ERECTION NOTES

- 1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303-16; Section 7.10).
- 2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
- 3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16; Section 7.14).
- 4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
- 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code -Steel or AWS D1.3 Structural Welding Code — Sheet as applicable, for the processes, positions, and materials involved.
- All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency. All documentation and records shall be the responsibility of the customer.
- 7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
- 8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
- Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16, Section 7.15). 10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
- 11. WARNING SOME PANÉLS AND TRIM PARTS ARE FURNISHÉD WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

- 1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
- The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
- 3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
- 4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
- The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing, it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
- 6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3).
- 7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
- 8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
- All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance
- 10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

- 1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
- 2. Oil—canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefor is not a cause for rejection.
- The Metal Building Provider's red—oxide and gray—oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water-holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de-icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
- All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
- 5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
- 6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the
- 7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES Building Code: Florida Building Code 2023 Hot-rolled version: AISC 360-16 AISI S100-16 Cold-formed version:

GENERAL LOADS Dead Load: 3.00 psf Roof Collateral Load: 3.00 psf (Spec) 0.00 psf Sprinkler Load: Roof Live Load:

20.00 psf Tributary Live Load Reduction: Yes Rainfall Intensity: 10.00

WIND LOAD

Wind Speed (3-sec gust) Vult: 121 mph 93 mph Vasd: 75 mph Vserv: В **Exposure Factor:** Wind Condition: Enclosed Internal Pressure Coefficient: +/- 0.18 Edge Zone Width: 8.40 Ft Wind borne debris region: Yes

Ground Snow Load (Pg):

DEFLECTION CRITERIA Main Frames Lateral: Main Frames Vertical: Bearina Frame Rafter:

H/60 Roof Panels: L/180 Purlins: L/180 Wall Panels: L/240 Girts: H/60

0.00 psf

For components, claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

SEISMIC LOAD

Endwall Columns:

Wind Frame (Horz):

Risk Category: II — Normal Seismic Importance Factor (le): 1.0000 Structural Response Acceleration (Ss): 0.0790 Structural Response Acceleration(S1): 0.0480 Site Class: D Design Spectral Response (Sds): 0.0843 0.0768 Design Spectral Response (Sd1): Seismic Design Category: В

Framing Direction: Lateral Longitudinal Structural Syst: 'Structural Steel Systems Not Specifically Detailed for Seismic Resistance' Response Modification Factor: 3.0 Deflection Amplification: 3.0 3.0 0.0281 0.0281 Seismic Response Coeff. Cs Design Base Shear V: 3.75 Kips 3.95 kips

Equivalent Lateral Force

ROOF PANEL

Analysis Procedure:

Profile: Super Seam Plus Gauge: 24 Color: Galvalume plus UL580 Class 90: Yes

Clip Type if Standing Seam: High Float

LEW PANEL Profile: REV.ROLL Super Span X Gauge: 26 Color: Burnished Slate **REW PANEL** Profile: Super Span X Gauge: 26 Color: SMP Light Stone FSW PANEL 26 Color: SMP Light Stone Profile: Super Span X BSW PANEL Profile: Super Span X 26 Color: SMP Light Stone PARAPET BACK PANEL Profile: Super Span X Gauge: 26 Color: Galvalume WALL LINERS Profile: Low Rib X 26 Color: Polar White SOFFIT PANFI Profile: Accent 11R 24 Color: Polar White

Erector Installation" can be considered complete

FOR CONSTRUCTION PERMIT:

FOR ERECTOR INSTALLATION:
Final drawings for constructio

proper interpretation of the project documents. Only drawings issued "F

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition and/or relocation of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building

APPROVAL SPECIFICATIONS

- 1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadinas.
- 2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
- 3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
- 4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
- 5. It is imperative that any changes to these drawings:

Gray Oxide Primer

- Be made in contrasting ink.
- 5.2. Be legible and unambiguous.
- Have all instances of changes clearly indicated.
- 6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
- The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
- 8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
- 9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

PRIMARY FRAMING

Built-Up & Hot-Rolled:

5mit Duration

L/60

L/60

L/240

L/150

5yr return Period

DRAWING SCHEDULE DWG NO. ISSUE DATE

P1 02.02.24 COVER SHEET

02.02.24 ANCHOR BOLT PLAN

P1 02.02.24 RIGID FRAME ELEVATION

P1 02.02.24 RIGID FRAME ELEVATION

P1 02.02.24 ROOF FRAMING PLAN

P1 02.02.24 ROOF SHEETING PLAN

02.02.24 ANCHOR BOLT DETAILS

02.02.24 ANCHOR BOLT REACTIONS

DESCRIPTION

SECONDARY FRAMING	
Purlins, Eave Struts:	Pre-Galvanized
Girts, Light Gage Columns:	Pre-Galvanized
Light Gage Jambs & Headers:	Pre-Galvanized
Base Channel Finish:	Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification. Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-

ti	ing G—90 specification.		E3	P1	02.02.24	ENDWALL FRAME & SHEETING ELEVATION
			E4	P1	02.02.24	ENDWALL FRAME & SHEETING ELEVATION
	TRIM COLOR:		E5	P1	02.02.24	SIDEWALL FRAME & SHEETING ELEVATION
		AUGE: 26	E6	P1	02.02.24	SIDEWALL FRAME & SHEETING ELEVATION
	SHADOW GOTTER. SMP Light Stone G.	AUGE: 26	E7	P1	02.02.24	WALL LINER SHEETING
	CORNER: SMP Light Stone G	AUGE: 26	E8	P1	02.02.24	SECTIONS DETAILS
	ACCESSORY: SMP Light Stone G	AUGE: 26	E9	P1	02.02.24	SECTIONS DETAILS
	DOWNSPOUT: <u>SMP Light Stone</u> G	AUGE: <u>26</u>	E10	P1	02.02.24	SECTIONS DETAILS
		AUGE: 26	D1	P1	02.02.24	STANDARD DETAIL PAGE
	LEW PARAPET CAP TRIM: Burnished Slate G.	AUGE: <u>26</u>	D2	P1	02.02.24	STANDARD DETAIL PAGE
	FSW PARAPET CAP TRIM: <u>SMP Light Stone</u> G	AUGE: 26			•	

Additional notes

LINER TRIM: Polar White

SOFFIT TRIM: Polar White

Roof and wall panels have been designed in accordance with section 2222.4 of the Florida Building Code, Eighth Edition (2020). Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B-72:

1. Super Seam Plus Roof Panel 17700.4 24 ga. Super Seam Plus Roof Panel over open framing HVHZ: No

2. Super Span X Wall Panel 17702.4 26 ga. Super Span X Wall Panel over open framing HVHZ: No

GAUGE: 26

GAUGE: 24

Roof top units are to be supported by Beams.

The framing provided by the manufacturer has been designed with roof top units at the following locations. The dimensions shown are to the center of the unit. Purlins are allowed to be cut for installation and support of RTU as shown in plan.

1800 lbs RTU 1 located at 35.3854 ft from Grid A and 38.000 ft from grid line 6.1. 1800 lbs RTU 2 located at 91.3854 ft from Grid A and 38.000 ft from grid line 6.1. 900 lbs RTU 3 located at 127.4662 ft from Grid A and 36.9349 ft from grid line 6.1.

The members provided by the metal building manufacturer above the masonry wall along line 6 are designed to support the wall and to deflect less than L/240. Flush girts behind masonry wall are not designed to support masonry wall. 60 psf wall weight has been considered in the seismic mass calculation.

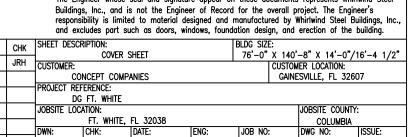
Other Loads:

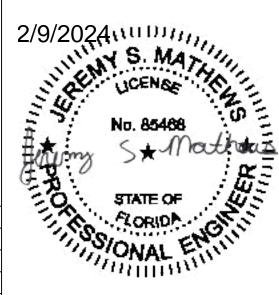
- 1. Support for 1400 Lb sign board at grid 6.1 between grids B & C 2. 1800 lbs RTU-1
- 3. 1800 lbs RTU-2
- 4. 900 lbs RTU-3

The Engineer whose seal and signature appear on these documents represents Whirlwind Stee Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings and excludes part such as doors, windows, foundation design, and erection of the building.

MAB 12240-33801

C1





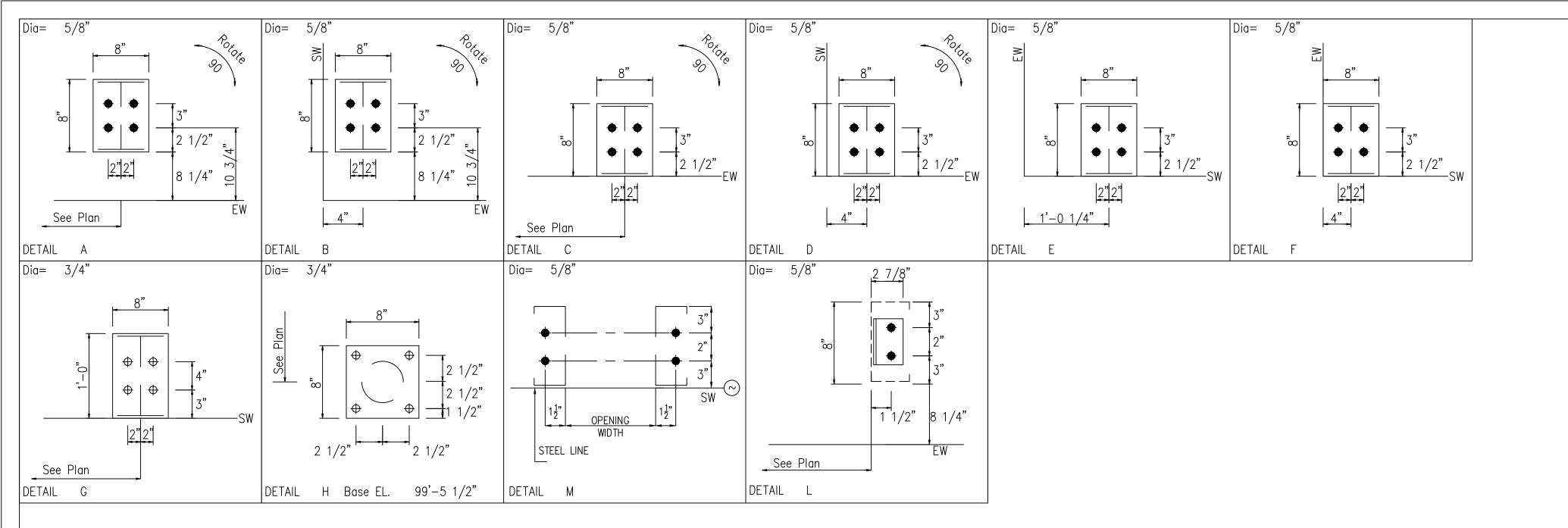
FOR APPROVAL:
These drawings, being for approval, are by definition not final and are ISSUE | DATE DESCRIPTION BY CHK PND JRH 02.02.24 FOR CONSTRUCTION PERMIT for conceptual representation only. Their purpose is to confirm the WHIRLWIND) STEEL BUILDINGS

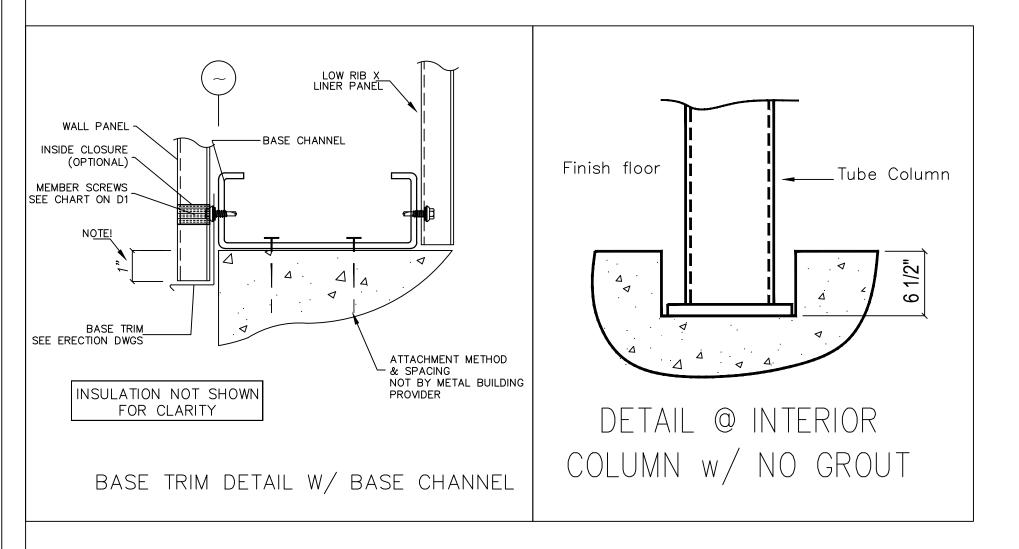
se drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. © 2005 Whirlwind Steel Buildings Inc.

PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600

All rights reserved

02.02.24





The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc. and excludes part such as doors, windows, foundation design, and erection of the building.

FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the for conceptual representation only. Iheir purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.

FOR CONSTRUCTION PERMIT:

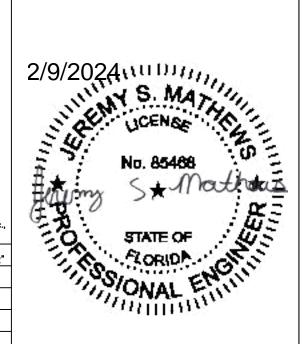
These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

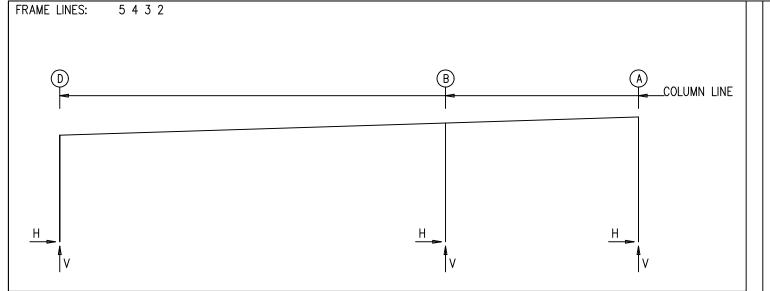
FOR ERECTOR INSTALLATION:

Final drawings for construction.

WHIRLWIND STEEL BUILDINGS	
P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600	
© 2005 Whirlwind Steel Buildings Inc.	ŀ

					ana exc	iuaes part su	icn as aoors, wir	idows, to	unaatio	on aesign, ana	erection of the	building.
SSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR					DG SIZE:		
						ANCHOR	BOLT DETAILS		7	76'-0" X 140'	-8" X 14'-0"/ [·]	16'–4 '
0	02.02.24	FOR ERECTOR INSTALLATION	PND	JRH	CUSTOMER:					CUSTOM	ER LOCATION:	
					COI	NCEPT COMPA	anies			GAINE	SVILLE, FL 3260	07
-					PROJECT REF	ERENCE:						
					DG	FT. WHITE						
					JOBSITE LOCA	ATION:					JOBSITE COUNTY	:
						FT. WHITE,	FL 32038				COLUMBIA	
					DWN:	CHK:	DATE:	ENG:	J	IOB NO:	DWG NO:	ISSUE:
					PND	PNR	02.02.24	I м	IAB 1	12240-33801	F2	(





RIGID	FRAME:		MAXIMUM	REACTION	IS, ANC	HOR BOLT	S, & BAS	E PLAT	ES				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id) Hmin H	V Vmin	Bol ⁻ Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
5*	D	3 8	3.1 2.3	6.8 11.5	5 4	-3.1 -2.8	-2.8 -5.7	4	0.750	8.000	12.00	0.375	0.0
5*	Α	6 9	3.5 -0.9	-0.8 4.6	2 8	-3.1 -2.3	0.9 -2.9	4	0.750	8.000	12.00	0.375	0.0
5*	В	4 1	0.0 0.0	-14.1 30.2	4	0.0	-14.1	4	0.750	8.000	8.000	0.500	-6.5
5*	Frame lin	ies:	5 2										

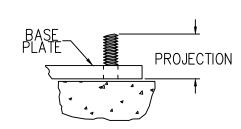
				ımn_Reac	tions(k)								
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length -	Thick	Ele [,] (in
4*	D	3 10	3.1 2.3	6.8 11.5	5 7	-3.1 0.3	-2.8 -7.8	4	0.750	8.000	12.00	0.375	0.
4*	Α	6 11	3.5 -0.9	-0.8 4.6	2 7	-3.1 -0.1	0.8 -3.4	4	0.750	8.000	12.00	0.375	0.
4*	В	4 1	0.0 0.0	-14.1 30.2	4	0.0	-14.1	4	0.750	8.000	8.000	0.500	-6

RIGII	D FRAN	лЕ:	BAS	IC COLUM	N REACT	IONS (k)				
Frame Line 5* 5* 5*	e Column Line D A B	Horiz 0.5 -0.5 0.0	-Dead Vert 2.4 0.5 5.9	Collo Horiz 0.4 -0.4 0.0	teral- Vert 1.9 -0.2 6.4	Horiz 1.4 -1.4 0.0	Live Vert 7.0 1.0 17.9	Wind Horiz -5.1 -2.7 0.0	I_Left1- Vert -12.0 -0.5 -29.3	-Wind_Right1- Horiz Vert 2.6 -6.3 6.0 -2.7 0.0 -17.4	Wind_Left2- Horiz Vert -5.7 -7.1 -2.1 0.4 0.0 -17.8
Frame Line 5* 5* 5*	e Column Line D A B	-Wind_ Horiz 2.3 6.3 0.0	Right2- Vert -1.6 -1.8 -5.8	Wind Horiz 0.0 0.4 0.0	_Long1- Vert -11.5 -2.0 -28.3	W Horiz 0.7 -0.8 0.0	find_Long2— Vert —7.4 —1.3 —17.7	-Seism Horiz -0.2 -0.3 0.0	ic_Left Vert -0.1 0.1 -0.1	Seismic_Right Horiz Vert 0.2 0.1 0.3 -0.1 0.0 0.1	F1PAT_LL_1- Horiz Vert 1.4 7.1 -1.4 -3.3 0.0 13.4
Frame Line 5* 5* 5*	Column Line D A B	F1PAT_ Horiz 0.0 0.0 0.0	LL_2- Vert -0.1 4.2 4.5								
Frame Line 4* 4* 4*	e Column Line D A B	Horiz 0.5 -0.5 0.0	-Dead Vert 2.4 0.5 5.9	Collo Horiz 0.4 -0.4 0.0	teral- Vert 2.0 -0.2 6.5	 Horiz 1.4 -1.4 0.0	Live Vert 7.0 1.0 17.9	Wind Horiz -5.1 -2.6 0.0	I_Left1- Vert -12.0 -0.5 -29.3	-Wind_Right1- Horiz Vert 2.6 -6.4 6.0 -2.7 0.0 -17.4	Wind_Left2- Horiz Vert -5.7 -7.1 -2.0 0.4 0.0 -17.8
Frame Line 4* 4* 4*	e Column Line D A B	-Wind_ Horiz 2.3 6.3 0.0	Right2- Vert -1.6 -1.8 -5.8	Wind Horiz 0.0 0.4 0.0	_Long1- Vert -15.4 -6.3 -28.3	W Horiz 0.7 -0.8 0.0	find_Long2— Vert —11.3 —5.6 —17.7	-Seism Horiz -0.2 -0.3 0.0	ic_Left Vert -0.1 0.1 -0.1	Seismic_Right Horiz Vert 0.2 0.1 0.3 -0.1 0.0 0.1	-Seismic_Long Horiz Vert 0.0 -0.9 0.0 -1.0 0.0 0.0
Frame Line 4* 4* 4*	Column Line D A B	F2PAT_ Horiz 1.4 -1.4 0.0	LL_1- Vert 7.1 -3.3 13.4	F2PAT_ Horiz 0.0 0.0 0.0	LL_2- Vert -0.1 4.2 4.5						
5* 4*	Frame lin		5 2 4 3								

GENERAL NOTES

- 1. All anchor bolts (by others) to have nuts and flat washers.
- 2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- 3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
- 4. The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

ENDV	VALL	COLUI	MN:	BASIC COL	UMN REACTIONS (k)				
Frm Line 6.1 6.1 6.1 6.1	Col Line D C B A	Dead Vert 0.7 1.7 1.7 0.8	Collat Vert 0.5 2.0 2.0 0.5	Live Vert 2.6 6.8 6.8 2.6	Wind_Left1 Horz Vert -2.7 -4.6 0.0 -6.9 0.0 -8.3 0.0 -3.0	Wind_Right1 Horz Vert 0.0 0.2 3.6 -6.8 0.0 -4.8 0.0 -2.0	Wind_Left2 Horz Vert -2.4 -3.4 0.0 -4.4 0.0 -5.6 0.0 -2.1	Wind_Right2 Horz Vert 0.0 1.1 3.5 -4.0 0.0 -2.1 0.0 -1.0	Wind Press Horz 0.9 -1.4 -2.0 0.3	Wind Suct Horz -0.5 2.2 2.6 -0.2
Frm Line 6.1 6.1 6.1 6.1	Col Line D C B A	Wind_L Horz 0.0 0.5 0.0	ong1 Vert -2.9 -8.5 -8.2 -3.1	Wind_Long2 Horz Vert 0.0 -1.8 0.2 -4.9 0.0 -4.8 0.0 -1.8	Seis_Left Horz Vert -1.2 -0.6 0.0 0.6 0.0 0.0 0.0 0.0	Seis_Right Horz Vert 0.0 0.7 1.2 -0.6 0.0 0.0 0.0 0.0	Horz H 0.0 0 0.3 0 0.3 0	lorz Vert 0.0 2.5 0.0 7.5 0.0 2.9	E1PAT_LL_2- Horz Vert 0.0 -0.2 0.0 2.9 0.0 7.5 0.0 2.5	
Frm Line 6.1 6.1 6.1 6.1	Col Line D C B A	E1PAT_ Horz 0.0 0.0 0.0 0.0	LL_3- Vert 2.9 3.4 3.4 2.9	E1PAT_LL_4- Horz Vert 0.0 -0.3 0.0 3.5 0.0 3.5 0.0 -0.3						
Frm Line 1 1 1	Col Line A B C D	Dead Vert 0.8 1.6 1.6	Collat Vert 0.4 1.5 1.5	Live Vert 2.5 6.6 6.6 2.5	Wind_Left1 Horz Vert 0.0 -1.9 -3.4 -6.4 0.0 -2.6 0.0 -1.8	Wind_Right1 Horz Vert 0.0 -2.9 0.0 -6.5 2.6 -9.3 0.0 -3.0	Wind_Left2 Horz Vert 0.0 -0.9 -3.3 -3.8 0.0 -0.1 0.0 -0.8	Wind_Right2 Horz Vert 0.0 -2.0 0.0 -4.1 2.3 -6.6 0.0 -2.0	Wind Press Horz 0.3 -2.8 -2.7 0.0	Wind Suct Horz -0.2 3.1 2.9 0.0
Frm Line 1 1 1	Col Line A B C D	Wind_L Horz 0.0 -0.5 0.0 0.0	ong1 Vert -3.0 -8.1 -7.6 -3.0	Wind_Long2 Horz Vert 0.0 -1.7 -0.2 -4.7 0.0 -4.5 0.0 -1.8	Seis_Left Horz Vert 0.0 0.0 -0.3 -0.2 0.0 0.2 0.0 0.0	Seis_Right Horz Vert 0.0 0.0 0.0 0.2 0.3 -0.2 0.0 0.0	Vert H 0.0 0 0.0 0 0.0 0	lorz Vert 0.0 2.4 0.0 7.2 0.0 2.8	E2PAT_LL_2- Horz Vert 0.0 -0.2 0.0 2.8 0.0 7.2 0.0 2.4	
Frm Line 1 1 1 1	Col Line A B C D	E2PAT_ Horz 0.0 0.0 0.0 0.0	LL_3- Vert 2.8 3.2 3.2 2.8	E2PAT_LL_4- Horz Vert 0.0 -0.3 0.0 3.4 0.0 3.3 0.0 -0.3						

Frm	Col	 Load	——— Col	umn_Reac V	tions(k) Load) Hmin		Rol	t(in)	Rasi	e_Plate(in)		Elev.
Line	Line	Id Id	H —	Vmax	Id	H	Vmin 	Qty	Dia	Width	Length	Thick	(in)
6.1	D	12 14	0.5 0.0	-1.3 4.1	13	-0.3	-2.3	4	0.625	8.000	8.000	0.375	0.0
6.1	С	15 16	1.3 0.0	-4.1 11.1	12 15	-0.9 1.3	-4.1 -4.1	4	0.625	8.000	8.000	0.375	0.0
6.1	В	13 17	1.6 0.0	-3.9 11.2	12 13	-1.2 1.6	-3.9 -3.9	4	0.625	8.000	8.000	0.375	0.0
6.1	Α	12 14	0.2 0.0	-1.4 4.2	15 12	-0.1 0.2	-1.4 -1.4	4	0.625	8.000	8.000	0.375	0.0
1	Α	12 18	0.2 0.0	-1.3 4.0	15 12	-0.1 0.2	-1.3 -1.3	4	0.625	8.000	8.000	0.375	0.0
1	В	15 19	1.8 0.0	-3.9 10.3	12 15	-1.7 1.8	-3.9 -3.9	4	0.625	8.000	8.000	0.375	0.0
1	С	20 21	1.7 0.0	-4.6 10.3	12 20	-1.6 1.7	-3.6 -4.6	4	0.625	8.000	8.000	0.375	0.0
1	D	7 18	0.0 0.0	-1.4 3.9	7	0.0	-1.4	4	0.625	8.000	8.000	0.375	0.0

MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

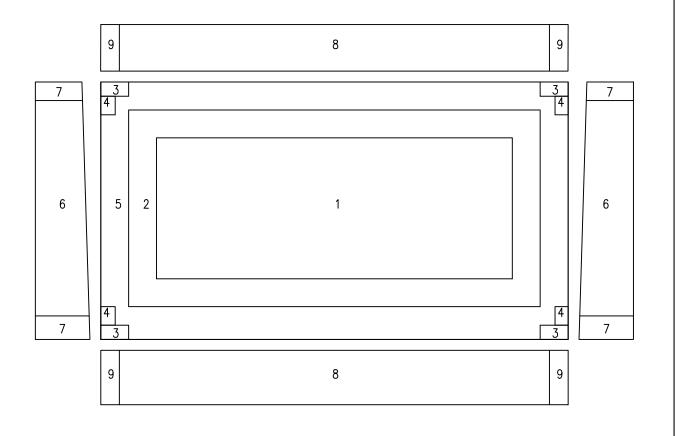
ANCHO	OR BOLT S	SUMMA	ARY (GI	RADE 3	6)
Qty	Locate	Dia (in)	Туре	Proj (in)	
 → 32 → 32 → 12 → 16 	Endwall Frame Jamb Int.Column	5/8" 3/4" 5/8" 3/4"	F1554 F1554 F1554 F1554	2.50 3.00 2.50 3.00	

FOR ERECTOR INSTALLATION:
Final drawings for construction

ENDWALL COLUMN:

BUIL	DING	BRACI	NG RE	ACTIO	DNS			
 wa	- II	- Col		Reacti	ons(k) - Sei	smic -	Panel_ - (lb/	
Loc	Line	Line	Horz "	Vert	Horz	Vert	Wind	Seis
L_EW F_SW R_EW B_SW	6.1 A 1 D	D,C 4,3 B,C 3,4	3.6 8.2 3.4 9.2	1.9 4.2 1.9 4.0	1.2 2.0 0.3 2.0	0.6 1.0 0.2 0.8		
Reactio	ons for	seismic	represent	shear t	orce, Eh			

NOTE: As Per AISI S-100-16 Appendix A Section I6.3.1a, the wind uplift loading in edge and corner zones is permitted to be multiplied by a factor of 0.67 for the standing seam roof panel used on this building. This reduction factor is not expressed in the ultimate wind uplift values tabulated on this page.



Components & Cladding

			5 5 p 5 5 t 5	50 0.mmm.g		
Zone	Width (ft)	Length (ft)	Pressure(psf Member) Panel	Suction(psf Member) Panel
1 2 3 4 5 6 7 8	8.40 2.80 5.60 8.40 5.60	8.40 8.40 2.80 8.40	16.00 16.00 16.00 16.00 16.00 16.00 16.00	16.00 16.00 16.00 16.00 16.00 19.61 19.61 19.60	-16.88 -24.48 -36.29 -36.29 -32.58 -17.49 -18.71 -17.50 -18.73	-19.61 -34.12 -61.38 -61.38 -45.10 -21.25 -26.14 -21.20 -26.08
				ards surfac ay from sur		

NOTES FOR REACTIONS

- 1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- 2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- 3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- 4. Loading conditions are:
 - Dead+Collateral+Live Dead+Collateral+0.75Live+0.45Wind_Left1 Dead+Collateral+0.75Live+0.45Wind_Right1
- 0.6Dead+0.6Wind_Left1 0.6Dead+0.6Wind_Left2 0.6Dead+0.6Wind_Right2

- 0.6Dead+0.6Wind_Long1L Dead+Collateral+F1PAT_LL_1 Dead+Collateral+F1PAT_LL_2 Dead+Collateral+F2PAT_LL_1
- Dead+Collateral+F2PAT_LL_2
- 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
- Dead+Collateral+E1PAT_LL_3
 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
- 16 Dead+Collateral+E1PAT_LL_1
 17 Dead+Collateral+E1PAT_LL_2
- 18 Dead+Collateral+E2PAT_LL_3
- Dead+Collateral+E2PAT_LL_1
- 20 0.6Dead+0.6Wind_Right1+0.6Wind_Suction 21 Dead+Collateral+E2PAT_LL_2

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc

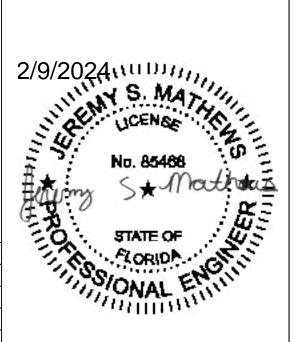
FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the

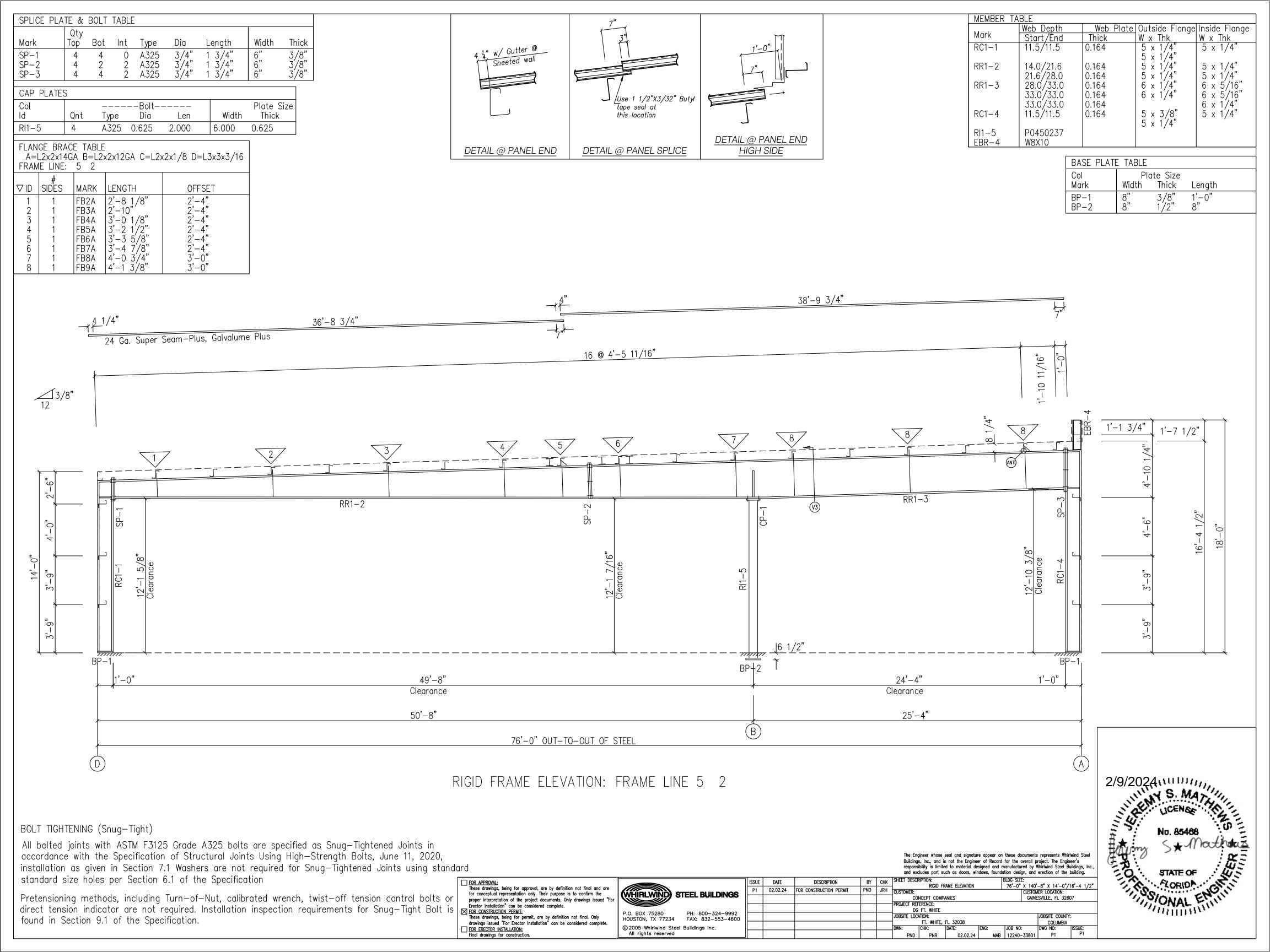
proper interpretation of the project documents. Only drawings issued "F Erector Installation" can be considered complete. FOR CONSTRUCTION PERMIT:

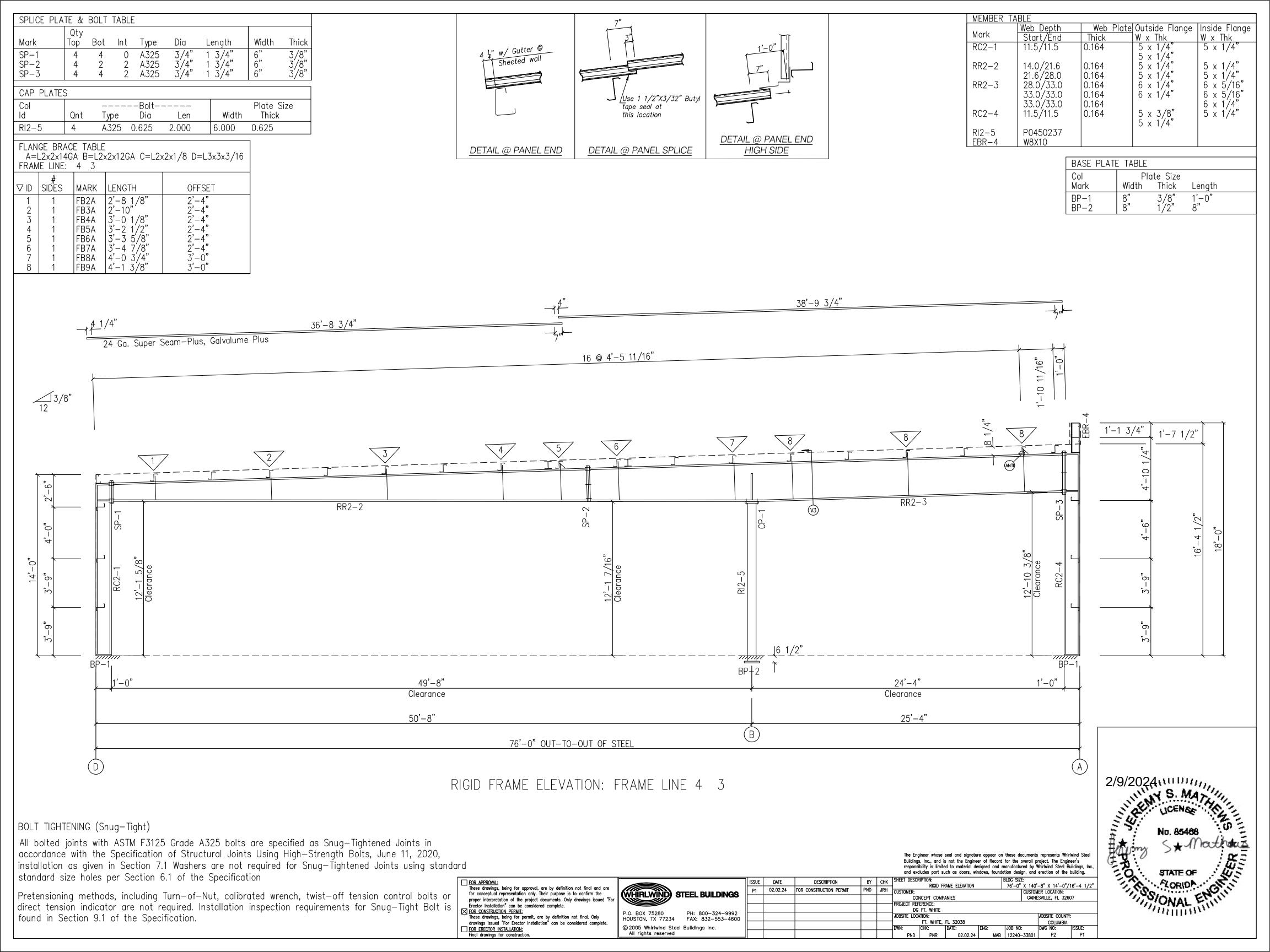
These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. P.O. BOX 75280 HOUSTON, TX 77234 © 2005 Whirlwind Steel All rights reserved

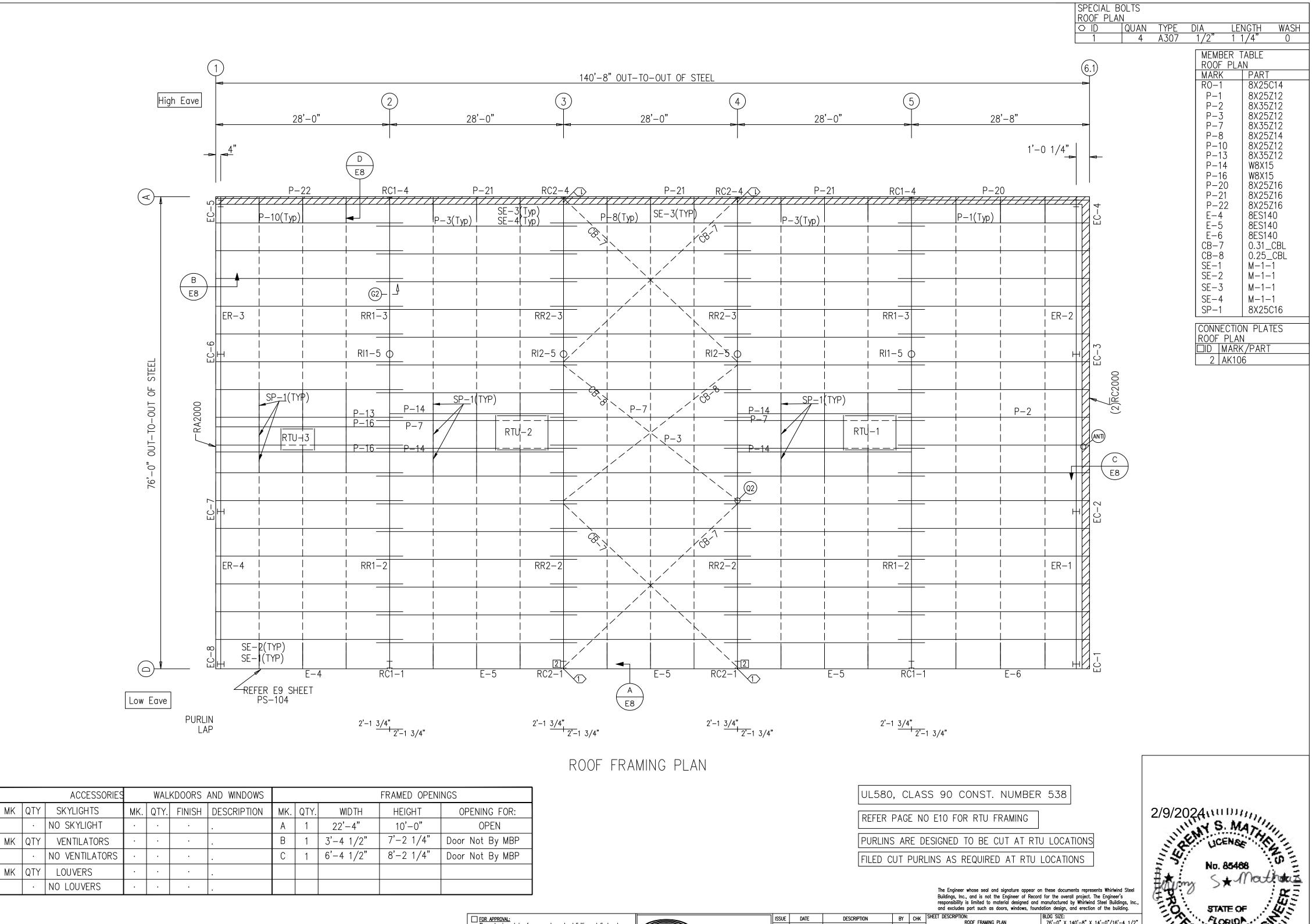


								ich as doors, wir				
	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DES		BOLT REACTIONS		BLDG SIZE:	'-8" X 14'-0"/	′16'_4_1/2"
TEEL BUILDINGS	0	02.02.24	FOR ERECTOR INSTALLATION	PND	JRH	CUSTOMER:		DOLI NEACTIONS			MER LOCATION:	10 -4 1/2
						CONCEPT COMPANIES			GAIN	AINESVILLE, FL 32607		
				1		PROJECT R	EFERENCE:					
PH: 800-324-9992					ļ		G FT. WHITE					
FAX: 832-553-4600						JOBSITE LO					JOBSITE COUNT	Y:
						FT. WHITE, FL 32038 COLUMBIA						
l Buildings Inc.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
						PND	PNR	02.02.24	MAB	12240-33801	F3	0









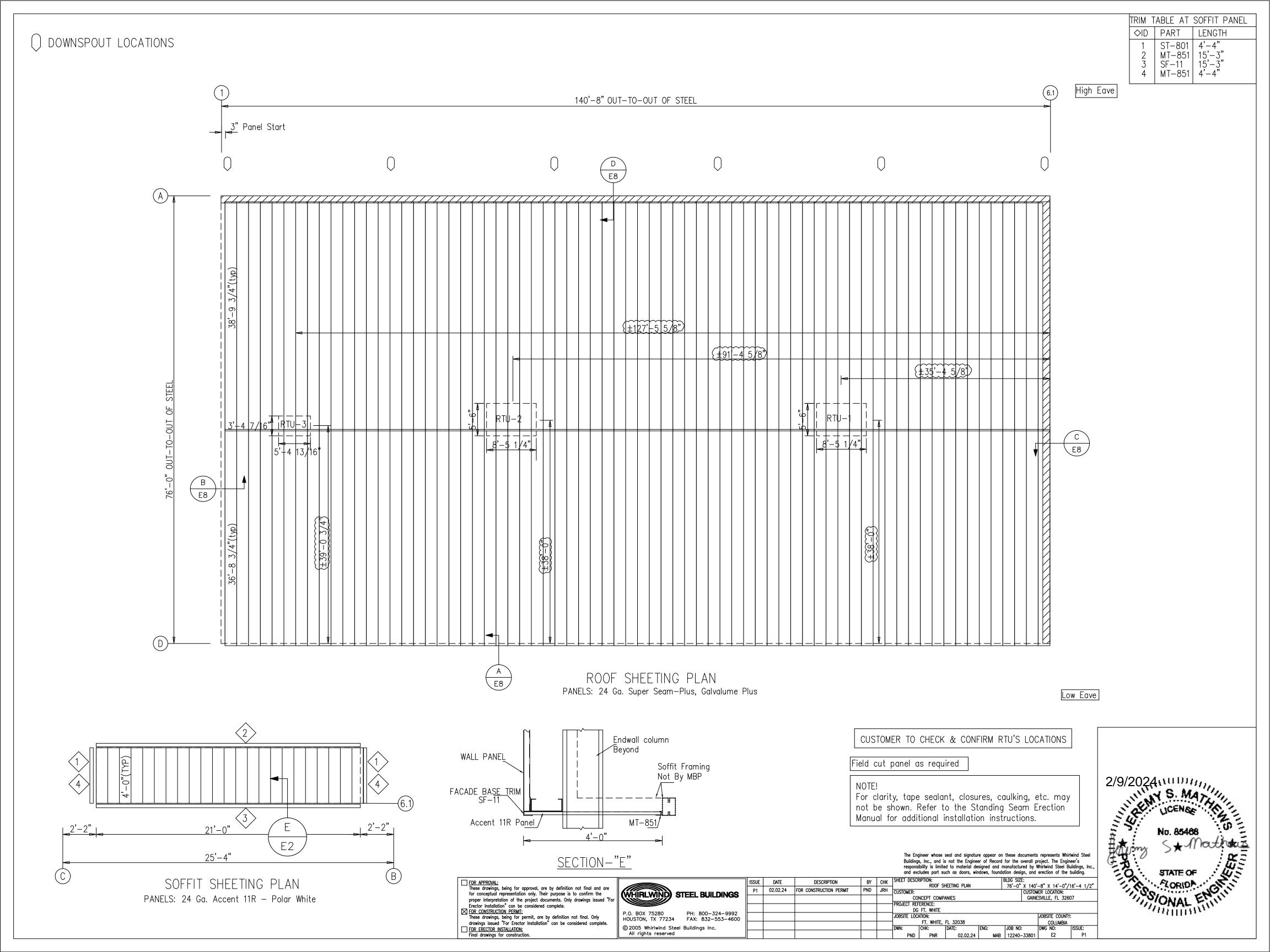
FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete. These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. FOR ERECTOR INSTALLATION:
Final drawings for construction

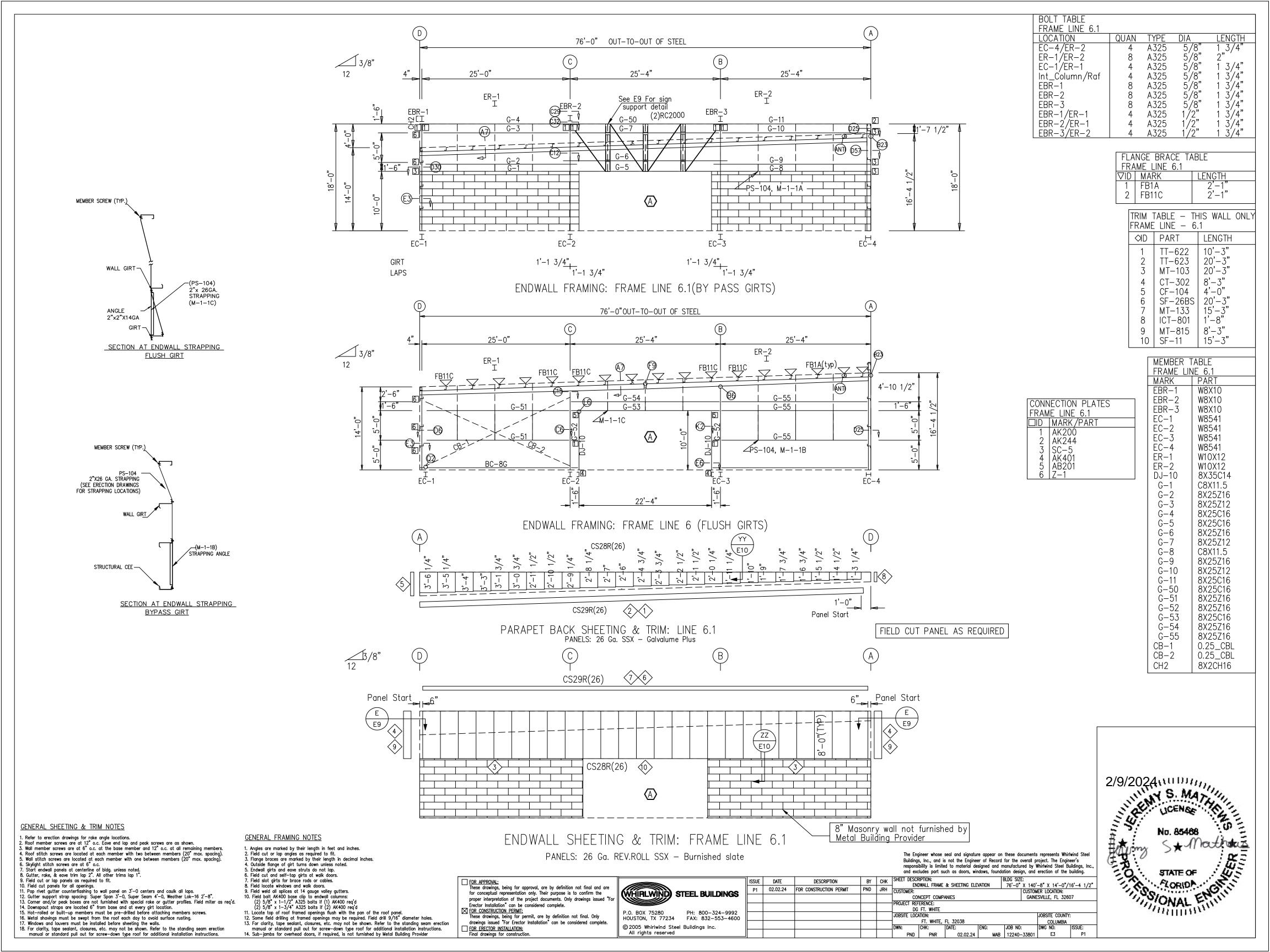
WHIRLWIND STEEL BUILDINGS P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600 © 2005 Whirlwind Steel Buildings Inc. All rights reserved

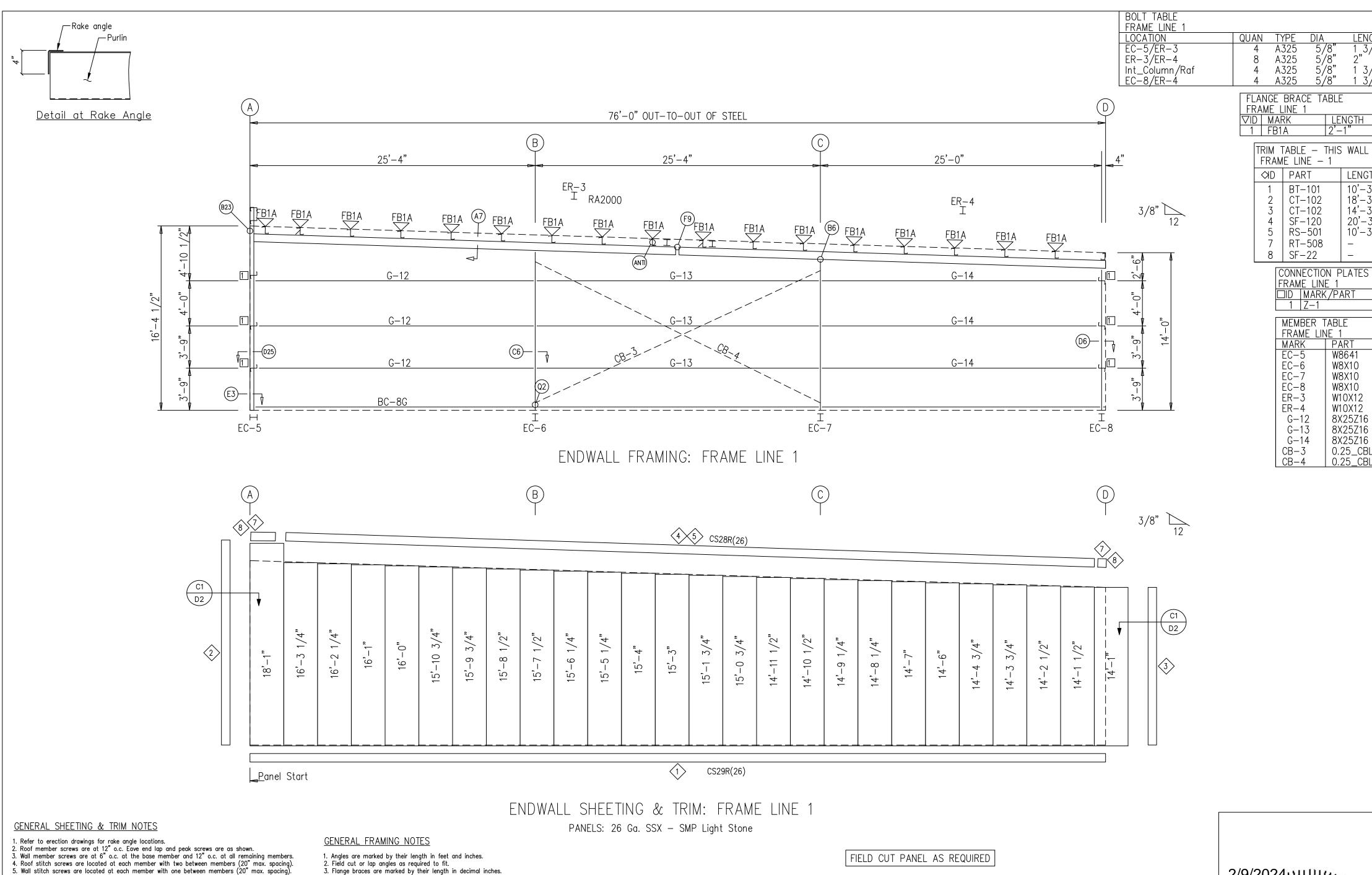
PND JRH CUSTOMER: P1 02.02.24 FOR CONSTRUCTION PERMIT

ROOF FRAMING PLAN 76'-0" X 140'-8" X 14'-0"/16'-4 1/2" CUSTOMER LOCATION: GAINESVILLE, FL 32607 CONCEPT COMPANIES PROJECT REFERENCE: DG FT. WHITE JOBSITE LOCATION: FT. WHITE, FL 32038 JOBSITE COUNTY: COLUMBIA DWG NO: JOB NO: ISSUE: PND PNR 02.02.24 MAB 12240-33801









- 6. Skylight stitch screws are at 6" o.c.
- 7. Start endwall panels at centerline of bldg. unless noted.
 8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
- 9. Field cut or lap panels as required to fit. 10. Field cut panels for all openings.
- 11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
 12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
- 13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as reg'd. 14. Downspout straps are located 6" from base and at every girt location.
- 15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- 16. Metal shavings must be swept from the roof each day to avoid surface rusting.
- 17. Windows and louvers must be installed before sheeting the walls.
- 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
- 4. Outside flange of girt turns down unless noted. 5. Endwall girts and eave struts do not lap.
- 6. Field cut and self-tap girts at walk doors.
- 7. Field slot girts for brace rods or cables. 8. Field locate windows and walk doors.
- 9. Field weld all splices at 14 gauge valley gutters.
 10. Field bolt AK400 base clip to endwall columns:
- (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
- 11. Locate top of roof framed openings flush with the pan of the roof panel.

 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
- 13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
- 14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "Fo

For Construction of the project documents. Sing aroungs issued.

For Construction PERMIT:
These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. FOR ERECTOR INSTALLATION:



	ISSUE	DATE	DESCRIPTION
WHIRLWIND) STEEL BUILDINGS	P1	02.02.24	FOR CONSTRUCTION PERMIT
WHIREWIND STEEL BOILDINGS			
P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600			
© 2005 Whirlwind Steel Buildings Inc.			
All rights reserved			

USE TAMPER RESIST FASTENER UPTO 8'-0" HT

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc. and excludes part such as doors, windows, foundation design, and erection of the building.

BY CHK SHEET DESCRIPTION: ENDWALL FRAME & SHEETING ELEVATION 76'-0" X 140'-8" X 14'-0"/16'-4 1/2" PND JRH CUSTOMER: GAINESVILLE, FL 32607 CONCEPT COMPANIES PROJECT REFERENCE: DG FT. WHITE JOBSITE LOCATION: JOBSITE COUNTY: COLUMBIA DWG NO: FT. WHITE, FL 32038

PND PNR 02.02.24 MAB 12240-33801

JOB NO:

E4

FLANGE BRACE TABLE
FRAME LINE 1

▼ID | MARK | LEN LENGTH 2'-1"

5/8" 5/8" 5/8" 5/8"

LENGTH 1 3/4" 2"

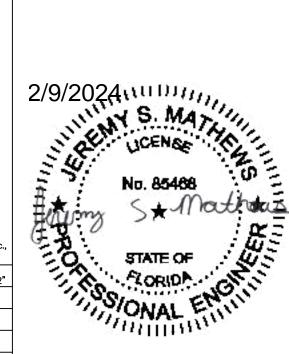
1 3/4" 1 3/4"

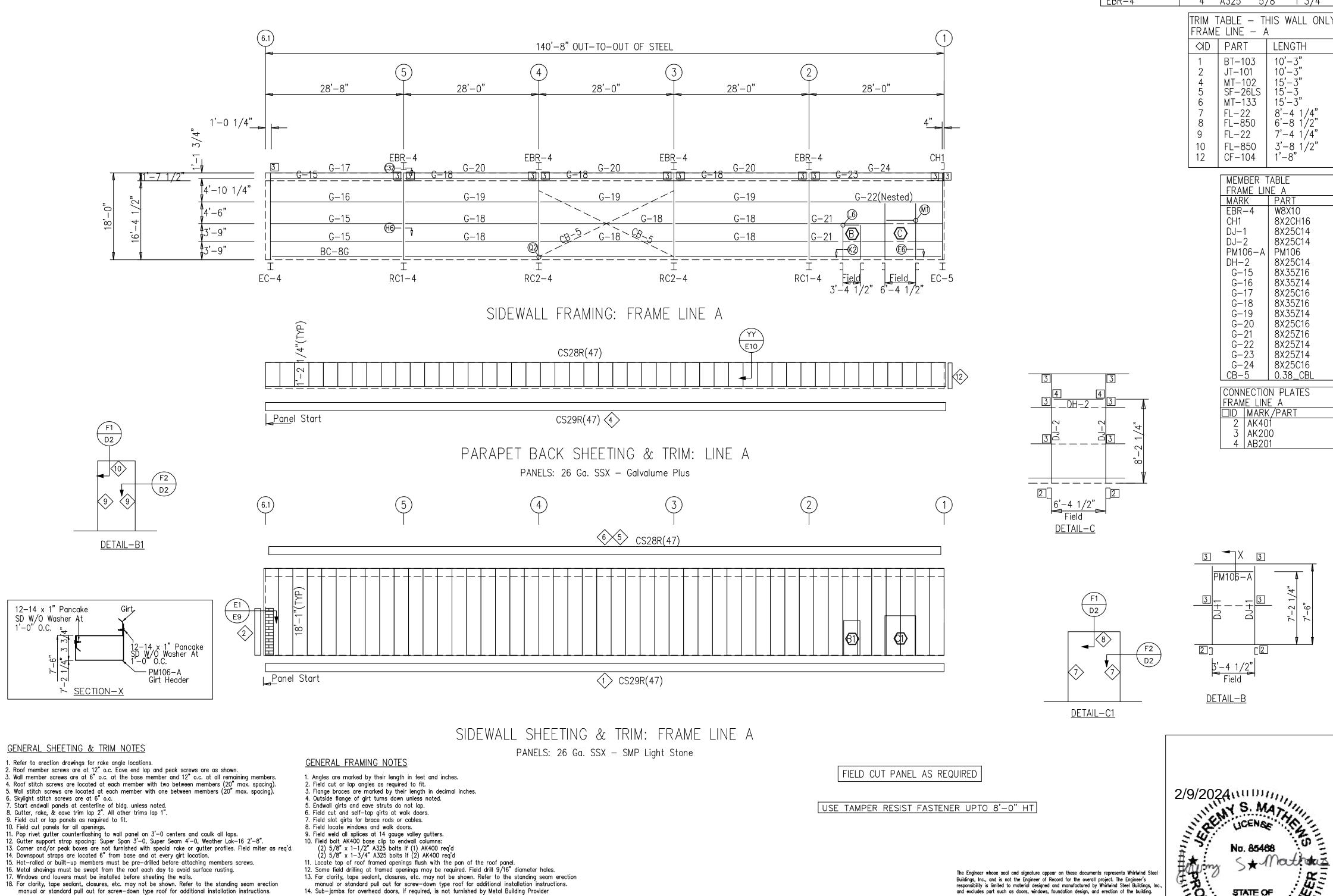
l	TABLE – THIS E LINE – 1	WALL ONL
⇔D	PART	LENGTH
1 2 3 4 5 7 8	BT-101 CT-102 CT-102 SF-120 RS-501 RT-508 SF-22	10'-3" 18'-3" 14'-3" 20'-3" 10'-3"

CONNECTION PLATES
FRAME LINE 1

DID | MARK/PART
1 | Z-1

MEMBER 7	TABLE
FRAME LIN	NE 1
MARK	PART
EC-5	W8641
EC-6	W8X10
EC-7	W8X10
EC-8	W8X10
ER-3	W10X12
ER-4	W10X12
G-12	8X25Z16
G-13	8X25Z16
G-14	8X25Z16
CB-3	0.25 CBL
CB-4	0.25 CBI





FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the

Erector Installation" can be considered complete.

FOR CONSTRUCTION PERMIT:

FOR ERECTOR INSTALLATION:
Final drawings for construction

proper interpretation of the project documents. Only drawings issued "Fo

These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

BOLT TABLE FRAME LINE A LOCATION EBR-4 QUAN TYPE DIA 4 A325 5/8"

MEMBER T	
MARK	PART
EBR-4	W8X10
CH1	8X2CH16
DJ-1	8X25C14
DJ-2	8X25C14
PM106-A	PM106
DH-2	8X25C14
G-15	8X35Z16
G-16 G-17	8X35Z14 8X25C16
G-17	8X35Z16
G-19	8X35714
$G = \frac{13}{20}$	8X25C16
G-21	8X25Z16
G-22	8X25Z14
G-23	8X25Z14
G-24	8X25C16
CB-5	0.38_CBL

No. 85488

No. 85488

State of

Corion

Storion

DESCRIPTION:
SIDEWALL FRAME & SHEETING ELEVATION
BLDG SIZE:
76'-0" X 140'-8" X 14'-0"/16'-4 1/2"

02.02.24 MAB 12240-33801

JOB NO:

CUSTOMER LOCATION: GAINESVILLE, FL 32607

JOBSITE COUNTY:

COLUMBIA DWG NO:

____E5

ISSUE:

SHEET DESCRIPTION:

PROJECT REFERENCE:

JOBSITE LOCATION:

CONCEPT COMPANIES

FT. WHITE, FL 32038

DG FT. WHITE

PND JRH CUSTOMER

DATE

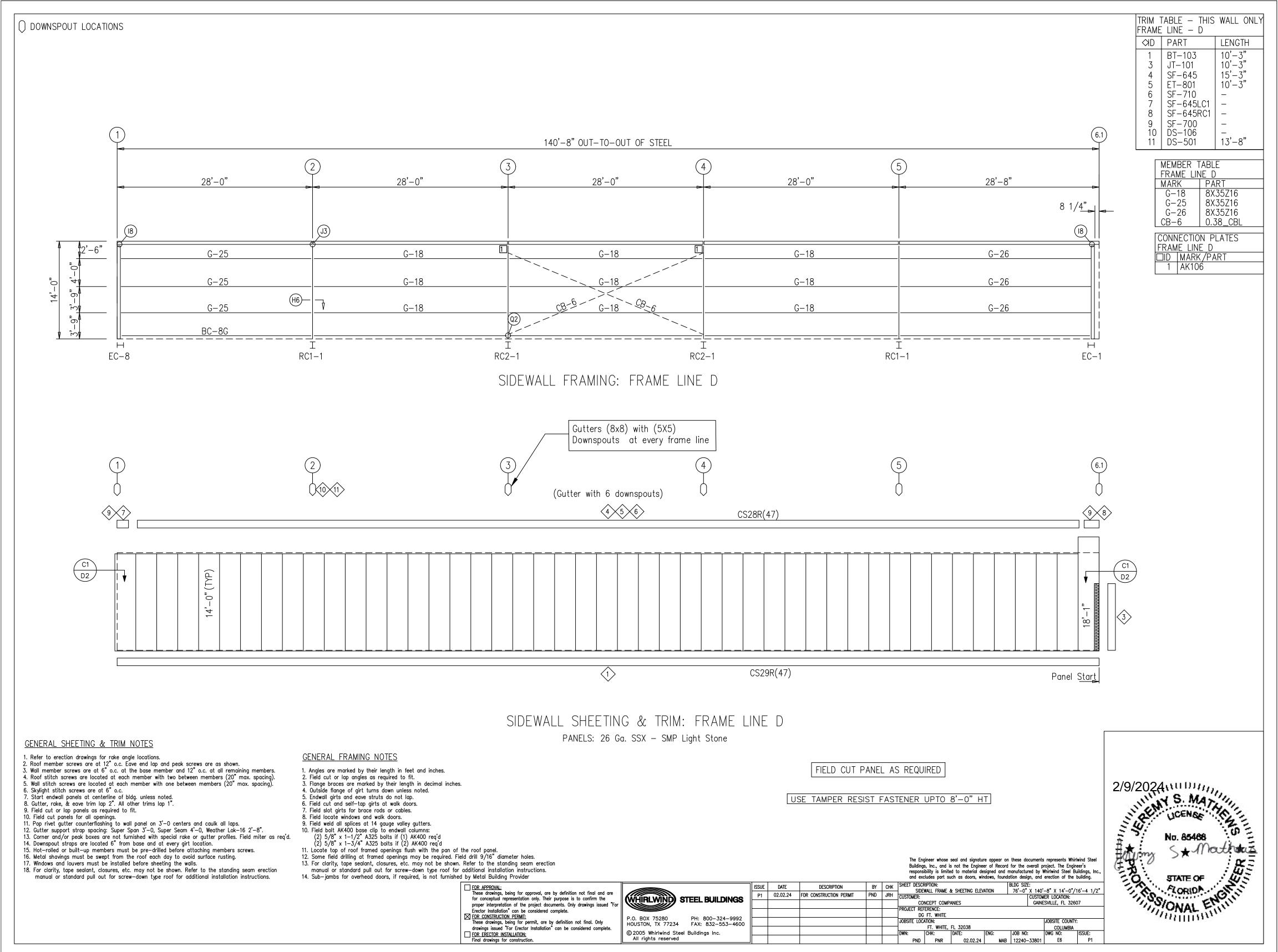
WHIRLWIND STEEL BUILDINGS

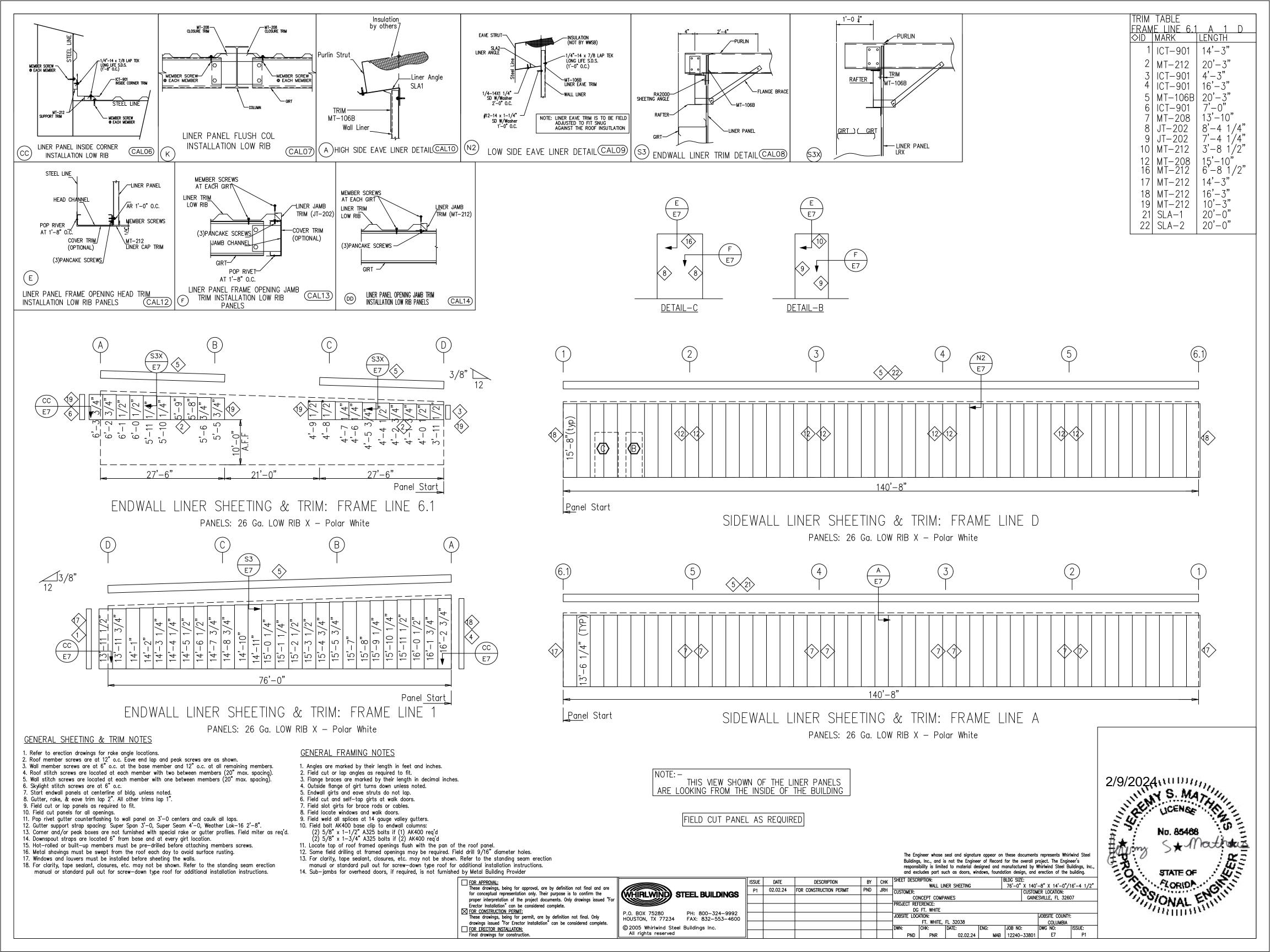
P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600

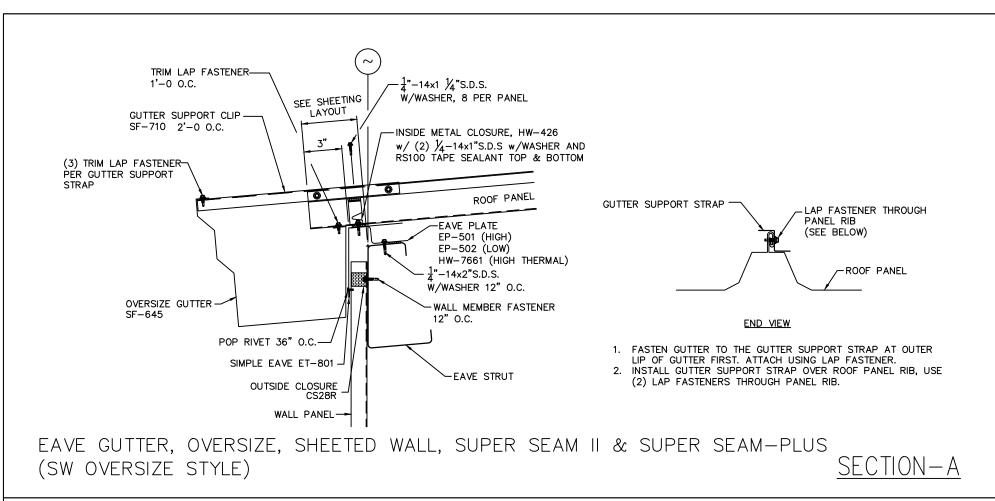
© 2005 Whirlwind Steel Buildings Inc. All rights reserved

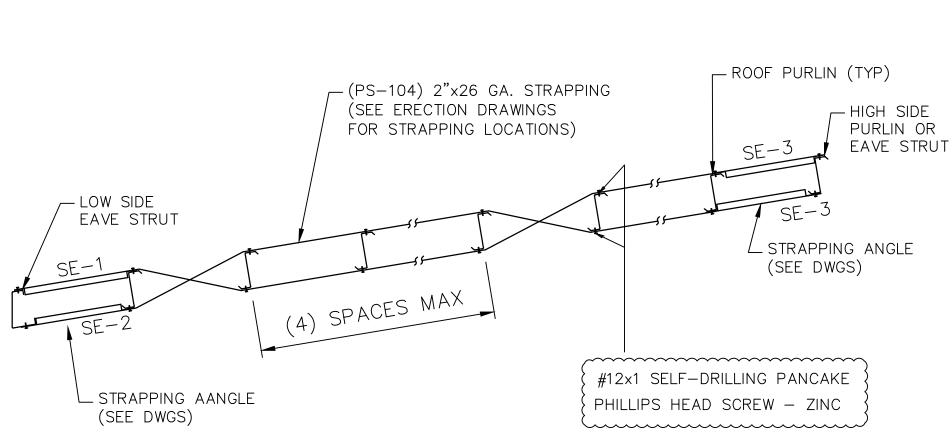
DESCRIPTION

02.02.24 FOR CONSTRUCTION PERMIT

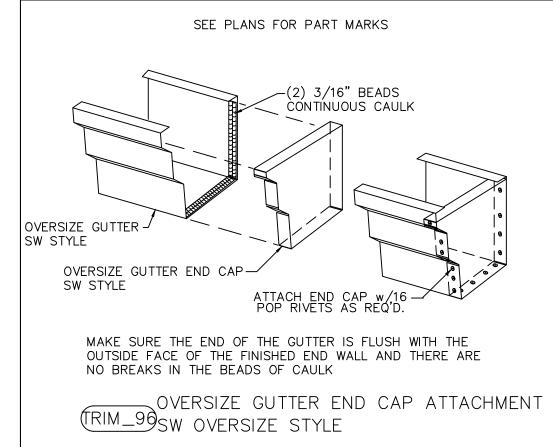


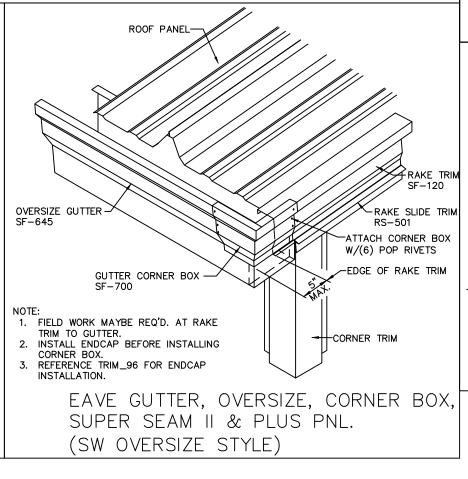


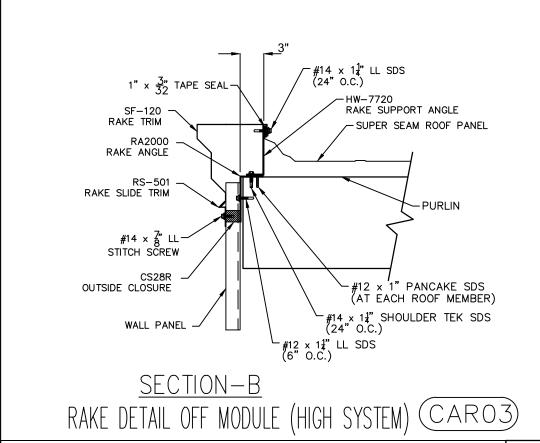


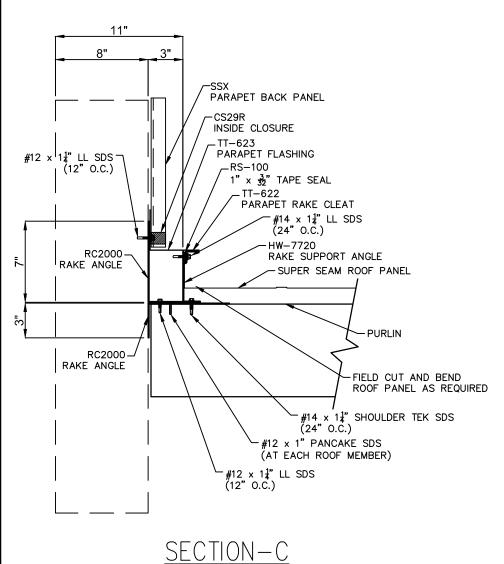




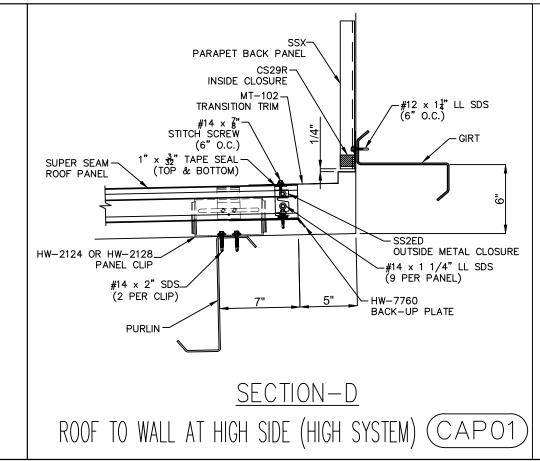


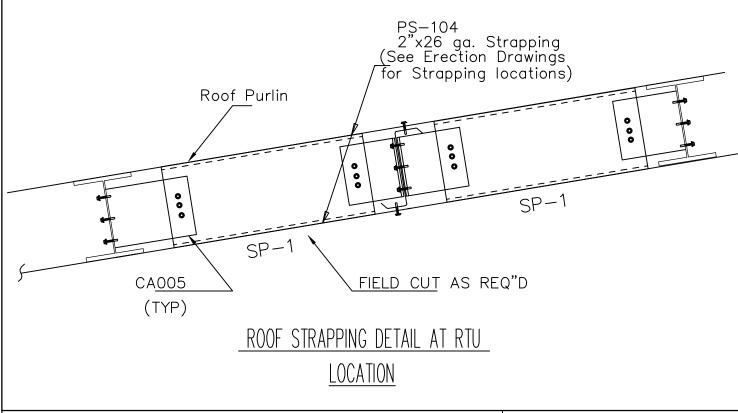


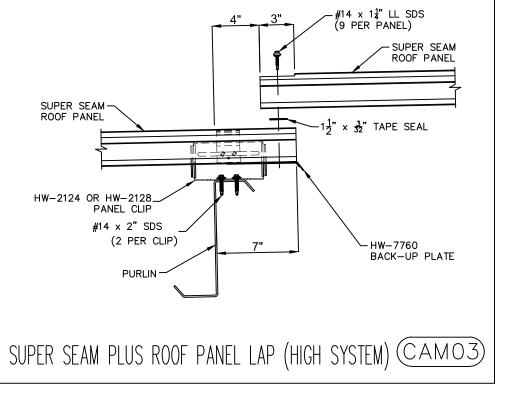




PARAPET RAKE DETAIL OFF MODULE (HIGH SYSTEM) CAPO4 -EAVE STRUT— Std.Clip Mk: AK106(TYP.) FRAME <u>St</u>d.Clip COLUMN \overline{M} k: AK106(TYP.) **SECTION ELEVATION** TYPICAL DETAIL@AK-106 SPECIAL BOLT ATTACHMENT







The E Buildir respor DATE DESCRIPTION BY CHK PND JRH 02.02.24 FOR CONSTRUCTION PERMIT PROJECT REFERENCE: DG FT. WHITE JOBSITE LOCATION: JOBSITE COUNTY: FT. WHITE, FL 32038 COLUMBIA DWG NO: 02.02.24 MAB 12240-33801 E8

Engineer whose seal and signature appear or dings, Inc., and is not the Engineer of Record onsibility is limited to material designed and excludes part such as doors, windows, found	for the overall project. The Engineer's manufactured by Whirlwind Steel Buildings, Inc.,	No. 85468 STATE OF
SCRIPTION: SECTION DETAILS	BLDG SIZE: 76'-0" X 140'-8" X 14'-0"/16'-4 1/2"	TORION AT
R: CONCEPT COMPANIES	CUSTOMER LOCATION: GAINESVILLE, FL 32607	TOO ENGLIS
REFERENCE: DG FT. WHITE	•	WAL
LOCATION:	JOBSITE COUNTY:	SATULT 1950

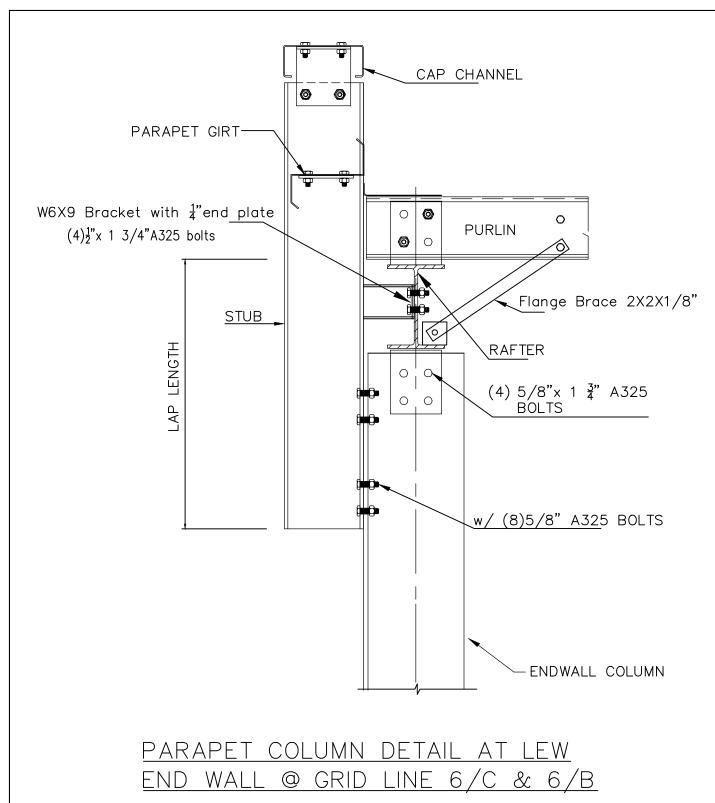
FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For FOR CONSTRUCTION PERMIT:

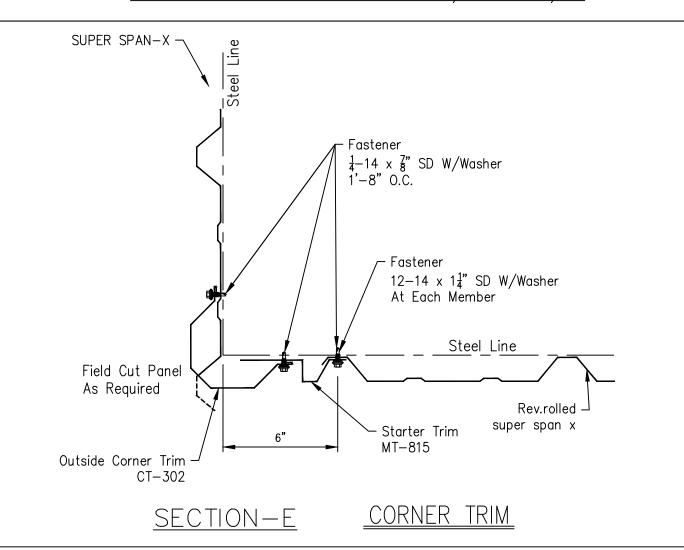
These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

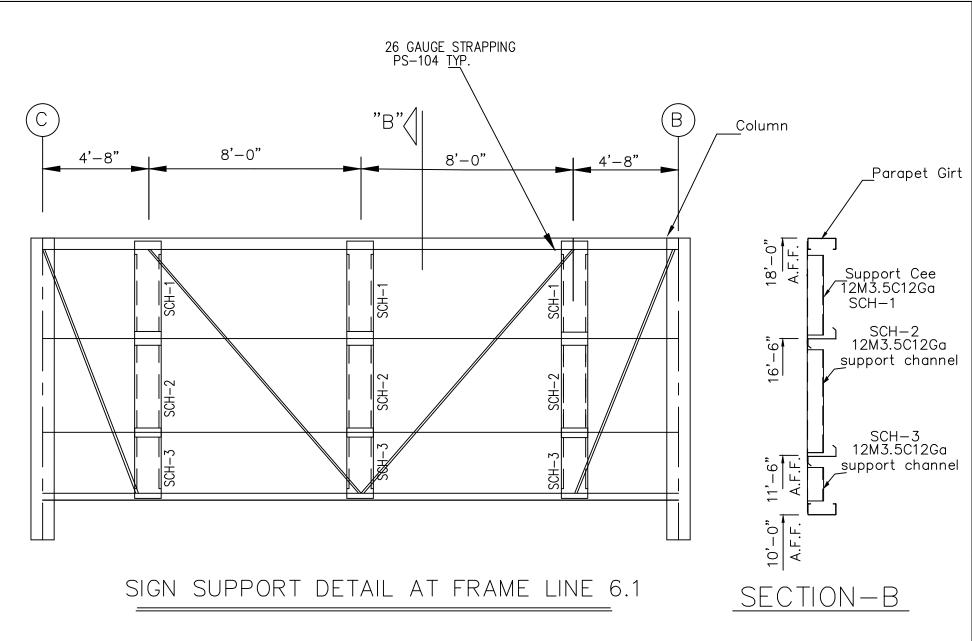
FOR ERECTOR INSTALLATION:
Final drawings for construction

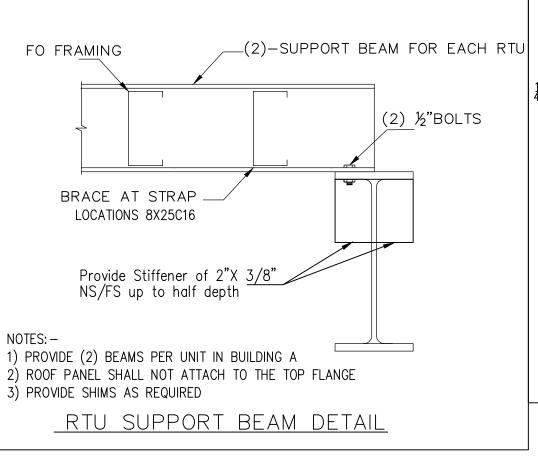
(CAF10)

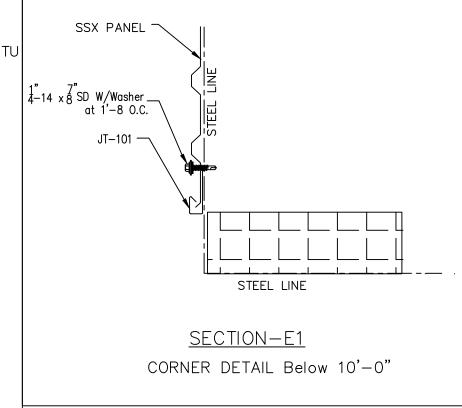
WHIRLWIND S	TEEL BUILDING
P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324-999 FAX: 832-553-46
© 2005 Whirlwind Steel	Buildings Inc.

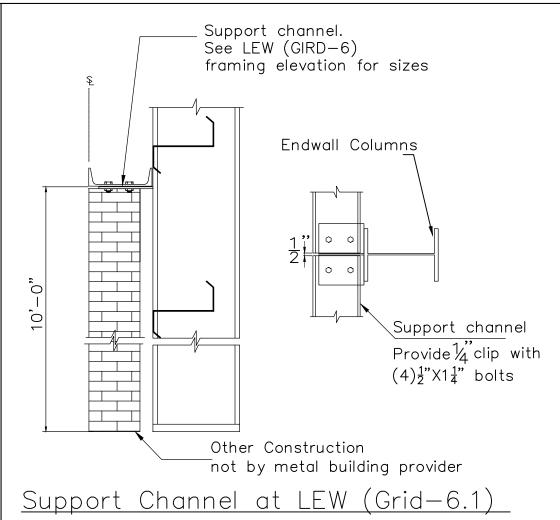




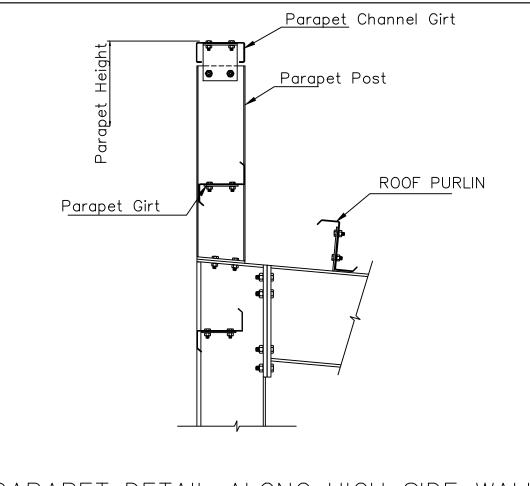








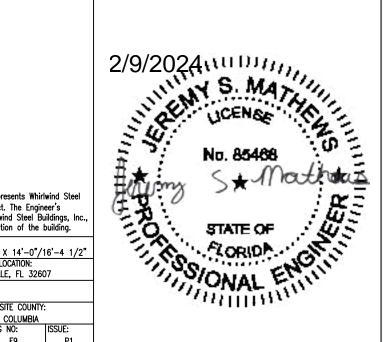
<u>at Bay 1 & 3</u>



PARAPET DETAIL ALONG HIGH SIDE WALL

Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc and excludes part such as doors, windows, foundation design, and erection of the building.

BLDG SIZE: 76'-0" X 140'-8" X 14'-0"/16'-4 1/2" ISSUE DATE DESCRIPTION PND PNR CUSTOMER: P1 | 02.02.24 | FOR CONSTRUCTION PERMIT CUSTOMER LOCATION: GAINESVILLE, FL 32607 CONCEPT COMPANIES PROJECT REFERENCE: DG FT. WHITE JOBSITE LOCATION: FT. WHITE, FL 32038 COLUMBIA DWG NO:

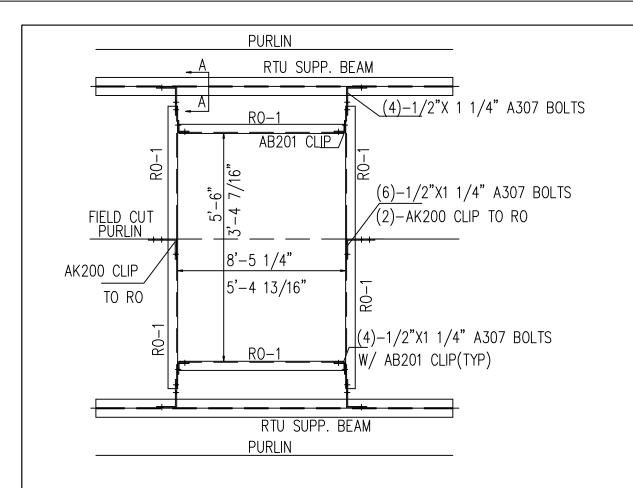


FOR APPROVAL:
These drawings, being for approval, are by definition not final and are
for conceptual representation only. Their purpose is to confirm the
proper interpretation of the project documents. Only drawings issued "I
Erector Installation" can be considered complete.
FOR CONSTRUCTION PERMIT:
These drawings, being for permit, are by definition not final. Only

drawings issued "For Erector Installation" can be considered complete.

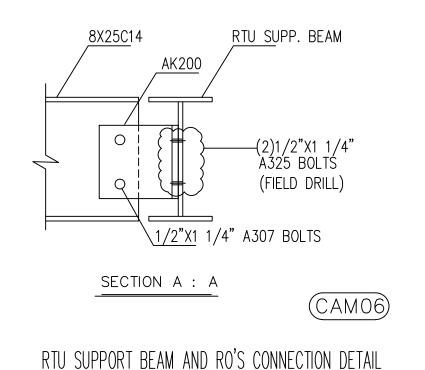
FOR ERECTOR INSTALLATION:
Final drawings for construction.

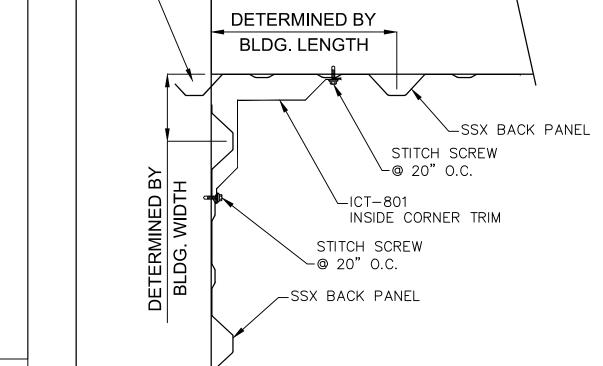
WHIRLWIND S	TEEL BUILDINGS	ŀ
P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324-9992 FAX: 832-553-4600	l
© 2005 Whirlwind Steel All rights reserved	Buildings Inc.	ŀ



BUILDER NOTE: -

- * RTU SUPPORT CHANNEL (RO-1) FIELD CUT AND FIELD DRILL Ø 9/16" HOLES AS REQUIRED FOR CURB OPENINGS.
- * PURLINS ARE DESIGNED TO BE CUT AT RTU LOCATION





INSIDE CORNER AT PARAPET BACK PANELS (CACO2)

3 5/8"

MT-133@15'-3"

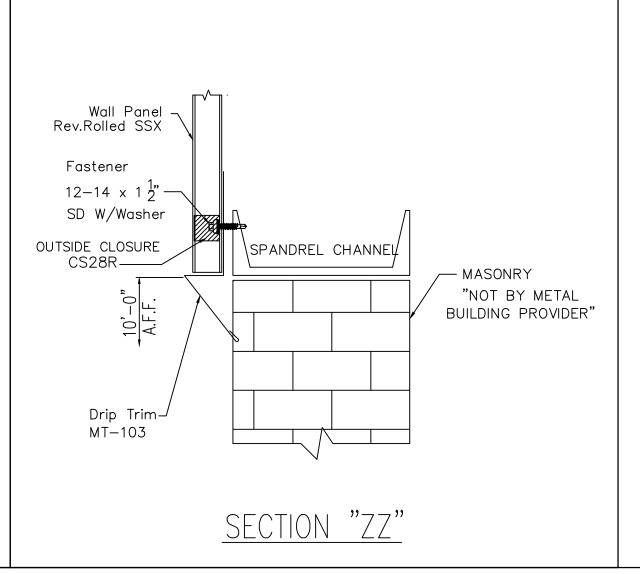
CONTINUOUS PARAPET CLEAT

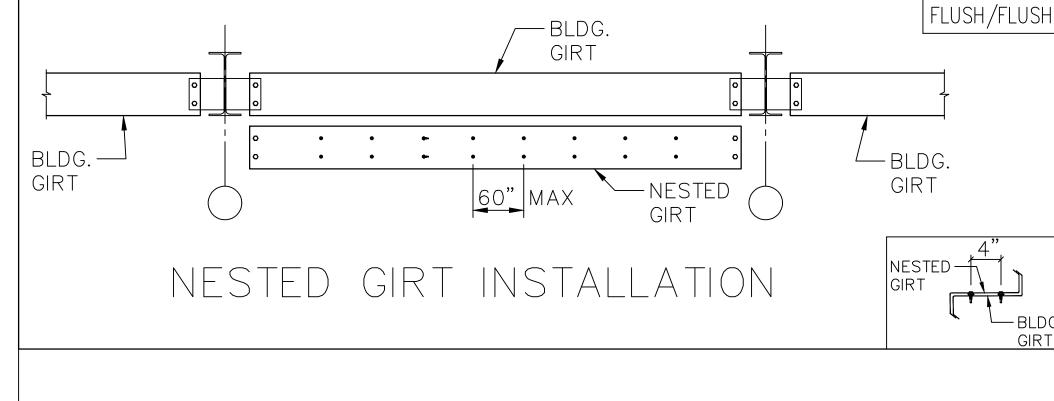
INSIDE / OUTSIDE CLOSURE

REV.ROLLED SSX/SSX PANEL_/

FIELD CUT SSX
PANEL AS REQUIRED

CS29R/CS28R





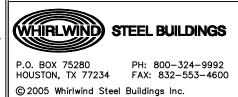
The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc.,

| FOR APPROVAL:

These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.

| FOR CONSTRUCTION PERMIT:

| FOR CONSTRUCTION PERMIT:
| These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.
| FOR ERECTOR INSTALLATION:
| Final drawings for construction.



All rights reserved

PARAPET CAP TRIM SF-20 /SF-19

7"
-1/4-14 x 8
SD W/WASHER AT
1'-0" O.C

- OUTSIDE CLOSURE

-SUPER SPAN X 26GA

CS28R

v 140'-8" WIDTH AT D

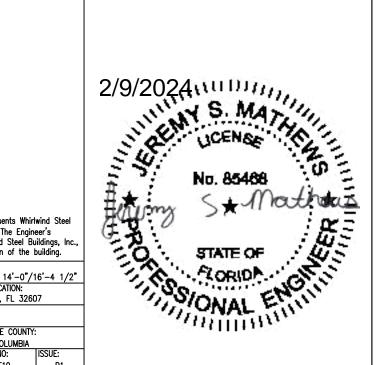
76'-0"

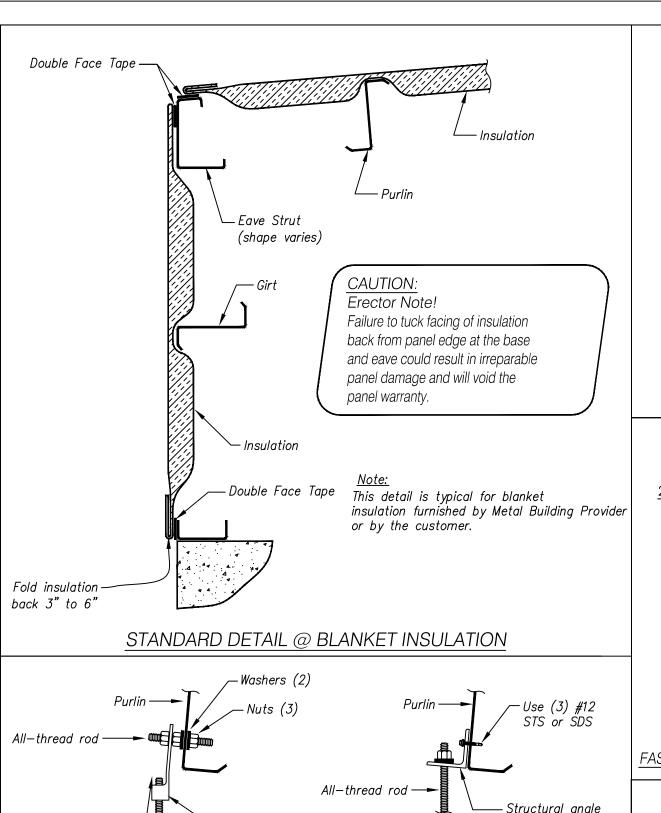
WIDTH AT 6

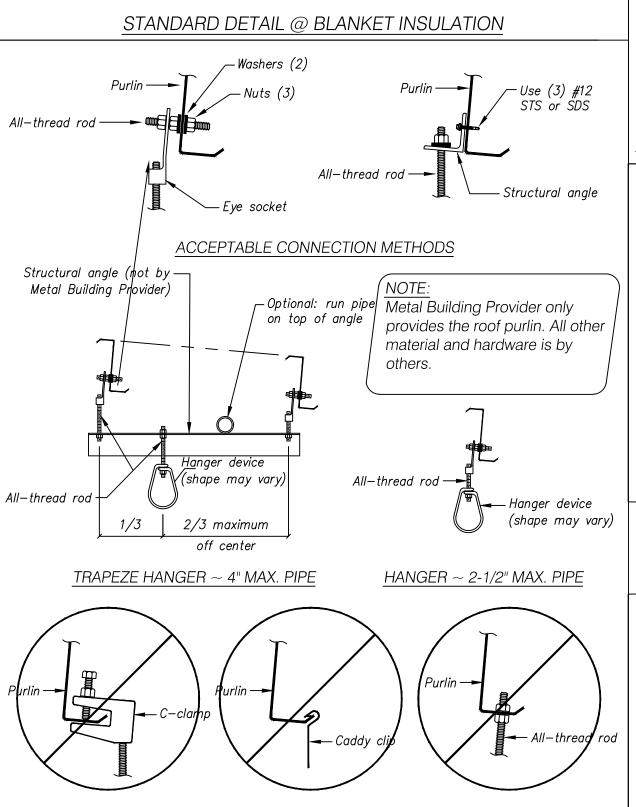
SECTION "YY"

and excludes part such as doors, windows, foundation design, and erection of the building. BY CHK SHEET DESCRIPTION: ISSUE DATE DESCRIPTION SECTION DETAILS 76'-0" X 140'-8" X 14'-0"/16'-4 1/2" PND JRH CUSTOMER: P1 02.02.24 FOR CONSTRUCTION PERMIT CUSTOMER LOCATION: GAINESVILLE, FL 32607 CONCEPT COMPANIES PROJECT REFERENCE: JOBSITE LOCATION: JOBSITE COUNTY: FT. WHITE, FL 32038

| CHK: | DATE: COLUMBIA DWG NO: JOB NO: PND PNR 02.02.24 MAB 12240-33801 E10



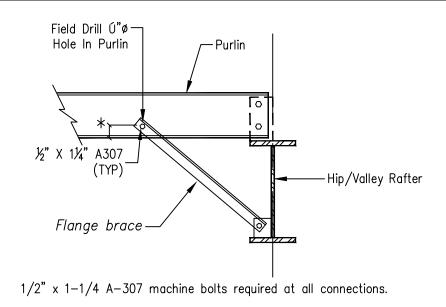




Flange C-Clamp is not Caddy clip attached to lip is Connection through the an acceptable connection not an acceptable connection flange is not acceptable

ACCEPTABLE CONNECTIONS FOR ALL

COLLATERAL LOADS FOR HANGER ATTACHMENT



*2" For 8" & 12" Purlin 3" For 10" & 14" Purlin

Flange Width

(in inches)

8 = 8

0 = 10

2 = 12

NOTES:

Seating Torque: 30 - 60 in-lbs

1/4-14 X 1-1/4" HWH TCP2 5/16" HEAD SELF-DRILLER NO SEALING WASHER - ZINC-PLATED

1/4-14 X 1-1/4" HWH SHOULDERED TCP3 5/16" HEAD SELF-DRILLER - NO SEALING WASHER - ZINC-PLATED

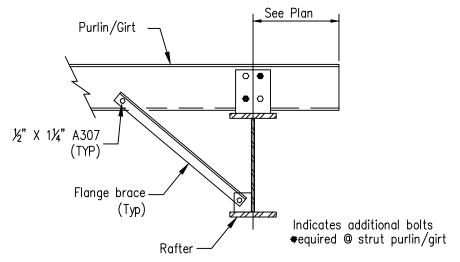
#12 x 1" PANCAKE HEAD SDS

QUADREX DRIVE, ZINC-PLATED

STANDARD FASTENERS MISCELLANEOUS

Recommended Driving Tool:
1800 RPM screw gun with depth sensing nosepiece
to prevent overdriving and stripout

FLANGE BRACE DETAIL AT HIP/VALLEY PURLIN

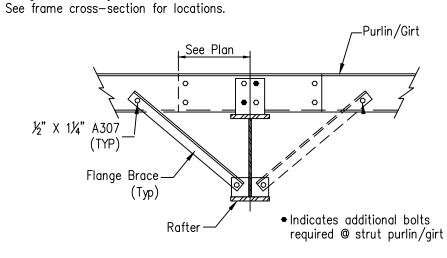


(6) 1/2" x 1-1/4 A-307 required at standard laps.

(8) 1/2" x 1-1/4 Required at strut members.

(A strut member is a purlin or girt at the bracing points. The struts continue across the length of the building.) See E-drawing for exceptions to size and quantity of bolts.

END DETAIL



(6) 1/2" x 1-1/4 A-307 required at standard laps.

Note: Flange braces may be on one or both sides.

(8) 1/2" x 1-1/4 Required at strut members. (A strut member is a purlin or girt at the bracing points.

The struts continue across the length of the building.) See E-drawing for exceptions to size and quantity of bolts.

LAP DETAIL

12"

PANEL

12"

-BEARING LEG

-STITCH SCREW

AT 1'-8" O.C.

"LOW RIB" PANEL

SHEETING DIRECTION

MEMBER SCREWS

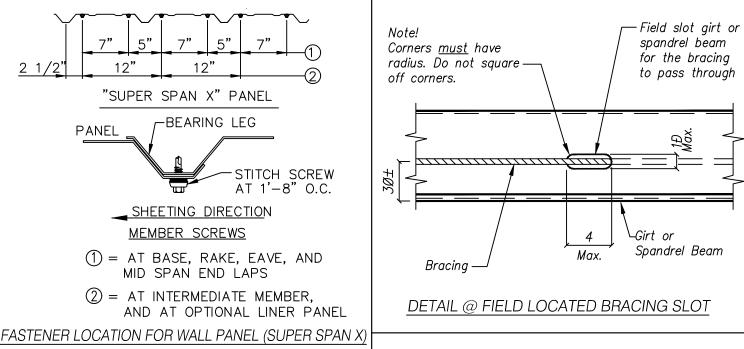
(1) = AT BASE, RAKE, EAVE, AND MID SPAN END LAPS

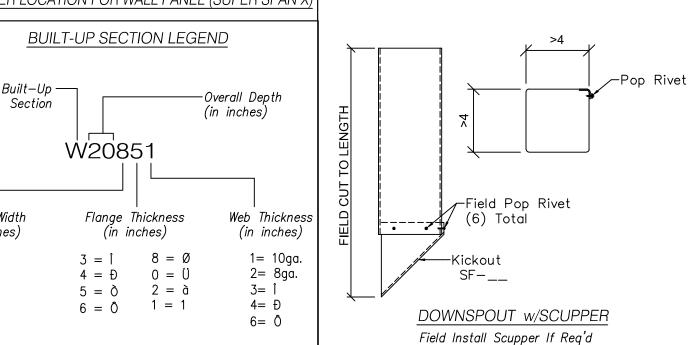
(2) = AT INTERMEDIATE MEMBER,

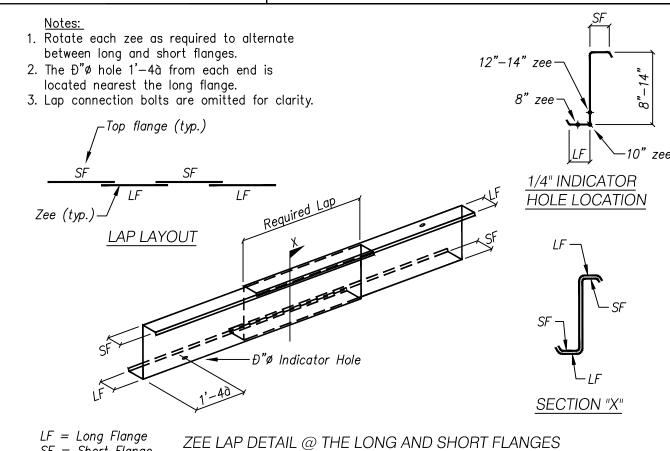
FASTENER LOCATION FOR

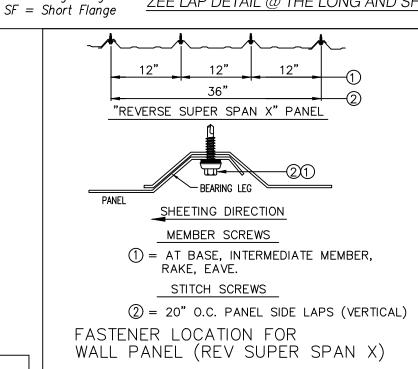
LINER PANEL (LOW RIB X)

AND AT OPTIONAL LINER PANEL





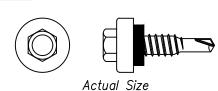




#1 Point Self—Drilling Lap Lap-Tek S.D.S.) Long-Life Zinc Die Cast Head

<u>Seating Torque:</u> 30 to 60 in-lbs Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout

Suggested Pre-Drill: None



<u>Description:</u> Đ-14 x ä Hex Head Undercut (#14 x ä Long-Life

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Ir

FOR APPROVAL:
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "Fo Erector Installation" can be considered complete FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION:
Final drawings for construction

(WHIRLWIND) STEEL BUILDINGS P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600

Actual Size

<u>Description:</u> 12-14 x 1Đ Hex Head Undercut (#12 x 1Đ Long-

Long-Life Zinc Die Cast Head

Recommended Driving Tool: 1800 RPM electric screw

gun with depth sensing nosepiece to prevent

<u>Seating Torque:</u> 30 to 60 in-lbs

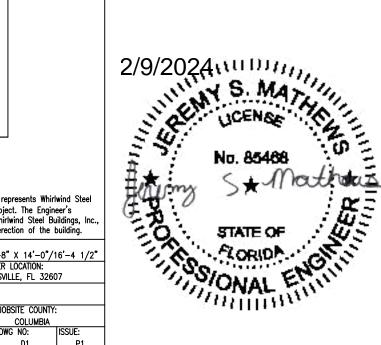
overdriving and stripout

Suggested Pre-Drill: None

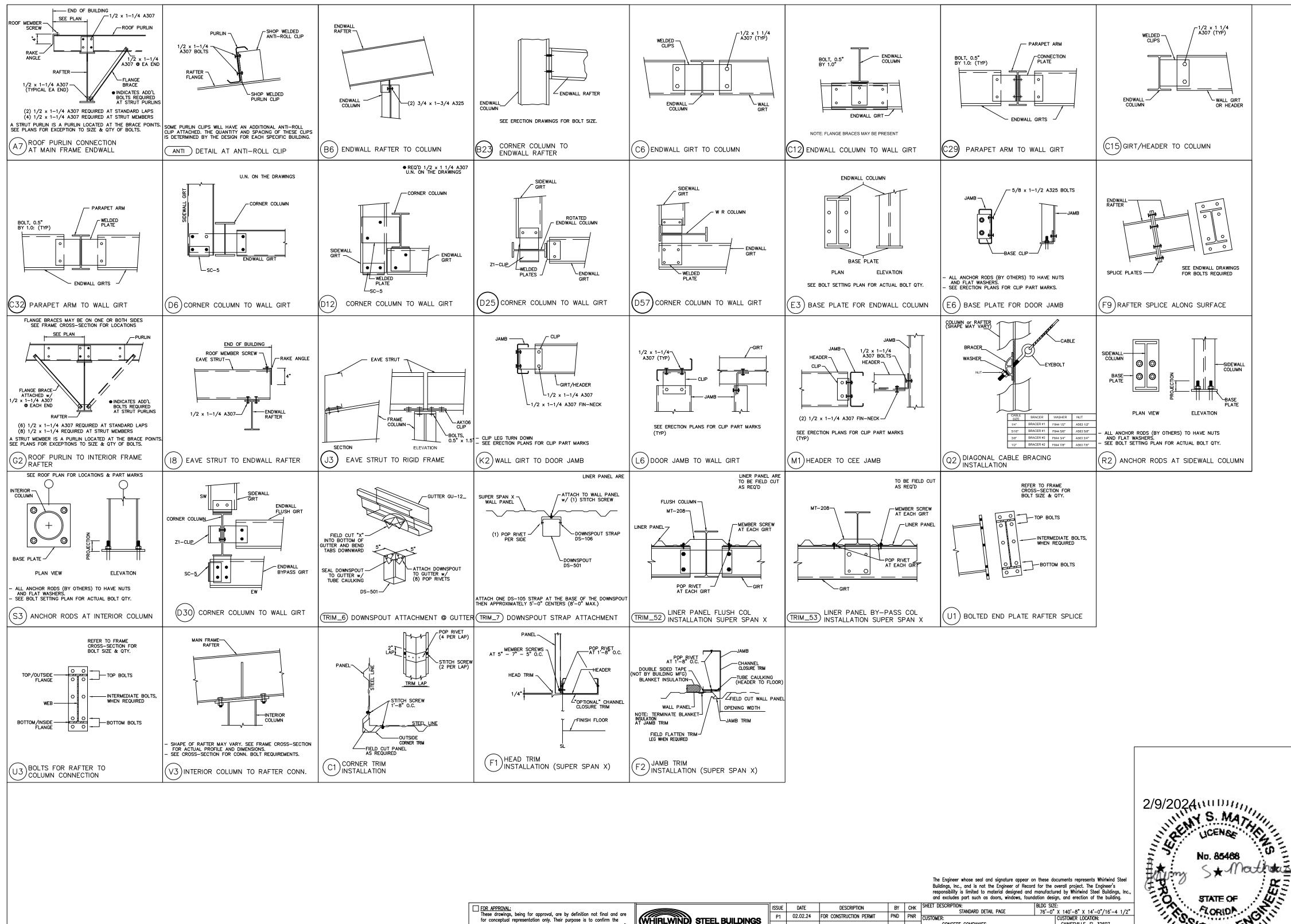
#3 Long Pilot Point Self-Drilling Life S.D.S.)

© 2005 Whirlwind Steel Buildings Inc. All rights reserved

and excludes part such as doors, windows, foundation design, and erection of the building. SHEET DESCRIPTION: STANDARD DETAIL PAGE 76'-0" X 140'-8" X 14'-0"/16'-4 1/2" CUSTOMER LOCATION: GAINESVILLE, FL 32607 PROJECT REFERENCE: DG FT. WHITE JOBSITE LOCATION: JOBSITE COUNTY: FT. WHITE, FL 32038 COLUMBIA DWG NO: JOB NO: ______D1 PND PNR 02.02.24 MAB 12240-33801



ISSUE DATE DESCRIPTION PND JRH CUSTOMER: P1 02.02.24 FOR CONSTRUCTION PERMIT



(WHIRLWIND) STEEL BUILDINGS

P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600

© 2005 Whirlwind Steel Buildings Inc.

All rights reserved

proper interpretation of the project documents. Only drawings issued "F

For Construction of the project documents. Sing didwings issued in the project documents. Sing didwings is supported in the

FOR ERECTOR INSTALLATION:
Final drawings for construction

SHEET DESCRIPTION: I BLDG SIZE: ISSUE DATE DESCRIPTION BY CHK STANDARD DETAIL PAGE 76'-0" X 140'-8" X 14'-0"/16'-4 1/2" PND PNR 02.02.24 FOR CONSTRUCTION PERMIT CUSTOMER LOCATION: GAINESVILLE, FL 32607 CONCEPT COMPANIES PROJECT REFERENCE: DG FT. WHITE JOBSITE LOCATION: JOBSITE COUNTY: FT. WHITE, FL 32038 COLUMBIA DWG NO: JOB NO:

PND PNR

02.02.24 | MAB | 12240-33801 | D2

