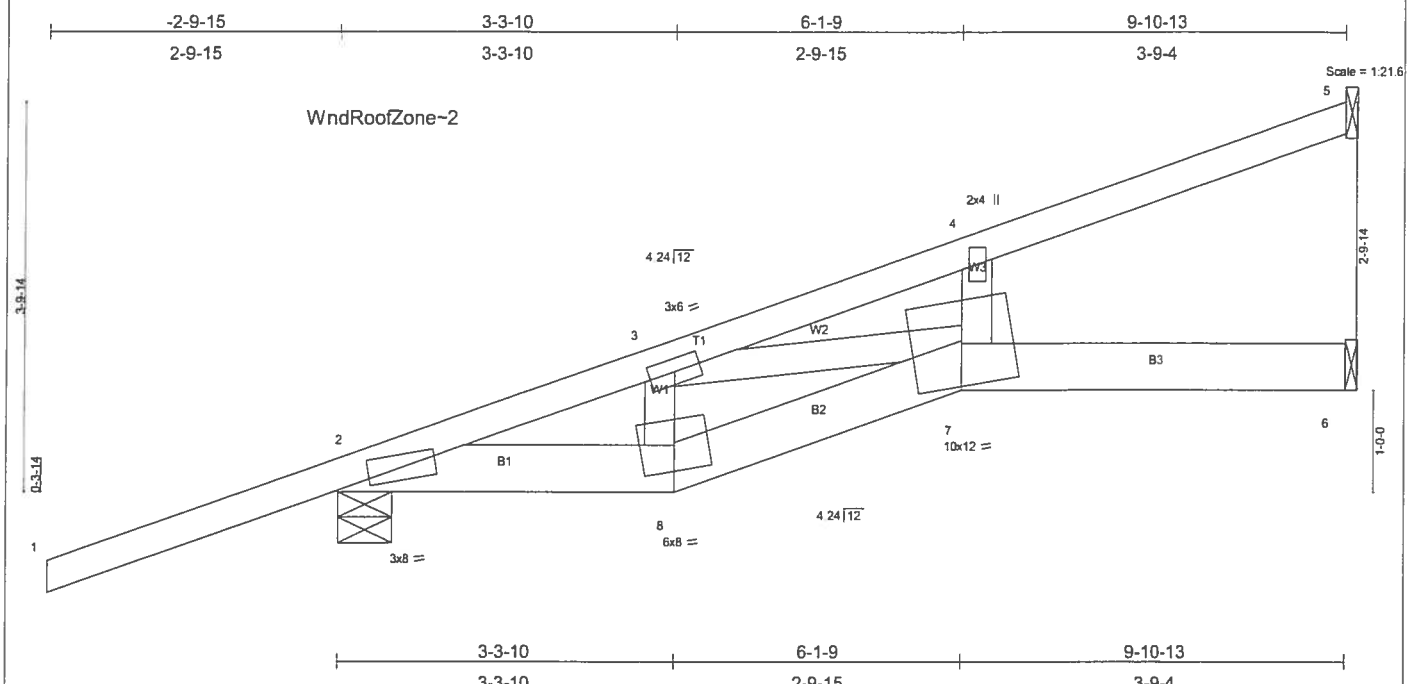

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.19	7	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.30	7	>378	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=245/Mechanical, 2=537/0-6-6, 6=397/Mechanical
Max Horz 2=273(load case 2)
Max Uplift5=-183(load case 2), 2=-286(load case 2), 6=-108(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-886/94, 3-4=-205/38, 4-5=-79/66
BOT CHORD 2-8=-250/823, 7-8=-216/778, 6-7=-1/0
WEBS 3-8=-46/141, 3-7=-869/284, 4-7=-211/253

JOINT STRESS INDEX
2 = 0.75, 3 = 0.25, 4 = 0.14, 7 = 0.83 and 8 = 0.29

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 5, 286 lb uplift at joint 2 and 108 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-5=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-8=-24(F=3, B=3), 8=-24(F=3, B=3)-to-7=-45(F=-8, B=-8), 7=-45(F=-8, B=-8)-to-6=-74(F=-22, B=-22)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061121

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T01	COMMON	3	1	Job Reference (optional)

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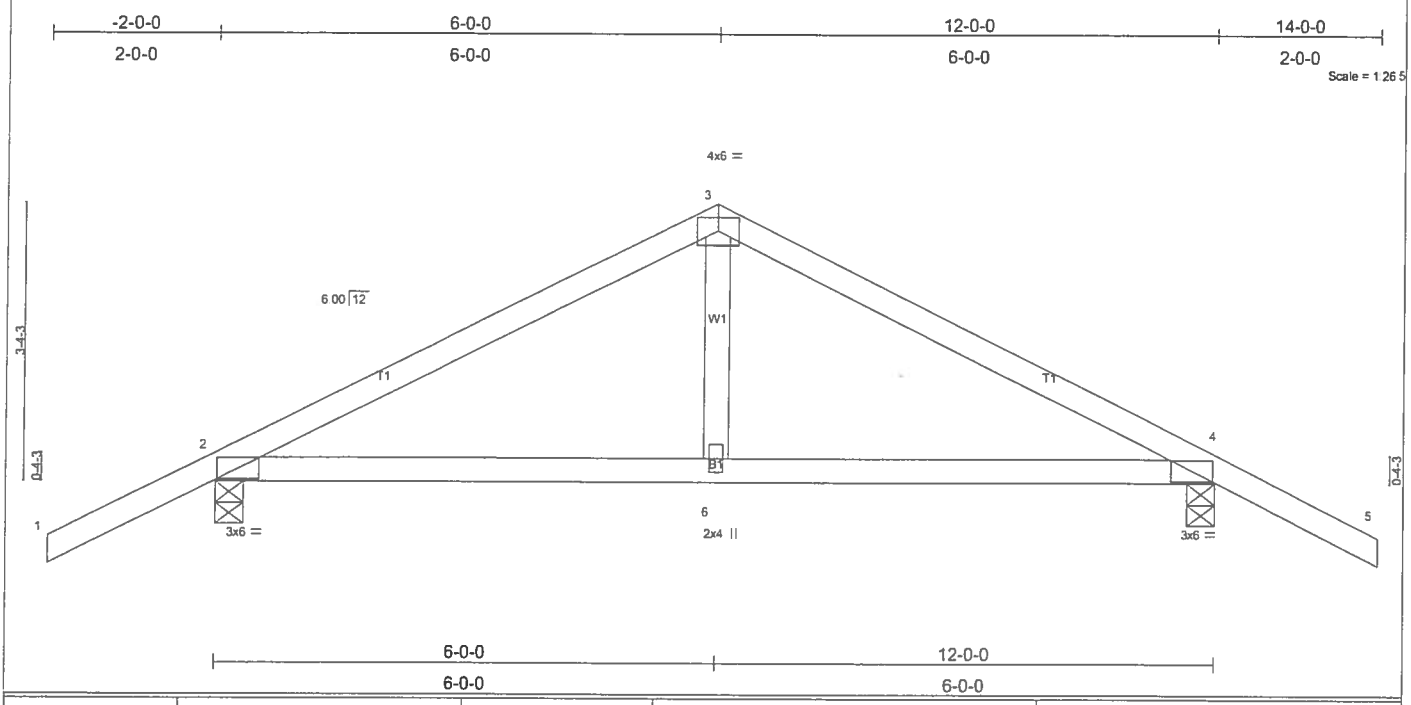

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.06	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=607/0-4-0, 4=607/0-4-0
Max Horz 2=80(load case 5)
Max Uplift2=-290(load case 5), 4=-290(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-653/172, 3-4=-653/171, 4-5=0/47
BOT CHORD 2-6=-71/520, 4-6=-71/520
WEBS 3-6=0/201

JOINT STRESS INDEX
2 = 0.42, 3 = 0.48, 4 = 0.42 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 2 and 290 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061122

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T01A	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:02 2006 Page 1

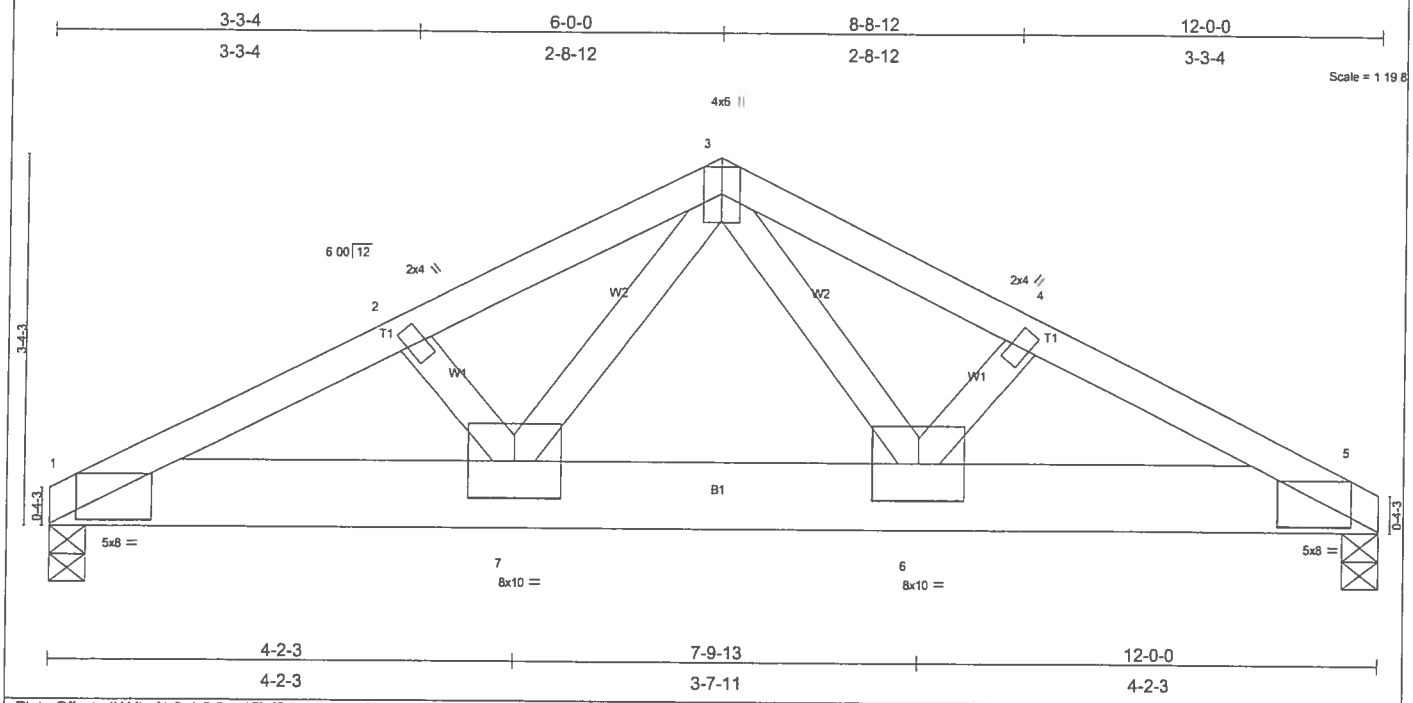

Plate Offsets (X,Y): [1:0-4-0,0-1-15], [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.45	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3675/0-4-0, 5=3675/0-4-0
Max Horz 1=-42(load case 2)
Max Uplift1=-1362(load case 4), 5=-1362(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5652/2090, 2-3=-5564/2084, 3-4=-5564/2085, 4-5=-5652/2091
BOT CHORD 1-7=-1878/5046, 6-7=-1250/3493, 5-6=-1838/5046
WEBS 2-7=-130/124, 3-7=-1045/2781, 3-6=-1045/2781, 4-6=-130/124

JOINT STRESS INDEX
1 = 0.63, 2 = 0.03, 3 = 0.68, 4 = 0.03, 5 = 0.63, 6 = 0.27 and 7 = 0.27

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1362 lb uplift at joint 1 and 1362 lb uplift at joint 5.
7) Girder carries tie-in span(s): 28-0-0 from 0-0-0 to 12-0-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-576(F=-546)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061123

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T01G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:03 2006 Page 1

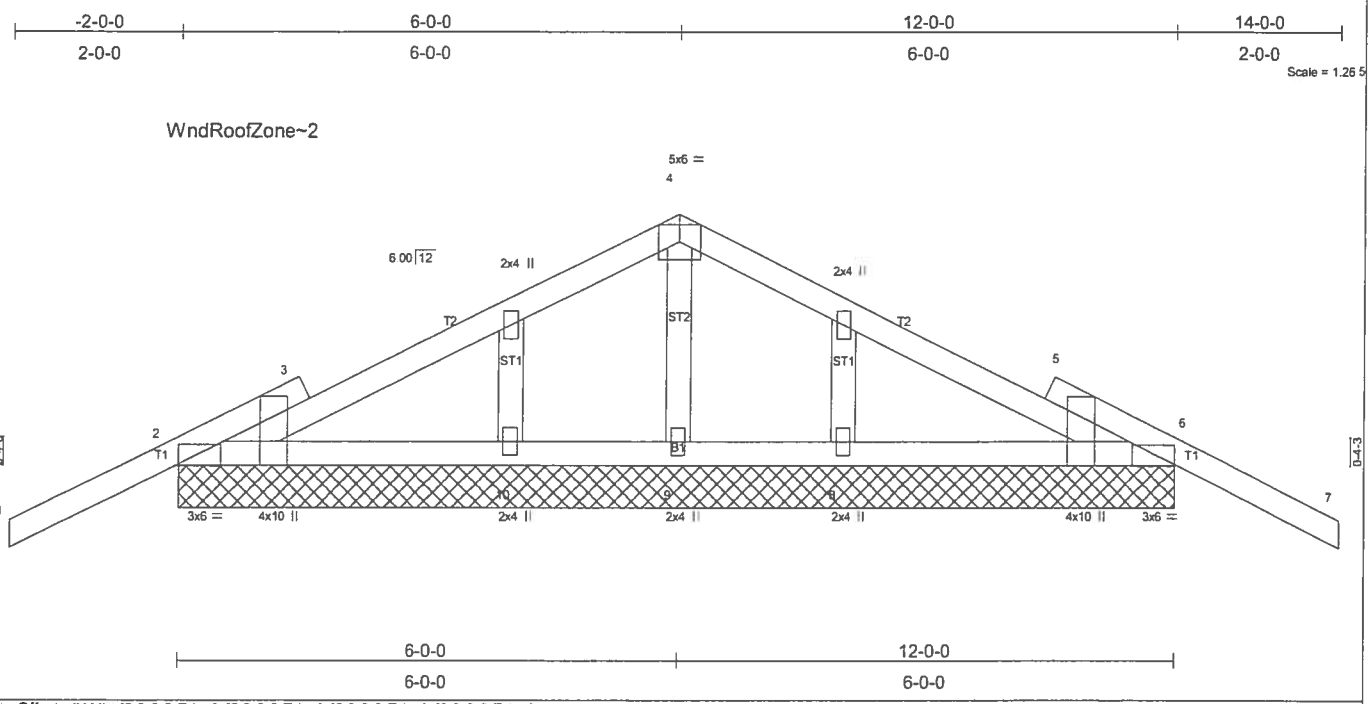

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.05	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	0.07	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=890/12-0-0, 6=890/12-0-0, 9=-108/12-0-0, 10=256/12-0-0, 8=256/12-0-0
Max Horz 2=-76(load case 6)
Max Uplift2=-454(load case 5), 6=-454(load case 6), 9=-108(load case 1), 10=-45(load case 5), 8=-45(load case 6)
Max Grav 2=890(load case 1), 6=890(load case 1), 9=63(load case 5), 10=256(load case 1), 8=256(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-19/99, 2-3=-909/527, 3-4=-820/525, 4-5=-820/525, 5-6=-909/527, 6-7=-19/99
BOT CHORD 2-10=-322/733, 9-10=-322/733, 8-9=-322/733, 6-8=-322/733

JOINT STRESS INDEX
2 = 0.93, 2 = 0.00, 3 = 0.00, 3 = 0.00, 4 = 0.82, 5 = 0.00, 5 = 0.00, 6 = 0.93, 6 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2, 454 lb uplift at joint 6, 108 lb uplift at joint 9, 45 lb uplift at joint 10 and 45 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061124

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T02	HIP	1	1	Job Reference (optional)

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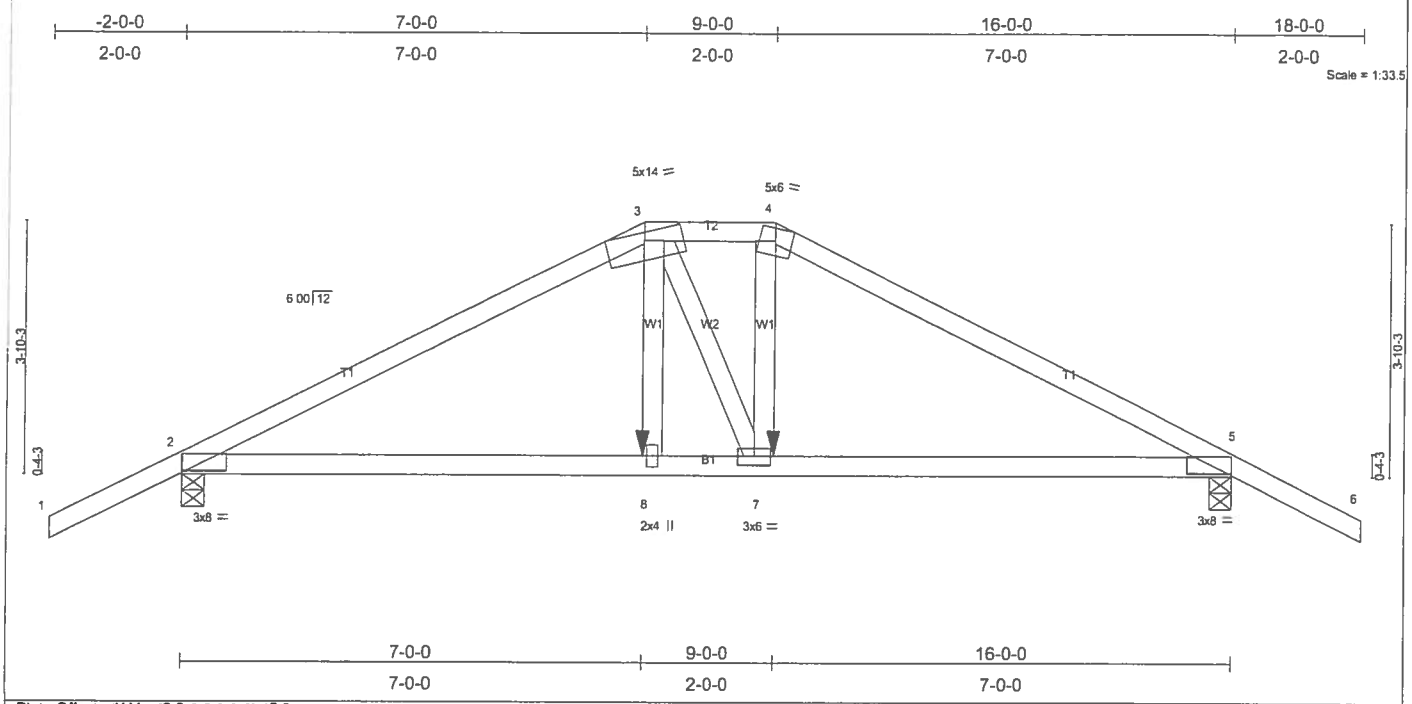

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.13	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.20	2-8	>953	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-15 oc bracing.

REACTIONS (lb/size) 2=1414/0-4-0, 5=1414/0-4-0
Max Horz 2=87(load case 4)
Max Uplift2=-866(load case 4), 5=-866(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2316/1223, 3-4=-2017/1171, 4-5=-2321/1225, 5-6=0/47
BOT CHORD 2-8=-1017/1984, 7-8=-1032/2013, 5-7=-997/1988
WEBS 3-8=-375/718, 4-7=-422/831, 3-7=-141/161

JOINT STRESS INDEX
2 = 0.74, 3 = 0.91, 4 = 0.64, 5 = 0.74, 7 = 0.54 and 8 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 866 lb uplift at joint 2 and 866 lb uplift at joint 5.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-121(F=-68), 4-6=-54, 2-8=-30, 7-8=-68(F=-38), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061125

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T03	COMMON	3	1	Job Reference (optional)

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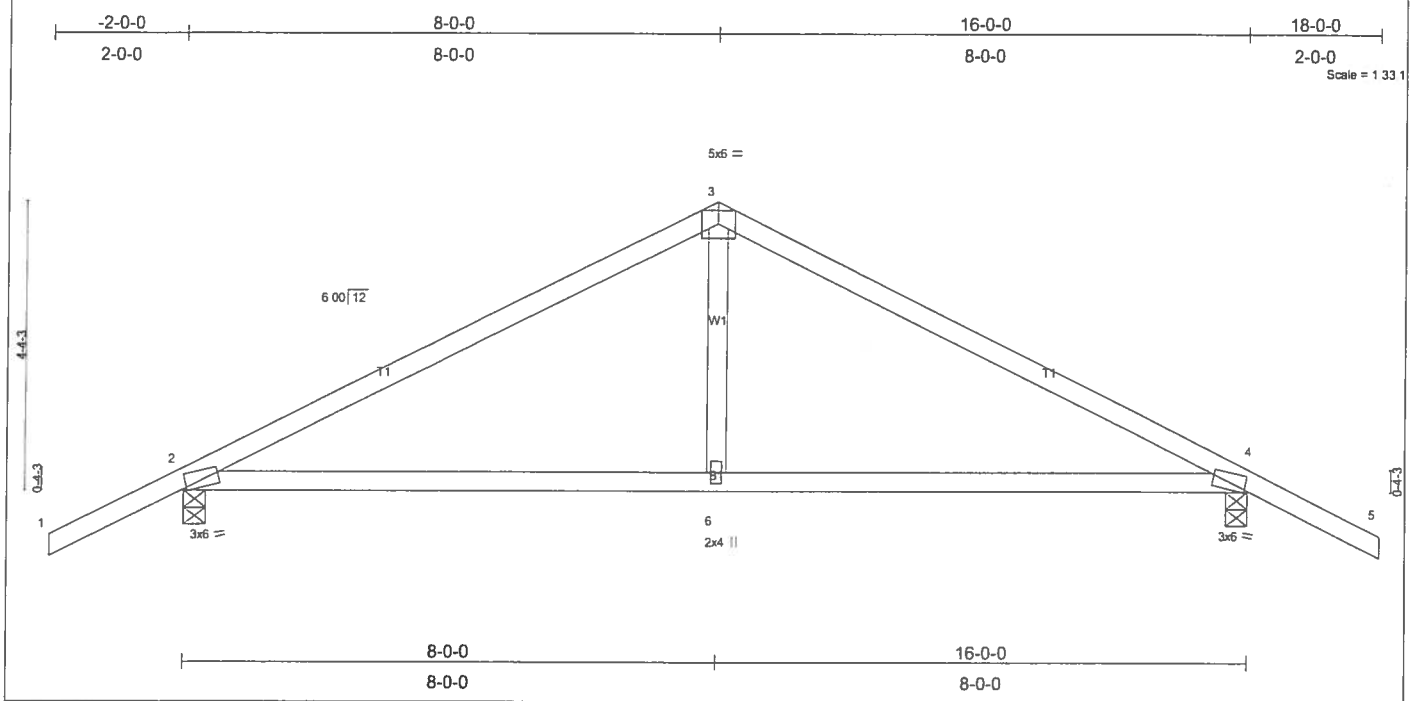

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.21	4-6	>890	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.20	4-6	>937	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 63 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-2 oc bracing.

REACTIONS (lb/size) 2=775/0-4-0, 4=775/0-4-0
Max Horz 2=-94(load case 6)
Max Uplift2=-539(load case 5), 4=-539(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-924/803, 3-4=-924/803, 4-5=0/47
BOT CHORD 2-6=-558/746, 4-6=-558/746
WEBS 3-6=-466/293

JOINT STRESS INDEX
2 = 0.76, 3 = 0.75, 4 = 0.76 and 6 = 0.21

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 2 and 539 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061126

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T04	COMMON	2	1	Job Reference (optional)

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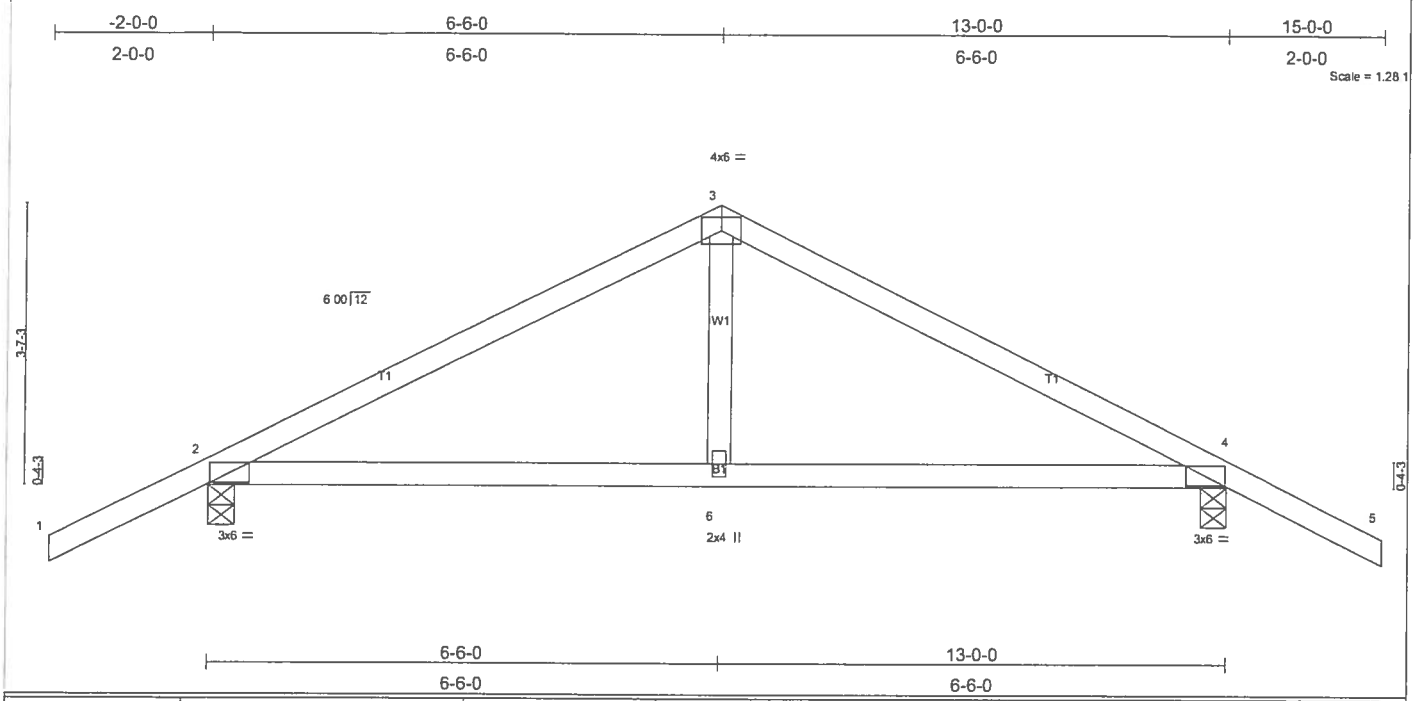

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.08	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=649/0-4-0, 4=649/0-4-0
Max Horz 2=-84(load case 6)
Max Uplift2=-303(load case 5), 4=-303(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-722/196, 3-4=-722/195, 4-5=0/47
BOT CHORD 2-6=-89/578, 4-6=-89/578
WEBS 3-6=0/225

JOINT STRESS INDEX
2 = 0.47, 3 = 0.58, 4 = 0.47 and 6 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 303 lb uplift at joint 4.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061127

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T04A	COMMON	1	2	Job Reference (optional)

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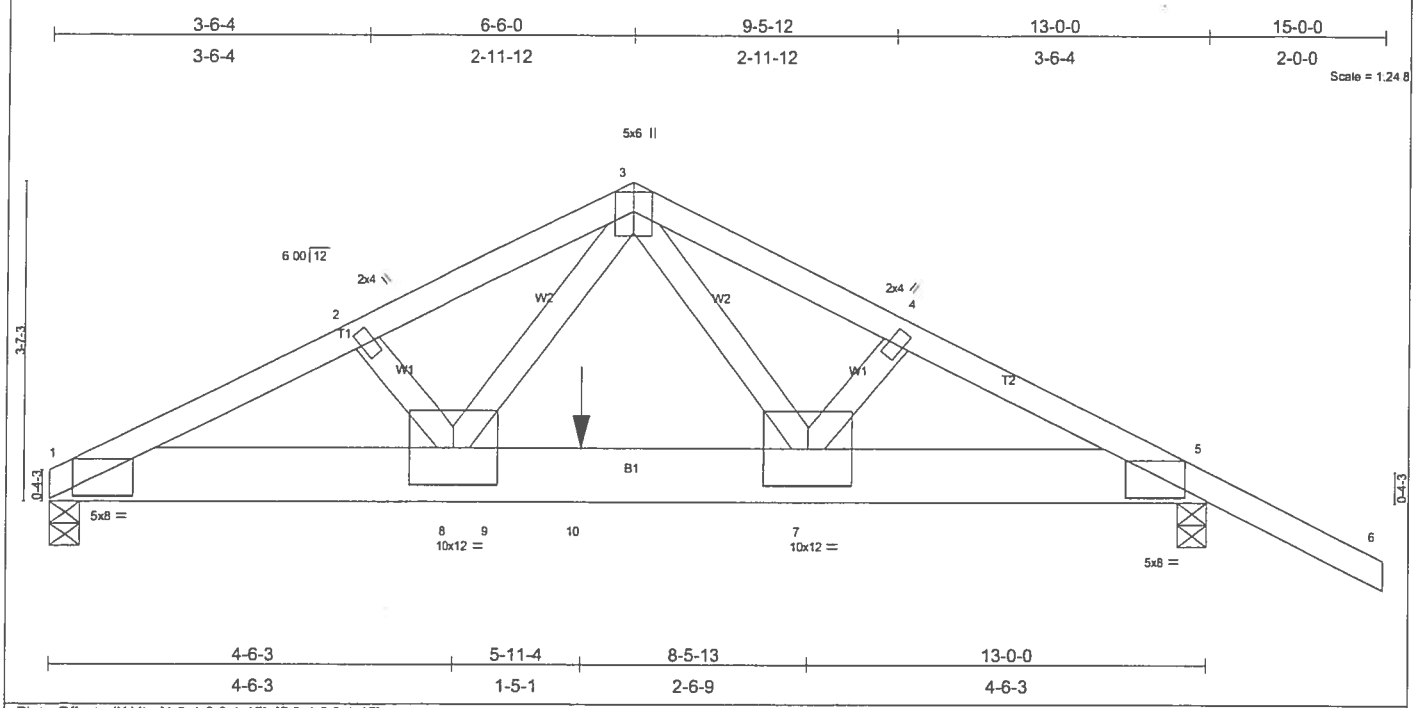

Plate Offsets (X,Y): [1:0-4-0,0-1-15], [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.09	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.14	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.72	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4214/0-4-0, 5=2465/0-4-0
Max Horz 1=-116(load case 5)
Max Uplift1=-1563(load case 4), 5=-990(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6882/2550, 2-3=-6789/2544, 3-4=-4941/1822, 4-5=-5037/1818, 5-6=0/54
BOT CHORD 1-8=-2260/6141, 8-9=-1283/3676, 9-10=-1283/3676, 7-10=-1283/3676, 5-7=-1565/4466
WEBS 2-8=-131/131, 3-8=-1686/4475, 3-7=-470/1387, 4-7=-94/112

JOINT STRESS INDEX
1 = 0.76, 2 = 0.03, 3 = 0.76, 4 = 0.03, 5 = 0.76, 7 = 0.38 and 8 = 0.38

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1563 lb uplift at joint 1 and 990 lb uplift at joint 5.
7) Girder carries tie-in span(s): 28-0-0 from 0-0-0 to 5-0-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2859 lb down and 1079 lb up at 5-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-576(F=-546), 5-9=-30
Concentrated Loads (lb)
Vert: 10=-2859(F)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061128

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T04G	COMMON	1	1	Job Reference (optional)

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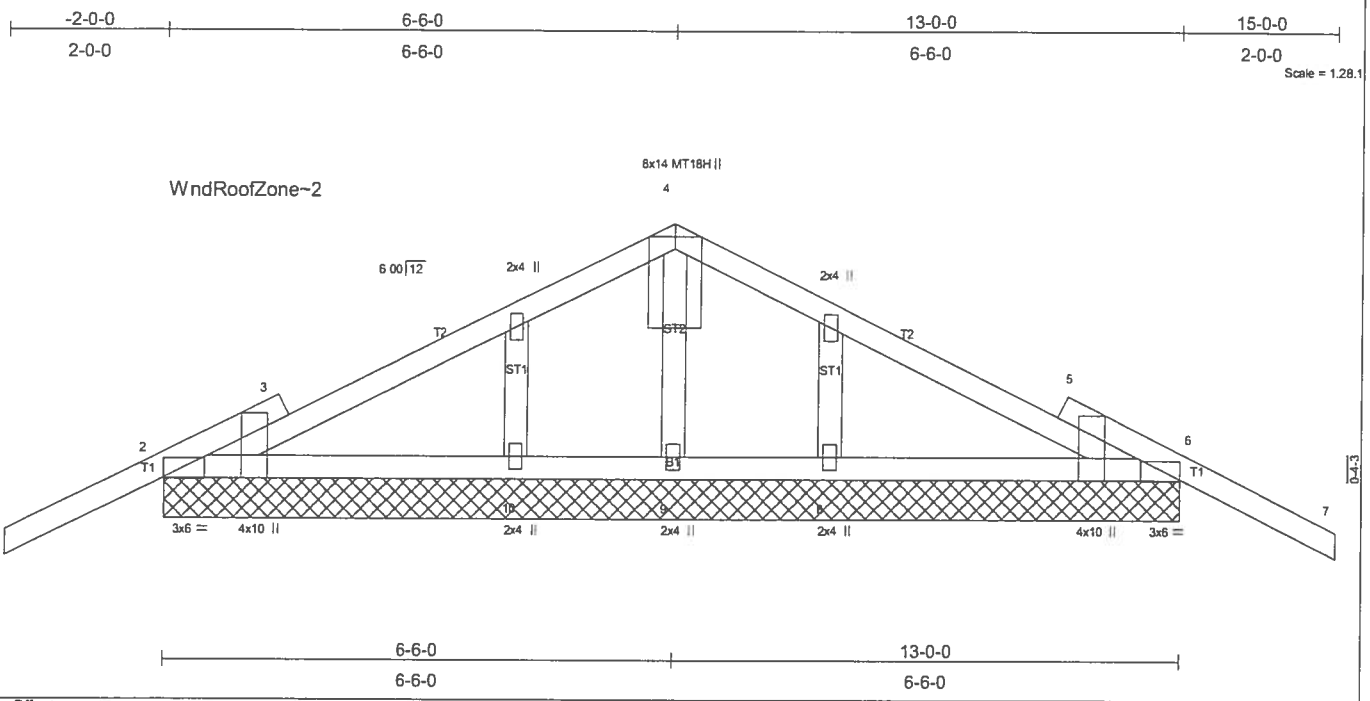

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.08	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	0.12	7	n/r	90	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 60 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-4 oc bracing.

REACTIONS (lb/size) 2=947/13-0-0, 6=947/13-0-0, 9=-155/13-0-0, 10=295/13-0-0, 8=295/13-0-0
Max Horz 2=-79(load case 6)
Max Uplift2=-475(load case 5), 6=-475(load case 6), 9=-155(load case 1), 10=-56(load case 5), 8=-56(load case 6)
Max Grav 2=947(load case 1), 6=947(load case 1), 9=76(load case 5), 10=295(load case 1), 8=295(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-18/99, 2-3=-993/561, 3-4=-898/561, 4-5=-898/561, 5-6=-993/561, 6-7=-18/99
BOT CHORD 2-10=-350/803, 9-10=-350/803, 8-9=-350/803, 6-8=-350/803

JOINT STRESS INDEX
2 = 1.00, 2 = 0.00, 3 = 0.00, 3 = 0.00, 4 = 0.91, 5 = 0.00, 5 = 0.00, 6 = 1.00, 6 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2, 475 lb uplift at joint 6, 155 lb uplift at joint 9, 56 lb uplift at joint 10 and 56 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061129

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T05	SPECIAL	1	2	Job Reference (optional)

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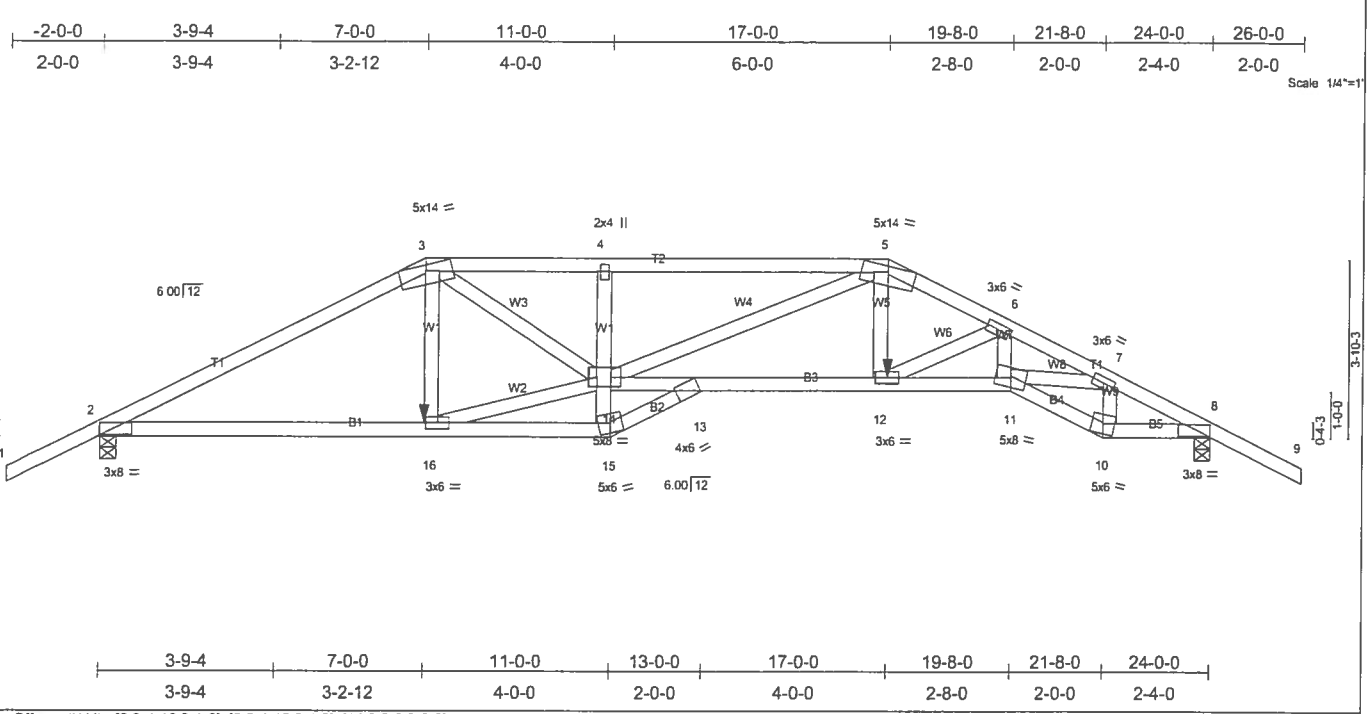

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [8:0-4-12,0-1-8], [14:0-2-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.25	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.40	12-13	>711	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 256 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-5-5 oc bracing.

REACTIONS (lb/size) 2=2208/0-4-0, 8=2203/0-4-0
Max Horz 2=115(load case 4)
Max Uplift2=-901(load case 4), 8=-901(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-4040/1484, 3-4=-5744/2156, 4-5=-5773/2160, 5-6=-5500/2060, 6-7=-6985/2494, 7-8=-4009/1415, 8-9=0/47
BOT CHORD 2-16=-1303/3515, 15-16=-256/432, 13-15=-188/262, 13-14=-1628/4772, 12-13=-1799/5051, 11-12=-2101/6057, 10-11=-1235/3609, 8-10=-1188/3452
WEBS 3-16=-53/17, 3-14=-1017/2697, 14-15=-35/296, 4-14=-558/315, 5-14=-323/887, 5-12=-579/1595, 6-12=-1217/432, 6-11=-396/1260, 7-11=-1030/2944, 7-10=-1477/541, 14-16=-1081/3184

JOINT STRESS INDEX
2 = 0.74, 3 = 0.46, 4 = 0.34, 5 = 0.74, 6 = 0.47, 7 = 0.72, 8 = 0.73, 10 = 0.47, 11 = 0.85, 12 = 0.52, 13 = 0.31, 14 = 0.91, 15 = 0.38 and 16 = 0.89

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 901 lb uplift at joint 2 and 901 lb uplift at joint 8.
8) Girder carries tie-in span(s): 4-4-11 from 11-0-0 to 17-0-0; 4-5-0 from 11-0-0 to 17-0-0
9) Girder carries hip end with 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 17-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-121(F=-68), 4-5=-104(F=-50), 5-9=-54, 2-16=-30, 15-16=-68(F=-38), 13-15=-81(F=-51), 13-14=-51(F), 12-13=-81(F=-51), 11-12=-30, 10-11=-30, 8-10=-30
Concentrated Loads (lb)
Vert: 16=-539(F) 12=-539(F)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061130

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T06	SPECIAL	1	1	Job Reference (optional)

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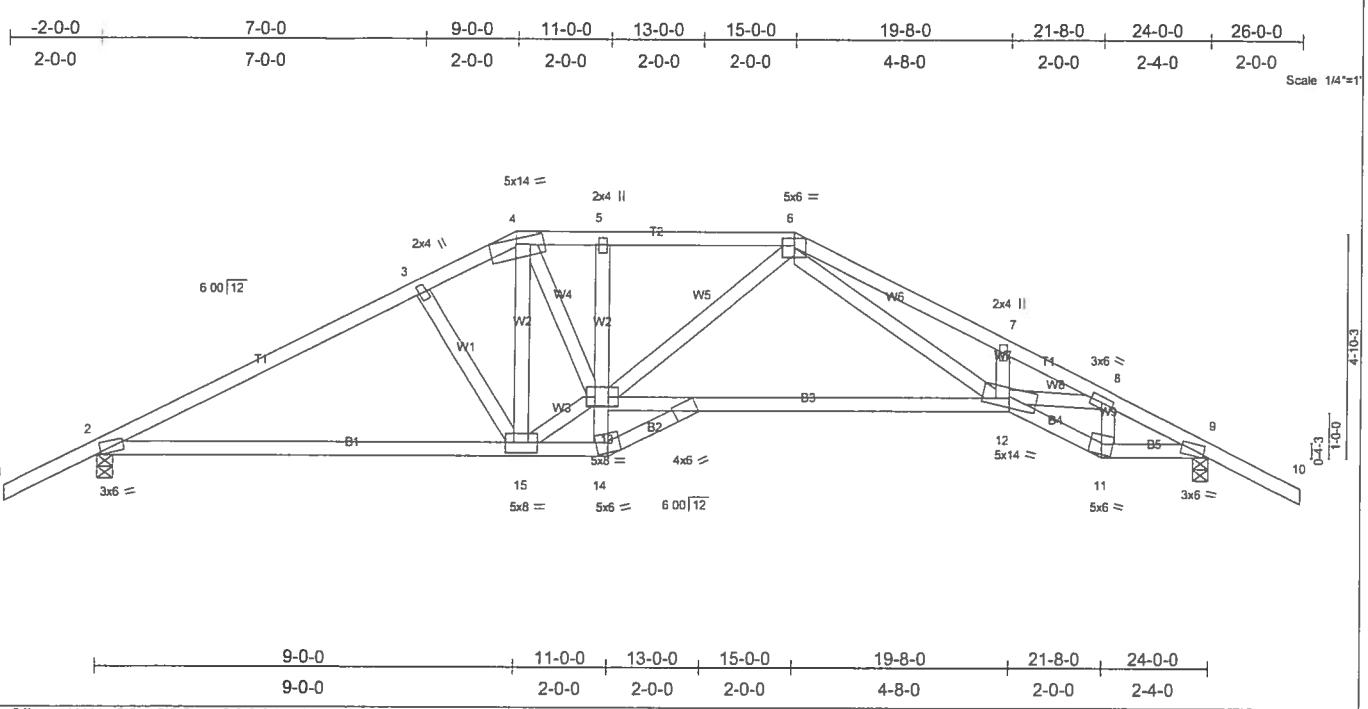

Plate Offsets (X,Y): [2:0-1-0,0-0-7], [6:0-3-0,0-2-0], [9:0-1-4,0-0-7], [13:0-2-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.32	12-13	>876	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.54	12-13	>524	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 133 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 9=1109/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift2=-431(load case 5), 9=-431(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1623/452, 3-4=-1426/437, 4-5=-1600/471, 5-6=-1602/470, 6-7=-3083/781, 7-8=-3079/679, 8-9=-1709/418, 9-10=0/47
BOT CHORD 2-15=-345/1376, 14-15=-9/14, 12-13=-273/1490, 11-12=-303/1556, 9-11=-299/1455
WEBS 3-15=-322/251, 4-15=-161/90, 13-14=-99/0, 5-13=-185/142, 6-12=-360/1578, 7-12=-147/190, 8-12=-240/1371, 8-11=-734/135, 4-13=-215/809, 13-15=-232/1352, 6-13=-81/267

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.42, 5 = 0.34, 6 = 0.78, 7 = 0.34, 8 = 0.67, 9 = 0.78, 11 = 0.49, 12 = 0.71, 13 = 0.71, 14 = 0.20, 15 = 0.63 and 16 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 2 and 431 lb uplift at joint 9.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061131

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:10 2006 Page 1

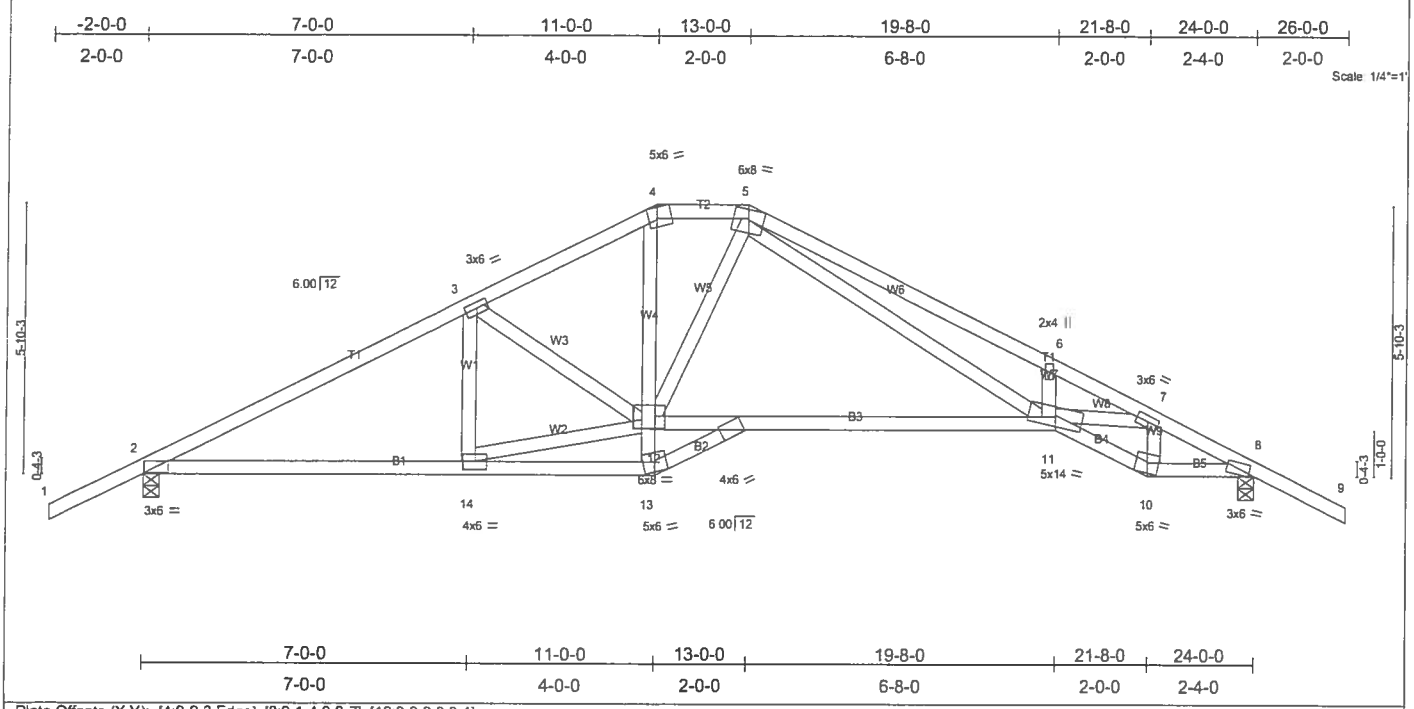

Plate Offsets (X,Y): [4:0-2-3,Edge], [8:0-1-4,0-0-7], [12:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.36	11-12	>790	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.60	11-12	>475	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 135 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=-115(load case 6)
Max Uplift2=-444(load case 5), 8=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1667/473, 3-4=-1460/457, 4-5=-1247/436, 5-6=-3171/913, 6-7=-3108/764, 7-8=-1702/436, 8-9=0/47
BOT CHORD 2-14=-377/1409, 13-14=-7/33, 11-12=-243/1261, 10-11=-320/1540, 8-10=-316/1443
WEBS 3-14=-139/126, 12-13=0/34, 4-12=-224/621, 5-11=-528/1845, 6-11=-255/277, 7-11=-321/1424, 7-10=-707/128, 3-12=-243/172, 12-14=-382/1418, 5-12=-143/151

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.52, 5 = 0.58, 6 = 0.34, 7 = 0.70, 8 = 0.78, 10 = 0.45, 11 = 0.65, 12 = 0.85, 13 = 0.20, 14 = 0.65 and 15 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 2 and 444 lb uplift at joint 8.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061132

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industnes, Inc. Thu Sep 28 09:19:11 2006 Page 1

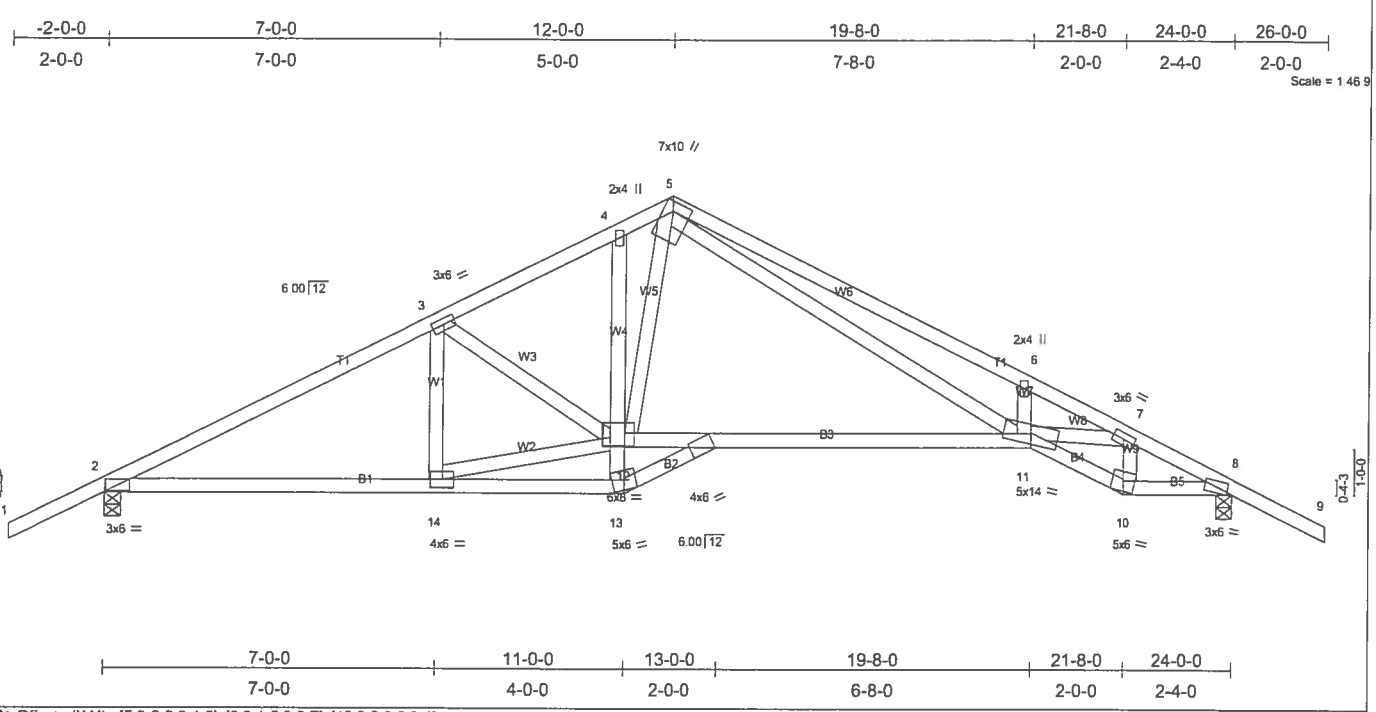

Plate Offsets (X,Y): [5:0-2-8,0-4-8], [8:0-1-5,0-0-7], [12:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.38	11-12	>745	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.63	11-12	>449	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-12 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=122(load case 5)
Max Uplift2=-450(load case 6), 8=-450(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1668/485, 3-4=-1459/473, 4-5=-1278/455, 5-6=-3207/977, 6-7=-3129/806, 7-8=-1701/444, 8-9=0/47
BOT CHORD 2-14=-395/1411, 13-14=-7/32, 11-12=-237/1180, 10-11=-331/1534, 8-10=-327/1439
WEBS 3-14=-140/131, 3-12=-259/167, 5-11=-608/1949, 6-11=-298/318, 7-11=-364/1452, 7-10=-694/123, 12-13=0/34, 4-12=-172/282, 5-12=-103/541, 12-14=-400/1421

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.34, 5 = 0.90, 6 = 0.34, 7 = 0.71, 8 = 0.78, 10 = 0.44, 11 = 0.63, 12 = 0.85, 13 = 0.20, 14 = 0.65 and 15 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 450 lb uplift at joint 8.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061133

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T09	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:12 2006 Page 1

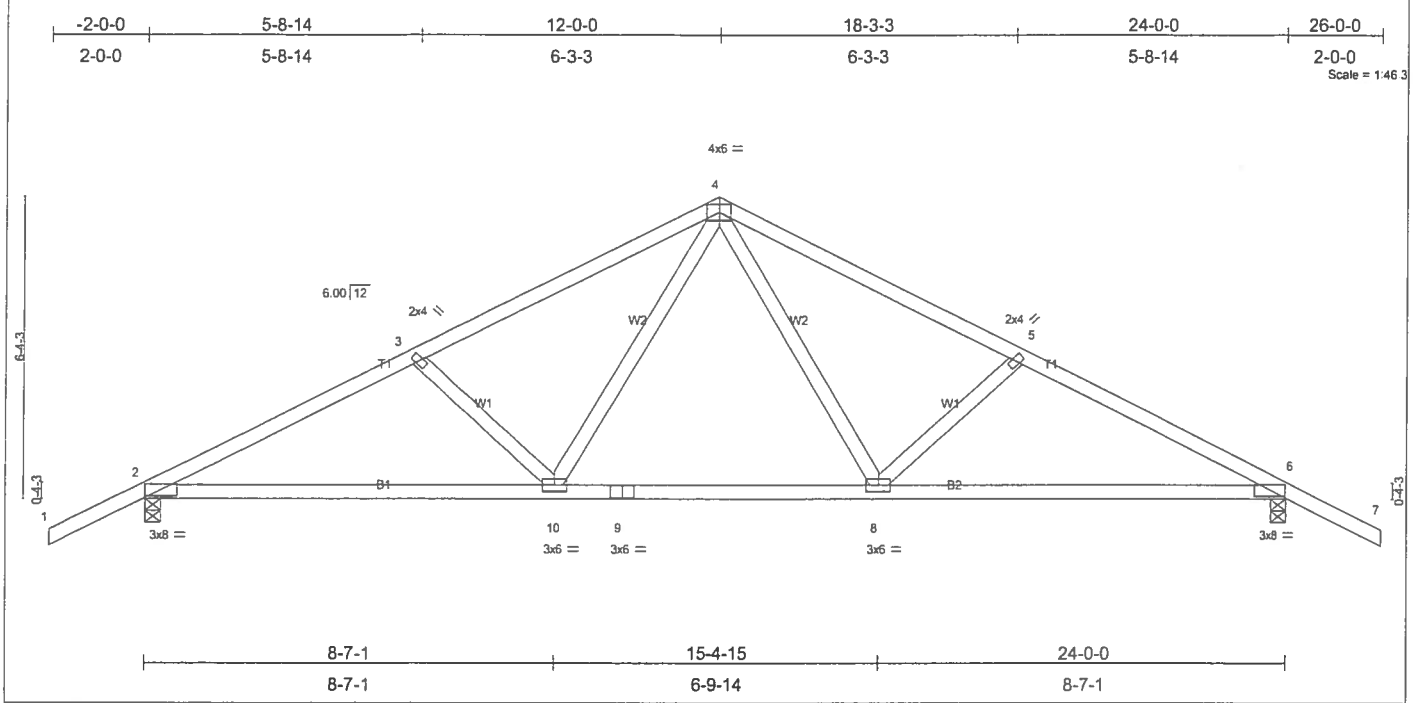

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [6:0-8-0,0-0-6]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.15	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.25	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-3-15 oc bracing.

REACTIONS (lb/size) 2=1111/0-4-0, 6=1111/0-4-0
Max Horz 2=122(load case 5)
Max Uplift2=-449(load case 5), 6=-449(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1668/532, 3-4=-1448/470, 4-5=-1448/471, 5-6=-1668/532, 6-7=0/47
BOT CHORD 2-10=-455/1442, 9-10=-194/980, 8-9=-194/980, 6-8=-344/1442
WEBS 3-10=-321/261, 4-10=-147/521, 4-8=-148/521, 5-8=-321/261

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.62, 5 = 0.34, 6 = 0.78, 8 = 0.44, 9 = 0.39 and 10 = 0.44

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 2 and 449 lb uplift at joint 6.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#092806l134

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T10	COMMON	1	2	Job Reference (optional)

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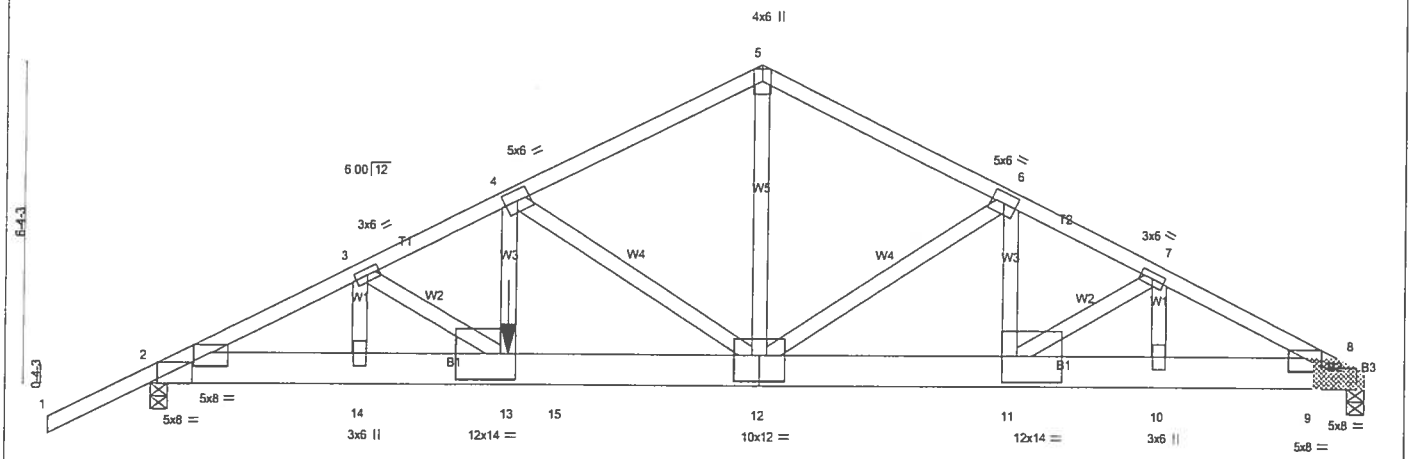
-2-0-0	4-1-6	7-0-12	12-0-0	16-11-4	19-10-10	24-0-0
2-0-0	4-1-6	2-11-6	4-11-4	4-11-4	2-11-6	4-1-6

Scale = 1:43.5



4-1-6	7-0-12	12-0-0	16-11-4	19-10-10	24-0-0
4-1-6	2-11-6	4-11-4	4-11-4	2-11-6	4-1-6

Plate Offsets (X,Y): [2:0-10-0,0-3-15], [2:0-1-11,Edge], [8:0-1-11,Edge], [8:0-10-0,0-3-15], [11:0-3-8,0-6-0], [12:0-6-0,0-6-0], [13:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.26	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.41	11-12	>690	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.70	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 343 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-11 oc bracing.

REACTIONS (lb/size) 8=7516/0-4-7 (0-4-0 + bearing block), 2=5892/0-4-0
Max Horz 2=155(load case 4)
Max Uplift8=-2786(load case 5), 2=-2256(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-11921/4317, 3-4=-12311/4552, 4-5=-8865/3326, 5-6=-8866/3315, 6-7=-12172/4527, 7-8=-13670/5052
BOT CHORD 2-14=-3899/10646, 13-14=-3899/10646, 13-15=-4032/10994, 12-15=-4032/10994, 11-12=-3928/10873, 10-11=-4481/12232, 9-10=-4481/12232, 8-9=-4481/12232
WEBS 3-14=-631/320, 3-13=-224/492, 4-13=-1251/3488, 4-12=-3777/1478, 5-12=-2780/7560, 6-12=-3628/1439, 6-11=-1211/3336, 7-11=-1634/662, 7-10=-485/1332

JOINT STRESS INDEX
2 = 0.81, 2 = 0.70, 3 = 0.49, 4 = 0.75, 5 = 0.87, 6 = 0.75, 7 = 0.49, 8 = 0.81, 8 = 0.00, 8 = 0.00, 8 = 0.70, 9 = 0.00, 9 = 0.00, 9 = 0.00, 10 = 0.22, 11 = 0.34, 12 = 0.72, 13 = 0.34 and 14 = 0.22

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) 2 X 8 SYP No.1D bearing block 12" long at jt. 8 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2786 lb uplift at joint 8 and 2256 lb uplift at joint 2.
8) Girder carries tie-in span(s): 28-0-0 from 8-0-0 to 24-0-0
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2658 lb down and 1004 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-15=-30, 8-15=-576(F=-546)
Concentrated Loads (lb)
Vert: 13=-2658(F)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061135

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T11	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:14 2006 Page 1

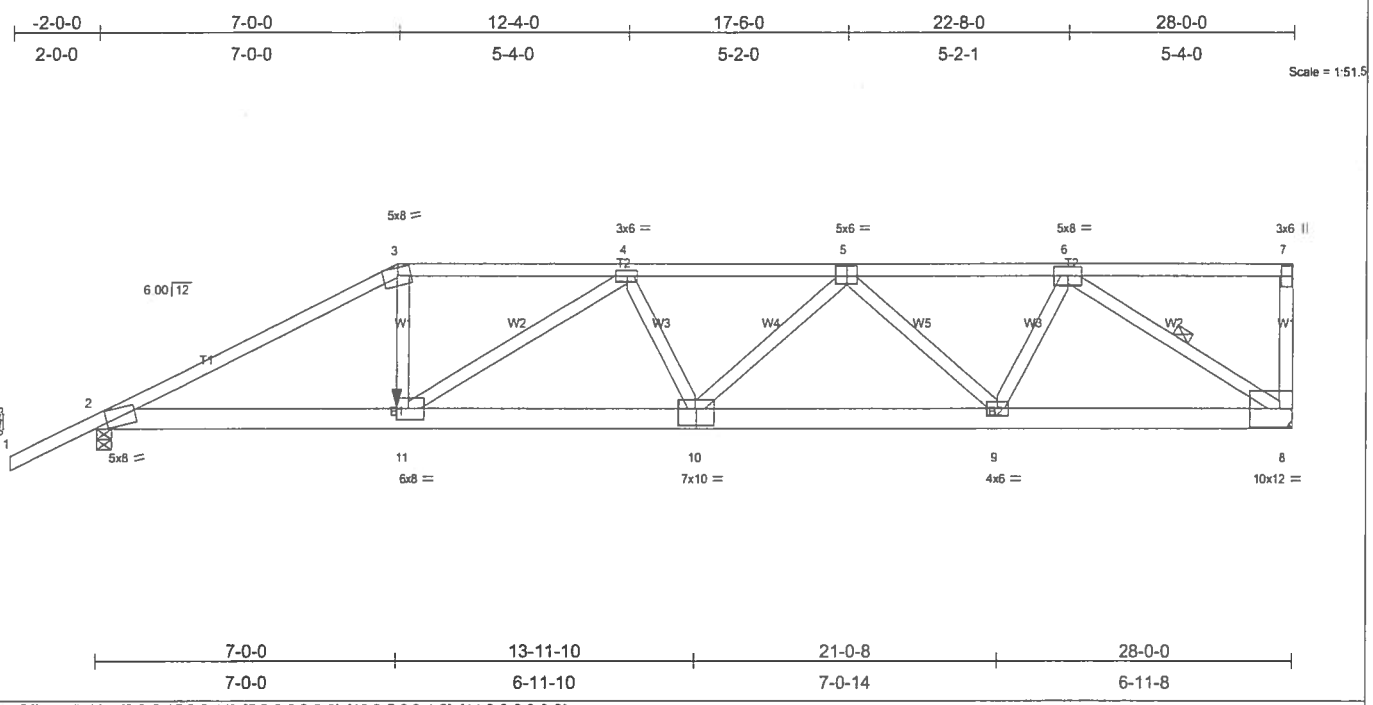


Plate Offsets (X,Y): [2:0-2-15,0-0-14], [5:0-3-0,0-3-0], [10:0-5-0,0-4-8], [11:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.29	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.47	10-11	>708	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-7 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=2658/Mechanical, 2=2508/0-4-0
Max Horz 2=228(load case 4)
Max Uplift8=-1202(load case 3), 2=-1079(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4792/2035, 3-4=-4273/1882, 4-5=-5413/2382, 5-6=-3981/1731, 6-7=-118/59, 7-8=-306/240
BOT CHORD 2-11=-1858/4205, 10-11=-2455/5369, 9-10=-2311/5015, 8-9=-1523/3287
WEBS 3-11=-594/1610, 4-11=-1319/745, 4-10=0/182, 5-10=-111/559, 5-9=-1447/813, 6-9=-489/1635, 6-8=-3814/1762

JOINT STRESS INDEX
2 = 0.93, 3 = 0.90, 4 = 0.46, 5 = 0.82, 6 = 0.94, 7 = 0.64, 8 = 0.66, 9 = 0.94, 10 = 0.88 and 11 = 0.44

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1202 lb uplift at joint 8 and 1079 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-122(F=-67), 2-11=-30, 8-11=-68(F=-38)
Concentrated Loads (lb)
Vert: 11=-539(F)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

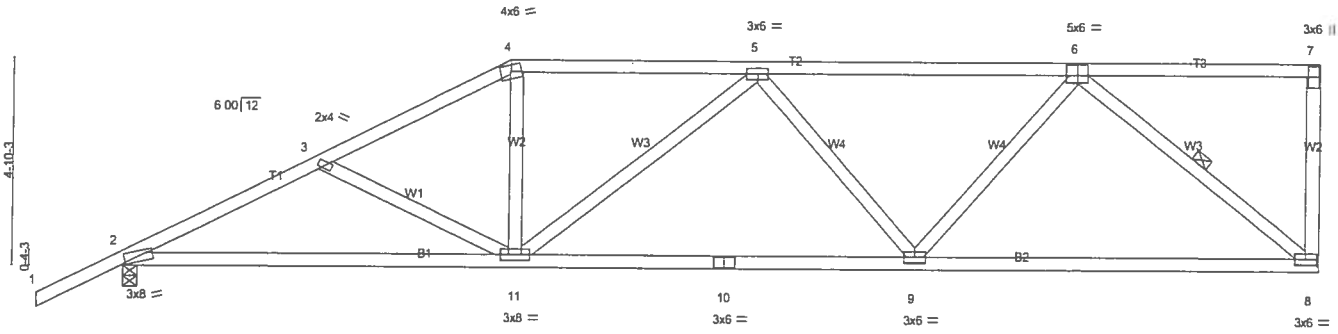
Dwg.#0928061136

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T12	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:15 2006 Page 1

-2-0-0 | 4-7-15 | 9-0-0 | 14-8-14 | 22-3-2 | 28-0-0
2-0-0 | 4-7-15 | 4-4-1 | 5-8-14 | 7-6-4 | 5-8-14

Scale = 1:51.5



9-0-0 | 18-6-0 | 28-0-0
9-0-0 | 9-6-0 | 9-6-1

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.18	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.29	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-9 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1158/Mechanical, 2=1284/0-4-0
Max Horz 2=272(load case 5)
Max Uplift8=-423(load case 4), 2=-459(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2057/588, 3-4=-1821/548, 4-5=-1598/521, 5-6=-1585/528, 6-7=-56/3, 7-8=-130/83
BOT CHORD 2-11=-635/1789, 10-11=-656/1776, 9-10=-656/1776, 8-9=-451/1156
WEBS 3-11=-232/199, 4-11=-95/530, 5-11=-230/221, 5-9=-299/200, 6-9=-121/675, 6-8=-1418/578

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.52, 5 = 0.39, 6 = 0.52, 7 = 0.29, 8 = 0.77, 9 = 0.46, 10 = 0.82 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 423 lb uplift at joint 8 and 459 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

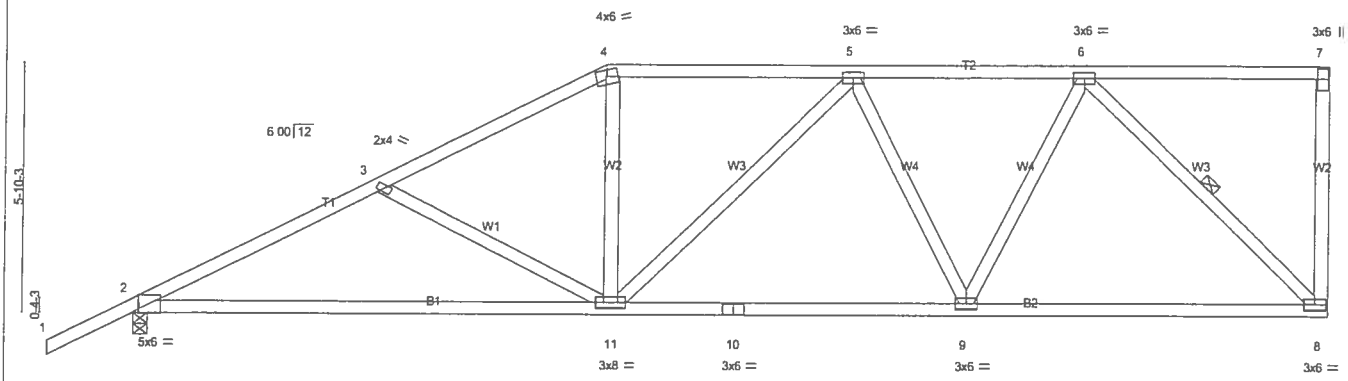
Dwg.#0928061137

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T13	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:16 2006 Page 1

-2-0-0	5-8-2	11-0-0	16-9-8	22-2-7	28-0-0
2-0-0	5-8-2	5-3-14	5-9-8	5-4-15	5-9-9

Scale = 1:51.5



11-0-0	19-5-15	28-0-0
11-0-0	8-6-0	8-6-1

Plate Offsets (X,Y): [2:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.34	2-11	>987	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.58	2-11	>574	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1158/Mechanical, 2=1284/0-4-0
Max Horz 2=318(load case 5)
Max Uplift8=-415(load case 4), 2=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1987/574, 3-4=-1670/474, 4-5=-1444/457, 5-6=-1231/412, 6-7=-43/7, 7-8=-147/96
BOT CHORD 2-11=-682/1730, 10-11=-496/1399, 9-10=-496/1399, 8-9=-354/947
WEBS 3-11=-336/266, 4-11=-40/422, 5-11=-65/118, 5-9=-383/191, 6-9=-133/649, 6-8=-1268/486

JOINT STRESS INDEX
2 = 0.73, 3 = 0.34, 4 = 0.60, 5 = 0.46, 6 = 0.54, 7 = 0.31, 8 = 0.64, 9 = 0.54, 10 = 0.47 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 8 and 469 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061138

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:17 2006 Page 1

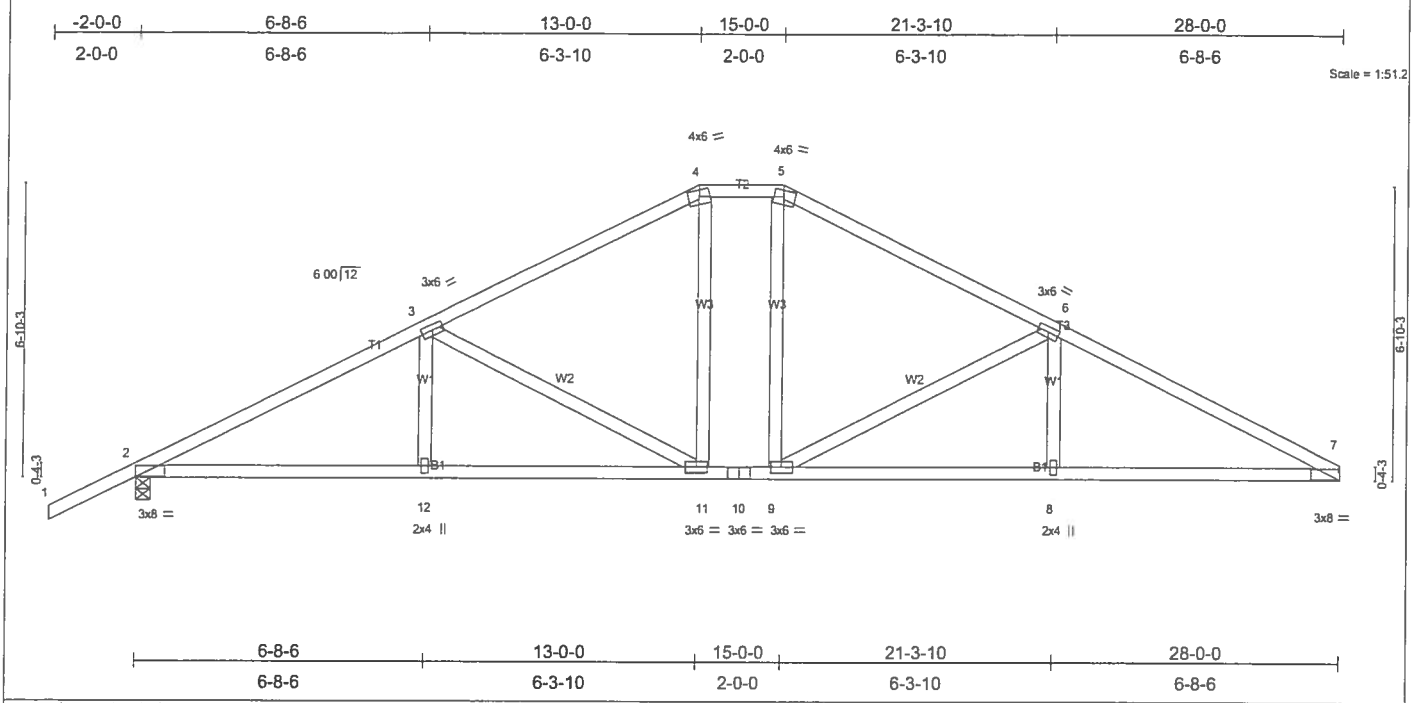


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [7:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.16	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.25	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-11 oc bracing.

REACTIONS (lb/size) 7=1162/Mechanical, 2=1288/0-4-0
Max Horz 2=154(load case 5)
Max Uplift7=-372(load case 6), 2=-501(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2084/611, 3-4=-1470/494, 4-5=-1250/483, 5-6=-1471/485, 6-7=-2130/662
BOT CHORD 2-12=-545/1784, 11-12=-545/1784, 10-11=-265/1250, 9-10=-265/1250, 8-9=-506/1835, 7-8=-506/1835
WEBS 3-12=0/237, 3-11=-656/318, 4-11=-112/406, 5-9=-114/419, 6-9=-712/370, 6-8=0/257

JOINT STRESS INDEX
2 = 0.78, 3 = 0.41, 4 = 0.55, 5 = 0.55, 6 = 0.41, 7 = 0.77, 8 = 0.34, 9 = 0.35, 10 = 0.53, 11 = 0.35 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 7 and 501 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061139

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T15	COMMON	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:18 2006 Page 1

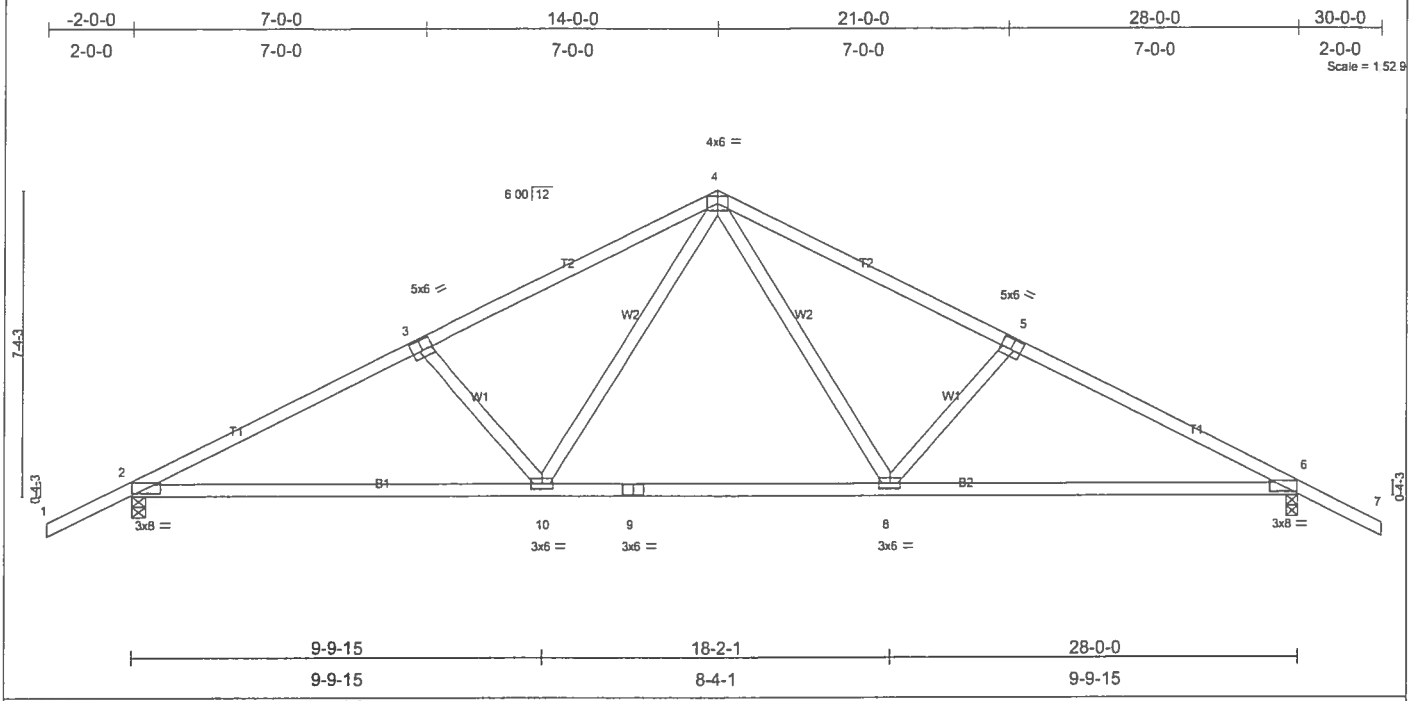


Plate Offsets (X,Y): [2:0-8-4,0-0-10], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.26	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.44	6-8	>748	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-15 oc bracing.

REACTIONS (lb/size) 2=1280/0-4-0, 6=1279/0-3-8
Max Horz 2=136(load case 5)
Max Uplift2=-504(load case 5), 6=-502(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1967/628, 3-4=-1737/584, 4-5=-1739/585, 5-6=-1971/630, 6-7=0/47
BOT CHORD 2-10=-540/1701, 9-10=-235/1150, 8-9=-235/1150, 6-8=-411/1705
WEBS 3-10=-373/308, 4-10=-207/657, 4-8=-209/660, 5-8=-376/310

JOINT STRESS INDEX
2 = 0.77, 3 = 0.63, 4 = 0.72, 5 = 0.63, 6 = 0.77, 8 = 0.52, 9 = 0.41 and 10 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 504 lb uplift at joint 2 and 502 lb uplift at joint 6.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061140

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T17	SPECIAL	1	1	Job Reference (optional)

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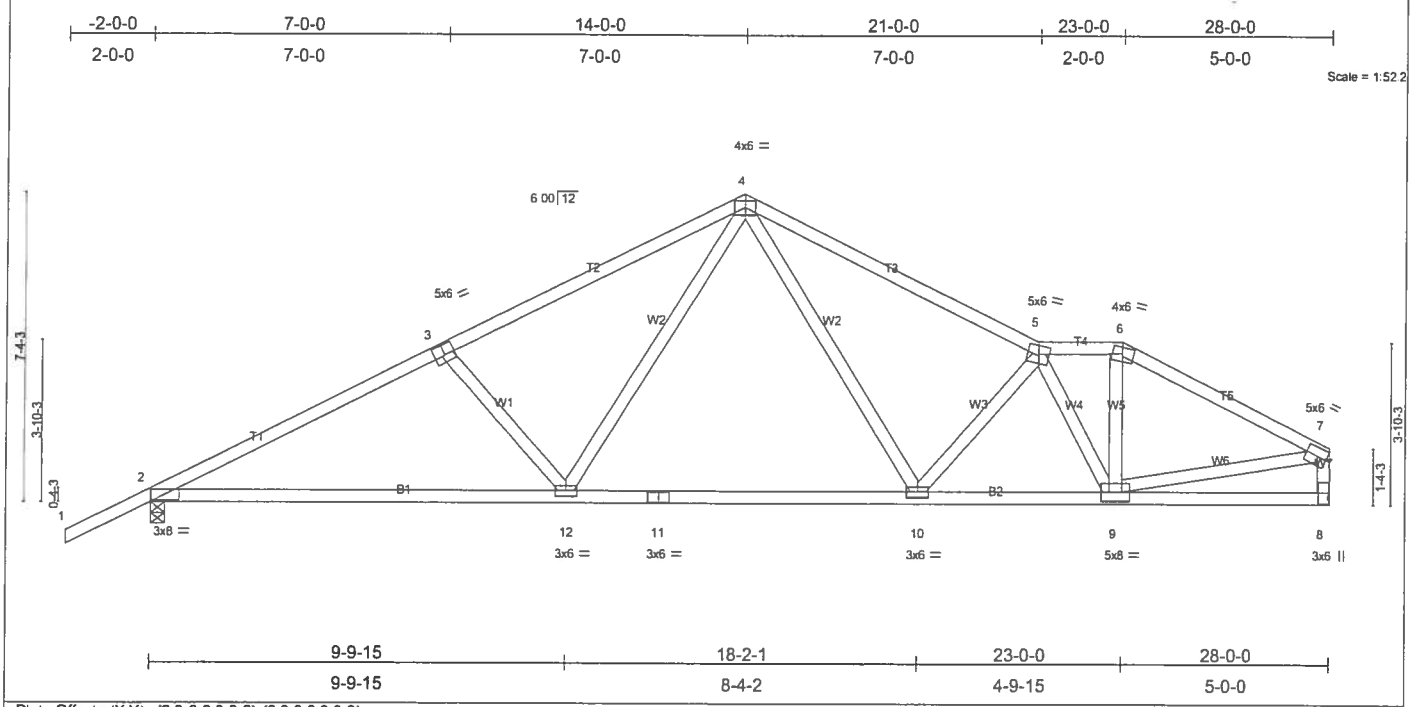


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.24	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.41	2-12	>813	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-12 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 8=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 8=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1982/629, 3-4=-1753/585, 4-5=-1712/605, 5-6=-1358/504, 6-7=-1560/508, 7-8=-1068/387
BOT CHORD 2-12=-601/1713, 11-12=-299/1157, 10-11=-299/1157, 9-10=-523/1717, 8-9=-96/206
WEBS 3-12=-368/307, 4-12=-203/676, 4-10=-225/624, 5-10=-436/297, 5-9=-775/284, 6-9=-155/549, 7-9=-297/1157

JOINT STRESS INDEX
2 = 0.78, 3 = 0.62, 4 = 0.75, 5 = 0.77, 6 = 0.32, 7 = 0.74, 8 = 0.36, 9 = 0.55, 10 = 0.49, 11 = 0.39 and 12 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 8.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061141

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:20 2006 Page 1

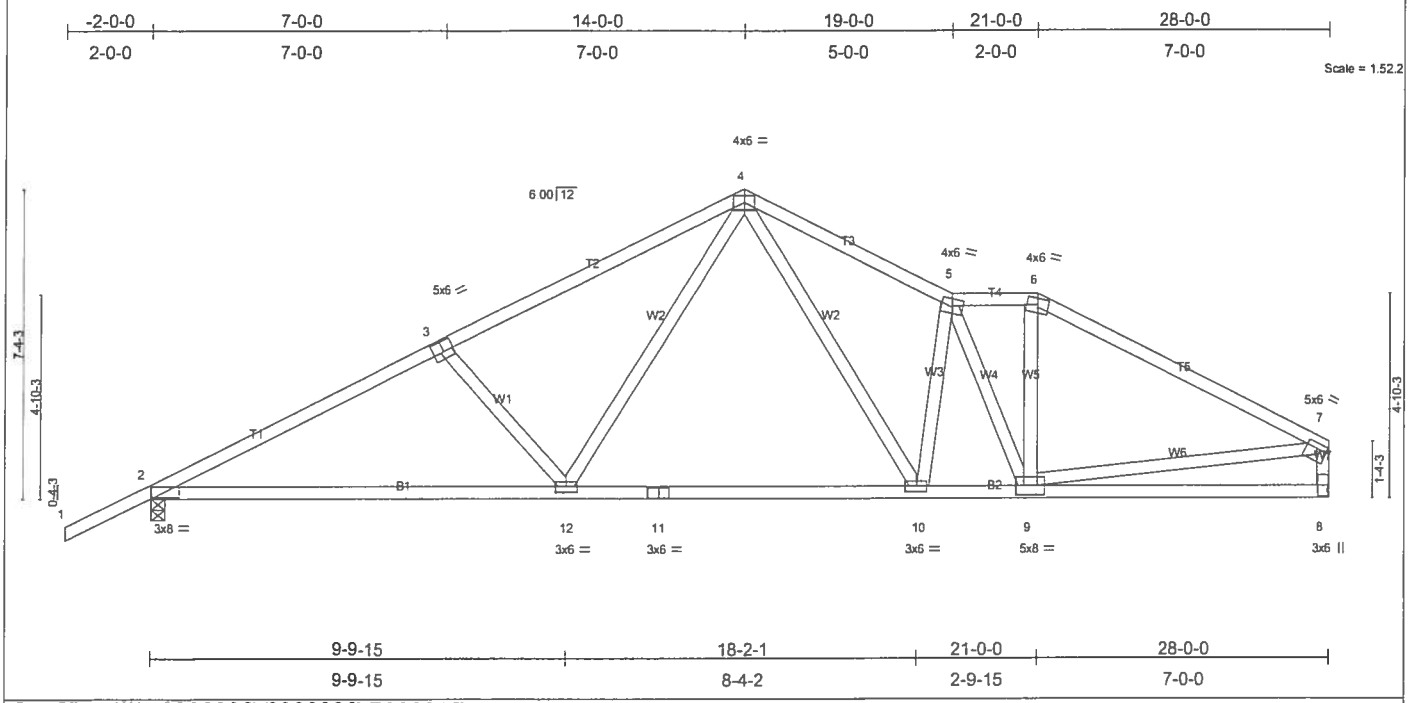


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-3-0,0-3-0], [7:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.24	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.40	2-12	>828	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-10 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 8=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 8=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1986/631, 3-4=-1754/586, 4-5=-1685/643, 5-6=-1375/534, 6-7=-1611/522, 7-8=-1038/395
BOT CHORD 2-12=-603/1717, 11-12=-297/1150, 10-11=-297/1150, 9-10=-404/1546, 8-9=-161/350
WEBS 3-12=-367/311, 4-12=-206/678, 4-10=-276/684, 5-10=-429/269, 5-9=-455/120, 6-9=-39/373, 7-9=-243/1025

JOINT STRESS INDEX
2 = 0.79, 3 = 0.64, 4 = 0.68, 5 = 0.43, 6 = 0.77, 7 = 0.77, 8 = 0.56, 9 = 0.47, 10 = 0.54, 11 = 0.39 and 12 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 8.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

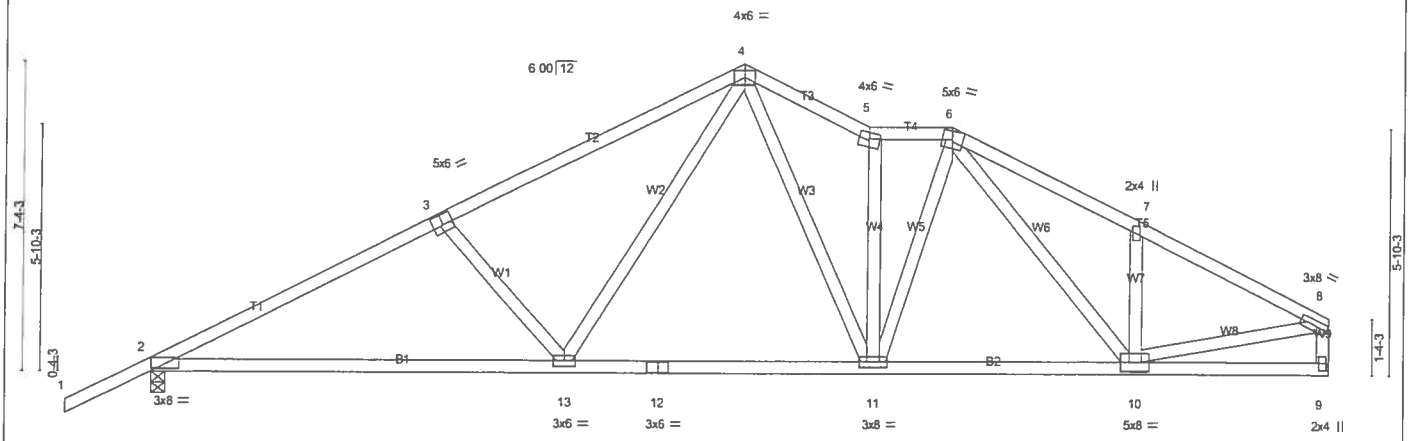
Dwg.#0928061142

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:21 2006 Page 1

-2-0-0	7-0-0	14-0-0	17-0-0	19-0-0	23-4-4	28-0-0
2-0-0	7-0-0	7-0-0	3-0-0	2-0-0	4-4-4	4-7-12

Scale = 1:52.2



9-9-15	17-0-0	23-4-4	28-0-0
9-9-15	7-2-1	6-4-4	4-7-12

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.25	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.43	2-13	>781	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 159 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 8-1-8 oc bracing: 2-13.

REACTIONS (lb/size) 2=1284/0-4-0, 9=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 9=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1981/631, 3-4=-1749/587, 4-5=-1552/607, 5-6=-1374/516, 6-7=-1543/619, 7-8=-1551/503, 8-9=-1088/383
BOT CHORD 2-13=-603/1714, 12-13=-296/1154, 11-12=-296/1154, 10-11=-309/1266, 9-10=-76/157
WEBS 3-13=-366/310, 4-13=-210/653, 4-11=-258/651, 5-11=-662/279, 6-11=-70/342, 6-10=-132/174, 7-10=-231/223, 8-10=-324/1213

JOINT STRESS INDEX
2 = 0.76, 3 = 0.64, 4 = 0.68, 5 = 0.73, 6 = 0.30, 7 = 0.34, 8 = 0.96, 9 = 0.82, 10 = 0.58, 11 = 0.76, 12 = 0.41 and 13 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 9.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061143

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T20	SPECIAL	1	1	Job Reference (optional)

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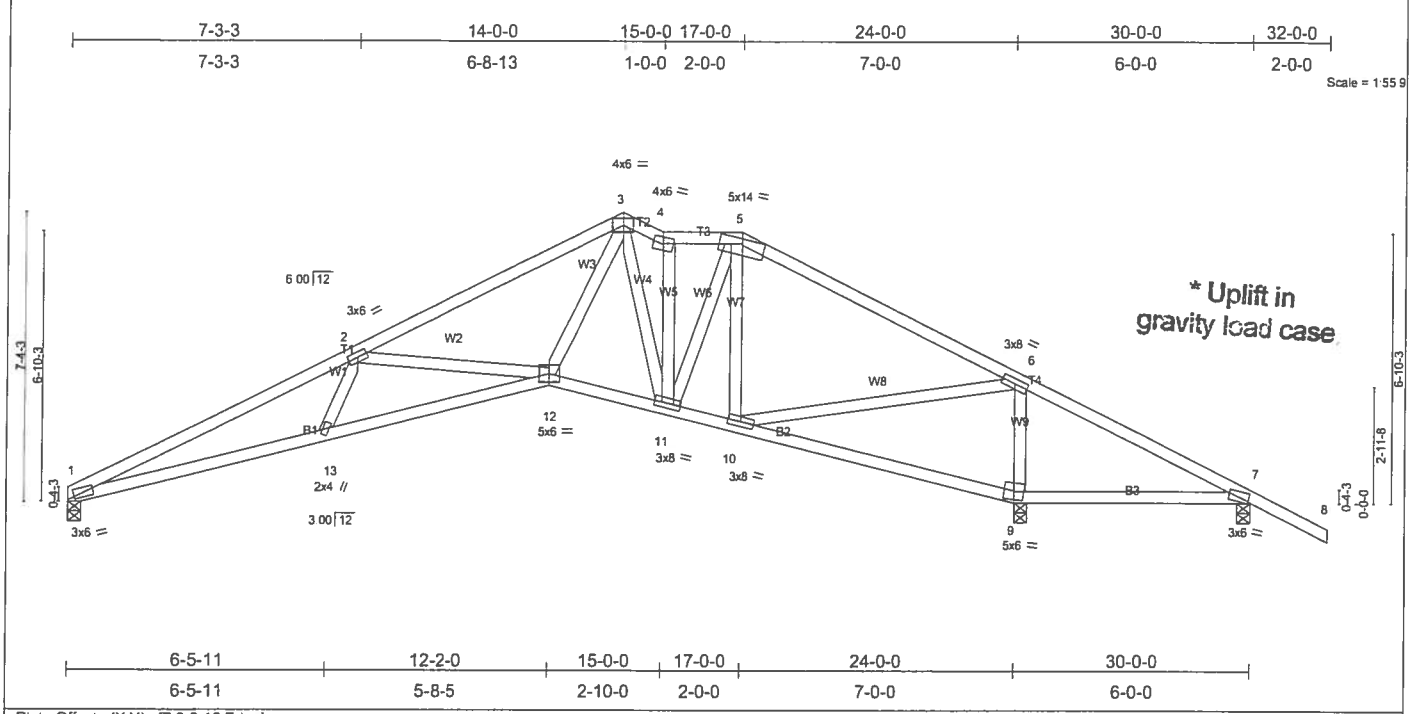

Plate Offsets (X,Y): [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.19	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.30	12-13	>941	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-15 oc bracing.

REACTIONS (lb/size) 1=846/0-4-0, 9=2013/0-4-0, 7=-250/0-4-0
Max Horz 1=-160(load case 6)
Max Uplift1=-287(load case 5), 9=-634(load case 5), 7=-379(load case 9)
Max Grav 1=846(load case 1), 9=2013(load case 1), 7=19(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2599/842, 2-3=-1532/451, 3-4=-729/265, 4-5=-734/290, 5-6=-722/266, 6-7=-365/1300, 7-8=0/47
BOT CHORD 1-13=-761/2331, 12-13=-763/2222, 11-12=-107/882, 10-11=-2/558, 9-10=-1217/421, 7-9=-1080/377
WEBS 2-13=0/259, 2-12=-866/527, 3-12=-270/1105, 3-11=-498/234, 4-11=-131/1, 5-11=-151/493, 5-10=-518/231, 6-10=-408/1741, 6-9=-1503/490

JOINT STRESS INDEX
1 = 0.80, 2 = 0.41, 3 = 0.92, 4 = 0.55, 5 = 0.92, 6 = 0.79, 7 = 0.83, 9 = 0.83, 10 = 0.76, 11 = 0.68, 12 = 0.81 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 1, 634 lb uplift at joint 9 and 379 lb uplift at joint 7.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061144

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T21	SPECIAL	6	1	Job Reference (optional)

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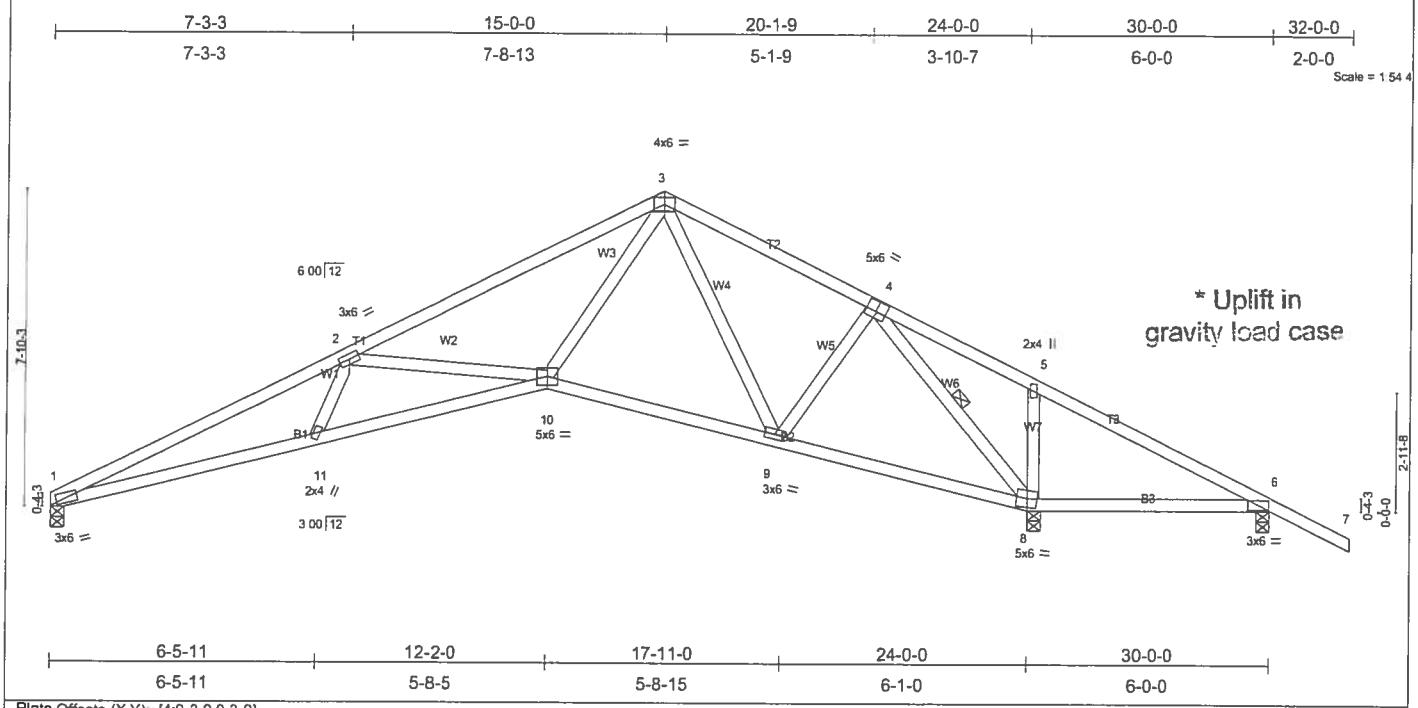


Plate Offsets (X,Y) [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.18	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.29	10-11	>990	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.17	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-0 oc bracing.
WEBS 1 Row at midpt 4-8

REACTIONS (lb/size) 1=832/0-4-0, 8=2084/0-4-0, 6=-307/0-4-0
Max Horz 1=-167(load case 6)
Max Uplift1=-282(load case 5), 8=-688(load case 5), 6=-443(load case 9)
Max Grav 1=832(load case 1), 8=2084(load case 1), 6=54(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2554/844, 2-3=-1463/426, 3-4=-443/242, 4-5=-349/1416, 5-6=-448/1452, 6-7=0/47
BOT CHORD 1-11=-772/2292, 10-11=-777/2189, 9-10=-41/659, 8-9=-218/166, 6-8=-1221/456
WEBS 2-11=0/248, 2-10=-912/568, 3-10=-288/1146, 3-9=-627/238, 4-9=-148/705, 4-8=-1937/514, 5-8=-315/243

JOINT STRESS INDEX
1 = 0.78, 2 = 0.41, 3 = 0.92, 4 = 0.54, 5 = 0.34, 6 = 0.69, 8 = 0.86, 9 = 0.51, 10 = 0.84 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 1, 688 lb uplift at joint 8 and 443 lb uplift at joint 6.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

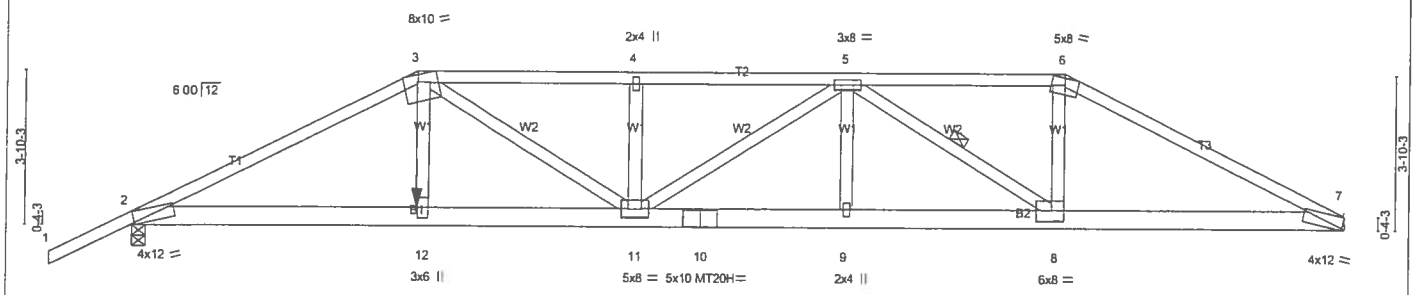
Dwg.#0928061145

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:24 2006 Page 1

-2-0-0	7-0-0	12-4-9	17-7-7	23-0-0	30-0-0
2-0-0	7-0-0	5-4-9	5-2-13	5-4-9	7-0-0

Scale = 1:54.5



7-0-0	12-4-9	17-7-7	23-0-0	30-0-0
7-0-0	5-4-9	5-2-13	5-4-9	7-0-0

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-4-3,Edge], [7:0-0-13,Edge], [8:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.40	9-11	>899	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.64	9-11	>561	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.60	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-6 oc bracing.
WEBS 1 Row at midpt 5-8

REACTIONS (lb/size) 7=2859/Mechanical, 2=2710/0-4-0
Max Horz 2=116(load case 4)
Max Uplift7=-1089(load case 5), 2=-1154(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5233/2170, 3-4=-6118/2635, 4-5=-6118/2636, 5-6=-4672/1922, 6-7=-5191/2069
BOT CHORD 2-12=-1932/4600, 11-12=-1941/4635, 10-11=-2532/6072, 9-10=-2532/6072, 8-9=-2532/6072, 7-8=-1799/4598
WEBS 3-12=-224/843, 3-11=-855/1862, 4-11=-661/546, 5-11=-66/99, 5-9=0/272, 5-8=-1797/889, 6-8=-602/1766

JOINT STRESS INDEX
2 = 0.78, 3 = 0.98, 4 = 0.34, 5 = 0.86, 6 = 0.98, 7 = 0.76, 8 = 0.49, 9 = 0.34, 10 = 0.86, 11 = 0.86 and 12 = 0.27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1089 lb uplift at joint 7 and 1154 lb uplift at joint 2.
7) Girder carries tie-in span(s): 7-0-0 from 23-0-0 to 30-0-0
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-121(F=-67), 6-7=-54, 2-12=-30, 8-12=-68(F=-38), 7-8=-135(F=-105)
Concentrated Loads (lb)
Vert: 12=-539(F)

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061146

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:25 2006 Page 1

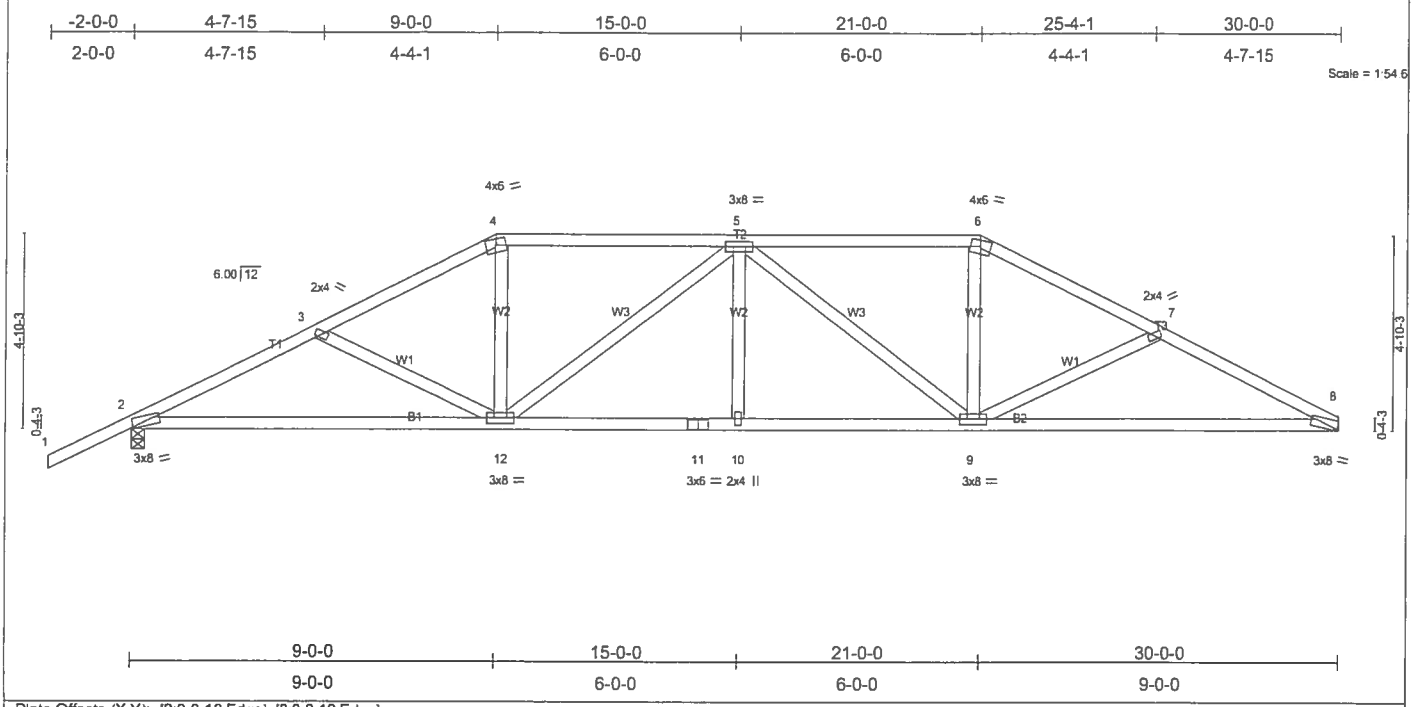


Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.23	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.38	8-9	>931	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-8 oc bracing.

REACTIONS (lb/size) 8=1246/Mechanical, 2=1372/0-4-0
Max Horz 2=126(load case 5)
Max Uplift8=-363(load case 6), 2=-491(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2233/628, 3-4=-2004/562, 4-5=-1766/545, 5-6=-1791/570, 6-7=-2034/593, 7-8=-2302/692
BOT CHORD 2-12=-548/1945, 11-12=-559/2071, 10-11=-559/2071, 9-10=-559/2071, 8-9=-557/2025
WEBS 3-12=-225/193, 4-12=-88/584, 5-12=-477/224, 5-10=0/125, 5-9=-455/213, 6-9=-101/604, 7-9=-286/243

JOINT STRESS INDEX
2 = 0.84, 3 = 0.34, 4 = 0.64, 5 = 0.57, 6 = 0.64, 7 = 0.34, 8 = 0.84, 9 = 0.57, 10 = 0.34, 11 = 0.72 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 8 and 491 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0928061147

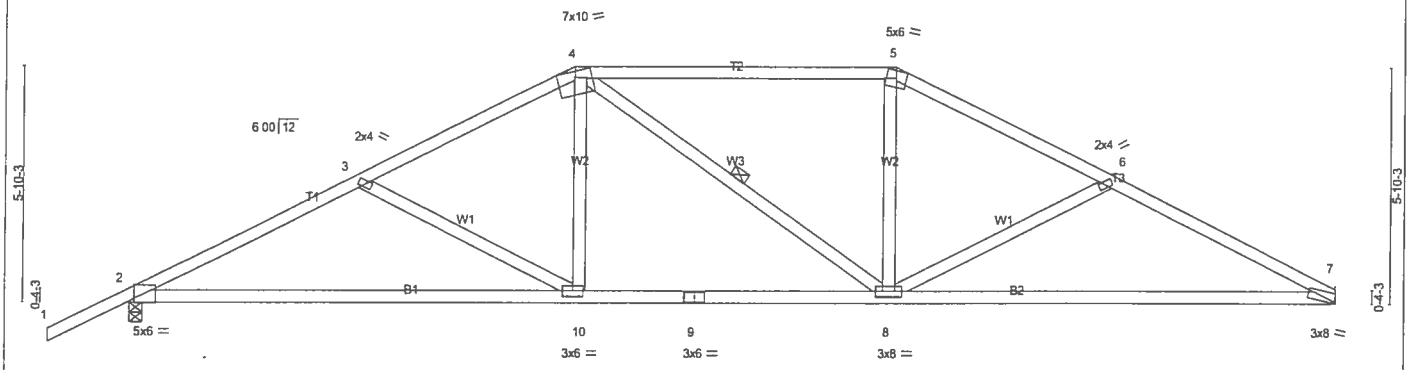
Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T24	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:26 2006 Page 1

-2-0-0 | 5-8-2 | 11-0-0 | 19-0-0 | 24-3-14 | 30-0-0

2-0-0 | 5-8-2 | 5-3-14 | 8-0-0 | 5-3-14 | 5-8-2

Scale = 1:54.6



11-0-0 | 19-0-0 | 30-0-0

11-0-0 | 8-0-0 | 11-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.43	7-8	>826	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.73	7-8	>487	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-2 oc bracing.
WEBS 1 Row at midpt 4-8

REACTIONS (lb/size) 7=1246/Mechanical, 2=1372/0-4-0
Max Horz 2=140(load case 5)
Max Uplift7=-380(load case 6), 2=-508(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2162/655, 3-4=-1863/531, 4-5=-1641/549, 5-6=-1882/553, 6-7=-2211/704
BOT CHORD 2-10=-575/1884, 9-10=-352/1625, 8-9=-352/1625, 7-8=-553/1942
WEBS 3-10=-305/253, 4-10=-57/423, 4-8=-133/161, 5-8=-61/447, 6-8=-353/295

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.76, 5 = 0.72, 6 = 0.34, 7 = 0.91, 8 = 0.57, 9 = 0.60 and 10 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 380 lb uplift at joint 7 and 508 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

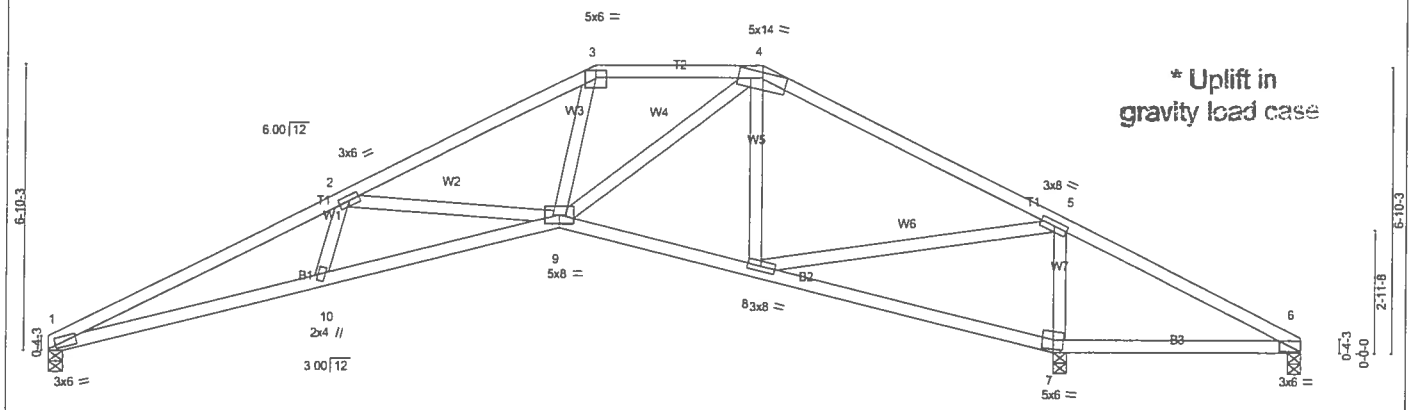
Dwg.#0928061148

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - DEMPSEY RES.
L212216	T25	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:19:27 2006 Page 1

7-1-8	13-0-0	17-0-0	24-0-0	30-0-0
7-1-8	5-10-8	4-0-0	7-0-0	6-0-0

Scale = 1:52.7



6-5-11	12-2-0	17-0-0	24-0-0	30-0-0
6-5-11	5-8-5	4-10-0	7-0-0	6-0-0

Plate Offsets (X,Y): [3:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.19	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	0.08	6-7	>901	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.16	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-15 oc bracing.

REACTIONS (lb/size) 1=847/0-4-0, 6=-385/0-4-0, 7=2030/0-4-0
Max Horz 1=96(load case 4)
Max Uplift1=-280(load case 5), 6=-447(load case 9), 7=-647(load case 5)
Max Grav 1=847(load case 1), 6=76(load case 5), 7=2030(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2606/839, 2-3=-1548/444, 3-4=-1213/436, 4-5=-719/220, 5-6=-383/1284
BOT CHORD 1-10=-778/2339, 9-10=-780/2262, 8-9=-46/577, 7-8=-1198/413, 6-7=-1060/369
WEBS 2-10=0/239, 2-9=-870/518, 3-9=-55/434, 4-9=-281/884, 4-8=-520/213, 5-8=-405/1716, 5-7=-1507/501

JOINT STRESS INDEX
1 = 0.80, 2 = 0.46, 3 = 0.72, 4 = 0.84, 5 = 0.78, 6 = 0.46, 7 = 0.83, 8 = 0.75, 9 = 0.71 and 10 = 0.34

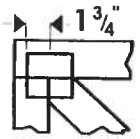
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 1, 447 lb uplift at joint 6 and 647 lb uplift at joint 7.

LOAD CASE(S) Standard

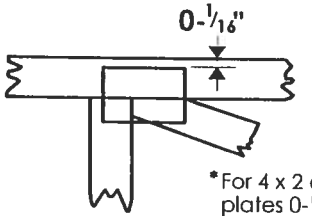
SEPTEMBER 28, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

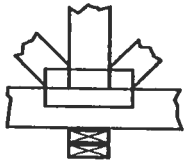
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

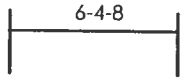


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

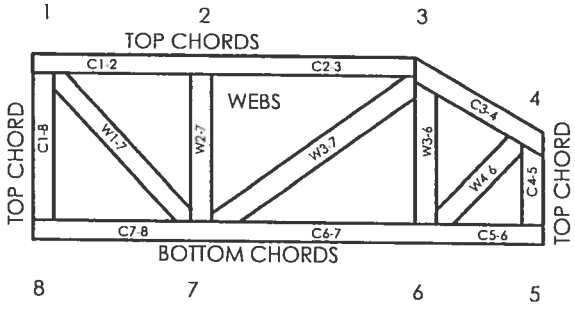
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.