

7 12

SCALE: 1/4"=1'-0"

2377.0
516.3
653.9
3547.2 8

ELECTRICAL PLAN

SCALE: 1/4"=1'-0"

ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	Ð	
can light	14	Image: Control of the
chandelier	4	0 0 0 0
fluorescent fixture	3	
vanity bar light	2	<u> </u>
wall mount 1	6	Q
electrical panel	1	11
fan with light	3	(
light	6	
outlet	36	Ф
outlet gfi	14	Фан
outlet wp gfi	3	mb ets
smoke detector	5	•
switch	30	\$
ewitch 3 way	15	\$3

SERVING BAR

ELECTRICAL PLAN NOTES

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFIC CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

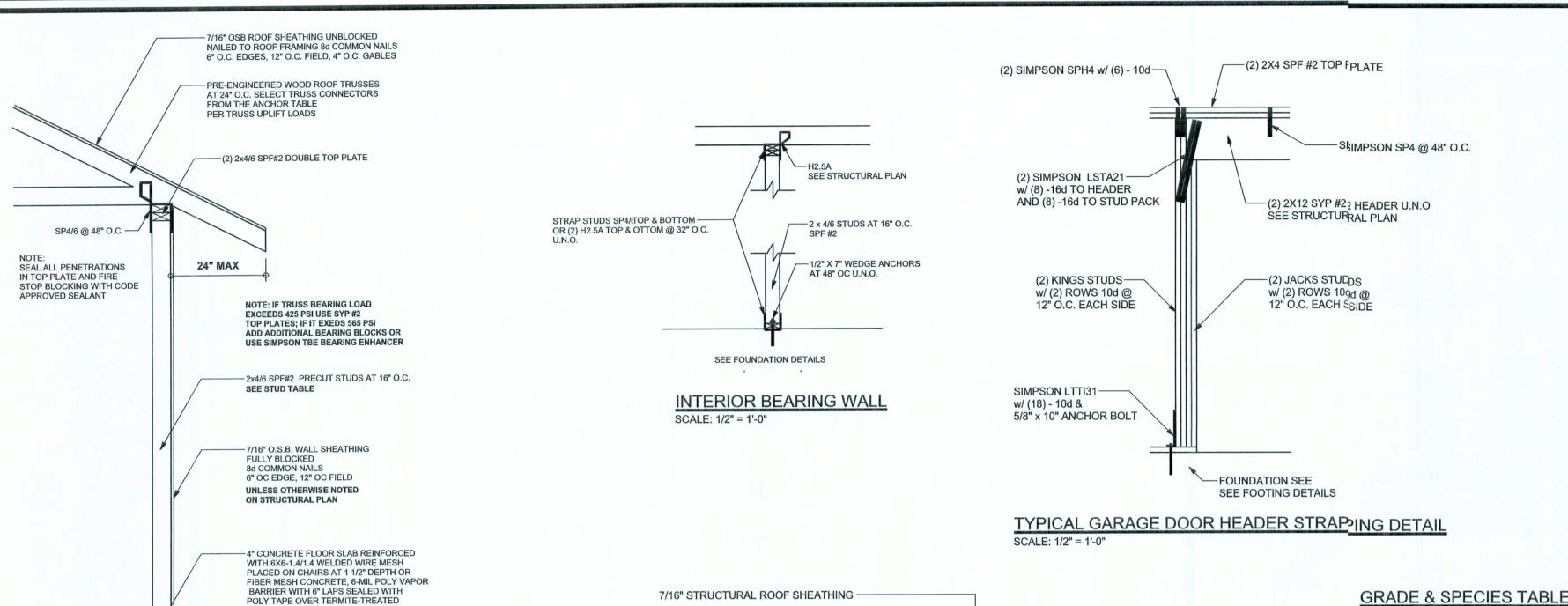
ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECTING MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE 4-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS. CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

AREA SUMMARY



7/16" STRUCTURAL ROOF SHEATHING -2X4 OUTRIGGER @ 48" OC. — HURRICANE CLIP H-2.5 OR EQUAL -2x4/6 P.T. PINE SOLE PLATE ANCHORED WITH 48" OC. BLOCKING REQUIRED BETWEEN OUTRIGGERS -WITH 1/2"X10" ANCHOR BOLTS WITH 2X2X.140" STEEL WASHER 48" O.C. & 8" FROM CORNERS 2X4 BARGE RAFTER CONT. (3) .131 X 3 1/4 " GUN NAILS - SHINGLE STRIP 2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END -- FASCIA TOP CHORD OF GABLE END TRUSS 2X4 SCAB CONT. TOP TO CHORD@ 8' FROM GABLE CONT. 2X4 SCAB FROM TOP TO BOTTOM CHORD @ X-BRACING 4 - 10d NAILS OR 4 - .131"x 3.25" (PROVIDE ADDITIONAL 2X4'S @ TYPICAL AT ALL CONNECTIONS VERTICAL IF HIGHER THAN 48", TO FORM AN "L" SHAPE.) 2X4 SCAB IF VERT. WEB IS NOT PRESENT -TOE NAIL TRUSS TO DOUBLE PLATE w/ 16d COM @8" OC. BOTTOM CHORD OF GABLE CONT. 2X4X8' #2 SYP LATERAL BRACE @ 48" OC. -END TRUSS - 2 - 2X4 TOP PLATE - SIMPSON LSTA 24 @ 48" OC. 2X4 BLOCKING @ 48" OC. BETWEEN GABLE AND FIRST - 2X4 STUDS @16" OC. TRUSS. 2X4 X-BRACE @ 6'-0" OC. -

TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

---- NON-SUPPORTIVE SUPPORTIVE (2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN SEE STRUCTURAL PLAN 3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON -OPPOSITE SIDE) EA. NAILED WITH 14-10d -(4)-2x4 SP#2 NAILED TOGETHEIW/2-16d MIN. (SEE TRUCTURAL PLAN)

BEAM MID-WALL CONECTION DETAIL

AND COMPACTED FILL

SP4/6 @ 48" O.C. -

SCALE: 3/4" = 1'-0"

ONE STORY WALL SECTION

(1) 2x4 @ 12" OC

(1) 2x6 @ 16" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O. -

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

SEE STRUCTURAL PLAN

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3,20B.

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

LOCATED WITHIN 4 EEET OF CORNERS FOR END ZONE LOADING

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

TO 13'-0" STUD HEIGHT

TO 18'-10' STUD HEIGHT

-(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SIMPSON ABU POST BASE

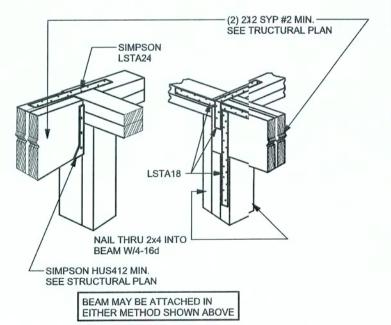
w/ (12) - 16d & 5/8" x 10"

—SEE FOOTING DETAILS

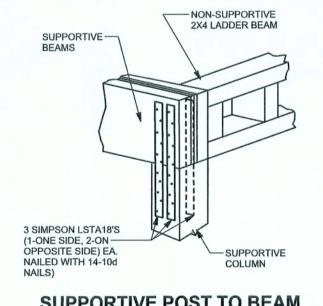
TYPICAL PORCH POST DETAIL

ANCHOR BOLT

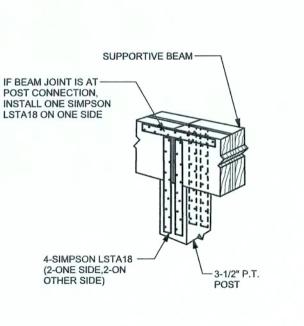
SEE STRUCTURAL PLAN



BEAM CORNER CONNECTION. DETAIL



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO ELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO DWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS. ANCHORS. ND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

 $\mbox{\bf NAILS}:$ ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

Fb (psi) E (10⁶ psi

1.6

1.6

1.6

1.7

1.9

1200

1050

975

2400

1600

2900

SYP #2

SYP #2

SYP #2

LSL TIMBERSTRAND | 1700 |

MICROLAM

PRE ENGINEERED ROOF TRUSS ---

PARALAM

GLB 24F-V3 SP

DODUBLE 2x4 SPF TOP PLATE NAILED -

Obgether W/2-16d NAILS AT 16" O.C.

4' MMIN. LAP w/ (12) - 16d OR 4" LAP w/

INTTERIOR CEILING AS ---SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4 ----

-LSTA18 (U.N.O.

CRIPPLES IF REQUIRED

(44) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU SILL -

INTO JACK STUD U.N.O.

Trypical Strapping (U.N.O.)

-SSP4 OR (2) H2.5A OR (2) SSP-

(1) 2X63 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X44 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 1110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-'-0"

ALL OPENINGS (U.N.O.)

(SEE STRUCTURAL PLAN)

CONTINUOUS FRAME TO

CELLING DIAPHRAGM DETAIL

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 4" O.C. FOR UPLIFT

SPF NAILED TO TOP

WITH 2-16d NAILS

SCALE: N.T.S.

(6) .131 x 3 1/4" GUN NAILS ---

INTO KING STUD

TOE NAILED THRU HEADER

AND BOTTOM PLATES

TO TOPP PLATE AT

BOTTOOM CHORD OF TRUSS

		SIBLE FOR THE FOLLOWING, WHICH ARE OAD ENGINEER'S SCOPE OF WORK.
	CONDITIONS, FOUNDATION BEARINGHT, WIND SPEED AND DEBRIS ZONI	
	ERIALS AND CONSTRUCTION TECHN IS FOR THE STATED WIND VELOCIT	NIQUES, WHICH COMPLY WITH FBCR 2004 Y AND DESIGN PRESSURES.
BELIEVE THE	NTINUOUS LOAD PATH FROM TRUS PLAN OMITS A CONTINUOUS LOAD P ID ENGINEER IMMEDIATELY.	
	RUSS MANUFACTURER'S SEALED EN EMENT PLANS, TEMPORARY AND PE	

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TOE NAILED THRU HEADER

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES:

ACI530.1-02 Section

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

2.4F Coating for corrosion protection

3.3.E.7 Movement joints

Coating for corrosion protection

IN WRITING

2.1 Mortar

2.2 Grout

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

or 304SS

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

medium surface finish, 8"x8"x16" running

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

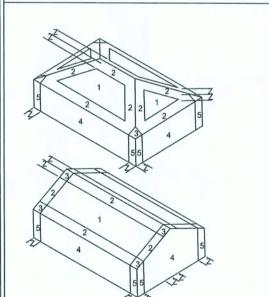
detailed on project drawings.

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

DESIGN DATA

ME/ ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WIT AN ROOF HEIGHT NOT EXCEEDING LEAST HO UPPER HALF OF HILL OR ESCARPMENT 60FT OPE AND UNOBSTRUCTED UPWIND FOR 50x I	DRIZONTAL D I IN EXP. B, 30	IMEN:	SION I EXP	OR 60	FT; NO	OT %
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRIO	CANE ZONE					
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS F	REGION					
1.)	BASIC WIND SPEED = 110 MPH						
2.)	WIND EXPOSURE = B						
3.)	WIND IMPORTANCE FACTOR = 1.0						
4.)	BUILDING CATEGORY = II						
5.)	ROOF ANGLE = 10-45 DEGREES						
6.)	MEAN ROOF HEIGHT = <30 FT						
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED B	UILDI	NG)			
8.)	COMPONENTS AND CLADDING DESIGN WIN	ID PRESSURE	ES (T	ABLE	R301.	.2(2))	
		Zone	Effect	tive W	ind Are	ea (ft2)	
		20116		0		100	
	XXXII	1	19.9	-21.8	18.1	-18.1	



32		Worst Case (Zone 5, 10 ft2)	10.5	22.0
2	2 4 5 5 5 5 2 2 2	8x7 Garage Door 16x7 Garage Door	19.5	-22.9
DESIGN	LOADS 40 PSF (ALL OTHER DWELLING ROOMS)			
LOOK	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			
NOT IN F	LOOD ZONE (BUILDER TO VERIFY)			

2 19.9 -25.5 18.1 -21.8

3 19.9 -25.5 18.1 -21.8

4 21.8 -23.6 18.5 -20.4

5 21.8 -29.1 18.5 -22.6

Doors & Windows | 21.8 | -29.1

2 O'hg -40.6

3 O'hg -68.3

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	-
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
HR-		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

SOFTPIAN

REVISIONS

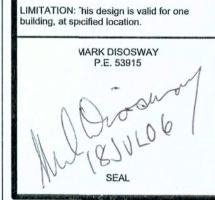
WINDLOAD ENGINEER: Mark Disosway PE No.53915,POB 868, Lake City, FL DIMENSIONS ated dimensons supercede scaled

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHT! AND PROPERTY RIGHTS: Mark Disoswa, P.E. hereby expressly reserve its common lav copyrights and property right in these instrumints of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written

> CERTIFICATION: I hereby certify that I have examined thisplan, and that the applicable portions of theplan, relating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my

permission and consent of Mark Disosway.

LIMITATION: his design is valid for one



Concept Construction Of North Florida, Inc

Spec House Lot 13 Late Jeffery S/D

ADDRESS:

Lot13 Lake Jeffery S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone (386) 754 - 5419 Fax: (386) 269 - 4871

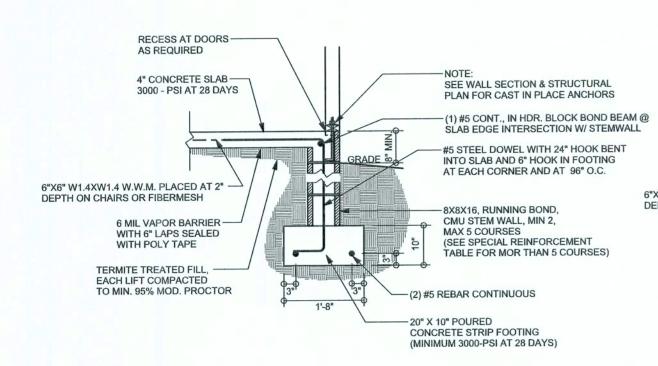
Jily 18, 2006 DRAWN EY: CHECKED BY: David Discsway

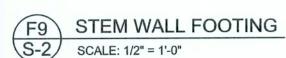
PRINTED DATE:

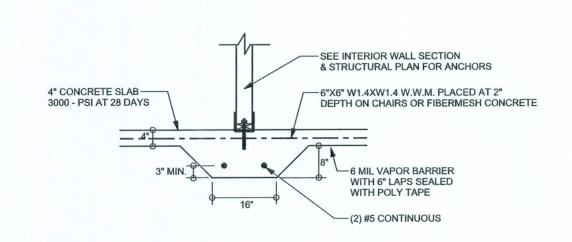
FINALS DATE: 18 / Jul /06

JOB NUMBER: 607141 DRAWING NUMBER

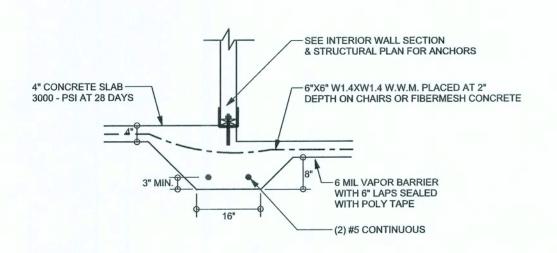
> S-1 OF 3 SHEETS



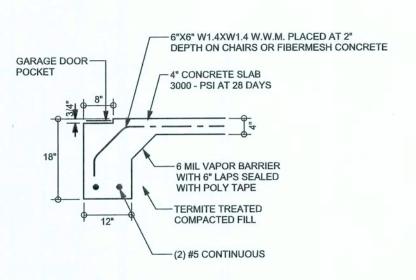




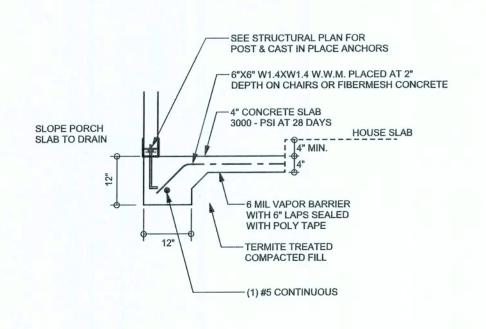
INTERIOR BEARING FOOTING SCALE: 1/2" = 1'-0"



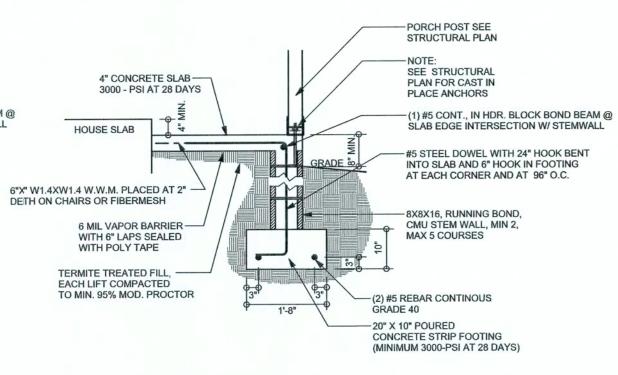
INTERIOR BEARING STEP FOOTING SCALE: 1/2" = 1'-0"



F4 GARAGE DO S-2 SCALE: 1/2" = 1'-0" GARAGE DOOR FOOTING



PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

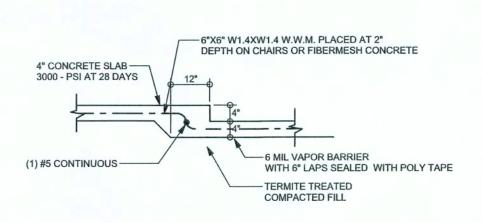


OPTIONAL STEM WALL PORCH FOOTING SCALE: 1/2" = 1'-0"

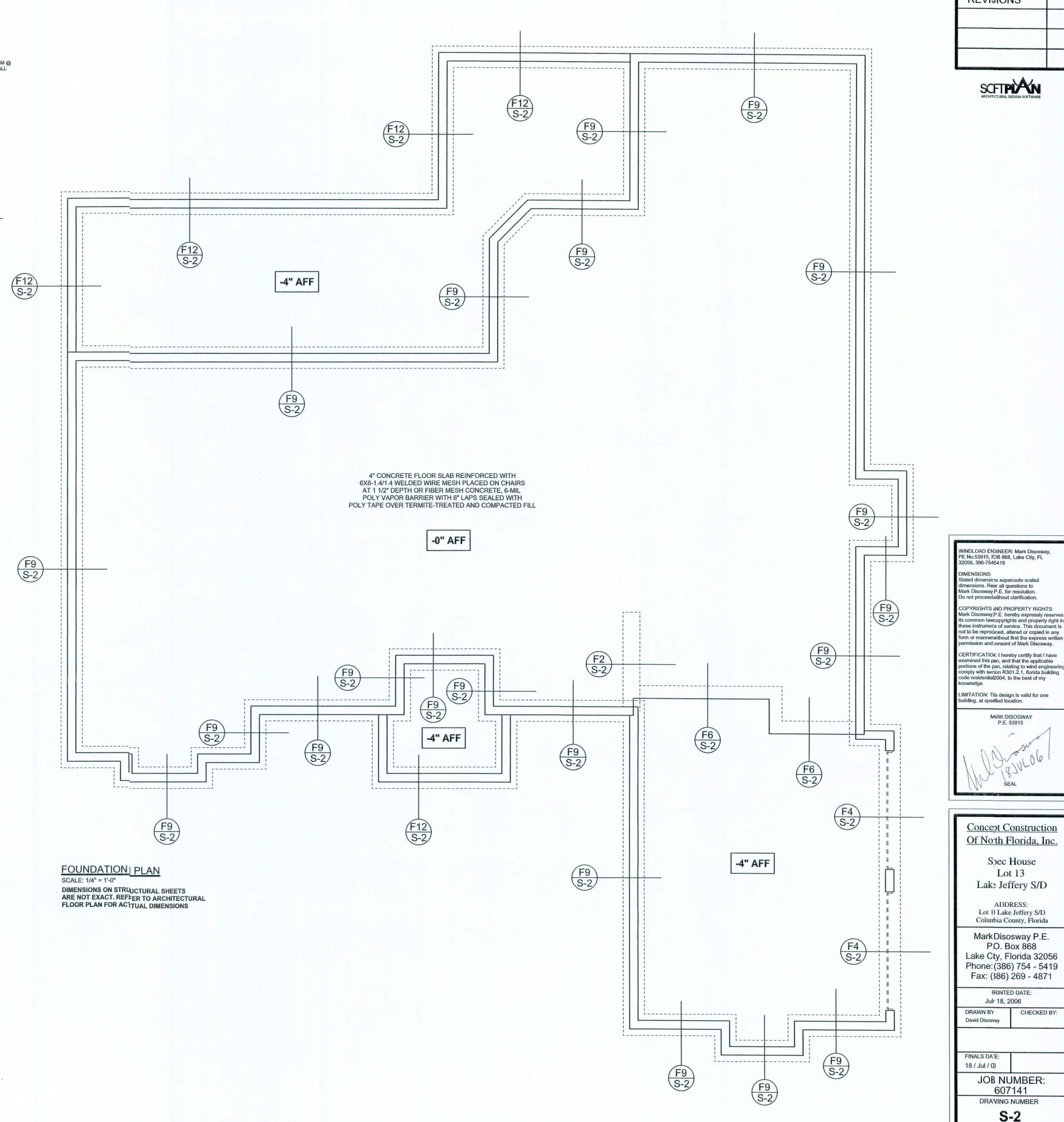
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	HEIGHT BACKFILL		BACKFILL FOR 8" CMU STEMWALL		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	



F6 TYPICAL NON - BEARING STEP FOOTING SCALE: 1/2" = 1'-0"



REVISIONS

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this pan, and that the applicable

code residential2004, to the best of my

LIMITATION: Tlis design is valid for one

MARK DISOSWAY P.E. 53915

Concept Construction

Spec House

Lot 13

Lake Jeffery S/D

ADDRESS: Lot 13 Lake Jeffery S/D

Columbia County, Florida

MarkDisosway P.E.

P.O. Box 868 Lake Cty, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> FRINTED DATE: July 18, 2006

JOB NUMBER:

S-2

CF 3 SHEETS

607141 DRAVING NUMBER

CHECKED BY:

DRAWN BY

David Disosvay

FINALS DA'E: 18 / Jul / 05

Of Noth Florida, Inc.

building, at speified location.

