

Office copy 1/10/22

| REVISIONS | |
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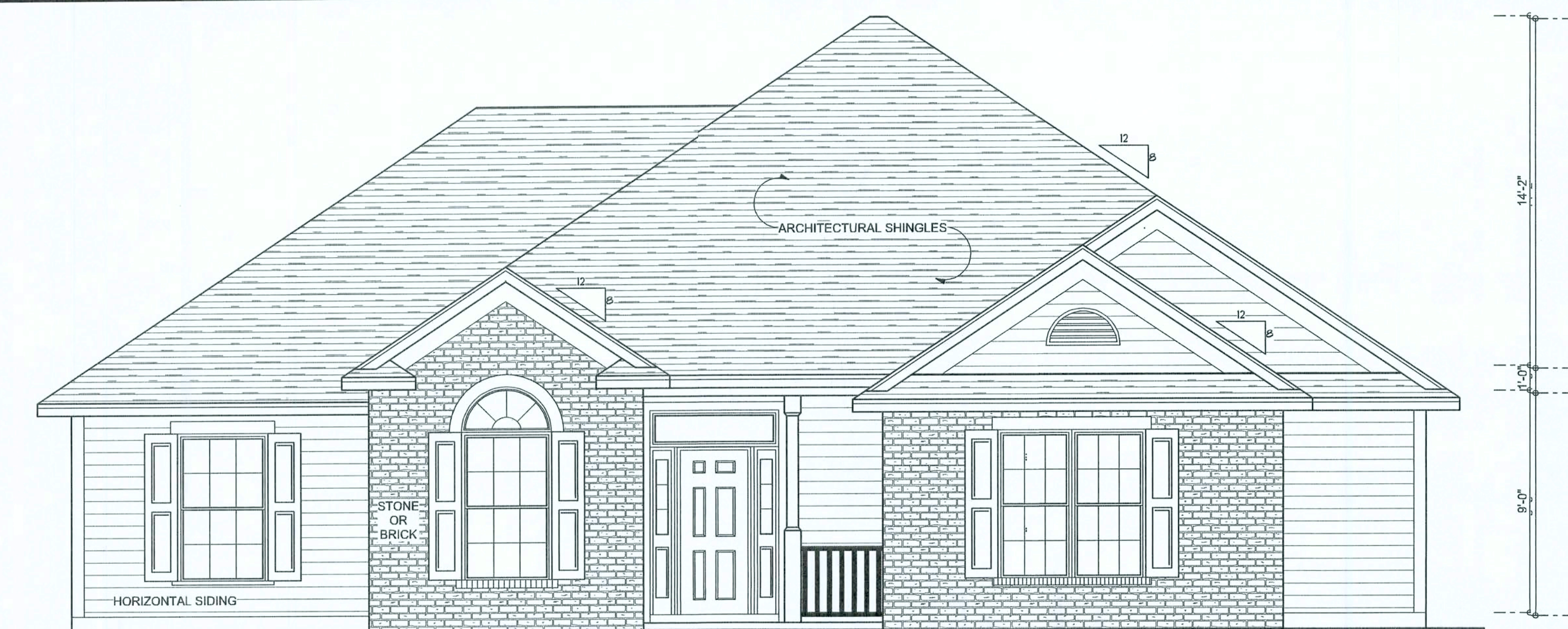
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2308.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)
2569 S.F. / 300 x 50% = 4.3 S.F. RIDGE VENT AREA REQUIRED
39.0 FEET OF RIDGE VENT or (4) OFF-RIDGE VENTS REQUIRED

SOFFIT VENT
2569 S.F. / 300 x 50% = 4.3 S.F. SOFFIT VENT AREA REQUIRED
144 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



FRONT ELEVATION
SCALE: 1/4" = 1'-0"
BUILDER IS TO VERIFY WINDOW HEIGHT ABOVE FINISHED FLOOR



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,
P.E. No. 53915, P.G. 868, Lake City, FL
33505, 386-754-6119

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida Building Code 2004 Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915
[Signature]
SEAL

BEN MARTIN
LANI III MODEL
SPEC HOUSE

ADDRESS:
Lot #16, Lolling Meadows S/D
Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
June 05, 2006

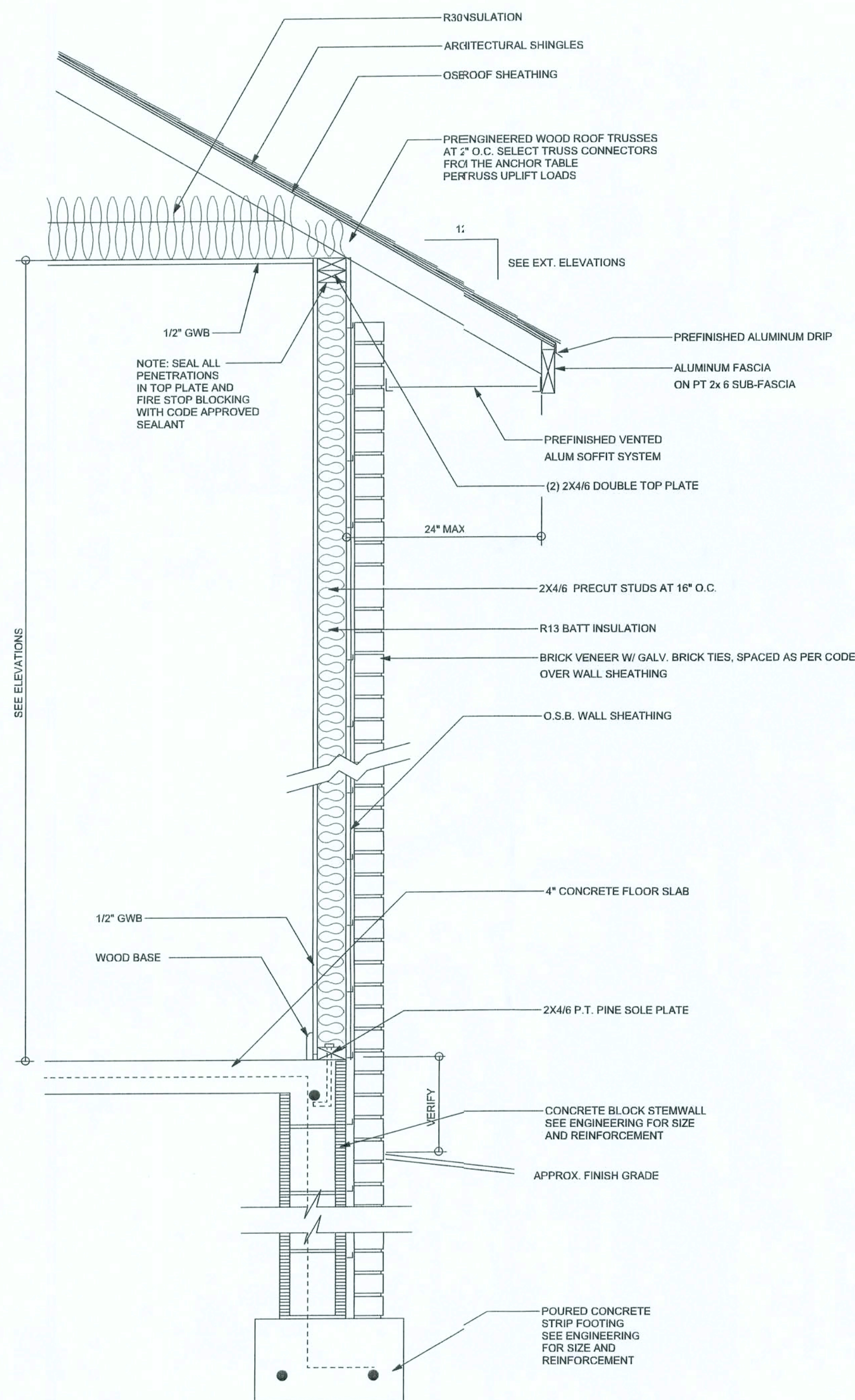
DRAWN BY: Even Beamsley
CHECKED BY:

FINALS DATE:
31 / May / 06

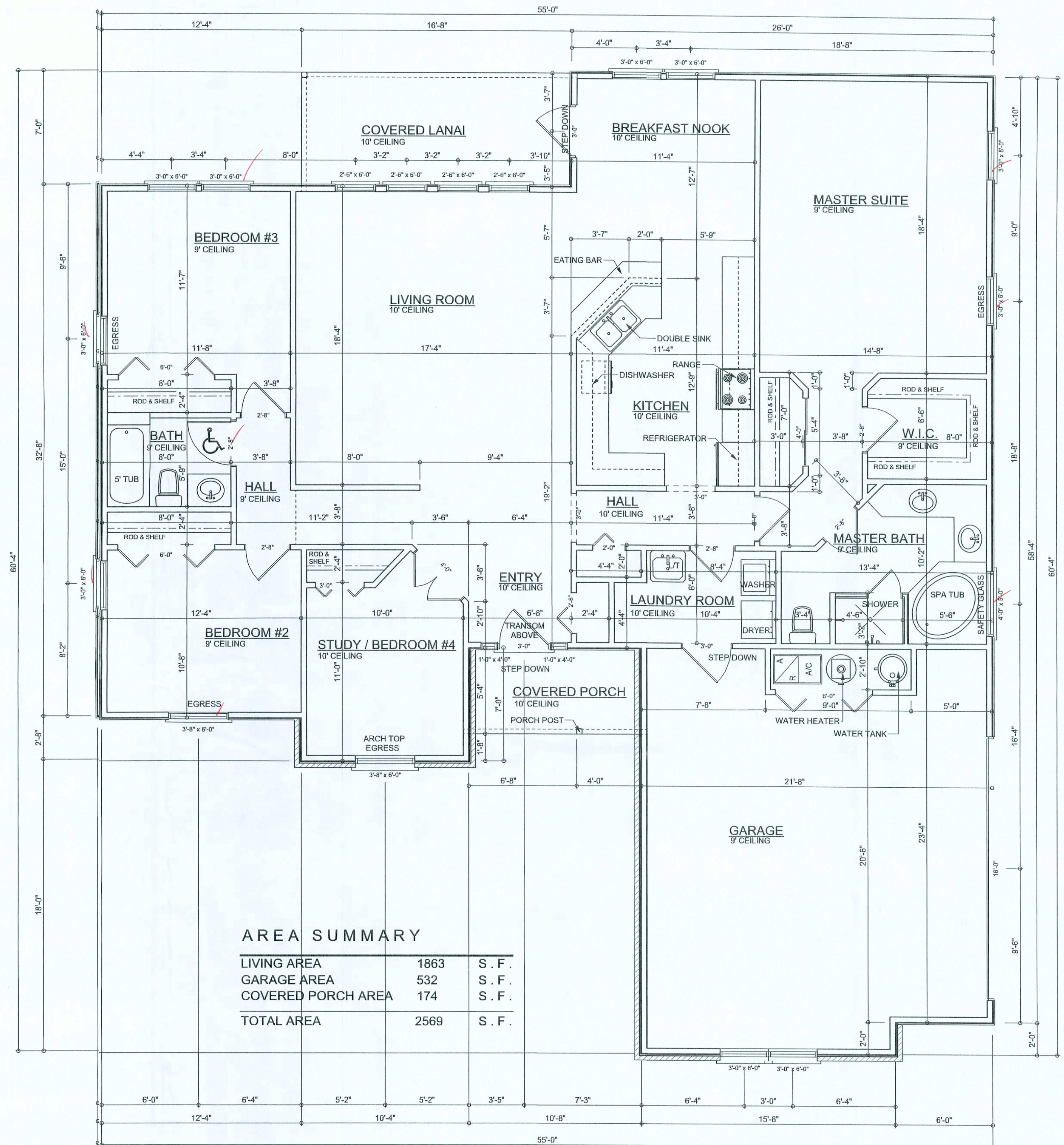
JOBNUMBER:
301251

DRAWING NUMBER
A1

OF 6 SHEETS



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1" = 1'-0"



AREA SUMMARY

| | | |
|--------------------|------|---------|
| LIVING AREA | 1863 | S . F . |
| GARAGE AREA | 532 | S . F . |
| COVERED PORCH AREA | 174 | S . F . |
| TOTAL AREA | 2569 | S . F . |

FLOOR PLAN
SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discosway
PE No. 53815, POB 888, Lake City, FL
32056, 386-754-5489

DIMENSIONS:
Stated dimensions are scaled dimensions. Refer all questions to Mark Discosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida Building Code 2004. Residual to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOSWAY
P.E. 53815

Mark Discosway
5666
SEAL

BEN MARTIN

LANI III MODEL
SPEC HOUSE

ADDRESS:
Lot #16, Rolling Meadows S/D
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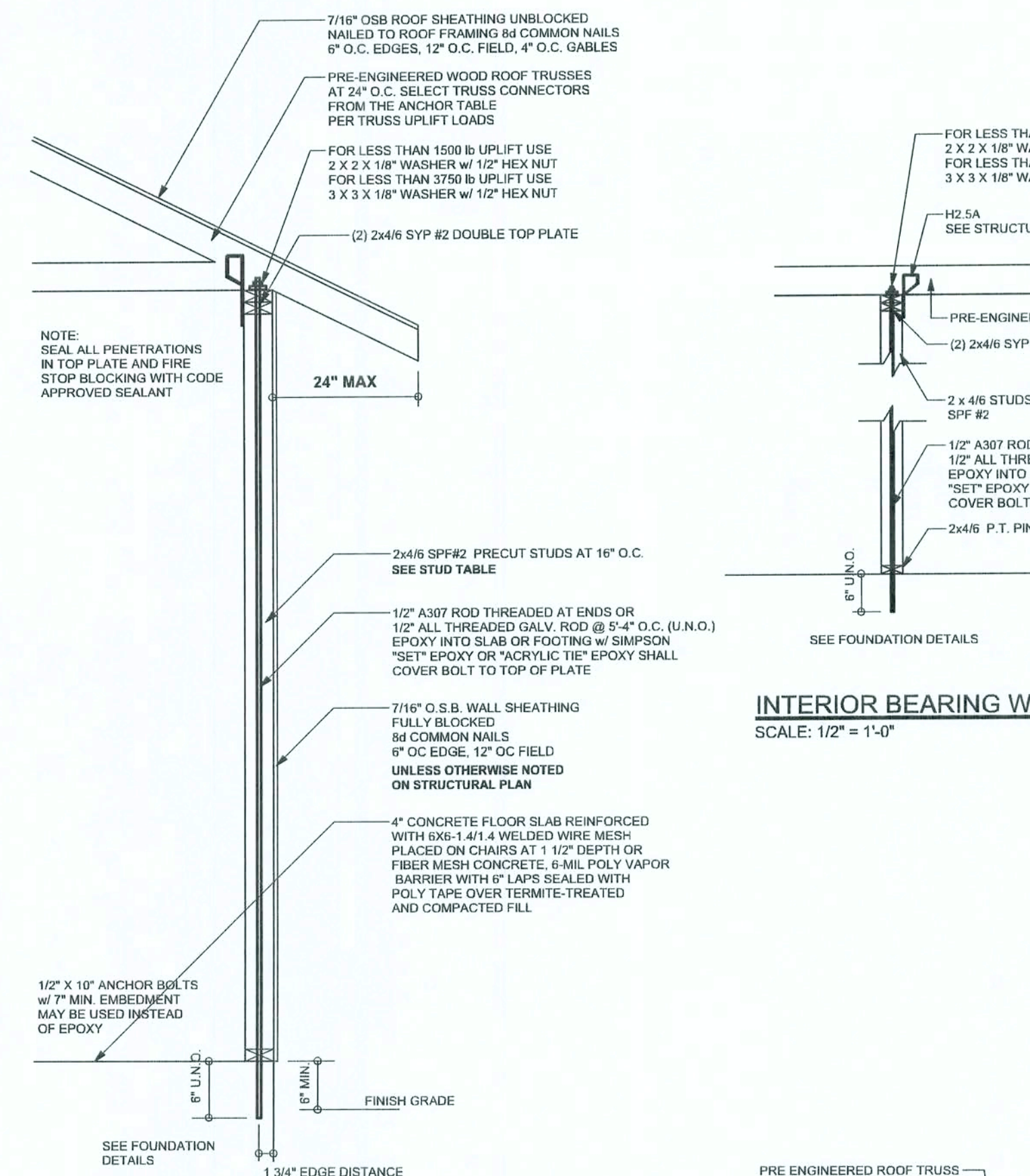
PRINTED DATE:
June 05, 2006

DRAWN BY: Evan Beamley
CHECKED BY:

FINALS DATE:
31 / May / 06

JOB NUMBER:
601251
DRAWING NUMBER

A2
OF 6 SHEETS

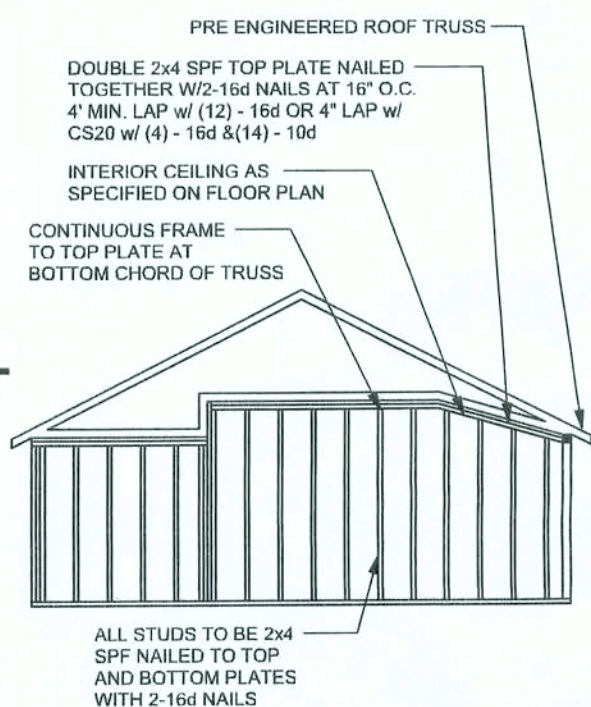


ONE STORY WALL SECTION
SCALE: 3/4\"/>

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

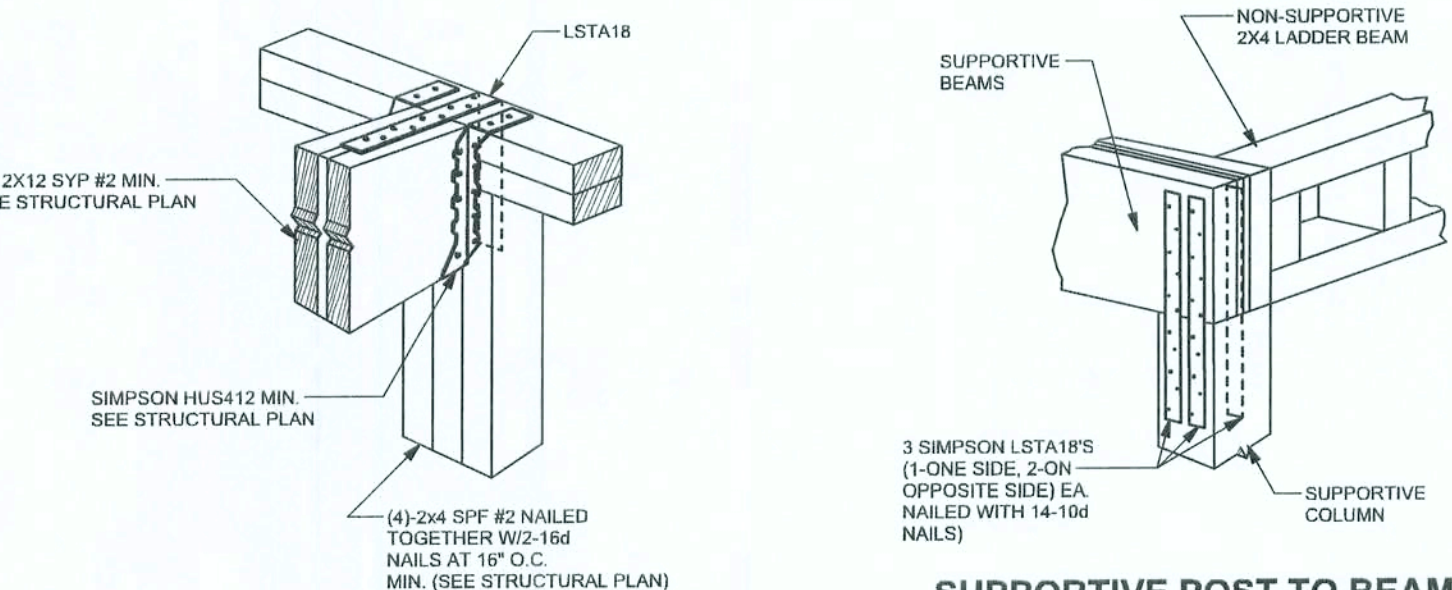
| | |
|------------------|---------------|
| (1) 2x4 @ 16\"/> | TO 11'-9\"/> |
| (1) 2x4 @ 12\"/> | TO 13'-0\"/> |
| (1) 2x6 @ 16\"/> | TO 18'-10\"/> |
| (1) 2x6 @ 12\"/> | TO 20'-0\"/> |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16\"/>



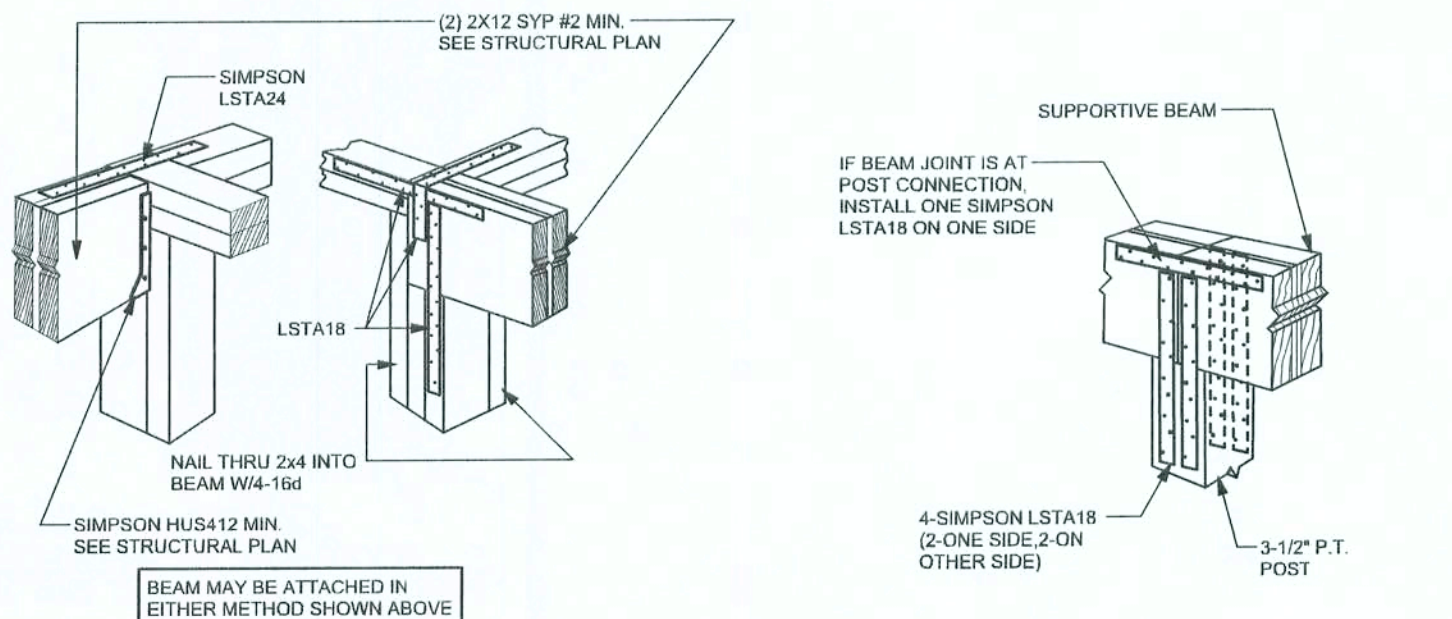
CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.

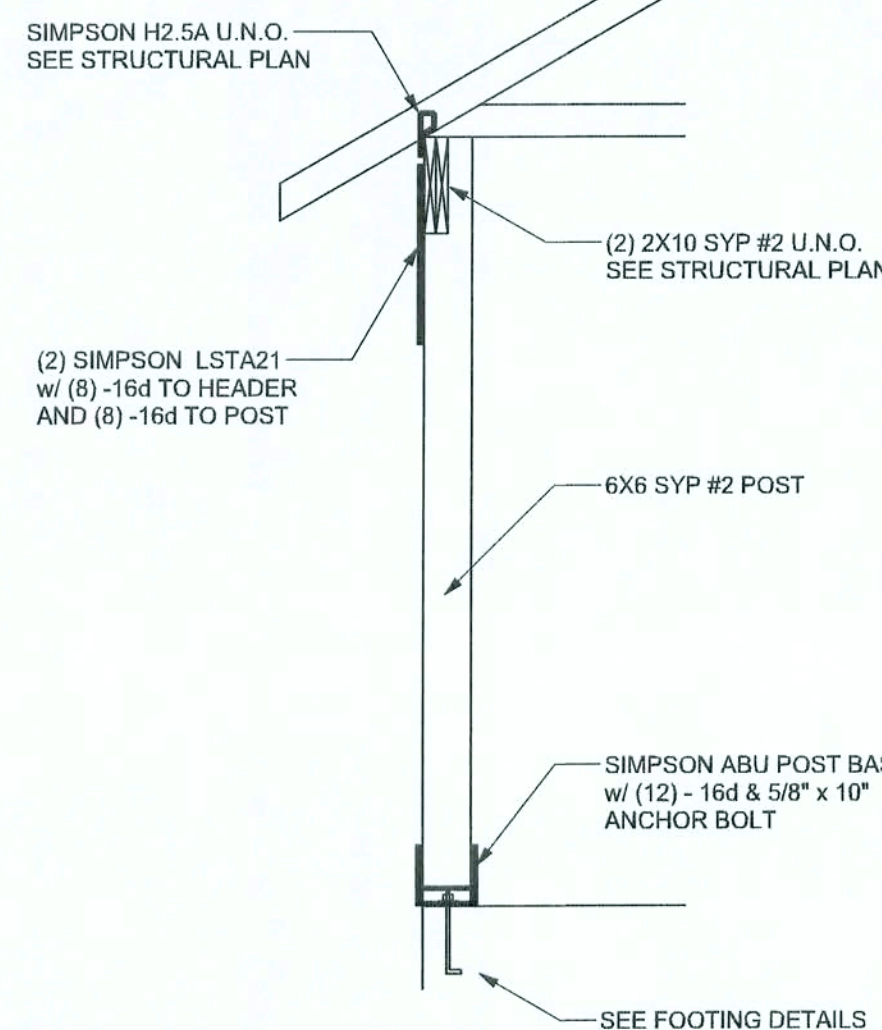


BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

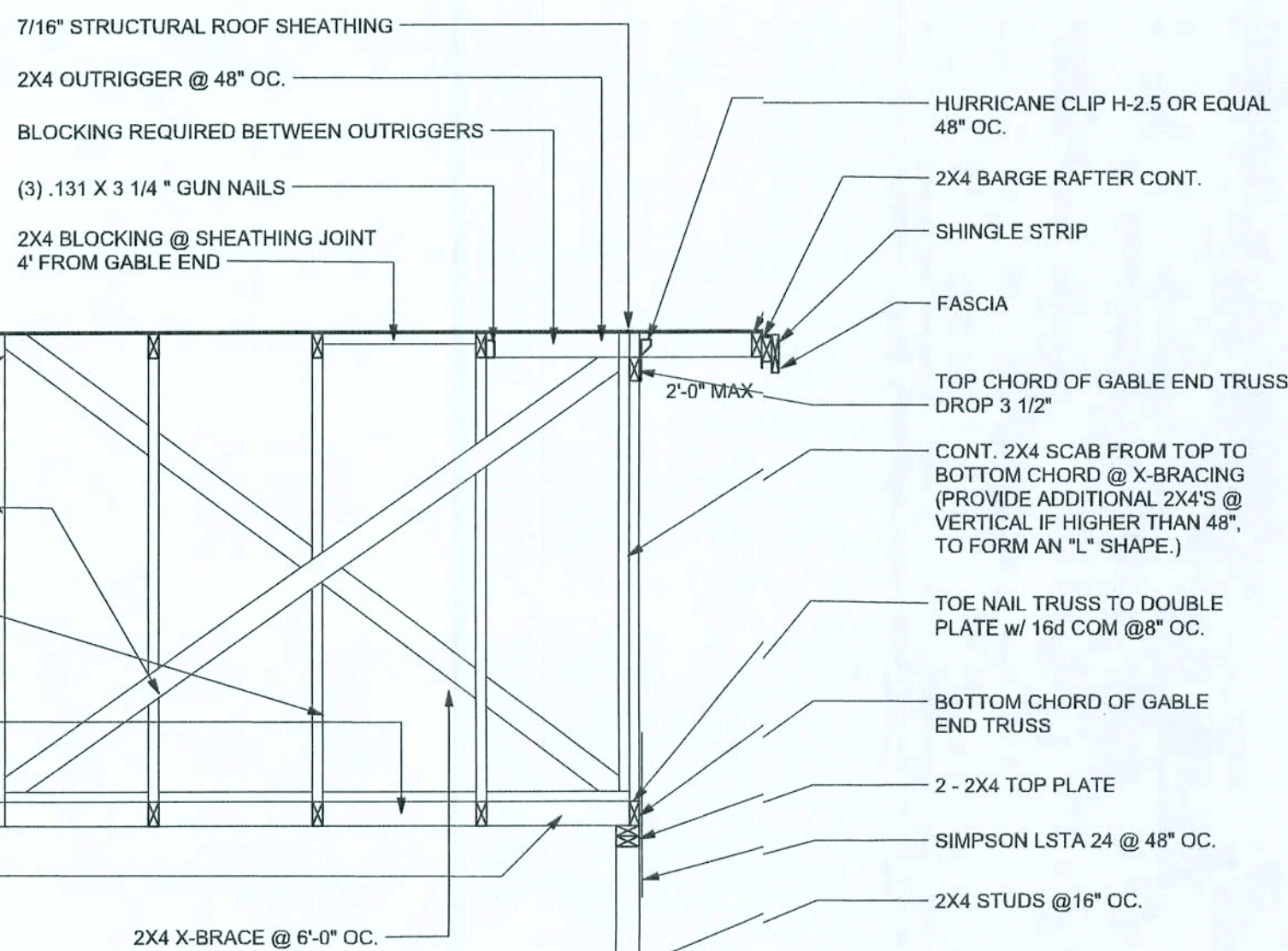
SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



TYPICAL PORCH POST DETAIL

SCALE: 1/2\"/>

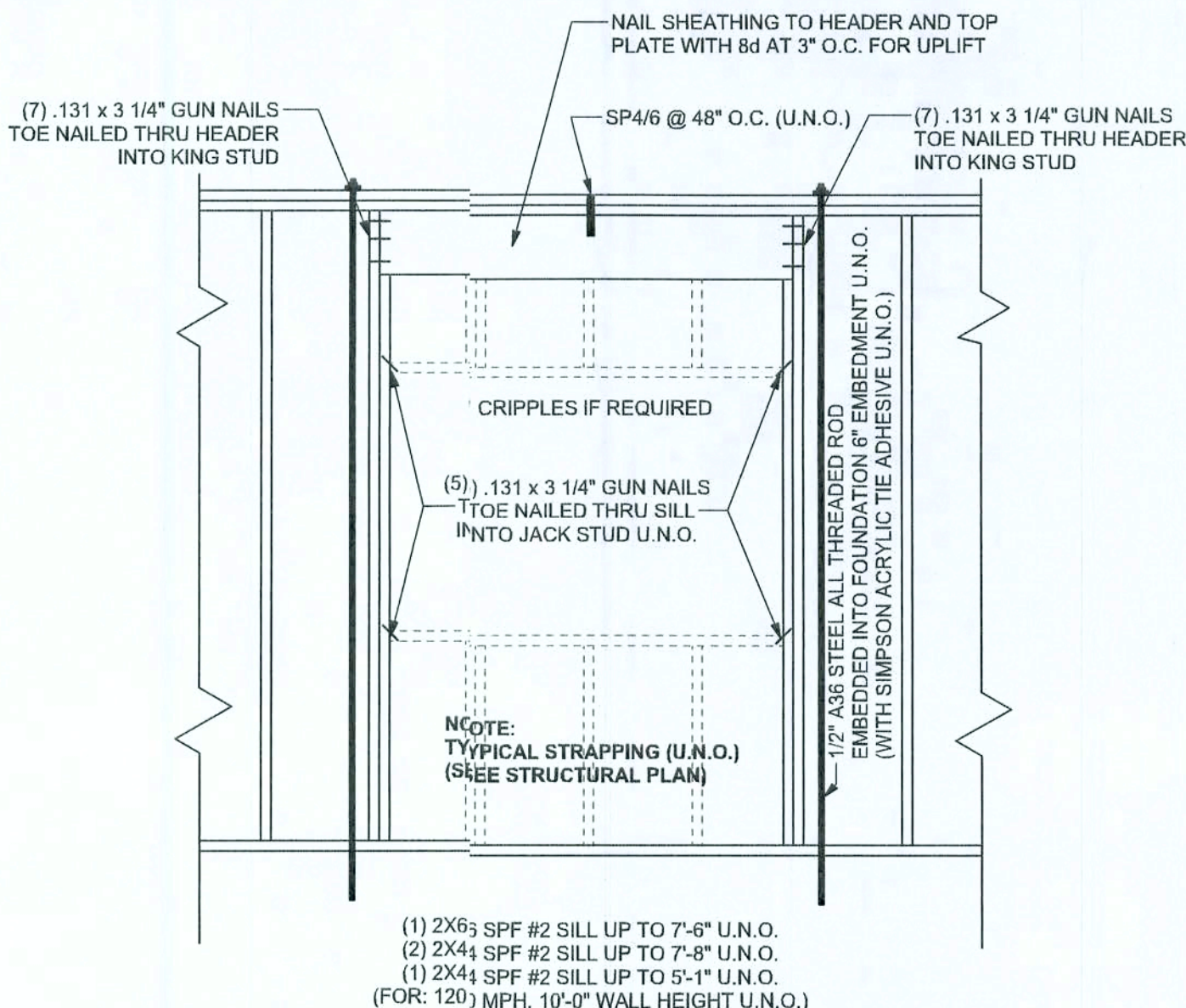


TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

NOTE:
IF TRUSSES TO WALL STRAPS ARE NAILED TO THE HEADER THE SP4/6 @ 48\"/>

FOR LESS THAN 1500 lb UPLIFT USE 2 X 2 X 1/8\"/>



TYPICAL 1 STORY HEADER STRAPPING DETAIL

SCALE: 1/2\"/>

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURERS ENGINEERING.

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|-----------------|-----------------|------------------------|-----------------|-----------------|---------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2 - HTS24 | | | |
| < 2050 | < 1785 | LG72 | 14 -16d | 14 -16d | |
| | | HEAVY GIRDER TIEDOWNS* | | | TO FOUNDATION |
| < 3865 | < 3330 | MGT | | 22 -10d | 1-5/8\"/> |
| < 10980 | < 6485 | HGT-2 | | 16 -10d | 2-5/8\"/> |
| < 10530 | < 9035 | HGT-3 | | 16 -10d | 2-5/8\"/> |
| < 9250 | < 9250 | HGT-4 | | 16 -10d | 2-5/8\"/> |
| | | STUD STRAP CONNECTOR* | | | TO STUDS |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3 -10d | | 4 -10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1 -10d | | 4 -10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6 -10d | | 8 -10d |
| < 825 | < 800 | DSP SINGLE SILL PLATE | 2 -10d | | 8 -10d |
| < 885 | < 760 | SP4 | | 6 -10d, 1 1/2" | |
| < 1240 | < 1065 | SPH4 | | 10 -10d, 1 1/2" | |
| < 885 | < 760 | SP6 | | 6 -10d, 1 1/2" | |
| < 1240 | < 1065 | SPH6 | | 10 -10d, 1 1/2" | |
| < 1235 | < 1165 | LSTA18 | 14 -10d | | |
| < 1235 | < 1235 | LSTA21 | 16 -10d | | |
| < 1030 | < 1030 | CS20 | 18 -8d | | |
| < 1705 | < 1705 | CS16 | 28 -8d | | |
| | | STUD ANCHORS* | | TO STUDS | TO FOUNDATION |
| < 1350 | < 1305 | LTT19 | 8 -16d | | 1/2\"/> |
| < 2310 | < 2310 | LTT131 | 18 -10d, 1 1/2" | | 1/2\"/> |
| < 2775 | < 2570 | HD2A | 2-5/8\"/> | | 5/8\"/> |
| < 4175 | < 3685 | HTT16 | 18 -16d | | 5/8\"/> |
| < 1400 | < 1400 | PAHD42 | 16 -16d | | |
| < 3335 | < 3335 | HPAH022 | 16 -16d | | |
| < 2200 | < 2200 | ABU44 | 12 -16d | | 1/2\"/> |
| < 2300 | < 2300 | ABU66 | 12 -16d | | 1/2\"/> |
| < 2320 | < 2320 | ABU88 | 18 -16d | | 2-5/8\"/> |

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS ENGINEERING FULLY SATISFIED WITH THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415lb EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD. PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH AC 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A JOINT LINE).

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2400$ E = 1800ksi. UNDO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING. UNDO. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE INTENDED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

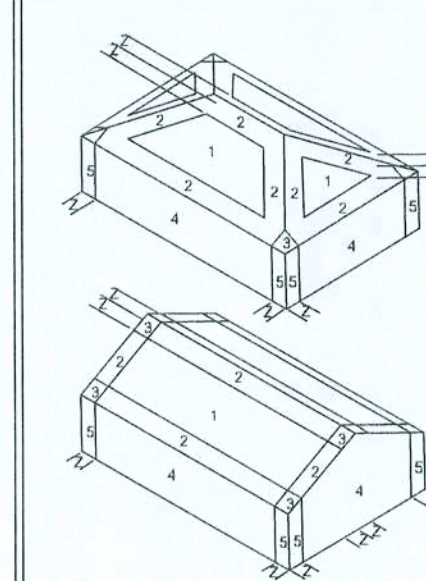
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



| Zone | Effective Wind Area (ft2) | | |
|---|---------------------------|-------|-------|
| | 10 | 100 | |
| 1 | 19.9 | -21.8 | -18.1 |
| 2 | 19.9 | -25.5 | -21.8 |
| 2 Other | - | -40.6 | -40.6 |
| 3 | 19.9 | -25.5 | -21.8 |
| 3 Other | - | -28.3 | -42.4 |
| 4 | 21.8 | -23.8 | -20.4 |
| 5 | 21.8 | -29.1 | -18.5 |
| Doors & Windows Worst Case (Zone 5, 10, 12) | 21.8 | -29.1 | |
| 6x7 Garage Door | 19.5 | -22.9 | |
| 16x7 Garage Door | 18.5 | -21.0 | |

DESIGN LOADS

- FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL DRAFTER

BEN MARTIN

LANI II MODEL
SPEC HOUSE

ADDRESS:
Lot #16, Rolling Meadows S/D
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PRINTED DATE:
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DRAWN BY:
Evan Beamsley

CHECKED BY:

FINALS DATE:
31 / May / 06

JOB NUMBER:
60'251

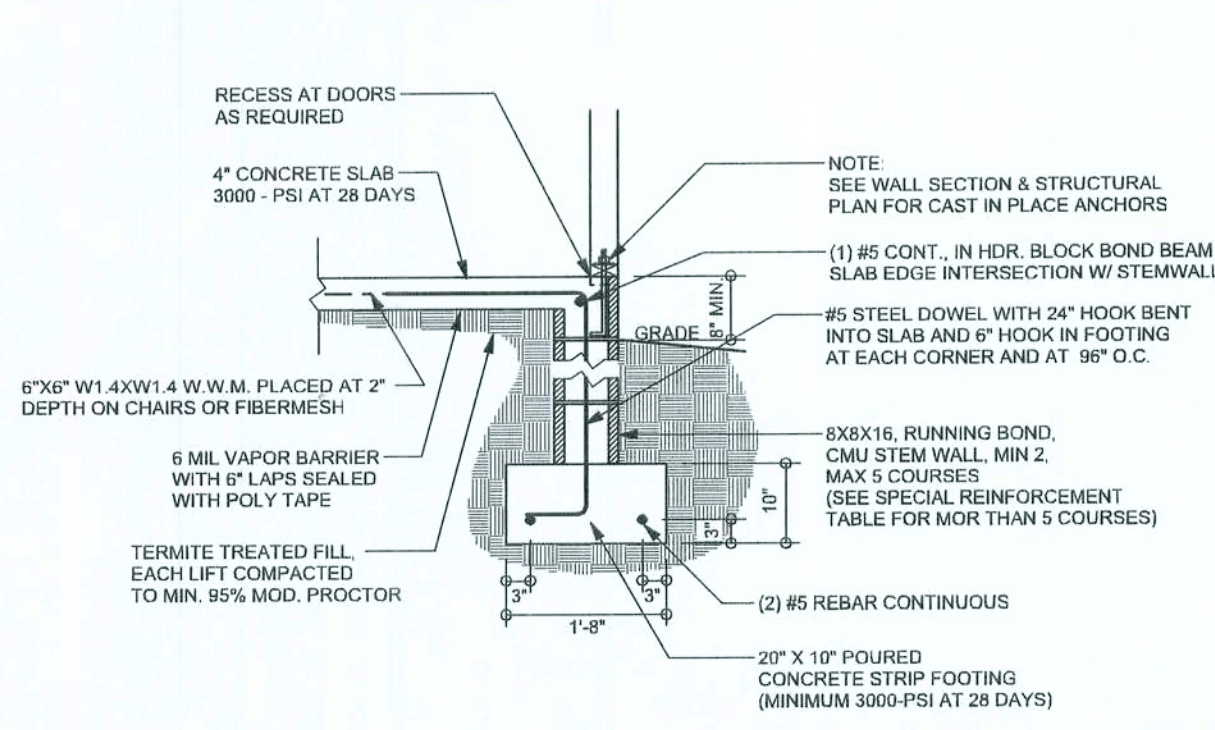
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S-1

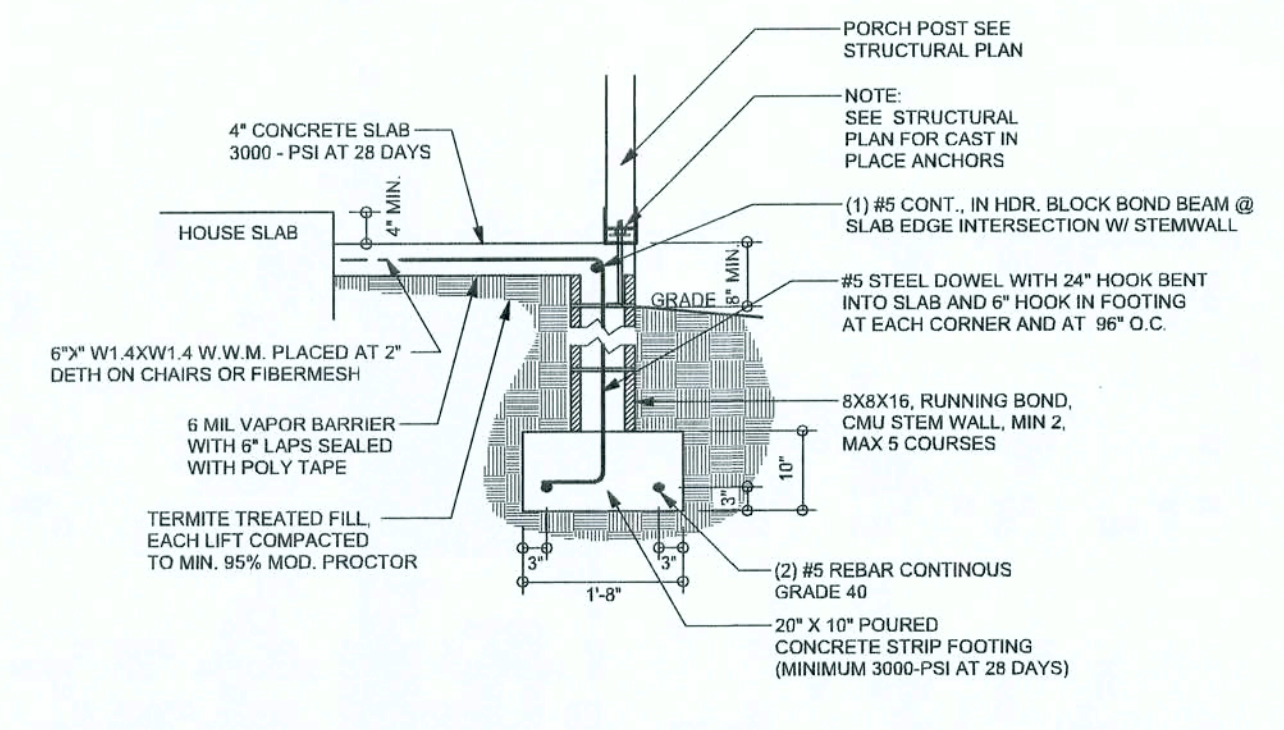
OF 6 SHEETS

REVISIONS

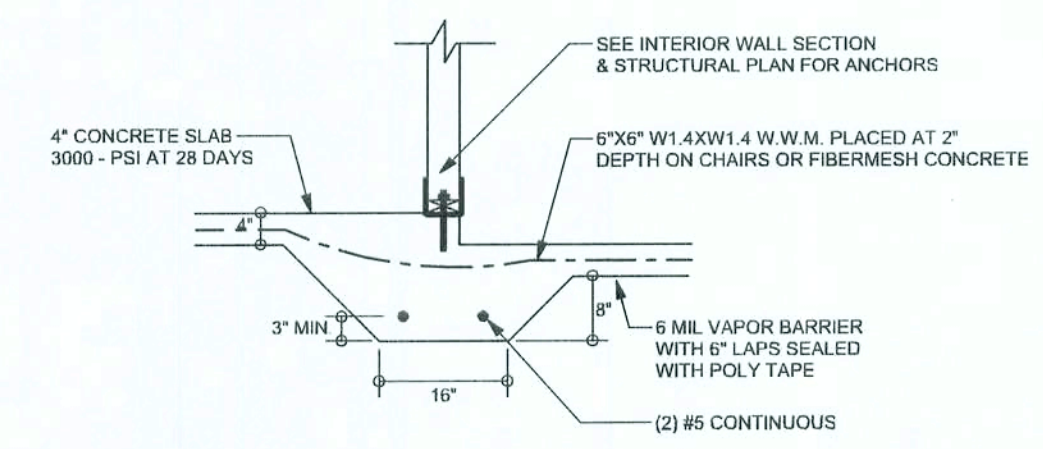
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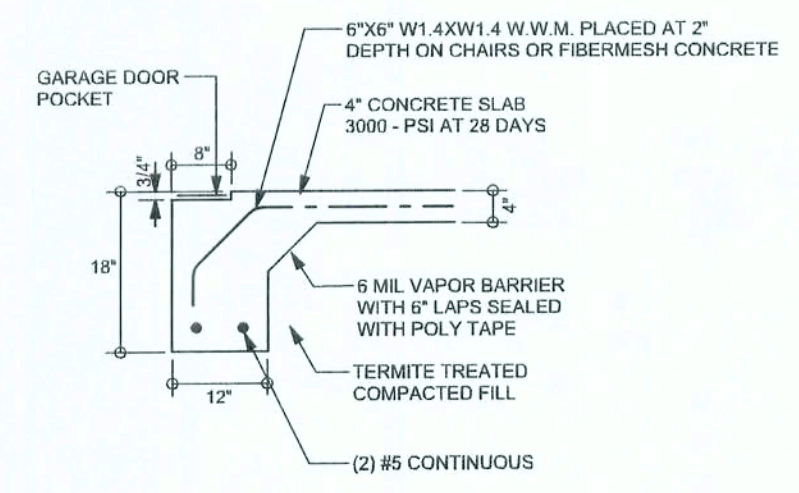
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



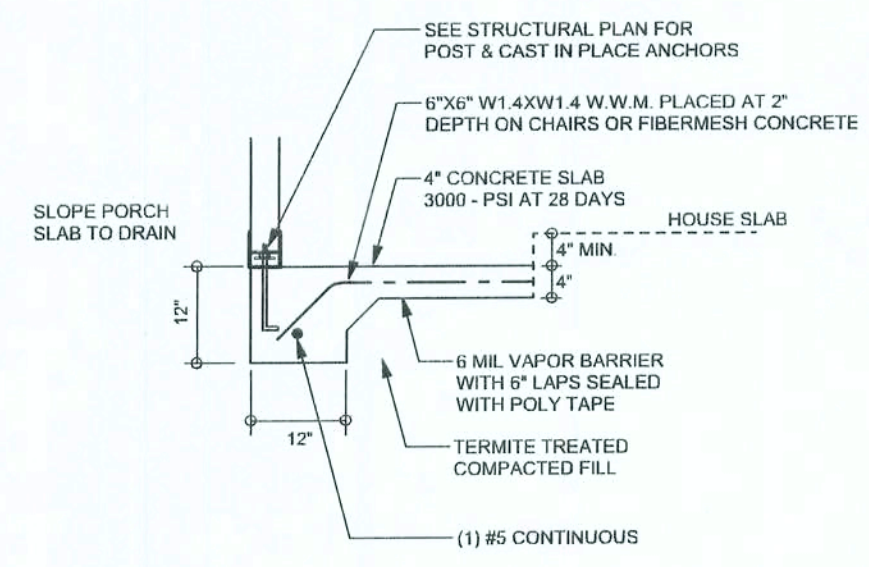
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

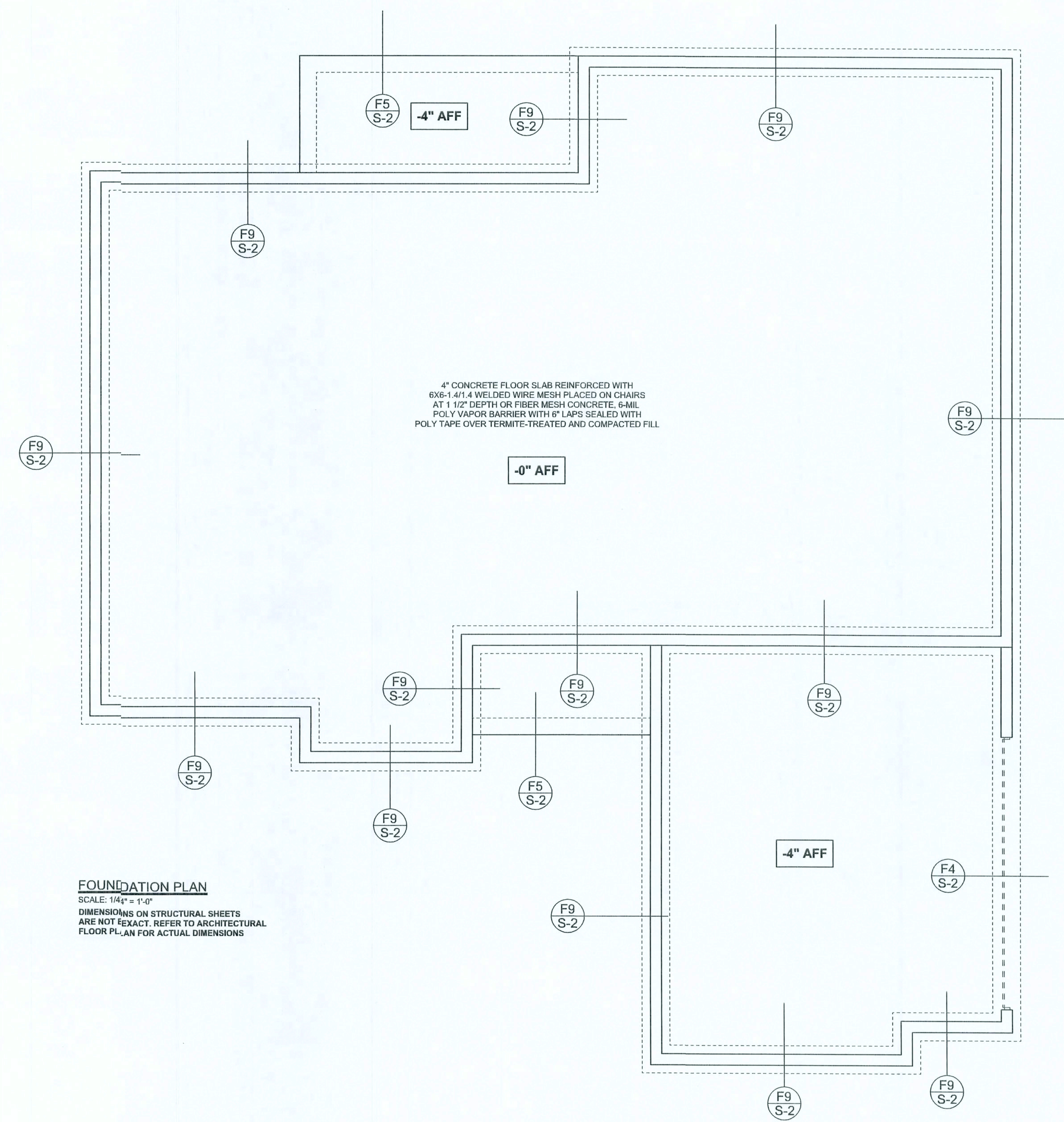


F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 145 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

| STEM WALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.) | | | VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.) | | |
|-------------------------|----------------------------|---|----|----|--|----|----|
| | | #5 | #7 | #8 | #5 | #7 | #8 |
| 3.3 | 3.0 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.0 | 3.7 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.7 | 4.3 | 88 | 96 | 96 | 96 | 96 | 96 |
| 5.3 | 5.0 | 56 | 96 | 96 | 96 | 96 | 96 |
| 6.0 | 5.7 | 40 | 80 | 96 | 80 | 96 | 96 |
| 6.7 | 6.3 | 32 | 56 | 80 | 56 | 96 | 96 |
| 7.3 | 7.0 | 24 | 40 | 56 | 40 | 80 | 96 |
| 8.0 | 7.7 | 16 | 32 | 48 | 32 | 64 | 80 |
| 8.7 | 8.3 | 8 | 24 | 32 | 24 | 48 | 64 |
| 9.3 | 9.0 | 8 | 16 | 24 | 16 | 40 | 48 |



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,
P.E. No. 53815, F90 N.W., Lake City, FL
32055, 386-7545419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disosway P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section F301.2.1, Florida Building
Code 2004 Residential to the best
of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53815
Mark Disosway
SEAL

BEN MARTIN
LAN III MODEL
SPEC HOUSE

ADDRESS:
Lot #16, Rolling Meadows S/D
Columbia County, Florida

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Phone: (386) 754 - 5419
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PRINTED DATE:
June 05, 2006

DRAWN BY:
Evan Beamley

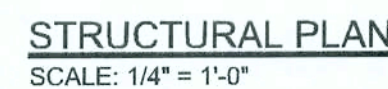
CHECKED BY:

FINALS DATE:
31 / May / 16

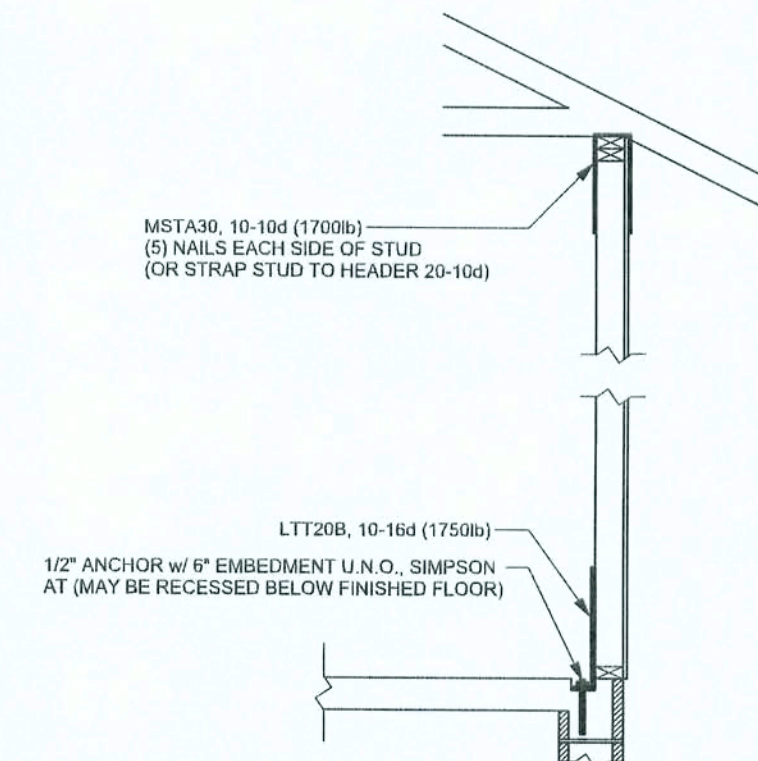
JOE NUMBER:
601251
DRAWING NUMBER

S-2
(OF 6 SHEETS)

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



| | |
|------|--|
| SN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.O.C.) |
| SN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.O.C.) |
| SN-3 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS |
| SN-4 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC511-43, BC511-81, BC511-82, & BC511-83. BC511-81, BC511-82, & BC511-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE |



SCALE: 1/2" = 1'-0"

THREADED ROD LEGEND

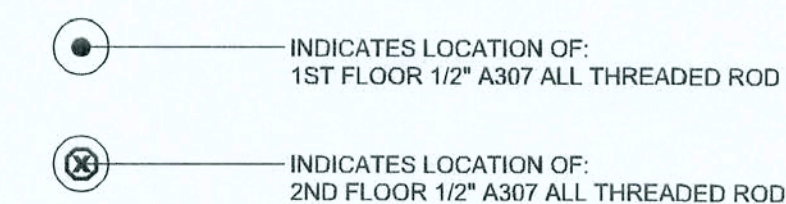


Diagram illustrating the calculation of the number of king studs (full length) for a header:

- Formula: $(2) 2X12X0', 1J 1K$
- Inputs:
 - NUMBER OF KING STUDS (FULL LENGTH)
 - NUMBER OF JACK STUDS (UNDER HEADER)
 - SPAN OF HEADER
 - SIZE OF HEADER MATERIAL
 - NUMBER OF PLIES IN HEADER

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

| | | |
|--------------|----------|--------|
| | REQUIRED | ACTUAL |
| TRANSVERSE | 36.8' | 85.0' |
| LONGITUDINAL | 34.1' | 55.5' |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #L165832

WINDLOAD ENGINEER Mark Disosway,
PE No.53915, POB 868,Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without certification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engine, comply with section R30. 2.1, Florida Building Code 2004 Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 5915

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12

Life

SEL

BEN MARTIN

LANI III MODI

SPEC HOUSE

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June 05, 2006

| | |
|---------|-------|
| OWN BY: | CHECK |
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| Beamsley | |
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| ISS DATE: | |
| May / 06 | |

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| JOB NUMBER | |

601251

DRAWING NUMBER
62

S-3

OF 6 SHEETS