

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID IUWE487-Z0323114643



Truss Fabricator **Anderson Truss Company**
Job Identification **13-164--Mike Todd Construction Bryant Towing Bldg -- 90E Lake City**
Truss Count **2**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 40.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

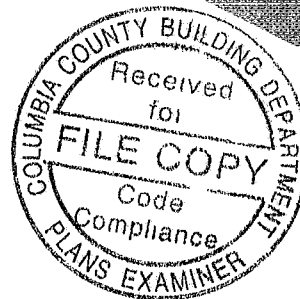
William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	01205--A	28' 6" Mono	13141001	05/21/13
2	01206--ADG	28' 6" Gable	13141002	05/21/13

ALPINE



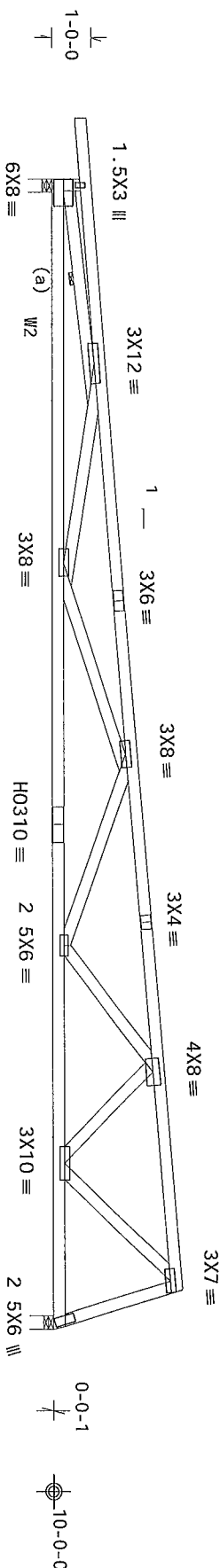
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP C, wind TC DL=5 0 psf, wind BC
DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



1-6-01

28-6-0 Over 2 Supports

R=1230 U=194 W=4" (4" min)
RL=61

R=1129 U=131 W=4" (4" min)

WILLIAM H. KRICK
LICENSED
1939

PLT TYP	20 Gauge HS, Wave

Design Crit	FBC2010Res/TP1-2007(STB)
FT/RT=20%(0%)/10(0)	

12 03 04 03 26 14 QTY 40 FL/-/3/-/-/R/-

Scale = 25"/Ft.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

****IMPORTANT****
****WARN INC**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

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TC LL	20 0 1
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REF R487-- 1204

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information) by TPI and WITCA for safety.

STAT OF R

TC DL	10.0
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DATE 05/21/13

practices prior to performing these functions. installers shall provide temporary bracing for the structure, unless noted otherwise. top chord shall have properly attached structural sheathing and bottom chord shall have a solid ceiling. installers shall ensure that the structure is properly braced and supported during the installation process.

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006-
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FREE
ATTENTION

BC DL	10.0
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DRW HCUSR487 131410

shall have a property attached to the ceiling. Locations shown for permanent lateral restraint of the cable shall have bracing installed per BCS sections 83 B7 or B10 as applicable.

RECEIVED
JAN 10 1964
U.S. AIR FORCE
HEADQUARTERS
WASHINGTON, D.C.

BC 11 001

HC-FENG SSB/MIHK

any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation, or application of the truss. The manufacturer shall not be responsible for any deviation from this design or for any damage to the truss or other components of the truss system.

ADDITIONAL

DO EL	0:0
TOT ID	40 0

NO LINK CODE/MAIL
SEON 203070

Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on the drawing or cover page stating this drawing indicates acceptance of professional engineering.

05/23/2013

101	ED	40	0
DIP EAC		1	25

010007 - 11070

responsibility for the design shown in the table may and use of the design for any structure is the responsibility of the Building Design group per AISC/TPI 1 Sec 2. For more information see the general notes page 17W-BGC www.ltwibgc.com TPI www.tpi.org WTCA www.sbcindustry.com

structure is
This job s

DOB: 1 MC 1:23

CDAC INC 34 011

INLT 41MTC407 70

ICC www.iccnate.org

Country	1950	1955	1960	1965	1970	1975	1980	1985	1990	1995	2000	2005	2010	2015	2020	2025	2030	2035	2040	2045	2050
Japan	7.0	7.5	8.0	8.5	9.0	9.5	10.0	10.5	11.0	11.5	12.0	12.5	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0
Germany	10.0	10.5	11.0	11.5	12.0	12.5	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0
France	11.0	11.5	12.0	12.5	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0
Italy	12.0	12.5	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0
Spain	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0
Sweden	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0
United Kingdom	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0
United States	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0
Canada	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0
Australia	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0
South Korea	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0
India	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0
China	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0
Brazil	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0	31.5	32.0
Argentina	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0	31.5	32.0	32.5	33.0
South Africa	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0	31.5	32.0	32.5	33.0	33.5	34.0
Indonesia	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0	31.5	32.0	32.5	33.0	33.5</			

SPACING	24.0
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JREF- 10WE481_20

(13-164--Mike Todd Construction Bryant Towing Bldg -- 90E Lake City FL - ADG 28 6' Gable)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A
Stack Chord SC1 2x4 SP_#1_12A Lt Slider 2x4 SP_#3_12A BLOCK LENGTH
= 4 059'

Lumber grades designated with "12A" use design values approved
1/5/2012 by ALSC

Truss spaced at 24 0" OC designed to support 2-0-0 top chord
outlookers Cladding load shall not exceed 10 00 PSF Top chord must
not be cut or notched

Stacked top chord must NOT be notched or cut in area (NML) Dropped
top chord braced at 24" o c intervals Attach stacked top chord (SC)
to dropped top chord in notchable area using 3x4 tie-plates 24" o c
Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length Splice top chord in notchable area
using 3x6

(**) 1 plate(s) require special positioning Refer to scaled plate
plot details for special positioning requirements

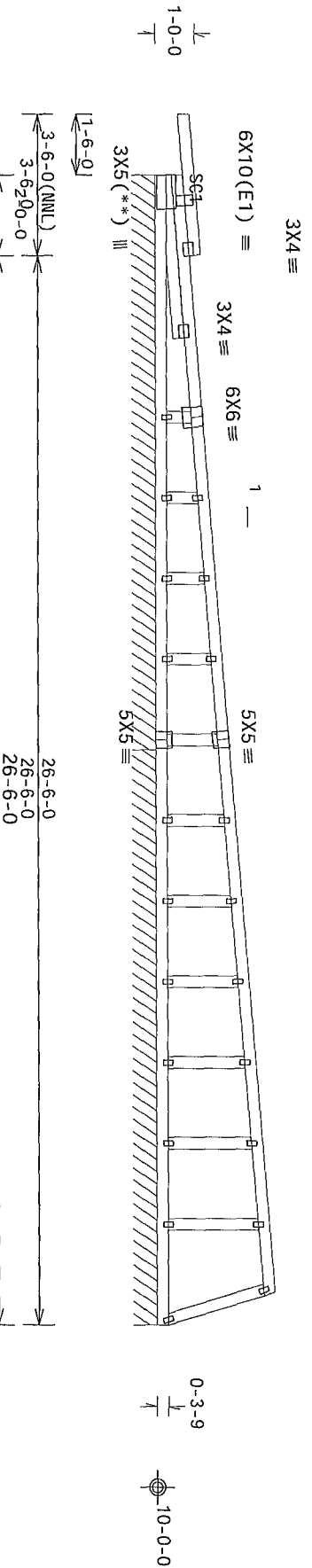
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP C, wind TC DL=5 0 psf, wind BC
DL=5 0 psf GCP(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member
design

See DWGS A12015ENC100212, GBLLETIM0212, & GABRST100212 for more
requirements

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



Note: All Plates Are 1 5X3 Except As Shown
PLT TYP Wave

Design Crit. FBC2010Res/TP1-2007(STB)
FT/RT=20%(0%)/10(0)

12 03 08 26 14 01

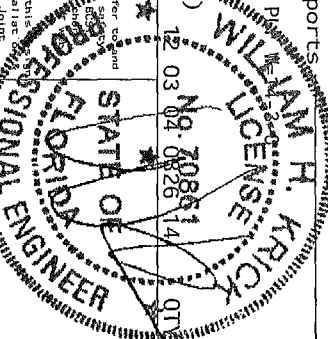
FL/-/3/-/-/R/-

Scale = .25"/Ft.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling and bracing
following the erection of BCS Building Component Safety Information by TPI and WFOA
Trusses shall be erected in accordance with the erection instructions provided
Unless noted otherwise top chord shall have properly attached structural sheathing and bottom
chord shall have a properly installed rigid ceiling. Locate one shown for permanent lateral restraint
shall have bracing installed per BCS1 section 83 B7 or B10 as applicable
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this
any failure to build the truss in conformance with ANSI/TP1 1 or for handling shipping and metal
bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint
details. Unless noted otherwise refer to drawings 1004-2 for standard plate positions. A seal on this
document is required for all trusses. The seal shall be applied to the top chord and bottom chord
responsibility solely for the design shown. The suitability and use of this design for any structure is
the responsibility of the building designer per ANSI/TP1 1 Sec 2 For more information on seal
general notes page ITW BCG www.tpiinc.com TPI www.tpinet.org WFOA www.structure.com This is Job 5
ITC www.tpiinc.com



TC LL	20 0 PSF	REF R487-- 1206
TC DL	10 0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HOURS487 13141002
BC LL	0.0 PSF	HC-ENG SSB/MMH
TOT. LD.	40 0 PSF	SEQN- 293986
DUR. FAC.	1.25	
SPACING	24.0"	JREF - 1UWF487_203

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLIP SHOWN ON SINGLE PLY SEALED DESIGNS TO T BRACING OR SCAB BRACING

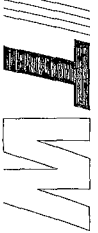
ALTERNATIVE BRACING SPECIFIED IN CHAPT BELOW MAY BE CONSERVATIVE FOR MINIMUM ALTERNATIVE BRACING PE-PUN DESIGN WITH APPROPRIATE BRACING

WEB MEMBER SIZE	SPECIFIED CL3 BRACING	ALTERNATIVE BRACING 1 DP 1 BRACE	CLAB BRACE
2X 1P 2X4	1 PDW	2X4	1 2X4
2X3 1P 2X4	2 PDW	2X6	2 2X4
X2	1 PDW	2X4	1 2X6
2X6	2 PDW	2X6	2 2X4*
2X8	1 PDW	2X6	1 2X8
2X8	2 PDW	2X6	2 2X6*

1 BRACE & BRACE AND SLAB BRACE TO BE SAME SPECIE AND GRADE
OR BETTER THAN WEB MEMBER UNLESS EFFECTED OTHERWISE ON
ENGINEER'S SEALED DESIGN

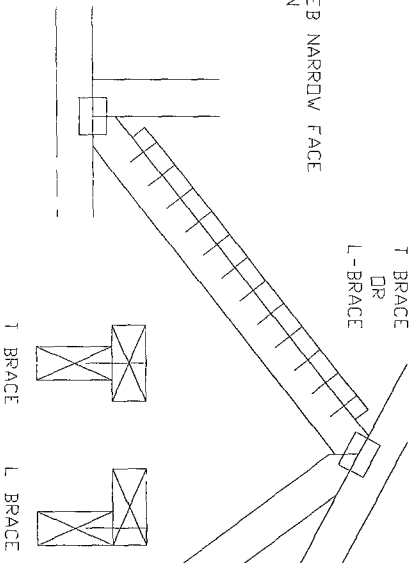
(*) CENTER SCAB ON WIDE FACE OF WEB APPLY (1) SCAB TO EACH FACE OF WEB

Building Components Group Inc.

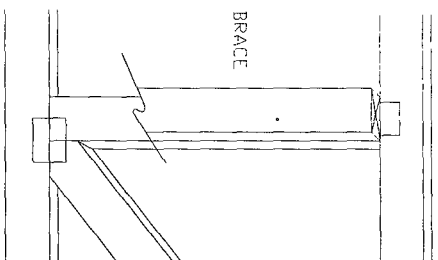


Earth City MO 63045

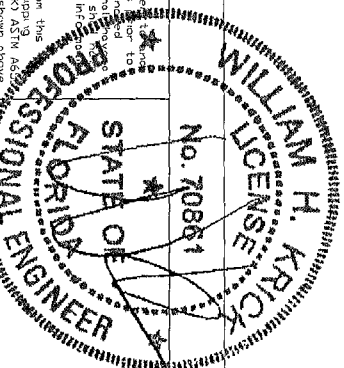
APPLY TO EITHER SIDE OF WEB NARROW FACE
ATTACH WITH 10d BOX OR GUN
@0128"x 3 MINd NAILS
AT 6" DC
BRACE 1S A
MINIMUM 80" OF WEB
MEMBER LENGTH



APPLY SCABS 10 WIDE FACE DF WTB
NO MORE THAN (1) SCAB PER FACE
ATTACH WITH 10d BOX DP GUN
CO128 x 3 MIN) NAILS
AT 6 OC
BRACE IS A MINIMUM
307 DF WEB MEMBER LENGTH



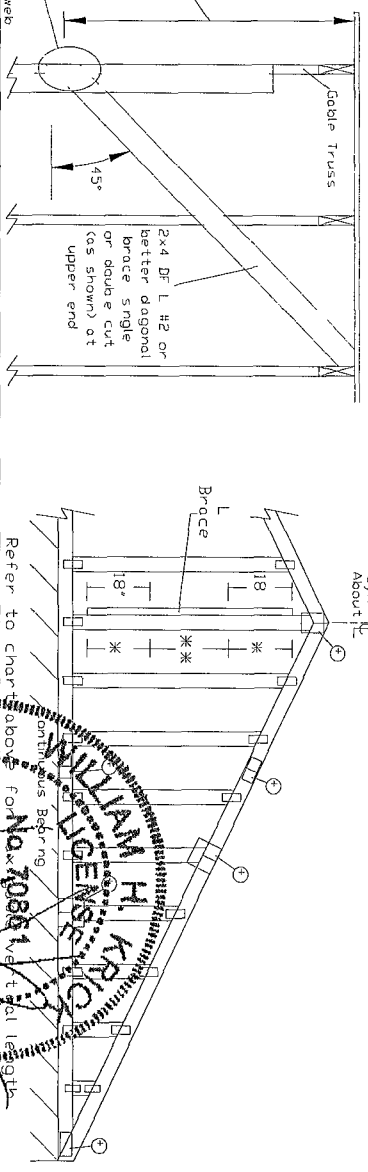
CLAB BRACE



TC LL	PSF	REF	CLB SUBST
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCB SUB0109
BC LL	PSF		
TOT LD	PSF		
DUR FAC			
SPACING			

ASCE 7-10 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00 Dr 100 mph Wind Speed 15' Mean Height Enclosed Exposure C Kzt = 1.00 Dr 100 mph Wind Speed 15' Mean Height Enclosed Exposure D Kzt = 1.00

Ma Gable Vertical Length																					
2x4 Gable Vertical Species		Brace		No Braces		(1) 1x4 L Brace *		(1) 2x4 L Brace *		(2) 2x4 "L" Brace **		(1) 2x6 L Brace *		(2) 2x6 L Brace *		Brace *					
Spacing	Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B				
12" oc	SPF	#1 / #2	4 10	8 2	8 6	9 8	10 1	11 6	12 0*	14 0	14 0*	14 0	14 0	14 0	14 0	14 0	14 0				
		#3	4 7	7 9	8 3	9 7	9 11*	11 5	11 10*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		Stud	4 7	8 1*	8 4*	9 7	9 11*	11 5*	11 10*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		Standard	4 7	8 1	8 4	9 7*	9 11	11 5*	11 10*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
	HF	#1	4 11	8 3	8 7	9 9	10 1*	11 7	12 1*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#2	4 10	8 2	8 6	9 8	10 1	11 6*	12 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#3	4 7	6 11	7 4	9 3	9 10	11 5	11 10	14 0*	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		Stud	4 7	6 11	7 4	9 3	9 10	11 5	11 10	14 0*	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
	DPL	Standard	5 6	9 5	9 9	11 1	11 6	13 2	13 9	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#1 / #2	5 6	9 5	9 9	11 1	11 6	13 2	13 9	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#3	5 3	9 3	9 9	10 11	11 4	13 0	13 7*	14 0	14 0	14 0	14 0	14 0*	14 0	14 0	14 0				
		Stud	5 3	9 3	9 9	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0	14 0*	14 0	14 0	14 0				
16" oc	HF	Standard	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#1	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#2	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#3	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
	CP	Standard	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#1	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#2	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#3	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
	DPL	Standard	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#1 / #2	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		#3	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
		Stud	5 3	9 3	9 7*	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0				
24" oc	SPF	#1 / #2	4 10	8 2	8 6	9 8	10 1	11 6	12 0*	14 0	14 0*	14 0	14 0	14 0	14 0	14 0	14 0				
		#3	4 7	7 9	8 3	9 7	9 11*	11 5	11 10*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		Stud	4 7	8 1*	8 4*	9 7	9 11*	11 5*	11 10*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		Standard	4 7	8 1	8 4	9 7*	9 11	11 5*	11 10*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
	HF	#1	4 11	8 3	8 7	9 9	10 1*	11 7	12 1*	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#2	4 10	8 2	8 6	9 8	10 1	11 6*	12 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#3	4 7	6 11	7 4	9 3	9 10	11 5	11 10	14 0*	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		Stud	4 7	6 11	7 4	9 3	9 10	11 5	11 10	14 0*	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
	DPL	Standard	5 6	9 5	9 9	11 1	11 6	13 2	13 9	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#1 / #2	5 6	9 5	9 9	11 1	11 6	13 2	13 9	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0				
		#3	5 3	9 3	9 9	10 11	11 4	13 0	13 7*	14 0	14 0	14 0	14 0	14 0	14 0*	14 0	14 0				
		Stud	5 3	9 3	9 9	10 11	11 4	13 0	13 7	14 0	14 0	14 0	14 0	14 0	14 0*	14 0	14 0				



Earth City MO 63045

Building Components Group Inc.

Diagonal brace option vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 335# at each end. Max web o c length is 14

Vertical length shown in table above

Connect diagonal at report of vertical web

WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

For use of this drawing, the user must first obtain a copy of the ASCE 7-10 Building Components Manual, which contains the full text of the provisions of the code. The user must also obtain a copy of the ASCE 7-10 Building Components Manual, which contains the full text of the provisions of the code. The user must also obtain a copy of the ASCE 7-10 Building Components Manual, which contains the full text of the provisions of the code.

11/11/2013

11/11/2013

May 23 '13

05/23/2013

MAX TOT LD 60 PSF

MAX SPACING 240

REF ASCE7-10-GAB2015

DATE 2/14/12

DRWG A12015ENC100212

Bracing Group Species and Grades

Group A		Group B	
Use Pine for	Use Fir	Use Pine for	Use Fir
#1 / #2 Standard	#2 Standard	#1 / #2 Standard	#2 Standard
#3 Stud	#3 Stud	#3 Stud	#3 Stud
Douglas fir Larch	Southern Pine**	Douglas fir Larch	Southern Pine**
#3 Stud	#3 Stud	#3 Stud	#3 Stud
Standard	Standard	Standard	Standard

Group A

1x4 L

#1 / #2

Group B

1x4 L

#1 / #2

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1x4 or 2x3
Greater than 4' 0" but less than 11' 6"	2x4
Greater than 11' 6"	2x4

* Refer to common truss design for peak splice and heel plates

Refer to the Building Designer for conditions not addressed by this detail

Truss Detail Notes

1. All bracing must be a minimum of 80% of web member length

2. For (1) L brace space n o t s at 2' o c

3. For (2) L brace space n o t s at 3' o c

4. For (3) L brace space n o t s at 4' o c

5. For (4) L brace space n o t s at 5' o c

6. For (5) L brace space n o t s at 6' o c

7. For (6) L brace space n o t s at 7' o c

8. For (7) L brace space n o t s at 8' o c

9. For (8) L brace space n o t s at 9' o c

10. For (9) L brace space n o t s at 10' o c

11. For (10) L brace space n o t s at 11' o c

12. For (11) L brace space n o t s at 12' o c

13. For (12) L brace space n o t s at 13' o c

14. For (13) L brace space n o t s at 14' o c

15. For (14) L brace space n o t s at 15' o c

16. For (15) L brace space n o t s at 16' o c

17. For (16) L brace space n o t s at 17' o c

18. For (17) L brace space n o t s at 18' o c

19. For (18) L brace space n o t s at 19' o c

20. For (19) L brace space n o t s at 20' o c

21. For (20) L brace space n o t s at 21' o c

22. For (21) L brace space n o t s at 22' o c

23. For (22) L brace space n o t s at 23' o c

24. For (23) L brace space n o t s at 24' o c

25. For (24) L brace space n o t s at 25' o c

26. For (25) L brace space n o t s at 26' o c

27. For (26) L brace space n o t s at 27' o c

28. For (27) L brace space n o t s at 28' o c

29. For (28) L brace space n o t s at 29' o c

30. For (29) L brace space n o t s at 30' o c

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32. For (31) L brace space n o t s at 32' o c

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36. For (35) L brace space n o t s at 36' o c

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86. For (85) L brace space n o t s at 86' o c

87. For (86) L brace space n o t s at 87' o c

88. For (87) L brace space n o t s at 88' o c

89. For (88) L brace space n o t s at 89' o c

90. For (89) L brace space n o t s at 90' o c

91. For (90) L brace space n o t s at 91' o c

92. For (91) L brace space n o t s at 92' o c

93. For (92) L brace space n o t s at 93' o c

94. For (93) L brace space n o t s at 94' o c

95. For (94) L brace space n o t s at 95' o c

96. For (95) L brace space n o t s at 96' o c

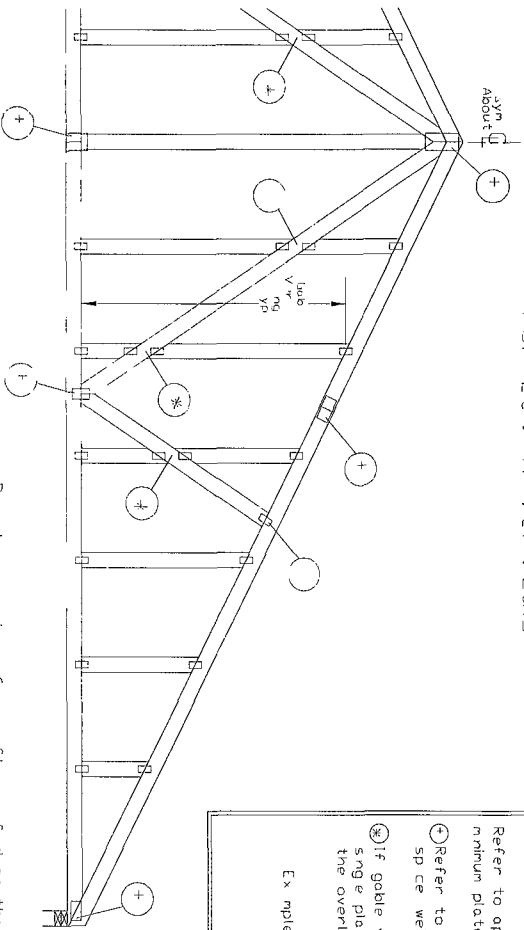
97. For (96) L brace space n o t s at 97' o c

98. For (97) L brace space n o t s at 98' o c

99. For (98) L brace space n o t s at 99' o c

100. For (99) L brace space n o t s at 100' o c

Gable Detail For Let-in Verticals



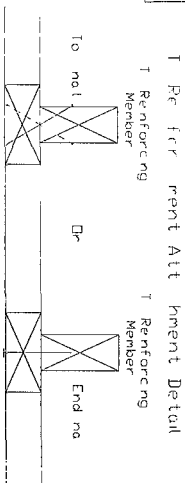
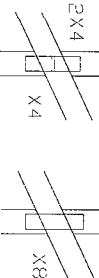
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

Refer to Engineered truss design for peak splice web and heel plates

If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



Provide connections for up to 4 ft specified on the engineered truss design

Actual reinforcing member with

End Dr. Nail

10d Common (0.148 x 3 in) Nails at 4 OC plus

(4d) nails in the top and bottom chords

Toenailed No. 5

10d Common (0.148 x 3 in) Toenails at 4 OC plus

(4d) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE

Wind load

ASCE 7-98 Gable Detail Drawings

A13015980109 A12015980109 A1015980109

A13030980109 A12030980109 A1030980109

ASCE 7-02 Gable Detail Drawings

A13015020109 A12015020109 A1015020109

A13030020109 A12030020109 A1030020109

ASCE 7-05 Gable Detail Drawings

A13015050109 A12015050109 A1015050109

A13030050109 A12030050109 A1030050109

ASCE 7-10 Gable Detail Drawings

A1301500212 A1201500212 A101500212

A1303000212 A1203000212 A103000212

A1303000212 A1203000212 A103000212

A1303000212 A1203000212 A103000212

A1303000212 A1203000212 A103000212

A1303000212 A1203000212 A103000212

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Building Components Group Inc.

Earth City MO 63045

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

1. Use of this drawing is limited to the specific conditions and details shown. Any deviation from the drawing, including but not limited to, the use of different materials, dimensions, or details, without the written approval of the manufacturer, is at the user's risk. The manufacturer is not responsible for any failure or damage resulting from the use of this drawing for any purpose other than the one intended.



REF LET-IN VERT

DATE 2/16/12

DRWG GBLTIN0212

MAX TOT LD 60 PSF

DUR FAC ANY

MAX SPACING 240

To convert from L to T reinforcing member, multiply T in rease by length (based on appropriate ITW gable detail)

Maximum allowable T reinforced gable vertical length is 14' from top to bottom chord

T reinforcing member material must match size, species and grade of the L reinforcing member

Web length increase w/ T brace

T Reinf	Member Size	Increase
2x4	30 7'	
2x6	20 7'	

Example

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft Kzt = 1.00

Gable Vertical = 24 OC Sp #3

T Reinforcing Member Size = 2x4

T Brace increase (from above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8 7'

Maximum T Reinforced Gable Vertical Length = 130 x 8 7' = 11 2'

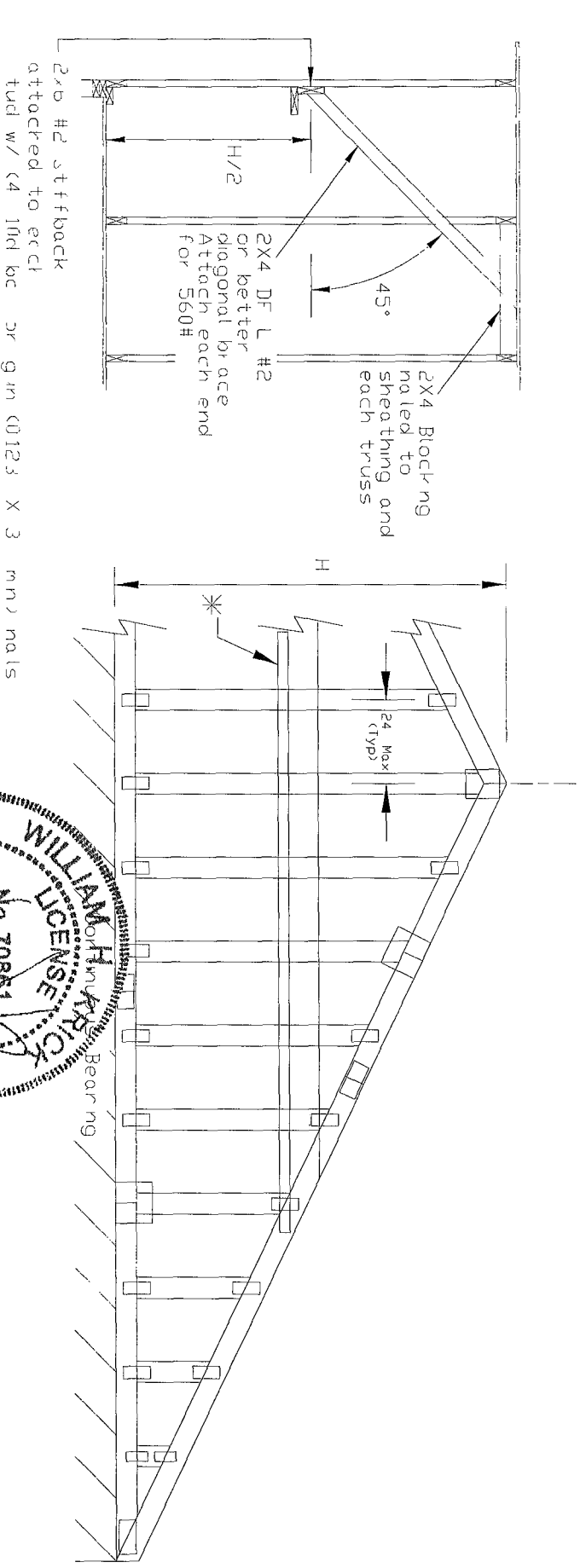
ACE 7-10 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirement - Stiffener

120 rpl 30ft Mean Hgt ASCE 7-10 Enclosed Exp C or
 100 mph 30ft Mean Hgt ASCE 7-10 Enclosed Exp D or
 100 mph 30ft Mean Hgt ASCE 7-10 Part Enclosed Exp C
 K=1 100 Wind TL DL-50 psf Wind BC DL 50 psf

Later al chord bracing requirement
 Top (continuous roof sheathing)
 Bot (or mous ceiling sheathing)

See Engineer's detail for reference to this detail
 for lumber plate and other information not shown
 in this detail

Nails 10d box or gnr (0128 x 3 min) nail



H Less than 46 no stud bracing required
 H Greater than 46 to 76 in length
 provide a 2x6 stiffback at mid height and brace stiffback
 to roof diaphragm every 40' (see detail below or
 refer to DWG A12030ENC10)
 H Greater than 76 to 120 m
 provide a 2x6 stiffback at mid-height and brace
 to roof diaphragm every 40' (see detail below or
 refer to DWG A12030ENC10)
 * Optional 2x L reinforcement attached
 to stiffback with 10d box or gnr
 (0128 x 3 min) nails @ 6' o.c.

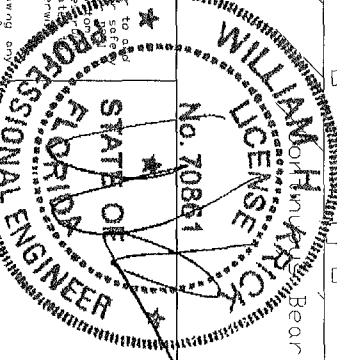


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Earth City MO 63045

WARNING READ AND OBLIO ALL NO E. ON HIS DRAWING
 IMPORTANT FINISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

These are minimum requirements for the design and construction of the structure. The designer is responsible for providing the necessary information for the contractor to construct the structure in accordance with the requirements of the building code. The contractor is responsible for providing the necessary information for the designer to design the structure in accordance with the requirements of the building code.



REF	GE	WHALE
DATE	2/14/12	
DRWG	GABRST100212	

MAX	TOT	LD	60	PSF
MAX	SPACING			