

2X4 X-BRACE @ 6'-0" OC.

T'PICAL GABLE END (X-BRACING)

SUPPORTIVE BEAMS

ALL MEMBERS SHALL BE SYP

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

	(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
	(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
	(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
Ì	(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

TOP CHORD OF GABLE END TRUSS

CONT. 2X4 SCAB FROM TOP TO

BOTTOM CHORD @ X-BRACING

(PROVIDE ADDITIONAL 2X4'S @

VERTICAL IF HIGHER THAN 48",

TO FORM AN "L" SHAPE.)

TOE NAIL TRUSS TO DOUBLE

PLATE w/ 16d COM @8" OC.

BOTTOM CHORD OF GABLE

SIMPSON LSTA 24 @ 48" OC.

END TRUSS

- 2 - 2X4 TOP PLATE

- 2X4 STUDS @16" OC.

(6) .131 x 3 1/4" GUN NAILS-

INTO KING STUD

TOE NAILED THRU HEADER

2'-0" MAX

2X4 LADDER BEAM

		Fb (psi)	E (10 ⁶ psi)
22x8	SYP #2	1200	1.6
2 _{2×10}	SYP #2	1050	1.6
2 _{2x12}	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
FPSL	PARALAM	2900	2.0

PRE ENGINEERED ROOF TRUSS -DOUBLE_{E 2x4} SPF TOP PLATE NAILED -TOGETHHER W/2-16d NAILS AT 16" O.C. 4' MIN. LLAP w/ (12) - 16d OR 4" LAP w/ CS20 w/ / (4) - 16d &(14) - 10d INTERIODR CEILING AS -SPECIFIFIED ON FLOOR PLAN CONTINUOUS FRAME -TO TOP PLATTE AT BOTTOM CHYORD OF TRUSS ALL S'STUDS TO BE 2x4 AND EBOTTOM PLATES WITH 1 2-16d NAILS

> CONITINUOUS FRAME TO CEIL ING DIAPHRAGM DETAIL SCALE: NN.T.S.

> > -NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 4" O.C. FOR UPLIFT

> > > -(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TOE NAILED THRU HEADER

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET ITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD

PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25* FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi: UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	NER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARI ART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	S, FOUNDATION BEARING CAPACITY, GRADE AND EED AND DEBRIS ZONE, AND FLOOD ZONE.
	CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 STATED WIND VELOCITY AND DESIGN PRESSURES.
	OAD PATH FROM TRUSSES TO FOUNDATION. IF YOU CONTINUOUS LOAD PATH CONNECTION, CALL IMMEDIATELY.
DESIGN, PLACEMENT PLAN	ACTURER'S SEALED ENGINEERING INCLUDES TRUSS S, TEMPORARY AND PERMANENT BRACING DETAILS, TIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN DATA

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TRUSS CONNECTOR*

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

H16-1

H16-2

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWNS

MGT

HGT-2

HGT-4

STUD STRAP CONNECTORS

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SP6

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS

LTT19

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

ABU44

ABU66

ABU88

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2'

12-8d, 1 1/2'

12-8d, 1 1/2"

6-10d

7-10d 1 1/2"

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

8-8d, 1 1/2"

10-10d, 1 1/2" 2-10d, 1 1/2"

10-10d, 1 1/2" 2-10d, 1 1/2"

12-10d 1 1/2" | 12-10d 1 1/2"

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

7-10d 1 1/2"

14 -16d

6 -10d

14-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2"

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

TO STUDS

TO FOUNDATION

1-5/8" THREADED ROD

-5/8" THREADED ROD

-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

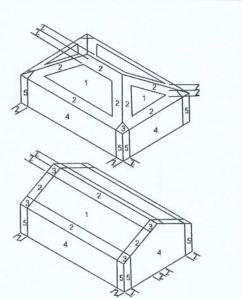
1/2" AB

2-5/8" AB

12" EMBEDMENT

12" EMBEDMENT

WIND LOADS PER FLORIDA BUILDING COD						
MEAN ROOF HEIGHT NOT EXCEEDING LEA ON UPPER HALF OF HILL OR ESCARPMENT	GS WITH FLAT, HIPPED, OR GABLE ROOFS; ST HORIZONTAL DIMENSION OR 60 FT; NOT F 60FT IN EXP. B, 30FT IN EXP. C AND >10% R 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.					
UILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE						
BUILDING IS NOT IN THE WIND-BORNE DEE	BRIS REGION					
1.) BASIC WIND SPEED = 110 MPH						
2.) WIND EXPOSURE = B						
3.) WIND IMPORTANCE FACTOR = 1.0						
4.) BUILDING CATEGORY = II						
5.) ROOF ANGLE = 10-45 DEGREES						
 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) 						
						Zone Effective Wind Area (ft2)



STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

Wo	s & Windows orst Case ne 5, 10 ft2)	21.8	-29.1
8x7 G	arage Door	19.5	-22.9
16x7 (arage Door	18.5	-21.0
		-	-
		J	J

-40.6

3 O'hg -68.3 -21.8 -21.8

4 21.8 -23.6 18.5 -20.4

5 21.8 -29.1 18.5 -22.6

DESIGN LOADS					
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)				
	30 PSF (SLEEPING ROOMS)				
	30 PSF (ATTICS WITH STORAGE)				
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)				
ROOF	20 PSF (FLAT OR <4:12)				
	16 PSF (4:12 TO <12:12)				
	12 PSF (12:12 AND GREATER)				

SOFTPLAN

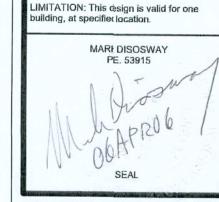
REVISIONS

VINDLOAD ENGINEER: Mark Disosway PE No.53915, POB368, Lake City, FL 32056, 386-754-549 Stated dimensions supercede scaled limensions. Refer al questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E hereby expressly reserve ts common law copyrights and property right these instruments o service. This document not to be reproducel, altered or copied in any

CERTIFICATION: I pereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineer comply with sectionR301.2.1, florida building code residential 2004, to the best of my

form or manner without first the express writte

mission and conent of Mark Disosway.



Patricia McFee Residence

AIDRESS: Lot # 34, Jarnell Hills S/D, Unit 1, Colmbia County, FL 10-3S- 6-02055-034

Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (336) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: April 0i, 2006 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 04 / Apr / 06

JOB NUMBER: 603293 DRAWING NUMBER

OF 3SHEETS

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY. BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

SIMPSON H2.5A U.N.O. -SEE STRUCTURAL PLAN -(2) 2X10 SYP #2 U.N.O. SEE STRUCTURAL PLAN (2) SIMPSON LSTA21w/ (8) -16d TO HEADER AND (8) -16d TO POST -6X6 SYP #2 POST -SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT — SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SCALE: N.T.S. LSTA18 BEAM W/4-16d SEE STRUCTURAL PLAN BEAM MAY BE ATTACHED IN EITHER METHOD SHOWN ABOVE

-- (4)-2x4 SPF #2 NILED

MIN. (SEE STRUTURAL PLAN)

NAILS AT 16" O.

BEAM MID-WALL CONNECTION DETAIL

2X4 SCAB CONT. TOP TO

CHORD@ 8' FROM GABLE -

2X4 SCAB IF VERT, WEB IS

CONT. 2X4X8' #2 SYP LATERAL

2X4 BLOCKING @ 48" OC.

BETWEEN GABLE AND FIRST

NOT PRESENT -

BRACE @ 48" OC. -

TRUSS.

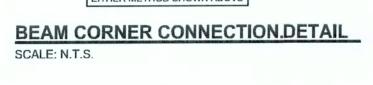
(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

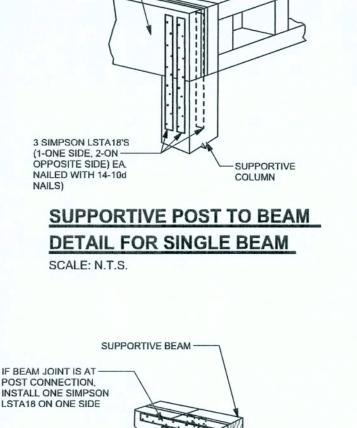
SIMPSON HUS412 MIN.

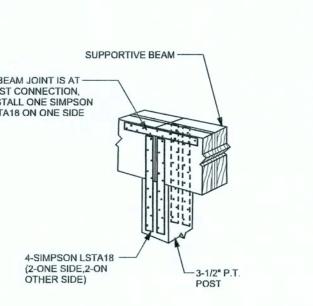
SEE STRUCTURAL PLAN

4 - 10d NAILS OR 4 - .131"x 3.25"

TYPICAL AT ALL CONNECTIONS



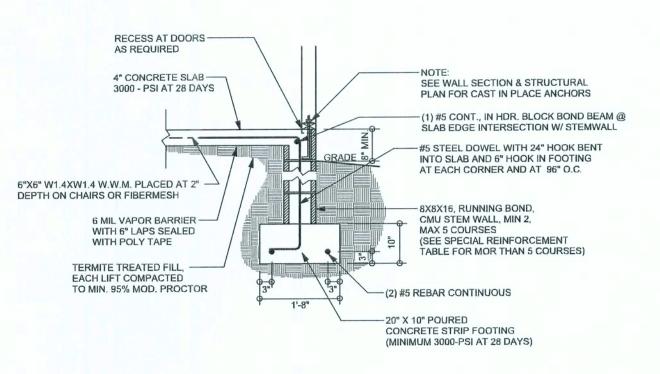




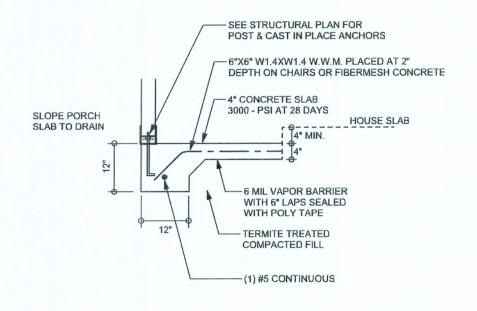
SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

CRIMPPLES IF REQUIRED (4) .1331 x 3 1/4" GUN NAILS TOE : NAILED THRU SILL -INTO) JACK STUD U.N.O. TYPIC AL STRAPPING (U.N.O.) (SEE SSTRUCTURAL PLAN) -SP4 ODR (2) H2.5A OR (2) SSP-ALL OOPENINGS (U.N.O.) (1) 2X6 SPF= #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPFF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPPH, 10'-0" WALL HIGHT U.N.O.)

-LSTA18 (U.N.O. -



F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"

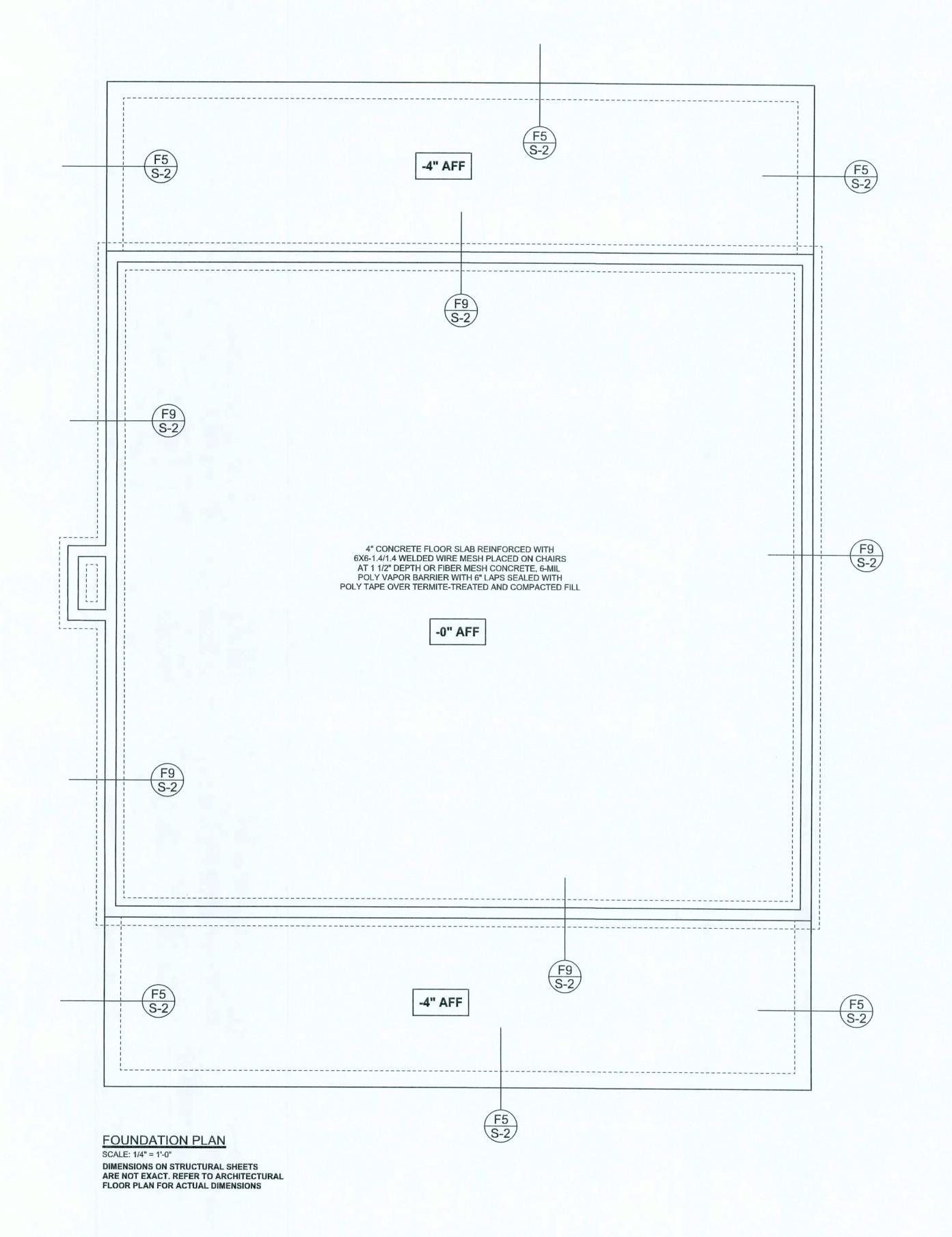


F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

	STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
			#5	#7	#8	#5	#7	#8
	3.3	3.0	96	96	96	96	96	96
	4.0	3.7	96	96	96	96	96	96
	4.7	4.3	88	96	96	96	96	96
	5.3	5.0	56	96	96	96	96	96
	6.0	5.7	40	80	96	80	96	96
	6.7	6.3	32	56	80	56	96	96
	7.3	7.0	24	40	56	40	80	96
	8.0	7.7	16	32	48	32	64	80
	8.7	8.3	8	24	32	24	48	64
	9.3	9.0	8	16	24	16	40	48



SOFTPIAN

REVISIONS

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 86i, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

Stated dimensions sup:rcede scaled dimensions. Refer all questions to Mark Disosway, P.E. fc resolution.
Do not proceed withou clarification.

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permission and consen of Mark Disosway.

CERTIFICATION: I herby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R31.2.1, florida building code residential 2004, b the best of my knowledge.

LIMITATION: This desi_ln is valid for one building, at specified loation.

MARK DSOSWAY
P.E. i3915

Patricia McFee Residence

ADDRESS: Lot # 34, Panell Hills S/D, Unit 1, Columbia County, FL 10-3S-16-02055-034

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PRINTED DATE:
April 06, 2006

DRAWN BY: CHECKED BY:

David Disosway

FINALS DATE: 04 / Apr / 06

> JOB NUMBER: 603293 DRAWINGNUMBER

> > S-2 OF 3 SHEETS

