

[illegible]

10F1

[illegible]

Roof Plane Sheathing Area = 537 sq. ft.
Gable Sheathing Area = 467 sq. ft.
Total Sheathing Area = 583 sq. ft.
Fascia Material = 330 linear ft.
Valley Flashing Material = 124 linear ft.
Ridge Cap Material = 134 linear ft.
Hip Ridge Material = 146 linear ft.

	Pt HGT 9'-1-18" AFF			
	Pt HGT 10'-1-18" AFF			
	Pt HGT 16'-7-00" AFF			

ATTACH TO T-11 WITH SIMPSON
SCREWS (2) ROW 6 OC STAGGERED

ROOF TRUSSES TO SHARE
WALL WITH FLOOR TRUSSES

2 X RAFTERS
& CEILING JOISTS

2 X RAFTERS
& CEILING JOISTS

2 X RAFTERS
& CEILING JOISTS

SEMINOLE TRUSSES INC.
30726 Blueslar Memorial Hwy
MIDWAY FL 32343
Phone (850) 575-0102
Fax (850) 575-4413
Design By Robert J. Little

Job Name: RIOS/FIDEL RESIDENCE
Customer: America's Home Place
Designer: ROBERT J. LITTLE
PlanName: LEXINGTON-MFH
Created : 01-10-2022
SemRef#: B54908a

JOB NO:
B54908a

PAGE NO:
1 OF 1



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COA #0 278
01/10/2022

Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B54908b
Job Description: RIOS/FIDEL RESIDENCE	
Address: 7313 WEST US HWY 90, LAKE CITY, FL 32055	

Job Engineering Criteria:
Design Code: FBC 7th Ed. 2020 Res.
IntelliVIEW Version: 21.01.03A
JRef #: 1Xc38570002
Wind Standard: ASCE710 Wind Speed (mph): 0
Design Loading (psf): 55.00
Building Type:

This package contains general notes pages, 9 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	010.22.0914.17083	F100 12'3"4 Floor Truss
3	010.22.0914.14450	F102 10'11"12 Floor Truss
5	010.22.0914.10403	F104 15'11"12 Floor Truss
7	010.22.0914.11883	F106 12'11"4 Floor Truss
9	010.22.0914.36747	FT1 22'11"12 Flat Girder
11	PB180160118	
13	STRBRIBR1014	

Item	Drawing Number	Truss
2	010.22.0914.06653	F101 12'3"4 Floor Truss
4	010.22.0914.08980	F103 15'11"12 Floor Truss
6	010.22.0914.13140	F105 12'11"4 Gable
8	010.22.0914.15713	F107 12'11"4 Floor Truss
10	PB160160118	
12	REPCHRD1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

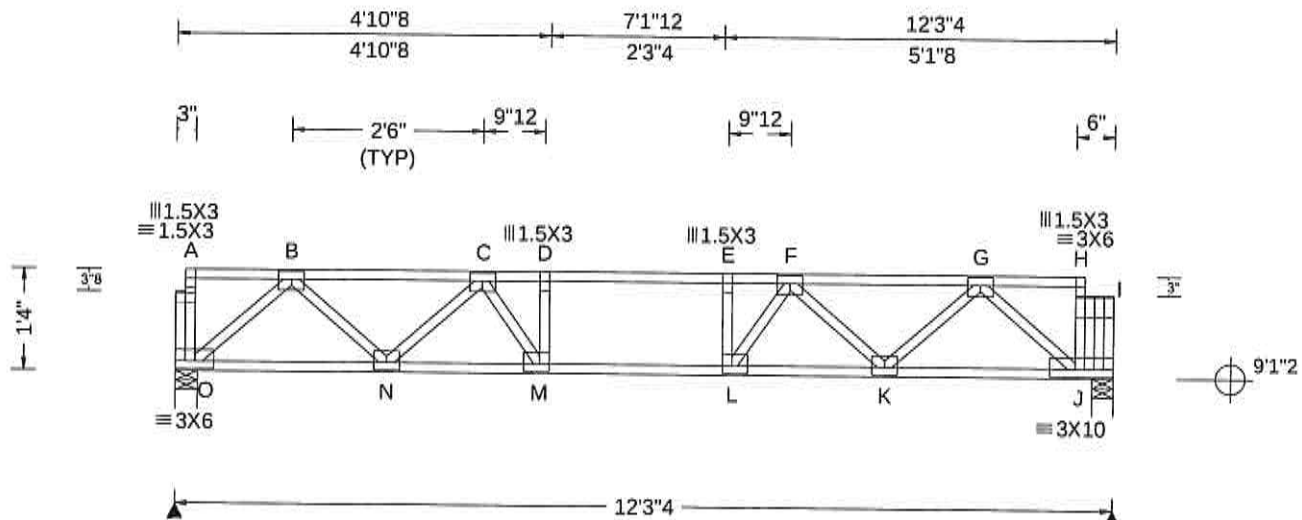
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 97375 FROM: RJL	SY42	Ply: 1 Qty: 12	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F100 12'3"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T8 DrwNo: 010.22.0914.17083 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.053 E 999 360 VERT(CL): 0.086 E 999 240 HORZ(LL): 0.012 B - - HORZ(TL): 0.020 B - - Creep Factor: 2.0 Max TC CSI: 0.392 Max BC CSI: 0.324 Max Web CSI: 0.214 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 525 - / - / - / - / - J 514 - / - / - / - / - O Brg Wid = 3.4 Min Req = 1.5 J Brg Wid = 3.4 Min Req = 1.5 Bearings O & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 - 836 E - F 0 - 1228 C - D 0 - 1227 F - G 0 - 884 D - E 0 - 1234

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

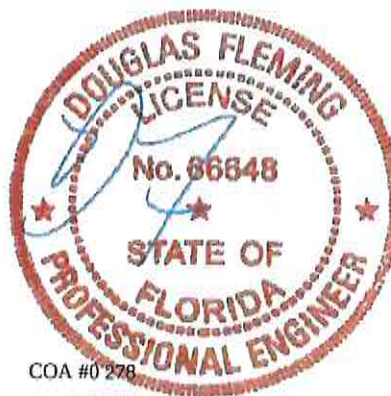
Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	524	L - K	1154
N - M	1131	K - J	562
M - L	1234		0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	0	K - G	449
B - N	435	G - J	0
N - C	0	I - J	191



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01/10/2022

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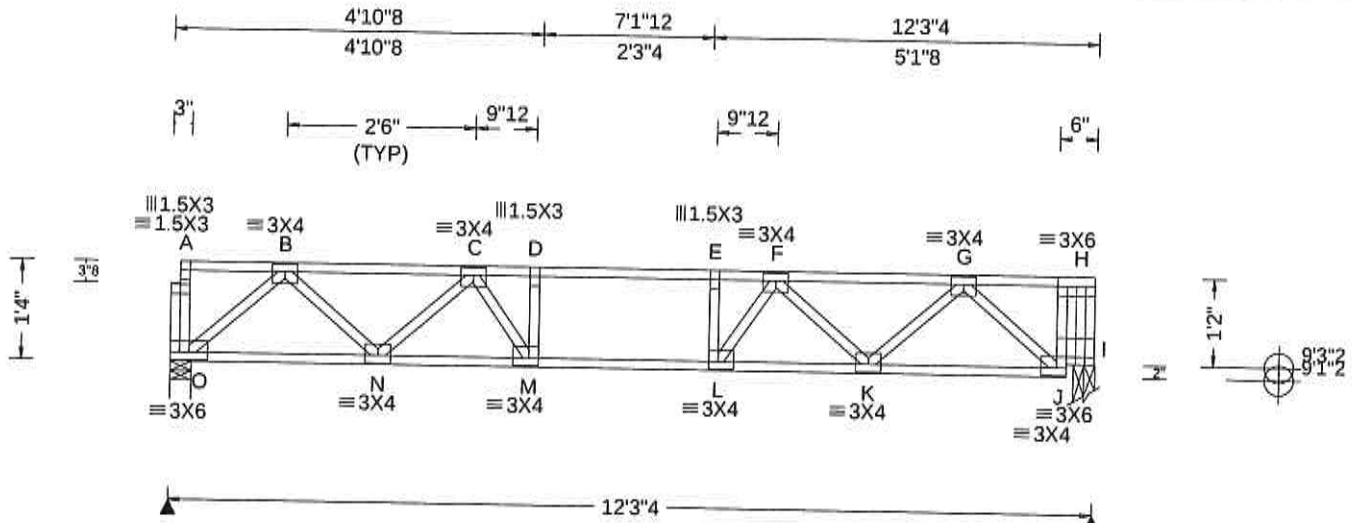
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org

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SEQN: 97346 FROM: RJL	SY42	Ply: 1 Qty: 8	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F101 12'3"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T1 DrwNo: 010.22.0914.06653 SSB / DF 01/10/2022
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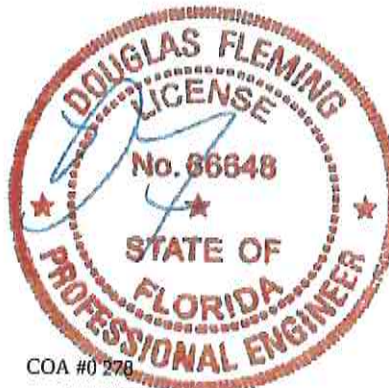
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, PI in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.054 E 999 360 VERT(CL): 0.088 E 999 240 HORZ(LL): 0.012 B - - HORZ(TL): 0.019 B - - Creep Factor: 2.0 Max TC CSI: 0.420 Max BC CSI: 0.332 Max Web CSI: 0.440 VIEW Ver: 21.01.03A.0805.15	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 530 -/- /- /- /- /- I 536 -/- /- /- /- /- O Brg Wid = 3.4 Min Req = 1.5 I Brg Wid = 3.4 Min Req = 1.5 Bearings O & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 - 845 E - F 0 - 1251 C - D 0 - 1249 F - G 0 - 908 D - E 0 - 1257

Lumber
Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;
Rt Bearing Leg: 4x2 SP #3;

Additional Notes
See detail STRBR1014 for bracing and bridging recommendations.
Truss must be installed as shown with top chord up.

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	528 0	L - K	1180 0
N - M	1145 0	K - J	612 0
M - L	1257 0		

Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	0 - 719	K - G	412 0
B - N	441 0	G - J	0 - 701
N - C	0 - 417	I - J	472 0
F - K	0 - 377	H - I	472 - 540



01/10/2022

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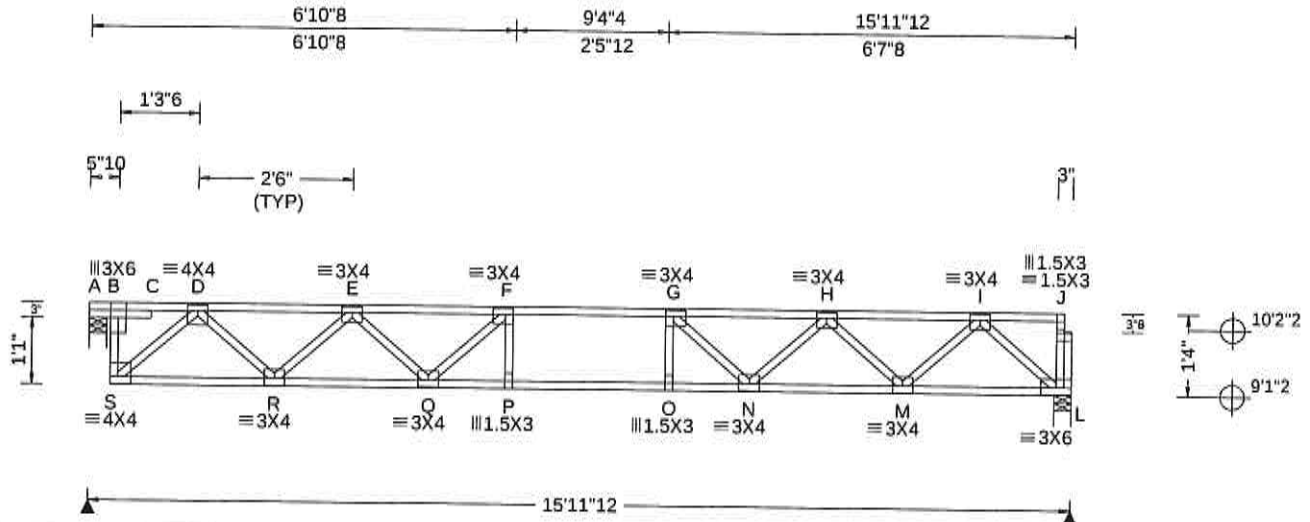
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SEQN: 97369 FROM: RJL	SY42 Qty: 5	Ply: 1 Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F103 15'11"12 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T2 DrwNo: 010.22.0914.08980 SSB / DF 01/10/2022
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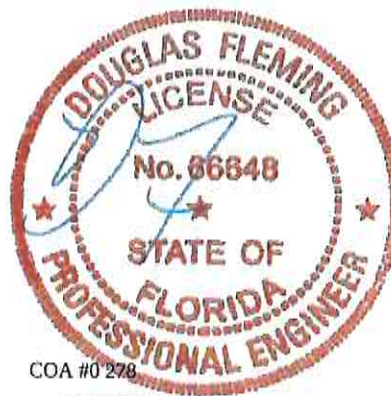
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.136 P 999 360 VERT(CL): 0.223 P 841 240 HORZ(LL): -0.022 S - - HORZ(TL): 0.037 S - - Creep Factor: 2.0 Max TC CSI: 0.577 Max BC CSI: 0.682 Max Web CSI: 0.365 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 699 /- /- /- /- /- L 694 /- /- /- /- /- A Brg Wid = 3.4 Min Req = 1.5 L Brg Wid = 3.4 Min Req = 1.5 Bearings A & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 0 - 1308 G - H 0 - 1927 E - F 0 - 1977 H - I 0 - 1197 F - G 0 - 2188

Lumber
Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes
See detail STRBRIBR1014 for bracing and bridging recommendations.
Truss must be installed as shown with top chord up.

Chords	Tens.Comp.	Chords	Tens. Comp.
S - R	851 0	O - N	2186 0
R - Q	1742 0	N - M	1664 0
Q - P	2188 0	M - L	704 0
P - O	2188 0		

Chords	Tens.Comp.	Chords	Tens. Comp.
S - B	766 0	G - N	0 - 495
S - D	0 - 1159	N - H	403 0
D - R	635 0	H - M	0 - 651
R - E	0 - 604	M - I	685 0
E - Q	375 0	I - L	0 - 958
Q - F	0 - 448		

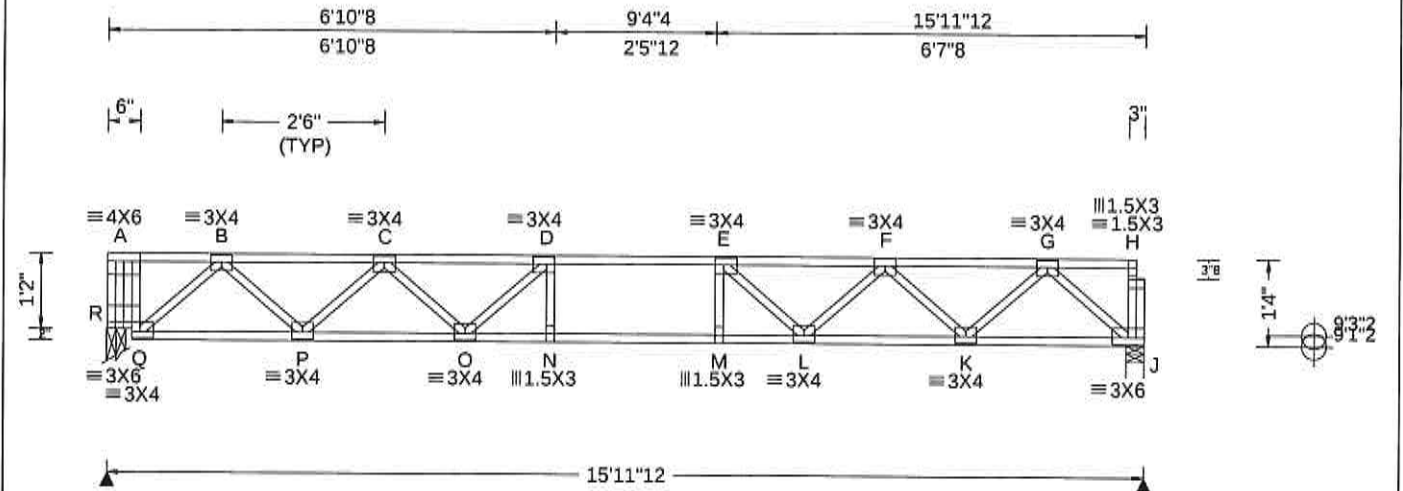


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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

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SEQN: 97366 FROM: RJL	SY42	Ply: 1 Qty: 8	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F104 15'11"12 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T3 DrwNo: 010.22.0914.10403 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.126 N 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.207 N 905 240	R 699	-/-	-/-	-/-	-/-	-/-	-/-
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.025 A - -	J 693	-/-	-/-	-/-	-/-	-/-	-/-
Des Ld: 55.00	EXP: NA Kzt: NA	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:12(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.039 A - -	R	Brg Wid = 3.4 Min Req = 1.5					
NCBCLL: 10.00	Mean Height: NA ft		Creep Factor: 2.0	J	Brg Wid = 3.4 Min Req = 1.5					
Soffit: 0.00	TCDL: NA psf		Max TC CSI: 0.429	Bearings R & J are a rigid surface.						
Load Duration: 1.00	BCDL: NA psf		Max BC CSI: 0.676	Members not listed have forces less than 375#						
Spacing: 19.2 "	MWFRS Parallel Dist: NA		Max Web CSI: 0.586	Maximum Top Chord Forces Per Ply (lbs)						
	C&C Dist a: NA ft			Chords	Tens.	Comp.	Chords	Tens.	Comp.	
	Loc. from endwall: NA			B - C	0	-1290	E - F	0	-1923	
	I: NA GCpl: NA			C - D	0	-1971	F - G	0	-1195	
	Wind Duration: NA		VIEW Ver: 21.01.03A.0805.15	D - E	0	-2181				
Lumber										

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;
Lt Bearing Leg: 4x2 SP #3;

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
Q - P	820	0	M - L	2180	0
P - O	1738	0	L - K	1662	0
O - N	2181	0	K - J	703	0
N - M	2181	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - R	631	-4	O - D	0	-447
R - A	0	-711	E - L	0	-492
R - Q	631	0	L - F	401	0
Q - B	0	-940	F - K	0	-650
B - P	654	0	K - G	683	0
P - C	0	-623	G - J	0	-956



COA #0 278

01/10/2022

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[illegible]

Lumber
Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

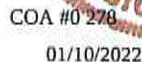
Bracing
Sheathing is required for any longitudinal(drag) forces. All connections to be designed by the building designer.

Plating Notes
All plates are 1.5X3 except as noted.

Additional Notes

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Truss must be installed as shown with top chord up.



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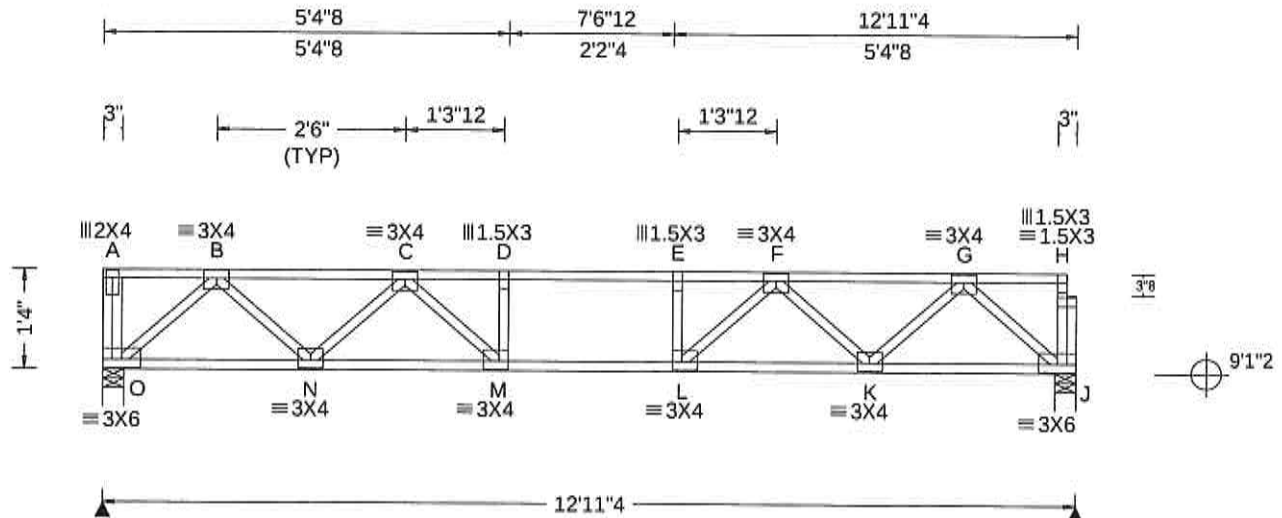
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6750 Forum Drive
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Orlando FL, 32821



SEQN: 97363 FROM: RJL	SY42	Ply: 1 Qty: 3	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F106 12'11"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T4 DrwNo: 010.22.0914.11883 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.059 E 999 360 VERT(CL): 0.097 E 999 240 HORZ(LL): 0.014 B - - HORZ(TL): 0.023 B - - Creep Factor: 2.0 Max TC CSI: 0.386 Max BC CSI: 0.342 Max Web CSI: 0.232 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 572 /- /- /- /- /- J 556 /- /- /- /- /- O Brg Wid = 3.4 Min Req = 1.5 J Brg Wid = 3.4 Min Req = 1.5 Bearings O & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 - 883 E - F 0 - 1382 C - D 0 - 1382 F - G 0 - 902 D - E 0 - 1389

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

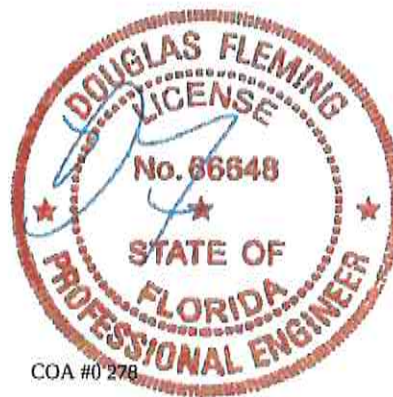
Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	533 0	L - K	1223 0
N - M	1211 0	K - J	557 0
M - L	1389 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	0 - 745	F - K	0 - 446
B - N	487 0	K - G	479 0
N - C	0 - 456	G - J	0 - 758
C - M	379 0		



COA #0 278

01/10/2022

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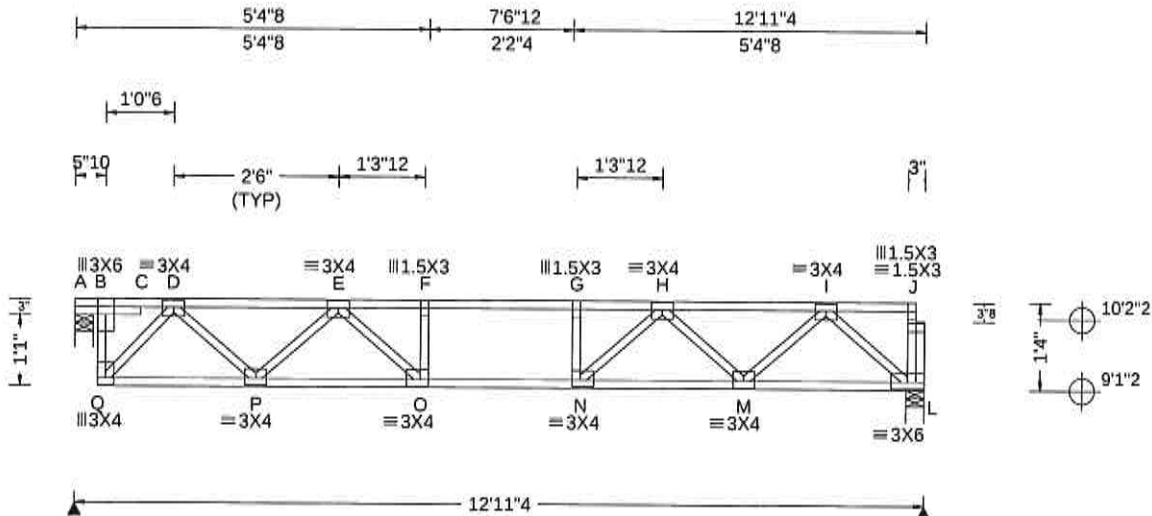
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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Orlando FL, 32821

SEQN: 97361 FROM: RJL	SY42	Ply: 1 Qty: 7	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F107 12'11"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T7 DrwNo: 010.22.0914.15713 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCCL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.067 F 999 360 VERT(CL): 0.109 F 999 240 HORZ(LL): -0.012 Q - - HORZ(TL): 0.020 Q - - Creep Factor: 2.0 Max TC CSI: 0.463 Max BC CSI: 0.344 Max Web CSI: 0.307 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 565 - / - / - / - / - L 560 - / - / - / - / - A Brg Wid = 3.4 Min Req = 1.5 L Brg Wid = 3.4 Min Req = 1.5 Bearings A & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 0 -922 G - H 0 -1404 E - F 0 -1404 H - I 0 -910 F - G 0 -1412

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes

See detail STRBRI1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	585 0	N - M	1236 0
P - O	1237 0	M - L	562 0
O - N	1412 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
Q - B	645 0	N - H	379 0
Q - D	0 -880	H - M	0 -453
D - P	469 0	M - I	485 0
P - E	0 -438	I - L	0 -764
E - O	376 0		



COA #0278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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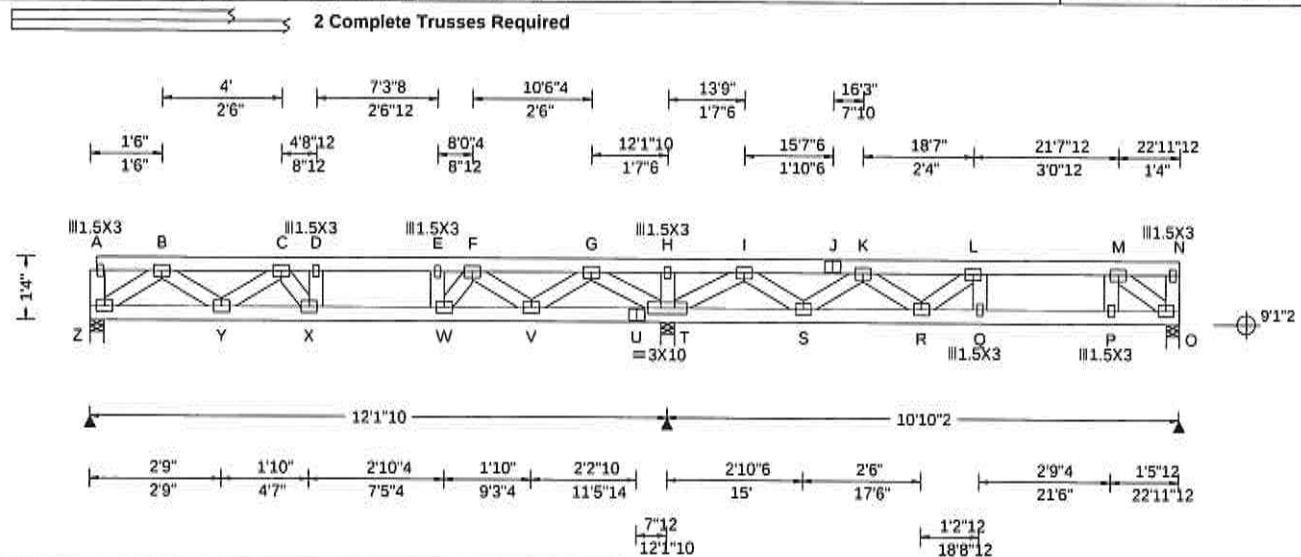
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97377 FROM: RJL	FLAT Ply: 2 Qty: 1	Job Number: B54908b RÍOS/FIDEL RESIDENCE Truss Label: FT1 22'11"12 Flat Girder	Cust: R 857 JRef:1xc38570002 T11 DrwNo: 010.22.0914.36747 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCCL: 10.00 BCCL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.022 Q 999 360 VERT(CL): 0.036 Q 999 240 HORZ(LL): 0.005 A - - HORZ(TL): 0.008 A - - Creep Factor: 2.0 Max TC CSI: 0.165 Max BC CSI: 0.167 Max Web CSI: 0.146 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Z 465 -/-/-/-/- T 1259 -/-/-/-/- O 386 -/-/-/-/- Z Brg Wid = 3.4 Min Req = 1.5 T Brg Wid = 3.5 Min Req = 1.5 O Brg Wid = 3.4 Min Req = 1.5 Bearings Z, T, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -429 E - F 37 -530 C - D 19 -544 G - H 632 0 D - E 26 -546 H - I 632 0 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. Y - X 543 0 W - V 448 -100 X - W 546 -26 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - T 0 -492 M - O 0 -398 T - I 0 -455

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.128"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

Truss must be installed as shown with top chord up.



COA #0278

01/10/2022

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Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

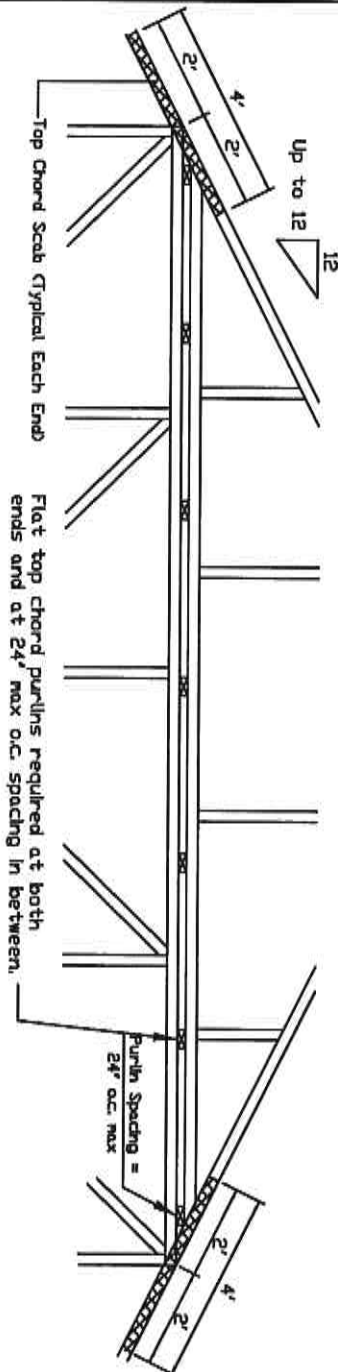
160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (min), Kzt=1.0, Dr 160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

See Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

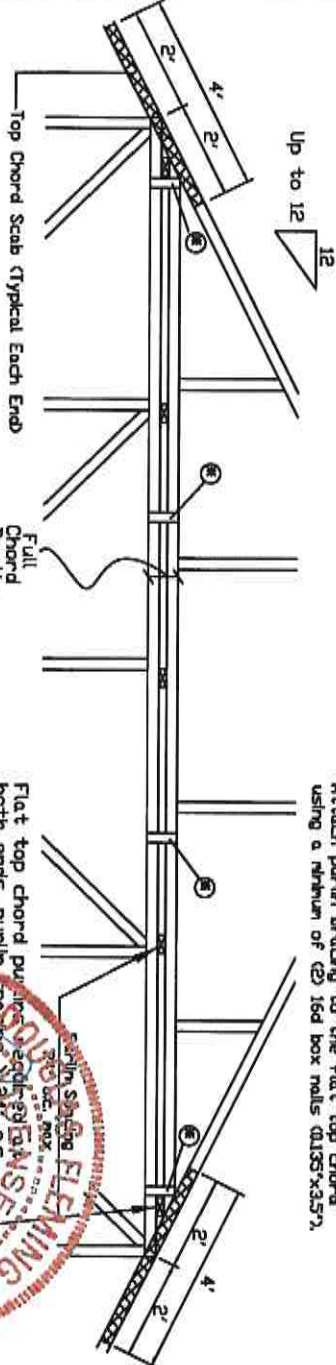


Piggyback cap truss shall nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Tru-lux plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (2) 2BPB wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss shall nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

■ In addition, provide connection with one of the following methods:

Tru-lux
Use 3X8 Tru-lux plates for 2x4 chord member, and 3X10 Tru-lux plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Tru-lux plates may be staggered 4' o.c. front to back faces.

APA Rated Gussset
8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach @ 8' o.c. with (8) 5d common (0.113"x2") nails per gussset. (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab. (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

2BPB Wave Piggyback Plate

One 2BPB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.



ALPINE COMPANY

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Suite 212
East Chicago, IN 46325

No. 06648

STATE OF FLORIDA

PROFESSIONAL ENGINEER

04/16/2018 10:27

REF PIGGYBACK

DATE 01/02/2018

DRWG PB160160118

SPACING 24.0"

180 mph Wind, 30,000 ft Mean Hgt, ASCE 7-16, Part I, Enclosed Bldg, located anywhere in roof, Exp C, Wind $U_e = 5.0$ psf (wn), Kzt=1.0, Dir 160 mph wind, 30,000 ft Mean Hgt, ASCE 7-16, Part I, Enclosed Bldg, located anywhere in roof, Exp D, Wind $U_e = 5.0$ psf (wn), Kzt=1.0.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 16d box nails (0.128"x3") at 4" o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods	
Trudox Use 3X8 Trudox plates for 2x4 chord member, and 3X10 Trudox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" bolts into cap bottom chord and (4) in base truss top chord. Trudox plates may be staggered @ o.c. front to back faces.	289B Wave Plyback Plate Use 289B wave plyback plate to each face @ 8' o.c. Attach teeth to plyback at time of construction, attach to supporting truss with (4) 0.120"x1.375" bolts per face per ply. Plyback plates may be staggered @ o.c. front to back faces.
APA Rated Gussset 8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach @ 8' o.c. with (8) 6d common nails @ 4x4s per gussset, (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered @ o.c. front to back faces.	2x4 Vertical Scales 2x4 SPF #2, full chord depth scales (each face). Attach @ 8' o.c. with (6) 10d box nails (0.125"x3") per scale, one cap bottom chord and (3) in base truss top chord. Scales may be staggered @ o.c. front to back faces.

514 Earth City Express
Suite 242
Earth City, MO 63045

[illegible]

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118
SPACING	24.0"

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(C) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2xL) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.

Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

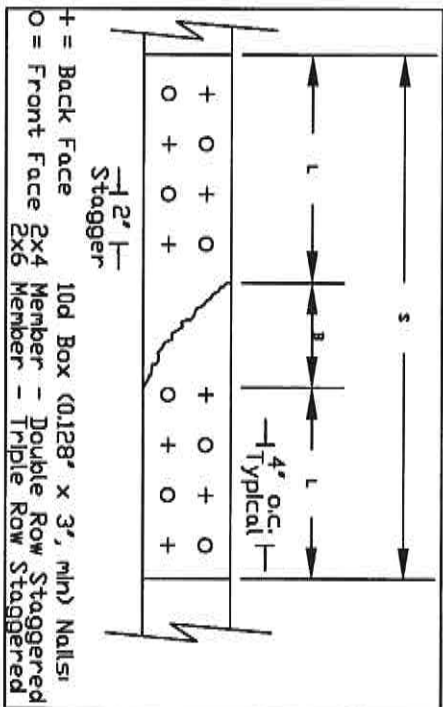
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

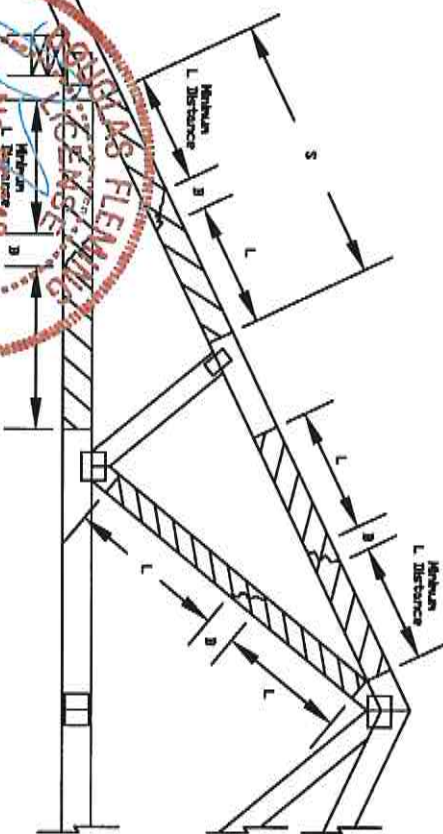
Broken chord may not support any tie-in loads.



Nail Spacing Detail

			Maximum Member Axial Force			
Member	Size	L	SPF-C	HF	DF-L	SYP
Web Only	2x4	12'	620#	635#	730#	800#
Web Only	2x4	18'	975#	1055#	1295#	1415#
Web or Chord	2x4	24'	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30'	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36'	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42'	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48'	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#

Load Duration = 0%
Member forces may be increased for Duration of Load



REF	MEMBER	REPAIR
1	1	1
2	2	2
3	3	3
4	4	4
5	5	5
6	6	6
7	7	7
8	8	8
9	9	9
10	10	10
11	11	11
12	12	12
13	13	13
14	14	14
15	15	15
16	16	16
17	17	17
18	18	18
19	19	19
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99	99	99
100	100	100

DATE 10/01/14

DRWG REPCHRD1014



514 Earth City Expressway
Suite 242
Earth City, MO 63045

[illegible]

SPACING 24.0' MAX

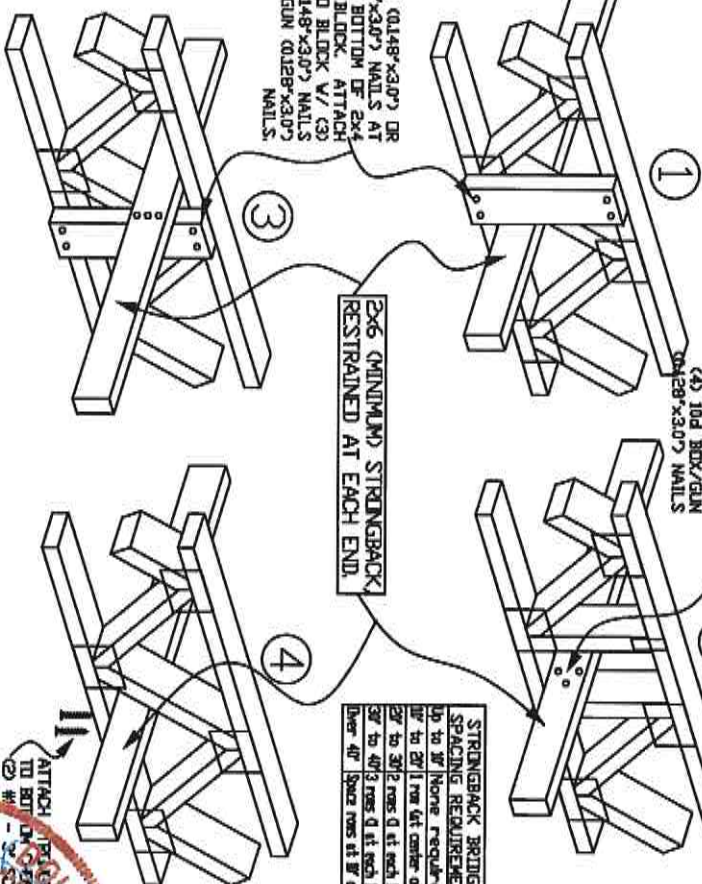
NOTE: IN LIEU OF SPLICING AS SHOWN, LAP STRONGBACK BRIDGING MEMBERS FOR AT LEAST ONE TRUSS SPACING.

2

STRINGBACK BRIDGING SPACING REQUIREMENTS	
Up to 30'	None required
30' to 201'	max (at center of span)
201' to 301'	max 4' at each 1/4 point
301' to 403'	max 4' at each 1/4 point
Over 403'	max 4' at each 1/4 point

ATTACH STRINGBACK TO WEB W/ C3 10d COMMON (0.148"x3") NAILS OR (4) 10d BDX/GLN (0.128"x3.0") NAILS

(2) 10d COMMON (0.148"x3.0") DR
BOX/GUN (0.128"x3.0") NAILS AT
TOP AND BOTTOM OF 2x4
SCAB-ON BLOCK. ATTACH
STRONGBACK TO BLOCK W/ (3)
10d COMMON (0.148"x3.0") NAILS
DR (4) 10d BOX/GUN (0.128"x3.0")
NAILS.



STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES



514 Earth City Expressway
Suite 242
Earth City, MO 63045

[illegible]

STRONGBACK BRIDGING SPACING REQUIREMENTS	
Up to 30'	None required
30' to 35'	1 row (at center of span)
35' to 40'	2 rows @ 1/4 each 1/3 painted
40' to 45'	3 rows @ 1/4 each 1/4 painted
Over 40'	Space rows at 30' oc, painted

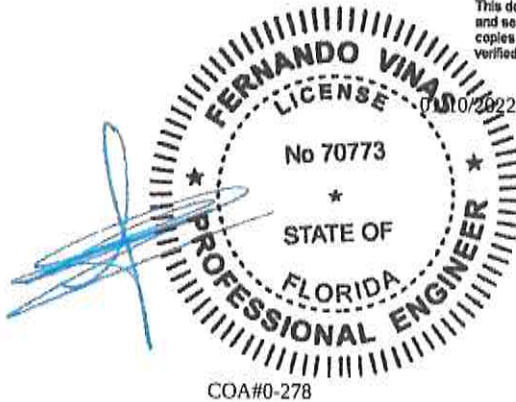
Douglas Fleming

- All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' -0" o.c. (max.)
- The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by Alpine.

For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).

TC LL	PSF	REF	STRONGBACK
TC DL	PSF	DATE	10/01/14
BC DL	PSF	DRWG STRBRIBR1014	
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			



This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.



Alpine, an ITW Company
6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B54908a
Job Description: RIOS/FIDEL RESIDENCE	
Address: 7313 WEST US HWY 90, Lake City, FL 32055	

Job Engineering Criteria:			
Design Code: FBC 7th Ed. 2020 Res.		IntelliVIEW Version: 21.01.03A	
		JRef #: 1Xc38570001	
Wind Standard: ASCE 7-16	Wind Speed (mph): 130	Design Loading (psf): 37.00	
Building Type: Closed			

This package contains general notes pages, 42 truss drawing(s) and 8 detail(s).

Item	Drawing Number	Truss
1	010.22.1204.21447	A1 33' Stepdown Hip
3	010.22.1203.00323	A1b 33' Stepdown Hip
5	010.22.1202.42607	A3 33' Stepdown Hip Girder
7	010.22.1202.44117	B2 33' Mono Hip
9	010.22.1203.51050	C1-SG 23' Gable
11	010.22.1203.37440	C2a 23' Common
13	010.22.1202.58910	C3 23' Stepdown Hip Girder
15	010.22.1202.39363	D2 23'8" Common
17	010.22.1204.04047	E1 14'7"8 Mono Hip
19	010.22.1204.05293	E3 14'7"8 Mono Hip
21	010.22.1200.08073	G1-G 15' Gable
23	010.22.1202.41057	G2a 15' Common
25	010.22.1204.24477	G4 14'7"8 Special
27	010.22.1202.55960	H1-G 13' Gable
29	010.22.1200.04670	L1-G 12' Gable
31	010.22.1203.26433	PB1 4'1"14 Common
33	010.22.1202.20423	JA 10'3"15 Hip Jack Girder
35	010.22.1202.37880	JC 6'1"8 End Jack
37	010.22.1204.35673	JE 3'2"3 Jack
39	010.22.1204.22800	JG 6'1"13 Jack
41	010.22.1204.13997	JJ 3'8"8 Hip Jack Girder
43	PB160160118	
45	REPCHRD1014	
47	GBLLETIN0118	
49	CNNAILSP1014	

Item	Drawing Number	Truss
2	010.22.1204.19820	A1a 33' Stepdown Hip
4	010.22.1202.28893	A2 33' Stepdown Hip
6	010.22.1200.06490	B1 33' Mono Hip
8	010.22.1200.09770	B3 33' Mono Hip Girder
10	010.22.1203.45450	C2 23' Common
12	010.22.1202.05937	C2b 23' Stepdown Hip
14	010.22.1203.13683	D1-SG 23'8"8 Gable
16	010.22.1202.36440	D3 23'8"8 Common Girder
18	010.22.1203.53943	E2 14'7"8 Mono Hip
20	010.22.1203.59653	E4 14'7"8 Mono Hip
22	010.22.1204.12160	G2 15' Common
24	010.22.1204.01473	G3 14'7"8 Special
26	010.22.1200.03050	G4a 14'7"8 Special
28	010.22.1204.17843	K1 12'3"8 Mono Hip Girder
30	010.22.1202.57447	L2 12' Common
32	010.22.1204.06487	PB2 3'4"8 Common
34	010.22.1204.38830	JB 6'1"8 End Jack
36	010.22.1204.34600	JD 4'7"11 Jack
38	010.22.1204.36707	JF 1'8"12 Jack
40	010.22.1204.33473	JH 3'4"13 Jack
42	010.22.1204.15687	JK 2'8" End Jack
44	PB180160118	
46	A14015ENC160118	
48	BRCLBSUB0119	
50	A14030ENC160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCDL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

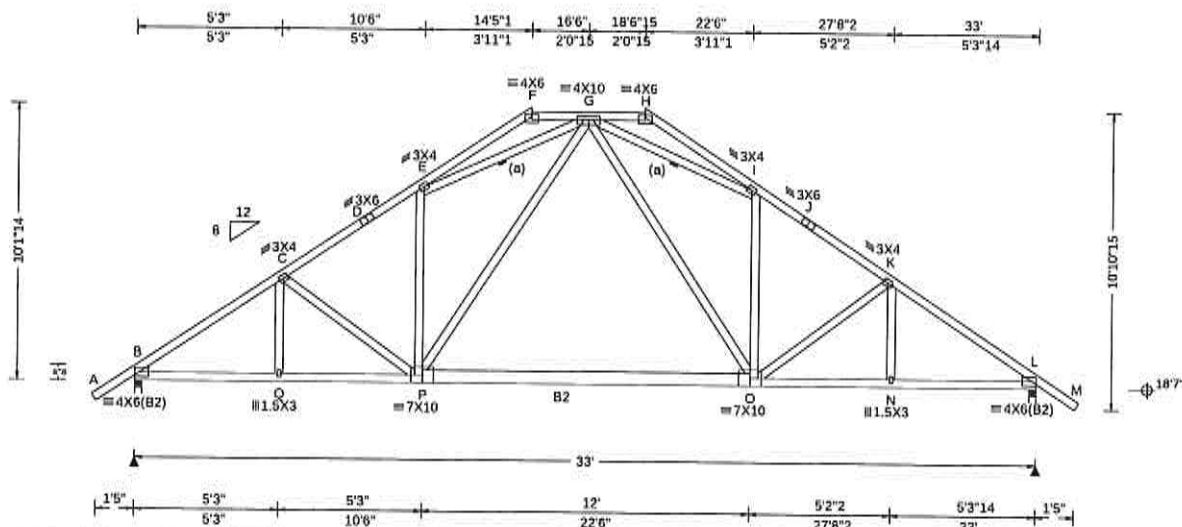
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 97562 FROM: RJL	HIPS Qty: 4	Ply: 1 Qty: 4	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A1 33' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T37 DrwNo: 010.22.1204.21447 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.095 P 999 360 VERT(CL): 0.165 P 999 240 HORZ(LL): 0.049 L - - HORZ(TL): 0.084 L - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.676 Max Web CSI: 0.724 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1495 - / - / 736 / 94 / 239 L 1495 - / - / 736 / 94 / - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 L Brg Wid = 3.0 Min Req = 1.8 Bearings B & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with Bd Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

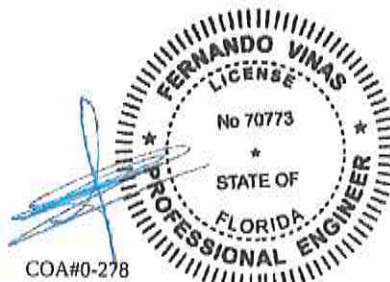
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Q	1651 -134	O - N	1651 -141
Q - P	1653 -135	N - L	1649 -140
P - O	1209 -33		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E - G	230 -1291	G - O	561 -57
P - G	562 -57	G - I	228 -1288



COA#0-278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to Job's General Notes page for additional information.

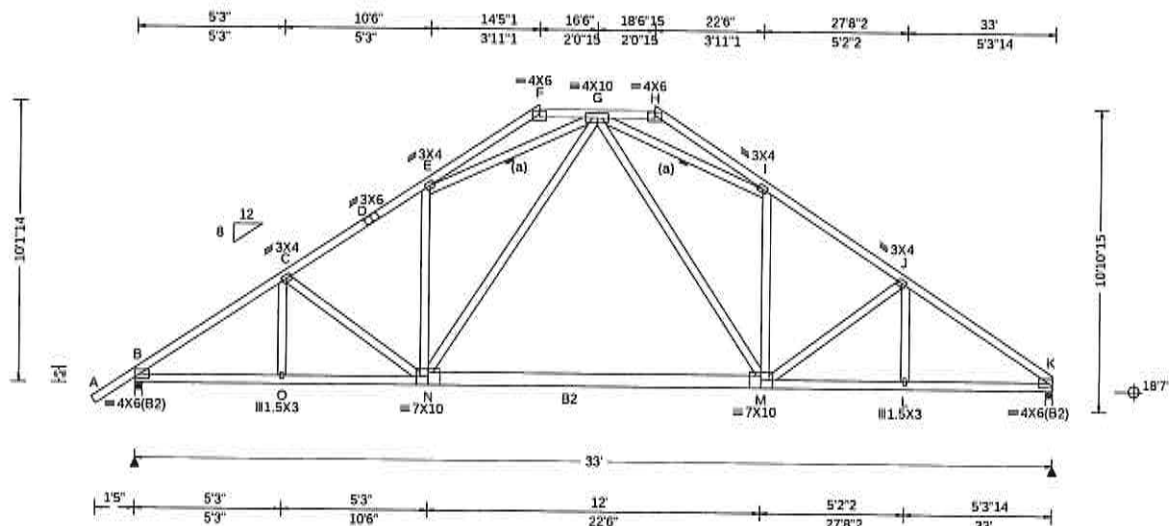
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org



6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97565 FROM: RJL	HIPS Qty: 3	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A1a 33' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T36 DrwNo: 010.22.1204.19820 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in Loc L/defl L/# VERT(LL): 0.094 N 999 360 VERT(CL): 0.164 N 999 240 HORZ(LL): 0.047 K - - HORZ(TL): 0.082 K - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.677 Max Web CSI: 0.363 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1497 - / - / - / 736 / 95 / 224 K 1408 - / - / - / 669 / 77 / - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 K Brg Wid = 3.0 Min Req = 1.7 Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

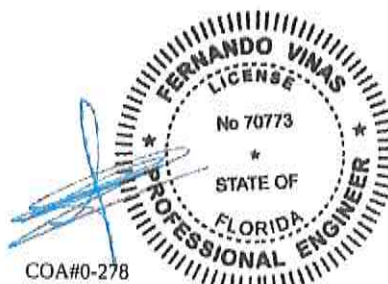
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1654 -172	M - L	1671 -181
O - N	1656 -173	L - K	1670 -180
N - M	1212 -66		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E - G	232 -1294	G - M	565 -59
N - G	561 -56	G - I	231 -1291



COA#0-278

01/10/2022

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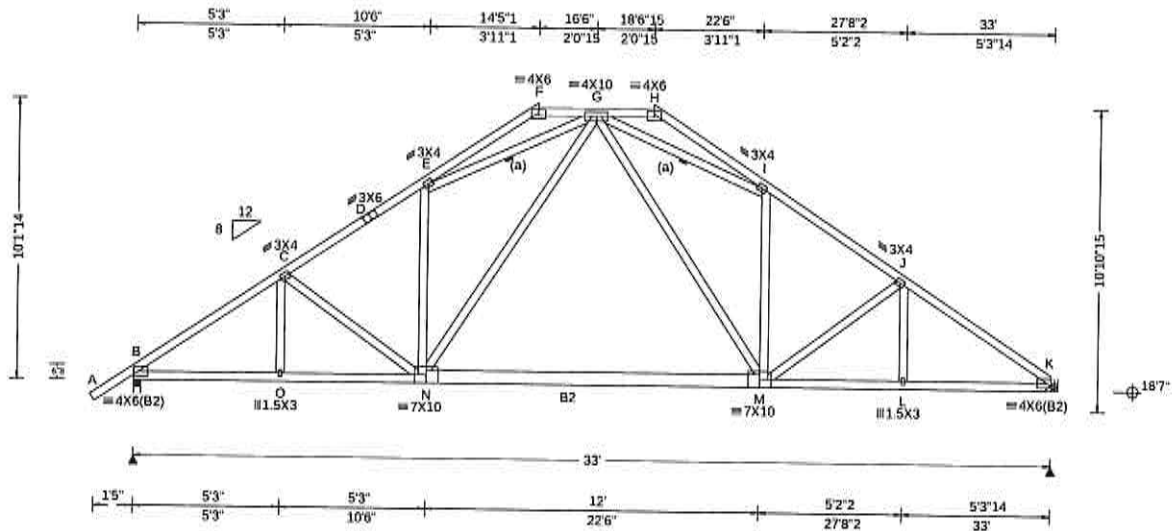
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97568 FROM: RJL	HIPS Qty: 6	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A1b 33' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T18 DrwNo: 010.22.1203.00323 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCPI: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.094 N 999 360 VERT(CL): 0.164 N 999 240 HORZ(LL): 0.047 K - - HORZ(TL): 0.082 K - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.677 Max Web CSI: 0.363 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1497 /- /- /736 /95 /224 K 1408 /- /- /669 /77 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 K Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

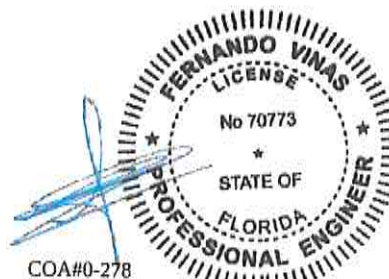
Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Special loads
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 57 plf at -1.58 to 57 plf at 33.00
PLB: From 40 plf at 12.95 to 40 plf at 20.05
BC: From 20 plf at 0.00 to 20 plf at 33.00

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1654 -172	M - L	1671 -181
O - N	1656 -173	L - K	1670 -180
N - M	1212 -66		

Maximum Web Forces Per Ply (lbs)							
Webs		Tens.Comp.		Webs		Tens. Comp.	
E - G		232 - 1294		G - M		565 - 59	
N - G		561 - 56		G - I		231 - 1291	



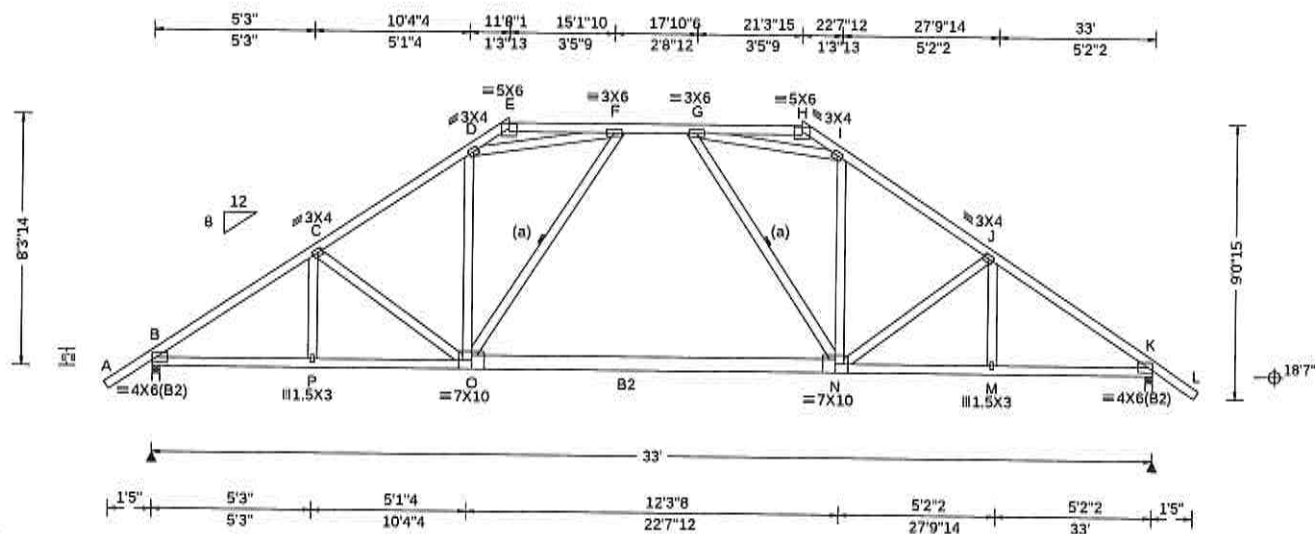
COA#0-278

01/10/2022

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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97573 FROM: RJL	HIPS Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A2 33' Stepped Hip	Cust: R 857 JRef: 1Xc38570001 T8 DrwNo: 010.22.1202.28893 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.49 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.105 F 999 360 VERT(CL): 0.223 F 999 240 HORZ(LL): 0.051 K - - HORZ(TL): 0.088 K - - Creep Factor: 2.0 Max TC CSI: 0.433 Max BC CSI: 0.713 Max Web CSI: 0.659 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1499 /- /- /732 /193 /200 K 1499 /- /- /732 /193 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 K Brg Wid = 3.0 Min Req = 1.8 Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with Bd Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Special loads

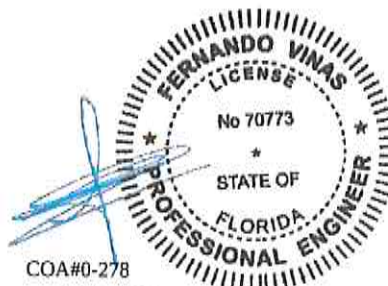
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 57 plf at -1.58 to 57 plf at 34.58
PLB: From 40 plf at 12.84 to 40 plf at 20.16
BC: From 20 plf at 0.00 to 20 plf at 33.00

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1657 -214	N - M	1657 -222
P - O	1659 -215	M - K	1655 -220
O - N	1501 -203		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - O	531 -50	G - I	289 -1328
D - F	292 -1334	N - I	532 -51



01/10/2022

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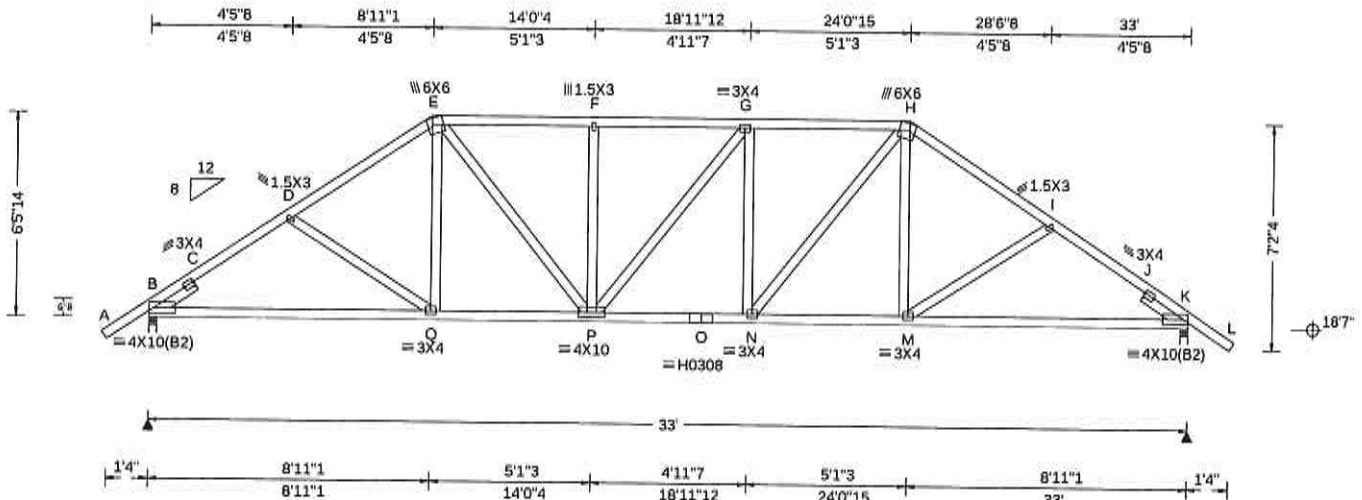
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97521 FROM: RJL	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A3 33' Steppedown Hip Girder	Cust: R 857 JRef: 1Xc38570001 T13 DrwNo: 010.22.1202.42607 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.60 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.30 ft Loc. from endwall: NA GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.164 F 999 360 VERT(CL): 0.321 F 999 240 HORZ(LL): 0.072 K - - HORZ(TL): 0.140 K - - Creep Factor: 2.0 Max TC CSI: 0.535 Max BC CSI: 0.725 Max Web CSI: 0.416 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 2496 /- /- /- /241 /- K 2496 /- /- /- /241 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 2.9 K Brg Wid = 3.0 Min Req = 2.9 Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 458 -3806 G - H 296 -3574 C - D 361 -3757 H - I 312 -3628 D - E 312 -3628 I - J 361 -3756 E - F 299 -3593 J - K 459 -3805 F - G 299 -3592

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Loading

#1 hip supports 6-1-8 jacks with no webs.
Left side jacks have 8-11-1 setback with 0-0-0 cant and 1-5-15 overhang. End jacks have 6-1-8 setback with 0-0-0 cant and 1-5-15 overhang. Right side jacks have 8-11-1 setback with 0-0-0 cant and 1-5-15 overhang.

Wind

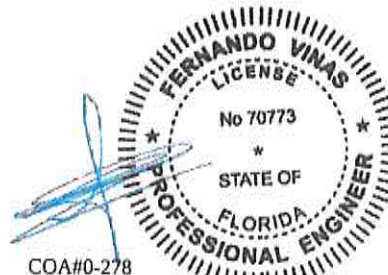
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	3044	-282	O - N	3600	-303
Q - P	2972	-243	N - M	2972	-244
P - O	3600	-303	M - K	3043	-282

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - Q	789	0	G - N	160	-555
E - P	975	-88	N - H	961	-84
F - P	161	-546	H - M	785	0



01/10/2022

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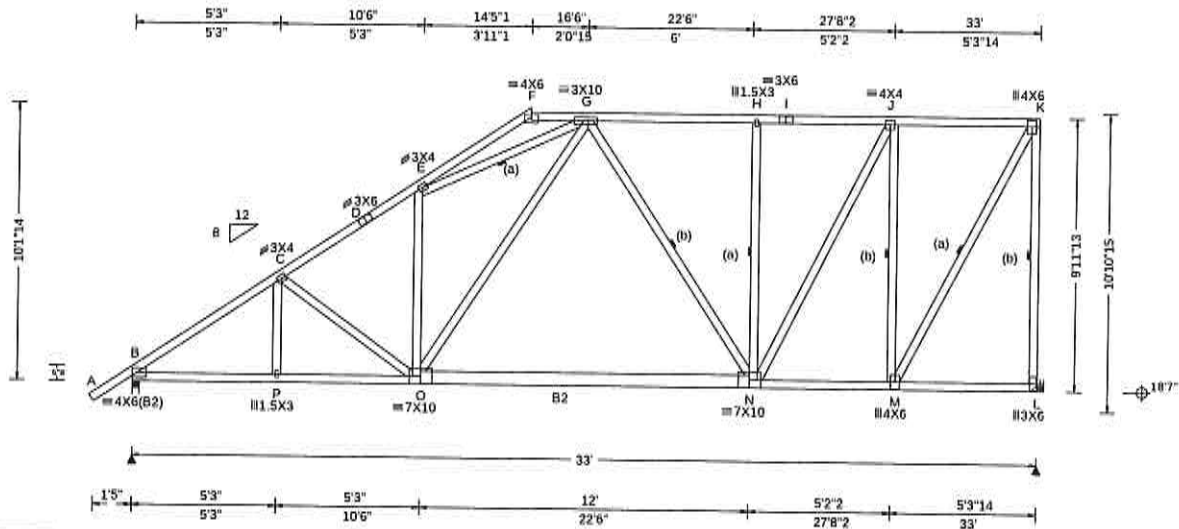
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97570 FROM: RJL	HIPM Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: B1 33' Mono Hip	Cust: R 857 JRef: 1Xc38570001 T3 DrwNo: 010.22.1200.06490 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/dell L/# VERT(LL): 0.100 O 999 360 VERT(CL): 0.166 O 999 240 HORZ(LL): 0.033 C - - HORZ(TL): 0.055 C - - Creep Factor: 2.0 Max TC CSI: 0.442 Max BC CSI: 0.687 Max Web CSI: 0.961 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1530 /- /- /774 /36 /205 L 1603 /- /- /628 /193 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 L Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement, 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.
(b) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement, 80% length of web member. Attached with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA#0-278

01/10/2022

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1698 - 471	O - N	1286 - 398
P - O	1700 - 472	N - M	764 - 228

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E - G	368 - 1498	J - M	453 - 1172
O - G	519 - 38	M - K	1543 - 455
N - J	959 - 252	K - L	494 - 1489

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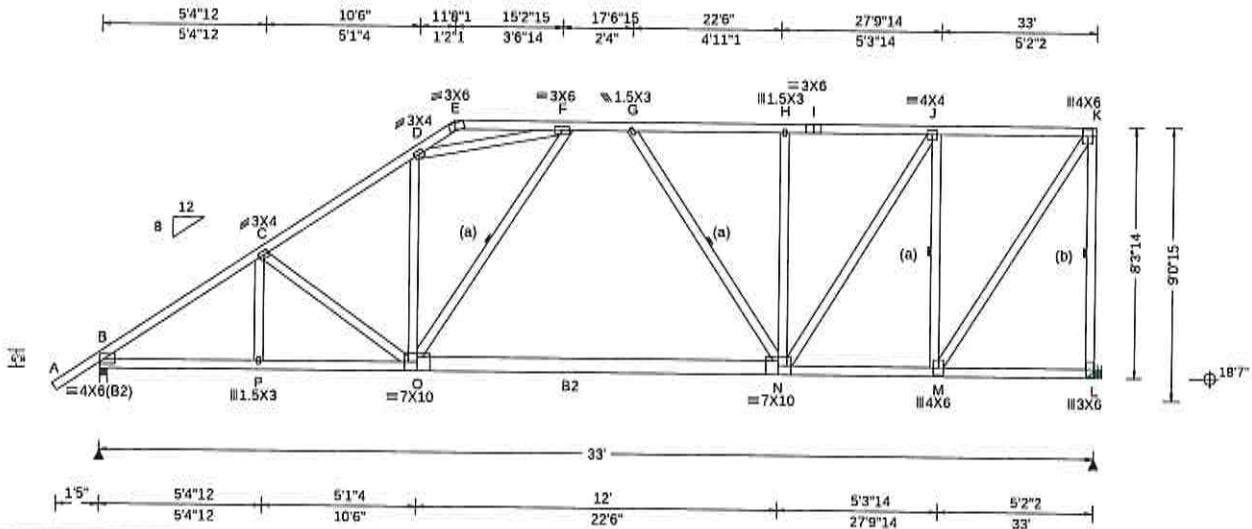
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97575 FROM: RJL	HIPM Qty: 1	Job Number: 854908a RIOS/FIDEL RESIDENCE Truss Label: B2 33' Mono Hip	Cust: R 857 JRef: 1Xc38570001 T14 DrwNo: 010.22.1202.44117 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.49 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Cc: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.117 G 999 360 VERT(CL): 0.233 G 999 240 HORZ(LL): 0.033 M - - HORZ(TL): 0.062 D - - Creep Factor: 2.0 Max TC CSI: 0.439 Max BC CSI: 0.716 Max Web CSI: 0.970 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1525 - / - / - / 757 / 45 / 167 L 1577 - / - / - / 608 / 173 / - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 L Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.
(b) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Special loads

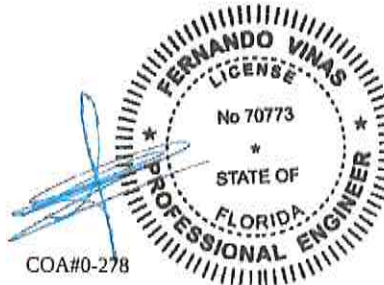
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 57 plf at -1.58 to 57 plf at 33.00
PLB: From 40 plf at 12.95 to 40 plf at 20.05
PLB: From 40 plf at 25.10 to 40 plf at 27.53
PLB: From 40 plf at 30.20 to 40 plf at 32.71
BC: From 20 plf at 0.00 to 20 plf at 33.00

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1691 - 479	O - N	1529 - 483
P - O	1693 - 480	N - M	908 - 277

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - O	521 - 62	J - M	454 - 1170
D - F	353 - 1431	M - K	1595 - 480
N - J	992 - 266	K - L	494 - 1468



COA#0-278

01/10/2022

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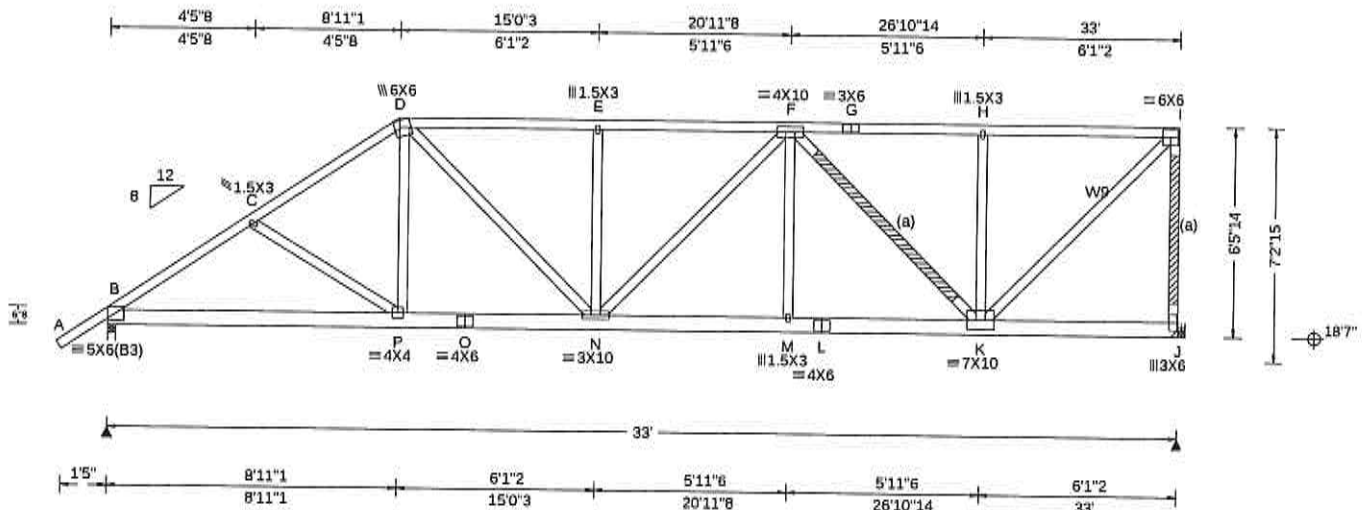
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97534 FROM: RJL	HIPM Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: B3 33' Mono Hip Girder	Cust: R 857 JRef: 1Xc38570001 T5 DrwNo: 010.22.1200.09770 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.149 E 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.296 E 999 240	B 2433 /- /- /- /232 /-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.042 D - -	J 2526 /- /- /- /197 /-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.083 D - -	Wind reactions based on MWFRS
NCBCLL: 0.00	Mean Height: 21.57 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.0 Min Req = 2.9
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.952	J Brg Wid = -
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.696	Bearing B is a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.616	Members not listed have forces less than 375#
	C&C Dist a: 3.30 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: NA	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 21.01.03A.0805.15	B - C 355 -3709 F - G 168 -2146
	Wind Duration: 1.60			

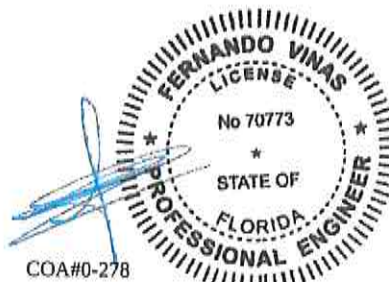
Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3; W9 2x4 SP #1;

Bracing
(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
#1 hip supports 6-1-8 jacks with no webs.
Left side jacks have 8-11-1 setback with 0-0-0 cant and 1-6-15 overhang. End jacks have 6-1-8 setback with 0-0-0 cant and 1-6-15 overhang.

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

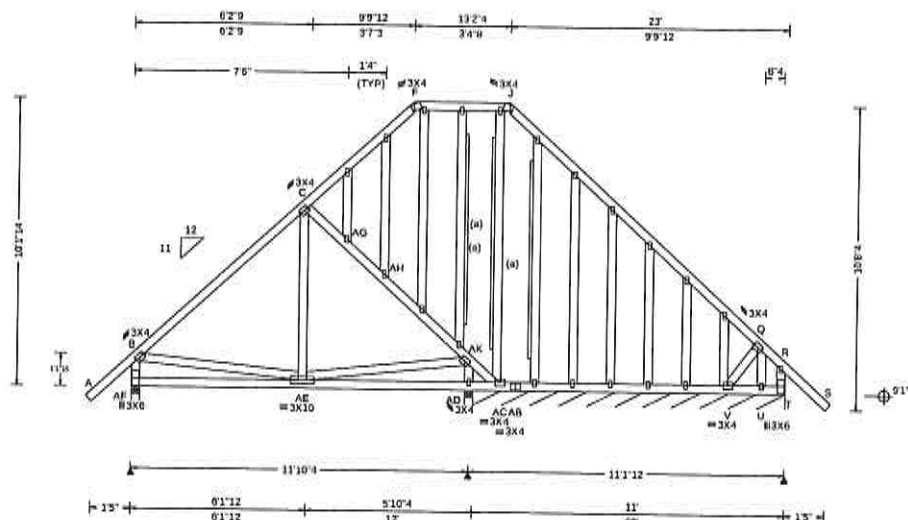


01/10/2022

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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97483 FROM: RJL	GABL Qty: 1	Ply: 1 Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C1-SG 23' Gable	Cust: R 857 JRef: 1Xc38570001 T24 DrwNo: 010.22.1203.51050 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.020 E 999 360 VERT(CL): 0.045 E 999 240 HORZ(LL): -0.015 AH - - HORZ(TL): 0.034 AH - - Creep Factor: 2.0 Max TC CSI: 0.466 Max BC CSI: 0.213 Max Web CSI: 0.178 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL AF 843 /- /- /371 /19 /275 AD 700 /- /- /408 /21 /- T* 104 /- /- /61 /17 /- V /-114 Wind reactions based on MWFRS AF Brg Wid = 3.5 Min Req = 1.5 AD Brg Wid = 3.5 Min Req = 1.5 T Brg Wid = 132 Min Req = - Bearings AF, AD, & AD are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) 1x4 #3SRB SPF-S or better "L" reinforcement.
80% length of web member. Attach with 8d Box or
Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers
and cladding load not to exceed 5.00 PSF one face
and 24.0" span opposite face. Top chord must not be
cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C
member design.

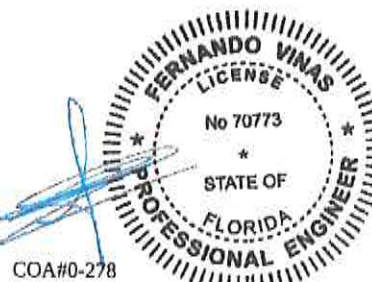
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for
gable wind bracing and other requirements.

Laterally brace top chord below filler and bottom chord
above filler at 24" o.c., including a lateral brace
at chord ends (if no rigid diaphragm exists at that point).



COA#0-278

01/10/2022

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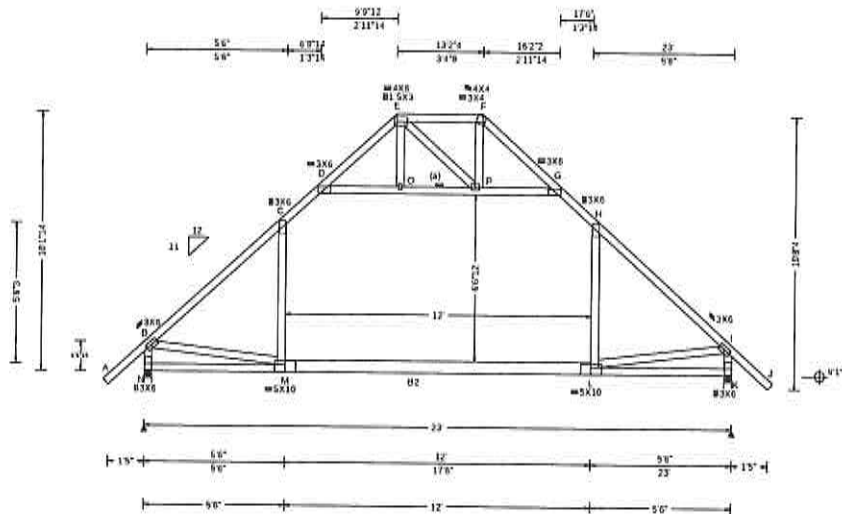
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97474 FROM: RJL	COMN	Ply: 1 Qty: 6	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C2 23' Common	Cust: R 857 JRef: 1Xc38570001 T23 DrwNo: 010.22.1203.45450 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.348 L 792 360 VERT(CL): 0.647 L 426 240 HORZ(LL): -0.337 H - - HORZ(TL): 0.627 H - - Creep Factor: 2.0 Max TC CSI: 0.956 Max BC CSI: 0.772 Max Web CSI: 0.310 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL N 1238 - / - / - / 547 / 42 / 244 K 1238 - / - / - / 547 / 42 / - Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 K Brg Wid = 3.5 Min Req = 1.5 Bearings N & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 199 -1332 G - H 272 -898 C - D 272 -898 H - I 199 -1332

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Collar-Tie braced with continuous lateral bracing @ 24" OC, or rigid sheathing

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 40 plf at -1.61 to 40 plf at 24.61
TC: From 19 plf at -1.61 to 19 plf at 24.61
PLB: From 40 plf at 5.50 to 40 plf at 17.50
BC: From 20 plf at 0.00 to 20 plf at 23.00

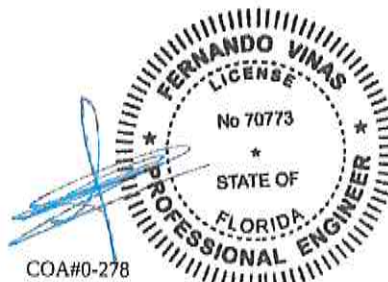
Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.

M - L 881 -2

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - N	239 -1230	P - G	196 -876
B - M	813 -35	L - H	440 0
C - M	450 0	L - I	813 -39
D - O	196 -874	I - K	239 -1230
O - P	196 -874		



01/10/2022

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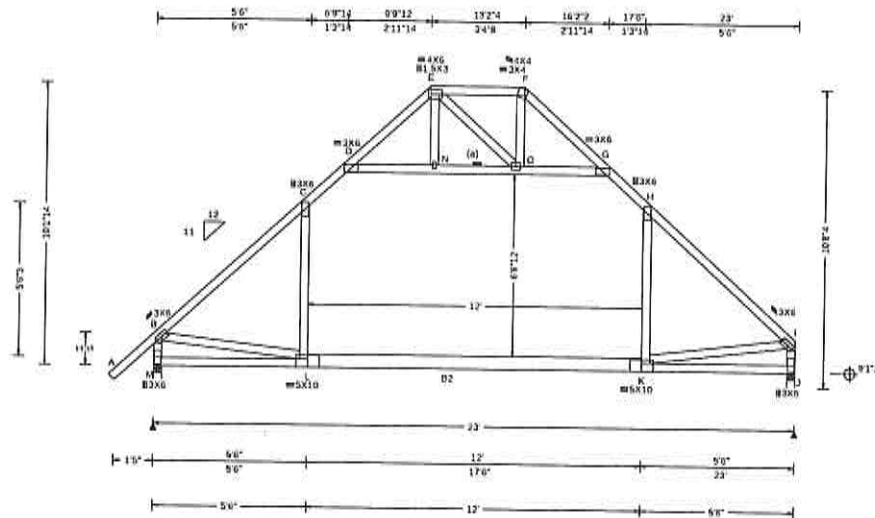
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97477 FROM: RJL	COMN Qty: 5	Ply: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C2a 23' Common	Cust: R 857 JRef: 1Xc38570001 T22 DrwNo: 010.22.1203.37440 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed, 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/H VERT(LL): 0.361 K 765 360 VERT(CL): 0.679 K 406 240 HORZ(LL): -0.350 H - - HORZ(TL): 0.659 H - - Creep Factor: 2.0 Max TC CSI: 0.985 Max BC CSI: 0.774 Max Web CSI: 0.312 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1241 /- /- /548 /42 /225 J 1145 /- /- /476 /28 /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 J Brg Wid = 3.5 Min Req = 1.5 Bearings M & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 202 -1340 G - H 275 -902 C - D 273 -903 H - I 194 -1334

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Collar-Tie braced with continuous lateral bracing @24" OC, or rigid sheathing

Special loads
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 59 plf at -1.61 to 59 plf at 23.00
PLB: From 40 plf at 5.50 to 40 plf at 17.50
BC: From 20 plf at 0.00 to 20 plf at 23.00

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp.
L - K 887 -53

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
B - M 241 -1236 O - G 205 -886
B - L 820 -36 K - H 436 0
C - L 454 0 K - I 803 -28
D - N 200 -884 I - J 169 -1135
N - O 200 -884



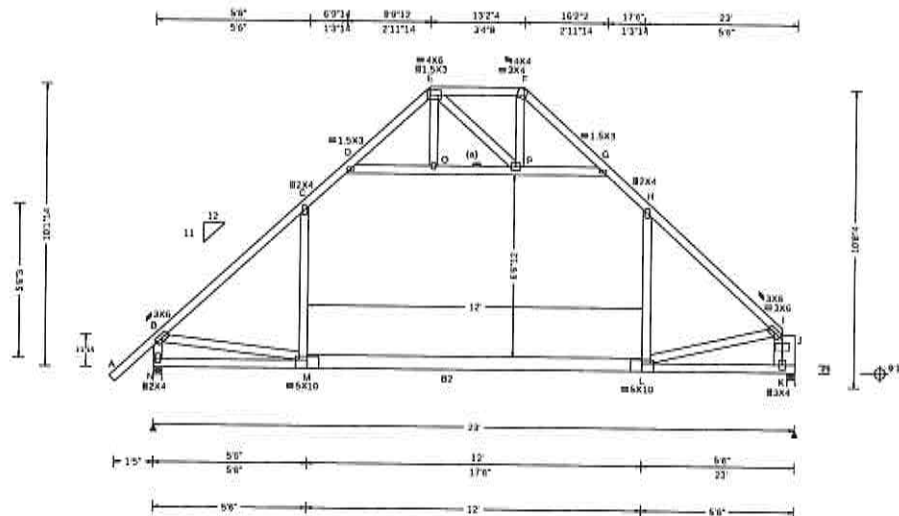
COA#0-278

01/10/2022

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SEQN: 97486 FROM: RJL	COMN Qty: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C2b 23' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T6 DrwNo: 010.22.1202.05937 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/H VERT(LL): 0.347 L 795 360 VERT(CL): 0.655 L 421 240 HORZ(LL): -0.337 H - - HORZ(TL): 0.637 H - - Creep Factor: 2.0 Max TC CSI: 0.967 Max BC CSI: 0.771 Max Web CSI: 0.534 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL N 1240 /- /- /548 /- /221 K 1119 /- /- /462 /- /- Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 K Brg Wid = 3.5 Min Req = 1.5 Bearings N & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x6 SP #1;

Bracing
(a) Continuous lateral restraint equally spaced on member.

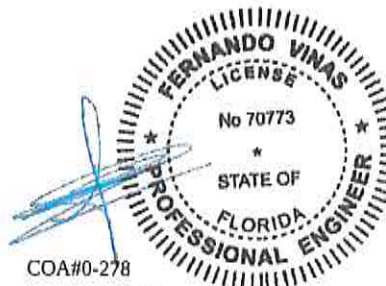
Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Collar-Tie braced with continuous lateral bracing @24" OC, or rigid sheathing
This truss has not been designed for habitation.
Truss not designed to be used as floor.
This truss must never be served by a fixed stairway.

Special loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 59 plf at -1.61 to 59 plf at 22.54
PLB: From 40 plf at 5.50 to 40 plf at 17.50
BC: From 20 plf at 0.00 to 20 plf at 23.00

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
M - L 885 -28 L - K 413 -30

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
B - N 149 -1235 P - G 110 -880
B - M 818 -34 L - H 440 0
C - M 452 0 L - I 485 -24
D - O 109 -880 I - J 97 -1112
O - P 109 -880 J - K 170 -1225

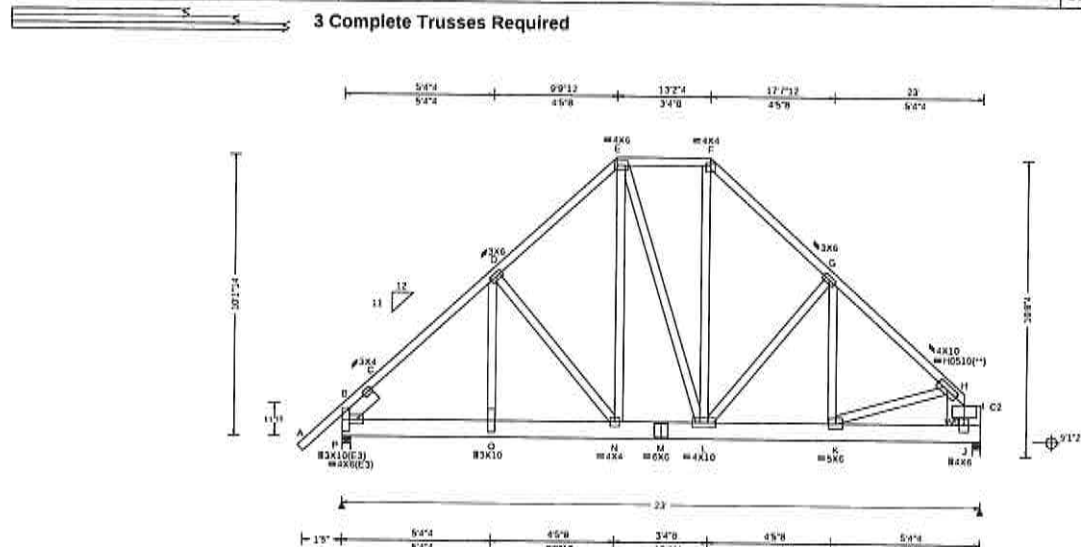


01/10/2022

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbaccomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
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6750 Forum Drive
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SEQN: 97502 FROM: RJL	HIPS Qty: 1	Ply: 3 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C3 23' Stepped Hip Girder	Cust: R 857 JRef: 1Xc39570001 T17 DrwNo: 010.22.1202.58910 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.058 N 999 360 VERT(CL): 0.111 N 999 240 HORZ(LL): 0.021 D - - HORZ(TL): 0.040 D - - Creep Factor: 2.0 Max TC CSI: 0.283 Max BC CSI: 0.217 Max Web CSI: 0.502 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL P 7758 -/- /- /366 -/ J 7653 -/- /- /352 -/ Wind reactions based on MWFRS P Brg Wid = 3.5 Min Req = 2.6 J Brg Wid = 3.5 Min Req = 2.6 Bearings P & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 129 -2728 E - F 69 -1533 C - D 126 -2710 F - G 100 -2086 D - E 99 -2062 G - H 125 -2657

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3; W9,C2 2x8 SP SS Dense;
Lt Slider: 2x6 SP #1; block length = 1.50'

Nailnote

Nail Schedule: 0.128"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.50" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

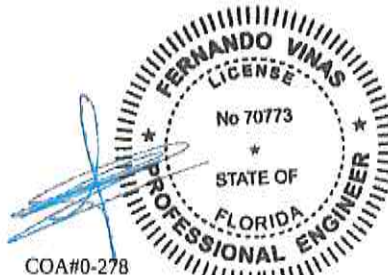
Girder supports 33-0-0 span to BC one face and 2-0-0 span to TC/BC split opposite face.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Blocking

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 5.50" if used)



COA#0-278

01/10/2022

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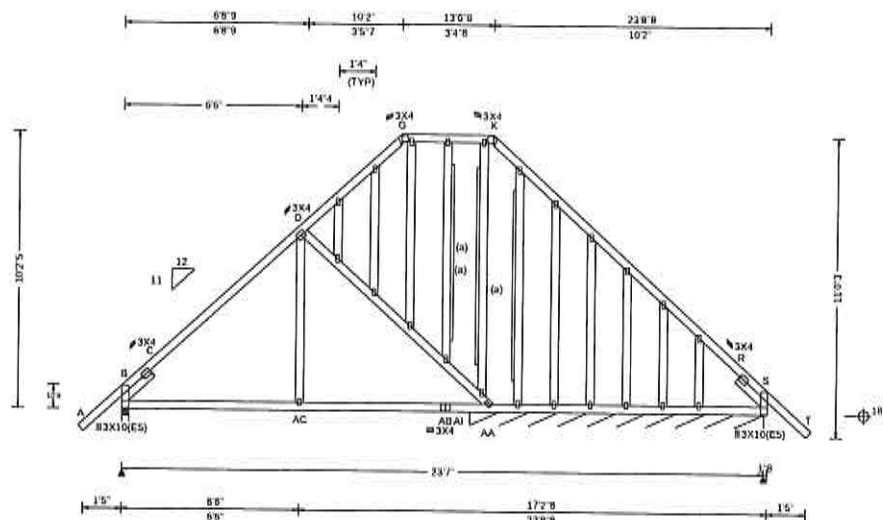
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6750 Forum Drive
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SEQN: 97515 FROM: RJL	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: D1-SG 23'8" Gable	Cust: R 857 JRef: 1Xc38570001 T20 DrwNo: 010.22.1203.13683 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.38 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.043 C 999 360 VERT(CL): 0.075 F 999 240 HORZ(LL): -0.050 C - - HORZ(TL): 0.062 C - - Creep Factor: 2.0 Max TC CSI: 0.658 Max BC CSI: 0.415 Max Web CSI: 0.190 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1103 /- /- /425 /103 /304 Al* 169 /- /- /76 /30 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 Al Brg Wid = 130 Min Req = - Bearings B & Al are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 290 -1268 G - K 191 -384 C - D 134 -1163 K - R 211 -613 D - G 206 -714 R - S 344 -693

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) 1x4 #3SRB SPF-S or better 'L' reinforcement.
80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

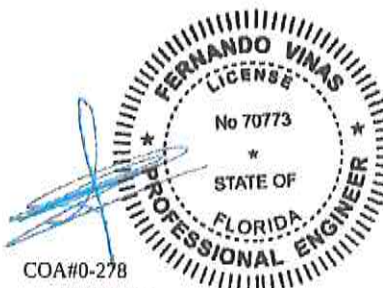
Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA#0-278

01/10/2022

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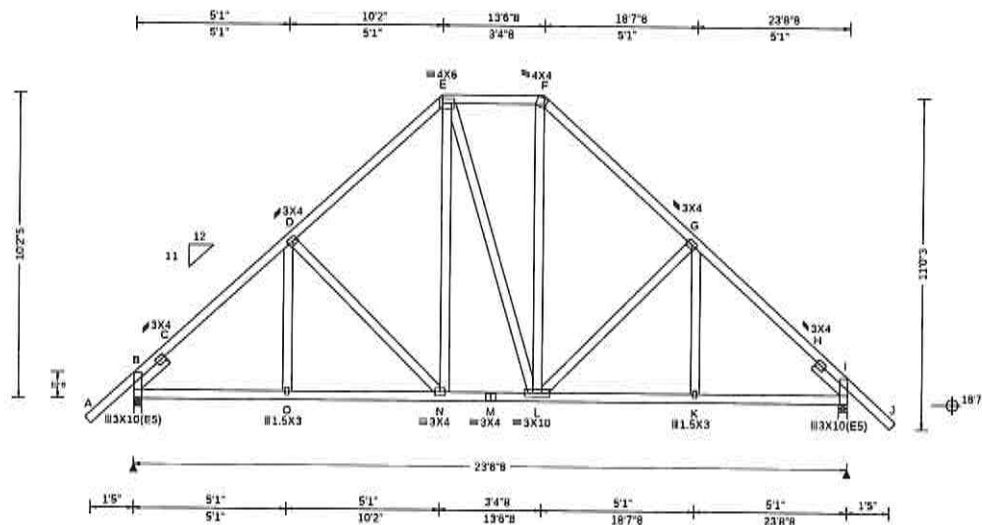
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97519 FROM: RJL	CONN Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: D2 23'8" Common	Cust: R 857 JRef: 1Xc38570001 T11 DrwNo: 010.22.1202.39363 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.38 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.025 N 999 360 VERT(CL): 0.049 N 999 240 HORZ(LL): 0.014 I - - HORZ(TL): 0.026 I - - Creep Factor: 2.0 Max TC CSI: 0.178 Max BC CSI: 0.217 Max Web CSI: 0.270 VIEW Ver: 21.01.03A.0805.15	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1026 /- /- /568 /49 /270 I 1026 /- /- /568 /49 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 I Brg Wid = 3.5 Min Req = 1.5 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 224 -1138 F - G 264 -856 C - D 193 -1093 G - H 192 -1093 D - E 266 -859 H - I 223 -1138 E - F 242 -556

Lumber

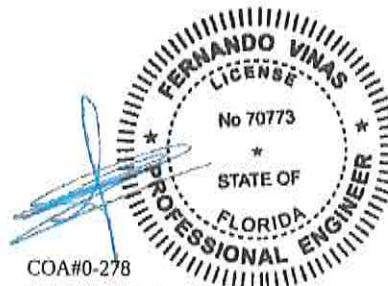
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
B - O 738 -92 M - L 555 -39
O - N 736 -92 L - K 735 -24
N - M 555 -39 K - I 738 -24



01/10/2022

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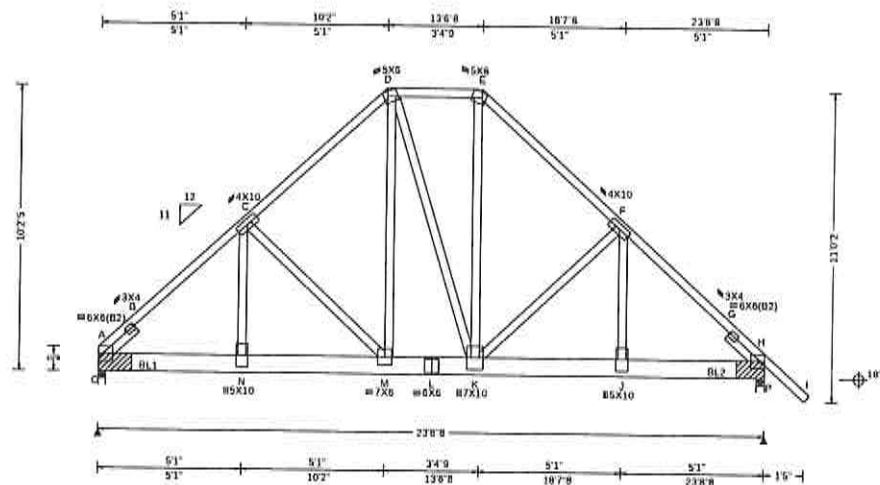
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97578 FROM: RJL	COMN Ply: 2 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: D3 23'8" Common Girder	Cust: R 657 JRef: 1Xc38570001 T9 DrwNo: 010.22.1202.36440 SSB / FV 01/10/2022
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.38 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCPl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Cs: NA Cg: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in Loc/Defl L/# VERT(LL): 0.106 M 999 360 VERT(CL): 0.195 M 999 240 HORZ(LL): 0.044 C - - HORZ(TL): 0.080 C - - Creep Factor: 2.0 Max TC CSI: 0.499 Max BC CSI: 0.469 Max Web CSI: 0.930 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 8801 /- /- /- /706 /- P 7020 /- /- /- /670 /- Wind reactions based on MWFRS O Brg Wid = 3.0 Min Req = - P Brg Wid = 3.5 Min Req = - Bearings O & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 425 -5064 E - F 349 -3790 B - C 415 -5035 F - G 427 -4637 C - D 334 -3747 G - H 437 -4668 D - E 250 -2807

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Nailnote

Nail Schedule: 0.126"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 5.00" o.c. (Each Row)
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 59 plf at 0.00 to 59 plf at 13.54
TC: From 29 plf at 13.54 to 29 plf at 17.52
TC: From 59 plf at 17.52 to 59 plf at 25.32
BC: From 10 plf at 0.00 to 10 plf at 17.52
BC: From 20 plf at 17.52 to 20 plf at 23.71
BC: 1408 lb Conc. Load at 1.52, 3.52, 5.52, 7.52, 9.52, 11.52
BC: 1603 lb Conc. Load at 13.52
BC: 1577 lb Conc. Load at 15.52
BC: 2526 lb Conc. Load at 17.52

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks: 0.126"x3" nails
brg x-loc #blocks length/blk #nails/blk wall plate
1 0.000' 1 14" 23 Rigid Surface
2 23.417' 1 12" 4 Rigid Surface
Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

Blocking

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)
Bearing 2 located at 23.4' (blocking >= 3.50" if used)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - N	3668 -296	L - K	2722 -231
N - M	3617 -293	K - J	3321 -299
M - L	2722 -231	J - H	3337 -301

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
N - C	1697 -69	K - E	2442 -196
C - M	88 -1271	K - F	74 -771
D - M	2125 -111	F - J	1064 -65

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA#0-278

01/10/2022

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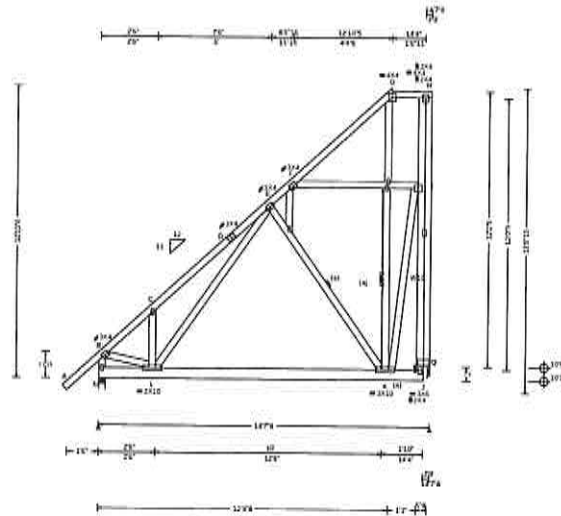
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SEQN: 97450 FROM: RJL	SPEC	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E1 14'7" Mono Hip	Cust: R 857 JRef: 1Xc38570001 T29 DrwNo: 010.22.1204.04047 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.018 G 999 360 VERT(CL): 0.031 G 999 240 HORZ(LL): 0.014 C - - HORZ(TL): 0.023 C - - Creep Factor: 2.0 Max TC CSI: 0.202 Max BC CSI: 0.300 Max Web CSI: 0.320 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 762 /- /- /379 /- /229 Q 668 /- /- /411 /88 /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 Q Brg Wid = 1.5 Min Req = 1.5 Bearings M & Q are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -829 D - E 26 -735 C - D 14 -876

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3; W10 2x4 SP #1;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

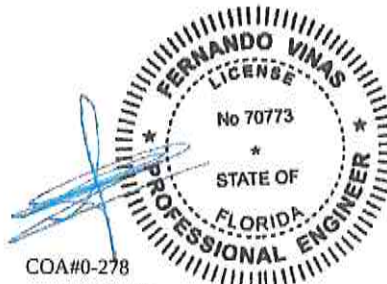
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



01/10/2022

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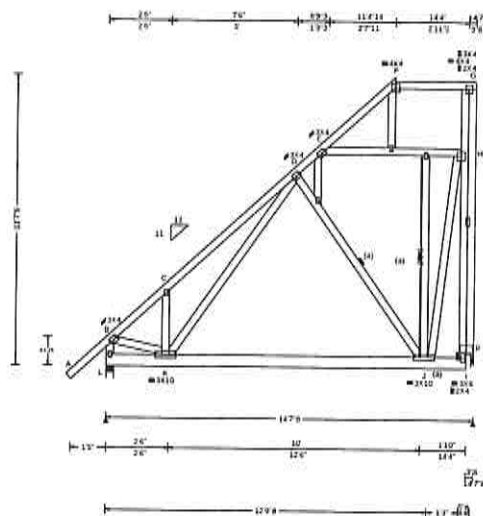
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97447 FROM: RJL	SPEC	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E2 147"8 Mono Hip	Cust: R 857 JRef: 1Xc38570001 T25 DrwNo: 010.22.1203.53943 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.74 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.031 F 999 360 VERT(CL): 0.053 F 999 240 HORZ(LL): 0.022 G - - HORZ(TL): 0.040 G - - Creep Factor: 2.0 Max TC CSI: 0.188 Max BC CSI: 0.300 Max Web CSI: 0.298 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 762 /- /- /389 /- /203 P 668 /- /- /376 /77 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 P Brg Wid = 1.5 Min Req = 1.5 Bearings L & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -829 C - D 67 -876

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

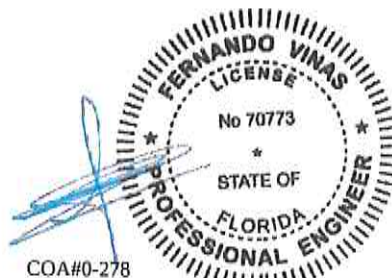
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



COA#0-278

01/10/2022

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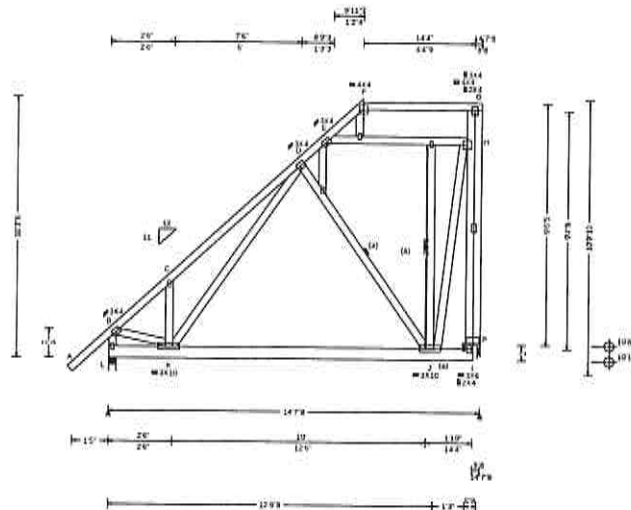
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97444 FROM: RJL	SPEC Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E3 14'7"8 Mono Hip	Cust: R 857 JRef: 1Xc38570001 T30 DrwNo: 010.22.1204.05293 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.031 F 999 360 VERT(CL): 0.059 F 999 240 HORZ(LL): 0.021 H - - HORZ(TL): 0.044 H - - Creep Factor: 2.0 Max TC CSI: 0.213 Max BC CSI: 0.299 Max Web CSI: 0.313 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 762 /- /- /394 /- /177 P 668 /- /- /345 /67 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 P Brg Wid = 1.5 Min Req = 1.5 Bearings L & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -828 C - D 103 -873

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

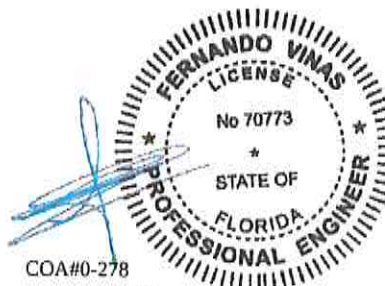
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)



COA#0-278

01/10/2022

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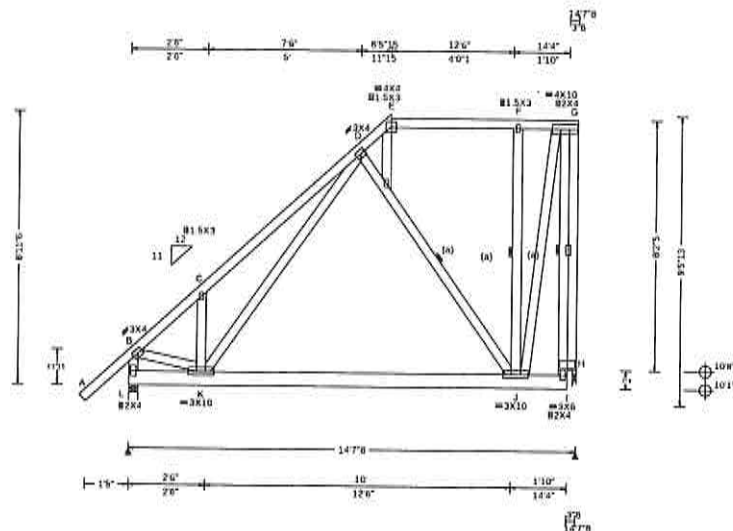
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97434 FROM: RJL	SPEC	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E4 14'7" Mono Hip	Cust: R 657 JRef: 1Xc36570001 T26 DrwNo: 010.22.1203.59653 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.015 C 999 360 VERT(CL): 0.026 C 999 240 HORZ(LL): 0.013 C - - HORZ(TL): 0.022 C - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.302 Max Web CSI: 0.371 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 762 /- /- /395 /- /153 H 668 /- /- /319 /60 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 H Brg Wid = 1.5 Min Req = 1.5 Bearings L & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 18 -831 C - D 157 -882

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

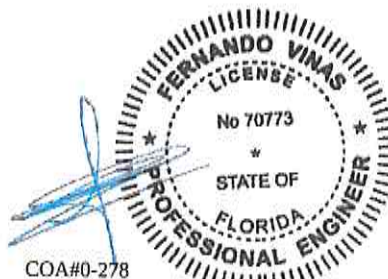
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - L	57 -866	J - G	837 -273
B - K	618 0	G - H	246 -710
K - D	458 -81		



COA#0-278

01/10/2022

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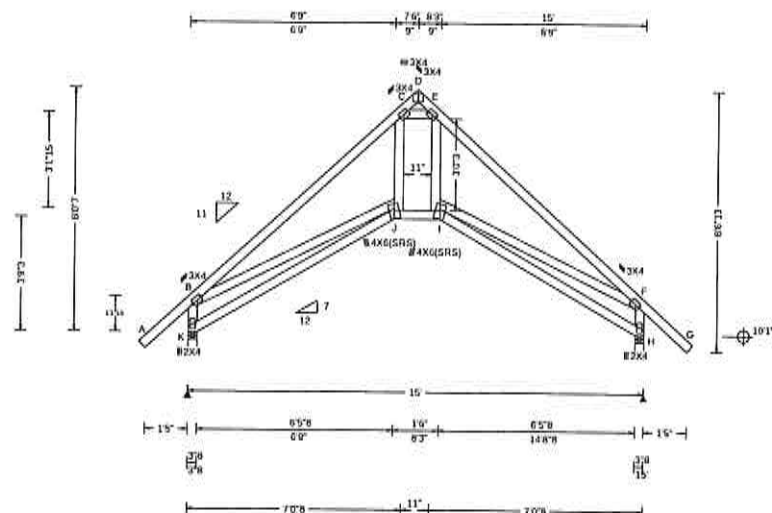
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97453 FROM: RJL	COMN	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G2 15' Common	Cust: R 857 JRef: 1Xc38570001 T32 DrwNo: 010.22.1204.12160 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.037 J 999 360 VERT(CL): 0.072 J 999 240 HORZ(LL): 0.045 H - - HORZ(TL): 0.087 H - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.306 Max Web CSI: 0.293 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh K 703 /- /- /390 /20 /199 H 703 /- /- /390 /20 /- Non-Gravity / Rw / U / RL Wind reactions based on MWFRS K Brg Wid = 3.5 Min Req = 1.5 H Brg Wid = 3.5 Min Req = 1.5 Bearings K & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 49 -1165 E - F 38 -1165

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

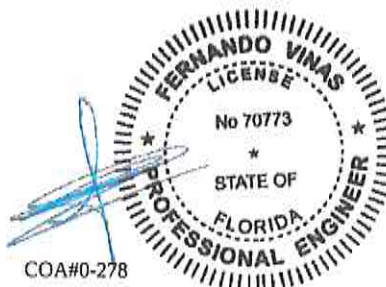
Chords Tens.Comp.

J - I 742 -60

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

B - K 261 -668 E - I 455 -54
B - J 768 0 I - F 768 -96
J - C 455 0 H - F 252 -668
C - E 619 -1144



01/10/2022

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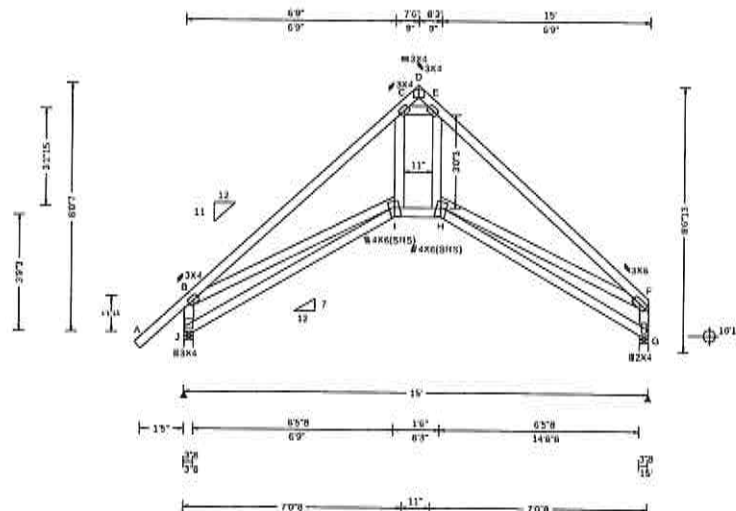
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97461 FROM: RJL	COMN	Ply: 1 Qty: 7	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G2a 15' Common	Cust: R 057 JRef: 1Xc38570001 T12 DrwNo: 010.22.1202.41057 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.038 H 999 360 VERT(CL): 0.076 H 999 240 HORZ(LL): 0.045 G - - HORZ(TL): 0.089 G - - Creep Factor: 2.0 Max TC CSI: 0.426 Max BC CSI: 0.309 Max Web CSI: 0.299 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 708 - / - / 391 / 20 / 180 G 609 - / - / 324 / 6 / - Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 3.5 Min Req = 1.5 Bearings J & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 158 - 1183 E - F 173 - 1176

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

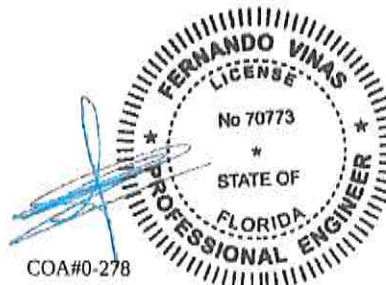
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.
I - H 755 - 10

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - J	294 - 672	E - H	455 - 22
B - I	786 0	H - F	766 - 15
I - C	461 0	G - F	182 - 582
C - E	744 - 1178		



COA#0-278

01/10/2022

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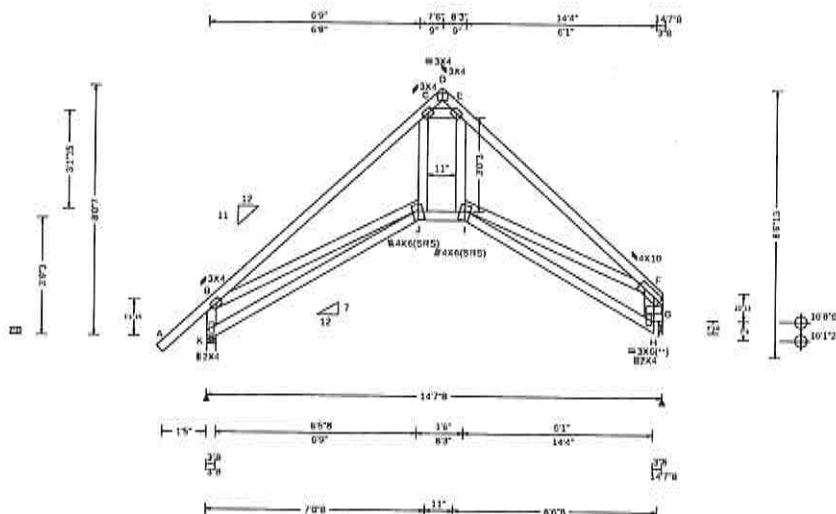
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97465 FROM: RJL	SPEC	Ply: 1 Qty: 4	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G3 14'7"8 Special	Cust: R 857 JRef: 1Xc38570001 T27 DrwNo: 010.22.1204.01473 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCPI: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.037 J 999 360 VERT(CL): 0.073 J 999 240 HORZ(LL): 0.038 H - - HORZ(TL): 0.075 H - - Creep Factor: 2.0 Max TC CSI: 0.418 Max BC CSI: 0.315 Max Web CSI: 0.273 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K 690 - / - / 382 / 20 / 177 G 589 - / - / 321 / 7 / - Wind reactions based on MWFRS K Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 1.5 Min Req = 1.5 Bearings K & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 56 -1109 E - F 95 -1102

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

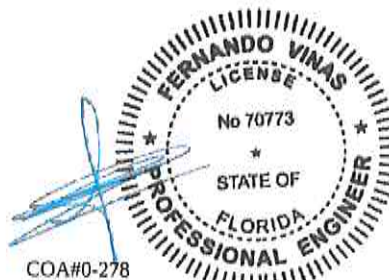
Chords	Tens.Comp.
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J - I 701 0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
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B - K 174 -654 E - I 442 -9
B - J 716 0 I - F 523 -88
J - C 443 0 F - G 478 -1131
C - E 427 -1044



COA#0-278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

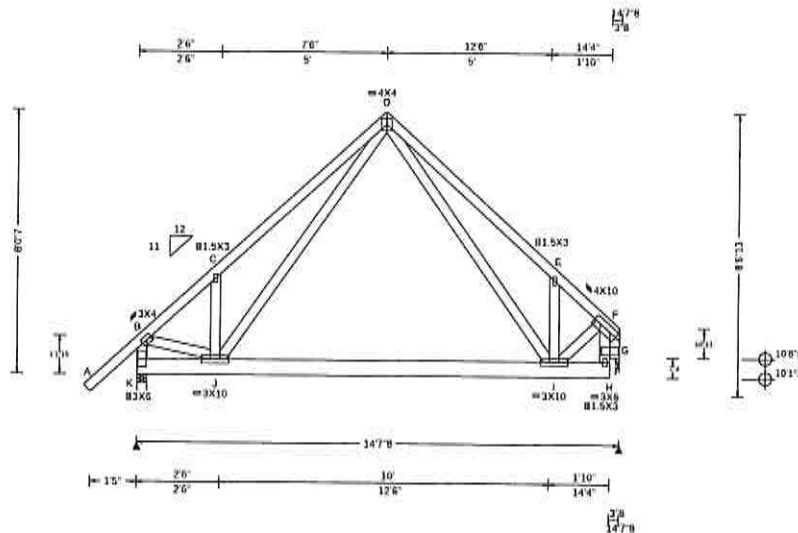
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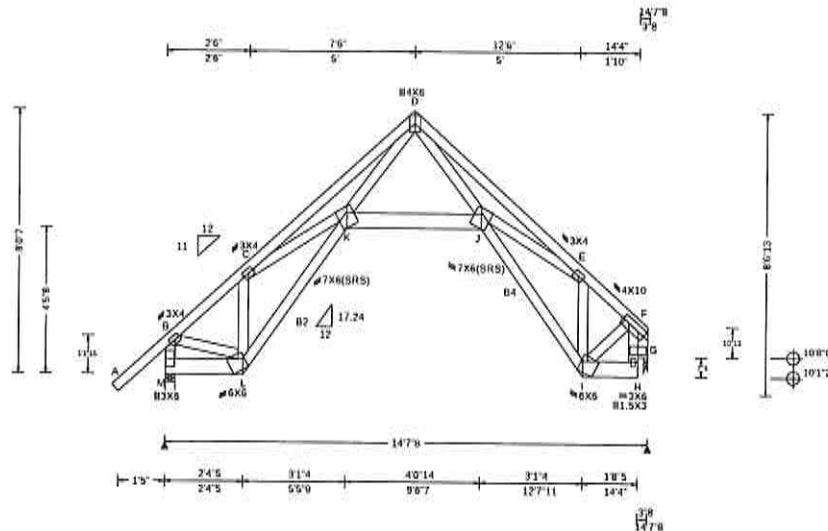
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCE: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97431 FROM: RJL	SPEC Ply: 1 Qty: 6	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G4 14'7"8 Special	Cust: R 857 JRef: 1Xc38570001 T45 DrwNo: 010.22.1204.24477 SSB / FV 01/10/2022
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SEQN: 97429 FROM: RJL	SPEC Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G4a 14'7"8 Special	Cust: R 857 JRef: 1Xc38570001 T1 DrwNo: 010.22.1200.03050 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.101 K 999 360 VERT(CL): 0.208 K 841 240 HORZ(LL): 0.176 H - - HORZ(TL): 0.362 H - - Creep Factor: 2.0 Max TC CSI: 0.304 Max BC CSI: 0.226 Max Web CSI: 0.572 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 714 /- /- /416 /4 /177 G 618 /- /- /346 /- /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 1.5 Min Req = 1.5 Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
				B - C 61 -657 D - E 0 -2120 C - D 0 -2174 E - F 41 -544

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1; B2,B4 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Wind

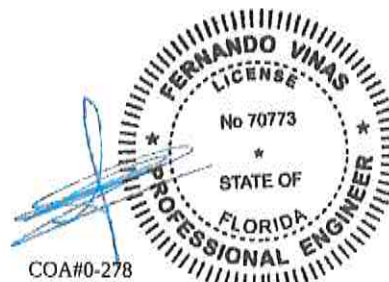
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
L - K	821 -105	J - I	740 -64
K - J	714 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - M	87 -710	D - J	1427 0
B - L	473 0	J - E	1206 0
L - C	105 -667	E - I	98 -707
C - K	1200 0	I - F	398 -29
K - D	1501 0	F - G	9 -577



01/10/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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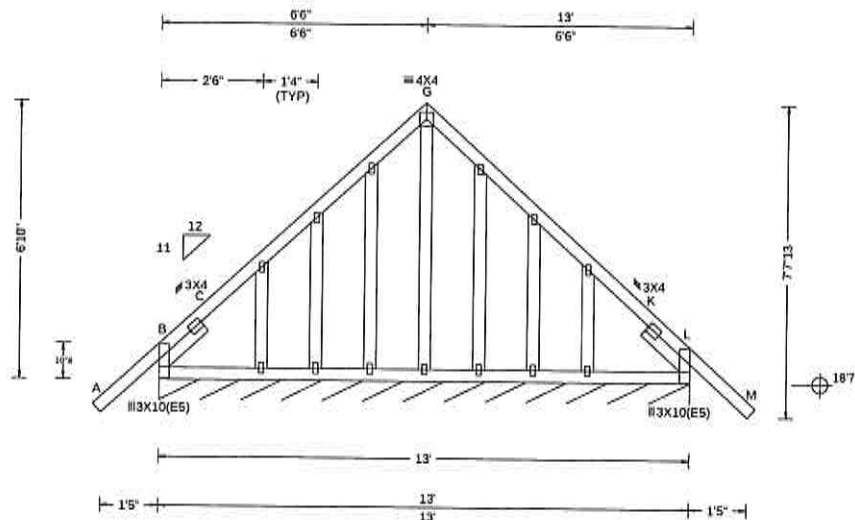
Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97512 FROM: RJL	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: H1-G 13' Gable	Cust: R 857 JRef: 1Xc38570001 T15 DrwNo: 010.22.1202.55960 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pl in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.70 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 5.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.002 K 999 360 VERT(CL): 0.004 K 999 240 HORZ(LL): -0.005 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.140 Max BC CSI: 0.030 Max Web CSI: 0.152 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L* 111 /- /- /50 /18 /17 Wind reactions based on MWFRS L Brg Wid = 156 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

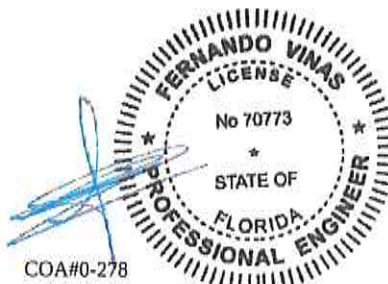
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA#0-278

01/10/2022

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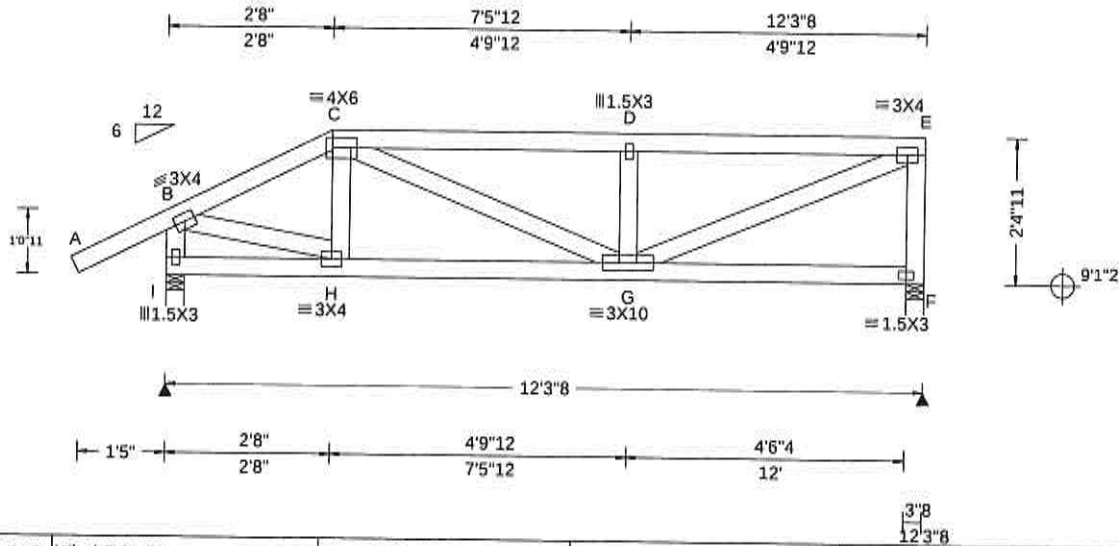
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97506 FROM: RJL	HIPM Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: K1 12'3" Mono Hip Girder	Cust: R 857 JRef: 1Xc38570001 T35 DrwNo: 010.22.1204.17843 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,PI in PSF)	Defl/CSI Criteria	12'3"8 ▲ Maximum Reactions (lbs)						
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.016 D 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.030 D 999 240	I	531	/-	/-	/-	/32	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 C - -	F	401	/-	/-	/-	/4	/-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.007 C - -	Wind reactions based on MWFRS						
NCBCLL: 0.00	Mean Height: 10.43 ft	Building Code:	Creep Factor: 2.0	I Brg Wid = 3.5 Min Req = 1.5						
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.175	F Brg Wid = 3.5 Min Req = 1.5						
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.132	Bearings I & F are a rigid surface.						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.268	Members not listed have forces less than 375#						
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: NA	Plate Type(s):		Chords	Tens.Comp.		Chords	Tens. Comp.		
	GCpi: 0.18	WAVE	VIEW Ver: 21.01.03A.0805.15	B - C	6	-538	D - E	5	-654	
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

#1 hip supports 2-8-0 jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

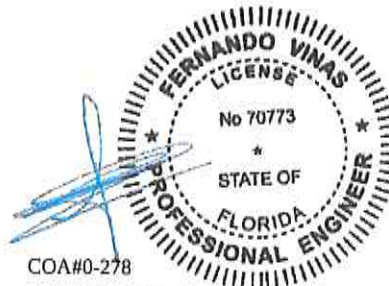
Chords Tens.Comp.

H - G 448 0

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

B - I 40 -514 G - E 703 -6
B - H 465 0



01/10/2022

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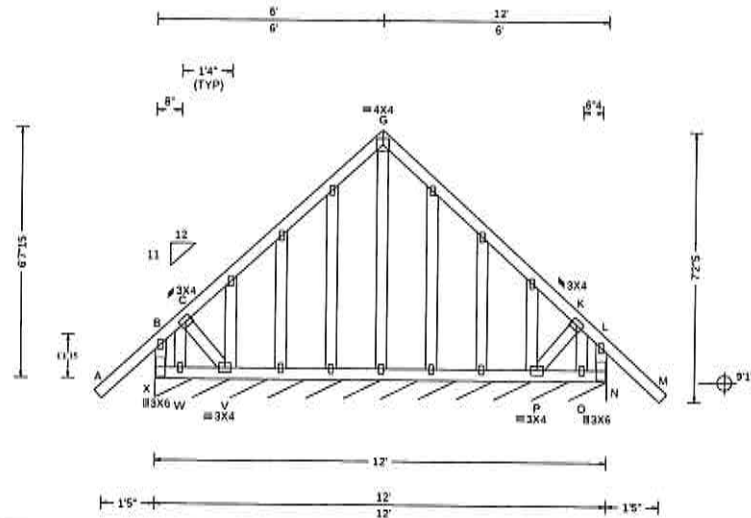
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97470 FROM: RJL	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: L1-G 12' Gable	Cust: R 857 JRef: 1Xc38570001 T2 DrwNo: 010.22.1200.04670 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 G 999 360 VERT(CL): 0.001 F 999 240 HORZ(LL): -0.003 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.179 Max BC CSI: 0.024 Max Web CSI: 0.195 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL X* 113 /- /- /51 /16 /16 Wind reactions based on MWFRS X Brg Wid = 144 Min Req = - Bearing X is a rigid surface. Members not listed have forces less than 375# Maximum Gable Forces Per Ply (lbs) Gables Tens. Comp. Gables Tens. Comp. X - B 398 - 239 K - O 277 - 435 W - C 242 - 433 L - N 397 - 236

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

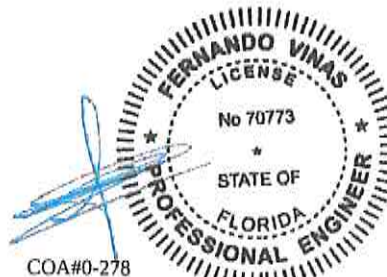
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA#0-278

01/10/2022

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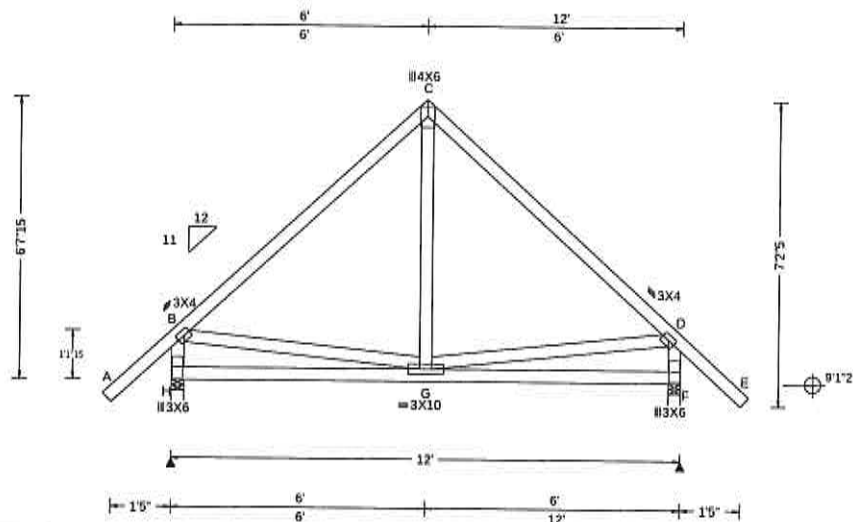
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97467 FROM: RJL	COMN Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: L2 12' Common	Cust: R 857 JRef: 1Xc38570001 T16 DrwNo: 010.22.1202.57447 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 G 999 360 VERT(CL): 0.009 G 999 240 HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.285 Max BC CSI: 0.237 Max Web CSI: 0.137 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 563 /- /- /315 /25 /170 F 563 /- /- /315 /25 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 F Brg Wid = 3.5 Min Req = 1.5 Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 166 -458 C - D 166 -458

Lumber

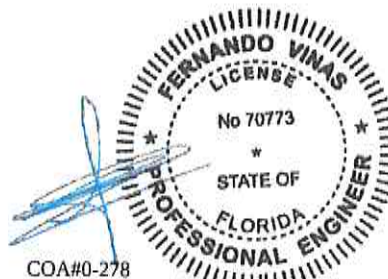
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	251 -515	D - F	251 -515



01/10/2022

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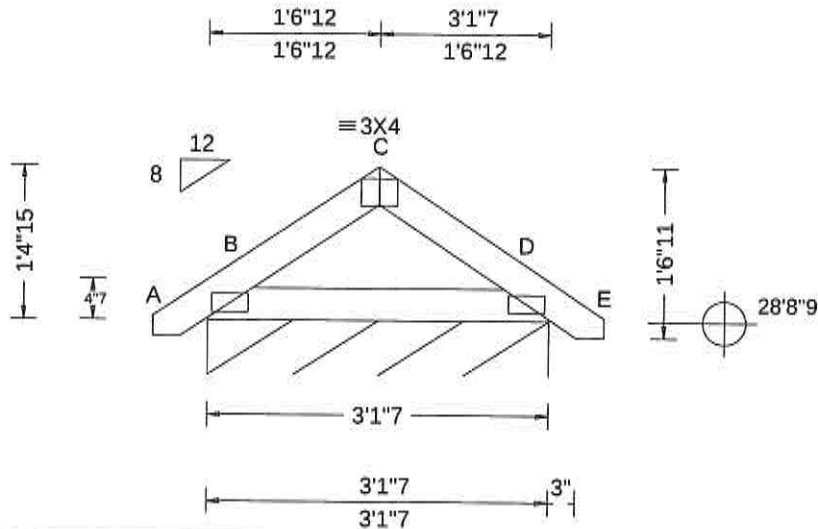
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97580 FROM: RJL	GABL Ply: 1 Qty: 12	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: PB1 4"1"14 Common	Cust: R 857 JRef: 1Xc38570001 T21 DrwNo: 010.22.1203.26433 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCCL: 7.00 BCLL: 0.00 BCCL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 29.44 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.015 Max BC CSI: 0.040 Max Web CSI: 0.000 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 95 /- /- /44 /24 /11 Wind reactions based on MWFRS B Brg Wid = 37.4 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

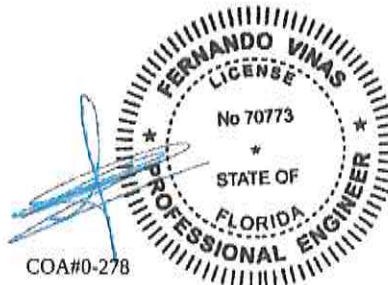
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Refer to drawing PB160160118 or piggyback detail.



01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

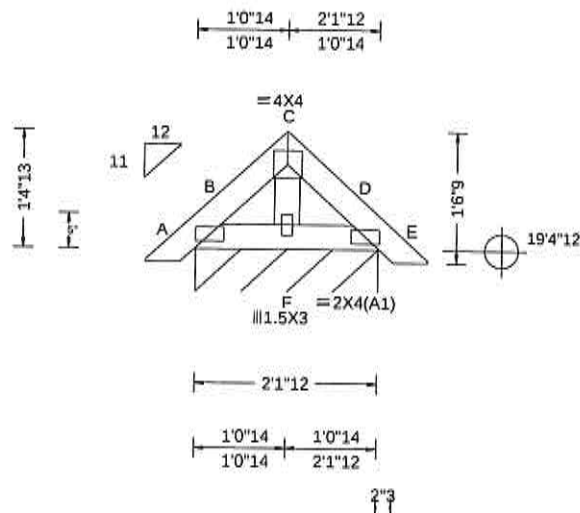
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pl in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pl: NA Ce: NA	VERT(LL): 0.000 C 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.000 C 999 240	B* 105 /- /- /55 /30 /20
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 B - -	Wind reactions based on MWFRS
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.000 B - -	B Brg Wld = 25.8 Min Req = -
NCBCLL: 10.00	Mean Height: 20.03 ft	Building Code:	Creep Factor: 2.0	Bearing B is a rigid surface.
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.015	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.005	
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Varies by Ld Case	Max Web CSI: 0.008	
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		
	Loc. from endwall: not in 10.00 ft	Plate Type(s):		
	GCpl: 0.18	WAVE	VIEW Ver: 21.01.03A.0805.15	
	Wind Duration: 1.60			

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3:

All plates are 2X4(B1) except as noted.

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Refer to drawing PB160160118 or pldggyback detail.



01/10/2022

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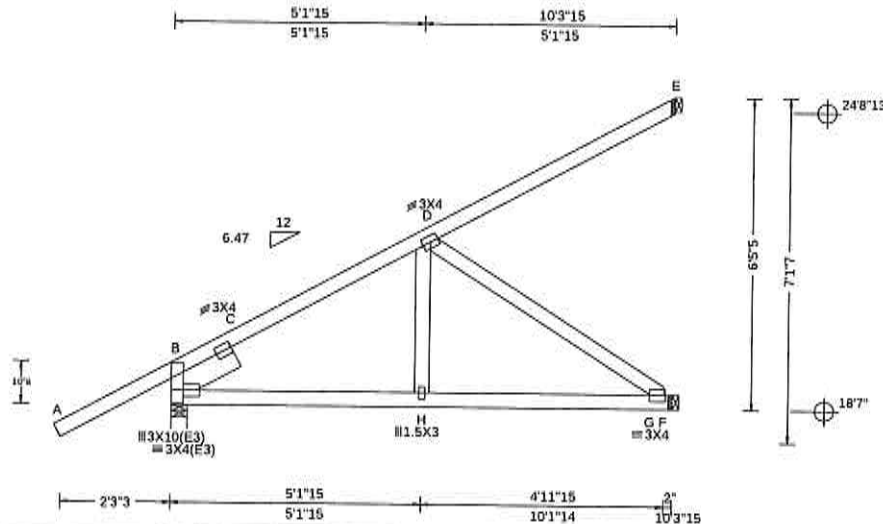
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6750 Forum Drive
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SEQN: 97536 FROM: RJL	HIP_	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JA 10'3"15 Hip Jack Girder	Cust: R 857 JRef: 1Xc38570001 T7 DrwNo: 010.22.1202.20423 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCCL: 20.00 TCDL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.59 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.023 C 999 360 VERT(CL): -0.043 C 999 240 HORZ(LL): -0.020 C - - HORZ(TL): 0.038 C - - Creep Factor: 2.0 Max TC CSI: 0.554 Max BC CSI: 0.340 Max Web CSI: 0.369 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 449 /- /- /- /71 /- F 423 /- /- /- /4 /- E 225 /- /- /- /77 /- Wind reactions based on MWFRS B Brg Wid = 4.1 Min Req = 1.5 F Brg Wid = 1.5 E Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP #1; block length = 1.50'

Loading

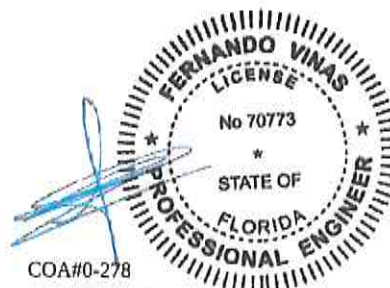
Hipjack supports 7-3-10 setback jacks. Jacks up to 7' have no webs. Longer jacks supported to BC.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
B - H 429 -51 H - G 425 -52

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp.
D - G 63 -516



COA#0-278

01/10/2022

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

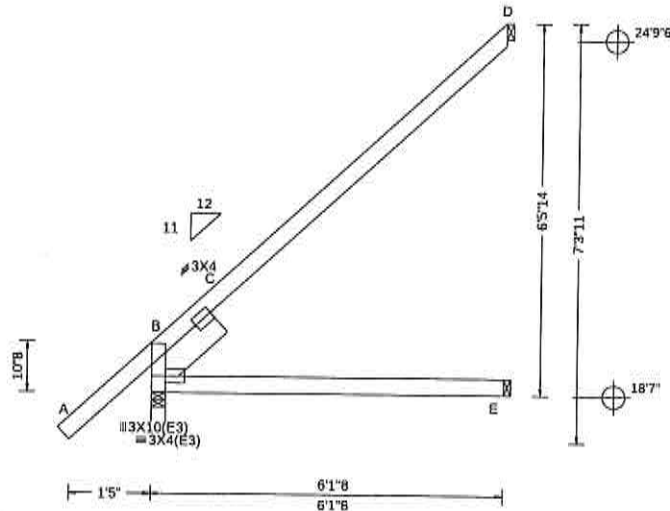
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCE) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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SEQN: 97538 FROM: RJL	EJAC Qty: 15	Ply: 1 Qty: 15	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JB 6'1"8 End Jack	Cust: R 857 JRef: 1Xc38570001 T57 DrwNo: 010.22.1204.38830 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, PI in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.53 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.058 C - - HORZ(TL): 0.107 C - - Creep Factor: 2.0 Max TC CSI: 0.487 Max BC CSI: 0.310 Max Web CSI: 0.070 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 342 /- /- /203 /- /208 E 117 /- /- /68 /- /- D 158 /- /- /118 /126 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 373 - 672

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

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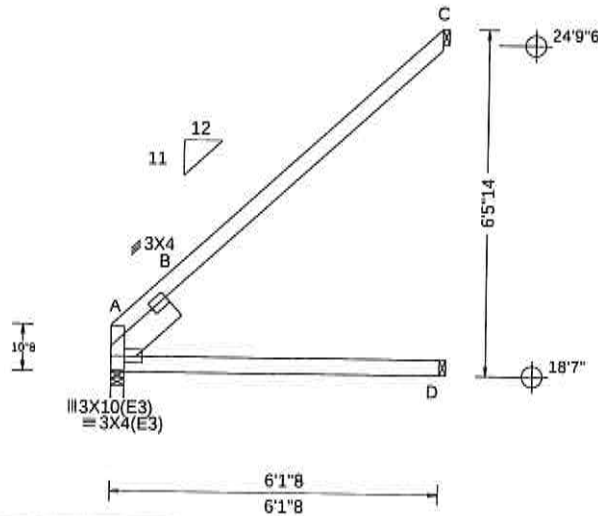
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Suite 305
Orlando FL, 32821

SEQN: 97540 FROM: RJL	EJAC	Ply: 1 Qty: 7	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JC 6'1"8 End Jack	Cust: R 857 JRef: 1Xc38570001 T10 DrwNo: 010.22.1202.37880 SSB / FV 01/10/2022
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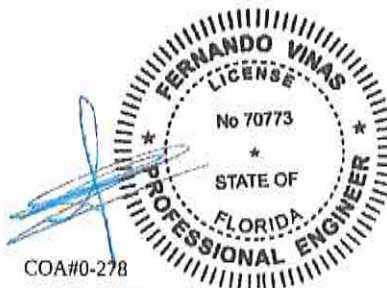
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.27 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCPl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.066 B - - HORZ(TL): 0.130 B - - Creep Factor: 2.0 Max TC CSI: 0.523 Max BC CSI: 0.317 Max Web CSI: 0.093 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 242 /- /- /120 /- /135 D 119 /- /- /72 /- /- C 165 /- /- /125 /96 /- Wind reactions based on MWFRS A Brg Wid = 3.0 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 611 -911

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

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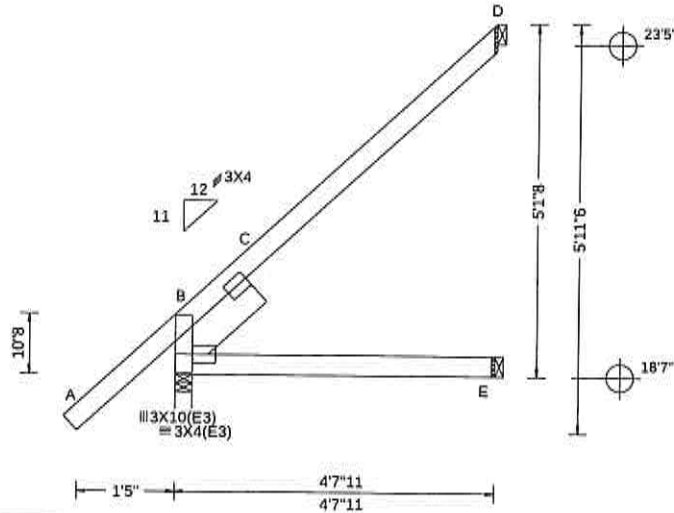
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SEQN: 97542 FROM: RJL	JACK Qty: 3	Ply: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JD 4'7"11 Jack	Cust: R 857 JRef: 1Xc38570001 T47 DrwNo: 010.22.1204.34600 SSB / FV 01/10/2022
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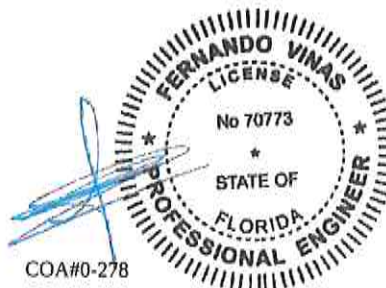
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.84 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.026 C - - HORZ(TL): 0.042 C - - Creep Factor: 2.0 Max TC CSI: 0.281 Max BC CSI: 0.168 Max Web CSI: 0.026 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 287 /- /- /176 /- /162 E 89 /- /- /50 /- /- D 116 /- /- /87 /94 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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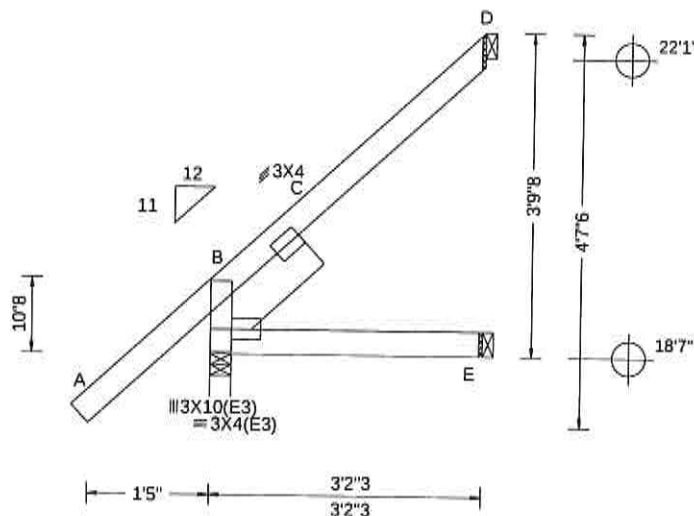
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Suite 305
Orlando FL, 32821

SEQN: 97544 FROM: RJL	JACK	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JE 3'2"3 Jack	Cust: R 857 JRef: 1Xc38570001 T48 DrwNo: 010.22.1204.35673 SSB / FV 01/10/2022
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	VERT(LL): NA	Gravity	Loc	R+ / R- / Rh	Non-Gravity					
TCDL: 7.00	Speed: 130 mph	Pf: NA	Ce: NA		VERT(CL): NA	HORZ(LL): -0.009 C - -			/ Rw / U / RL						
BCLL: 0.00	Enclosure: Closed	Lu: NA	Cs: NA		HORZ(TL): 0.011 C - -	Creep Factor: 2.0									
BCDL: 10.00	Risk Category: II	Building Code:				Max TC CSI: 0.173									
Des Ld: 37.00	EXP: B Kzt: NA	FBC 7th Ed. 2020 Res.				Max BC CSI: 0.073									
NCBCLL: 10.00	Mean Height: 20.18 ft	TPI Std: 2014				Max Web CSI: 0.010									
Soffit: 0.00	TCDL: 4.2 psf	Rep Fac: Yes													
Load Duration: 1.25	BCDL: 5.2 psf	FT/RT:20(0)/10(0)													
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Plate Type(s):													
	C&C Dist a: 3.00 ft	WAVE													
	Loc. from endwall: not in 4.50 ft														
	GCpi: 0.18														
	Wind Duration: 1.60														

B	236	/-	/-	/153	/-	/119
E	60	/-	/-	/34	/-	/-
D	71	/-	/-	/53	/63	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5
E Brg Wid = 1.5
D Brg Wid = 1.5

Bearing B is a rigid surface.
Members not listed have forces less than 375#

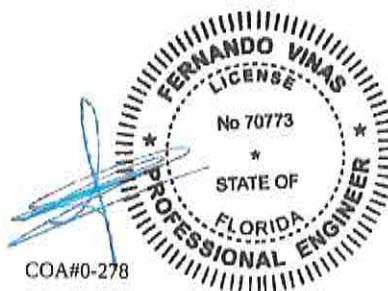
VIEW Ver: 21.01.03A.0805.15

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.50'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

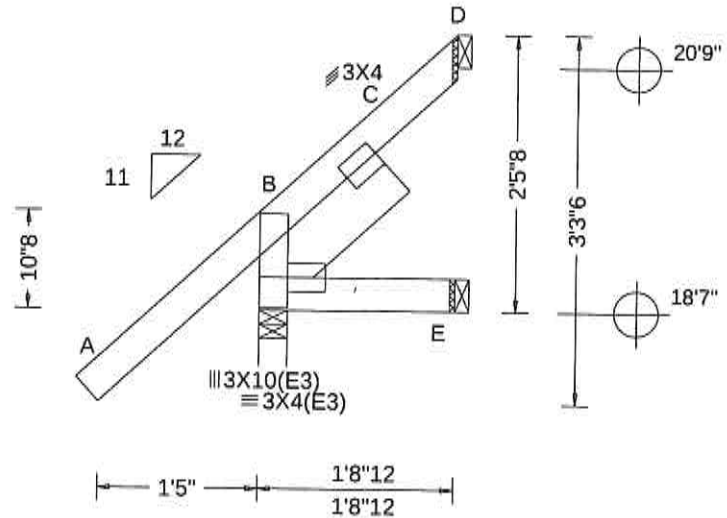
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

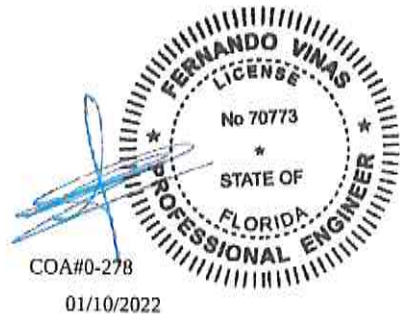
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Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.51 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed, 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.213 Max BC CSI: 0.019 Max Web CSI: 0.008 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 197 /- /- /140 /10 /78 E 33 /- /- /18 /- /- D 12 /- /- /28 /30 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

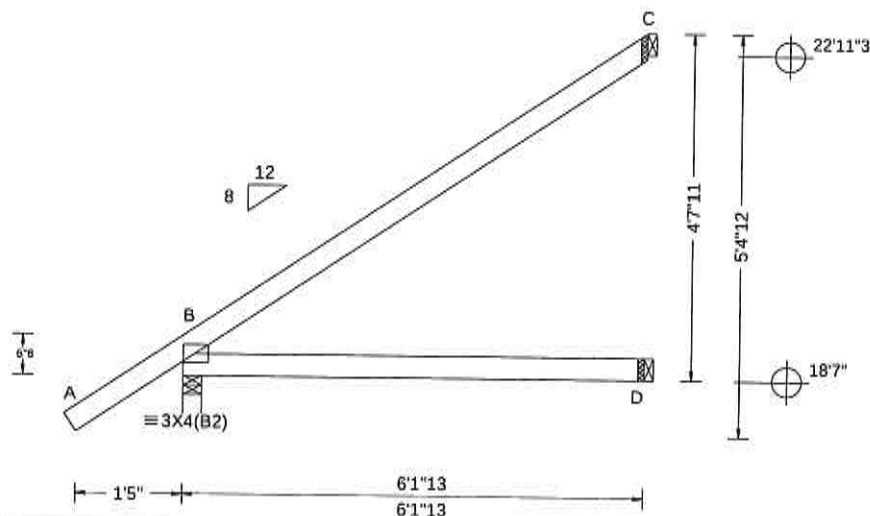
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 97548 FROM: RJL	JACK Qty: 3	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JG 6'1"13 Jack	Cust: R 857 JRef: 1Xc39570001 T43 DrwNo: 010.22.1204.22800 SSB / FV 01/10/2022
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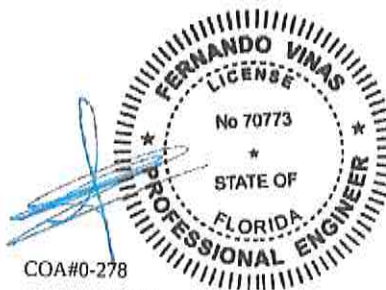
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.65 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.007 B - - HORZ(TL): 0.012 B - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.304 Max Web CSI: 0.000 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 336 /- /- /206 /11 /147 D 116 /- /- /62 /- /- C 155 /- /- /89 /89 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org

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▲ Maximum Reactions (lbs)						
	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	240	/-	/-	/155	/18	/89
D	63	/-	/-	/35	/-	/-
C	75	/-	/-	/40	/45	/-

Wind reactions based on MWFRS

B Brg Wid = 3.0 Min Req = 1.5

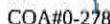
D Brg Wid = 1.5

C Brg Wid = 1.5

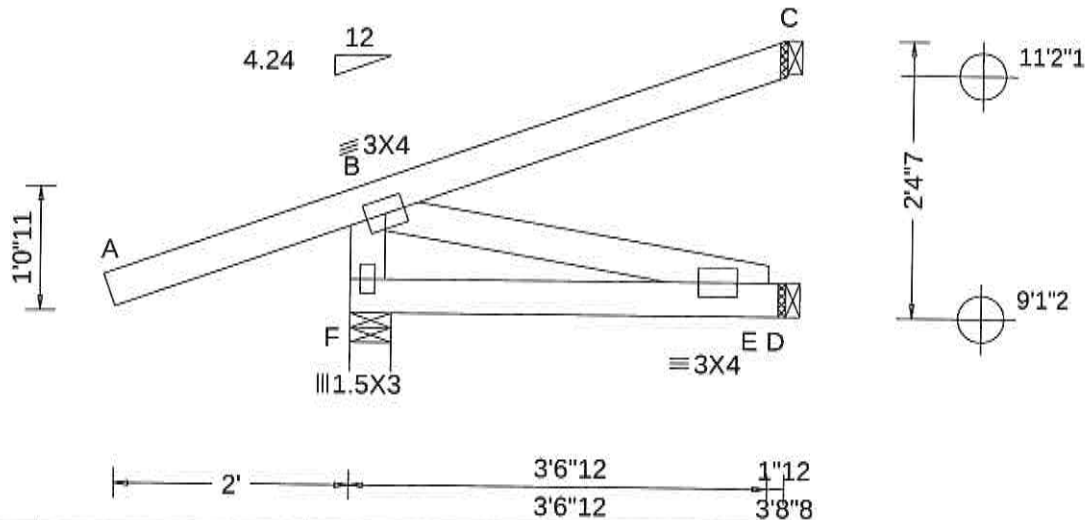
Bearing B is a rigid surface.

Members not listed have forces less than 375#

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 E 999 360 VERT(CL): 0.001 E 999 240 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.064 Max BC CSI: 0.025 Max Web CSI: 0.007 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 94 /- /- /- /11 /- D 20 /- /- /10 /- /- C 48 /- /- /- /13 /- Wind reactions based on MWFRS F Brg Wid = 4.2 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing F is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

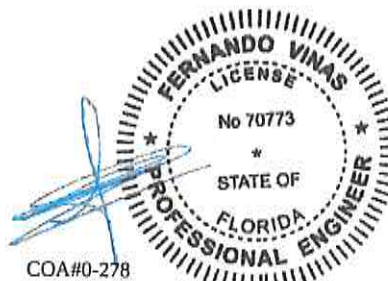
Hipjack supports 2-7-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



01/10/2022

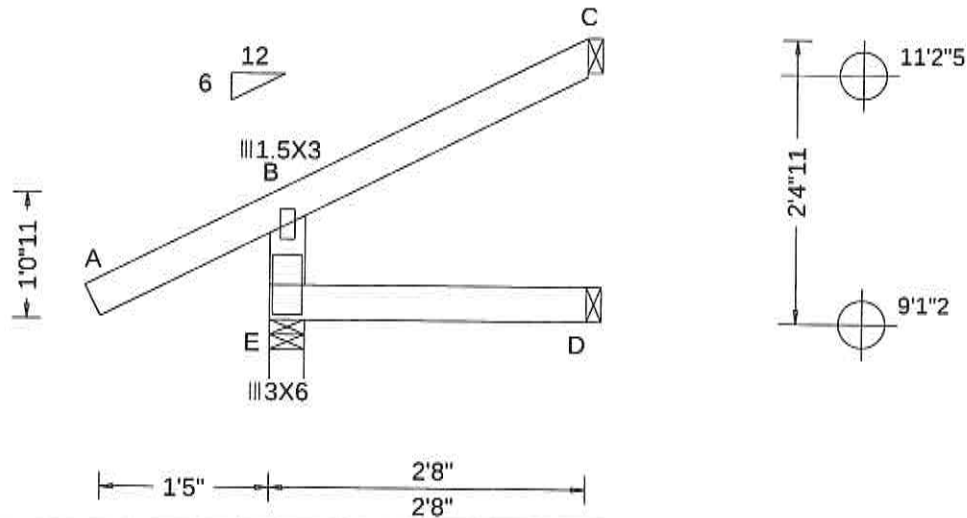
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 B 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.000 B 999 240	E 206 /- /- /144 /41 /-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 B - -	D 53 /- /- /27 /1 /-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.000 B - -	C 51 /- /- /28 /11 /46
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.144	E Brg Wid = 3.5 Min Req = 1.5
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.055	D Brg Wid = 1.5
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.090	C Brg Wid = 1.5
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Bearing E is a rigid surface.
	Loc. from endwall: Any	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.03A.0805.15	

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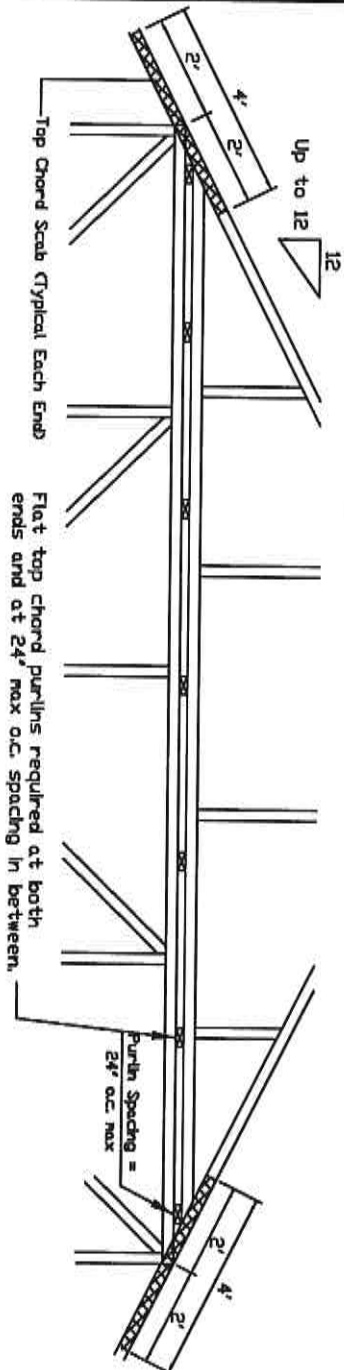
Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL= 50 psf (gh), Kzt=1.0, Dr 140 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp B, wind DL= 50 psf (gh), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends. Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

See Refer to Engineer's sealed design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

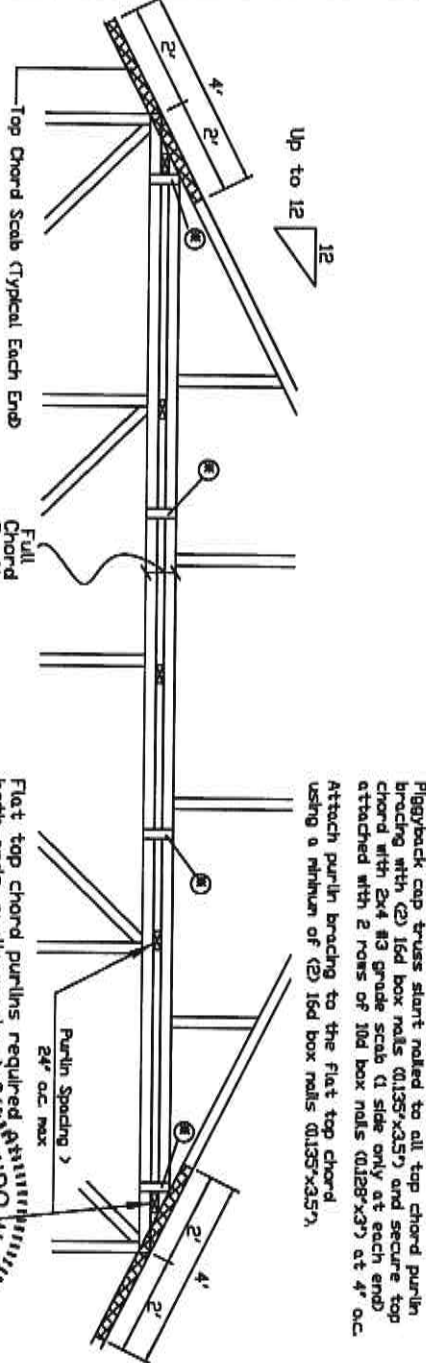


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.57") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.57").

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3x8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (2) 2x8 wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.57") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.57").

■ In addition, provide connection with one of the following methods:

Trulox
Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gussset
8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach 8' o.c. with (8) 0.13"x2" nails per gussset. (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach 8' o.c. with (8) 10d box nails (0.128"x3") per scab. (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

2x8 Wave Piggyback Plate
One 2x8 wave piggyback plate to each face 8' o.c. Attach teeth to piggyback at top of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.



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Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

Flat top chord purlins required at both ends, purlin spacing > 24' o.c. max.

ENGINEERING FLOORING TRUSS BRACING TO ALL CONSTRUCTION INCLUDING THE DETAILS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, wind DL= 50 psf (gh), Kzt=1.0, Dr 140 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp B, wind DL= 50 psf (gh), Kzt=1.0. Trusses shall have properly attached structural sheathing and bottom chord shall have bracing installed per ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, wind DL= 50 psf (gh), Kzt=1.0. Trusses shall be installed as shown above and on the left details unless noted otherwise. Above, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from the drawings, any failure to build the truss in accordance with ASCE 7-16, or for handling, shipping, installing or bracing of trusses. A seal on this drawing is the seal of the Engineer of Record. The Engineer of Record shall be responsible for any structure in the responsibility of the design engineer per ASCE 7-16, Section 1.6. For more information see the job's general notes page and these web sites: ALPINE@alpineinc.com, ITV@itvbuilding.com, TRUSS@trusscomponents.com, ITD@itdinc.com



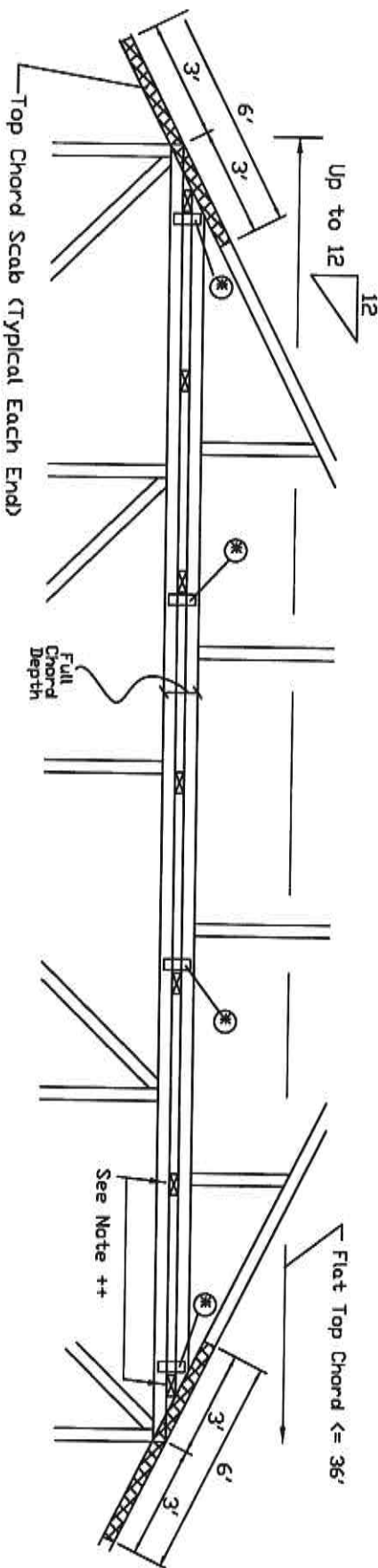
COA#0-278

SPACING 24.0"

REF PIGGYBACK
DATE 01/02/2018
DRWG PB160160118

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part: Enclosed Bldg, located anywhere in roof, Exp C, Wind II = 5.0 psf (min), Kzt=1.0.
 Dr 180 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part: Enclosed Bldg, located anywhere in roof, Exp II, Wind II = 5.0 psf (min), Kzt=1.0.
 Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.
 Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.
 Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

In addition, provide connection with one of the following methods:

Trulox Use 3x8 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28P3 Wave Piggyback Plate Use 28P3 wave piggyback plate to each face @ 8' o.c. Attach each to piggyback at top of (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gussset 8"x8"x1/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, Full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ICC Building Component Safety Information, by ITI and ICC for safety instructions prior to performing these functions. Installers shall provide temporary bracing per ICC. Trusses shall have a properly attached and properly attached structural sheathing and bottom chord shall have bracing installed per ICC sections 301.17 or 301.18. Trusses shall be installed in accordance with the manufacturer's instructions and on the joint details, unless noted otherwise. Refer to drawings 100-2 for standard plate positions.

Alpine, a division of ITI Building Components Group, Inc. shall not be responsible for any deviation from these instructions. Any failure to build the truss in accordance with these instructions shall be the responsibility of the building designer per ASCE 7-16, Section 1.5. For more information see the Job's general notes page and those with other drawings. For more information see the Job's general notes page and those with other drawings.



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REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118

COA#0-278

SPACING 24.0"

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face.
Minimum side member lengths = $(2 \times L) + (B)$

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered. Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

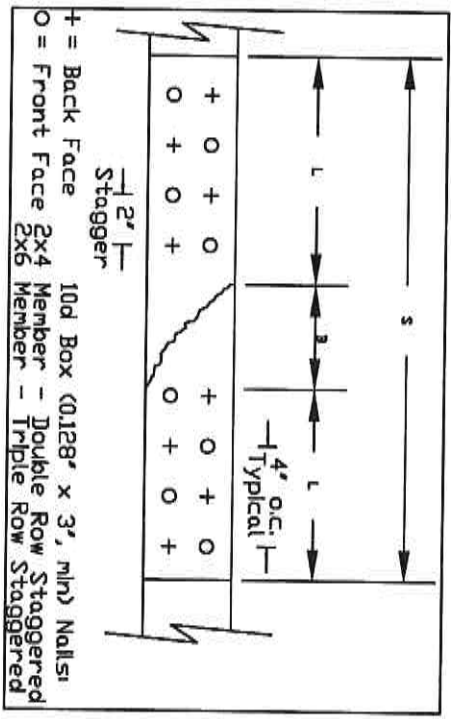
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650F.

This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

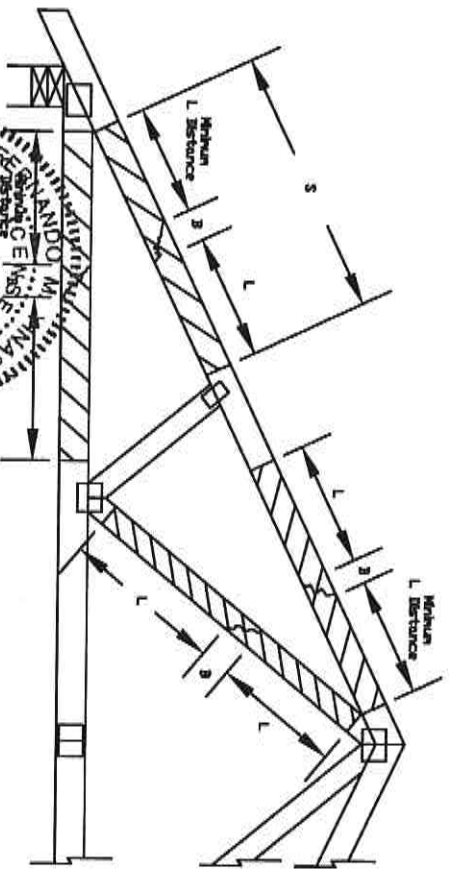
Broken chord may not support any tie-in loads.



Nail Spacing Detail

Member	Size	L	Maximum Member Axial Force			
			SPF-C	HF	DF-L	SYP
Web Only	2x4	12'	620#	635#	730#	800#
Web Only	2x4	18'	975#	1055#	1295#	1415#
Web or Chord	2x4	24'	975#	1055#	1495#	1745#
Web or Chord	2x6	24'	1465#	1585#	2245#	2620#
Web or Chord	2x4	30'	1910#	1960#	2315#	2555#
Web or Chord	2x6	30'	2230#	2365#	3125#	3575#
Web or Chord	2x4	36'	2470#	2530#	2930#	3210#
Web or Chord	2x6	36'	3535#	3635#	4295#	4745#
Web or Chord	2x4	42'	2975#	3045#	3505#	3835#
Web or Chord	2x6	42'	4395#	4500#	5225#	5725#
Web or Chord	2x4	48'	3460#	3540#	4070#	4445#
Web or Chord	2x6	48'	5165#	5280#	6095#	6660#

Load Duration = 0%
Member forces may be increased for Duration of Load



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Bart, CT, 06015

MANUFACTURER REPAIR AND FOLLOWING ALL NOTES ON THIS DRAWING.

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of BC21 Building Component Safety Information, by ITI and SCSO for safety practices prior to performing these functions. Installers shall provide temporary bracing per BC21. Trusses shall have a properly attached structural sheathing and bottom chord shall have bracing installed per BC21 section 32.37 or 32.38 as applicable. Trusses shall be installed in accordance with the manufacturer's instructions. Refer to drawings 1004-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from these drawings. Any failure to build the truss in accordance with AWS/P1.1, or for handling, shipping, installing, or bracing, shall be the responsibility of the truss manufacturer. The liability and use of this drawing for any structure is the responsibility of the Building Designer per AWS/P1.1 Section 1.2. For more information see the job's general notes page and these web sites: ALPINE: www.alpine.com ITI: www.iti.com SCSO: www.scsocorp.com ITD: www.itdcorp.com



COA#0-278

REF MEMBER REPAIR	
DATE	10/01/14
DRWG	REPCHRD1014
SPACING	24.0" MAX

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, $K_z = 1.00$

Dr1	120 mph	Wind Speed, 15'	Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Dr2	120 mph	Wind Speed, 15'	Mean Height, Enclosed, Exposure D, Kzt = 1.00
Dr3	100 mph	Wind Speed, 15'	Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Bracing Group Species and Grades

Group A ₁		
Service-Pine-Fir #1 / #2 Standard #3 Stud Standard	Hein-Fir #2 Stud #3 Standard	Douglas Fir-Larch #3 Stud Standard
Southern Pinebeam #3 Stud Standard		

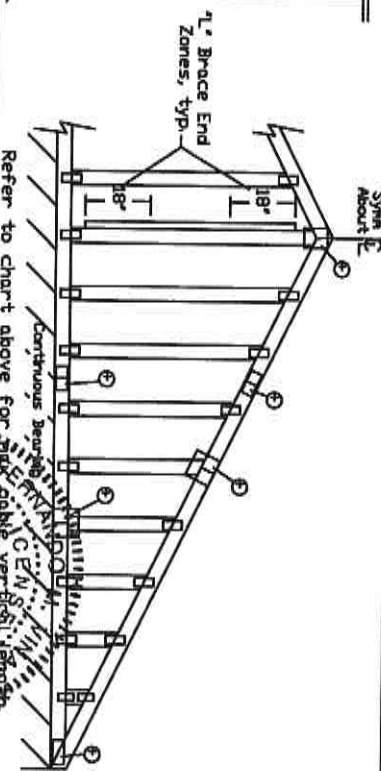
Group B ₁		
Hein-Fir #1 & Btr #1		
Douglas Fir-Larch #1 #2	Southern Pinebeam #1 #2	

1x4 Joists shall be SRS (Stress-Rated Board-
 major for 1x4 So. Pine use only Industrial SS or
 Industrial #3 Stress-Rated Boards. Group B
 values may be used with these grades.

Provide uplift connections for 55 pcf over continuous bearing (3 psf TC Dead Load).

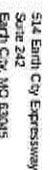
Attach L^1 braces with 10d (0.125" x 3.0" min) nails.
 * For (1) L^1 braces space nails at 2' o.c.
 In 18" end zones and 4' o.c. between zones.
 * For (2) L^1 braces space nails at 3' o.c.
 In 18" end zones and 6' o.c. between zones.
 L^1 bracing must be a minimum of 80% of web member length.

Refer to the Building Designer for conditions not addressed by this detail.



MANAGEMENT BEAD AND TITLING ALL NOTES IN THIS BEARING
IMPORTANT! FURNISH THIS BEARING TO ALL CONTRIBUTORS INCLUDING THE INSTALLERS.

These require active care in fabricating, handling, installing and bending. Refer to and follow the latest edition of BESS Guiding Component Safety Information, by IPI and BESS for safety instructions pertaining to performing these functions. Installers shall provide temporary bracing per BESS standards. A bracing plan shall be submitted to the project engineer for approval. Bracing shall have a properly attached end cap. Location shall be indicated on the bracing plan. Bracing shall be installed per BESS sections 10, 17 or 21B, as applicable. Apply plates to each face of the bead and position as shown above and on the joint bracing, unless noted otherwise.

[illegible]

COA#0-278

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0'

REF	ASCE7-16-GABI4015
DATE	01/26/2018
DRWG	A14015ENC160118

Sum of About

Gable Vertical Length Typ.

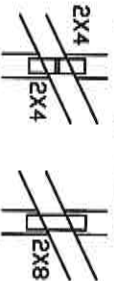
Examples

Refer to appendix plates for minimum plate sizes, welds, etc.

② If gable & single plate over the overhanging

- ➔ Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.
- ➔ Refer to Engineered truss design for peak, splice, web, and heel plates.

Example:



Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

T' reinforcing member material must match size, specie, and grade of the L' reinforcing member.

T' Relnf. Mbr. Size	T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

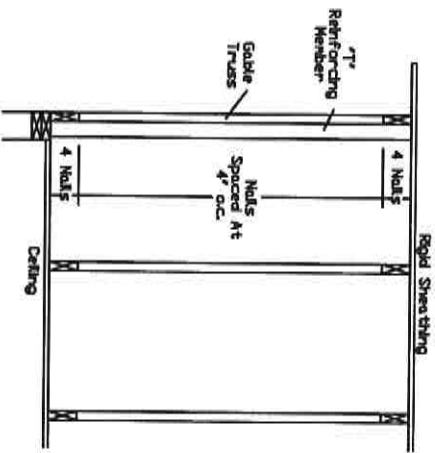
Gable Vertical = 24'ac SP #3

Reinforcing Member Size = 2x4

(1) $3 \times 4 \text{ m}$ Base | south = 84.7%

Maximum T' Reinforced Gable Vertical Length
 $1.30 \times 8' 7" = 11' 2"$

See appropriate Alpine gable detail for maximum unreinforced, gabled Vertical length



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It means regular extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ICCI Building Component Safety Information, by TPI and SCSA, for safety practices prior to performing these functions. Installers must provide temporary bracing per ICCI Tables noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of ribs shall have bracing installed per ICCI sections 90, 97 or 310, as applicable. Apply plates to each face of truss and position as shown above and on the left. Details, unless noted otherwise.

Alpham, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with AWS/D1.1, or for handling, shipping, installation & bracing of trusses.

For more information, see this job's general notice page and these web sites: ALJMS.enr.construction.com/resources/special/ www.fishbase.org/ www.fishbase.org/ID/ www.fishbase.org/ID/

COA#0-278



REF | FT-IN VERT

DATE 01/02/2018

DRWG GBULLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING	24.0'
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This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

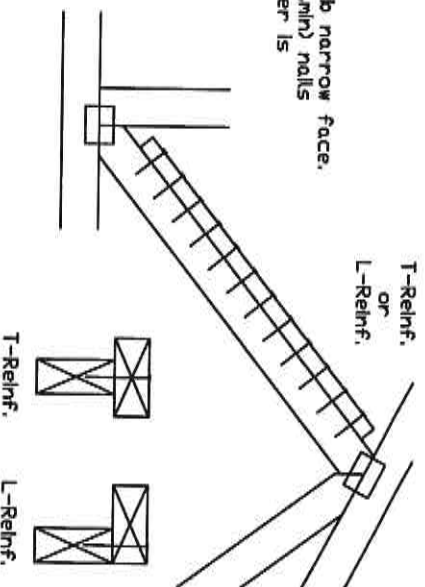
Use scabs instead of L- or T-reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(2)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(2)

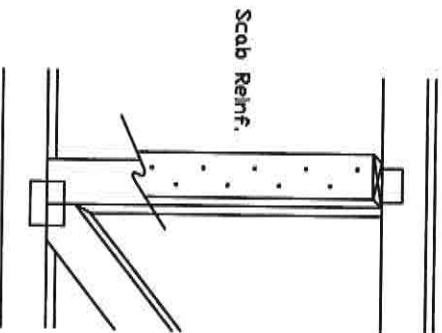
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

- (3) Center scab on wide face of web. Apply (1) scab to each face of web.

Apply to either side of web narrow face
At 6" o.c. Reinforcing member is
a minimum 80% of web
member length.



Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0") nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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Earth City, MO 63045

READER SERVICE READ AND FOLLOW ALL NOTES ON THIS BOARD. IMPORTANT: PLEASE PRINT YOUR NAME AND PHONE NUMBER IN THE SPACE PROVIDED.

Traverse regular attendees care in facilitating, handling, assigning, handling and leading. Refer to and follow the latest edition of ECRI Publishing's Safety Information, by TPI and SSCD for safety practices prior to performing these functions. For more information, contact TPI at 800-368-5877. Unless noted otherwise, top chord shall have properly attached and secured temporary bracing per ECRI shall have a properly attached and secured. Locations shown for permanent lateral restraint of chutes shall have bracing installed per ECRI sections 33, 37 or 318, as applicable. Apply bracing to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1504-2 for standard plate positions.

Alpha, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with AWS/TPI 1, or for handling, erecting, installation, bracing or use.

A seal on this drawing or cover page listing the drafter, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of the drawing for any structure is the responsibility of the Building Department per ASCE/PEI 1 Sec2.

For more information see this job's general notice page and these web sites:
ALJMS www.aljms.com TPD www.tpdfla.com STPA www.stpa.com/elements.com ITD www.itd.com

ALPINE www.alpinefrance.com TPJ www.tpjnsf.org SBCA www.sbcacomponents.com ID www.idcsa.org



PSF	REF	CLR Subst.
PSF	DATE	01/02/19
PSF	DRWG	BRCLBSUB0119

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCE THIS DETAIL.

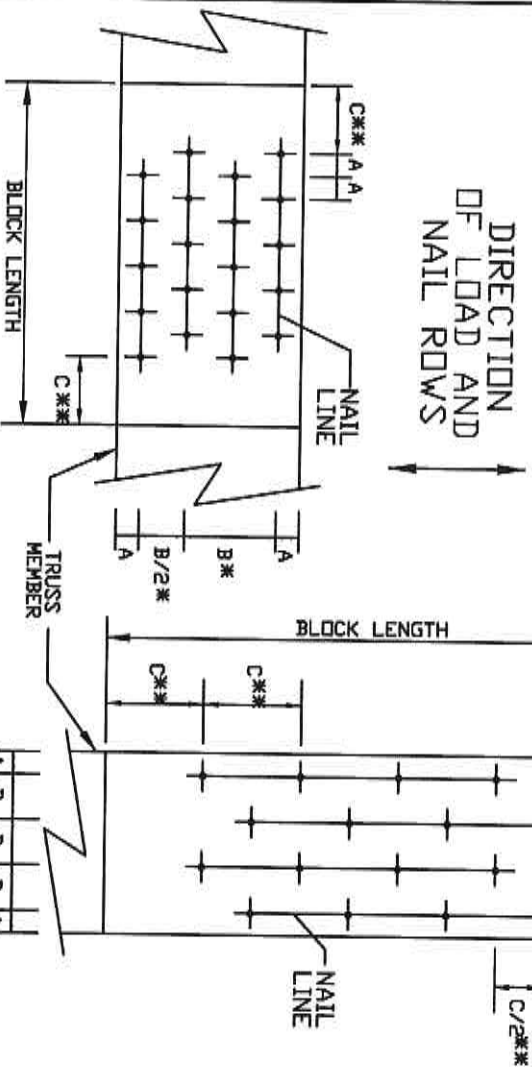
- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

- A - EDGE DISTANCE (6 NAIL DIAMETERS)
- C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BDX (0.113"x 2.5", MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BDX (0.128"x 3", MIN)	7/8"	1 5/8"	2"	1"
12d BDX (0.128"x 3.25", MIN)	7/8"	1 5/8"	2"	1"
16d BDX (0.135"x 3.5", MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BDX (0.148"x 4", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"x 2.5", MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"x 3", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"x 3.25", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"x 3.5", MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"x 2.5", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"x 2.5", MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"x 3", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"x 3", MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN



514 East Cuy Expressway
Suite 242
East Cuy, MO 63045

DESIGNER: FERNANDO M. VINAS
ENGINEER: FERNANDO M. VINAS
DATE: 10/01/14
DRAWN: CANNAILSP1014

COA#0-278



ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Gable Stud Reinforcement Detail

Dr. 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Dr. 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Dr. 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Species		Brace		No. Braces											
Spacing	Species	Grade	No. Braces	(1) 1x4 1" L' Brace	(2) 2x4 1" L' Brace	(3) 2x4 1" L' Brace	(4) 2x6 1" L' Brace	(5) 2x6 1" L' Brace	(6) 2x6 1" L' Brace	(7) 2x6 1" L' Brace	(8) 2x6 1" L' Brace	(9) 2x6 1" L' Brace	(10) 2x6 1" L' Brace	(11) 2x6 1" L' Brace	(12) 2x6 1" L' Brace
12" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"

1x4 Braces shall be SDB (Stress-Rated Board) or 1x4 Ss. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind load deflection criterion is L/240.
 Provide uplift connections for 100 plf over continuous bearing 45 psf TC Dead Load.
 Gable end supports load from 4' 0" overhangs with 2' 0" overhang or 12" plywood overhang.

Group A1		Group B1	
Service-Pine-Tr	#1 / #2	Service-Pine-Tr	#1 / #2
Standard	Standard	Standard	Standard

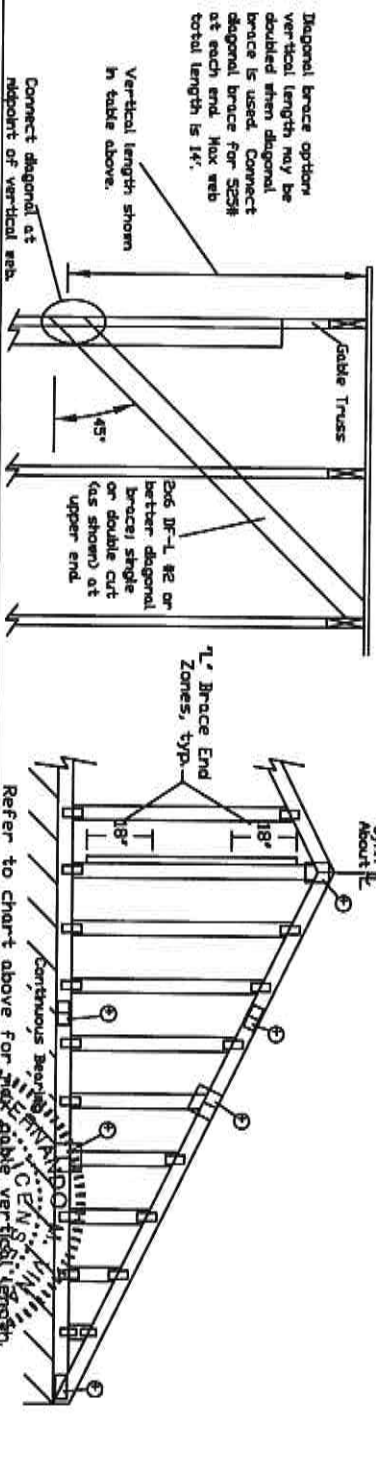
Group A2		Group B2	
Service-Pine-Tr	#3	Service-Pine-Tr	#3
Standard	Standard	Standard	Standard

Group A3		Group B3	
Service-Pine-Tr	#4	Service-Pine-Tr	#4
Standard	Standard	Standard	Standard

Group A4		Group B4	
Service-Pine-Tr	#5	Service-Pine-Tr	#5
Standard	Standard	Standard	Standard

Group A5		Group B5	
Service-Pine-Tr	#6	Service-Pine-Tr	#6
Standard	Standard	Standard	Standard

Group A6		Group B6	
Service-Pine-Tr	#7	Service-Pine-Tr	#7
Standard	Standard	Standard	Standard



Attach 1x4 braces with 10d (16d for 3x10) nails.
 * For 1x4 brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 * For 2x4 brace: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 1x4 bracing must be a minimum of 80% of web member length.
 Refer to the Building Designer for conditions not addressed by this detail.

Refer to the Building Designer for conditions not addressed by this detail.



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 Suite 242
 Earth City, MO 63045

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of ASCE 7-16 (Minimum Design Loads and Associated Criteria for Buildings and Other Structures) for bracing requirements. Truss bracing shall be designed and installed in accordance with the ASCE 7-16 (Minimum Design Loads and Associated Criteria for Buildings and Other Structures) and the ASCE 7-16 (Minimum Design Loads and Associated Criteria for Buildings and Other Structures) and the ASCE 7-16 (Minimum Design Loads and Associated Criteria for Buildings and Other Structures).
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the design shown on this drawing or for any damage to the structure or for any loss of life or property resulting from the use of this drawing. The liability and use of this drawing for any structure is the responsibility of the Building Designer per ASCE 7-16, Sec. 2.4.
 For more information see the job's general notes page and these web sites:
 ALPINE BUILDING COMPONENTS GROUP INC. www.alpinebuildingcomponents.com ITD webdesign



MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0'

REF ASCE 7-16-GAB14030
 DATE 01/26/2018
 DRWG A14030ENC160118