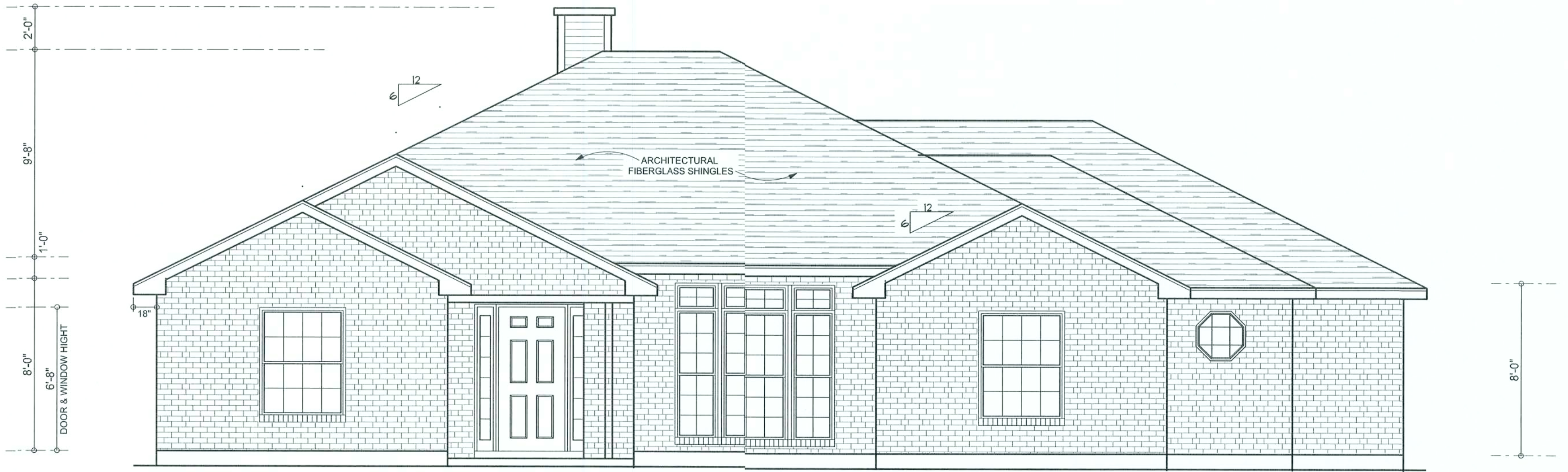
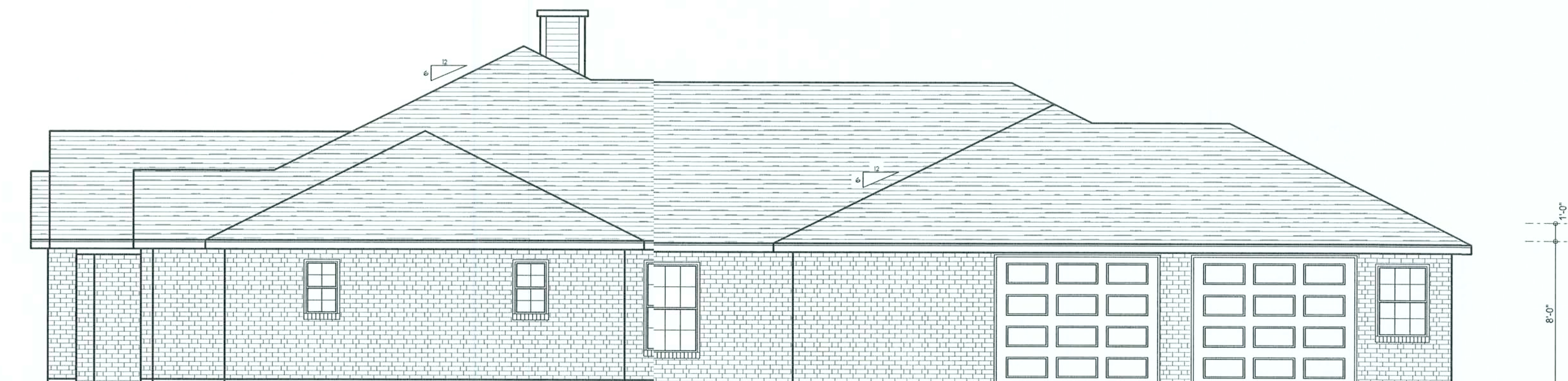




FRONT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 3/16" = 1'-0"



RIGHT ELEVATION
SCALE: 3/16" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS	
10 Apr 03	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
33056, 386-754-5419

CERTIFICATION: These plans and
"Windload Engineering", Sheet S-1, attached,
comply with Florida Building Code 2001,
Section 1606 wind loads, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location, permitted within
90 days of signature date. In case of conflict,
structural requirements, scope of work, and
builder responsibilities on sheet S-1 control.

MARK DISOSWAY
P.E. 53915

Mark Disosway
09 APR 03
SEAL

RICHARD & RENA
SCAFF
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Email: mdpe@bellsouth.net

PRINTED DATE:
April 10, 2003

DRAWN BY: Evan Beamsley CHECKED BY:

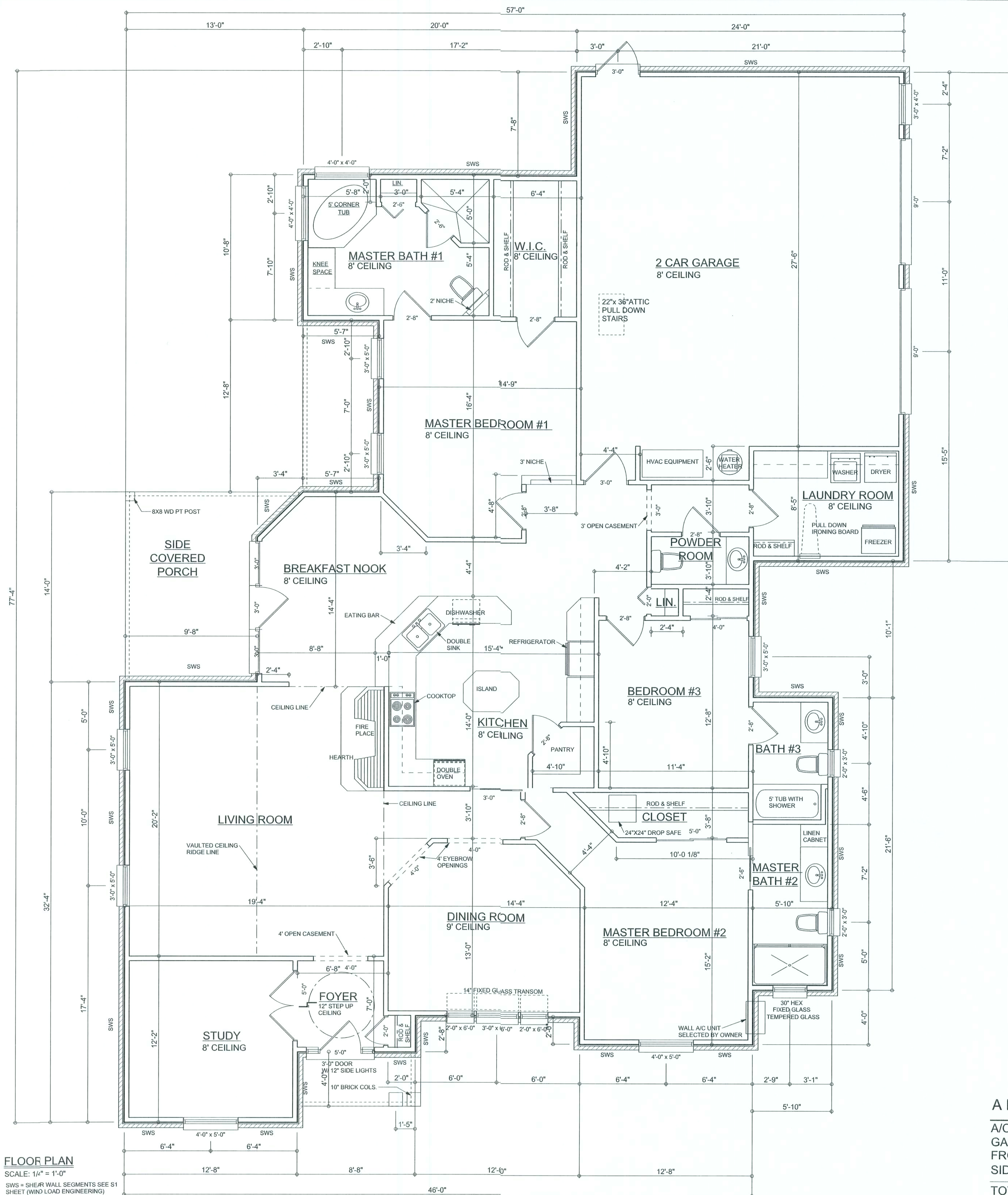
DESIGNED BY:
Teena Ruffo

FINALS DATE:
27 / MAR / 03

JOB NUMBER:
302132

DRAWING NUMBER
A-1

OF 5 SHEETS



FLOOR PLAN
SCALE: 1/4" = 1'-0"
SWS = SHEET WALL SEGMENTS SEE S1 SHEET (WIND LOAD ENGINEERING) FOR TOTAL REQUIRED & ACTUAL SHEAR WALL.

AREA SUMMARY

A/C LIVING AREA	2386	S . F .
GARAGE AREA	706	S . F .
FRONT PORCH AREA	36	S . F .
SIDE PORCH AREA	197	S . F .
TOTAL AREA	3325	S . F .

REVISIONS

10 Apr 03

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway
P.E. No. 53915, POB 865, Lake City, FL
32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering" Sheet S-1, attached, comply with Florida Building Code 2001, Section 1606 wind loads, to the best of my knowledge.

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Evan Beamsley

CHECKED BY:
Teena Ruffo

DESIGNED BY:
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27 / MAR / 03

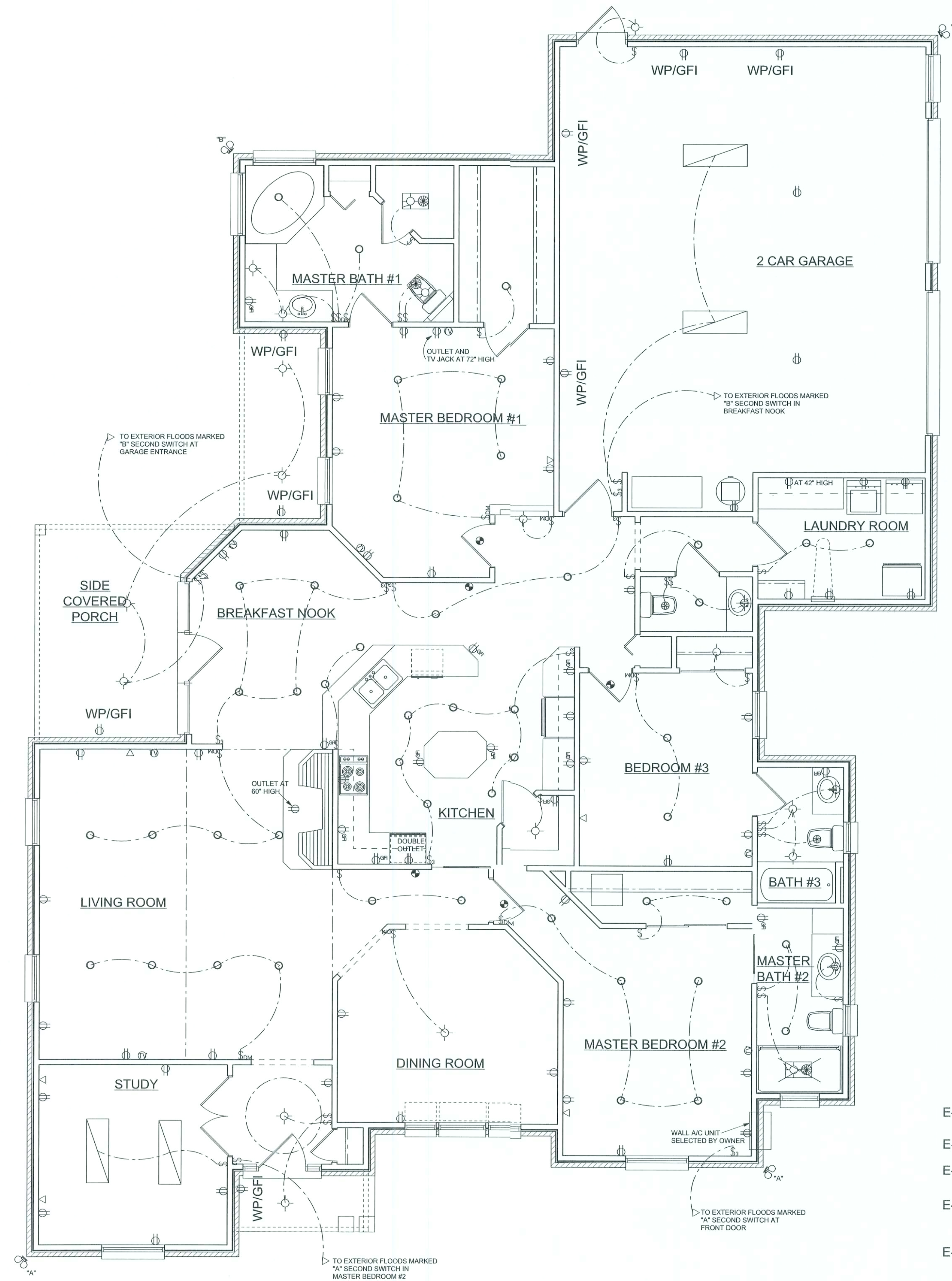
JOB NUMBER:
302132

DRAWING NUMBER
A-2

OF 5 SHEETS

REVISIONS

10 Apr 03

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE


ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2x4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	DIMMER SWITCH
	WP/GFI WATER PROOF GFI OUTLET
	PHONE
	TELEVISION

ELECTRICAL PLAN NOTES

- E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E-6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E-8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

WINDLOAD ENGINEER: Mark Disosway,
FE No. 55915, P.O. Box 868, Lake City, FL
32066, 386-754-5419

CERTIFICATION: These plans and
"Windload Engineering", Sheet S-1, attached,
comply with Florida Building Code 2001,
Section 1606 wind loads, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location, permitted within
90 days of signature date. In case of conflict,
structural requirements, scope of work, and
builder responsibilities on sheet S-1 control.

MARK DISOSWAY
P.E. 55915

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PRINTED DATE:

April 10, 2003

DRAWN BY:

Evan Beamsley

CHECKED BY:

TEENA RUFFO

DESIGNED BY:

TEENA RUFFO

FINALS DATE:

27 / MAR / 03

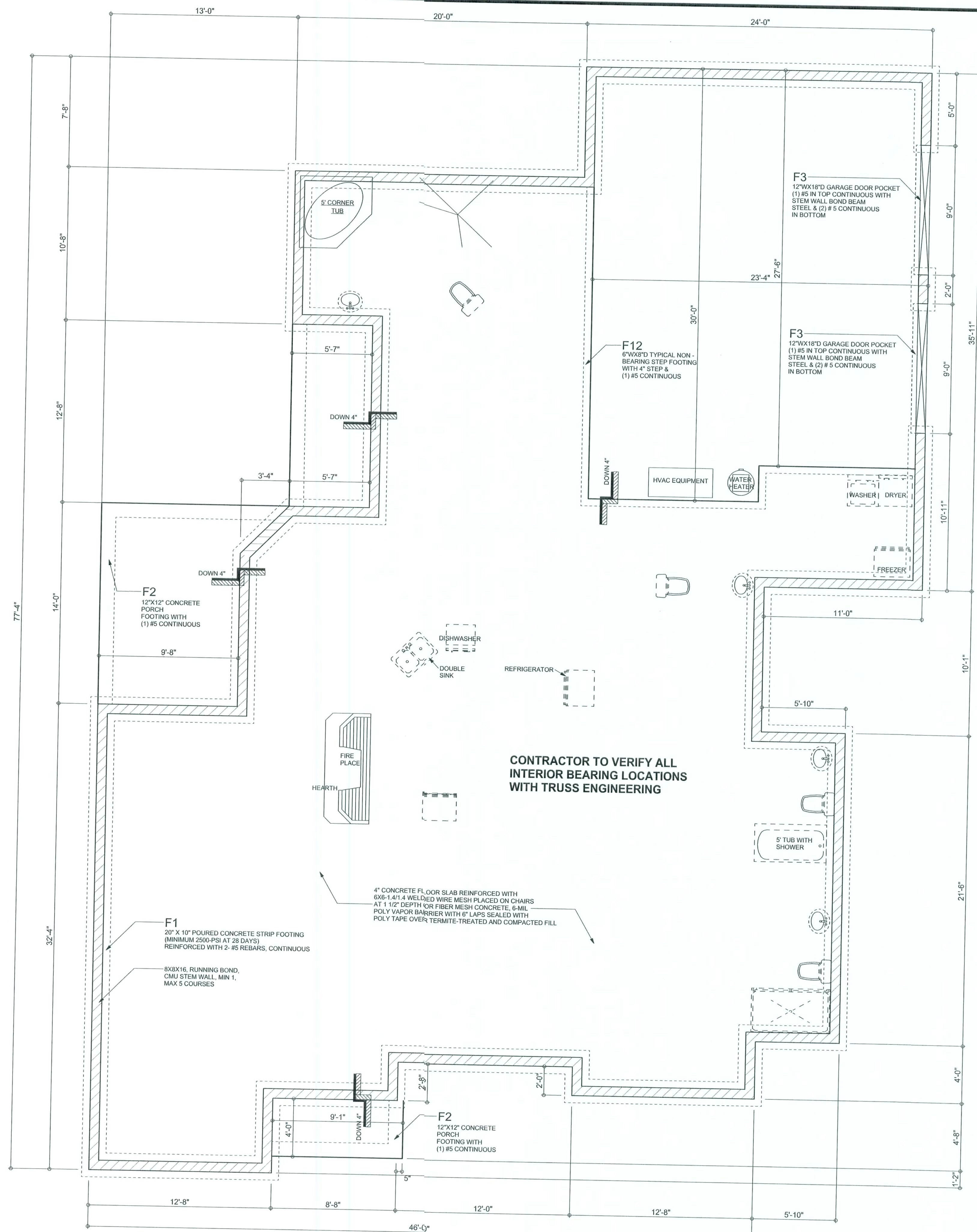
JOB NUMBER:

302132

DRAWING NUMBER

A-3

OF 5 SHEETS



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

REVISIONS
10 Apr 03

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway,
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32056, 386-754-5419

CERTIFICATION: These plans and
"Windload Engineering", Sheet S-1, attached,
comply with Florida Building Code 2001,
Section 1606 wind loads, to the best of my
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structural requirements, scope of work, and
builder responsibilities on sheet S-1 control.

MARK DISOSWAY
P.E. 53915

Mark Disosway
09 APR 2003
SEAL

**RICHARD & RENA
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April 10, 2003

DRAWN BY: Evan Beamsley

CHECKED BY:

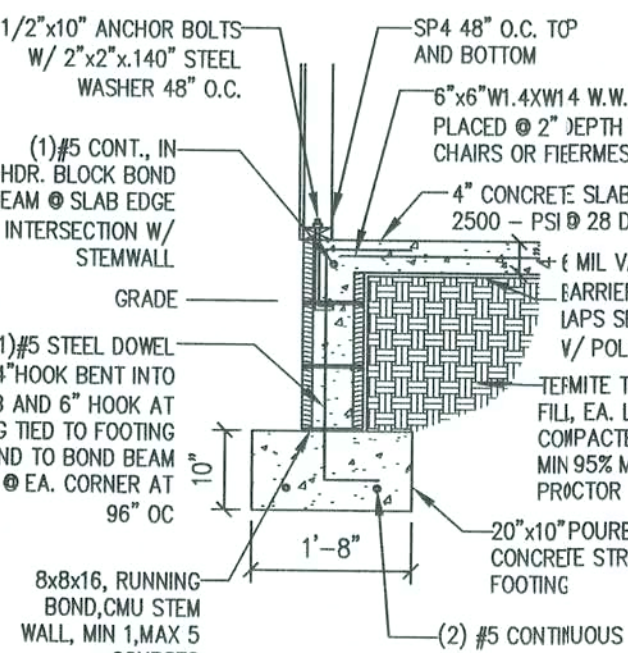
DESIGNED BY:
Teena Ruffo

FINAL DATE:
27 / MAR / 03

JOB NUMBER:
302132

DRAWING NUMBER
A-4

OF 5 SHEETS



6" x 6" W6 x 4 W4 W.M. PLACED @ 2" DEPTH ON CHAIRS OR FIREBRICKS

4" CONCRETE SLAB 2500 - PSI @ 28 DAYS

HOUSE SLAB

6 MIL VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE

TERMITE TREATED FILL, EA. LIFT COMPACTED TO MIN 95% MOD. PROCTOR

(1) #5 CONTINUOUS

SLOPE PORCH SLAB TO DRAIN

1'-0"

1'-0"

Diagram illustrating the cross-section of a concrete slab on grade, showing various layers and dimensions:

- Garage Door Pocket:** Indicated at the top left.
- Reinforcement:**
 - (1) #5 CONTINUOUS W/ STEEL WALL BOND BEAM STEEL
 - (2) #5 CONTINUOUS W/ STEEL WALL BOND BEAM STEEL
- Concrete Slab:** 4" CONCRETE SLAB '2500 - PSI @ 28 DAYS
- Dimensions:**
 - 6" x 6" W. 4X14" W. W.N. PLACED @ 2" DEPTH ON CHAIRS OR FIBERESH
 - 8"
 - 1'-6"
 - 1'-0"
- Barrier and Sealant:** 6 MIL VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TIE
- Fill Material:** TERMITE TREATED FILL, EA. LIFT COMPACTED TO MIN 95% MOD. PROCTOR

PRE-ENGINEERED WOOD ROOF TRUSSES 8'-2" O.C.

SELECT TRUSS CONNECTORS FROM ANCHOR TABLE PER TRUSS UPLIFT LOADS

1/2" X 7" WEDGE ANCHORS - SPACING PER TABLE

4" CONCRETE SLAB 2500 - PSI AT 28 DAYS

2X4 / 2X6 STUDS AT 16" OC SPF #2

STRAP STUDS TOP AND BOTTOM W/ SP4 / SP6 SPACING PER TABLE

6" X 6" W/ 4X16/4 W.N.M. PLACED AT 2" DEPTH ON CHAIRS OR FIBERMESH

6 MIL VAPOR BARRIER WITH 6" LAP'S SEALED WITH POLY TAPE

TERMITE TREATED LUMBER, LEAF COMPACTED TO MIN. 95% MOD. PROCTOR

TYPICAL	WEDGE ANCHOR	SP4 / SP6	TRUSS
---------	--------------	-----------	-------

PRE-ENGINEERED WOOD ROOF TRUSSES @ 24" O.C.
 SELECT TRUSS CONNECTORS FROM ARCH TABLE PER TRUSS UPLIFT LOADS

2X4 / 2X6 STUDS AT 16" O.C.
 SPF #2

STRAP STUDS TOP AND BOTTOM
 W/ SP4 / SP6 SPACING PER TABLE

1/2" X 7" WEDGE ANCHORS SPACING PER TABLE

4" CONCRETE SLAB
 2500 - PSI AT 28 DAYS

6" X 6" W/ 6XW1.4 W.W.M. PLACED AT 2" DEPTH ON CHAIRS OR FIREBRICK

6 MIL. VAPOR BARRIER WITH 6" LAPS SEALED WITH POLY TAPE

TERMITE TREATED FILL, EACH LIFT COMPACTED TO MIN. 95% MOD. PROCTOR

TYPICAL	WEDGE ANCHOR	SP4 / SP6	TRUSS
---------	--------------	-----------	-------

NOTE:
TYPICAL STRAPPING (LINO)

NOTE:
OPENINGS TO 5" WIDTH USE (1) SP4 OPENINGS 5" TO 7" WIDTH USE (2) SP4

NOTE:
ANCHOR BOLTS MAY BE LOCATED AT EITHER SIDE OF KING STUDS. PLATE MUST BE CONT. BETWEEN BOLT AND KING STUDS

NOTE:
ANCHOR BOLTS MAY
BE LOCATED AT
EITHER SIDE OF KING
STUDS. PLATE MUST
BE CONT. BETWEEN
BOLT AND KING
STUDS

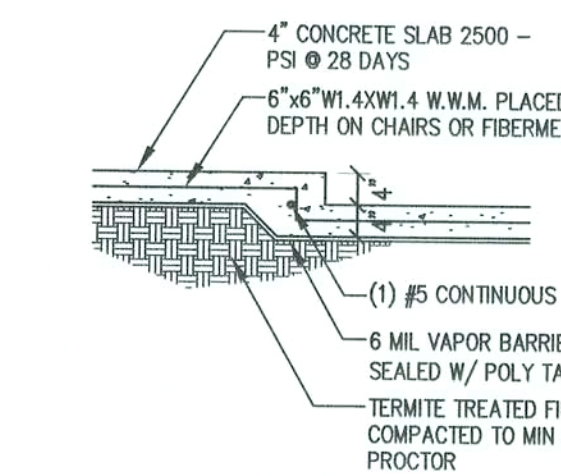
DETAIL

Load Bearing Header Sizing Methods (by Builder)			
1. Determine header size from FBCI 2001, Tables 2308.3 A, B, & C, or 2308.5.			
2. Calculate header size per allowable working stresses, FBCI 2001, 2301.3.1.			
3. Use engineering number beams use supplier published data & sizing software.			
Jack Studs and King Studs			
4. Lookup jack studs from FBCI 2001, Tables 2308.3 A, B, & C, or 2308.5.			
5. Use one jack stud for every 3000 lb vertical load.			
6. Total king plus jack studs = studs needed to be there if no opening was there			
Header 1/4" Connection			
7. Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header.			
8. Select header connections from table below or info. catalog to connect header to stud (top connection) and stud to foundation (bottom connection).			
Uplift, lb	Top connection	Bottom connection	
≤ 800	End nail or toe nail w/ail	SPA, 6-6	60%
800-1500	12d-12A	13T-13C	
1500-1750	12d-12A12	755 - 2-SPA, 6-10d-12T ¹ /AB	138
1750-2150	12d-12A18	1055 12T200-nail	175
2150-2500	12d-12A18	2110 12T18, 18-10d, 5"x10"AB	218
2500-3485	12d-12A18 16-10d	3480 HT116, 18-16d, 5"x10"AB	348
Uplift greater than 3485 lb requires engineering design.			

Header Spans (ft-in)	Building Width / Truss Span (ft)			
	20		28	
	Span	NJ	Span	NJ
2-2x4	3-6	1	3-2	1
2-2x6	5-5	1	4-8	1
2-2x8	6-10	2	5-2	2
2-2x10	8-5	2	7-3	2
2-2x12	9-0	2	8-2	2
2-2x14	9-0	2	8-3	2
2-2x16	10-6	1	9-1	2
2-2x18	12-2	1	10-2	2
2-2x20	12-2	1	8-1	2
2-4x10	11-8	1	10-1	2
2-4x12	14-1	1	12-2	2

NOTES: NJ = Number of jacks
required to support each end.
Building width is measured
perpendicular to the header.
For widths between those shown, spans
may be interpolated. Spans are
based on uniform loads on header.

NOTES: NJ = Number of jack required to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, s may be interpolated. Spans are based on uniform loads on h



(1) #5 CONTINUOUS

- 4" CONCRETE SLAB 2500 - PSI @ 28 DAYS
- 6" x 6" W/ 4XW/1.4 W.W.M. PLACED @ 2" DEPTH ON CHAIRS OR FIBERMESH
- TERMITE TREATED FILL, E.A. LIFT COMPACTED TO MIN 95% MOD. PROCTOR
- 6 MIL VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE

Dimensions shown: 8'0" height, 6" width.

Diagram illustrating an alternative to balloon framing for gypsum ceiling construction, showing the assembly of trusses, bracing, and endwall studs.

Labels and specifications:

- 2 X 4 CONT. LATERAL BRACING AT 6' O.C. W/ 2-10d NAILS EA. TRUSS
- TRUSSES
- 2 X 4 BLOCK NAILED TO EA. BRACE W/ 4-10d NAILS
- GABLE END TRUSS
- 1/2 GYP. BOARD W/ 5d COOLER NAILS AT 10 IN. O.C.
- 5d COOLER NAILS AT 7 IN. O.C.
- SIMPSON LSTA30 STR. W/ 10-6d NAILS IN 2 X 4 BRACE & 10 d NAILS IN ENDWALL STUD
- ENDWALL STUDS

NOTE: EXTEND RAT RUN BRACES GABLE TO GABLE OR A LENGTH EQUAL TO 1/2 OF GABLE WIDTH FOR ROOFS UP TO 7:12 AND 5/8 OF GABLE WIDTH OVER 7:12

2X4 OUTRIGGER @ 48" OC.

BLOCKING REQUIRED BETWEEN OUTRIGGERS

2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END

7/16" STRUCTURAL ROOF SHEATHING

SIMPSON A35

2X4 BARGE RAFTER CONT.

HURRICANE CLIP H=2.5 OR EQUAL 48" OC.

TOP CHORD OF GABLE END TRUSS DROP 3 1/2"

NOTE: ALL MEMBERS SHALL BE SYP

CONT. 2X4 SCAB FROM TOP TO BOTTOM CHORD @ X-BRACING (PROVIDE ADDITIONAL 2X4'S @ VERTICAL IF HIGHER THAN 8' TO FORM AN "L" SHAPE.)

4 - 10d NAILS OR 4 - 1.31"x 3.25" TYPICAL AT ALL CONNECTIONS

2X4 SCAB IF VERT. WEB IS NOT PRESENT

CONT. 2X4X8" #2 SYP LATERAL BRACE @ 48" OC.

TOE NAIL TRUSS TO DOUBLE PLATE w/ 16d COM @ 8" OC.

BOTTOM CHORD OF GABLE END TRUSS

(2) - 2X4 TOP PLATE

SIMPSON LSTA 2x4 @ 48" OC.

2X4 STUDS @ 16" OC.

2X4 X-BRACE @ 8'-0" OC.

TYPICAL GABLE END (X-BRACING)

SCALE: 1/2"=1'-0"

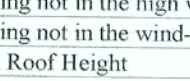
SY# 42 PT WD POSTS		
TYPICAL POST LIFE/FT	POST BASE ANCHOR	HEADER STRAPPING
555 LB	AB444 W/ (6)-10d & 3/8" AB	(2) LSTA21 W/ (6)-10d EA
720 LB	AB468 W/ (8)-10d & 3/8" AB	(2) LSTA21 W/ (8)-10d EA
2200 LB	AB144 W/ (12)-16d, (2) 3/8" BOLTS & 3/8" AB	(2) LSTA21 W/ (16)-10d EA
2300 LB	AB166 W/ (12)-16d, (2) 3/8" BOLTS & 3/8" AB	(2) LSTA21 W/ (16)-10d EA
HOLLOW COLUMN		
1500 LB	3/8" X 10" AB ATTACHED TO 3/8" THREADED ROD WITH 3/8" COUPLER THRU COLUMN & HEADER WITH 2" WASHER & NUT TOP	
2300 LB	3/8" X 10" AB ATTACHED TO 3/8" THREADED ROD WITH 3/8" COUPLER THRU COLUMN & HEADER WITH 3" WASHER & NUT TOP	

TRUSS UPLIFT CONNECTOR			
All connectors are Simpson Strongtie, uno. Select top and bottom plate or SST catalog to meet truss uplift. Use fasteners as specified.			
Uplift SPF	Uplift SYP	Truss Connector	To Plate
24S	350	H5A	3-8d
53S	600	H2.5A	5-8d

TRUSS UPLIFT CONNECTOR TABLE					
All connectors are Simpson Strong-Tie, unless noted. Use Select top and bottom connectors from the table to meet truss types and truss spacing. Use Flats as specified.					
Uplift Uplift	Truss	Use Flats	Use Flats	Use Flats	Use Flats
SFP	SFP	SFP	SFP	SFP	SFP
445	H15A	3-8d			
535	60 H25A	5-8d			
620	720 L12S12	6-10d $\times\frac{1}{2}$ "			
850	990 H16	8-8d $\times\frac{1}{2}$ "			
1245	1245 H16	10-10d or 12-10d $\times\frac{1}{2}$ "			
1260	1470 H16H16	10-10d or 12-10d $\times\frac{1}{2}$ "			
1785	2000 LG12	14-16d Skiner			
3565	4200 MG1	5/8" Thd. Rod			
5065	4200 MG2	5/8" Thd. Rod			
Uplift Uplift	Truss	Use Flats	Use Flats	Use Flats	Use Flats
SFP	SFP	SFP	SFP	SFP	SFP
760	885 SP4	6-10d $\times\frac{1}{2}$ "			
865	1005 CS20	9-8d or 7-10d			
1085	1215 L18AB-24				
1170	1360 SP24	12-10d $\times\frac{1}{2}$ "			
1420	1650 CS16	14-8d or 11-10d			
Uplift Uplift	Truss	Use Flats	Use Flats	Use Flats	Use Flats
SFP	SFP	SFP	SFP	SFP	SFP
1160	1350 L1T19	5/8"x16" AB			
1180	2310 L1T19	5/8"x16" AB			
2385	2725 HD24	5/8"x16" AB			
2590	4115 L1T16	5/8"x16" AB			
2590	4115 L1T16	5/8"x16" AB			

Manufacturer and product number are listed for example not endorsement. An equivalent device or name or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads. All connections exposed directly to the weather shall be hot dipped galvanized after fabrication. Leads are increased for wind duration. Strap uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes ($10d=84^{\circ}$ 16d, $10d\frac{1}{2}=80^{\circ}$ 10d, $10d=12d$ 16d similar). $SPF=86^{\circ}$ SPY.

Studs Supporting Trusses: The builder is responsible for gravity loads, but you should put an extra stud under truss bearing location for each 3000 lb of reaction. Check the minimum bearing requirements of the truss and top plate ($SPF, F_c=424psi=2230lb/ply$).

WIND LOAD DESIGN DATA			
(Wind loads are per FBC 2001, Section 1606.2 for enclosed multi-story buildings with mean roof height less than 60' or the least horizontal dimension, not sited on the upper half of an unbraced 60' high hill with >10% slope.)			
Basic Wind Speed	110 MPH		
Wind Exposure	B		
Wind Importance Factor	1.0		
Building Category	II		
Internal Pressure Coefficient	N/A (Enclosed)		
Building not in the high velocity hurricane zone			
Building not in the wind-borne debris region			
Mean Roof Height	< 30 ft		
Roof Angle	10-45 degrees		
Components And Cladding Wind Pressures (FBC Table 1606.2 B(2))			
	Zone	Effective Wind Area (A _{ef})	
		10	100
	1	19.9	-21.8
	2	19.9	-42.1
	3	19.9	-42.1
	4	21.8	-23.6
	5	21.8	-29.1
	2 0° hg	-40.6	
	3 0° hg	-68.3	
	4	21.8	-23.6
Total Shear Wall Segments			
Required	Transverse	75.00	
	Longitudinal		75.00

NOTE: Z IS MEASURED AS A HORIZONTAL PROJECTION ON THE BUILDING ROOF AND WALLS

WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is an engineer of record for compliance of the structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not engineer of record for the trusses and did not design the trusses or delegate to the truss designer.

BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part of the wind load engineer's scope of work.

- Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise).
- Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures.
- Provide continuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately.
- Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations.
- Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions, including interior bearing walls.
- Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads.

DOCUMENT CONTROL AND PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and is the responsibility of the builder.

WINDLOAD ENGINEER: Mark Dissoway, PE No.53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of the signature date.

Mark Disosway, PE
POB 868, Lake City, FL 32056 Phone: (386)754-5419
Fax: (386)754-6749 Email: windloadengineer@bellsouth.net

Location: Holly Hill Lot 13 (Columbia Co., FL)

**Richard & Rene Scaff
Residence**

Builder: Norton Builders

Designer: Teena Ruffo

Approved: FLPE#53915	Revisions: 10 APR 03
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[illegible]

 27MAR03	Sheet S-1 Windload Engineering
	302132