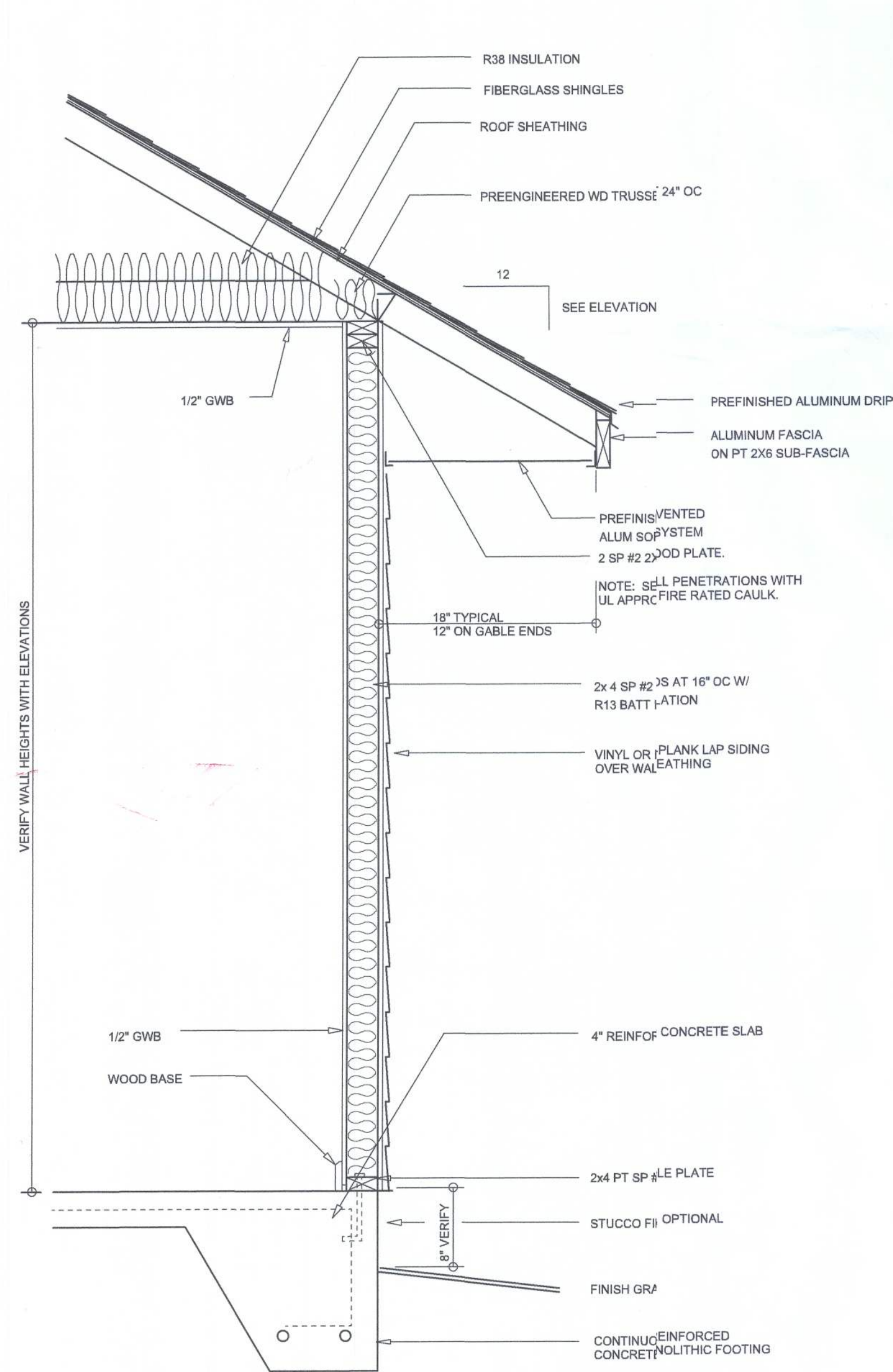




LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



TYPICAL WALL SECTION
SCALE: 1" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

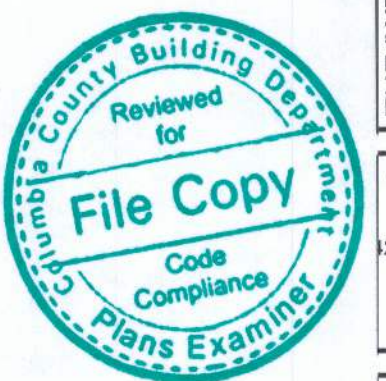
REVISIONS
January 27, 2023

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS
SCALE: 1/4" = 1'-0"

TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

THE WESTERN MODEL CUSTOM FOR:
LOT 20, HILLS OF ROSE CREEK
PROJECT ADDRESS: Parcel: 05-55-17-09116-120, Lake City, Florida 32024 (Columbia County)
GIBALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA



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LAKE CITY, FL 32025
(386) 758-6406
wm@wmdesigns.net

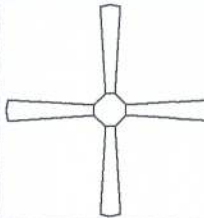
















JOB NUMBER
20230127

SHEET NUMBER

A.1

Wm C. [Signature]

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	CIRCUIT FOR MINI-SPLIT A/C UNIT
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL INTERIOR RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER
NEC 406.11

ALL INTERIOR & EXTERIOR LIGHTING SHALL MEET OR EXCEED THE MIN. 75% HIGH-CLENCY
LIGHTING PER FBC-ENERGY CONSERVATION R404.

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR
AND SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEA
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE 2017 (N/A) NATIONAL
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
January 27, 2023

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

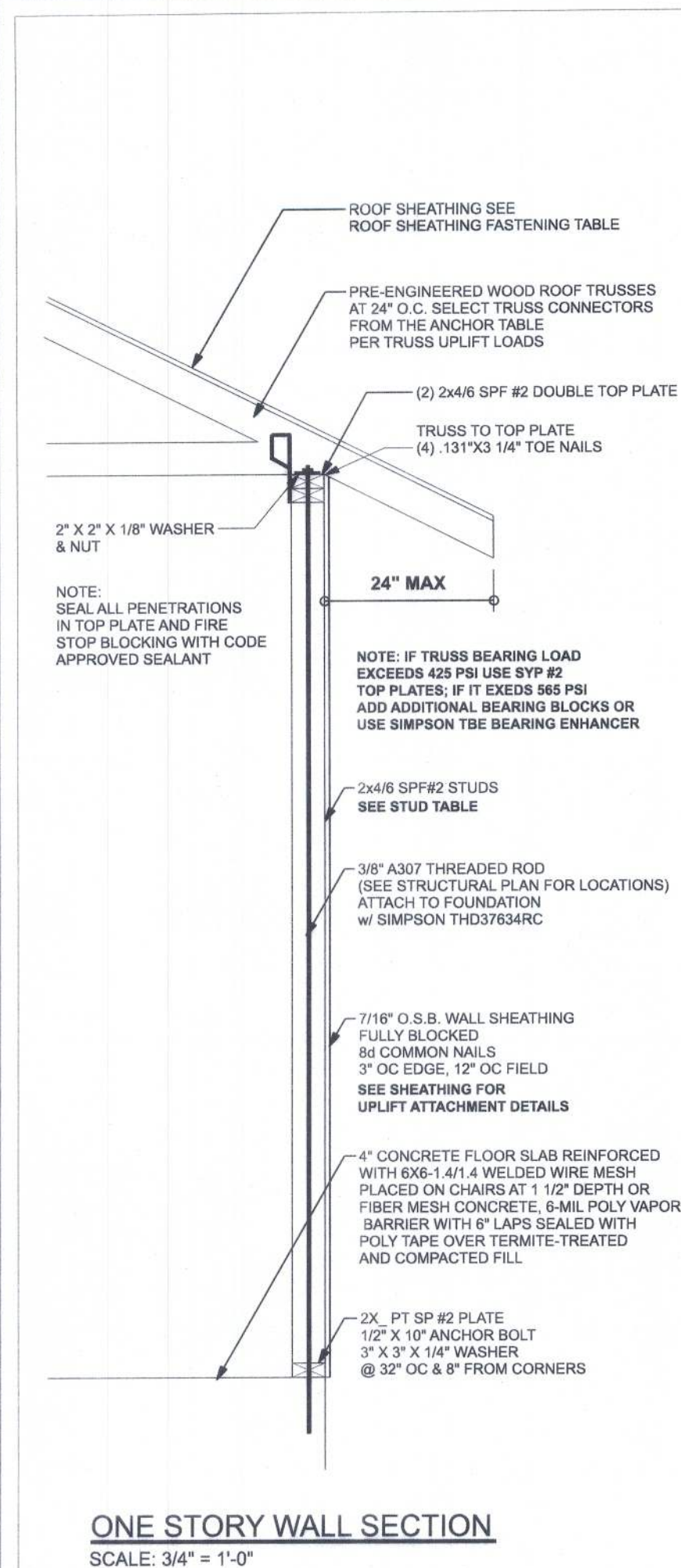
THE WESTERN MODEL CUSTOM FOR:
LOT 20, HILLS OF ROSE CREEK
PROJECT ADDRESS: Parcel: 05-55-17-08116-120, Lake City, Florida 32024 (Columbia County)

GIBALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

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(386) 758-8406
wll@gwllmyers.net

JOB NUMBER
20230127

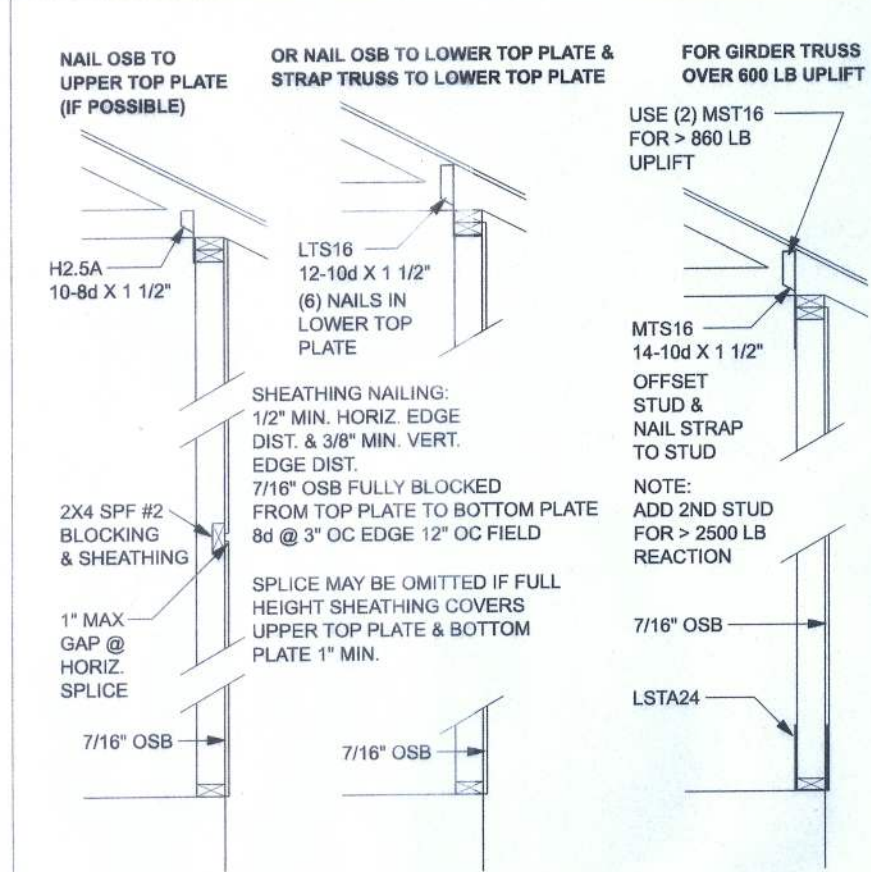
SHEET NUMBER
A.3



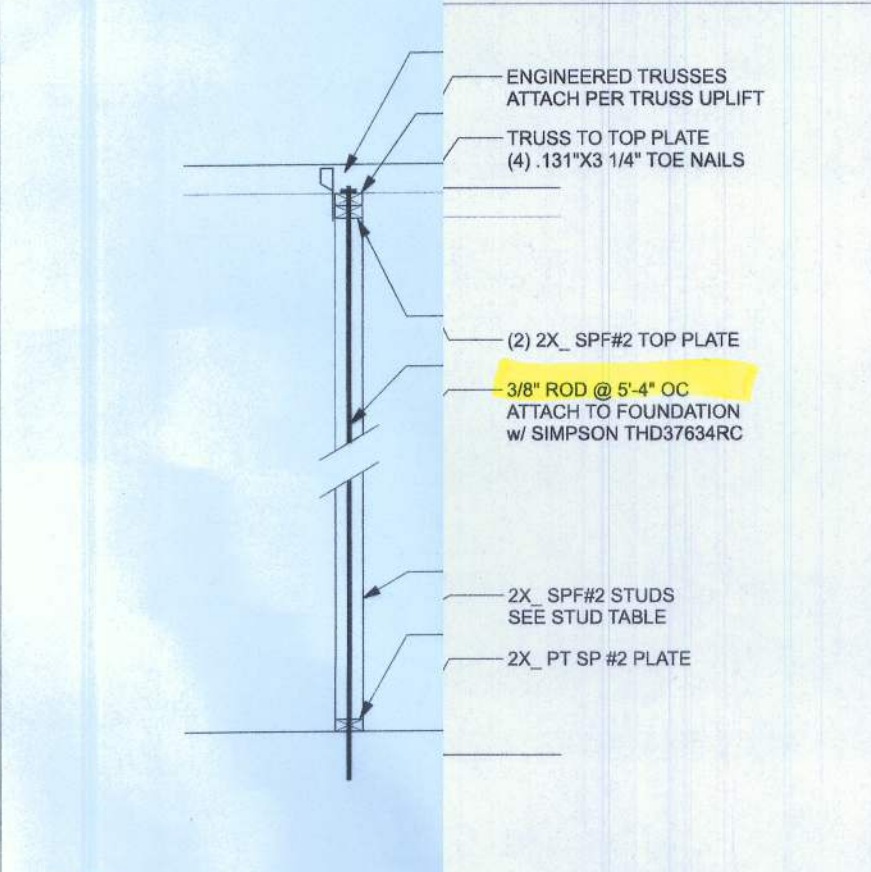
ROOF SHEATHING FASTENING TABLE (RAP / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail Spacing Along Intermediate Supports in the Panel Field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" o
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" o
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" o
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" o
130 mph Exp. C	15/32"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" o
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" o
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" o
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" o
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" o
150 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" o
150 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	4" o

Note: For sheathing located a minimum of 4 feet from the edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be as on center along panel edges and 8 inches on center along intermediate supports in the panel field. This table specifies the code minimum thickness of roof sheathing. The thickness of sheathing may need to be increased based in the type of roofing material being used. See manufacturer's product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL ATTACHMENT DETAILS
ONE STORY WOOD FRAME

CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600		
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	485	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	8-10d1 1/2"	8-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	18-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stems	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3840	HTT4	18-16d x 12"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3840	HTT4	18-16d x 12"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stems	To Post	Anchor
2200	ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300	ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300	ABU66	12-16d	5/8"x7" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20BS, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

Stud Spacing	Stud Height
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 8" W14 x W14, FB = 86KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACINGS OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTRACTOR IS NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-16, U.O.D.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS' INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

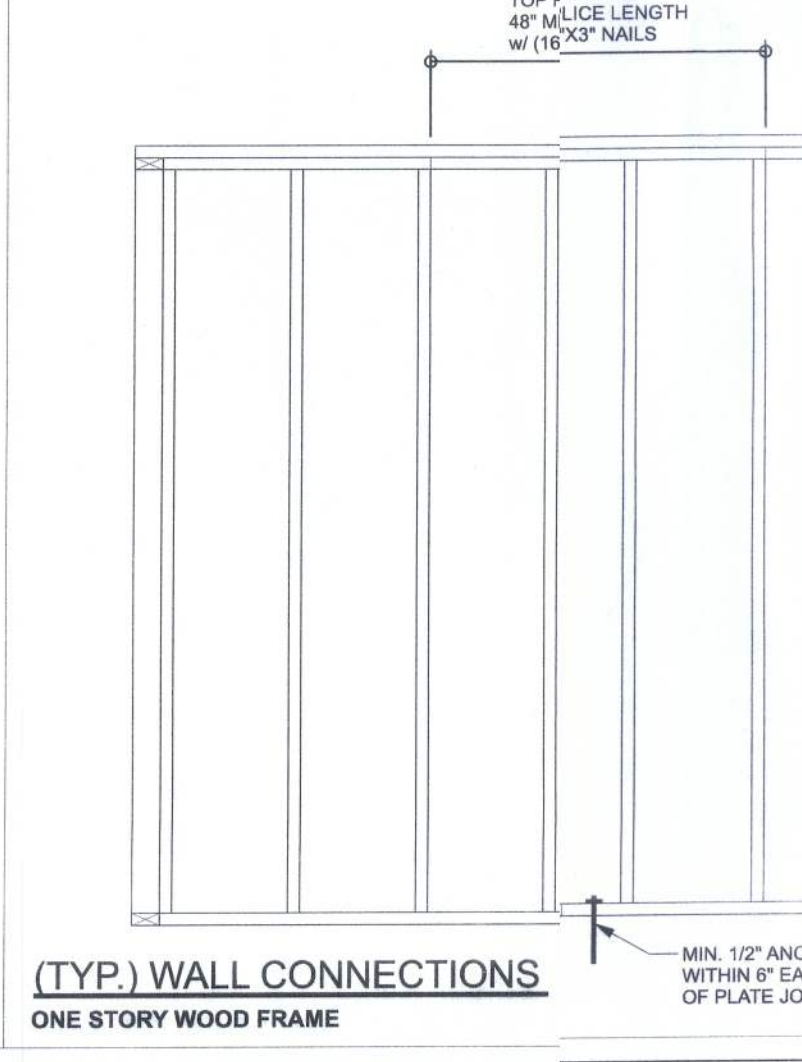
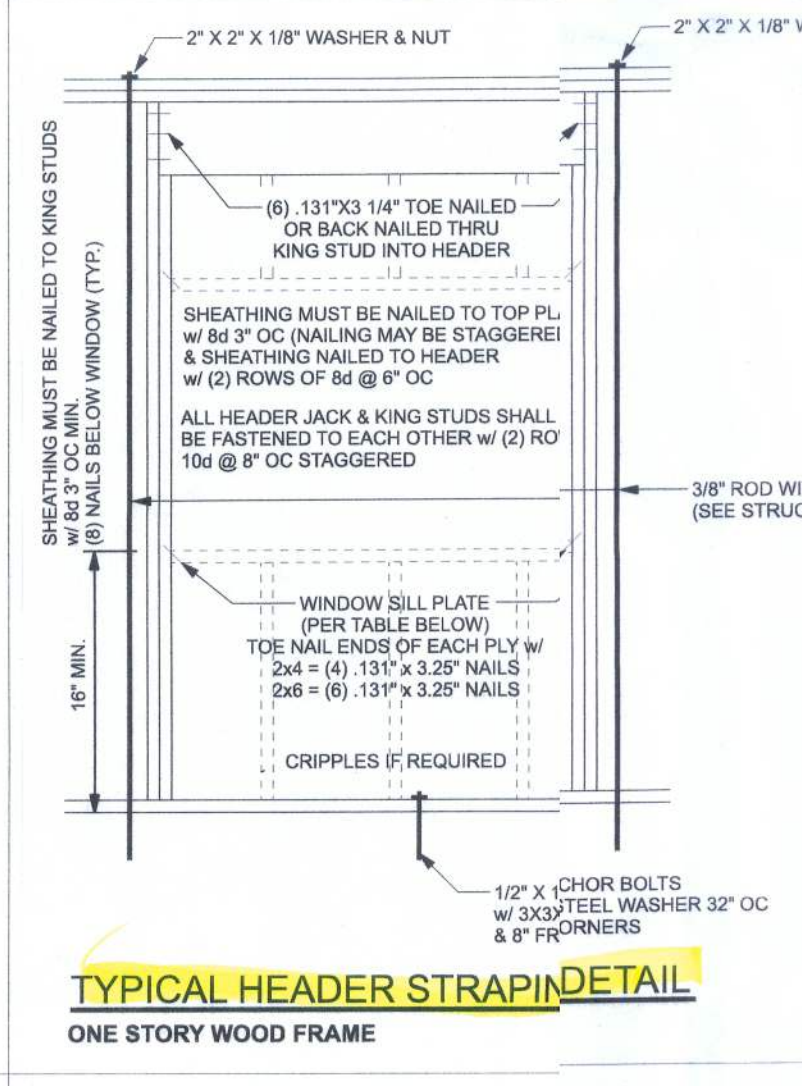
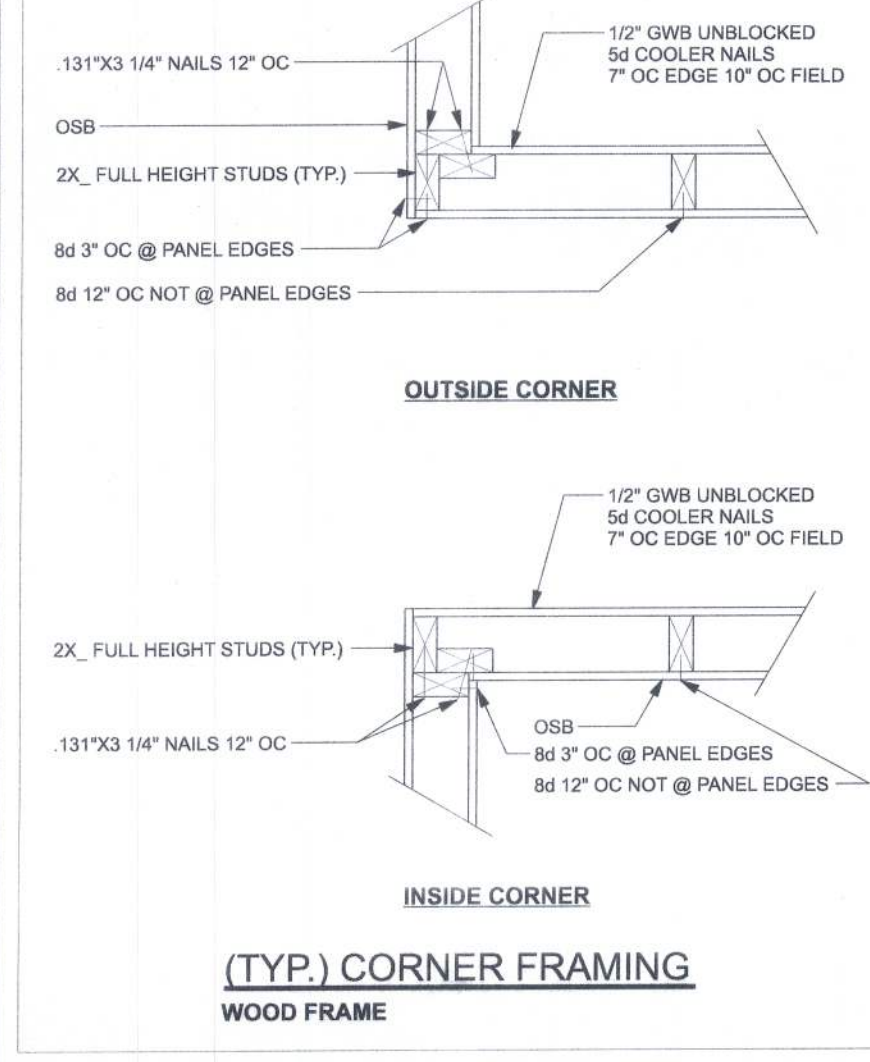
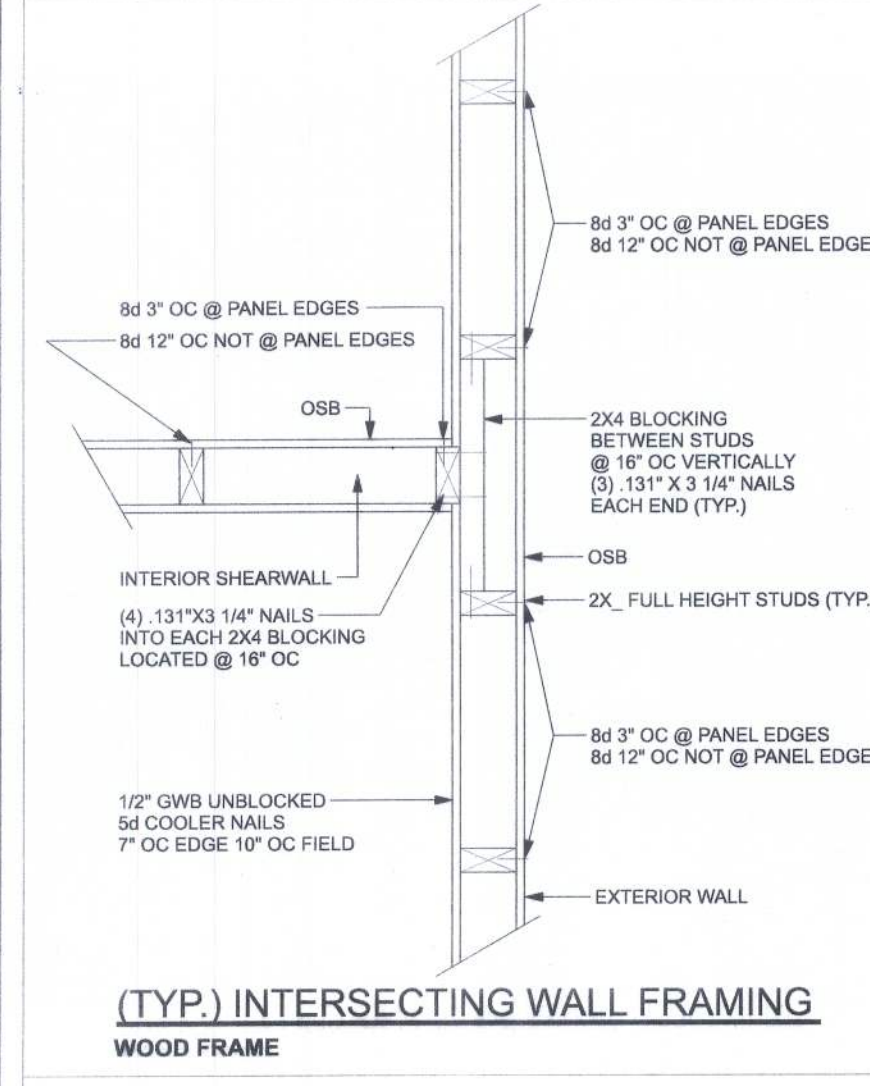
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER TRUSS DESIGNER'S SEAL.

ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

(TYP.) GABLE BRACING: TAIL WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME

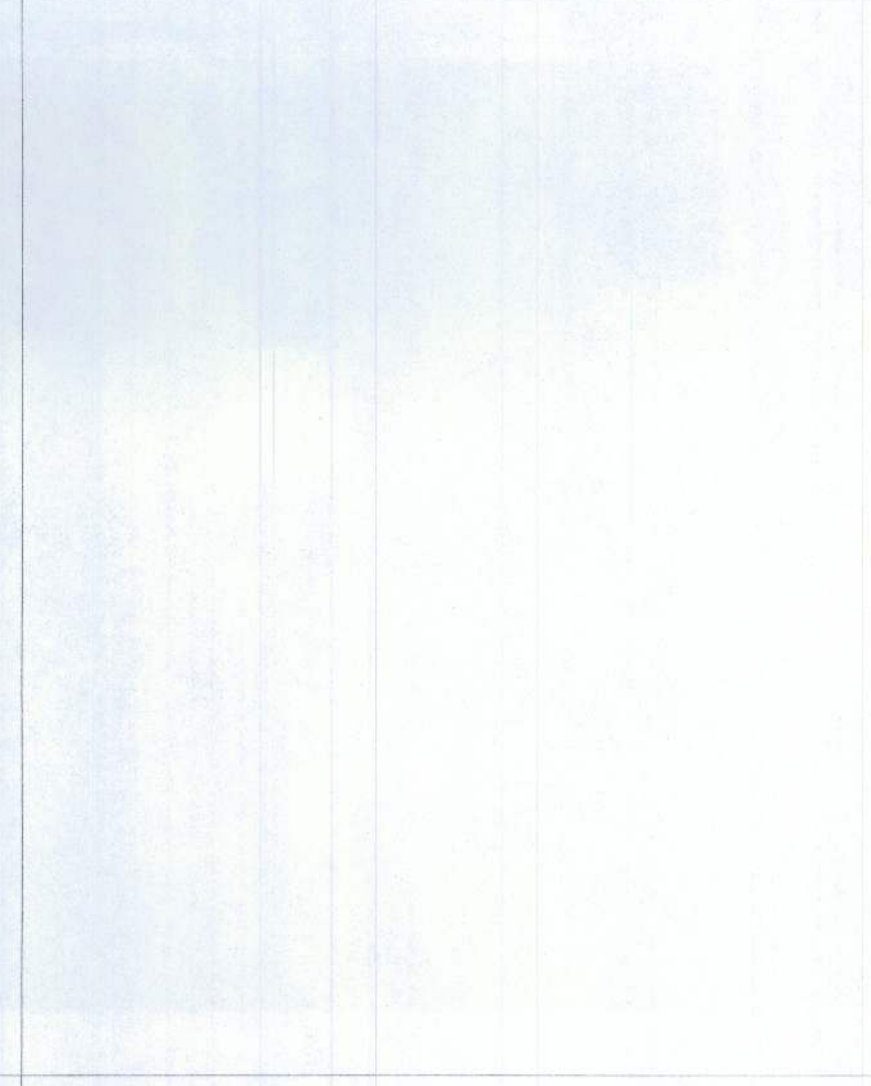
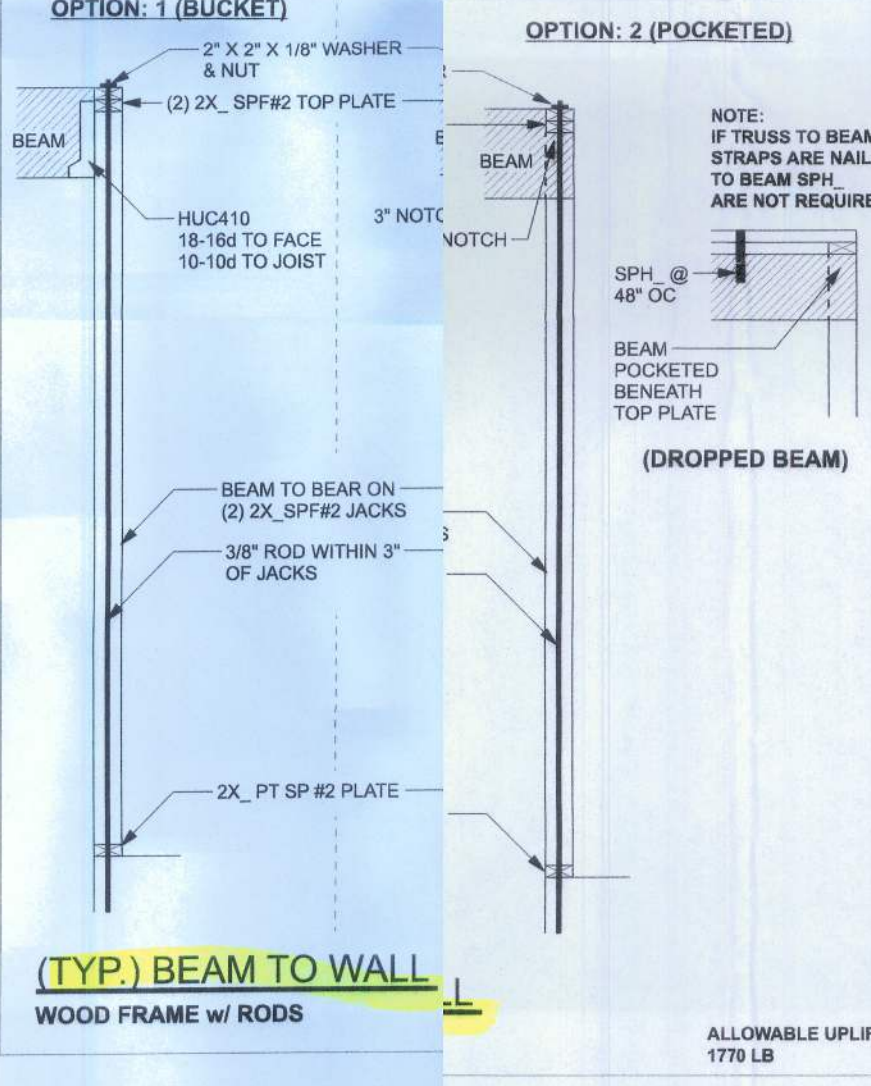
(TYP.) PORCH POST ONE STORY WOOD



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.23B
(1) 2x4	(1) 2x4	(1) 2x6
(2) 2x4	(2) 2x4	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"
	7'-9"	11'-5"

FOR OTHER WALL HEIGHTS (IF SILL SPAN SHALL BE DIVIDED BY (H/10))



DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2005)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS DESIGN IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0-20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0-20	+42.6(Vasd) -46.2(Vasd)	+42.6(Vasd) -57(Vasd)
20-200	+22.6(Vasd) -25.5(Vasd)	+22.6(Vasd) -25.5(Vasd)
200-2400	+21.7(Vasd) -24.1(Vasd)	+21.7(Vasd) -24.1(Vasd)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)

DESIGN CRITERIA & LOADS:

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CODE FOR DESIGN LOADS	ASCE 7-16
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20-200	+22.6(Vasd) -25.5(Vasd)	+22.6(Vasd) -25.5(Vasd)
200-2400	+21.7(Vasd) -24.1(Vasd)	+21.7(Vasd) -24.1(Vasd)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)

Gibraltar Contracting, LLC.

Western Model - Lot 20 Hills of Rose Creek

PROJECT ADDRESS: Lot 20 Hills of Rose Creek, Columbia County, FL

FL PE 53915

This has been digitally signed and sealed by Mark Disoway, P.E. for resolution. Do not proceed without clarification.

2/13/2023

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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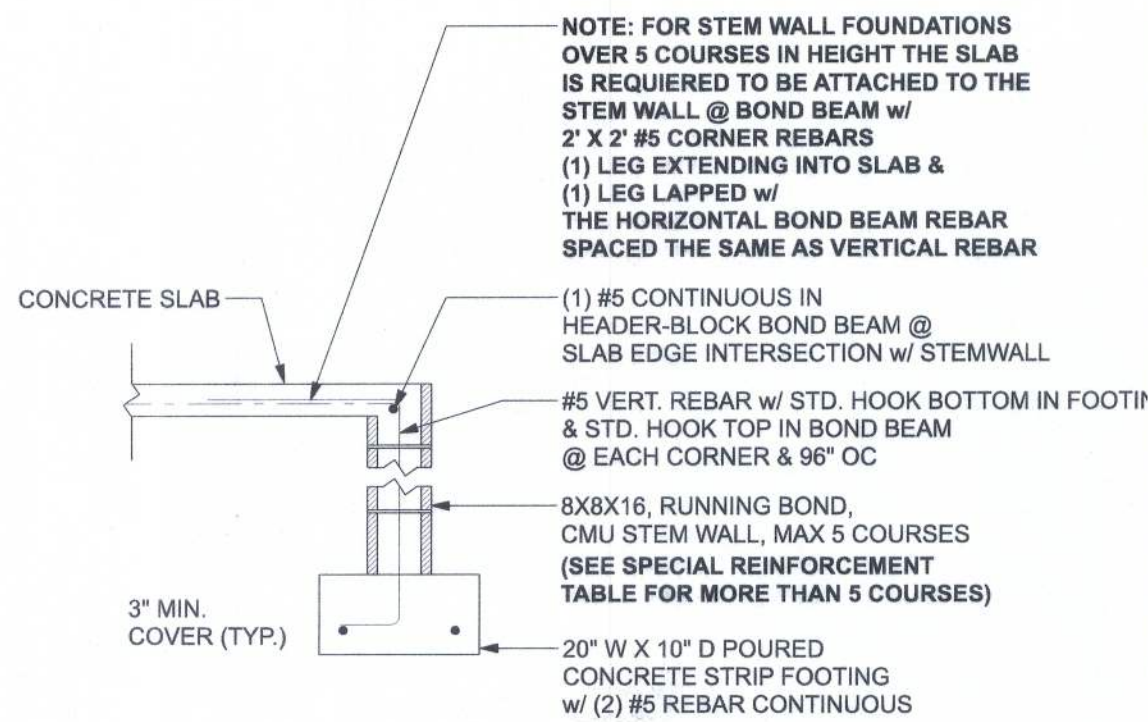
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building at specified location.

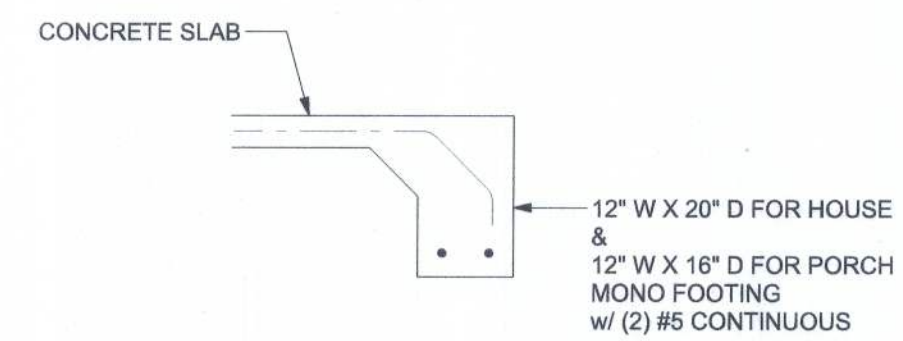
Mark Disoway P.E.
113 SW Midtown Place
Suite 103
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386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
230144

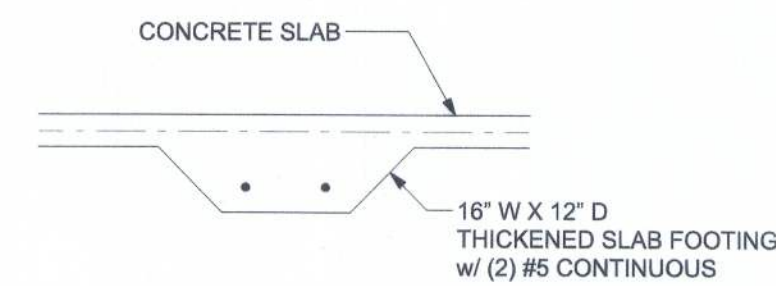
S-1
OF 3 SHEETS



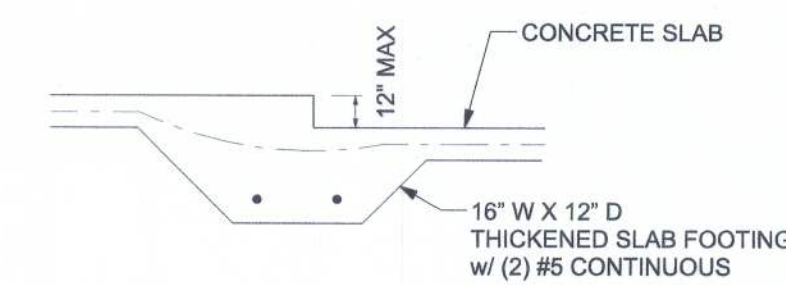
F1 S-2 **OPTIONAL STEM WALL FOOTING**
SCALE: 1/2" = 1'-0"



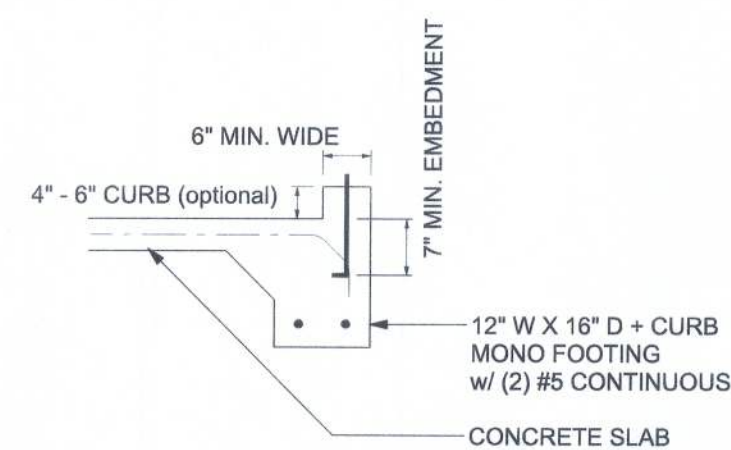
F1 S-2 **MONOLITHIC FOOTING**
SCALE: 1/2" = 1'-0"



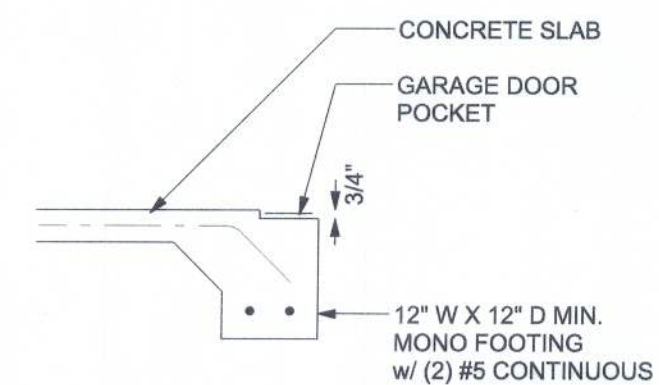
F2 S-2 **INTERIOR BEARING FOOTING**
SCALE: 1/2" = 1'-0"



F3 S-2 **INTERIOR BEARING STEP FOOTING**
SCALE: 1/2" = 1'-0"



F4 S-2 **MONOLITHIC CURB FOOTING**
SCALE: 1/2" = 1'-0"



F5 S-2 **GARAGE DOOR POCKET FOOTING**
SCALE: 1/2" = 1'-0"

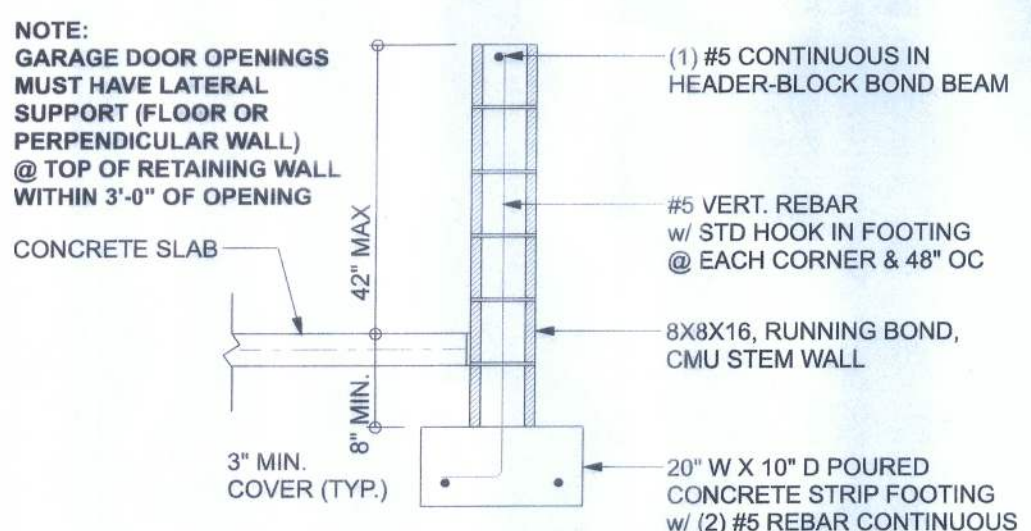
FULL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI/ASCE/TMS Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL.



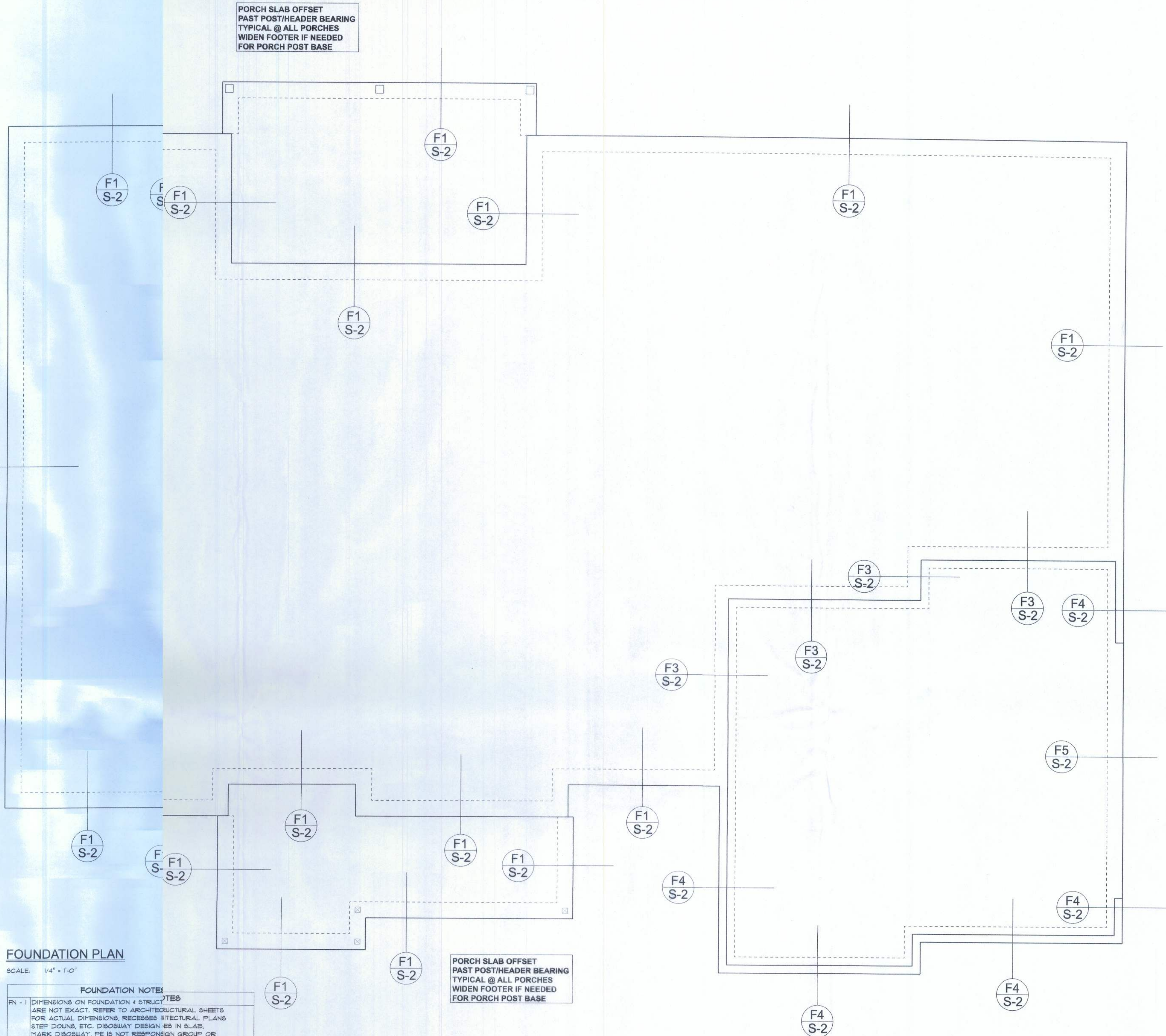
F4 S-2 **OPTIONAL STEM WALL CURB FOOTING**
SCALE: 1/2" = 1'-0"

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES:

- DIMENSIONS ON FOUNDATION & STRUCTURES ARE NOT EXACT. REFER TO ARCHITECTURAL SHEETS FOR ACTUAL DIMENSIONS, RECESSES, STRUCTURAL PLANS, STEP DOWN, ETC. DISCREPANCY DESIGNER'S IN SLAB, MARK DISCREPANCY, PE IS NOT RESPONSIBLE GROUP OR DIMENSION ERRORS ON THIS PLAN. RESPONSIBLE FOR CONTRACTOR SHALL VERIFY NEED FOR.
- IN ALL AREAS BY REVIEWING THE ROOM FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINAL DRUM/ROOF TRUSS PLAN.
- THE SLAB SHALL BE 4" CONCRETE SLAB ON FOUNDATION PLAN 6X6-1/4" WELDED WIRE MESH PLACED SLAB REINFORCED w/ #1 1/2" DEPTH OR FIBER MESH CONCRETE ON CHAIRS POLY VAPOR BARRIER w/ 6" LAPS 6" MIN. POLY TAPE OVER THERMITE-TREATED #5 SEALED w/ ED & COMPACTED FILL.



Gibraltar Contracting, LLC

Western Model - Lot 20 Hills of Rose Creek

PROJECT ADDRESS:
Lot 20 Hills of Rose Creek
Columbia County, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disosway P.E. for resolution. Do not proceed without clarification.



2/13/2023

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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Mark Disosway P.E. hereby expressly reserves its common law copyright and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the Florida Building Code (residential 2020) to the best of my knowledge.

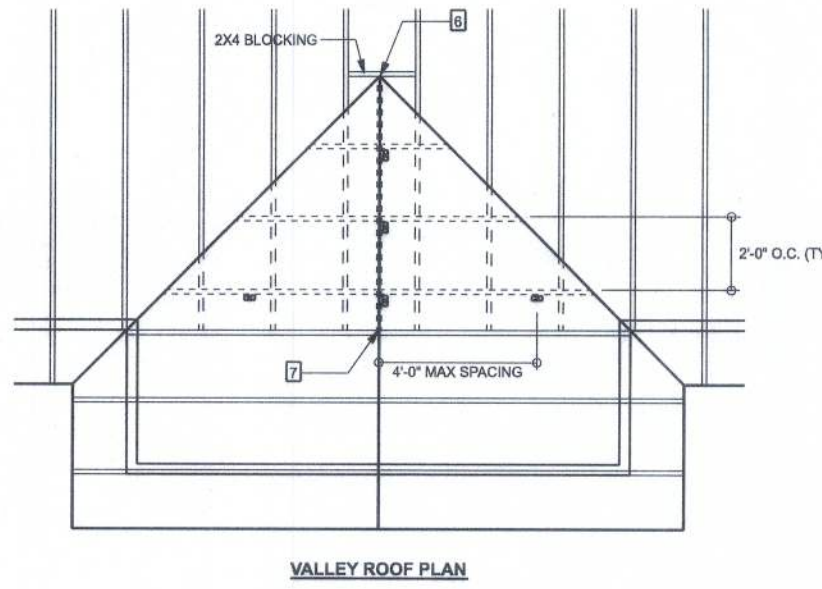
LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SV Midtown Place
Suite 103
Lake City, Florida 32025
36.754.5419
disoswaydesign@gmail.com

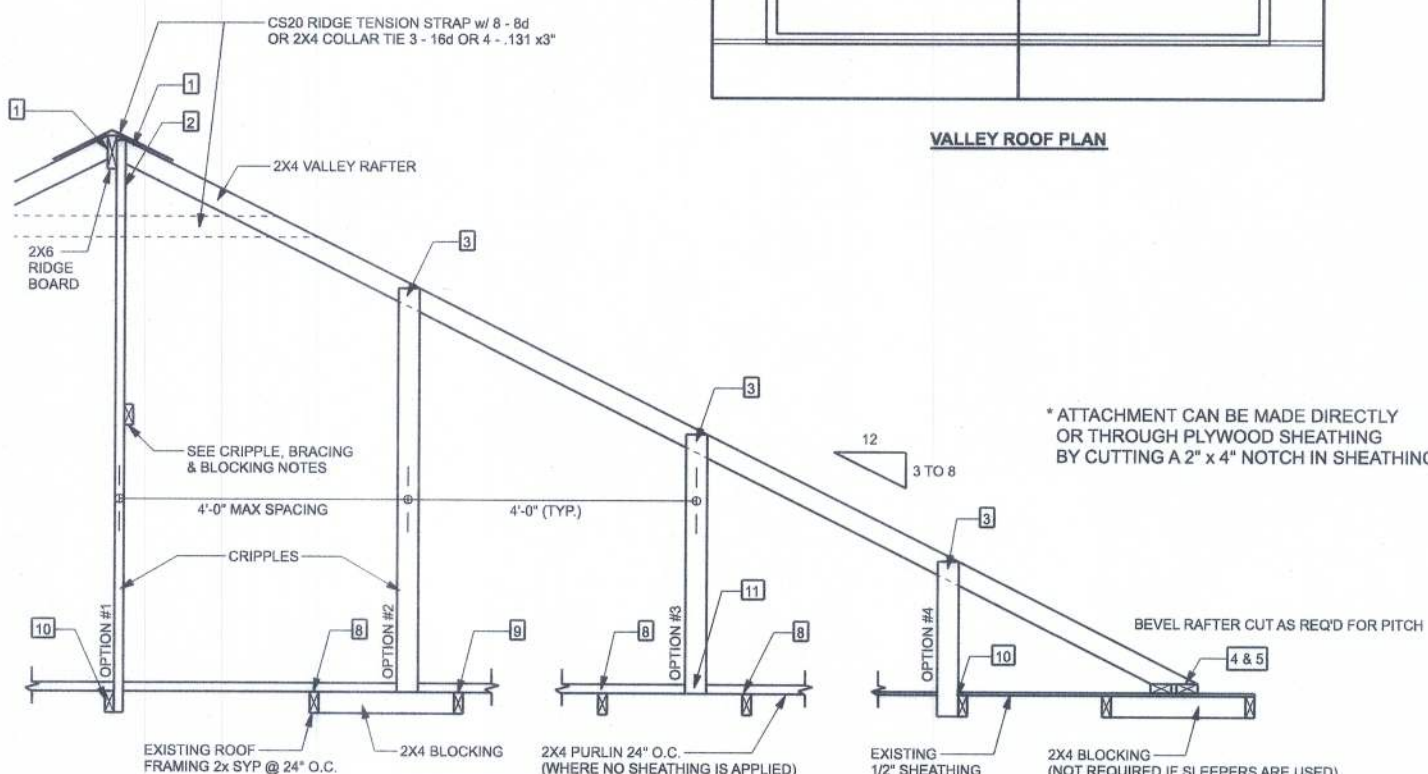
JOI NUMBER:
230144

S-2
OF 3 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
ROOF BOARD	2X6 SYP #2
RAFTER SPANS 24" O.C. OR LESS	2X4 SYP #2
PURLINS (LATERAL BRACING)	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER BEAT CUT) SYP #3 OR 2X WALLS 2X4 SYP #2
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSSES BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN



SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S.

VALLEY ROOF PLAN MEMBER LEGEND

TRUSS
TRUSS UNDER VALLEY FRAMING
VALLEY RAFTER OR RIDGE
CRIPPLE

CRIPPLES 4" O.C. FOR 20' (T/L) AND 10' (T/L) (TYP. SHINGLE ROOF) MAX.

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3-16d OR 6-13d x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3-16d OR 6-13d x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3-16d OR 6-13d x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	3-16d OR 6-13d x 3" TOE NAILS
5	SLEEPER TO TRUSS	4-16d OR 6-13d x 3" TOE NAILS EACH TRUSS
6	RIDGE BOARD TO RIDGE	3-16d OR 6-13d x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3-16d OR 6-13d x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3-16d OR 6-13d x 3" NAILS
9	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 6-13d x 3" NAILS
10	TRUSS TO BLOCKING	3-16d OR 6-13d x 3" END NAILS
11	CRIPPLE TO TRUSS	3-16d OR 6-13d x 3" FACE NAILS
12	CRIPPLE TO PURLIN	3-16d OR 6-13d x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPAN:
16'0" FOR 2X4, 8'0" FOR 2X6 SYP #2 OR SYP #2
18'0" OR 2'0" O.C. OF SPAN - 18'0"
PURLINS REQUIRED 2'0" O.C. IF EXISTING SHEATHING IS SPACED 2'0" O.C. OR GREATER. PURLINS SHOULD BE INSTALLED OVER SHEATHING A MINIMUM OF 8" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 3-16d COMMON WIRE NAILS.
THIS DRAWING APPLIED TO VALLEYS WITH THE FOLLOWING CONDITIONS:
- SPAN DISTANCE BETWEEN VALLEYS 40'0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'0" OR LESS
- MAXIMUM WIND SPEED: 130 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 PSF
- MEETS THE 2015 IBC WIND REQUIREMENTS
- EXPOSURE CATEGORY "C" (1=1, 2=1, 3=1)
- UNOCURED BUILDING

CRIPPLE BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 6'0" TO 10'0" LONG. NAILS 4-16d OR 2X4" OR SCAB NAILS NEEDED TO PLATE EDGE OF CRIPPLE WITH 8d NAILS @ 6" O.C. "T" OR SCAB MUST BE 80% OF CRIPPLE LENGTH. CRIPPLES OVER 10'0" LONG REQUIRE TWO CLBs ON BOTH SIDES 4" TYP. OR SCAB. USE STEEL GRADED LUMBER & BOX OR COMMON NAILS.
NAILING END OF CRIPPLE TO FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN OTHER TRUSSES.
- CRIPPLES AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-189 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

