FORM 1100A-08

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Performance Method A

Project Name:	New Project	Builder Name:	RON CLARK CONST
Street:		Permit Office:	
City, State, Zip:	, FL ,	Permit Number:	
Owner:	MICHAEL RADZIMINSKI	Jurisdiction:	
Design Location:	FL, Gainesville		

1. New construction or existing	New (From Plans)
2. Single family or multiple family	Single-family
3. Number of units, if multiple family	1
4. Number of Bedrooms	2
5. Is this a worst case?	No
6. Conditioned floor area (ft²)	960

7. Windows	Description	Area
a. U-Factor:	Dbl, U=0.40	102.00 ft²
SHGC:	SHGC=0.28	
b. U-Factor:	N/A	ft²
SHGC:		
c. U-Factor:	N/A	ft²
SHGC:		
d. U-Factor:	N/A	ft²
SHGC:		
e. U-Factor:	N/A	ft²
SHGC:		

8. Floor Types	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=0.0	960.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

9. Wall Types	Insulation	Area
a. Concrete Block - Int Insul, Exterior	R=5.0	1024.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types	Insulation	Area
a. Under Attic (Unvented)	R=30.0	960.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

11. Ducts
a. Sup: Attic Ret: Attic AH: Interior Sup. R= 6, 192 ft²

12. Cooling systems
a. Central Unit — Cap: 23 kBtu/hr, SEER: 13

13. Heating systems
a. Electric Heat Pump — Cap: 24 kBtu/hr, HSPF: 8.4

14. Hot water systems
a. Electric — Cap: 50 gallons, EF: 0.94
b. Conservation features
None

15. Credits — Pstat

Glass/Floor Area: 0.106

Total As-Built Modified Loads: 21.60
Total Baseline Loads: 25.43

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

SUNCOAST INSULATORS
825 NW 253rd Terrace
Newberry, FL 32669
(352) 472-6696
Fax: (352) 472-3633

PREPARED BY: JMD
DATE: 12/23/10

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Ronald Clark
DATE: 12-23-10

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
DATE: _____

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

PROJECT

Title:	New Project	Bedrooms:	2	Adress Type:	Street Address
Building Type:	FLAsBuilt	Bathrooms:	0	Lot #	
Owner:	MICHAEL RADZIMINSKI	Conditioned Area:	960	SubDivision:	
# of Units:	1	Total Stories:	1	PlatBook:	
Builder Name:	RON CLARK CONST	Worst Case:	No	Street:	
Permit Office:		Rotate Angle:	0	County:	COLUMBIA
Jurisdiction:		Cross Ventilation:		City, State, Zip:	, FL ,
Family Type:	Single-family	Whole House Fan:			
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

✓	Design Location	TMY Site	IECC Zone	Design Temp 97.5 %	Design Temp 2.5 %	Int Design Temp Winter	Int Design Temp Summer	Heating Degree Days	Design Moisture	Daily Temp Range
____	FL, Gainesville	FL_GAINESVILLE_REGI	2	32	92	75	70	1305.5	51	Medium

FLOORS

✓	#	Floor Type	Perimeter	R-Value	Area	Tile	Wood	Carpet
____	1	Slab-On-Grade Edge Insulation	128 ft	0	960 ft²	0	0	1

ROOF

✓	#	Type	Materials	Roof Area	Gable Area	Roof Color	Solar Absor.	Tested	Deck Insul.	Pitch
____	1	Gable or shed	Composition shingles	1040 ft²	200 ft²	Medium	0.96	No	0	22.6 deg

ATTIC

✓	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
____	1	Full attic	Vented	300	960 ft²	N	N

CEILING

✓	#	Ceiling Type	R-Value	Area	Framing Frac	Truss Type
____	1	Under Attic (Vented)	30	960 ft²	0.11	Wood

WALLS

✓	#	Ornt	Adjacent To	Wall Type	Cavity R-Value	Area	Sheathing R-Value	Framing Fraction	Solar Absor.
____	1	N	Exterior	Concrete Block - Int Insul	5	320 ft²		0	0.75
____	2	E	Exterior	Concrete Block - Int Insul	5	192 ft²		0	0.75
____	3	S	Exterior	Concrete Block - Int Insul	5	320 ft²		0	0.75
____	4	W	Exterior	Concrete Block - Int Insul	5	192 ft²		0	0.75

DOORS

√	#	Ornt	Door Type	Storms	U-Value	Area
____	1	N	Insulated	None	0.46	24 ft²
____	2	S	Insulated	None	0.46	24 ft²

WINDOWS

Window orientation below is as entered. Actual orientation is modified by rotate angle shown in "Project" section above.

√	#	Ornt	Frame	Panes	NFRC	U-Factor	SHGC	Storms	Area	Overhang Depth	Overhang Separation	Int Shade	Screening
____	1	N	Metal	Double (Clear)	Yes	0.4	0.28	N	40 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	2	E	Metal	Double (Clear)	Yes	0.4	0.28	N	15 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	3	E	Metal	Double (Clear)	Yes	0.4	0.28	N	9 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	4	S	Metal	Double (Clear)	Yes	0.4	0.28	N	9 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	5	W	Metal	Double (Clear)	Yes	0.4	0.28	N	9 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	6	N	Metal	Double (Clear)	Yes	0.4	0.28	N	20 ft²	14 ft 0 in	6 ft 0 in	HERS 2006	None

INFILTRATION & VENTING

√	Method	SLA	CFM 50	ACH 50	ELA	EqLA	— Forced Ventilation — Supply CFM	Exhaust CFM	Run Time Fraction	Fan Watts
____	Default	0.00036	907	7.08	49.8	93.6	0 cfm	0 cfm	0	0

COOLING SYSTEM

√	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Ductless
____	1	Central Unit	None	SEER: 13	23 kBtu/hr	690 cfm	0.75	False

HEATING SYSTEM

√	#	System Type	Subtype	Efficiency	Capacity	Ductless
____	1	Electric Heat Pump	None	HSPF: 8.4	24 kBtu/hr	False

HOT WATER SYSTEM

√	#	System Type	EF	Cap	Use	SetPnt	Conservation
____	1	Electric	0.94	50 gal	50 gal	120 deg	None

SOLAR HOT WATER SYSTEM

√	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
____	*None*	*None*			ft²		

DUCTS

√	#	— Supply — Location	R-Value	Area	— Return — Location	Area	Leakage Type	Air Handler	CFM 25	Percent Leakage	QN	RLF
____	1	Attic	6	192 ft²	Attic	48 ft²	Default Leakage	Interior				

TEMPERATURES

Programable Thermostat: Y Ceiling Fans:

Cooling	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec
Heating	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec
Venting	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec

Thermostat Schedule: *HERS 2006 Reference*

Schedule Type		Hours 1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	*68*	*68*	*68*	*68*	*68*	*68*	*68*	*68*	*68*	*68*	*66*	*66*
Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

wrightsoft

Load Short Form
Entire House
P.O.Box 1617

Job: Bounds HVAC
Date: Dec 22, 2010
By: Robert Bounds

25645 W. Newberry Rd., Newberry, Fl. 32669 Phone: 352-472-2761 Fax: 352-472-1809 Email: mrobertbounds@aol.com Web: wwww.boundshvac.com

Project Information

For: Ronald Clark Construction, Radziminski Job

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	68	75
Design TD (°F)	35	17
Daily range	-	M
Inside humidity (%)	50	50
Moisture difference (gr/lb)	10	***50***

Infiltration	
Method	Simplified
Construction quality	Tight
Fireplaces	0

HEATING EQUIPMENT

Make	Carrier
Trade	BASE 13 PURON HP
Model	25HBB324(A,W)30
ARI ref no.	3252857

Efficiency	8.2 HSPF	
Heating input		
Heating output	24000	Btuh @ 47°F
Temperature rise	29	°F
Actual air flow	753	cfm
Air flow factor	0.033	cfm/Btuh
Static pressure	1.00	in H2O
Space thermostat		

COOLING EQUIPMENT

Make	Carrier
Trade	BASE 13 PURON HP
Cond	25HBB324(A,W)30
Coil	FY4ANF024
ARI ref no.	3252857

Efficiency	10.5 EER, 13 SEER	
Sensible cooling	15820	Btuh
Latent cooling	6780	Btuh
Total cooling	22600	Btuh
Actual air flow	753	cfm
Air flow factor	0.047	cfm/Btuh
Static pressure	1.00	in H2O
Load sensible heat ratio	0.90	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
Bath	60	2756	2194	92	102
Closet	70	1183	616	39	29
Bed	182	4875	3881	162	181
Dining Living	384	6125	4823	204	225
Wash Room	40	676	352	22	16
Bath 1	70	2925	2004	97	93
Bed 1	154	4101	2289	136	107
Entire House d	960	22642	16160	753	753
Other equip loads		0	0		
Equip. @ 0.97 RSM			15675		
Latent cooling			1737		
TOTALS	960	22642	17411	753	753

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

wrightsoft

Duct System Summary
Entire House
P.O.Box 1617

Job: Bounds HVAC
Date: Dec 22, 2010
By: Robert Bounds

25645 W. Newberry Rd., Newberry, Fl. 32669 Phone: 352-472-2761 Fax: 352-472-1809 Email: mrobertbounds@aol.com Web: wwww.boundshvac.com

Project Information

For: Ronald Clark Construction, Radziminski Job

	Heating	Cooling
External static pressure	1.00 in H2O	***1.00*** in H2O
Pressure losses	0.30 in H2O	0.30 in H2O
Available static pressure	0.70 in H2O	0.70 in H2O
Supply / return available pressure	0.51 / 0.19 in H2O	0.51 / 0.19 in H2O
Lowest friction rate	***0.100*** in/100ft	***0.100*** in/100ft
Actual air flow	753 cfm	753 cfm
Total effective length (TEL)	407 ft	

Supply Branch Detail Table

Name		Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
Bath	c	2194	92	102	0.100	7.0	0x0	VlFx	31.0	195.0	st2
Bath 1	h	2925	97	93	0.100	7.0	0x0	VlFx	9.0	100.0	st1
Bed	c	3881	162	181	0.100	8.0	0x0	VlFx	33.0	265.0	st2
Bed 1	h	4101	136	107	0.100	8.0	0x0	VlFx	13.0	170.0	st1
Closet	h	1183	39	29	0.100	5.0	0x0	VlFx	24.0	275.0	st2
Dining Living	c	2411	102	112	0.100	7.0	0x0	VlFx	11.0	190.0	st2
Dining Living-A	c	2411	102	112	0.100	7.0	0x0	VlFx	19.0	190.0	st2
Wash Room	h	676	22	16	0.100	4.0	0x0	VlFx	4.0	195.0	st2

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st1	Peak AVF	234	200	0.100	468	8.4	12 x 6	RectFbg	
st2	Peak AVF	520	553	0.100	664	11.6	12 x 10	RectFbg	

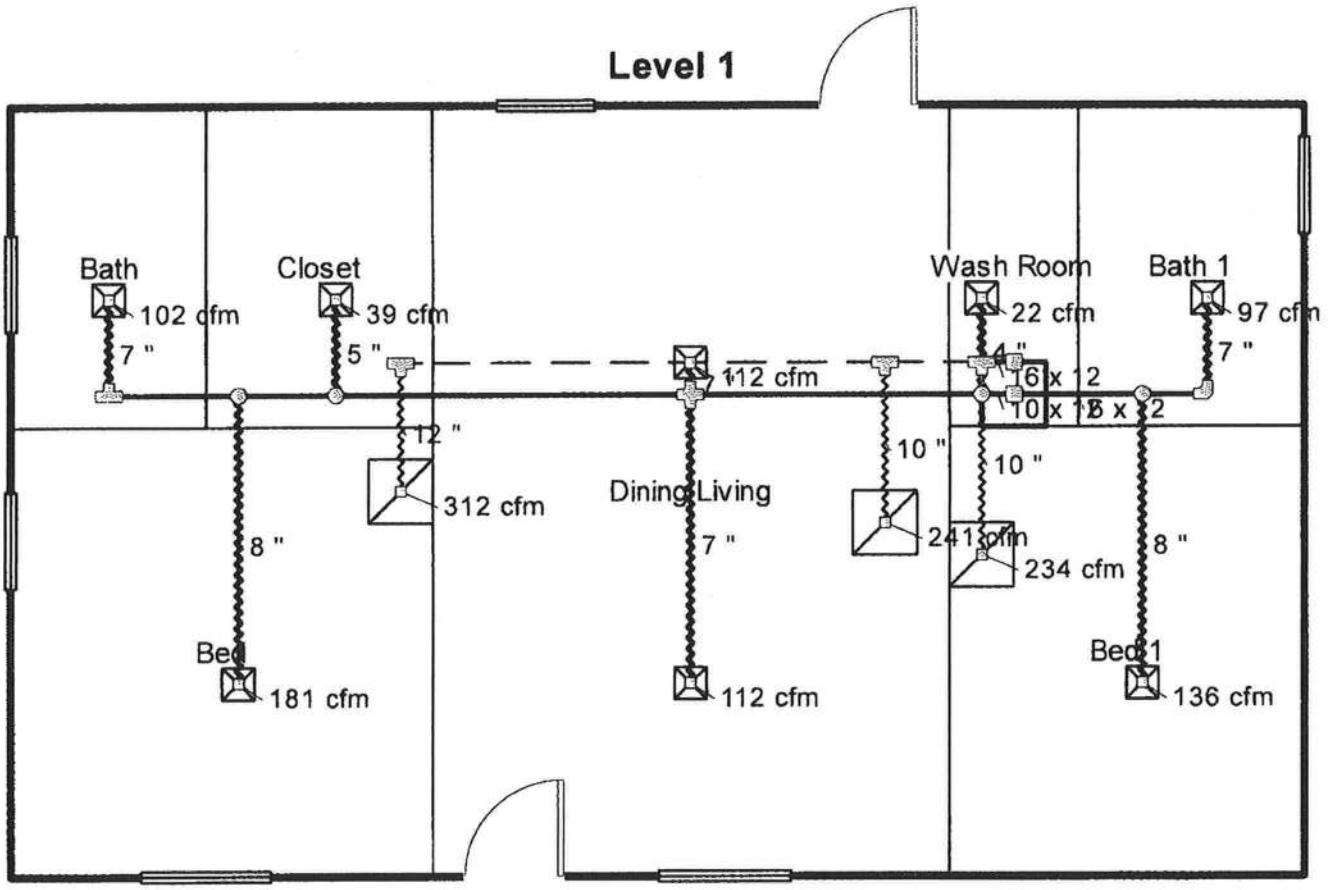
Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	226	241	99.0	0.050	442	10.0	0x 0		VlFx	rt1
rb2	0x0	293	312	108.0	0.050	397	12.0	0x 0		VlFx	rt1
rb3	0x0	234	200	87.0	0.050	429	10.0	0x 0		VlFx	rt1

Return Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
rt1	Peak AVF	753	753	0.050	565	15.0	12 x 16	RectFbg	


Level 1
Bath
102 cfm
7 "
Closet
39 cfm
5 "
12 "
312 cfm
8 "
Bed
181 cfm
112 cfm
7 "
Dining/Living
10 "
241 cfm
Wash Room
22 cfm
4 "
6 x 12
10 x 12
10 "
234 cfm
Bath 1
97 cfm
7 "
8 "
Bed 1
136 cfm

Schafer Engineering, LLC

14705 Main St. Alachua FL 32615

Prepared for:

RONALD CLARK CONSTRUCTION
THE RADZIMINSKI RESIDENCE
COLUMBIA COUNTY, FLORIDA

By:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

NO COPIES ARE TO BE PERMITTED

SCHAFER ENGINEERING, LLC

December 17, 2010

SUMMARY: Wind Load Analysis for Ronald Clark Construction \ Radziminski Residence
Wind Speed: 110 M.P.H. \ No Copies Permitted \ FBC-2007 \ Not Valid without raised seal

Foundation:
20" wide x 10" deep stemwall footing with (2) #5 rebar continuous minimum. CMU walls must have #5 dowels at 72" o.c. maximum with a standard 90 degree ACI hook in footing and a 4" slab on grade. Monolithic slab to be 12" wide x 20" deep minimum with (2) #5 rebar continuous with 12" minimum coverage on face of foundation. It is assumed that ideal soil conditions and pad preparation are provided.

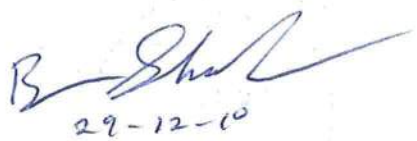
Walls:
8" CMU block with vertical #5 reinforcing bar in grout filled cell at 72" o.c. maximum spacing. Wall heights are 8' maximum. Provide an 8" x 8" bond beam with 1-#5 rebar horizontal continuous at the top course. Install pre-cast, pre-engineered lintels spanning over all openings. One #5 rebar each corner. One #5 rebar each side of door and window openings. Two #5 rebar in openings wider than 12'-0". One #5 rebar where girders or girder trusses bear on masonry wall. 16 x 16 cmu porch columns with 1-#5 rebar in each cell with a maximum clear span of 18'.

Shearwalls:
Transverse: 38'-0"
Longitudinal: 56'-0"

Allowable pounds per foot unit shear on shearwalls: 314
Unit shear transferred from diaphragm: Trs: 100 plf Long: 48 plf

Trusses:
Pre-engineered Pre-fabricate trusses with the bracing system designed by the manufacturer. Trusses must be installed and anchored according to the truss engineering requirements.

Roof Sheathing:
7/16" osb minimum attached to the top chords of the trusses with 8d/113 gauge ring shank nails spaced at 4" o.c. edges and 6" interior.



Bruce Schafer P. E. #48984
7104 N. W. 42nd Lane \ Gainesville, Florida 32606

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606

PHONE: 386-462-1340 \ 352-375-6329

HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA19	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3130
Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.				

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2775
to 3665	3-TS22	HD5A	4010
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	9540
Two 12d common toenials are required per truss for each bearing point into top plate. It is the contractors responsibility to provide a continuous load path from truss to foundation.			

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2300

1. Simpson or equivient hardware may be used.
   For nailing into spruce members, multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a replacement to the specified values of any manufactures values.

ASCE 7-05

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	14.67	ft
Mean Roof Height (Ht)	11.33	ft
Width Perp. to Wind (B)	38.00	ft
Width Parallel to Wind (L)	40.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.30	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.9137	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8741	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.74	
Nb	4.6*NatFreq*B/Vzm	2.47	
Nd	15.4*NatFreq*Depth/Vzm	8.69	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.6477	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.3239	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.1085	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.8201	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.14	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.87	Gust Factor (G)	0.87

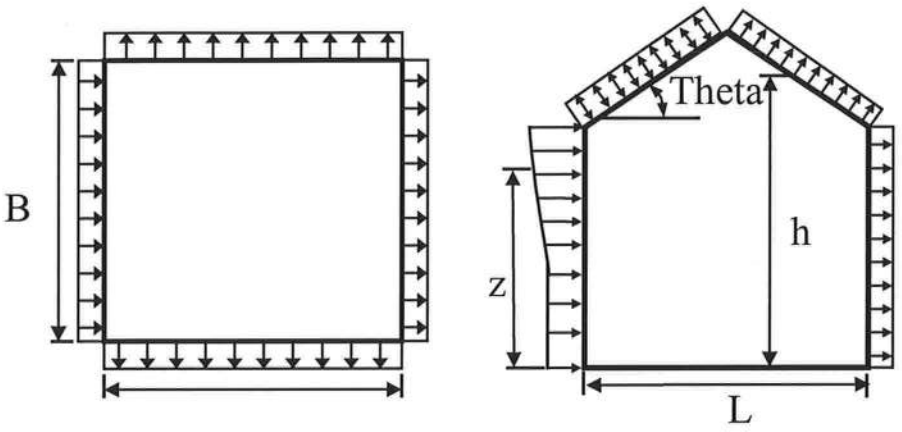
ASCE 7-05

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
15	0.70	1.00	1.00	21.70	11.97	18.38

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 38 ft wall)	-0.49	-10.82	-4.41
Leeward Walls (Wind Dir Parallel to 40 ft wall)	-0.50	-10.98	-4.58
Side Walls	-0.70	-14.10	-7.69
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.21	-6.46	-0.05
Windward - Max Positive	0.29	1.26	7.67
Leeward Normal to Ridge	-0.60	-12.54	-6.13
Overhang Top	-0.21	-3.25	-3.25
Overhang Bottom	0.80	0.70	0.70
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 5.665 ft	-0.90	-17.21	-10.80
Dist from Windward Edge: 5.665 ft to 11.33 ft	-0.90	-17.21	-10.80
Dist from Windward Edge: 11.33 ft to 22.66 ft	-0.50	-10.98	-4.58
Dist from Windward Edge: > 22.66 ft	-0.30	-7.87	-1.46

ASCE 7-05

* Horizontal distance from windward edge

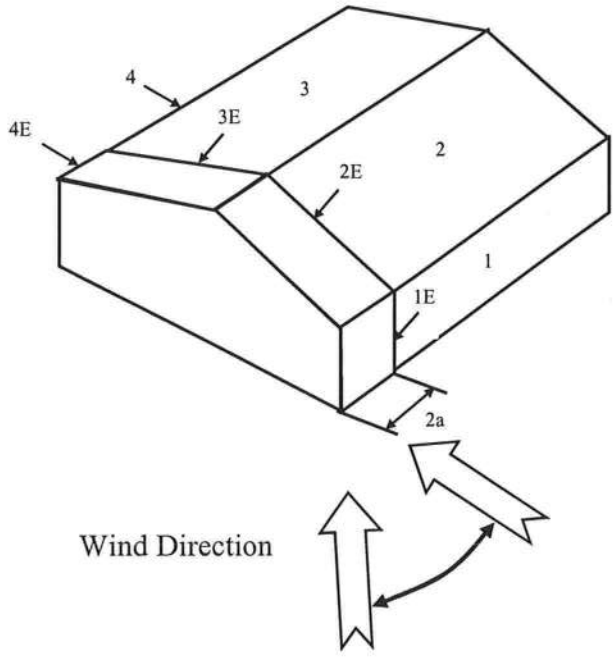
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-05

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

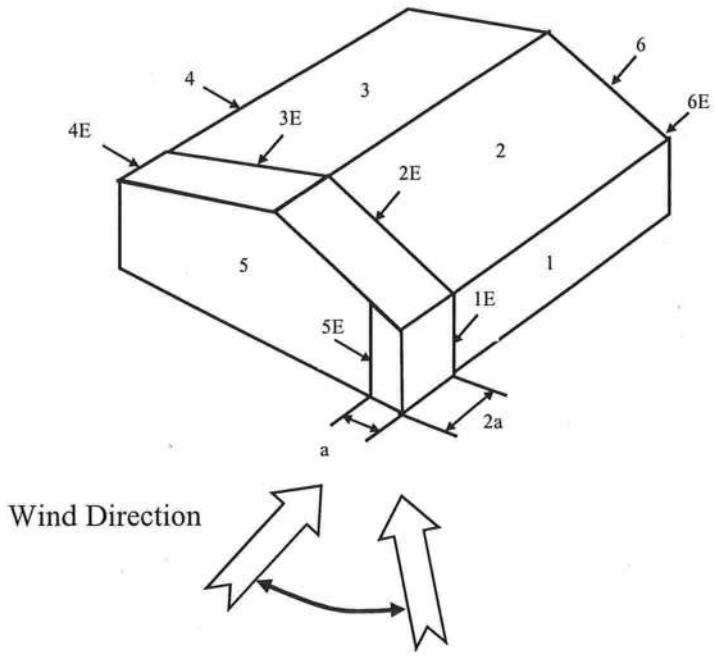
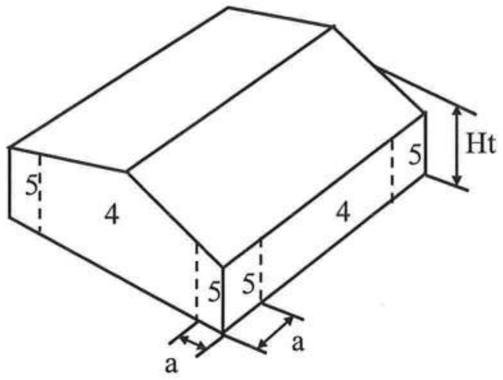
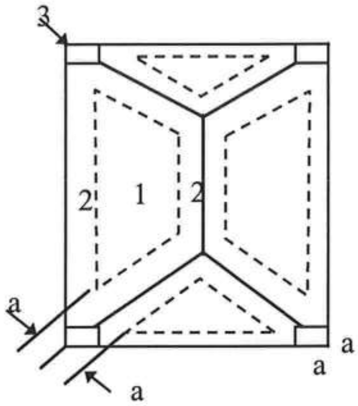


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

ASCE 7-05





Hipped Roof
10 < Theta <= 30

a = 3.8 ==> 3.80 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp Max	GCp Min	Wind Press (lb/ft^2) Max	Wind Press (lb/ft^2) Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi Max +	Gcpi Max -
Open Buildings	0.00	0.00

ASCE 7-05

Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.26	-18.76	-10.98
	P (-GCpi) -psf	5.15	-12.36	-4.58
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.33	-18.76	-10.98
	P (-GCpi) -psf	3.33	-18.76	-10.98

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	40.00	ft
B	Roof dimension parallel to wind direction	38.00	ft
L/B	Ratio of L to B	1.053	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.19	
X	Distance to center of pressure from windward edge	0.41	ft

Julius Lee

RE: 357050 - RONALD CLARK CONST. - RADZIMINSKI RES.

1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

Site Information:
Project Customer: RONADL CLARK CONST. Project Name: 357050 Model: RADZIMINSKI RES.
Lot/Block: Subdivision:
Address: 158 SW CONESTOGA RD
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: RONALD W. CLARK License #: CRC1326560
Address: 15816 NW CR 1491
City: ALACHUA State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date
1	I4576055	CJ1	12/21/010
2	I4576056	CJ3	12/21/010
3	I4576057	CJ5	12/21/010
4	I4576058	EJ7	12/21/010
5	I4576059	HJ9	12/21/010
6	I4576060	T01	12/21/010
7	I4576061	T02	12/21/010
8	I4576062	T03	12/21/010
9	I4576063	T04	12/21/010
10	I4576064	T05	12/21/010
11	I4576065	T06	12/21/010
12	I4576066	T07	12/21/010
13	I4576067	T08	12/21/010
14	I4576068	T09	12/21/010
15	I4576069	T10	12/21/010

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

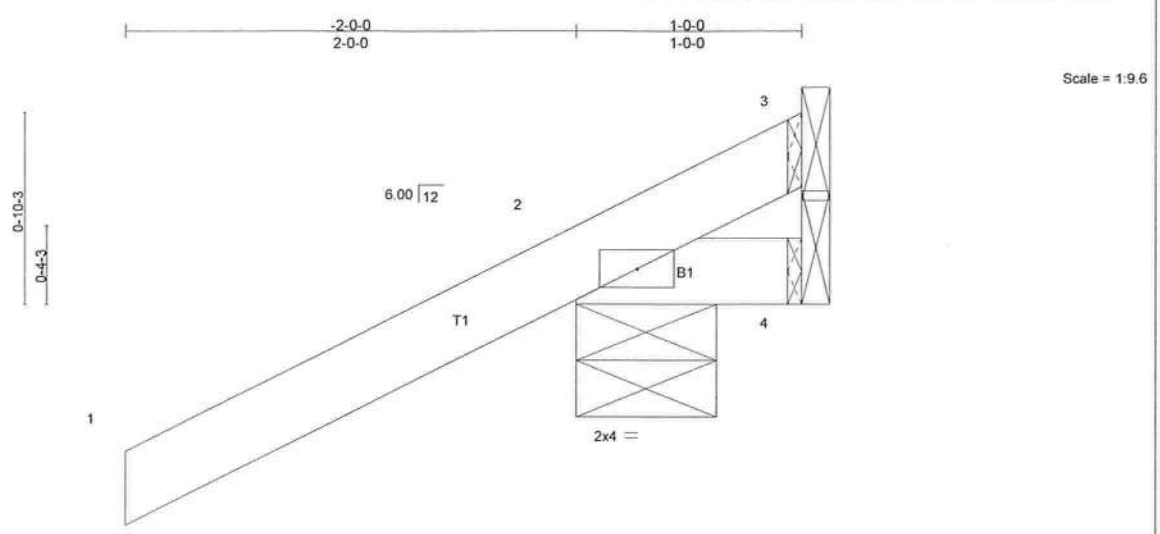
NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

December 21,2010

Julius Lee

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	CJ1	JACK	10	1	I4576055
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:06 2010 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.00	2	****	240	Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

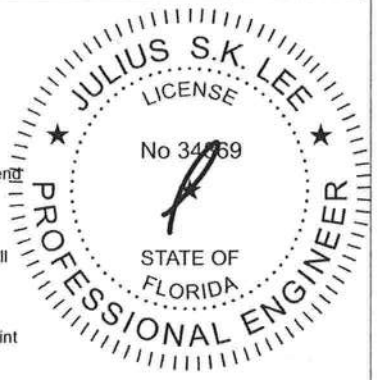
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=265/0-7-8, 4=5/Mechanical, 3=-99/Mechanical
Max Horz 2=109(LC 6)
Max Uplift 2=-370(LC 6), 3=-99(LC 1)
Max Grav 2=265(LC 1), 4=14(LC 2), 3=176(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2 .
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2 and 99 lb uplift at joint 3.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

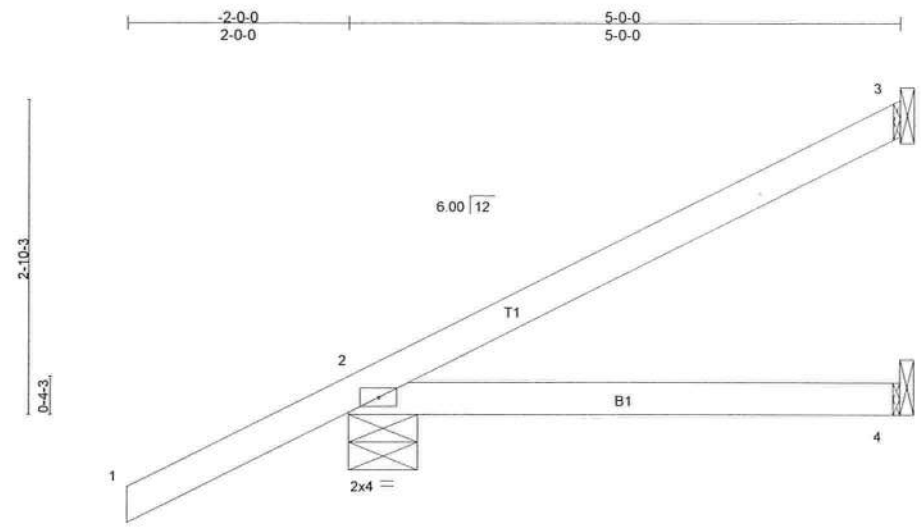


December 21,201

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	CJ5	JACK	10	1	I4576057
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:07 2010 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.02	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.04	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.09	2-4	>585	240	Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

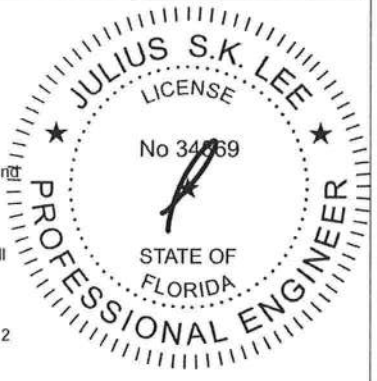
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=94/Mechanical, 2=304/0-7-8, 4=23/Mechanical
Max Horz 2=224(LC 6)
Max Uplift 3=-104(LC 6), 2=-353(LC 6), 4=-59(LC 4)
Max Grav 3=94(LC 1), 2=304(LC 1), 4=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2 .
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 3, 353 lb uplift at joint 2 and 59 lb uplift at joint 4.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



December 21,201(

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	HJ9	MONO TRUSS	5	1	I4576059
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:08 2010 Page 1

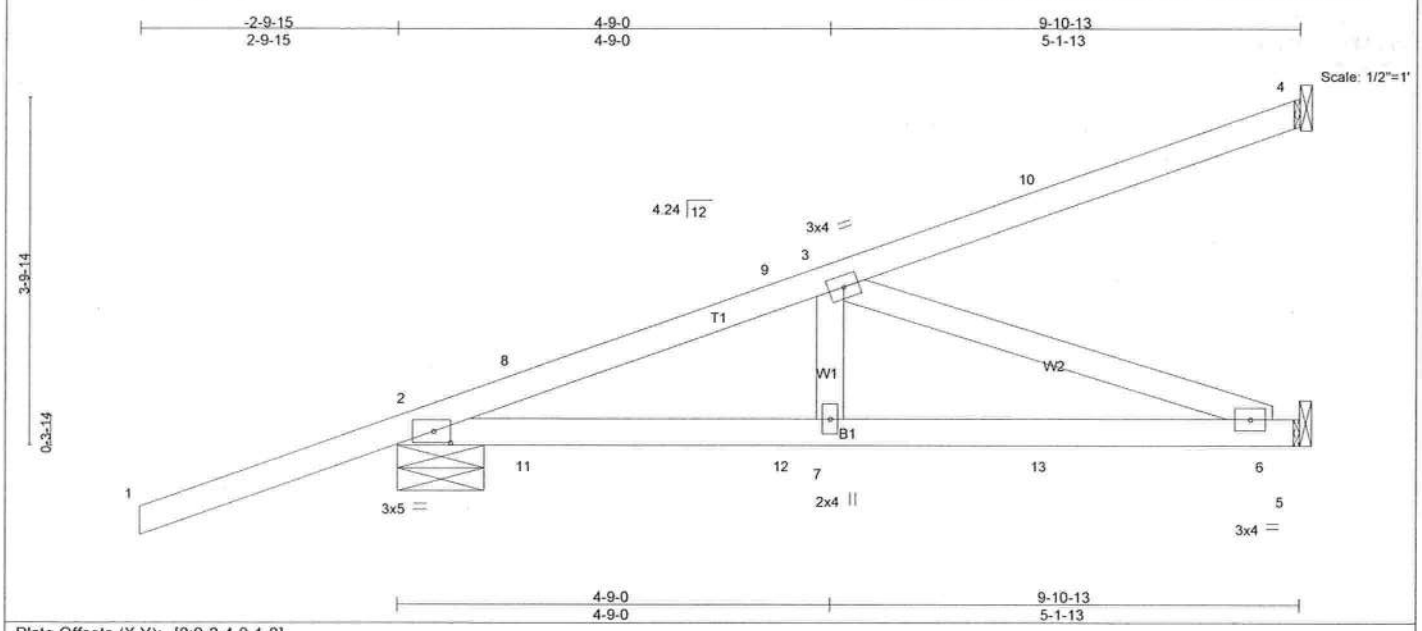


Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.10	6-7	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.11	6-7	>999	240	Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=152/Mechanical, 2=413/0-11-6, 5=159/Mechanical
Max Horz 2=283(LC 3)
Max Uplift 4=-168(LC 3), 2=-677(LC 3), 5=-316(LC 6)
Max Grav 4=152(LC 1), 2=413(LC 1), 5=215(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=-418/656, 8-9=-424/648, 3-9=-400/646
BOT CHORD 2-11=-711/368, 11-12=-711/368, 7-12=-711/368, 7-13=-711/368, 6-13=-711/368
WEBS 3-6=-390/753

NOTES (10-11)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2 .
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 4, 677 lb uplift at joint 2 and 316 lb uplift at joint 5.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, 40 lb up at 1-5-12, 20 lb down and 38 lb up at 4-3-11, 20 lb down and 38 lb up at 4-3-11, and 40 lb down and 87 lb up at 7-1-10, and 40 lb down and 87 lb up at 7-1-10 on top chord, and 16 lb up at 1-5-12, 16 lb up at 1-5-12, 9 lb down and 8 lb up at 4-3-11, 9 lb down and 8 lb up at 4-3-11, and 39 lb down and 34 lb up at 7-1-10, and 39 lb down and 34 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 21,201

Continued on page 2

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	T01	HIP	1	1	I4576060
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:08 2010 Page 1

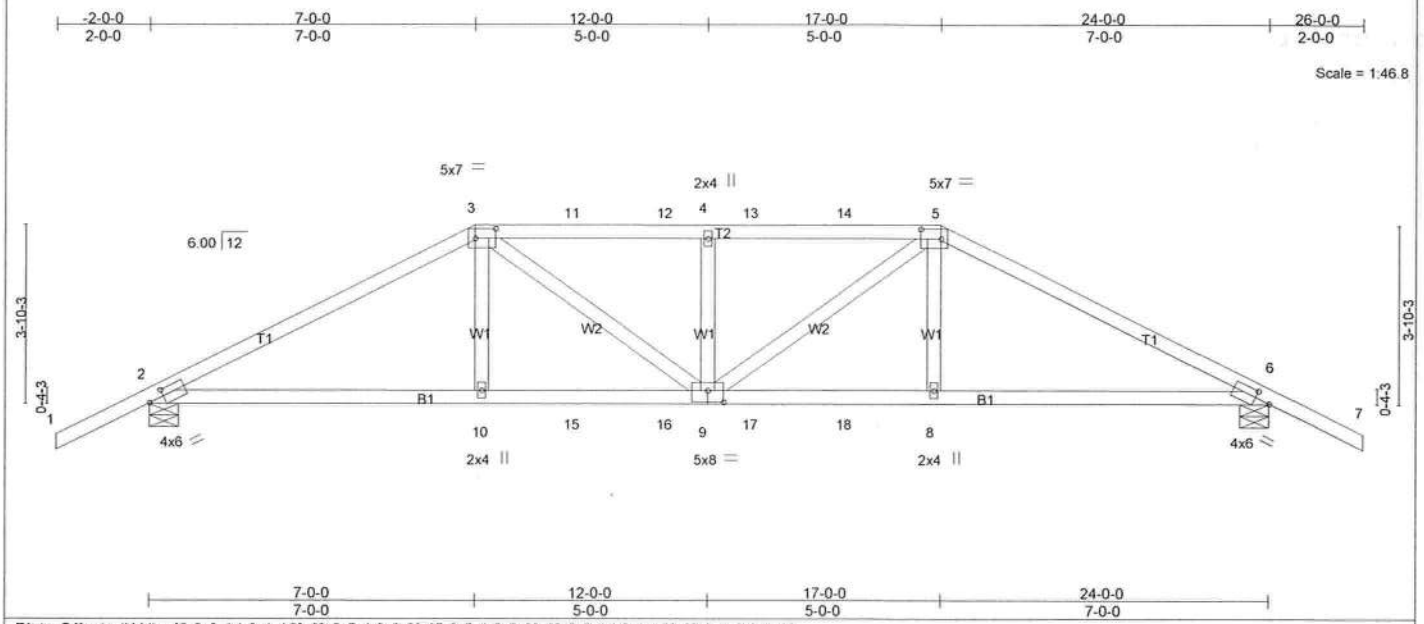


Plate Offsets (X,Y): [2:0-3-14,0-1-12], [3:0-5-4,0-2-8], [5:0-5-4,0-2-8], [6:0-3-14,0-1-12], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.12	9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.24	8-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.38	Horz(TL)	-0.09	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.26	8-9	>999	240	Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1499/0-7-8, 6=1499/0-7-8
Max Horz 2=97(LC 5)
Max Uplift 2=-1399(LC 5), 6=-1389(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2564/2512, 3-11=-2668/2701, 11-12=-2667/2701, 4-12=-2667/2701, 4-13=-2667/2701, 13-14=-2667/2701, 5-14=-2668/2701, 5-6=-2564/2495
BOT CHORD 2-10=-2164/2186, 10-15=-2181/2198, 15-16=-2181/2198, 9-16=-2181/2198, 9-17=-2145/2198, 17-18=-2145/2198, 8-18=-2145/2198, 6-8=-2127/2187
WEBS 3-10=-432/493, 3-9=-627/661, 4-9=-569/428, 5-9=-608/642, 5-8=-432/493

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1399 lb uplift at joint 2 and 1389 lb uplift at joint 6.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 195 lb down and 254 lb up at 7-0-0, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 11-0-12, 97 lb down and 103 lb up at 12-11-4, and 97 lb down and 103 lb up at 14-11-4, and 235 lb down and 254 lb up at 17-0-0 on top chord, and 248 lb down and 412 lb up at 7-0-0, 63 lb down and 90 lb up at 9-0-12, 63 lb down and 90 lb up at 11-0-12, 63 lb down and 90 lb up at 12-11-4, and 63 lb down and 90 lb up at 14-11-4, and 248 lb down and 412 lb up at 16-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 21,201

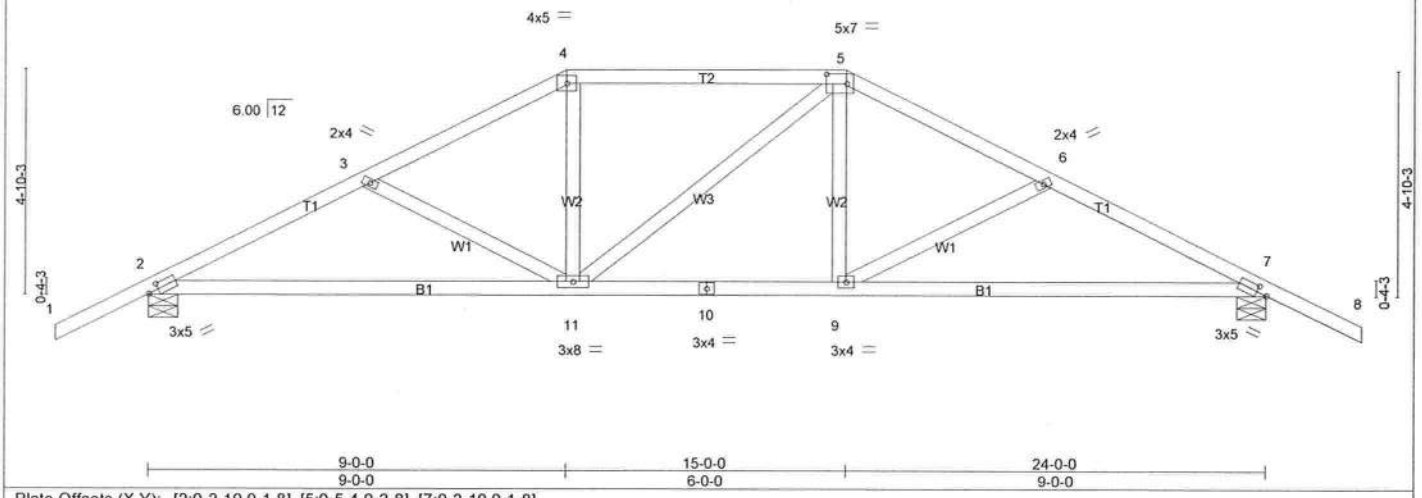
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	T02	HIP	1	1	I4576061
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:09 2010 Page 1

-2-0-0	4-9-5	9-0-0	15-0-0	19-2-11	24-0-0	26-0-0
2-0-0	4-9-5	4-2-11	6-0-0	4-2-11	4-9-5	2-0-0

Scale = 1:46.8



9-0-0	15-0-0	24-0-0
9-0-0	6-0-0	9-0-0

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [5:0-5-4,0-2-8], [7:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.16	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.30	7-9	>936	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.08	9-11	>999	240	Weight: 119 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-10 oc bracing.

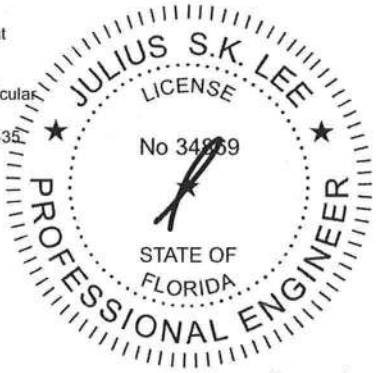
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=873/0-7-8, 7=873/0-7-8
Max Horz 2=112(LC 6)
Max Uplift 2=-363(LC 6), 7=-363(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1270/904, 3-4=-1048/775, 4-5=-896/760, 5-6=-1048/775, 6-7=-1270/904
BOT CHORD 2-11=-593/1062, 10-11=-371/896, 9-10=-371/896, 7-9=-593/1062
WEBS 3-11=-198/253, 4-11=-65/253, 5-9=-65/253, 6-9=-198/253

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 2 and 363 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



December 21,201

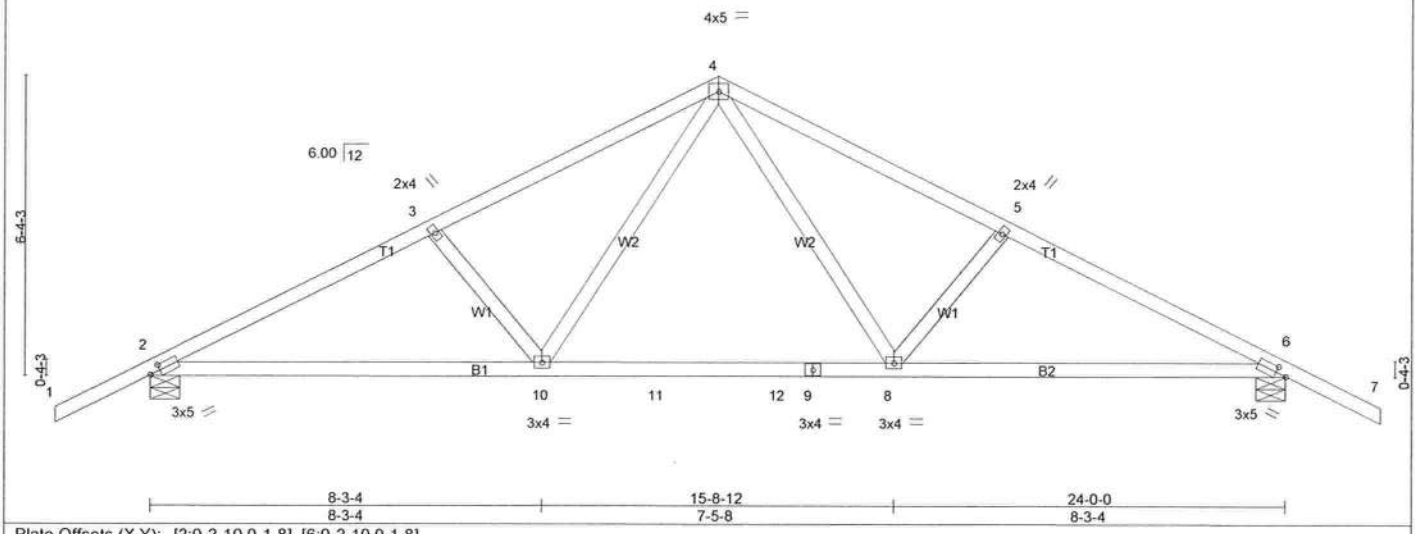
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	T04	COMMON	5	1	I4576063
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:10 2010 Page 1

-2-0-0 / 2-0-0 | 6-0-2 / 6-0-2 | 12-0-0 / 5-11-14 | 17-11-14 / 5-11-14 | 24-0-0 / 6-0-2 | 26-0-0 / 2-0-0

Scale = 1:46.0



8-3-4 / 8-3-4 | 15-8-12 / 7-5-8 | 24-0-0 / 8-3-4

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.15	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.21	8-10	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.09	8-10	>999	240	Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-4 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=924/0-7-8, 6=924/0-7-8
Max Horz 2=134(LC 6)
Max Uplift 2=-383(LC 6), 6=-383(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1393/923, 3-4=-1200/878, 4-5=-1200/878, 5-6=-1393/923
BOT CHORD 2-10=-604/1168, 10-11=-242/789, 11-12=-242/789, 9-12=-242/789, 8-9=-242/789, 6-8=-604/1168
WEBS 4-8=-267/423, 5-8=-282/359, 4-10=-267/423, 3-10=-282/359

NOTES (8-9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 383 lb uplift at joint 2 and 383 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 21,201

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	T06	MONO HIP	1	1	I4576065
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:11 2010 Page 1

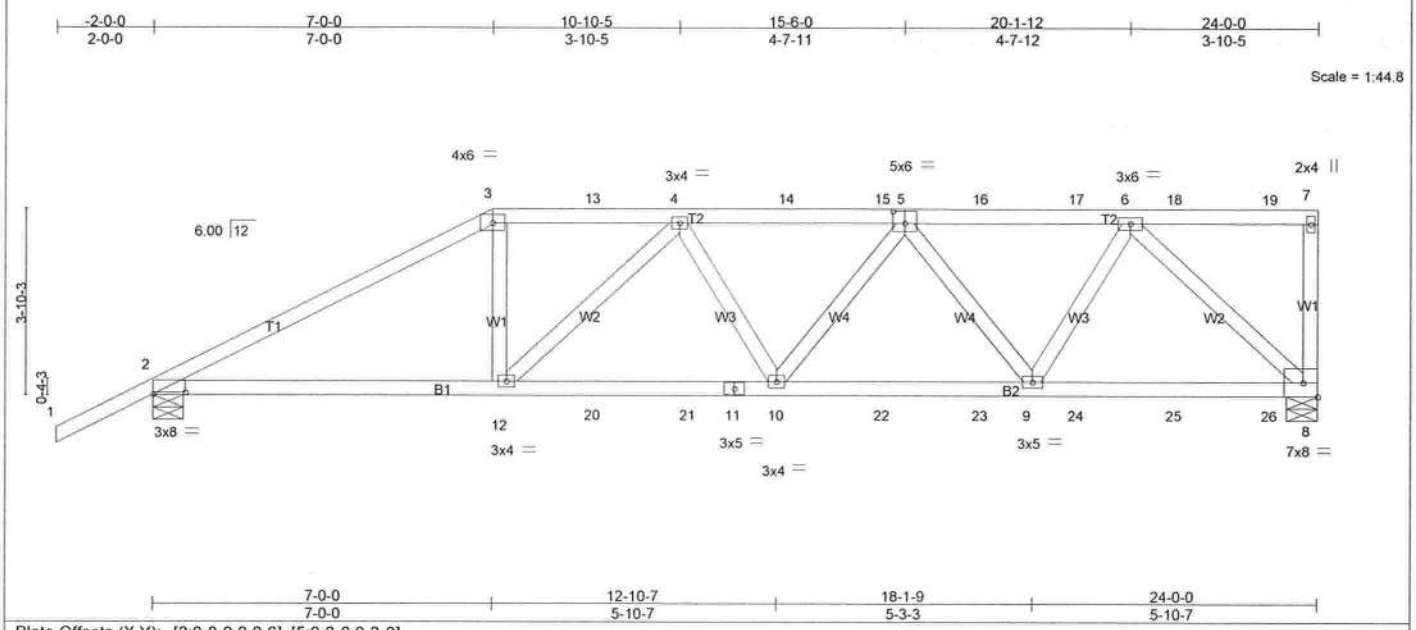


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.11	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.24	10-12	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.89	Horz(TL)	-0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.27	10-12	>999	240	Weight: 124 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-10-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=1529/0-7-8, 2=1486/0-7-8
Max Horz 2=205(LC 5)
Max Uplift 8=-1440(LC 4), 2=-1330(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2536/2403, 3-13=-2187/2191, 4-13=-2187/2191, 4-14=-2512/2446, 14-15=-2512/2446, 5-15=-2512/2446, 5-16=-1844/1766, 16-17=-1844/1766, 6-17=-1844/1766
BOT CHORD 2-12=-2161/2160, 12-20=-2475/2556, 20-21=-2475/2556, 11-21=-2475/2556, 10-11=-2475/2556, 10-22=-2240/2359, 22-23=-2240/2359, 9-23=-2240/2359, 9-24=-1278/1371, 24-25=-1278/1371, 25-26=-1278/1371, 8-26=-1278/1371
WEBS 3-12=-760/710, 4-12=-511/450, 5-10=-363/326, 5-9=-864/796, 6-9=-991/960, 6-8=-1871/1738

NOTES (10-11)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1440 lb uplift at joint 8 and 1330 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 195 lb down and 254 lb up at 7-0-0, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 11-0-12, 97 lb down and 103 lb up at 13-0-12, 97 lb down and 103 lb up at 15-0-12, 97 lb down and 103 lb up at 17-0-12, 97 lb down and 103 lb up at 19-0-12, and 97 lb down and 103 lb up at 21-0-12, and 97 lb down and 103 lb up at 23-0-12 on top chord, and 248 lb down and 412 lb up at 7-0-0, 63 lb down and 90 lb up at 9-0-12, 63 lb down and 90 lb up at 11-0-12, 63 lb down and 90 lb up at 13-0-12, 63 lb down and 90 lb up at 15-0-12, 63 lb down and 90 lb up at 17-0-12, 63 lb down and 90 lb up at 19-0-12, and 63 lb down and 90 lb up at 21-0-12, and 63 lb down and 90 lb up at 23-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

December 21,201(

Continued on page 2

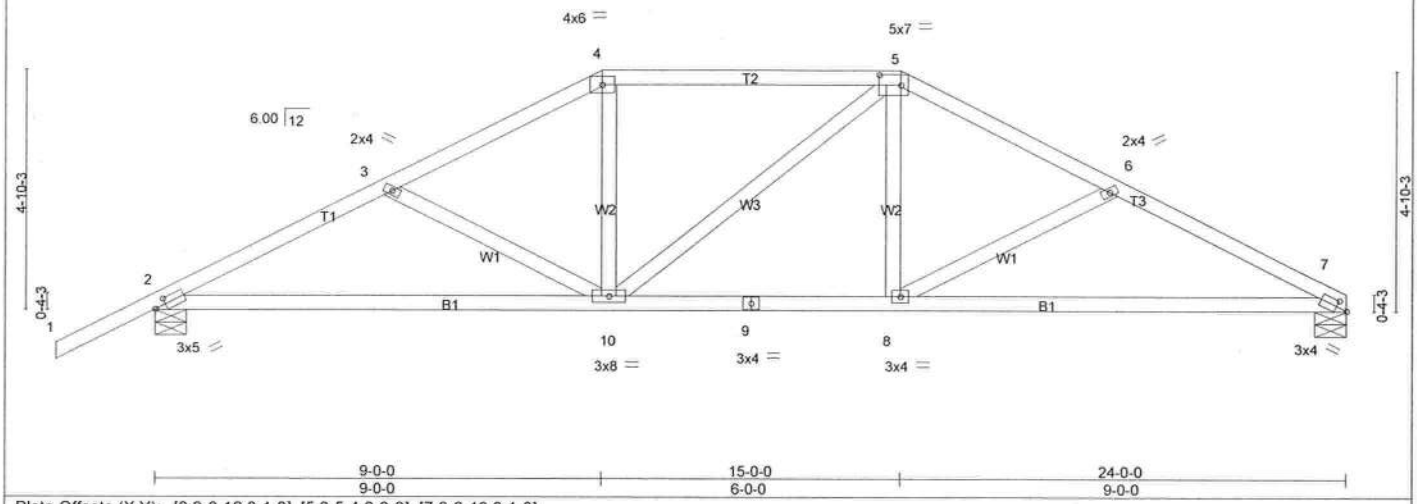
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.	I4576066
357050	T07	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:11 2010 Page 1

-2-0-0	4-9-5	9-0-0	15-0-0	19-2-11	24-0-0
2-0-0	4-9-5	4-2-11	6-0-0	4-2-11	4-9-5

Scale = 1:43.9



9-0-0	15-0-0	24-0-0
9-0-0	6-0-0	9-0-0

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [5:0-5-4,0-2-8], [7:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.17	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.32	7-8	>876	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.08	8-10	>999	240	Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-7 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=742/0-7-8, 2=879/0-7-8
Max Horz 2=129(LC 6)
Max Uplift 7=-228(LC 7), 2=-364(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1282/930, 3-4=-1061/802, 4-5=-908/784, 5-6=-1074/829, 6-7=-1308/1007
BOT CHORD 2-10=-713/1073, 9-10=-512/918, 8-9=-512/918, 7-8=-810/1119
WEBS 3-10=-198/251, 4-10=-75/254, 5-8=-118/260, 6-8=-238/339

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 7 and 364 lb uplift at joint 2.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 21,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	T09	HIP	1	1	I4576068
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:12 2010 Page 1

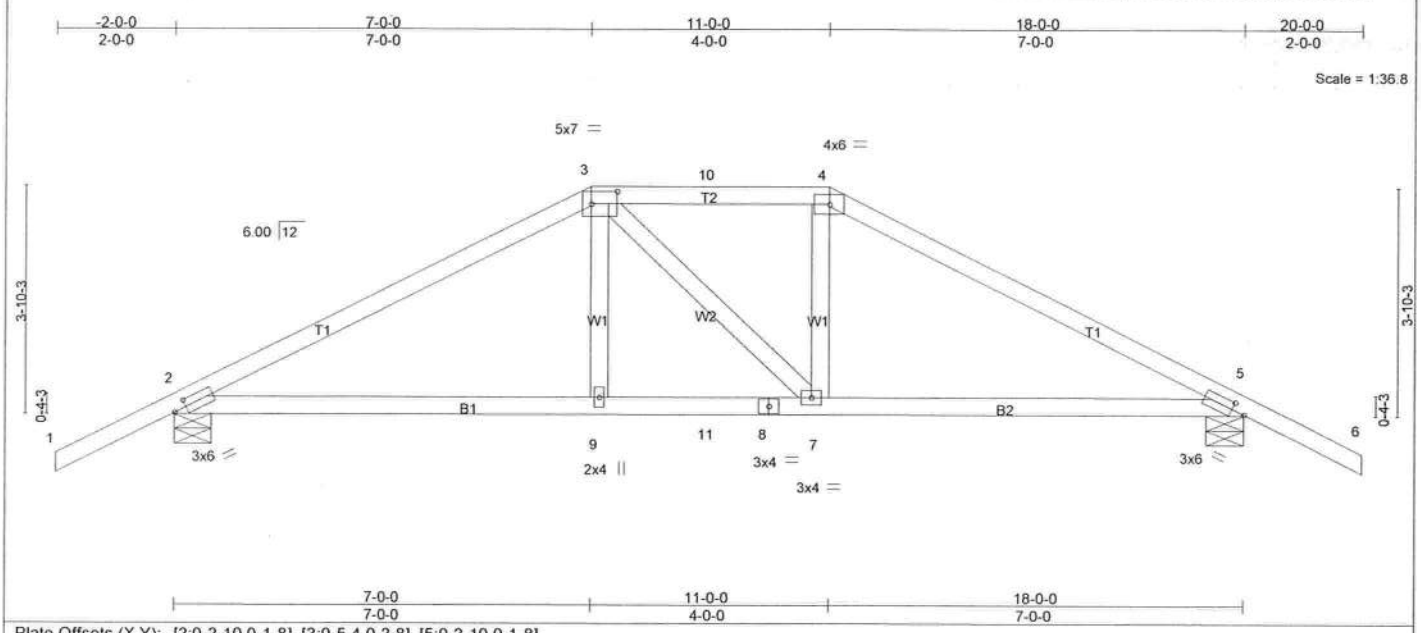


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-5-4,0-2-8], [5:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.07	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.15	2-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.14	Horz(TL)	-0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.17	2-9	>999	240	Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-8-3 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=1117/0-7-8, 2=1117/0-7-8
Max Horz 2=-97(LC 6)
Max Uplift 5=-1257(LC 6), 2=-1266(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1746/2025, 3-10=-1474/1849, 4-10=-1474/1849, 4-5=-1747/2025
BOT CHORD 2-9=-1737/1463, 9-11=-1758/1474, 8-11=-1758/1474, 7-8=-1758/1474, 5-7=-1717/1464
WEBS 3-9=-532/448, 4-7=-546/449

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1257 lb uplift at joint 5 and 1266 lb uplift at joint 2.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 195 lb down and 254 lb up at 7-0-0, and 97 lb down and 103 lb up at 9-0-0, and 235 lb down and 254 lb up at 11-0-0 on top chord, and 248 lb down and 349 lb up at 7-0-0, and 63 lb down and 59 lb up at 9-0-0, and 248 lb down and 349 lb up at 10-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-6=-54, 2-5=-10

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

Continued on page 2

December 21,201

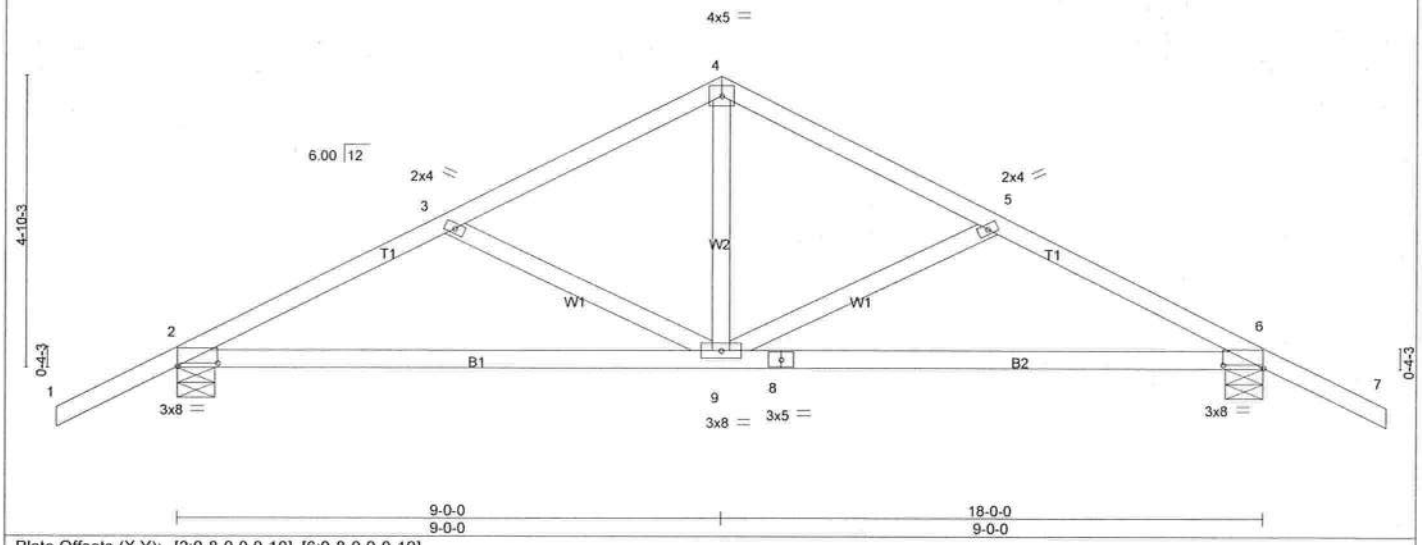
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK CONST. - RADZIMINSKI RES.
357050	T10	COMMON	3	1	I4576069
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Dec 21 09:08:13 2010 Page 1

-2-0-0 / 2-0-0 | 4-7-1 / 4-7-1 | 9-0-0 / 4-4-15 | 13-4-15 / 4-4-15 | 18-0-0 / 4-7-1 | 20-0-0 / 2-0-0

Scale = 1:36.1



9-0-0 / 9-0-0 | 18-0-0 / 9-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [6:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.11	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.19	2-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.34	Horz(TL)	-0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.39	2-9	>535	240	Weight: 84 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-5 oc bracing.

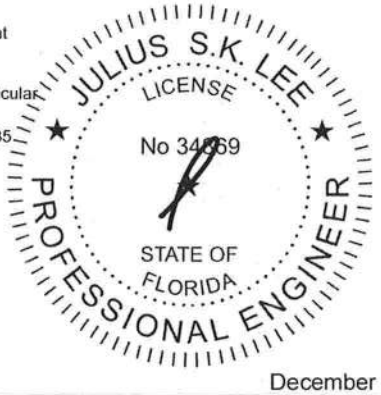
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=681/0-7-8, 2=681/0-7-8
Max Horz 2=-112(LC 7)
Max Uplift 6=-595(LC 7), 2=-595(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-885/1469, 3-4=-647/1308, 4-5=-647/1308, 5-6=-885/1469
BOT CHORD 2-9=-1113/727, 8-9=-1113/727, 6-8=-1113/727
WEBS 4-9=-920/358, 5-9=-239/300, 3-9=-239/300

NOTES (8-9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 6 and 595 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



December 21,201

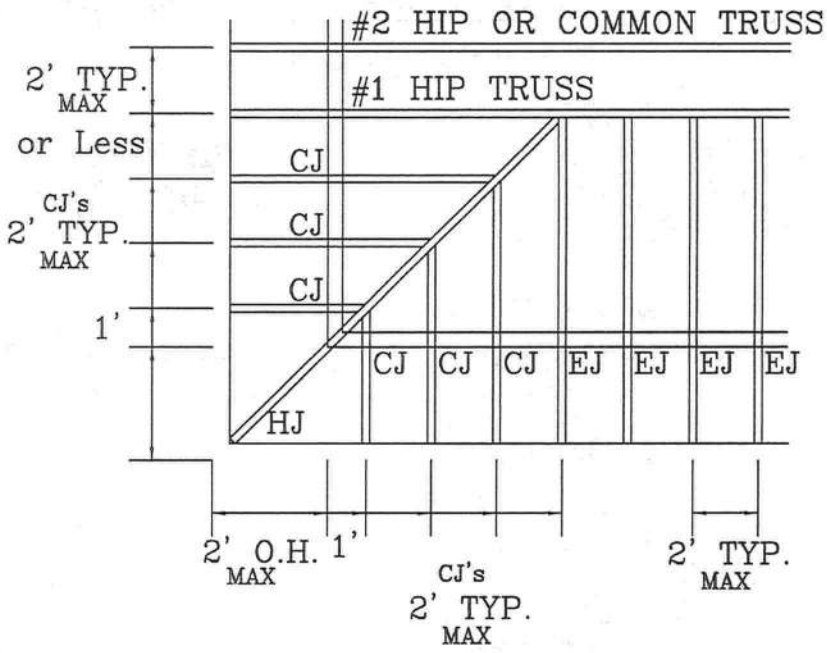
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

STEPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

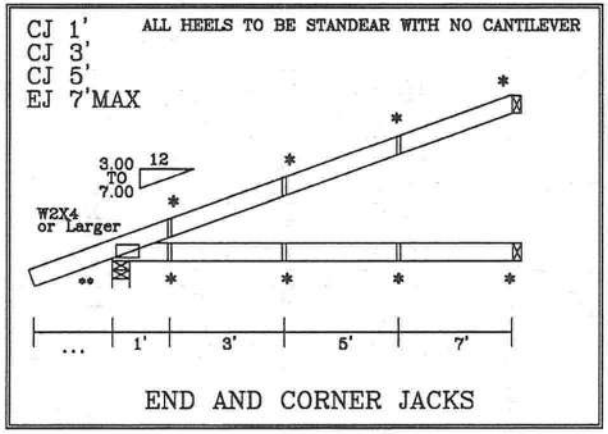
Setback 7' or Less



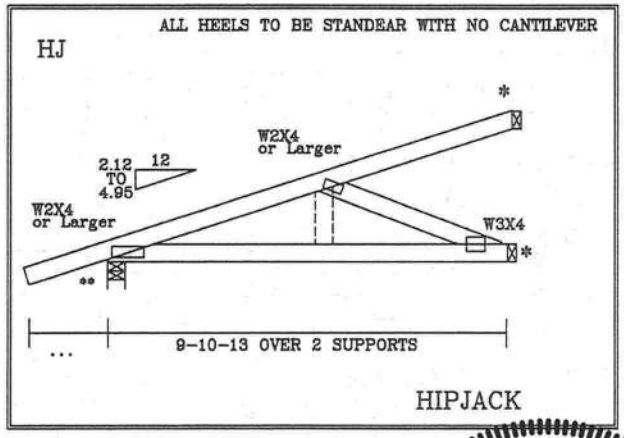
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



END AND CORNER JACKS



HIPJACK

* (3) 16d TOENAILS

** SEE EOR FOR TIE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET SETBACK
7'0" MAX

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

JULIUS LEE CONS. ENGINEERS, P.A.
1455 SW 4TH AVENUE
DELRAY BEACH, FL 33444-2161
NO. 34869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SHINGLE			
TC	LL	20	MAX PSF
TC	DL	7	MAX PSF
BC	LL	10*	MAX PSF
BC	DL	5	MAX PSF
TILE			
TC	LL	20	MAX PSF
TC	DL	20	MAX PSF
BC	LL	10*	MAX PSF
BC	DL	5	MAX PSF
DUR. FAC.		1.25	
SPACING		2' MAX	

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
By julius lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 5"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 5"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 8"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

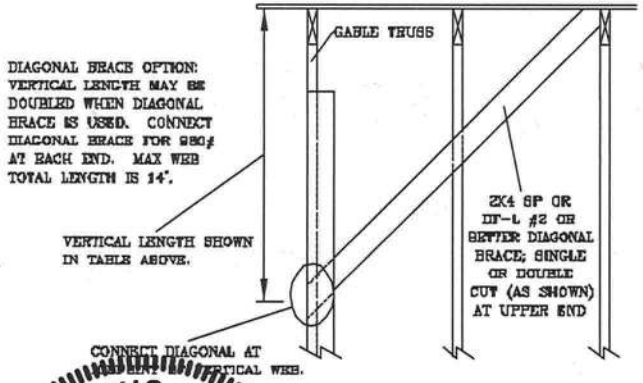
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

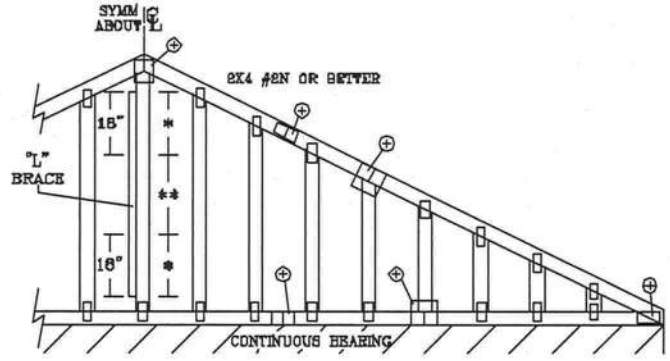
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	MID SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13030
DATE	11/26/03
DWG	MWFRS STD GABLE 30' E HT
	-ENG

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

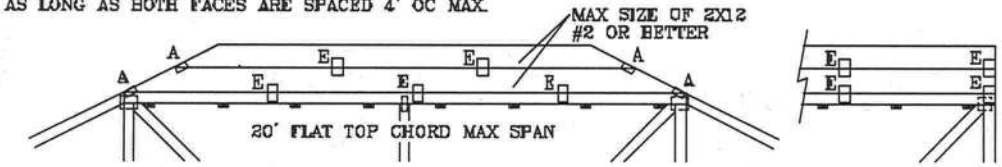
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

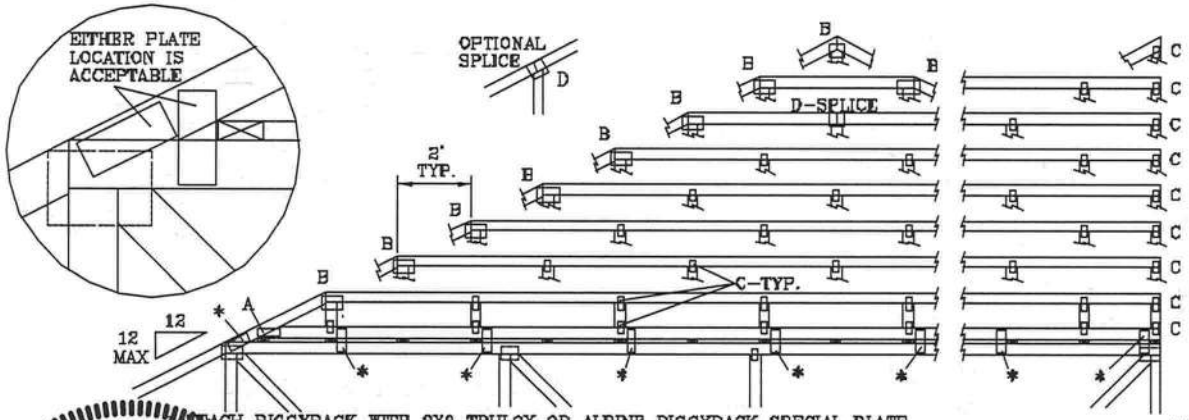
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

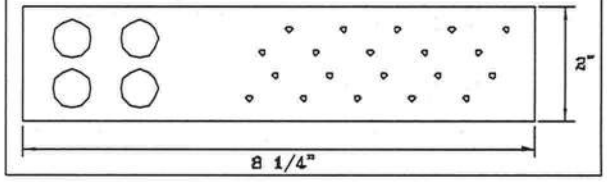
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 537199 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

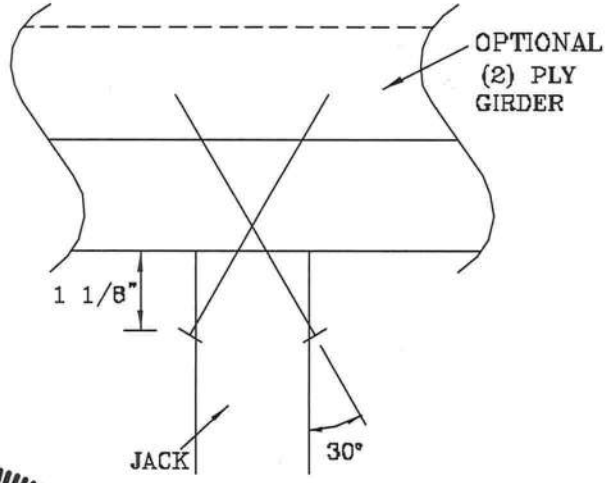
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

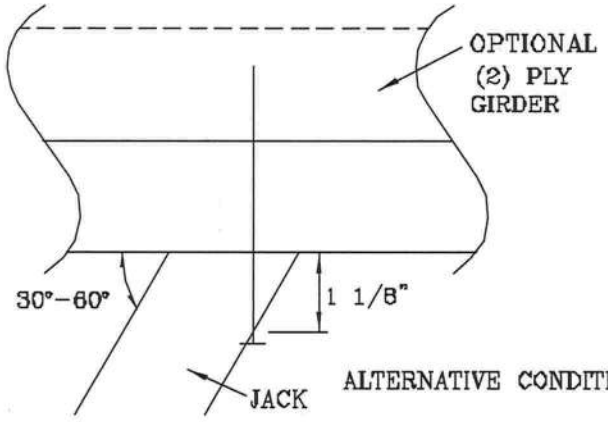
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

TRULOX CONNECTION DETAIL

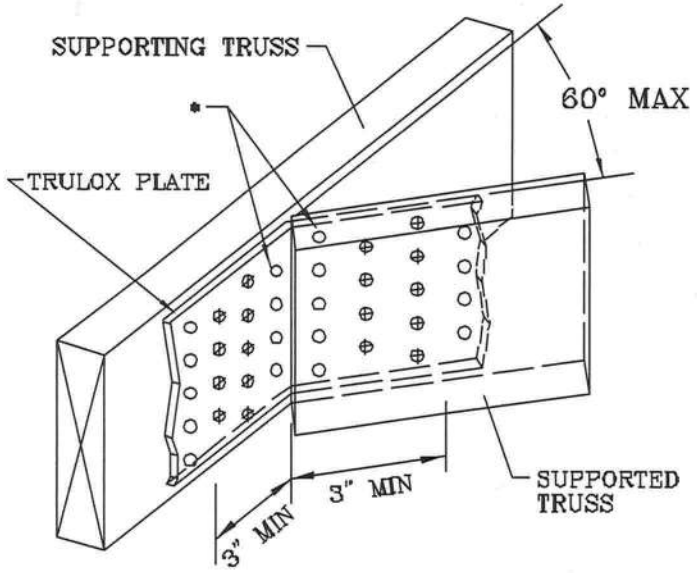
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

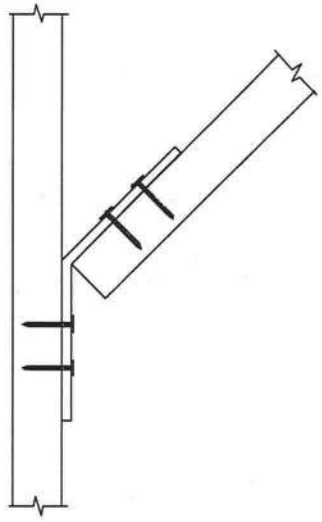
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

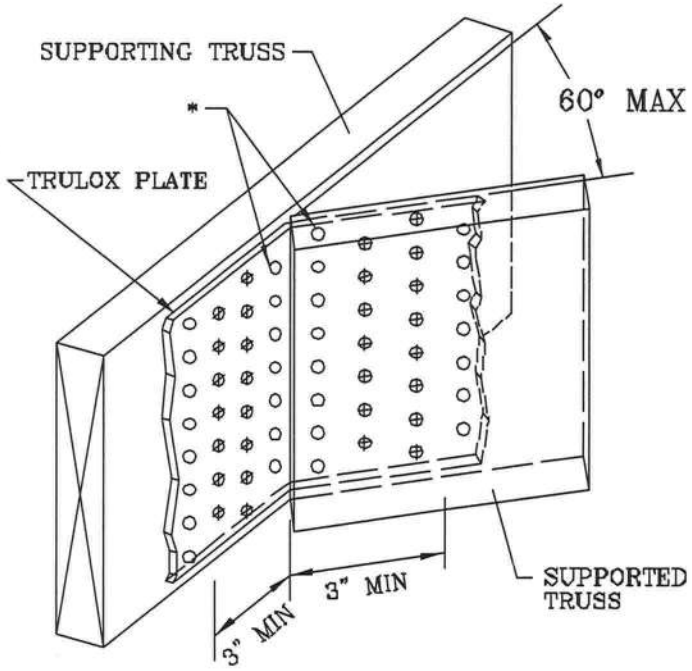
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERVISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35[4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6[3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅜" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9/16" maximum.

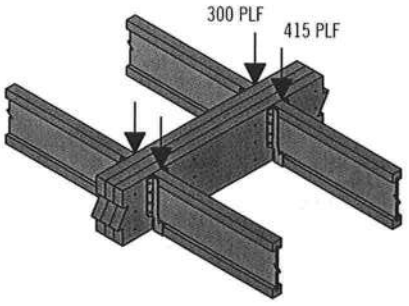
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

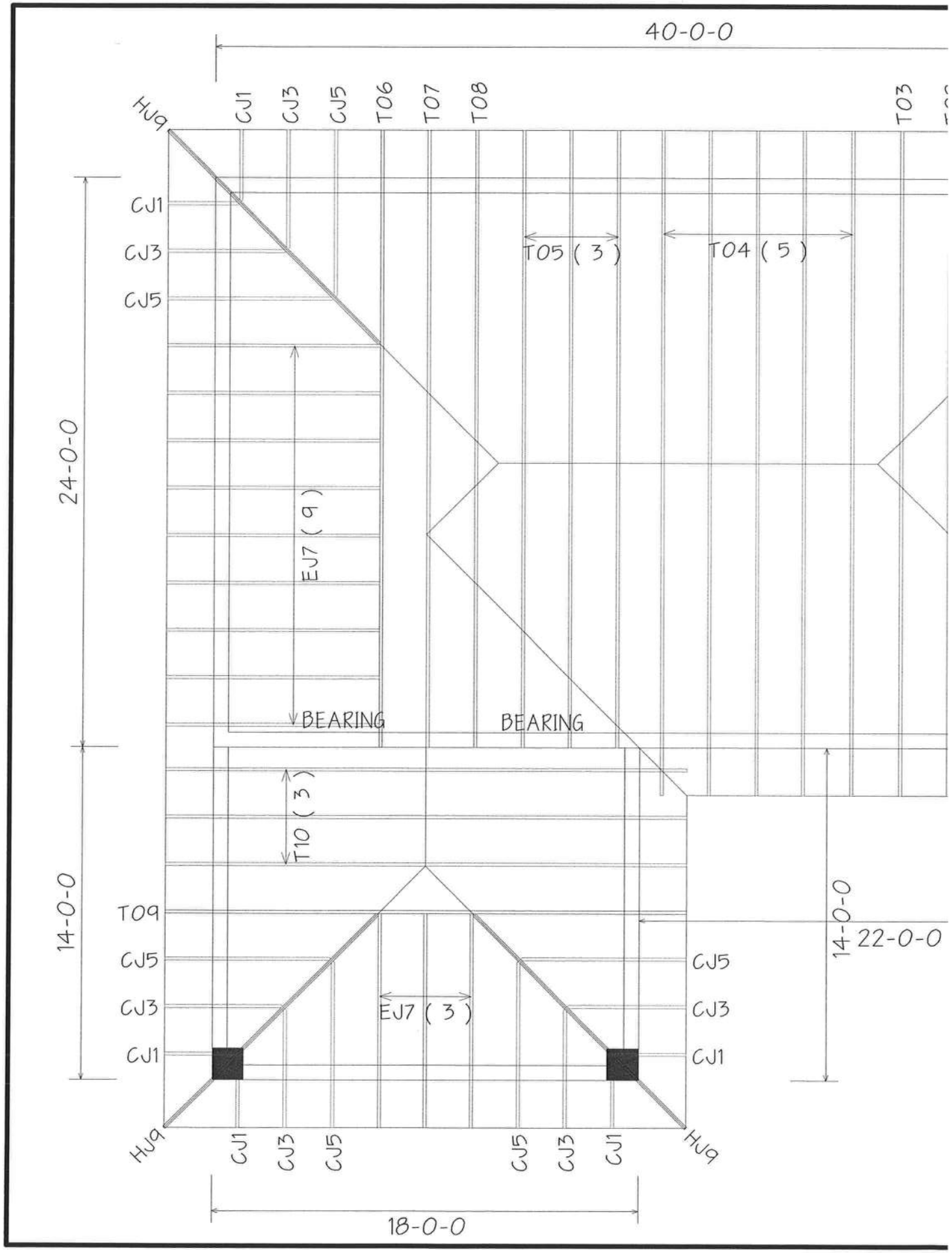
- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.


40-0-0
HJ9
CJ1
CJ3
CJ5
TO6
TO7
TO8
TO3
CJ1
CJ3
CJ5
TO5 (3)
TO4 (5)
24-0-0
EJ7 (9)
BEARING
BEARING
T10 (3)
14-0-0
TO9
CJ5
CJ3
CJ1
EJ7 (3)
14-0-0
22-0-0
CJ5
CJ3
CJ1
HJ9
CJ1
CJ3
CJ5
CJ5
CJ3
CJ1
HJ9
18-0-0

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 14-7S-16-04226-112 **Building permit No.** 000029123

Use Classification SFD SUITE **Fire:** 0.00

Permit Holder RONALD CLARK **Waste:**

Owner of Building MICHAEL & MARIA RADZIMINSKI **Total:** 0.00

Location: 158 SW CONESTOGA WAY, FORT WHITE, FL 32038

Date: 05/24/2011

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

NOTICE OF INSPECTION AND/OR TREATMENT

Date of Inspection

4/11/11

Date of Treatment

Aggressor 75WSP @ .05%

Pesticide Used

Sub-Termites

Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
13618 NW 270th Ave.
Alachua, FL 32615

Call: **386-418-4387**
for a free inspection & estimate

DW Pest Control, Inc.
13618 NW 270th Avenue
Alachua, FL 32615
(386)-418-4387

Permanent Notice Of Termite Protection

(As required by Florida Building Code (FBC) 104.2.7)

This structure has been treated for the prevention of subterranean termites by the above named company. An annual inspection and a renewal of the termite protection contract is necessary for continued protection. Call the phone number above for inspection and contract renewal.

153 SW Coventry Way White 2012

Address of treatment or lot/block of treatment — Renewal Date

NOTICE OF INSPECTION AND/OR TREATMENT

153 SW Coventry Way White

Address

Date of Inspection

2/08/11 29123

Date of Treatment - Full ☑ Spot ☐

Aggressor @ .05%

Pesticide Used

Sub-Termites

Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
13618 NW 270th Ave.
Alachua, FL 32615

Call: **386-418-4387**
for a free inspection & estimate

NOTICE OF INSPECTION AND/OR TREATMENT

Date of Inspection

4/11/11

Date of Treatment

Aggressor 75WSP @ .05%

Pesticide Used

Subterranean Termites

Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
13618 NW 270th Ave.
Alachua, FL 32615

Call: **386-418-4387**
for a free inspection & estimate

DW Pest Control, Inc.
13618 NW 270th Avenue
Alachua, FL 32615
(386)-418-4387

Permanent Notice Of Termite Protection

(As required by Florida Building Code (FBC) 104.2.7)

This structure has been treated for the prevention of subterranean termites by the above named company. An annual inspection and a renewal of the termite protection contract is necessary for continued protection. Call the phone number above for inspection and contract renewal.

153 SW Lewistown Alley White — 2012

Address of treatment or lot/block of treatment — Renewal Date

NOTICE OF INSPECTION AND/OR TREATMENT

153 SW Lewistown Alley White

Address

Date of Inspection

2/03/11 29123

Date of Treatment - Full ☑ Spot ☐

Aggressor @ .05%

Pesticide Used

Subterranean Termites

Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
13618 NW 270th Ave.
Alachua, FL 32615

Call: **386-418-4387**
for a free inspection & estimate

NOTICE OF INSPECTION AND/OR TREATMENT

153 SW Conestoga Hwy / Ft White
Address

Date of Inspection
2/08/11 29123

Date of Treatment - Full ☑ Spot ☐
Aggressor @ .05%
Pesticide Used

Sub-Termites
Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
13618 NW 270th Ave.
Alachua, FL 32615

Call: **386-418-4387**
for a free inspection & estimate

DW Pest Control, Inc.
13618 NW 270th Avenue
Alachua, FL 32615
(386)-418-4387

Permanent Notice Of Termite Protection

(As required by Florida Building Code (FBC) 104.2.7)

This structure has been treated for the prevention of subterranean termites by the above named company. An annual inspection and a renewal of the termite protection contract is necessary for continued protection. Call the phone number above for inspection and contract renewal.

153 SW Conestoga Hwy Ft White — 2/12
Address of treatment or lot/block of treatment — Renewal Date

---

NOTICE OF INSPECTION AND/OR TREATMENT

Date of Inspection
4/11/11
Date of Treatment

Aggressor 75WSP @ .05%
Pesticide Used

Sub-Termites
Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
13618 NW 270th Ave.
Alachua, FL 32615