

CONNECTOR TABLE Uplift SP Uplift SPF Truss Connector To Truss/Rafter 4-8dx1 1/2 5-8dx1 1/2' 9-10d1 1/2" 9-10d1 1/2 6-10d1 1/2 LTS12-20 6-10d1 1/2" 7-10d1 1/2 MTS12-30 HTS20-30 2-10d1 1/2 Uplift SP Uplift SPF Strap Ties To One Member To Other Member Uplift SP Uplift SPF Stud Plate Ties To Stud To Plate LSTA24 10-10d wrap under or over plat LSTA24 14-10d wrap under or over plat Uplift SP Uplift SPF Holdowns @ Stemwall To Stud / Post Anchor 8-SDS 1/4"x1 1/2" 1/2"x12" Titen HD 18-16dx2 1/2" 1/2"x12" Titen HD Uplift SP Uplift SPF Holdowns @ Mond To Stud / Post 8-SDS 1/4"x1 1/2" 1/2"x6" Titen HD 1/2"x12" Titen HD 8-16dx2 1/2" Uplift SP Uplift SPF Post Bases @ Stemwall 5/8"x12" Drill & Epoxy 5/8"x12" Drill & Epoxy 5/8"x7" Drill & Epoxy 5/8"x7" Drill & Epoxy 12-16d

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE O STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. END ZONE EXAMPLE 16" \cap C \vee 0.8 = 12.8" \cap C \vee

| (END ZONE EXAMPLE 16 O.C. X U.8 - 12.8 O.C.) | | |
|--|-----------------------|--|
| (1) 2x4 @ 16" OC | TO 10'-1" STUD HEIGHT | |
| (1) 2x4 @ 12" OC | TO 11'-2" STUD HEIGHT | |
| (1) 2x6 @ 16" OC | TO 15'-7" STUD HEIGHT | |
| (1) 2x6 @ 12" OC | TO 17'-3" STUD HEIGHT | |

24" OC

18" OC

16" OC 3" OC

5" OC

4" OC

5" OC

4" OC

3" OC

GRADE & SPECIES TABLE

| | | Fb | Е | |
|-----|--------------|------|-----|--|
| 8x | SP #2 | 925 | 1.4 | |
| x10 | SP #2 | 800 | 1.4 | |
| x12 | SP #2 | 750 | 1.4 | |
| LB | 24F-V3 SP | 2600 | 1.9 | |
| SL | TIMBERSTRAND | 1700 | 1.7 | |
| VL | MICROLAM | 2950 | 2.0 | |

GENERAL NOTES:

RUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN PLACEMENT PLANS TEMPORARY AND PERMANENT BRACING DETAILS RUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR LL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIEY THE TRUSS DESIGNER. BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END. SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOLINDATION: CONFIRM THAT THE FOLINDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE) CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1 4 x W1 4 FB = 85KSL WELDED WIRE

THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185: LOCATED IN MIDDLI

CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN CCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS: SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT

CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS. ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN FOLIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

HE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN RUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS. MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES

RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN CRITERIA & LOADS:

CODE FOR DESIGN LOADS

(BUILDER MUST FIELD VERIFY)

(BUILDER MUST FIELD VERIFY

ENCLOSURE CLASSIFICATION

C&C DESIGN PRESSURES SEE TABLE

SOIL BEARING CAPACITY 1500 PSF

TOPOGRAPHIC FACTOR

INTERNAL PRESSURE

MEAN ROOF HEIGHT

FLOOR LOADING

ROOMS OTHER THAN

SLEEPING ROOMS

ROOF LOADING

12:12 & GREATER

FLOOD ZONE

WIND AREA (FT2)

FLAT OR < 4:12

4:12 TO < 12:12

WINDLOADS

(ASCE 7-22, 3S GUST)

WIND EXPOSURE

RISK CATEGORY

COEFFICIENT

OOF ANGLE

FLORIDA BUILDING CODE RESIDENTIAL

ASCE 7-22

40 PSF LIVE LOAD

30 PSF LIVE LOAD

20 PSF LIVE LOAD

16 PSF LIVE LOAD

12 PSF LIVE LOAD

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)

THIS BUILDING IS NOT IN THE FLOOD ZONE

+21.8(Vasd) -23.6(Vasd) +21.8(Vasd) -29.1(Vasd)

+36.3(Vult) -39.3(Vult) +36.3(Vult) -48.6(Vult)

END 4' FROM ALL

OUTSIDE CORNER

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DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to

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Building Code Residential (2023) to the best of my knowledge. LIMITATION: This design is valid for one

building, at specified location. Mark Disosway P.E.

163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

JOB NUMBER: 240851 **S-1**

OF 3 SHEETS

| r c | | · · · | | | t' I i | | |
|------------------|---|---|----|------------------------------------|--------|----|---------|
| | bent 24" into th | | | | | | |
| | ension side of to vall). If the wall | | | | | | |
| | a horizontal bo | | | | | | |
| | CMU may be u | | | | | | arto or |
| STEMWALL | UNBALANCED | VERTICAL REINFORCEMENT VERTICAL REINFORCEMENT | | | | | |
| HEIGHT (FEET) | BACKFILL HEIGHT | FOR 8" CMU STEMWALL (INCHES O.C.) | | FOR 12" CMU STEMWALL (INCHES O.C.) | | | |
| (1221) | 112.0111 | | Ì | Í | , | | |
| | | #5 | #7 | #8 | #5 | #7 | #8 |
| 3.3 | 3.0 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.0 | 3.7 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.7 | 4.3 | 88 | 96 | 96 | 96 | 96 | 96 |
| 5.3 | 5.0 | 56 | 96 | 96 | 96 | 96 | 96 |
| 6.0 | 5.7 | 40 | 80 | 96 | 80 | 96 | 96 |
| 6.7 | 6.3 | 32 | 56 | 80 | 56 | 96 | 96 |
| 7.3 | 7.0 | 24 | 40 | 56 | 40 | 80 | 96 |
| 8.0 | 7.7 | 16 | 32 | 48 | 32 | 64 | 80 |
| 8.7 | 8.3 | 8 | 24 | 32 | 24 | 48 | 64 |
| 9.3 | 9.0 | 8 | 16 | 24 | 16 | 40 | 48 |
| | | | | | | | |

The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the

OPTIONAL STEM WALL FOOTING

NOTE: FOR STEM WALL FOUNDATIONS

OVER 5 COURSES IN HEIGHT THE SLAB

THE HORIZONTAL BOND BEAM REBAR SPACED THE SAME AS VERTICAL REBAR

SLAB EDGE INTERSECTION w/ STEMWALL

-#5 VERT. REBAR w/ STD. HOOK BOTTOM IN FOOTING

STEM WALL @ BOND BEAM w/

HÉADER-BLOCK BOND BEAM @

& STD. HOOK TOP IN BOND BEAM

CMU STEM WALL, MAX 5 COURSES

TABLE FOR MORE THAN 5 COURSES)

(SEE SPECIAL REINFORCEMENT

@ EACH CORNER & 96" OC

-8X8X16, RUNNING BOND,

-20" W X 10" D POURED

CONCRETE STRIP FOOTING

w/ (2) #5 REBAR CONTINUOUS

2' X 2' #5 CORNER REBARS (1) LEG EXTENDING INTO SLAB &

(1) LEG LAPPED w/

-(1) #5 CONTINUOUS IN

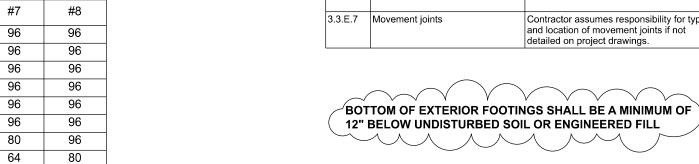
CONCRETE SLAB —

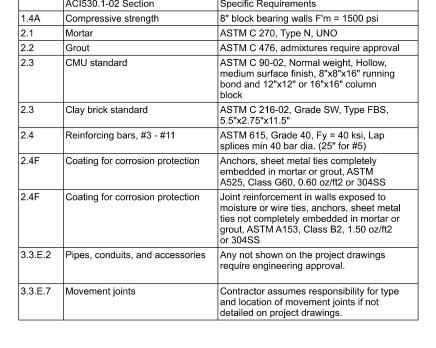
COVER (TYP.)

TALL STEM WALL TABLE:

SCALE: 1/2" = 1'-0"

IS REQUIERED TO BE ATTACHED TO THE





MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT

SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION

FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE

PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY

BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

MASONRY NOTE:

| | ACI530.1-02 Section | Specific Requirements |
|---------|----------------------------------|--|
| 1.4A | Compressive strength | 8" block bearing walls F'm = 1500 psi |
| 2.1 | Mortar | ASTM C 270, Type N, UNO |
| 2.2 | Grout | ASTM C 476, admixtures require approval |
| 2.3 | CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block |
| 2.3 | Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" |
| 2.4 | Reinforcing bars, #3 - #11 | ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5) |
| 2.4F | Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS |
| 2.4F | Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. |
| 3.3.E.7 | Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. |

POCKET FOR DOOR AS NEEDED

WET-SET ANCHORS INTO PAD FOOTER (2) 1" DIA. HIGH STRENGTH **ANCHOR BOLTS** USE SIMPSON

-6x6 EMBEDDED STEEL COLUMN w/ (2) #5 X 18" REBAR

THROUGH STEEL COLUMN AT 6-18" FROM END OF COLUMN

(SEE DETAIL ON SHEET S-3)

- 24" DIA. POURED CONCRETE

FOOTING @ COLUMN TOTAL DEPTH FROM SLAB 6'-0"

(SEE DETAIL ON SHEET S-3)

-46" x 66" x 22" DEEP WIDENED

PAD FOOTER @ SHEARWALL

w/ #5 @ 8" OC EACH WAY

@ 3" FROM BOTTOM

(ADDJUST SPACING AS NEEDED

TO AVOID ANCHORS AND POST)

FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB,

STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWINGTHE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/

- 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/
6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS
@ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL
POLY VAPOR BARRIER W/ 6" LAPS SEALED W/
POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL
(ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT
METHOD CAN BE USED INSTEAD)

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

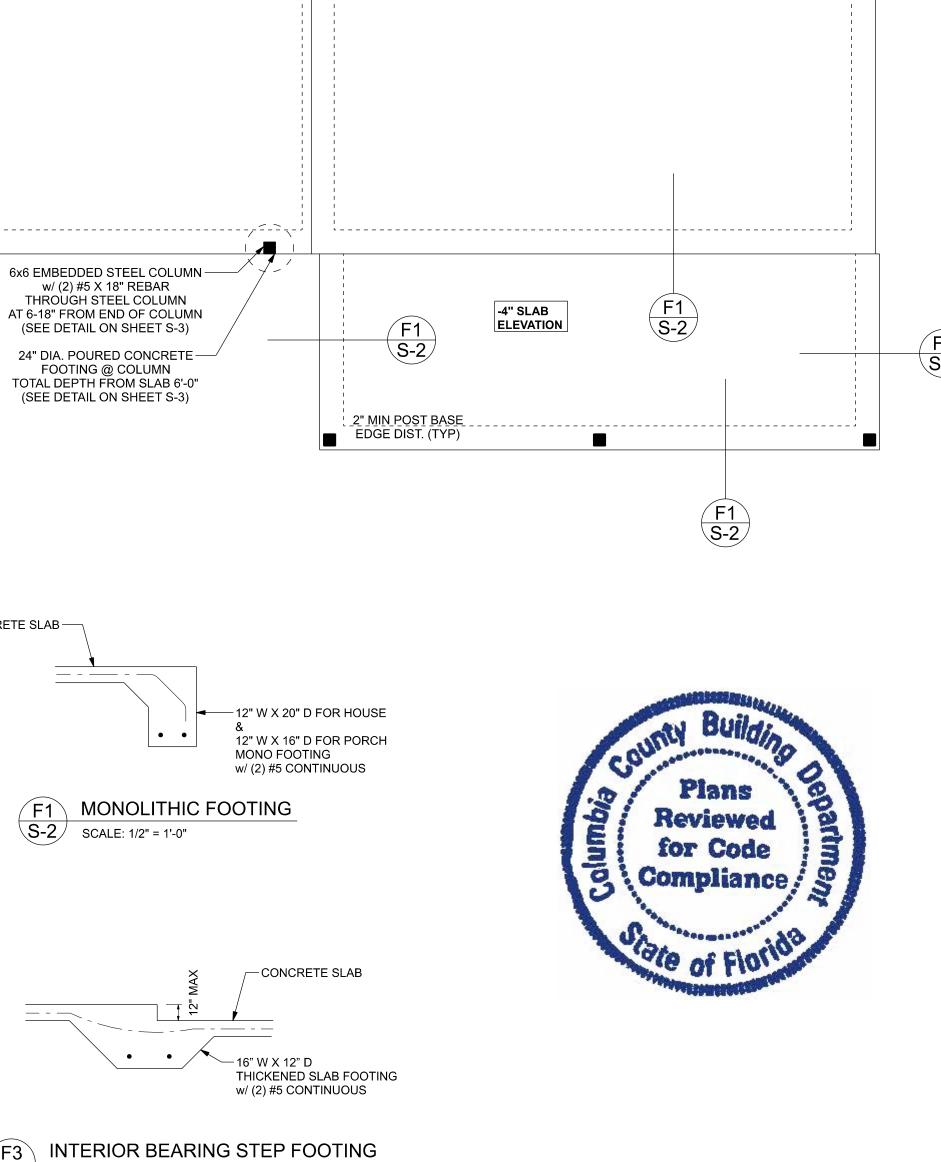
1'-9" 2'-0" 1'-9"

(2) #3 HAIRPIN GRADE 60 ---

AROUND ANCHORS

INTERIOR BEARING STEP FOOTING SCALE: 1/2" = 1'-0"

CONCRETE SLAB —



0" SLAB ELEVATION

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LIMITATION: This design is valid for one building, at specified location. Mark Disosway P.E.

163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

> JOB NUMBER: 240851 **S-2** OF 3 SHEETS

