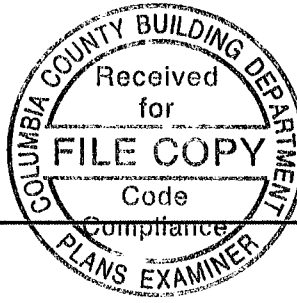


JULIUS LEE PE.



RE: 540831 - FRANKS - SPEC HOUSE

**1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435**

Site Information:

Project Customer: FRANKS - O/B Project Name: 540831 Model Custom
Lot/Block: 5 Subdivision: Huntington
Address:
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Unknown at time of seals License #: Unknown at time of seals
Address: Unknown at time of seals
City: Unknown at time of seals State: Unknown at time of seals

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2010/TPI2007 Design Program: MiTek 20/20 7.3
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 31 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet
conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany
this coversheet. The latest approval dates supersede and replace the previous drawings.**

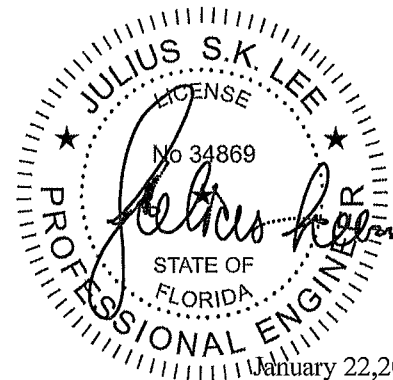
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I7738544	CJ01	1/22/014	18	I7738561	T07	1/22/014
2	I7738545	CJ01A	1/22/014	19	I7738562	T08	1/22/014
3	I7738546	CJ02	1/22/014	20	I7738563	T09	1/22/014
4	I7738547	CJ02A	1/22/014	21	I7738564	T10	1/22/014
5	I7738548	CJ03	1/22/014	22	I7738565	T11	1/22/014
6	I7738549	CJ03A	1/22/014	23	I7738566	T12	1/22/014
7	I7738550	EJ01	1/22/014	24	I7738567	T13	1/22/014
8	I7738551	EJ01A	1/22/014	25	I7738568	T14	1/22/014
9	I7738552	HJ01	1/22/014	26	I7738569	T14G	1/22/014
10	I7738553	HJ01A	1/22/014	27	I7738570	T15	1/22/014
11	I7738554	T01	1/22/014	28	I7738571	T15G	1/22/014
12	I7738555	T01G	1/22/014	29	I7738572	T16	1/22/014
13	I7738556	T02	1/22/014	30	I7738573	T17	1/22/014
14	I7738557	T03	1/22/014	31	I7738574	T18	1/22/014
15	I7738558	T04	1/22/014				
16	I7738559	T05	1/22/014				
17	I7738560	T06	1/22/014				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 540831	Truss CJ01	Truss Type Jack-Open Truss	Qty 8	Ply 1	FRANKS SPEC HOUSE	17738544
Builders FirstSource, Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries Inc. Wed Jan 22 10:12:11 2014 Page 1 ID ePugdD4_Dt74n8lkY4jOVzy8jd1-aPhxIWlhqjg60EcMBV4yJfbux9bOduDXtufczsqmY				

Job Reference (optional)

Plate Offsets (X,Y) [2.0-6.0,0-1-2]				
LOADING (psf) TCCL 20.0 TCCL 7.0 BCCL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.32 BC 0.06 WB 0.00 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.00 8 >999 240 Vert(TL) 0.00 8 >999 180 Horiz(TL) 0.00 2 n/a n/a	PLATES GRIP MT20 244/190 Weight: 7 lb FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=184/0-3-8 (min 0-1-8) 5=-24/Mechanical, 3=-21/Mechanical
 Max Horz 2=67(LC 12)
 Max Uplift 2=-138(LC 12) 5=-30(LC 2), 3=-27(LC 2)
 Max Grav 2=225(LC 2) 5=24(LC 16) 3=18(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

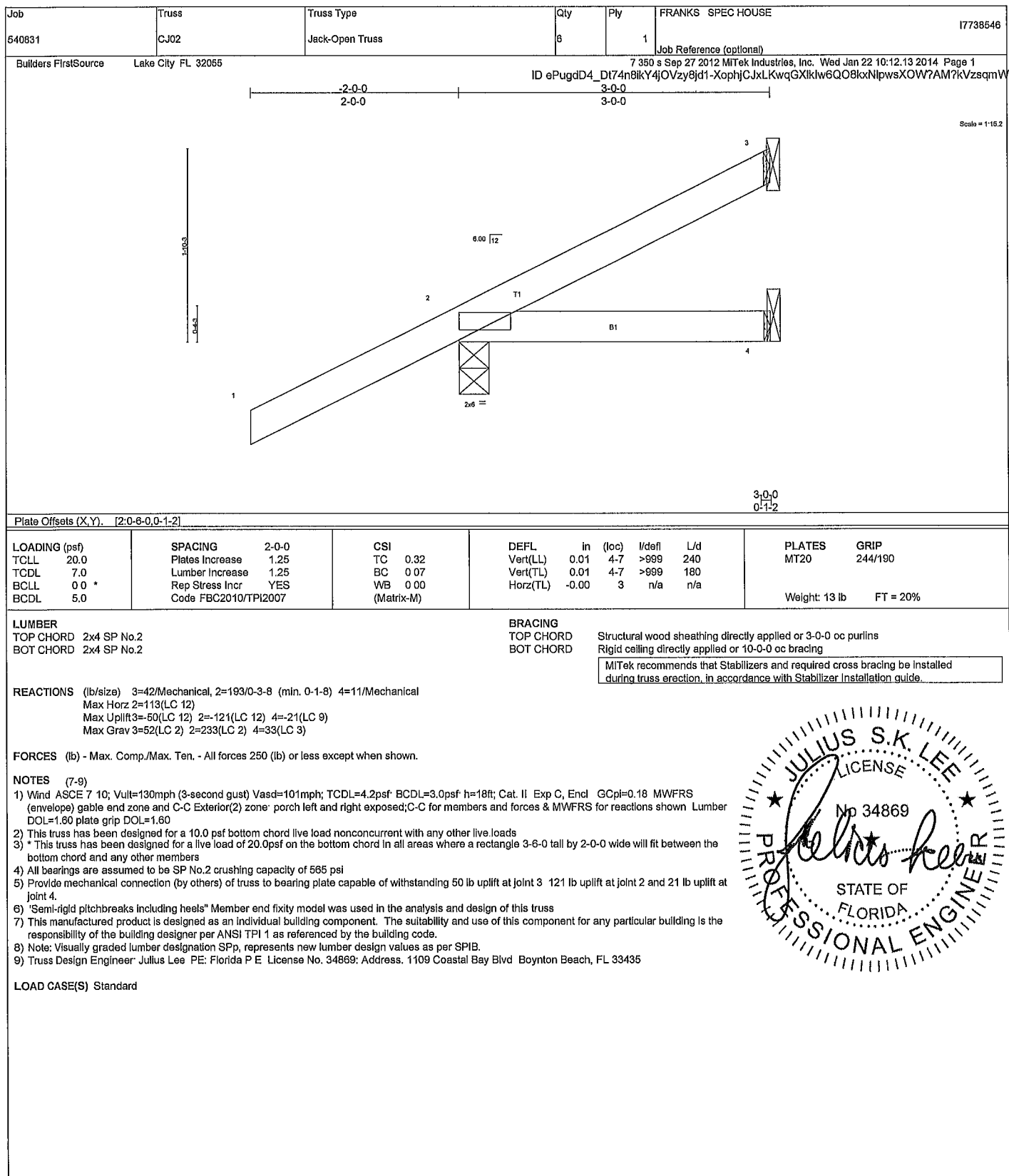
NOTES (7-9)
 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, End GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 30 lb uplift at joint 5 and 27 lb uplift at joint 3.
 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 9) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI 1 Quality Criteria, DSB-87 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE.
1109 Coastal Bay
Boynton Beach, FL 33435

Job 540831	Truss CJ03	Truss Type Jack-Open Truss	Qty 6	Ply 1	FRANKS SPEC HOUSE Job Reference (optional)	I7738548
Builders FirstSource, Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:14 2014 Page 1 ID'ePugdD4_Dt74n8lkY4jOVzy8jd1-?_N3wYKZ6e2hthKxsddfxLH6797eb_dgDq6YGxzsqrnV				

Plate Offsets (X,Y). [2.0-4.4-0-0-4]							
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.32	Vert(LL) 0.07	4-7	>904	240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.23	Vert(TL) 0.06	4-7	>999	180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	2	n/a	n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight: 19 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=79/Mechanical 2=253/0-3-8 (min. 0-1-8), 4=23/Mechanical
Max Horz 2=162(LC 12)
Max Uplift 3=-93(LC 12) 2=-148(LC 12) 4=-35(LC 9)
Max Grav 3=97(LC 2), 2=304(LC 2) 4=56(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-330/589
BOT CHORD 2-4=-952/504

NOTES (7-9)
1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3, 148 lb uplift at joint 2 and 35 lb uplift at joint 4.
6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer Julius Lee, PE Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 540831	Truss EJ01	Truss Type Jack-Partial Truss	Qty 15	Ply 1	FRANKS SPEC HOUSE Job Reference (optional)	17738550
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Builders FirstSource, Lake City FL 32055
7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:16 2014 Page 1

ID ePugdD4_Dt74n8ikY4jOVzy8jd1-xNUqLELqeFJP7?UJz2g70mMOAyIT3u7zh8bfKqzsqm

Plate Offsets (X,Y) [2-0-6-0-0-10]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL)	0.24	4-7	>346	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	0.21	4-7	>401	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.01	2	n/a	n/a		
BCDL 5.0	Code	FBC2010/TPI2007	(Matrix-M)							
									Weight: 26 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

REACTIONS (lb/size) 3=113/Mechanical 2=318/0-3-8 (min 0-1-8) 4=32/Mechanical

Max Horz 2=144(LC 12)

Max Uplift 3=88(LC 12) 2=-141(LC 9), 4=-48(LC 9)

Max Grav 3=139(LC 2), 2=380(LC 2), 4=79(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-739/1501

BOT CHORD 2-4=-2264/1148

NOTES (7-9)

1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf BCDL=3.0psf h=18ft; Cat. II Exp C, End GCpl=0.18 MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 141 lb uplift at joint 2 and 48 lb uplift at joint 4.

6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss

7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

9) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 540831	Truss HJ01	Truss Type Diagonal Hip Girder	Qty 3	Ply 1	FRANKS SPEC HOUSE	I7738552
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:19 2014 Page 1 ID ePugdD4_Dt74n8lkY4jOVzy8jd1-LxAy_F0ixAh_SDufBDqeP_wDAnnGB9PN6pJx9zsqmQ				

Job Reference (optional)

Scale = 1/24.7

Plate Offsets (X,Y): [2:0-0:7,Edge]				
LOADING (psf) TCCL 20.0 TCCL 7.0 BCCL 0.0 * BCCL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2010/TP12007	CSI TC 0.56 BC 0.42 WB 0.30 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.09 6-7 >999 240 Vert(TL) -0.09 6-7 >999 180 Horz(TL) -0.01 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 44 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
 Rigid ceiling directly applied or 7-3-1 oc bracing.

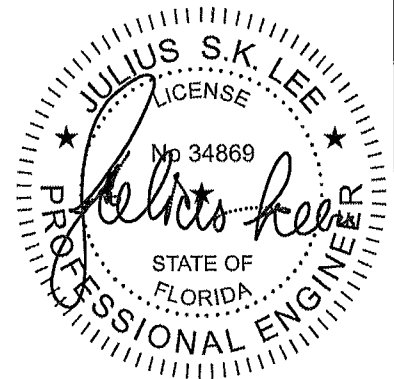
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

REACTIONS (lb/size) 4=116/Mechanical, 2=370/0-4-15 (min. 0-1-8) 5=198/Mechanical
 Max Horz 2=234(LC 4)
 Max Uplift 4=133(LC 4) 2=372(LC 4), 5=263(LC 4)
 Max Grav 4=141(LC 2), 2=454(LC 2) 5=230(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
 TOP CHORD 2-11=-605/489 11-12=-554/515 3-12=-552/500
 BOT CHORD 2-14=-594/552 14-15=-594/552 7-15=-594/552 7-16=-594/552, 6-16=-594/552
 WEBS 3-6=-590/635

NOTES (9-11)
 1) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph: TCCL=4.2psf: BCCL=3.0psf: h=18ft; Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) gable end zone; porch left and right exposed: Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 4, 372 lb uplift at joint 2 and 263 lb uplift at joint 5.
 6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 43 lb down and 46 lb up at 1-5-12 43 lb down and 46 lb up at 1-5-12, 3 lb down and 50 lb up at 4-3-11 3 lb down and 50 lb up at 4-3-11 and 43 lb down and 93 lb up at 7-1-10 and 43 lb down and 93 lb up at 7-1-10 on top chord, and 19 lb down and 29 lb up at 1-5-12 19 lb down and 29 lb up at 1-5-12, 3 lb down and 31 lb up at 4-3-11 3 lb down and 31 lb up at 4-3-11 and 26 lb down and 45 lb up at 7-1-10 and 26 lb down and 45 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 8) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 11) Truss Design Engineer: Julius Lee PE Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-44, 5-8=-10

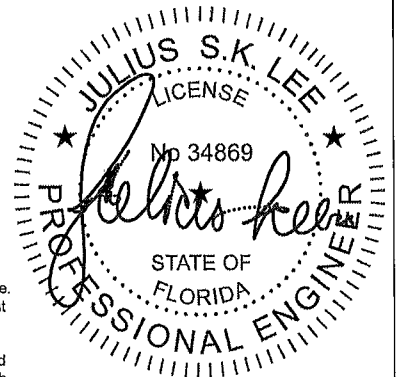


January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
 Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component.
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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 540831	Truss HJ01A	Truss Type Diagonal Hip Girder	Qty 1	Ply 1	FRANKS SPEC HOUSE	17738553																																																							
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries Inc. Wed Jan 22 10:12:20 2014 Page 1 ID ePugdD4_Dt74n8kY4jOVzy8jd1-q8kKBbOKIUprrco4Cuk3BcX4zZ87?F7YcmZtTbzsqmP																																																											
<table border="1"> <tr> <td>LOADING (psf)</td> <td>SPACING</td> <td>2-0-0</td> <td>CSI</td> <td>DEFL</td> <td>in</td> <td>(loc)</td> <td>l/defl</td> <td>L/d</td> <td>PLATES</td> <td>GRIP</td> </tr> <tr> <td>TCCL 20.0</td> <td>Plates Increase</td> <td>1.25</td> <td>TC 0.56</td> <td>Vert(LL)</td> <td>0.05</td> <td>9</td> <td>>999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCCL 7.0</td> <td>Lumber Increase</td> <td>1.25</td> <td>BC 0.35</td> <td>Vert(TL)</td> <td>-0.07</td> <td>9</td> <td>>999</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCCL 0.0 *</td> <td>Rep Stress Incr</td> <td>NO</td> <td>WB 0.26</td> <td>Horz(TL)</td> <td>0.02</td> <td>6</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCCL 5.0</td> <td>Code FBC2010/TPI2007</td> <td></td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>Weight: 46 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	TCCL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	0.05	9	>999	240	MT20	244/190	TCCL 7.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.07	9	>999	180			BCCL 0.0 *	Rep Stress Incr	NO	WB 0.26	Horz(TL)	0.02	6	n/a	n/a			BCCL 5.0	Code FBC2010/TPI2007		(Matrix-M)						Weight: 46 lb	FT = 20%
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP																																																			
TCCL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	0.05	9	>999	240	MT20	244/190																																																			
TCCL 7.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.07	9	>999	180																																																					
BCCL 0.0 *	Rep Stress Incr	NO	WB 0.26	Horz(TL)	0.02	6	n/a	n/a																																																					
BCCL 5.0	Code FBC2010/TPI2007		(Matrix-M)						Weight: 46 lb	FT = 20%																																																			
<table border="1"> <tr> <td>LUMBER</td> <td>BRACING</td> <td></td> </tr> <tr> <td>TOP CHORD 2x4 SP No.2</td> <td>TOP CHORD</td> <td>Structural wood sheathing directly applied or 6-0-0 oc purlins.</td> </tr> <tr> <td>BOT CHORD 2x4 SP No.2</td> <td>BOT CHORD</td> <td>Rigid ceiling directly applied or 8-0-1 oc bracing.</td> </tr> <tr> <td>WEBS 2x4 SP No.3</td> <td></td> <td>MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.</td> </tr> </table>							LUMBER	BRACING		TOP CHORD 2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.	BOT CHORD 2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 8-0-1 oc bracing.	WEBS 2x4 SP No.3		MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.																																											
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WEBS 2x4 SP No.3		MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.																																																											
<p>REACTIONS (lb/size) 5=76/Mechanical 6=234/Mechanical 2=447/0-3-13 (min. 0-1-8) Max Horz 2=234(LC 4) Max Uplift 5=80(LC 4) 6=188(LC 8), 2=370(LC 4) Max Grav 5=93(LC 2) 6=274(LC 2) 2=534(LC 2)</p> <p>FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-739/417 3-16=-858/568, 4-16=-713/455 BOT CHORD 2-9=-506/855, 3-9=-409/562 8-9=-529/702, 8-18=-529/702 7 18=-529/702 WEBS 4-7=-749/564, 4-8=-146/278</p> <p>NOTES (10-12) 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone: Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 5, 188 lb uplift at joint 6 and 370 lb uplift at joint 2. 7) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6 lb down and 47 lb up at 4-3-11 and 6 lb down and 47 lb up at 4-3-11 and 24 lb down and 70 lb up at 7 1 10 on top chord, and 4 lb down and 3 lb up at 4-6-15, 4 lb down and 3 lb up at 4-6-15, and 68 lb down and 79 lb up at 7-1-10, and 44 lb down and 23 lb up at 7-1-10 on bottom chord The design/selection of such connection device(s) is the responsibility of others. 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B) 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 11) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 12) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435</p> <p>LOAD CASE(S) Standard 1) Regular Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert. 1-5=-44, 9-13=-10, 6-9=-10</p>																																																													



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 BEFORE USE.
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Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 540831	Truss T01	Truss Type Common Truss	Qty 5	Ply 1	FRANKS SPEC HOUSE Job Reference (optional)	17738554
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7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jan 22 10:12:21 2014 Page 1
 ID ePugdD4_Dl74n8kY4jOVzy8jd1-lKlOxPyTnxIDmMHmcFJlq4HrzMDk5CiqQlQ01zsqmO

Builders FirstSource, Lake City FL 32055

Scale = 1/32"

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCCL 20.0	Plates Increase 1.25	TC 0.42	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.87	Vert(LL) 0.25 8-10 >945 240		
BCCL 0.0 *	Rep Stress Incr NO	WB 0.27	Vert(TL) -0.38 8-10 >629 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.04 6 n/a n/a		
				Weight: 96 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-3-9 oc purlins
 BOT CHORD Rigid ceiling directly applied or 6-6-7 oc bracing

REACTIONS (lb/size) 2=801/0-3-8 (min 0-1-8) 6=801/0-3-8 (min 0-1-8)
 Max Horz 2=126(LC 12)
 Max Uplift 2=455(LC 12) 6=455(LC 13)
 Max Grav 2=953(LC 2), 6=953(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1713/992 3-4=-1572/955, 4-5=-1571/954, 5-6=-1712/992
 BOT CHORD 2-10=-757/1527 9-10=-392/1012, 8-9=-392/1012 6-8=-765/1542
 WEBS 4-8=-370/640, 5-8=-270/281 4-10=-370/640, 3-10=-271/281

NOTES (9-11)
 1) Unbalanced roof live loads have been considered for this design
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf h=18ft; Cat. II Exp C Encl. GCpl=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 455 lb uplift at joint 2 and 455 lb uplift at joint 6
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-44, 4-7=-44 10-11=-10 8-10=-81(F=51), 8-14=-10



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 540831	Truss T02	Truss Type Common Truss	Qty 5	Ply 1	FRANKS - SPEC HOUSE Job Reference (optional)	17738556
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jan 22 10:12:24 2014 Page 1
ID: ePugdD4_DT74n8kY4jOVzy8jd1-lvzr1zRmIJG4D5sRkp7LSlo5BOyxSs8WVOX4cMzsqmL

Scale = 1/32.2

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.42	In (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.87	Vert(LL) 0.25 7-9 >944 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.27	Vert(TL) -0.38 7-9 >632 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) 0.04 6 n/a n/a		
	Code FBC2010/TPI2007			Weight: 93 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 6=711/0-3-8 (min. 0-1-8) 2=802/0-3-8 (min. 0-1-8)

Max Horz 2=89(LC 12)

Max Uplift 6=-218(LC 13) 2=-256(LC 12)

Max Grav 6=843(LC 2) 2=955(LC 2)

FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1716/997, 3-4=-1575/960, 4-5=-1572/966, 5-6=-1712/1004

BOT CHORD 2-9=-816/1481 8-9=-444/951 7-8=-444/951 6-7=-824/1476

WEBS 4-7=-378/635, 5-7=-268/266, 4-9=-368/639 3-9=-271/261

NOTES (9-11)

- Unbalanced roof live loads have been considered for this design.
- Wind. ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf; h=18ft; Cat. II, Exp C, Encl. GCPI=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 6=218, 2=256.
- 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
- In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-44 4-6=-44, 9-13=-10, 7-9=-61(F=51), 7 10=-10

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-9 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-3-9 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown
is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

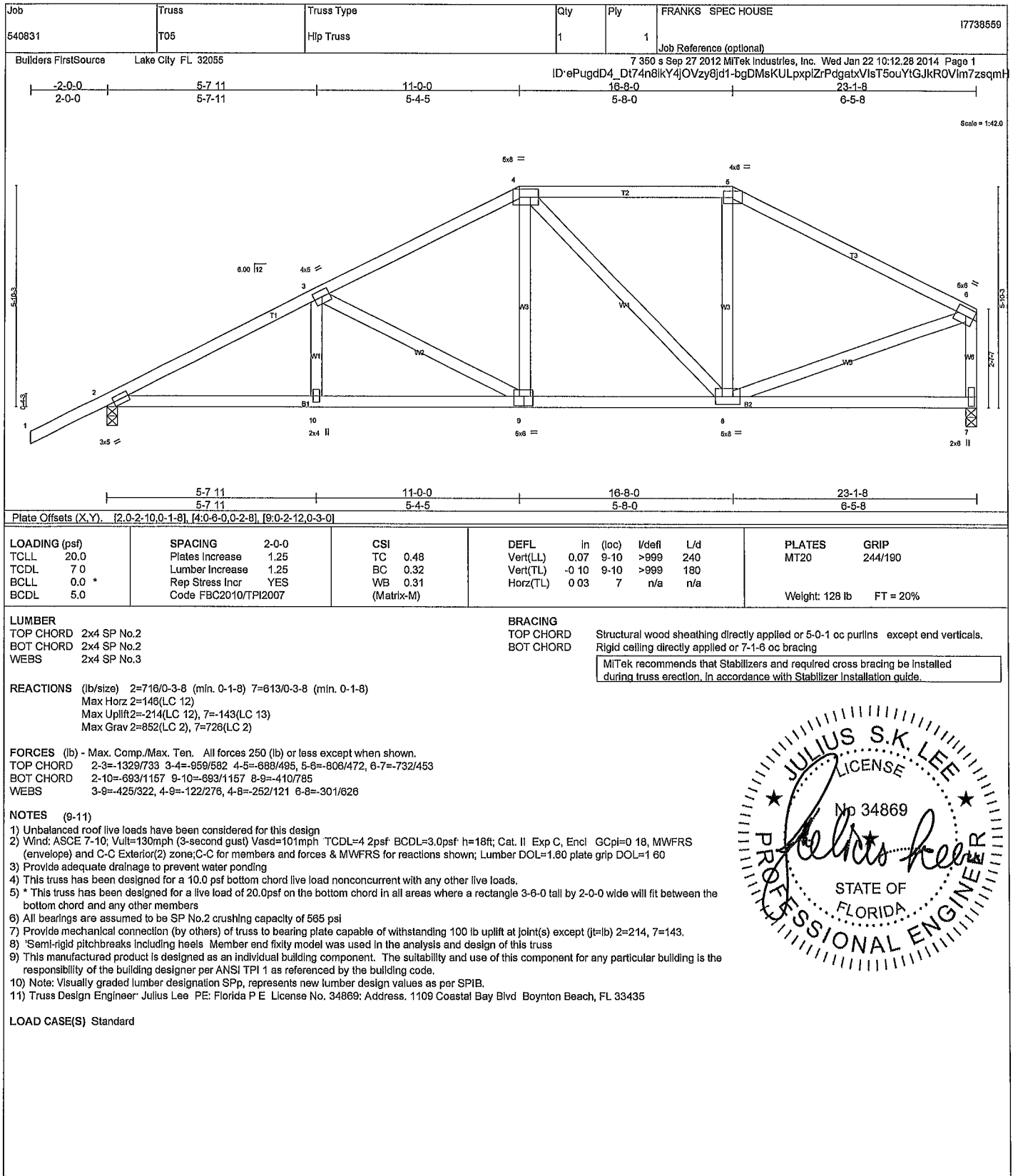
Julius Lee PE,
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	FRANKS - SPEC HOUSE	17738567
540831	T03	Half Hip Truss	1	1	Job Reference (optional)	
Builders FirstSource		Lake City FL 32055	7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:26 2014 Page 2			
ID ePugdD4_DI74n8lkY4jOVzy8jd1-el5bSFT5HKZ_KXFEZ9rTQln4Z_7wPJIR_i0BhFzsqm.						
LOAD CASE(S) Standard						
1) Regular: Lumber Increase=1.25, Plate Increase=1.25						
Uniform Loads (plf)						
Vert: 1-3=-44, 3-7=-44, 8-13=-10						
Concentrated Loads (lb)						
Vert: 3=-69(F) 5=-69(F) 11=-22(F) 12=-210(F) 16=-69(F) 17=-69(F) 18=-69(F) 19=-69(F) 20=-69(F) 21=-69(F) 22=-69(F) 23=-22(F) 24=-22(F) 25=-22(F) 26=-22(F) 27=-22(F) 28=-22(F) 29=-22(F)						



WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown
 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCIS1 Building Component
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 540831	Truss T07	Truss Type Special Truss	Qty 3	Ply 1	FRANKS SPEC HOUSE Job Reference (optional)	17738561
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Builders FirstSource, Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:32 2014 Page 1
ID ePugdD4_Dt74n8kY4jOVzy8jd1-TSSstIYstAK82SiOvPytg813CP9epxFJMeTVvuzsqmD

Scale = 1/48.6

Plate Offsets (X,Y). [3:0-3:0,0-3-4]	LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
	TCLL 20.0	2-0-0	TC 0.86	in (loc) l/defl L/d	MT20	244/190
	TCDL 7.0	Plates Increase 1.25	BC 0.69	Vert(LL) 0.39 8-9 >843 240		
	BCLL 0.0 *	Lumber Increase 1.25	WB 0.82	Vert(TL) -0.63 8-9 >527 180		
	BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.41 6 n/a n/a		
		Code FBC2010/TPI2007			Weight: 122 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 6=745/0-3-8 (min. 0-1 8), 2=837/0-3-8 (min. 0-1-8)

Max Horz 2=113(LC 12)

Max Uplift 6=-215(LC 13), 2=-254(LC 12)

Max Grav 6=882(LC 2), 2=996(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3019/1683, 3-4=-2100/1111 4-5=-2101/1112, 5-6=-3037/1687

BOT CHORD 2-9=-1422/2704, 8-9=-1426/2713, 7-8=-1451/2730 6-7=-1449/2723

WEBS 4-8=-687/1426, 5-8=-696/702 3-8=-879/677

NOTES (9-11)

- Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasc=101mph TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C End GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Bearing at joint(s) 6, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (1=lb) 6=215 2=254.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied.

BOT CHORD Rigid ceiling directly applied or 4-10-6 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



January 22, 2014

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Julius Lee PE.
1109 Coastal Bay
Boynton Beach FL 33435

Job 540831	Truss T09	Truss Type Half Hip Truss	Qty 1	Ply 1	FRANKS - SPEC HOUSE Job Reference (optional)	17738563
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7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:35 2014 Page 1
ID ePugdD4_Dt74n8ikY4jOVzy8jd1-l087KkakA5liivwRzbYVaHnffFc7KON7m2bhAVDzsqm

Builders FirstSource, Lake City FL 32055

Scale = 1/32

Plate Offsets (X,Y): [4,0-6-0,0-2-8], [14,0-2-7,0-1-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.55	Vert(LL)	0.15 12-13	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-0.27 11 12	>842	180		
BCCL 0.0 *	Rep Stress Incr	NO	WB 0.49	Horz(TL)	0.14 8	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)						

Weight: 117 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD
BOT CHORD 2x4 SP No.2 *Except*	
B3, 2x4 SP No.3	BOT CHORD
WEBS 2x4 SP No.3	

REACTIONS (lb/size) 8=992/0-3-8 (min. 0-1-8) 2=941/0-3-8 (min. 0-1-8)
Max Horz 2=146(LC 8)
Max Uplift 8=827(LC 5) 2=441(LC 8)
Max Grav 8=1176(LC 2) 2=1117(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3195/1407 3-4=-2175/964 4-18=-1871/911 18-19=-1871/911 5-19=-1871/911
5-20=-1815/886, 20-21=-1815/886 6-21=-1815/886, 6-22=-886/466, 7-22=-886/466
7-8=-1150/621

BOT CHORD 2-13=-1357/2863 12-13=-1285/2709 12-23=-907/1964, 23-24=-907/1964, 11-24=-907/1964
5-11=-441/251

WEBS 3-13=-272/609, 3-12=-805/405, 4-12=-257/636, 9-11=-381/802 6-11=-530/1176, 6-9=-1068/520
7-9=-682/1296

NOTES (12-14)

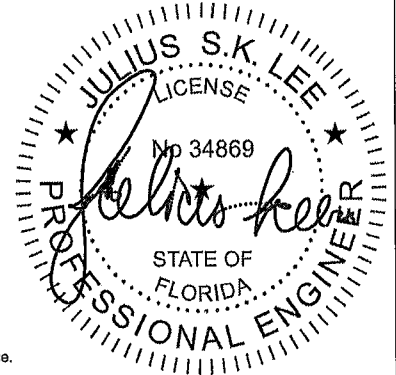
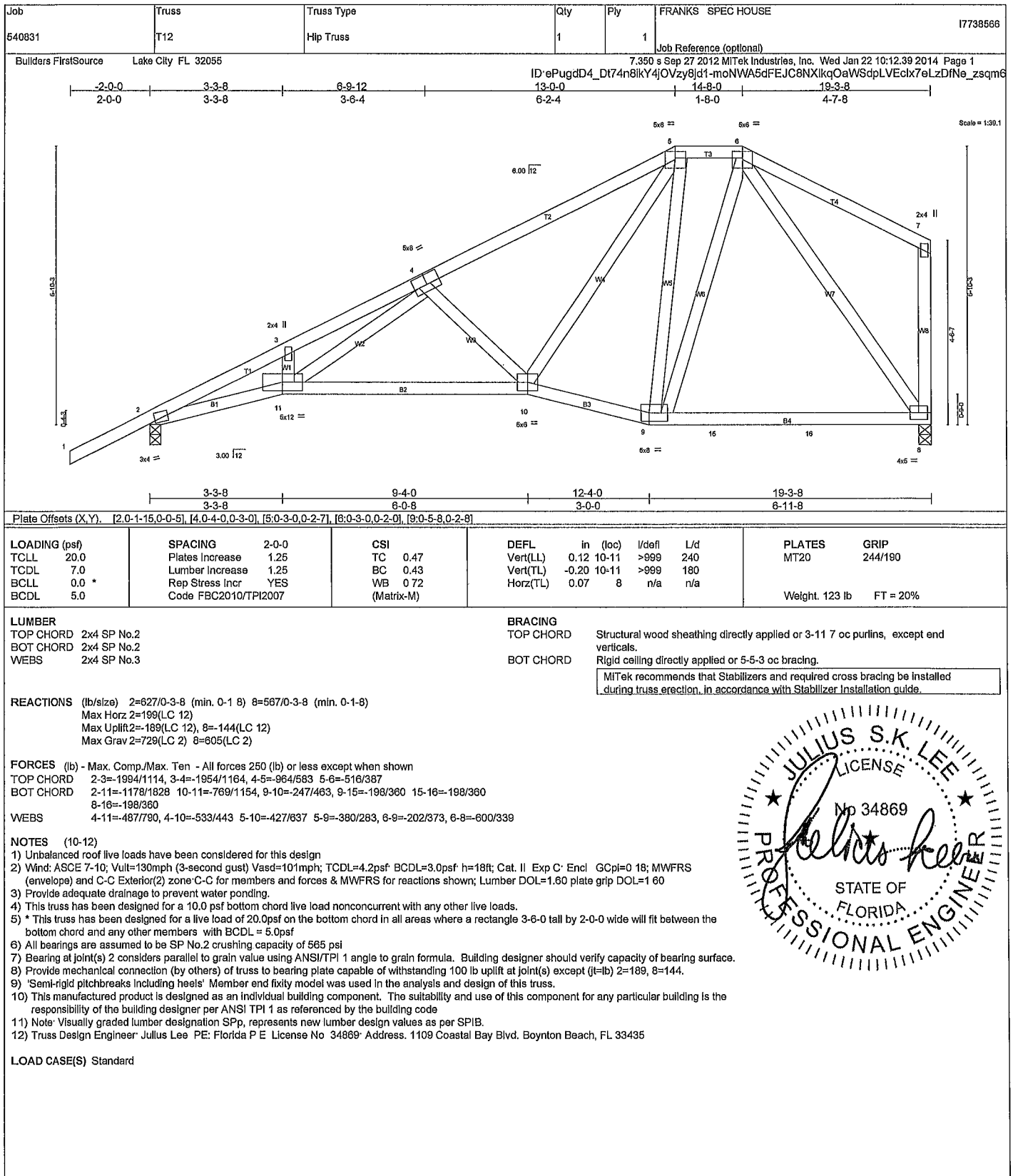
- Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl. GCpi=0.18, MWFRS (envelope) Lumber DOL=1 60 plate grip DOL=1 60
- Provide adequate drainage to prevent water ponding
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 585 psi.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (Jt=lb) 8=627, 2=441
- "Semi-rigid pitchbreaks including heels. Member end fixity model was used in the analysis and design of this truss
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 76 lb up at 7-0-0 63 lb down and 76 lb up at 9-0-12, 63 lb down and 76 lb up at 11-0-12 85 lb down and 92 lb up at 13-0-12, 85 lb down and 92 lb up at 15-0-12, and 85 lb down and 92 lb up at 17-0-12 and 85 lb down and 92 lb up at 19-1-12 on top chord and 264 lb down and 198 lb up at 7-0-0 49 lb down and 58 lb up at 13-0-12, and 49 lb down and 58 lb up at 15-0-12 and 49 lb down and 58 lb up at 17-0-12 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869; Address, 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435



January 22, 2014

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Julius Lee PE,
1109 Coastal Bay
Boynton Beach, FL 33435

Job 540831	Truss T14	Truss Type SCISSOR TRUSS	Qty 1	Ply 1	FRANKS - SPEC HOUSE	17738568
Builders FirstSource, Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:41 2014 Page 1 ID ePugdD4_Dt74n8kY4jOVzy8jd1-iAVGbnfVlxSsdru7xoc_X2vjX1KiPAbeRX8UjtZsqm4				

Plate Offsets (X,Y) [2-0-2,7,0-0-5], [4-0-2,7,0-0-5]	
--	--

LOADING (psf) TCCL 20.0 TCCL 7.0 BCCL 0.0 * BCDL 5.0	SPACING Plates Increase 2-0-0 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.37 BC 0.31 WB 0.17 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.07 6-12 >999 240 Vert(TL) -0.11 6-9 >999 180 Horz(TL) 0.05 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 51 lb FT = 20%
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LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-3-10 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=439/0-3-8 (min. 0-1 8) 4=439/0-3-8 (min. 0-1-8)

Max Horz 2=89(LC 12)

Max Uplift 2=-252(LC 12) 4=-252(LC 13)

Max Grav 2=524(LC 2), 4=524(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-990/451 3-4=-989/449

BOT CHORD 2-6=-366/853, 4-6=-358/853

WEBS 3-6=-117/499

NOTES (9-11)

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10: Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C; Encl GCpl=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

6) Bearing at joint(s) 2 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=252, 4=252.

8) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

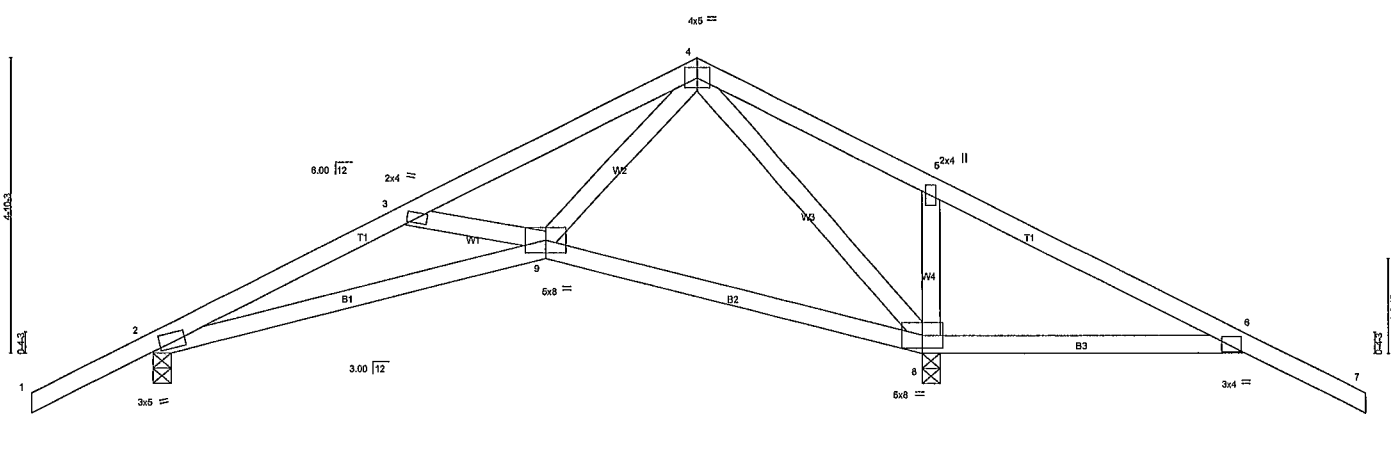
11) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 22, 2014

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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach FL 33435

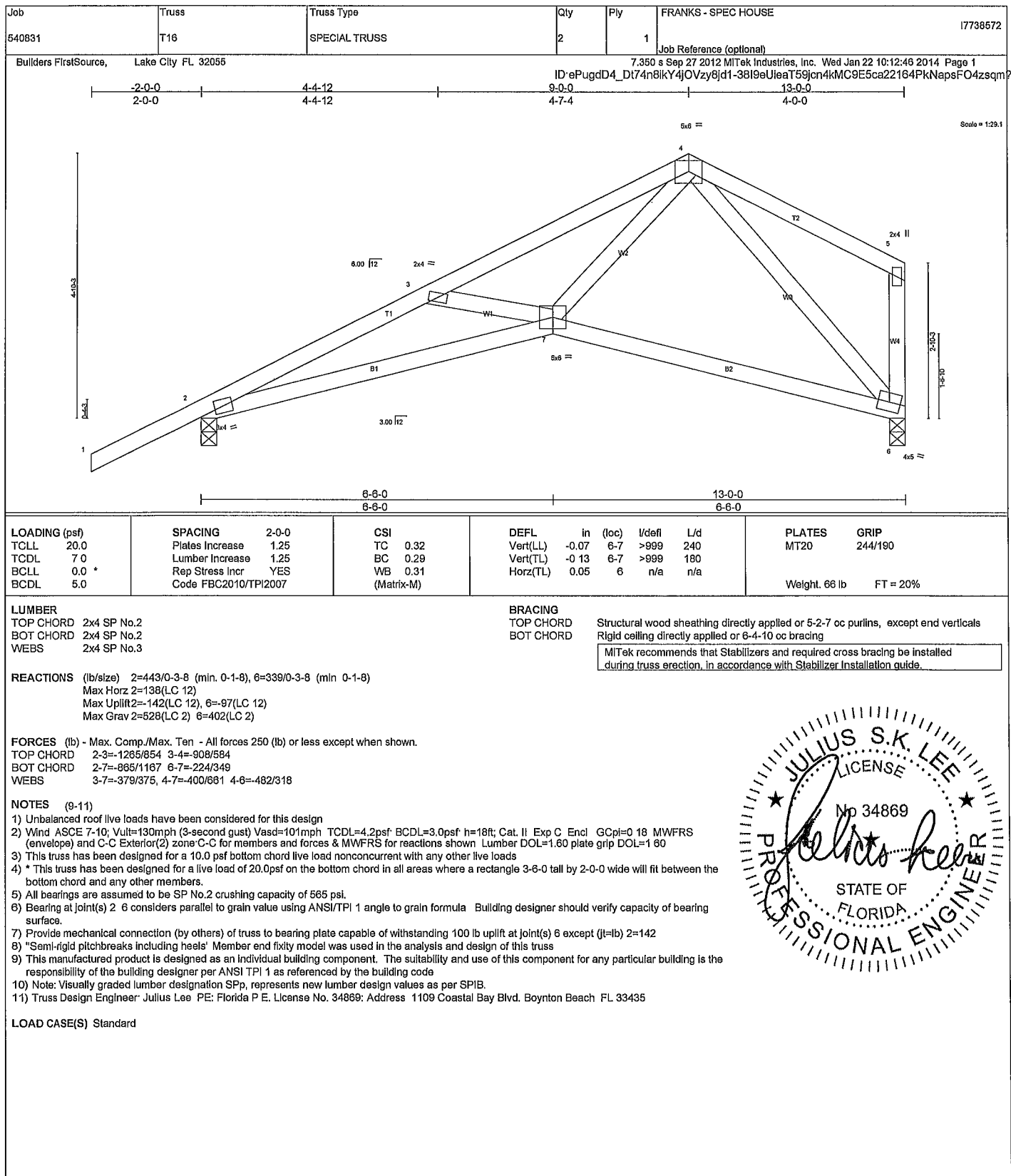
Job 540831	Truss T15	Truss Type SCISSOR TRUSS	Qty 2	Ply 1	FRANKS SPEC HOUSE	17738570																																																												
Builders FirstSource Lake City FL 32055		7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:44 2014 Page 1 ID ePugdD4_Dt74n8ikY4jOVzy8jd1-7IBPDphO2sqRUldicxhAh9gXDzFHUcRB47VN8KCzsqm1																																																																
<div style="display: flex; justify-content: space-between; font-size: small;"> 2-0-0 4-4-12 9-0-0 12-8-8 18-0-0 20-0-0 2-0-0 4-4-12 4-7 4 3-8-8 5-3-8 2-0-0 </div> 																																																																		
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REACTIONS (lb/size) 2=333/0-3-8 (min. 0-1-8) 8=815/0-3-8 (min. 0-1-8) Max Horz 2=116(LC 13) Max Uplift 2=232(LC 12) 8=451(LC 13) Max Grav 2=456(LC 27), 8=971(LC 2)																																																																		
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-868/1174, 3-4=-530/843, 4-5=-775/854, 5-6=-907/861 BOT CHORD 2-8=-917/772 8-9=-164/323, 6-8=-550/957 WEBS 3-9=-366/466, 4-9=-982/454, 4-8=-833/1154, 5-8=-284/288																																																																		
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl. GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever right exposed porch right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 5) All bearings are assumed to be SP No 2 crushing capacity of 585 psi. 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=232 8=451 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 11) Truss Design Engineer Julius Lee, PE: Florida P.E. License No. 34869; Address, 1109 Coastal Bay Blvd Boynton Beach FL 33435																																																																		
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January 22, 2014

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Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435



January 22, 2014

Job 540831	Truss T18	Truss Type Common Truss	Qty 1	Ply 1	FRANKS - SPEC HOUSE Job Reference (optional)	17738574
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Builders FirstSource, Lake City FL 32055 7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 22 10:12:48 2014 Page 1
 ID ePugdD4_DT74n8IkY4jOVzy8jd1-7XQw3Aku64LszvxTrmEdJWnsYsgCYKfg27LMSzsqz

Plate Offsets (X,Y): [1,0-2-10,0-1-8], [3,0-2-10,0-1-8]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL)	0.22 4-10	>865	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.51	Vert(TL)	0.19 4-10	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.15	Horz(TL)	-0.02 3	n/a	n/a		
BCDL 5.0	Code FBC2010/TP12007		(Matrix-M)						
								Weight: 56 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 1=432/0-3-8 (min. 0-1-8) 3=432/0-3-8 (min. 0-1-8)

Max Horz 1=54(LC 9)

Max Uplift 1=239(LC 9) 3=239(LC 8)

Max Grav 1=512(LC 2) 3=512(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1024/1742 2-3=-1024/1742

BOT CHORD 1-4=-2325/1440 3-4=-2325/1440

WEBS 2-4=-474/242

NOTES (8-10)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C Encl. GCPI=0.18 MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 1=239 3=239.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 9) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- 10) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

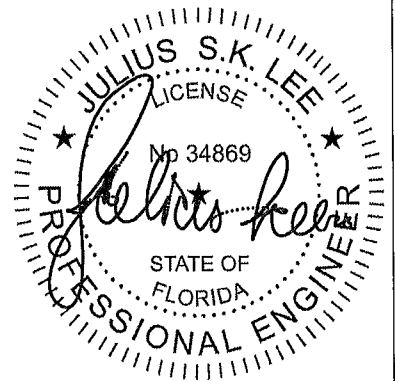
LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-7 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-5-15 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



January 22, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

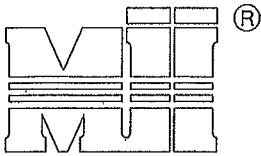
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Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

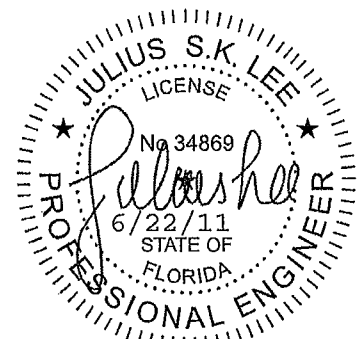
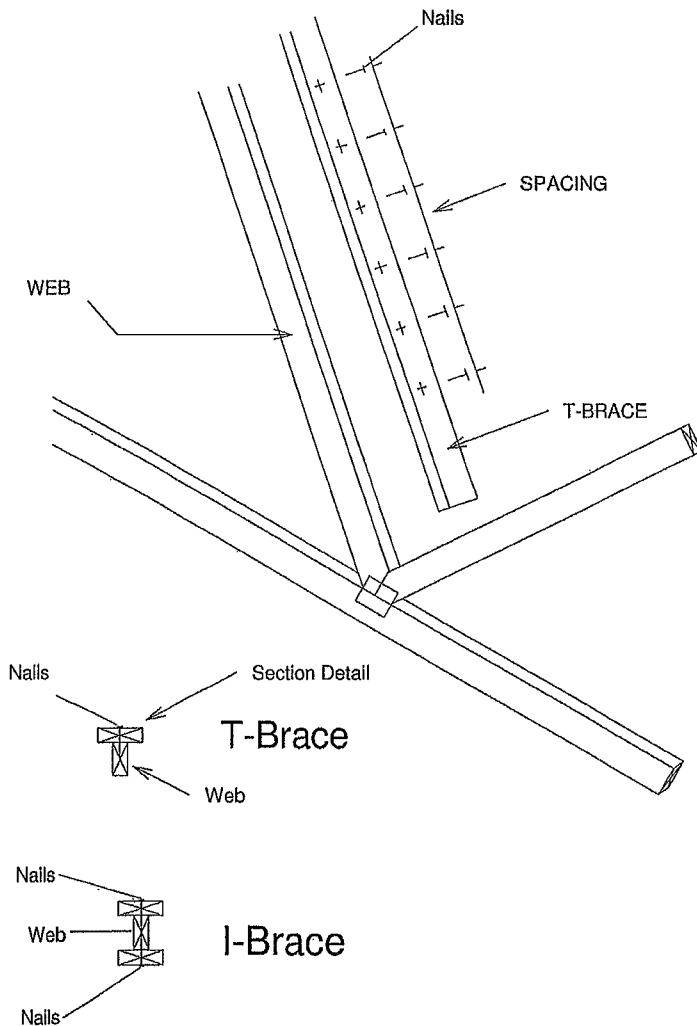
Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

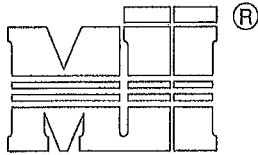


1109 COASTAL BAY
BOYNTON BC, FL 33435

JANUARY 1, 2009

LATERAL TOE-NAIL DETAIL

ST-TOENAIL_SP



MITek Industries, Inc.

MITek Industries, Chesterfield, MO Page 1 of 1

NOTES

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

EXAMPLE

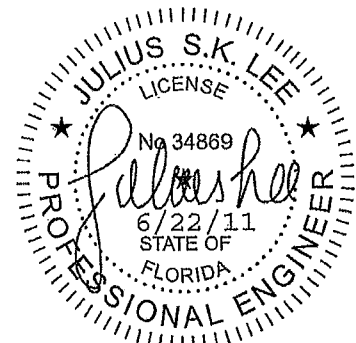
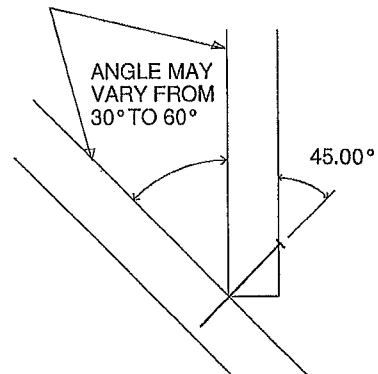
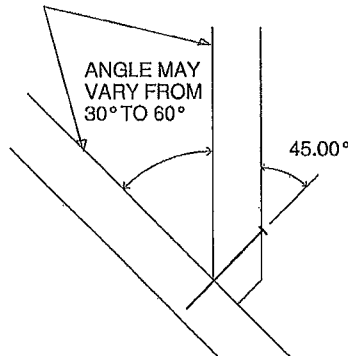
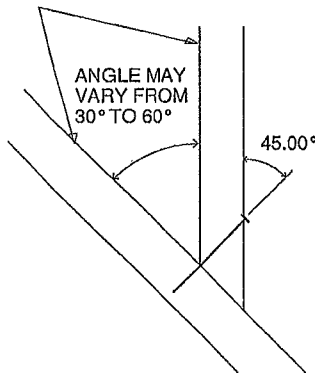
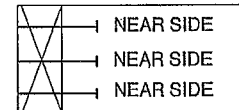
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

SIDE VIEW

3 NAILS



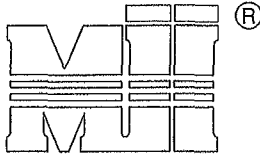
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FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

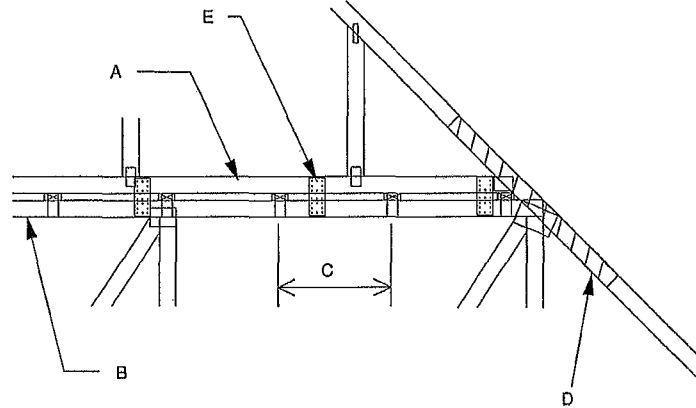


MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES)
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

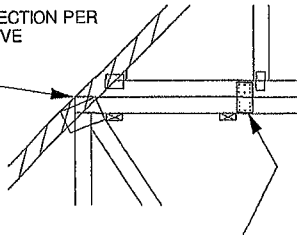
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH
- D - 2 X 4" X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



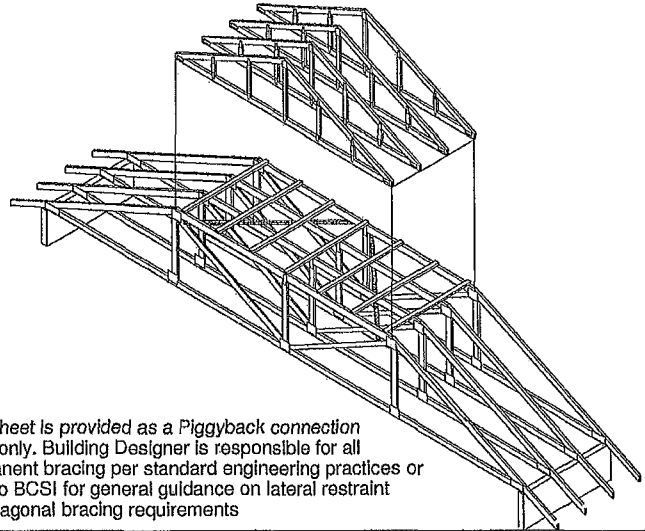
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER
NOTE D ABOVE

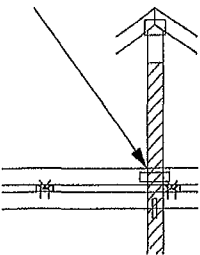


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE



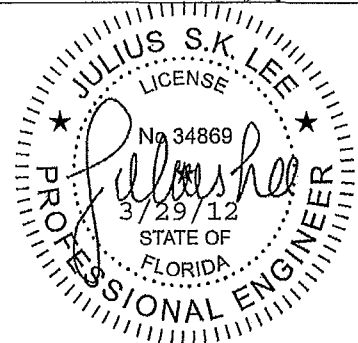
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

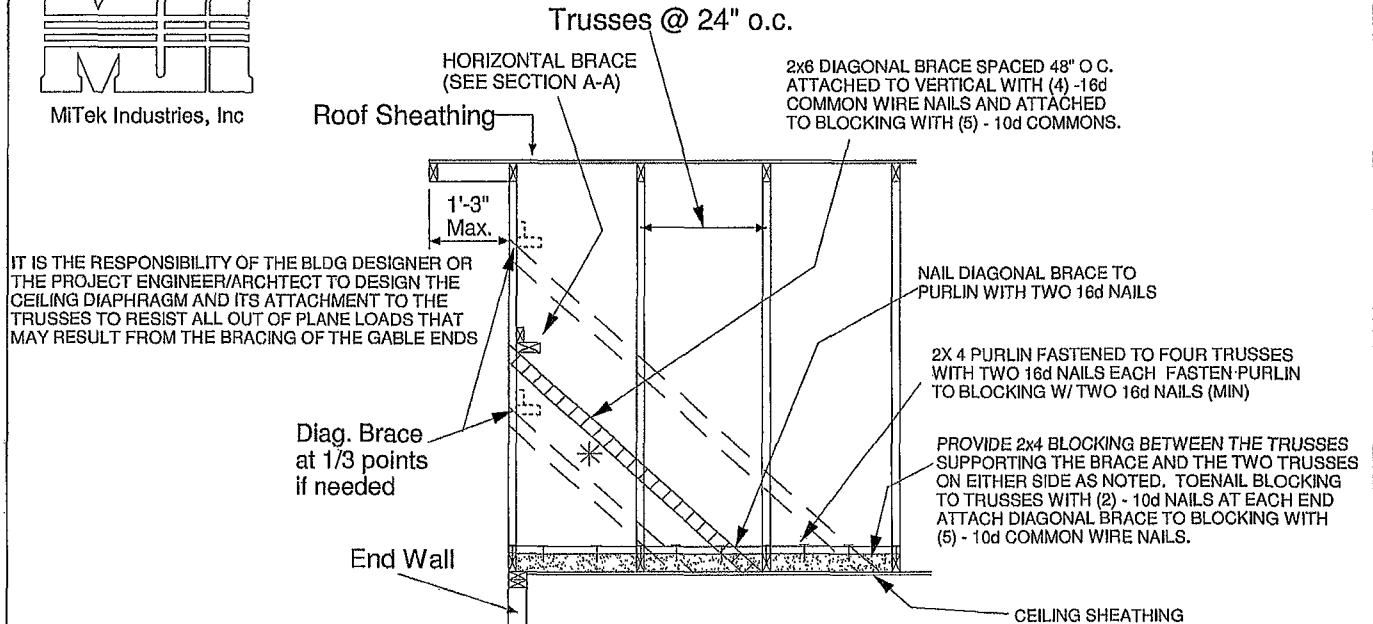
- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- 2) ATTACH 2 x 4" X 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15) REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

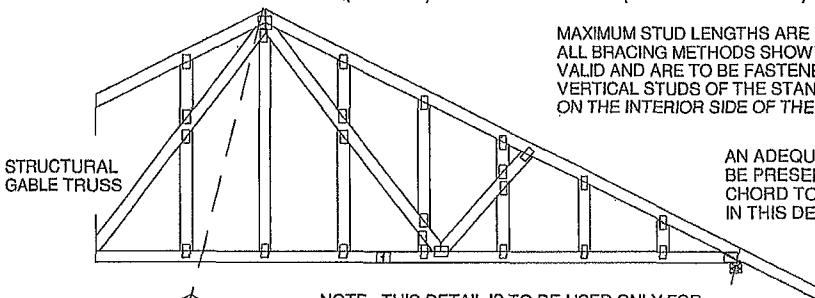
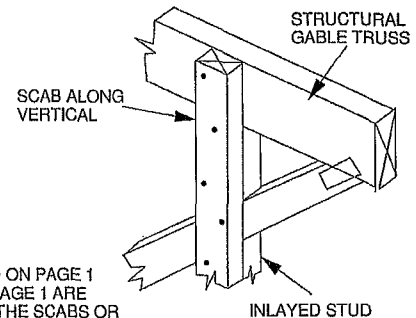
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED.

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2 : ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE

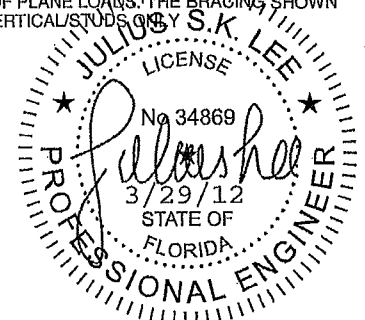
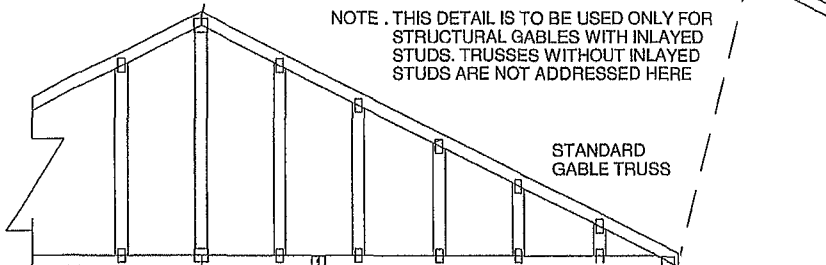
- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O.C (2X 4 STUDS MINIMUM)



MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1 ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY

NOTE . THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYS STUDS. TRUSSES WITHOUT INLAYS STUDS ARE NOT ADDRESSED HERE



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6/12 PITCH
24" O/H

