

FOUNDATION NOTES:

01) TIE-DOIN STRAPS TO BE 1/4"x3/8" GALVANIZED STEEL, FEDERAL SPECIFICATION 1005-184-11 TYPE 1, FINISH "B", GRADE "1". TIE-DOIN STRAPS & CONNECTING HARDWARE TO HAVE 4.75" MINIMUM ULTIMATE CAPACITY. LISTED ANCHORS TO HAVE 3,150 LBS MINIMUM WORKING LOAD CAPACITY.

02) SEE FOUNDATION PLAN FOR TIE-DOIN STRAP PLACEMENT.

03) ALL PIERS SHALL BE CONSTRUCTED OF 8"x8"x1'-4" CHL. BEAR UPON A65 PADS (EXCEPTIONS, WHERE NOTED OTHERWISE ON FOUNDATION PLAN).

04) MINIMUM SOIL BEARING CAPACITY IS 2,000PSF. (ASSUMED). OWNER / BUILDER VERIFY SOIL BEARING CAPACITY.

05) IT WILL BE THE BUILDING OWNER'S RESPONSIBILITY TO INSURE THAT ALL GRASS, LOOSE DEBRIS, ETC. ARE REMOVED FROM UNDER THE BUILDING, (SEE SECTIONS FOR PIER BEARING CAPACITY) TO BE LEVEL TO WITH IN 6" AND FLATLY COMPACTED. (SEE SECTIONS FOR PIER BEARING CAPACITY)

06) WOOD SHIMS MAY BE INSTALLED, WHEN NECESSARY, BETWEEN THE "I-BEAM" & TOP OF THE PIER. SHIMS SHALL BE FREE OF KNOTS, CHECKS, OR DISCOLORATION. SHIMS SHALL BE P1.

07) LUMBER OR CEDAR, NO EXCEPTIONS, BEARING AT ALL CONTACT POINTS SHALL NOT BE LESS THAN 3/4" THICK.

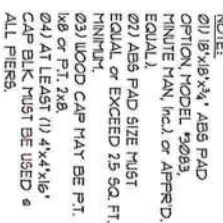
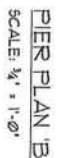
08) THE BEARING PRIOR TO ADDING SHIMS.

09) TIE CONDITIONS "W" DRY STACK PIERS GREATER THAN 34" HIGH, SHALL REQUIRE FOUNDATIONS TO BE ENGINEERED. LOCALLY, BY BEYOND THIS APPROVAL.

10) THE TIE-DOIN STRAP SPACING MAY VARY DUE TO UNFORESEEN SITE CONDITIONS. SPACING SHALL NOT EXCEED FOUNDATION PLAN MAXIMUMS.

11) MAXIMUM WIND SPEED OF 140MPH, ASCE-7-02, ALL TIE-DOIN ANCHORS SHALL HAVE A ULTIMATE OF 4.75 LBS. CAPACITY. 1" SHALL BE INSTALLED IN ACCORDANCE W/ NIG. RECOMMENDATIONS.

12) FOUNDATION PLAN INDICATES MINIMUM PIER REQUIREMENTS TO RESIST GRAVITY LOADS. ACTUAL FOUNDATION CONSTRUCTION / INSTALLATION IS SUBJECT TO LOCAL CODES & INSPECTION.



FLOOD LIVE LOAD: 1.  
 A. 100 PSF (LOADS & CORROSION)  
 50 LB. (REINFORCED)  
 1000 LB. (CONCENTRATED)  
 ROOF LIVE LOAD: 2.  
 A. 20 PSF  
 ROOF SNOW LOAD: N/A  
 WIND LOAD: 1.  
 140 MPH  
 2. W = 10  
 3. B  
 4. ENCLOSED CLASSIFICATION:  
 GCF = .018  
 5. COMPONENT & CLADDING LOAD:  
 (ROOF S)  
 F<sub>r</sub> = .35/20 PSF ZONE 1  
 F<sub>r</sub> = .9/20 PSF ZONE 2  
 F<sub>r</sub> = .9/20 PSF ZONE 3  
 (WALLS / WINDOWS / DOORS)  
 F<sub>w</sub> = .38/20 PSF ZONE 4  
 F<sub>w</sub> = .41/20 PSF ZONE 5  
 6. THIS BUILDING IS NOT DESIGNED FOR PLACEMENT ON THE UPPER  
 HALF OF A HILL OR ESCARPMENT EXCEEDING 15 FT. IN HEIGHT.  
 1. THIS BUILDING IS NOT DESIGNED FOR PLACEMENT IN A HIGH  
 VELOCITY HURRICANE ZONE AS DEFINED BY THE FBC.  
 SEISMIC LOAD: N/A  
 FLOOD LOAD: THIS BUILDING IS NOT DESIGNED TO BE SUBMERGED OR SUBJECTED  
 TO WAVE ACTION WHEN LOCATED IN A FLOOD PRONE OR ZONE AREA  
 ABOVE THE BOTTOM OF THE STRUCTURAL "V"-BEAM MUST BE LOCATED  
 ABOVE THE BUILDING SITE FLOOD PROBE LEVEL FOR THIS BUILDING  
 TO BE LOCATED IN A FLOOD PRONE OR ZONE AREA, OR THE GRADE  
 AT THE BUILDING SITE MUST BE ABOVE THE FLOOD PLANE LEVEL.

PROFESSIONAL STATEMENT: TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE ENCLOSED PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINERAL BUILDING CODES AND THE APPLICABLE MINERAL BUILDING SAFETY STANDARDS AS DESCRIBED IN AN APPROVED GENERAL CONTRACT DOCUMENT, AND I AM A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF CALIFORNIA.

REVIEWS		
NO.	DATE	DESCRIPTION

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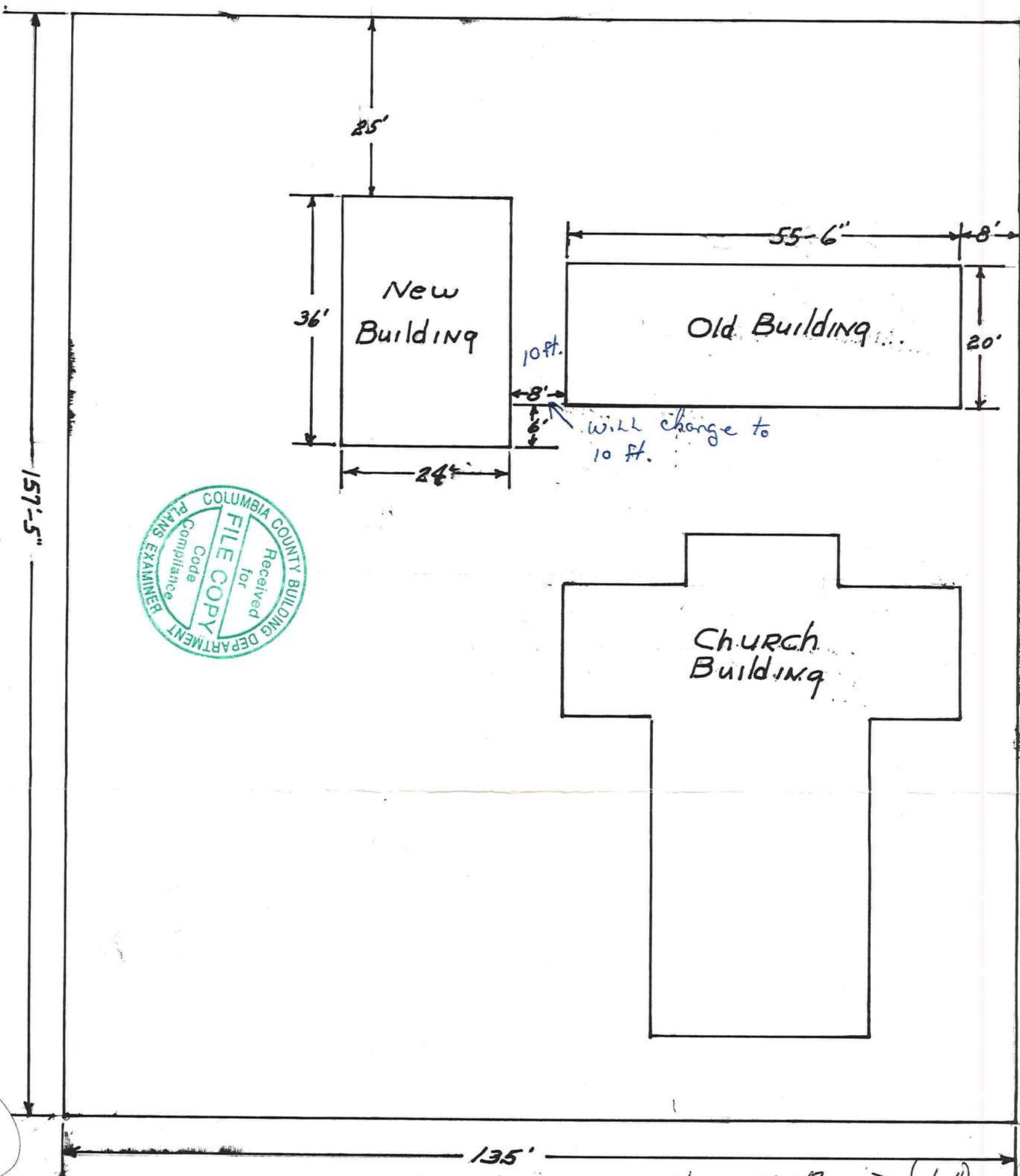
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CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND COORDINATE ALL FIELD CONDITIONS. ALL DISCREPANCIES AND CONFLICTS SHALL BE REPORTED TO THE ARCHITECT IN WRITING PRIOR TO PROCEEDING OR CONSULTING WITH CONSTRUCTION. UNREPORTED DISCREPANCIES AND CONFLICTS SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.

DRAWN BY:	M.T.
DATE:	12-17-2009
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Fort White Baptist Church



$\frac{1}{16}$  Scale = 1 Ft. Hwy SR 47 → (North)