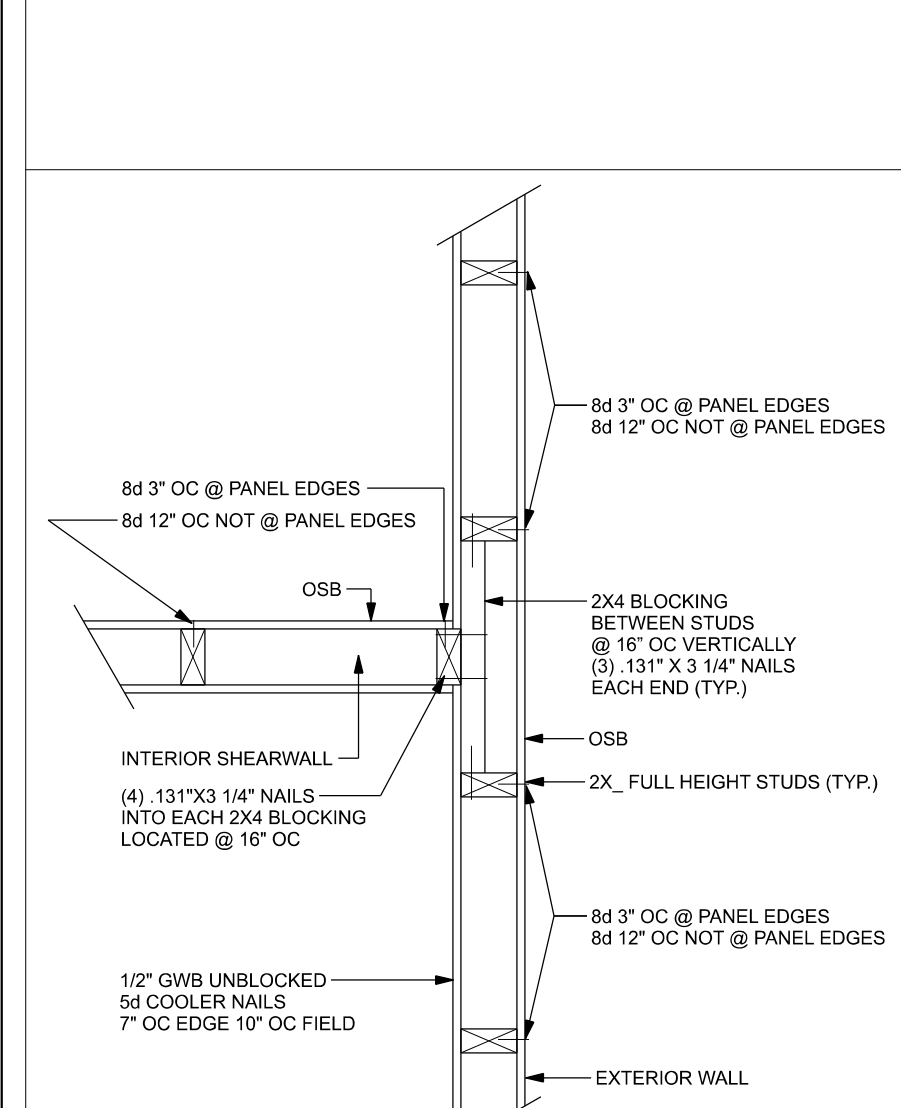
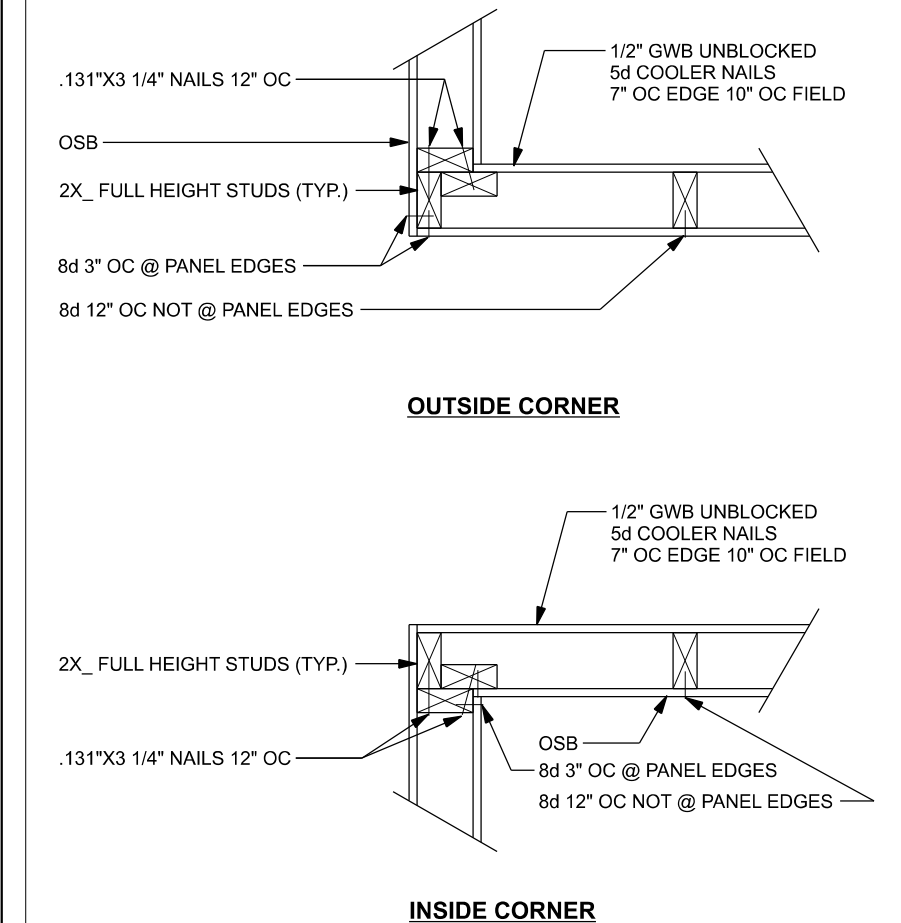


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



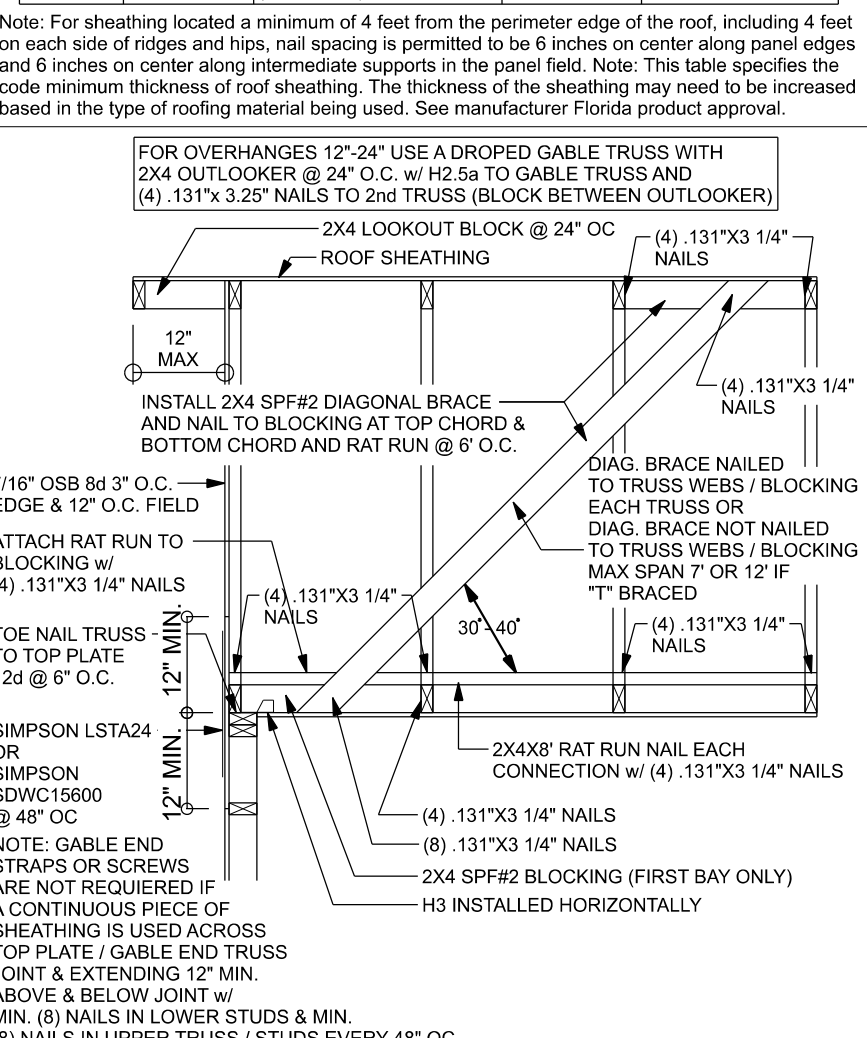
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



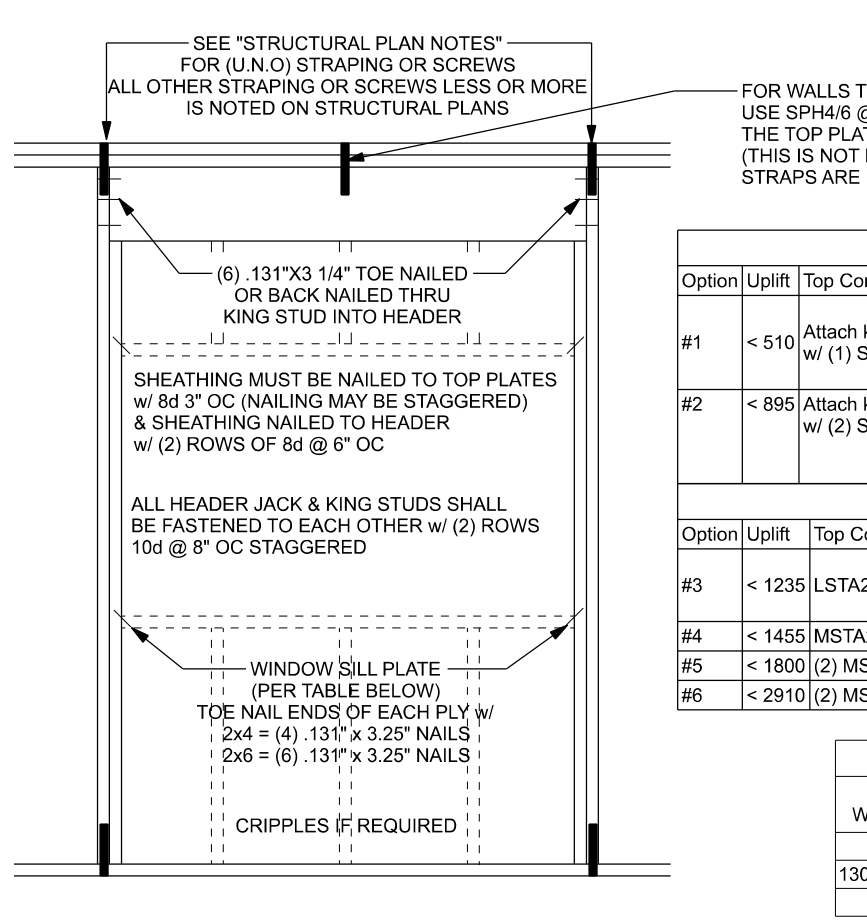
**(TYP.) CORNER FRAMING**  
WOOD FRAME

**ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)**

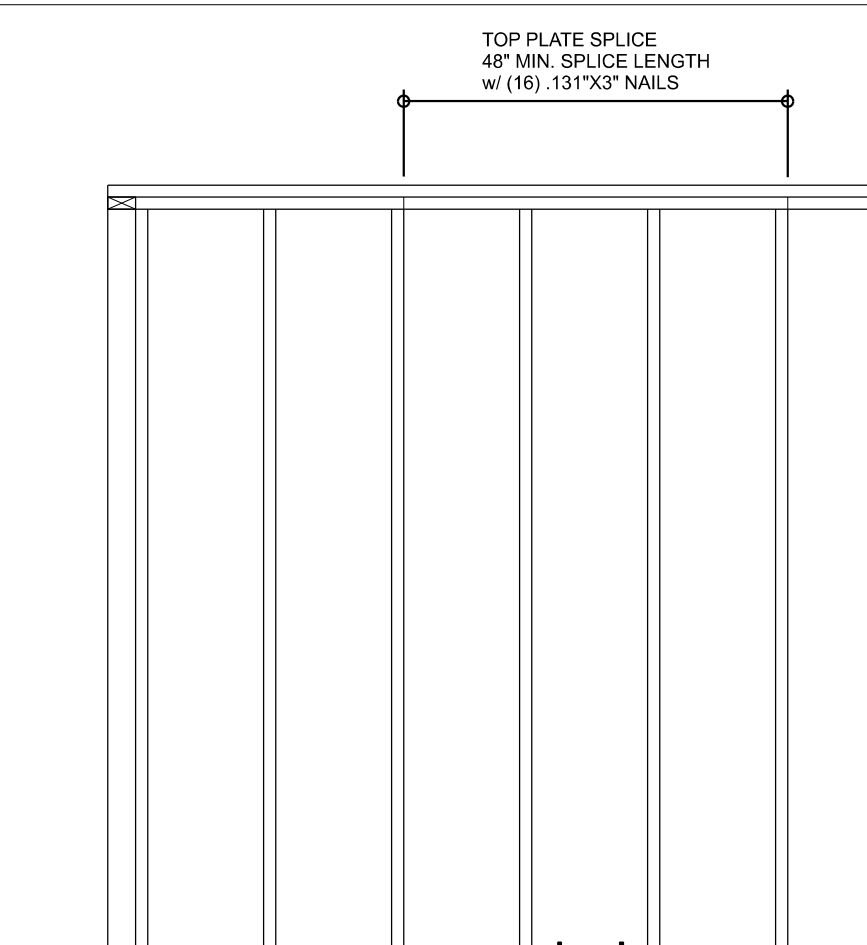
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
120 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	4" oc	4" oc



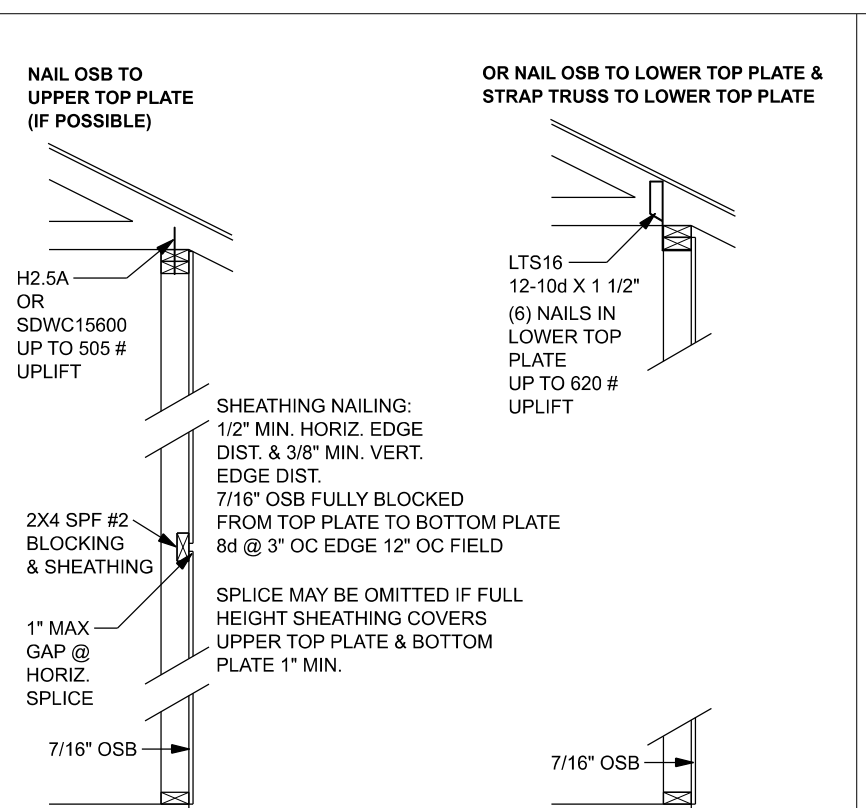
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



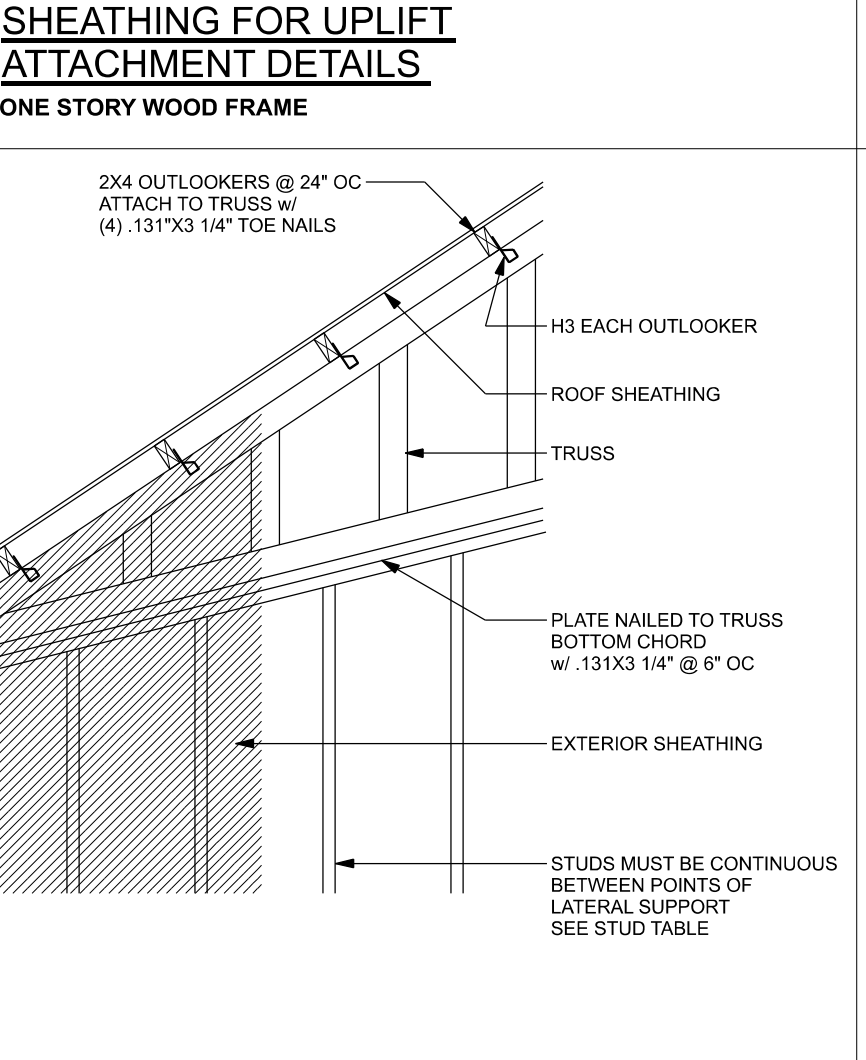
**TYPICAL HEADER STRAPPING OR SCREWS DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



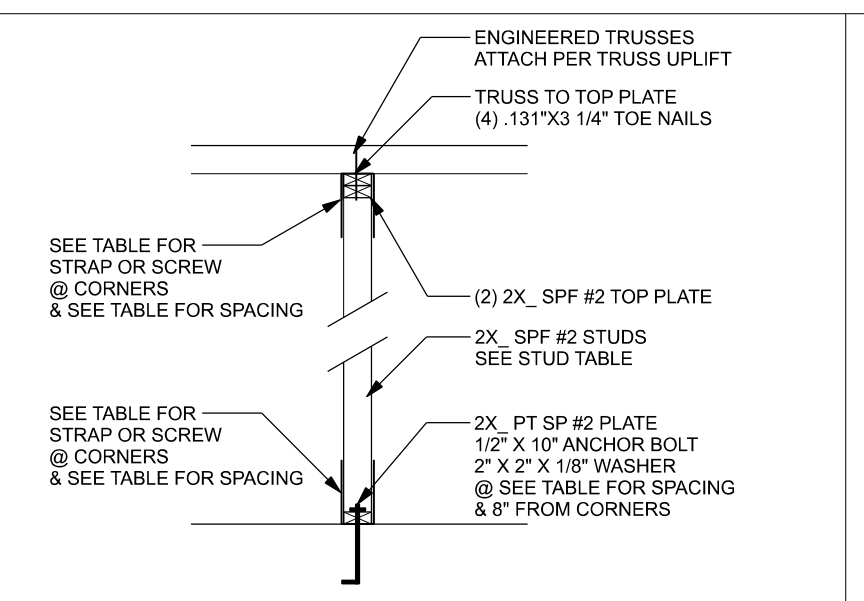
**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



**SHEATHING FOR UPLIFT ATTACHMENT DETAILS**  
ONE STORY WOOD FRAME

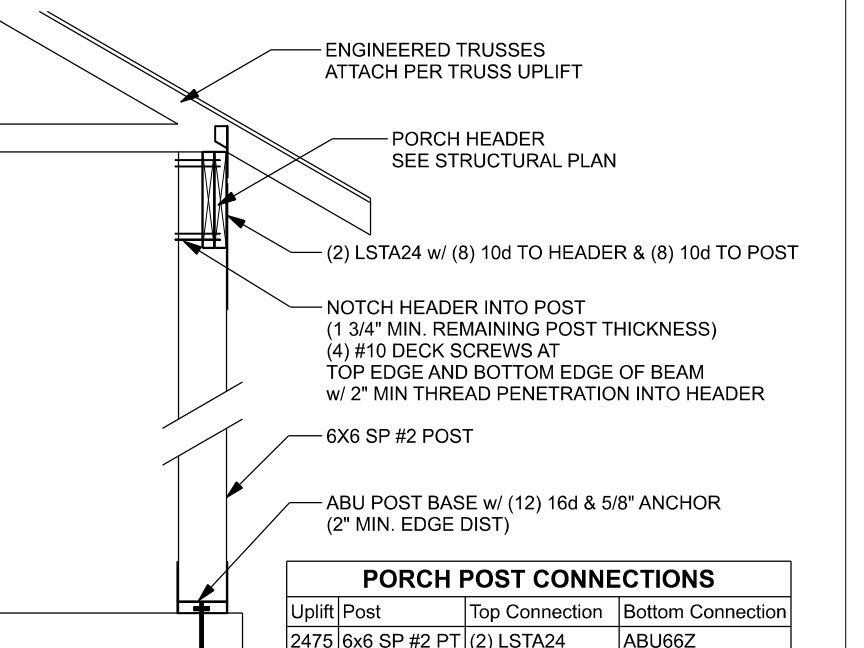


**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME

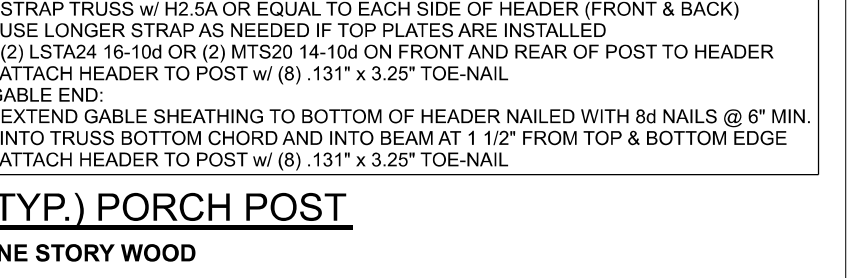


**INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE**

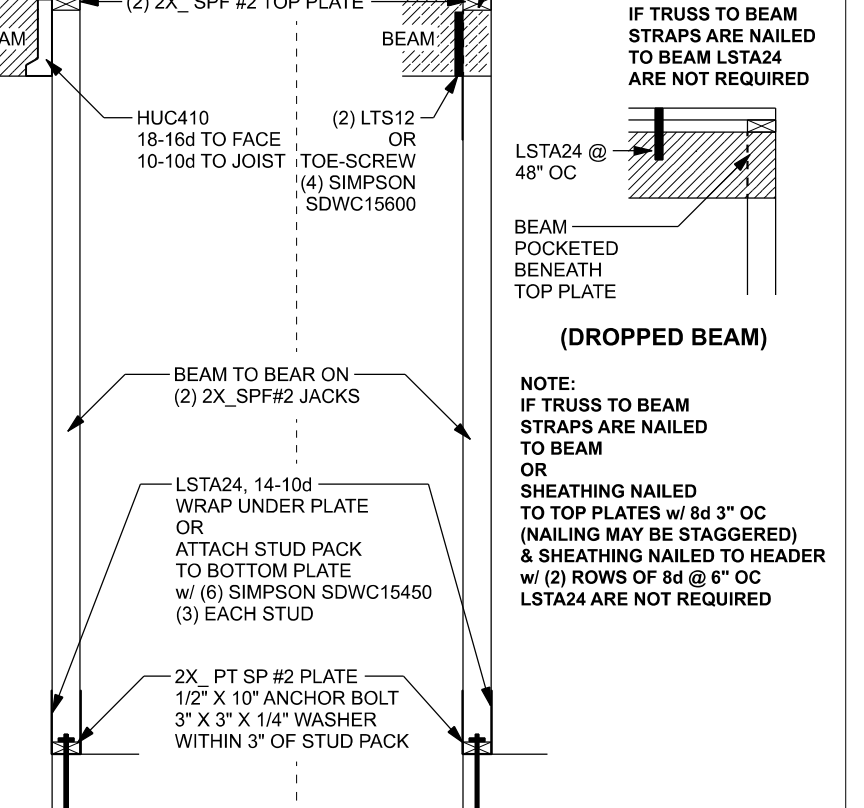
Uplift on wall	Top Connection	Bottom Connection	Anchor Bot Spacing
227 plf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 plf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
223 plf	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
336 plf	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
257 plf	SPH46, (12) 148" x 1 1/2" @ 48" OC	SPH46, (12) 148" x 1 1/2" @ 48" OC	48" OC
387 plf	SPH46, (12) 148" x 1 1/2" @ 32" OC	SPH46, (12) 148" x 1 1/2" @ 32" OC	48" OC
309 plf	LSTA24, (14) 148" x 1 1/2" @ 48" OC Wrap Under Plate	LSTA24, (14) 148" x 1 1/2" @ 48" OC	48" OC
465 plf	LSTA24, (14) 148" x 1 1/2" @ 32" OC Wrap Under Plate	LSTA24, (14) 148" x 1 1/2" @ 32" OC	48" OC



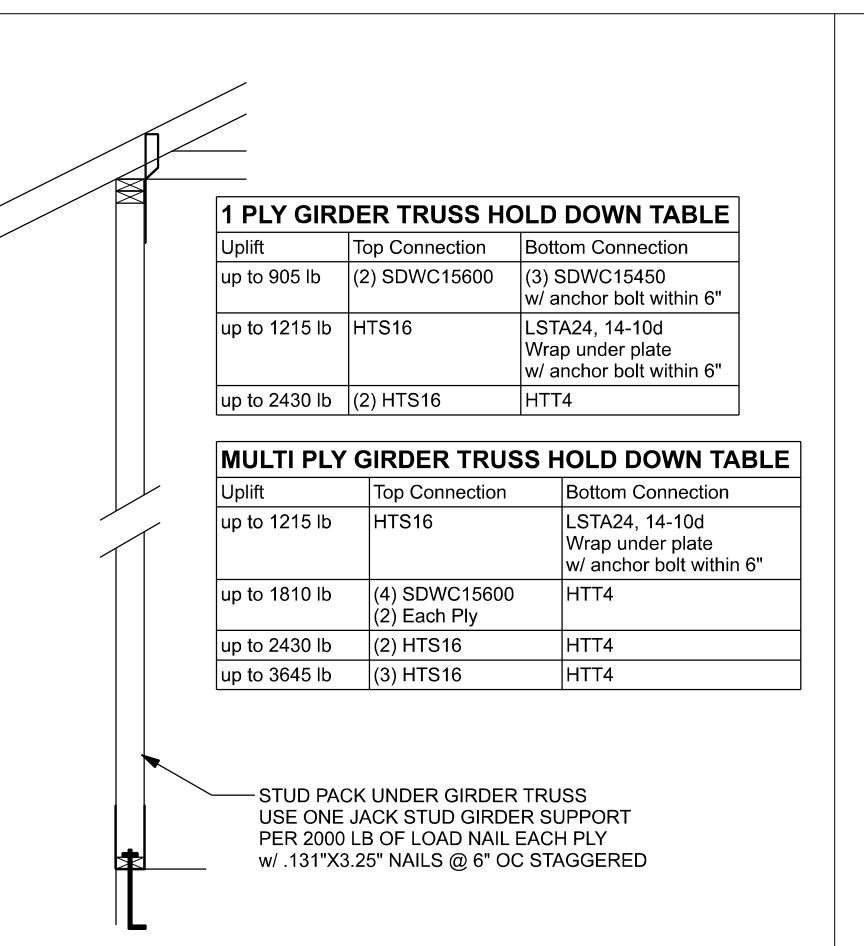
**(TYP.) INTERIOR BEARING WALL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



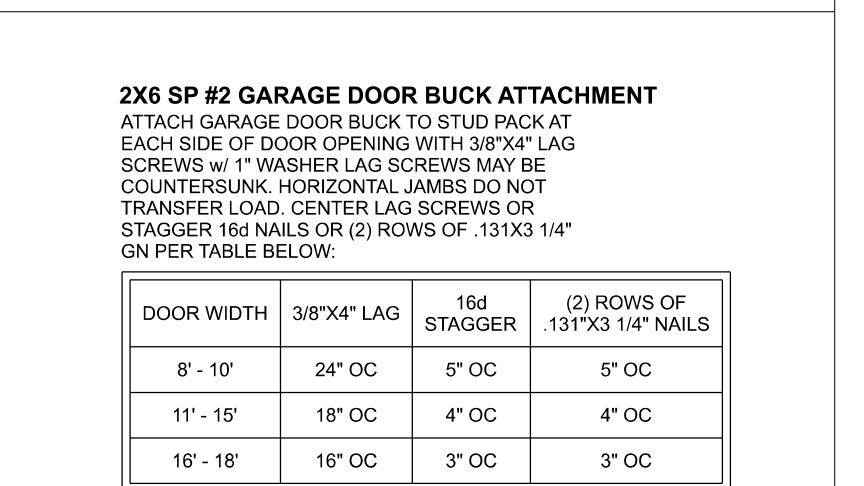
**(TYP.) PORCH POST**  
ONE STORY WOOD



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ STRAPS & ANCHORS



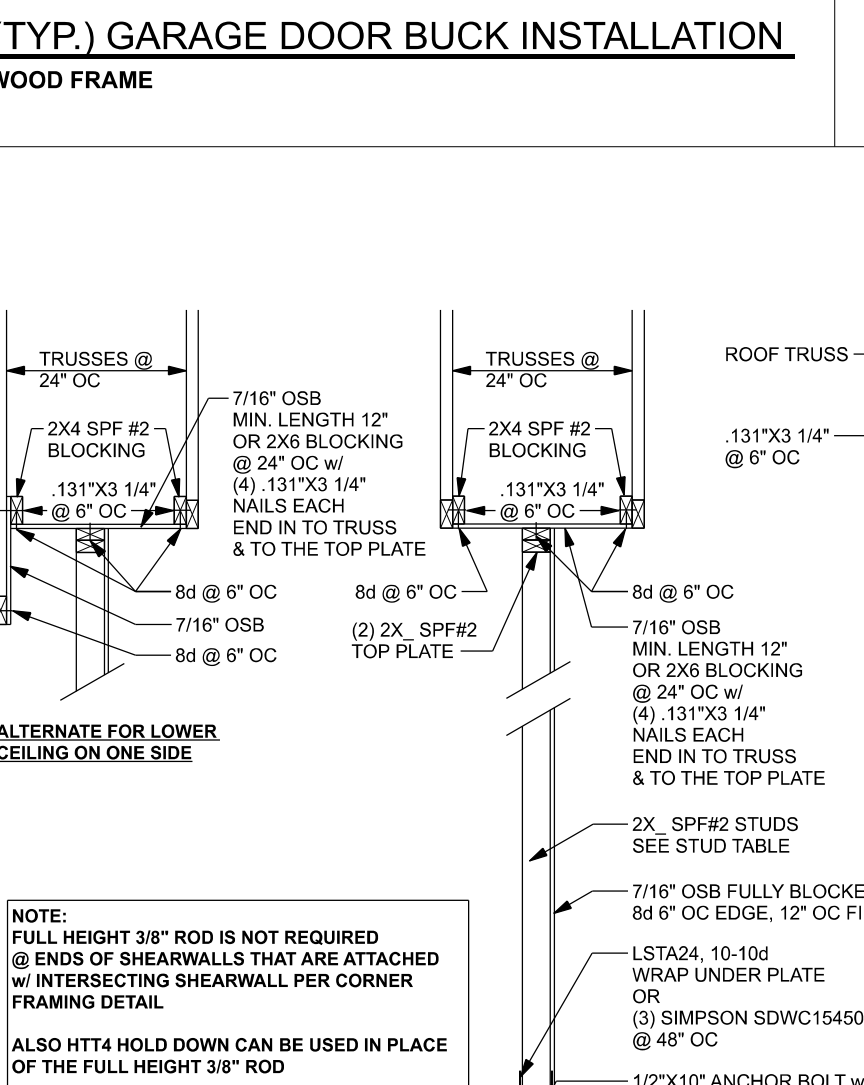
**(TYP.) GIRDER TRUSS HOLD DOWN DETAIL**  
WOOD FRAME w/ STRAPS & ANCHORS



**DOOR WIDTH**

Door Width	3/8" x 3/4" LAG	16d STAGGER	(2) ROWS OF 131x3 1/4" NAILS
8' - 11'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME



**INTERIOR SHEAR WALL**  
ONE STORY WOOD FRAME w/ STRAPS & AB

**CONNECTOR TABLE**

Uplift SP	Uplift SP#	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	-	-
400	290	H3	4-131x1 1/2"	4-131x1 1/2"
625	540	H2.5A	5-131x1 1/2"	5-131x1 1/2"
1040	1015	H10A	9-148x1 1/2"	9-148x1 1/2"
645	515	LTS12-20	6-148x1 1/2"	6-148x1 1/2"
990	850	MTS12-30	7-148x1 1/2"	7-148x1 1/2"
1415	1215	HTS16-30	8-148x1 1/2"	8-148x1 1/2"
Uplift SP	Uplift SP#	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-148x1 1/2"	8-148x1 1/2"
1640	1460	MSTA24	9-148x1 1/2"	9-148x1 1/2"
1030	1030	CS20	7-148x1 1/2"	7-148x1 1/2"
Uplift SP	Uplift SP#	Stud Plate Ties	To Stud	To Plate
555	535	SP1	4-148x3"	4-148x3"
1010	805	SP2	6-148x3"	6-148x3"
1280	1100	SPH46	12-148x1 1/2"	wrap under or over plate
771	771	LSTA24	10-148x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-148x1 1/2"	wrap under or over plate
Uplift SP	Uplift SP#	Holdowns @ Stewall	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	12"x12" Titen HD
4235	3640	HTT4	18-162x2 1/2"	12"x12" Titen HD
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	12"x12" Titen HD
4235	3640	HTT4	18-162x2 1/2"	12"x12" Titen HD
Uplift SP	Uplift SP#	Post Bases @ Stewall	To Post	Anchor
1900	1900	ABU442	12-162x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU662	12-162x3 1/2"	5/8"x12" Drill & Epoxy
1900	1900	ABU442	12-162x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU662	12-162x3 1/2"	5/8"x12" Drill & Epoxy

**STUD HEIGHT TABLE FOR SPF #2 STUDS:**

DOOR WIDTH	3/8" x 3/4" LAG	16d STAGGER	(2) ROWS OF 131x3 1/4" NAILS
8' - 11'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

**THIS STUD HEIGHT TABLE IS PER 2017 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)**

DOOR WIDTH	3/8" x 3/4" LAG	16d STAGGER	(2) ROWS OF 131x3 1/4" NAILS
8' - 11'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC

**GRADE & SPECIES TABLE**

	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED WITH THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON THE BUILDING STRUCTURE. STRAP 2X4 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 2500$  PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W4 x W14. FB = 85KSI. WELDED WIRE REINFORCEMENT (WWR) CONFORMING TO ASTM A185. LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/WR OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. JOINTS TO AVOID INTERFERENCE WITH TRUSS TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAP SPICES 40" DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FIBER REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OR A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

Amira Custom Homes

Bousquet Residence

PROJECT ADDRESS:  
SW 1st Avenue  
(Columbia County), FL 32643

FL PE 53915

This item has been digitally signed and sealed by Mark Disoway P.E. on digital signature data. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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CN-Mark d  
Disoway  
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(9)

DIMENSIONS:  
Scaled dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

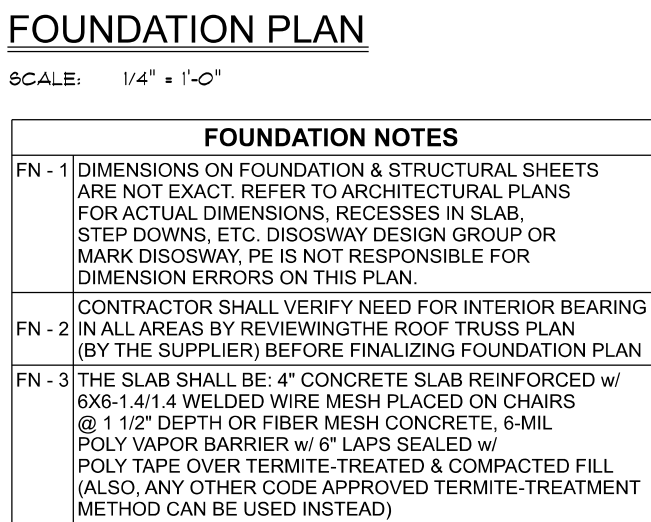
Mark Disoway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

JOB NUMBER:  
2503070

**S-1**  
OF 3 SHEETS




<b>MASONRY NOTE:</b>	
<b>MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF SPECIFICATION FOR MASONRY STRUCTURES' (ACI 530.1-02, TYPE N, UNO). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE SPECIFICATIONS, WITH ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.</b>	
	Specific Requirements
1.AA	ACI530.1-02 Section 6.3 brick facing with Fm = 1500 psi
2.1	ASTM C 270, Type N, UNO
2.2	Mortar ASTM 4, grade 4, admixtures require approval
2.3	CMU standard ASTM C 90, Normal weight, hollow, medium surface finish, 8" thick, 16" high, 16" wide, 16" deep and 12"x12"x16" 16"x16" column block.
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.5"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 60, fy = 40 ksi, max 1/4" lap steel.
2.4F	Coating for corrosion protection Anchors, steel metal ties completely embedded in mortar or grout, ASTM A305, Grade 60, 0.60 surface 304SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or water, steel, anchors, steel metal ties not completely embedded in mortar or grout, ASTM A193, Class B2, 1.50 x 0.12 304SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints. If not shown on project drawings, use 1/2" x 1/2" x 1/2" expansion joint.



FL P.E. was digitally signed by  
This item has been digitally signed and sealed by  
Mark Disoway P.E.  
Printed copies of this document are not considered  
signed and sealed and the signature must be  
viewed on any electronic copy.

**C=US,  
o=Unaffiliated,  
ou=Qualifiaid,A014  
10C000000A014  
A3F70003AF06,  
CN=Mark D  
Disoway**  
**2025-04-14 09:15:  
31**

  
The seal is circular with a red border. Inside the border, it says "NO. 53045" at the top, "Mark Disoway" in the middle, "Digital Signature" below that, and "P.E." at the bottom. In the center of the seal, there is a small graphic of a person's head and shoulders.

**DIMENSIONS:**  
Scaled dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disoway, P.E. for resolution.  
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**CERTIFICATION:** I hereby certify that I have  
examined this plan, and the scaleable  
portions of the plan, relating to ward engineering  
with the 8th Edition Florida Building Code Residential (2023)  
to the best of my knowledge.

**LIMITATION:** This design is valid for one  
building, at specified location.

**Mark Disoway P.E.**  
**163 SW Midtown Place**  
**Suite 103**  
**Lake City, Florida 32025**  
**386.754.5419**  
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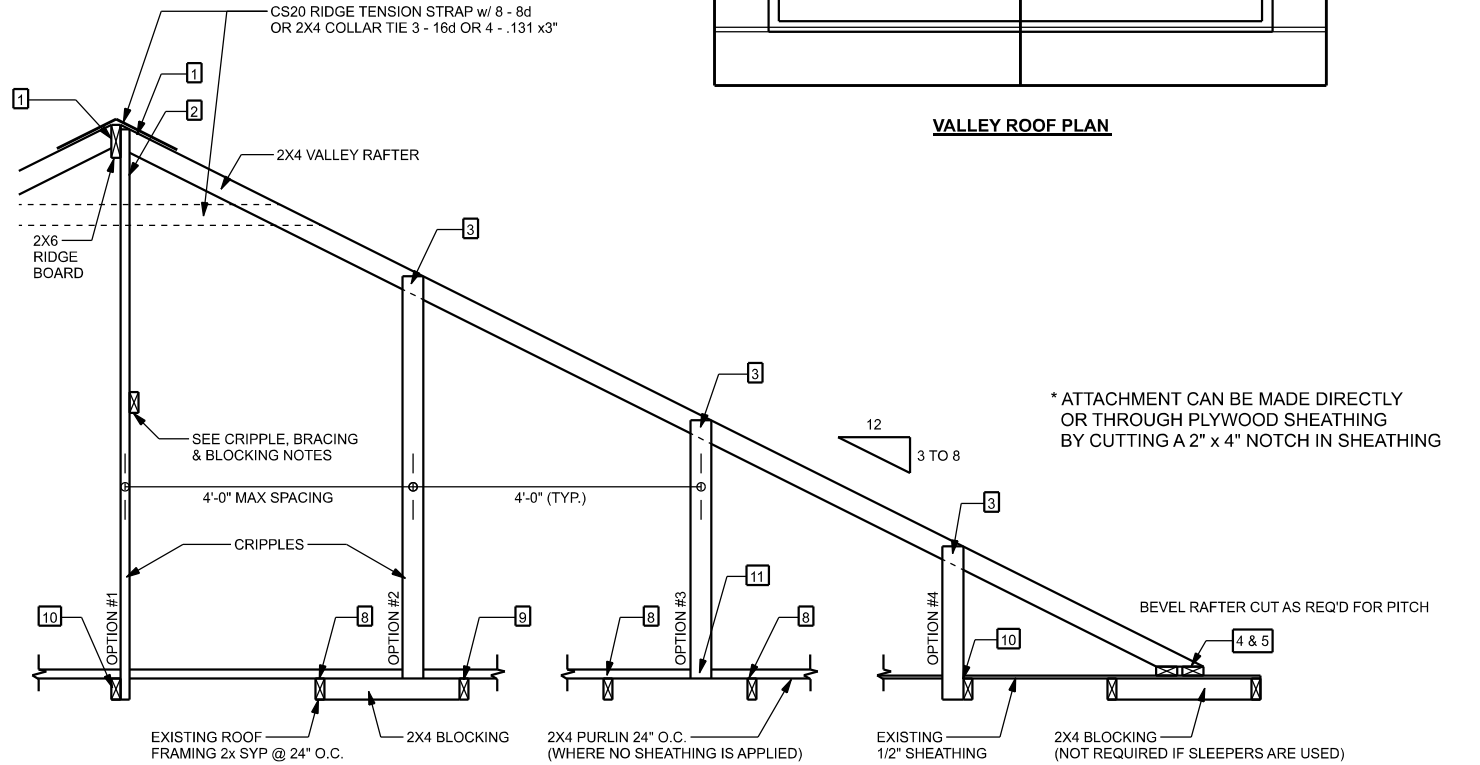
**JOB NUMBER:**  
**250370**

**S-**  
**2**  
**OF 3 SHEETS**



# LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

TRUSS	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLIN (LATERAL BRACING)	2X4 SPF #2
SLEEPERS	2" WIDTH OF RAFTER (BEAT CUT) SPF #3 OR 2" PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



## SECTION CUT PARALLEL TO VALLEY RAFTER

### VALLEY ROOF PLAN MEMBER LEGEND

TRUSS	2X6 SYP #2
TRUSS UNDER VALLEY FRAMING	2X6 SYP #2
VALLEY RAFTER OR RIDGE	2X4 SYP #2
CRIPPLE	2X4 SPF #2

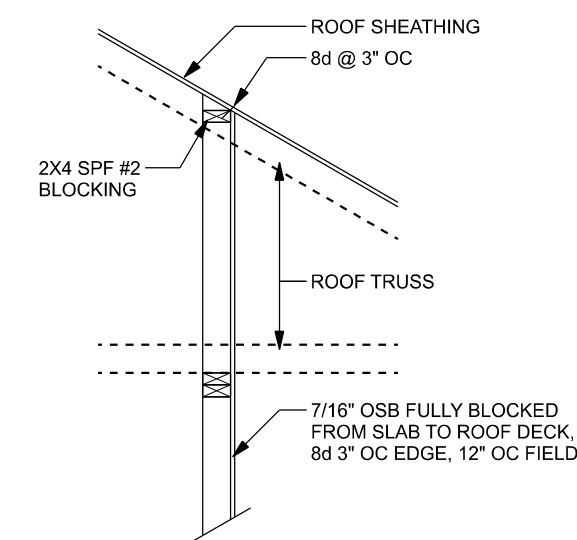
CRIPPLES 4'-0" O.C. FOR 20' PL (TD) AND 10' PL (TD) (TYP. SHINGLE ROOF) MAX

### CONNECTION REQUIREMENT NOTES

1. 2X4 RAFTERS TO RIDGE	3-16d OR 6-... 131 x 3" TOE NAILS
2. CRIPPLE TO RIDGE	3-16d OR 6-... 131 x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	3-16d OR 6-... 131 x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	3-16d OR 6-... 131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4-16d OR 8-... 131 x 3" FACE NAILS EACH TRUSS
6. RIDGE BOARD TO ROOF BLOCK	3-16d OR 6-... 131 x 3" TOE NAILS
7. RIDGE BOARD TO TRUSS	3-16d OR 6-... 131 x 3" TOE NAILS
8. PURLIN TO TRUSS (TYP)	3-16d OR 6-... 131 x 3" NAILS
9. PURLIN TO TRUSS IF CRIPPLE IS ATTACHED TO PURLIN	4-16d OR 8-... 131 x 3" NAILS
10. TRUSS TO BLOCKING	3-16d OR 6-... 131 x 3" END NAILS
11. CRIPPLE TO PURLIN	3-16d OR 6-... 131 x 3" FACE NAILS

## ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.



## ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL

NOTE:  
IF THE ABOVE DETAIL IS USED ON THE REAR PORCH WALL, THE REAR PORCH CEILING DOES NOT NEED TO BE SHEATHED

\* ATTACHMENT CAN BE MADE DIRECTLY OR THROUGH PLYWOOD SHEATHING BY CUTTING A 2" X 4" NOTCH IN SHEATHING

BEVEL RAFTER CUT AS REQ'D FOR PITCH

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## STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

## STRUCTURAL PLAN NOTES

SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS)	
***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE: (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

## ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	33254 LBF	16160 LBF
REQUIRED	21216 LBF	15031 LBF

## HEADER LEGEND

(2) 2X6X8'-1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
(2) 2X6X8'-1J 1K	NUMBER OF KING STUDS EACH SIDE OF OPENING (FULL LENGTH)
(2) 2X6X8'-1J 1K	NUMBER OF JACK STUDS EACH SIDE OF OPENING (UNDER HEADER)
(2) 2X6X8'-1J 1K	SPAN OF HEADER
(2) 2X6X8'-1J 1K	SIZE OF HEADER MATERIAL
(2) 2X6X8'-1J 1K	NUMBER OF PLIES IN HEADER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #4496337

Amira Custom Homes

Bouquet Residence

PROJECT ADDRESS:  
SW Maynard Drive, High Springs  
(Columbia County), FL 32645

FL PE 53915  
This item has been digitally signed and sealed by Mark Disoway P.E. for resolution. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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O-Unaffiliated,  
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DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.  
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JOB NUMBER:  
250370

S-3  
OF 3 SHEETS