

MiTek USA, Inc. 6904 Parke East Blvd.

RE: 2435696 - LERNER - STEVEN & AMY

## Site Information:

Customer Info: Lerner Luxury Prop. Project Name: Steven & Amy Model: Custom Tampa, FL 33610-4115 Lot/Block: N/A Subdivision: N/A Address: TBD Hermatige Glen, N/A City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Wind Code: ASCE 7-10 Roof Load: 37.0 psf Design Program: MiTek 20/20 8.2 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 6 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

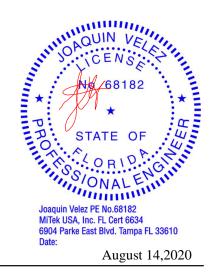
No.	Seal#	Truss Name	Date
1	T21027604	T01	8/14/20
2	T21027605	T01G	8/14/20
3	T21027606	T02	8/14/20
4	T21027607	T03	8/14/20
5	T21027608	T03G	8/14/20
6	T21027609	T04	8/14/20

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2021.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



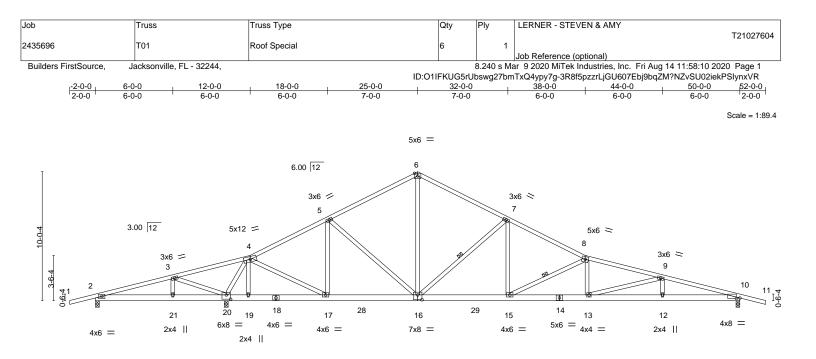


Plate Offsets (X,Y	6-0-0 4-1-12 1-10-4	18-0-0         25-0-0           6-0-0         7-0-0	<u>32-0-0</u> 7-0-0	<u>38-0-0</u> 6-0-0	44-0-0 6-0-0	50-0-0 6-0-0
Plate Olisets (X, Y	<u>/</u> ) [10:0-3-4,0-0-7], [16:0-4-0,0-4-8], [20:0-4	1-0,0-4-0]				
LOADING(psf)TCLL20.0TCDL7.0BCLL0.0BCDL10.0	* SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	<b>CSI.</b> TC 0.99 BC 0.85 WB 0.80 Matrix-MS			PLATES MT20 Weight: 312 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2     BRACING- TOP CHORD 2x6 SP No.2       BOT CHORD 2x6 SP No.2     BOT CHORD BOT CHORD Rigid ceiling directly applied or 5-0-11 oc bracing.       WEBS 2x4 SP No.3     WEBS 1 Row at midpt 7-16, 8-15						
REACTIONS.       (size)       2=0-3-8, 20=0-3-8, 10=0-3-8         Max Horz       2=-210(LC 13)         Max Uplift       2=-310(LC 8), 20=-871(LC 12), 10=-631(LC 13)         Max Grav       2=182(LC 23), 20=2427(LC 1), 10=1465(LC 1)						
FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown.         TOP CHORD       2-3=-388/1119, 3-4=-1062/1842, 4-5=-1107/602, 5-6=-1359/847, 6-7=-1359/847, 7-8=-2360/1290, 8-9=-3652/1920, 9-10=-4157/2167						
BOT CHORD	2-21=-1010/613, 20-21=-1010/613, 19-20=-59 15-16=-892/2058, 13-15=-1708/3490, 12-13=	00/552, 17-19=-589/547, 16-1	,			
WEBS	10-10-092/2006, 13-13-17100/3490, 12-13- 3-20825/899, 4-202289/1237, 4-17915/ 6-16457/788, 7-161228/788, 7-15383/8 9-13545/367	1722, 5-17=-664/465, 5-16=-	100/388,			

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2, 871 lb uplift at joint 20 and 631 lb uplift at joint 10.

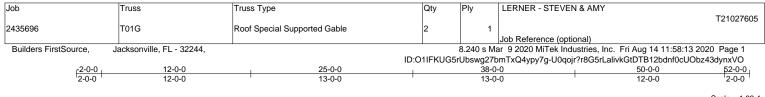


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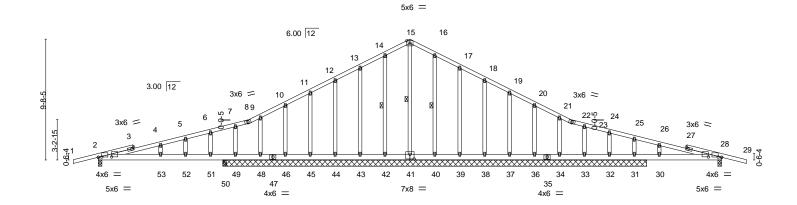
August 14,2020



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Scale = 1:92.4



	10-0-0			50-0-0	1
	10-0-0			40-0-0	
ate Offsets (X,Y)	[2:0-3-4,0-0-4], [2:1-0-12,0-0-6], [28:0-	3-4,0-0-4], [28:1-0-12,0-0-6],	[41:0-4-0,0-4-8]		
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc) l/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.25	TC 0.36	Vert(LL) 0.1	( ,	MT20 244/190
CDL 7.0	Lumber DOL 1.25	BC 0.64	Vert(CT) -0.1		
CLL 0.0 *	Rep Stress Incr YES	WB 0.11	Horz(CT) 0.0		
CDL 10.0	Code FBC2017/TPI2014	Matrix-S	- (- )		Weight: 338 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF DTHERS 2x4 SF	P No.2		BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling directly applied	rectly applied or 10-0-0 oc purlins. or 6-0-0 oc bracing. /5-41, 14-42, 16-40
(lb) - Max H	earings 34-0-0 except (jt=length) 2=0-3 lorz 49=-203(LC 17) Jplift All uplift 100 lb or less at joint(s) 2=-348(LC 8), 28=-289(LC 9), 48=	42, 43, 44, 45, 46, 40, 39, 38,	, , ,		

Max Grav All reactions 250 lb or less at joint(s) 41, 42, 43, 44, 45, 46, 48, 40, 39, 38, 37, 36, 34, 33, 32 except 2=411(LC 23), 28=339(LC 24), 49=743(LC 8), 31=520(LC 24), 50=1453(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 14-15=-87/282, 15-16=-87/282

BOT CHORD 2-53=-97/274, 52-53=-97/274, 51-52=-97/274, 50-51=-97/274, 49-50=-97/274,

- 48-49=-97/253, 46-48=-97/253, 45-46=-97/253, 44-45=-97/253, 43-44=-97/253,
  - 42-43=-97/253, 41-42=-97/253, 40-41=-97/253, 39-40=-97/253, 38-39=-97/253,
    - 37-38=-97/253, 36-37=-97/253, 34-36=-97/253, 33-34=-97/253, 32-33=-97/253,
    - 31-32=-97/253, 30-31=-97/253, 28-30=-97/253

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 42, 43, 44, 45, 46, 40, 39, 38, 37, 36, 34, 32 except (jt=lb) 2=348, 28=289, 48=122, 49=943, 33=163, 31=408, 50=1157.

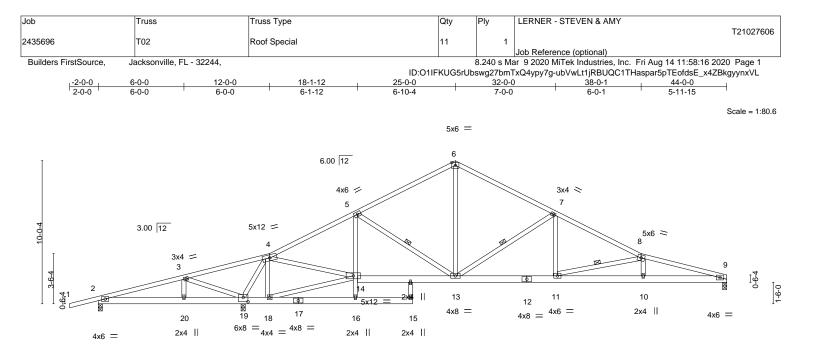


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 14,2020



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	6-0-0 10-1-12 6-0-0 4-1-12	12-0-0 18-1-12 1-10-4 6-1-12	22-0-0 25-0-0	32-0-0	38-0-1         43-8-8         44-0           6-0-1         5-8-7         0-3	
Plate Offsets (X,	Y) [19:0-4-0,0-3-12]					
LOADING         (psf)           TCLL         20.0           TCDL         7.0           BCLL         0.0           BCDL         10.0	Plate Grip DOL         1           Lumber DOL         1           *         Rep Stress Incr         Y	0-0 <b>CSI.</b> .25 TC 0.69 .25 BC 0.70 /ES WB 0.64 14 Matrix-MS	Vert(CT) -	0.20 10-11 >999 24 0.36 10-11 >999 18		
BOT CHORD 2 WEBS 2 REACTIONS.	TOP CHORD       2x4 SP No.2       TOP CHORD       Structural wood sheathing directly applied or 2-10-2 oc purlins.         BOT CHORD       2x6 SP No.2 *Except*       BOT CHORD       BOT CHORD       Structural wood sheathing directly applied or 2-10-2 oc purlins.         WEBS       2x4 SP No.3       BOT CHORD       BOT CHORD       Structural wood sheathing directly applied or 5-8-4 oc bracing. Except:         NEBS       2x4 SP No.3       WEBS       1 Row at midpt       5-13, 7-13, 8-11         REACTIONS.       (size) 9=0-3-8, 2=0-3-8, 19=0-3-8       Max Horz 2=262(LC 12)       Structural wood sheathing directly applied or 2-10-2 oc purlins.         Max Horz 2=262(LC 12)       Max Upliff 9=-448(LC 13), 2=-338(LC 8), 19=-795(LC 12)       Structural wood sheathing directly applied or 5-8-4 oc bracing. Except:					
Max Grav 9=1155(LC 1), 2=195(LC 23), 19=2198(LC 1)         FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-260/891, 3-4=-896/1581, 4-5=-1360/588, 5-6=-1302/731, 6-7=-1302/731, 7-8=-2197/1121, 8-9=-3460/1736         BOT CHORD       2-20=-796/368, 19-20=-796/368, 18-19=-473/376, 14-16=0/319, 5-14=-293/347, 13-14=-399/1173, 11-13=-847/1916, 10-11=-1637/3319, 9-10=-1630/3321         WEBS       3-19=-803/884, 4-19=-2020/1045, 14-18=-467/401, 4-14=-765/1690, 5-13=-267/203, 6-13=-356/758, 7-13=-1000/657, 7-11=-198/588, 8-11=-1454/818						

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=448, 2=338, 19=795.

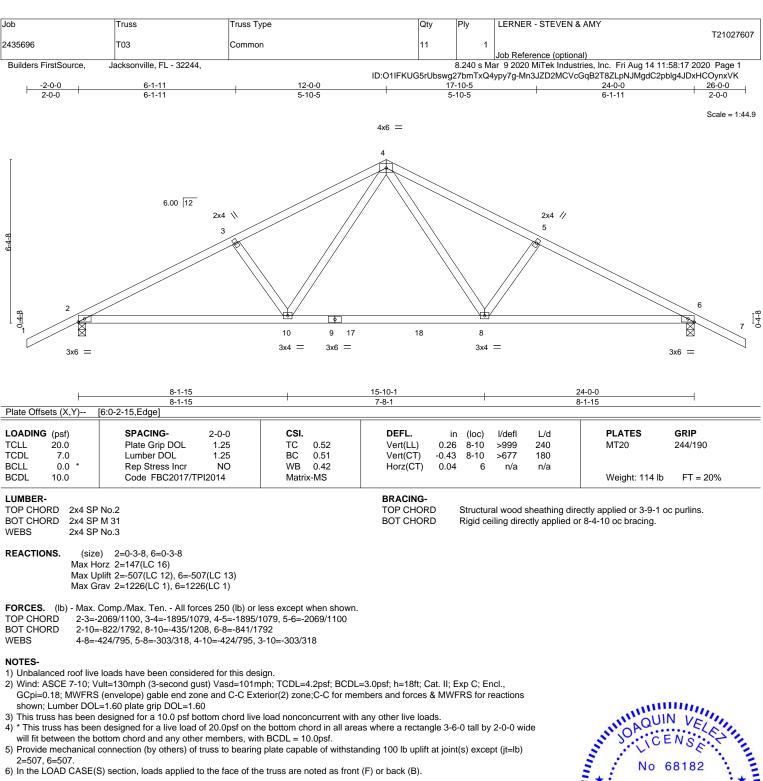


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August 14,2020



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#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 10-11=-20, 8-10=-80(F=-60), 8-14=-20



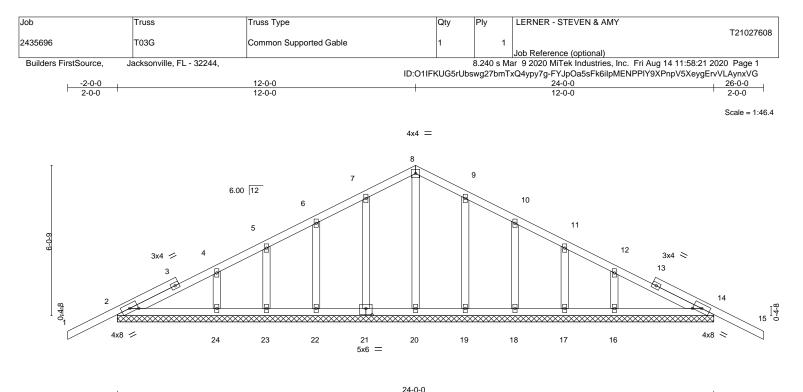
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DADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.25	TC 0.31	Vert(LL) -0.02 15 n/r 120	MT20 244/190
CDL 7.0	Lumber DOL 1.25	BC 0.10	Vert(CT) -0.02 15 n/r 120	
CLL 0.0 *	Rep Stress Incr YES	WB 0.08	Horz(CT) 0.01 14 n/a n/a	
CDL 10.0	Code FBC2017/TPI2014	Matrix-S		Weight: 133 lb FT = 20%

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3

#### REACTIONS. All bearings 24-0-0.

Max Horz 2=140(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 21, 22, 23, 19, 18, 17 except 2=-100(LC 12), 14=-124(LC 13), 24=-119(LC 12), 16=-125(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 20, 21, 22, 23, 24, 19, 18, 17, 16 except 2=266(LC 23), 14=266(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 22, 23, 19, 18, 17 except (jt=lb) 2=100, 14=124, 24=119, 16=125.



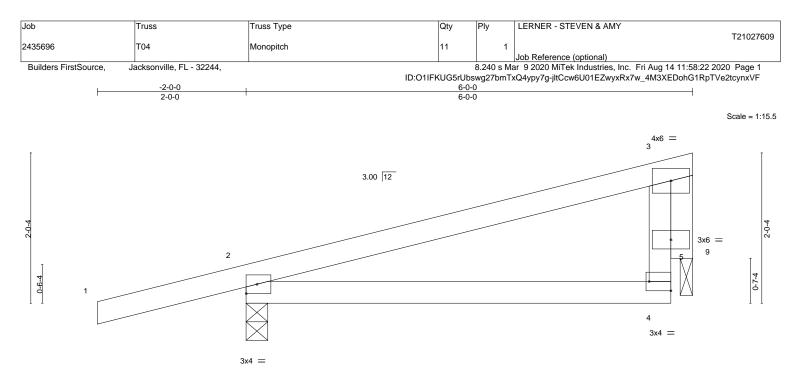
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 Satisfies
 Ansi/TPI Qu

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			L				6-0-0					_
Diata Offa		[4.Edge 0.4.0]					6-0-0					•
Plate Offs	ets (X, Y)	[4:Edge,0-1-8]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.46	Vert(LL)	0.05	4-8	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.27	Vert(CT)	-0.04	4-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(CT)	-0.01	2	n/a	n/a		
BCDL	10.0	Code FBC2017/TF	PI2014	Matri	x-MR						Weight: 25 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. (size) 2=0-3-8, 9=0-2-0 Max Horz 2=100(LC 8) Max Uplift 2=-291(LC 8), 9=-141(LC 8) Max Grav 2=344(LC 1), 9=175(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-198/265

BOT CHORD 2-4=-307/161

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,
- GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 9.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=291, 9=141.



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