

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Page 1 of 1 Document ID:1UQ89114Z0211162123



Truss Fabricator: Anderson Truss Company
Job Identification: 12-261--Jerry Castagna Constructi Willmann Res. -- Columbia County, FL
Truss Count: 4
Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Versions 10.03, 12.02.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 130 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

-Truss Design Engineer-
William H. Krick

1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	39535-HJ7	9'10"13 Hip	12285040	10/11/12
2	39538-HJ5	6'10"12 Hip	12285041	10/11/12
3	39539-H5C	18' Stepdown	12285042	10/11/12
4	39540-H7B	29' Stepdown	12285043	10/11/12

ALPINE

(12-261--Jerry Castagna Constructi Willmann Res. -- Columbia County . FL - E 4'8"8 Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 6Cpf(+/-)=0.18

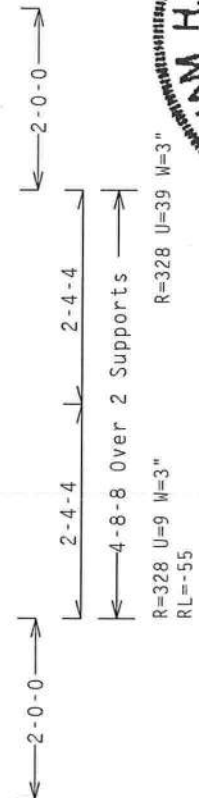
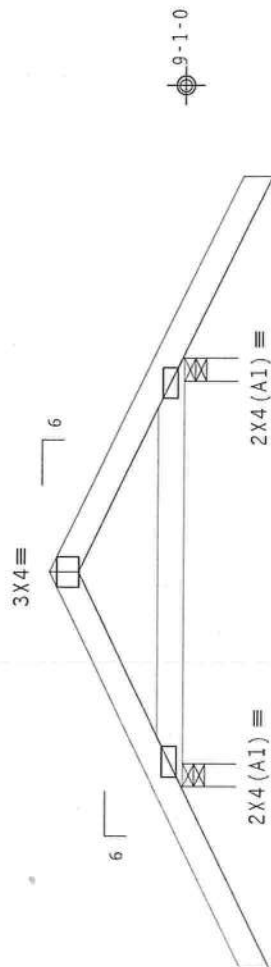
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Professional Engineer Seal for William H. Krick, License No. 70861, State of Florida. Includes text: "I am a duly Licensed Professional Engineer in the State of Florida. My license is valid for the practice of engineering in the State of Florida. I am not responsible for any deviation from this design for any reason other than the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITN-BGC: www.itnbgc.com; TPI: www.tpinet.org; MECA: www.sbcindustry.com; ICC: www.iccsafe.org"

PLT TYP. Wave	Design Crit: FBC2010Res/TPI-2007 (STD) FT/RT=20%(0%)/10(0)	Scale = 5"/Ft.
	FL / - / 5 / - / - / R / -	
	TC LL	20.0 PSF
	TC DL	10.0 PSF
	BC DL	10.0 PSF
	BC LL	0.0 PSF
	TOT.LD.	40.0 PSF
	DUR.FAC.	1.25
	SPACING	24.0"
	REF	R9114- 39507
	DATE	10/11/12
	DRW	HCUSR9114 12285001
	HC-ENG	WHK/WHK
	SEQN-	22392
	JREF-	1UQ89114Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

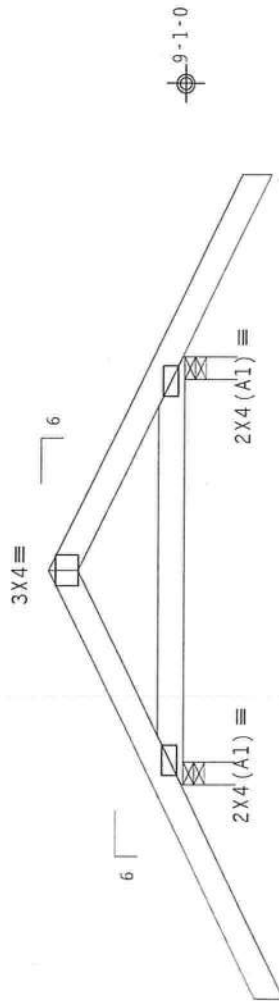
Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=363 U=16 W=3"
RL=57/-58

R=315 U=8 W=3"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

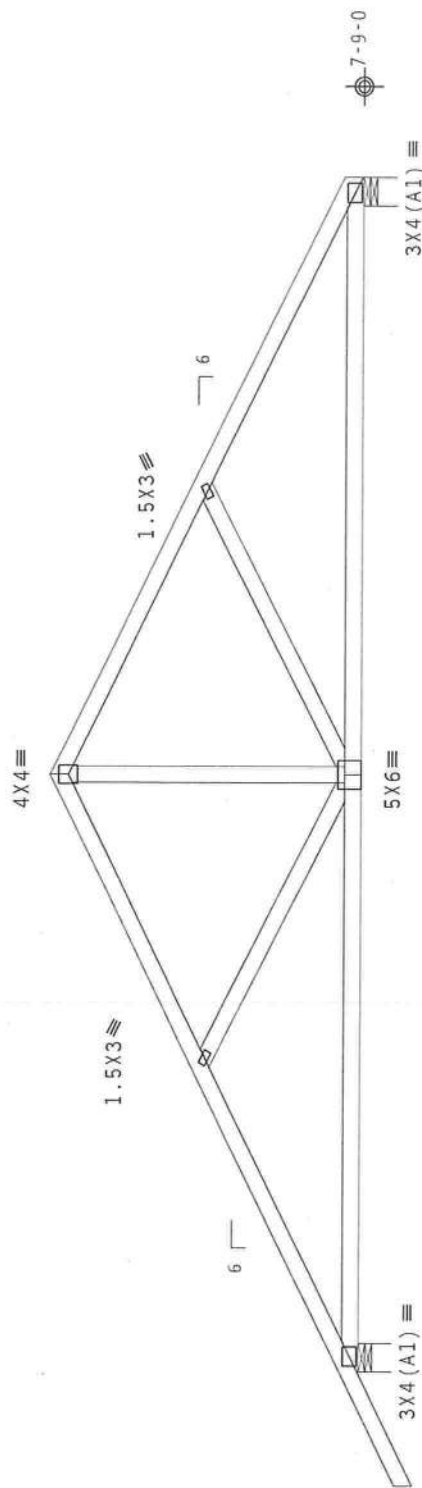
Scale = 5"/Ft.

ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!		**IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.	
		Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) practices used to performing these functions. Installers shall provide temporary bracing and shoring. Top chord shall have properly attached structural sheathing and bolting. All connections shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.			
		The Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design shown on this drawing. ITWBCG shall not be responsible for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and install all required bracing details, unless noted otherwise. Refer to drawings IWB-2 for standard plate positions. A seal of approval drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: ITWBCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.sbcindustry.com; ICC: www.iccsafe.org			
		IC LL	20.0 PSF	REF	R9114- 39508
		TC DL	10.0 PSF	DATE	10/11/12
		BC DL	10.0 PSF	DRW	HCUSR9114 12285002
BC LL	0.0 PSF	HC-ENG	WHK/WHK		
TOT.LD.	40.0 PSF	SEON-	22393		
DUR.FAC.	1.25				
SPACING	24.0"	JREF-	1U089114Z02		

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



-21-0-0 Over 2 Supports

R=857 U=26 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = .3125"/Ft.

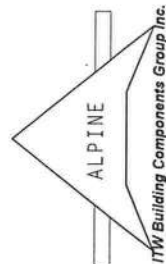
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSP (Building Component Safety Information, by TPI and WCA) and the manufacturer's instructions prior to performing these functions. Installers shall provide temporary bracing per BCSP. Trusses shall be installed in accordance with the design and specifications of the manufacturer. Trusses shall have a properly attached rafter ceiling. Locations shown for permanent lateral restraint for trusses shall have a bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (INFORMCS) shall not be responsible for any deviation from any failure to build the Truss in compliance with ANSI/TPI 1, or for handling, shipping, installing, fabricating or trusses. Apply plates to each Truss and position as shown above and on the Joist and Rafter drawings. Trusses shall be installed in accordance with the design and specifications of the manufacturer, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A steel member shall be used for the rafter ceiling. The drawing indicates acceptance of professional engineering responsibility solely for the design of this truss. The use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI section 2.2.

For more information, contact:
general notes page: TPI-BCG: www.tpiinfo.org; WCA: www.wcainc.com; BCSP: www.bcspsafety.com

IC LL	20.0 PSF	REF R9114- 39509
TC DL	10.0 PSF	DATE 10/11/12
BC DL	10.0 PSF	DRW HCUSR9114 12285003
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	40.0 PSF	SEQN- 22395
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UQ89114Z02



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP-#1_12A
Bot chord 2x4 SP-#1_12A
Webs 2x4 SP-#3_12A

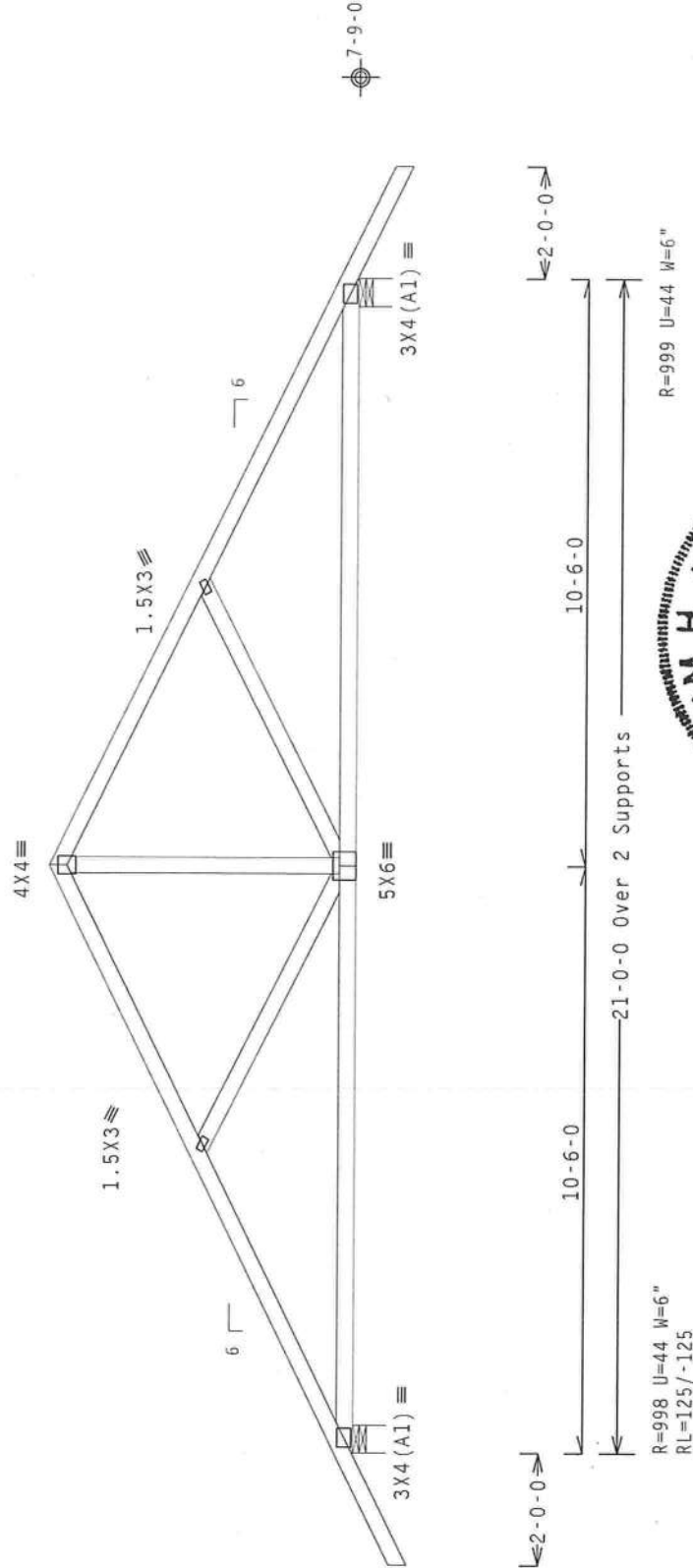
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STO)

Scale = .3125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET:
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and BCSI) for safety and bracing details. Truss installers shall provide temporary bracing for all trusses. Truss installers shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the bracing of trusses. Apply plates to each face of truss and position as shown above and on the details, unless noted otherwise. Refer to drawings 160A-7 for standard plate positions. A section drawing or cover page listing this drawing, the suitability and use of this design for any structural application, and the responsibility of the user, shall be provided with the truss. For more information see: general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpiusa.com; ITGC: www.itgcusa.com; ITCC: www.itccsafe.org

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WILLIAM H. KRICK
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
10/11/2012

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R9114- 39510
DATE	10/11/12
DRW	HCUSR9114 12285004
HC-ENG	WHK/WHK
SEON	22397
JREF	1U089114Z02

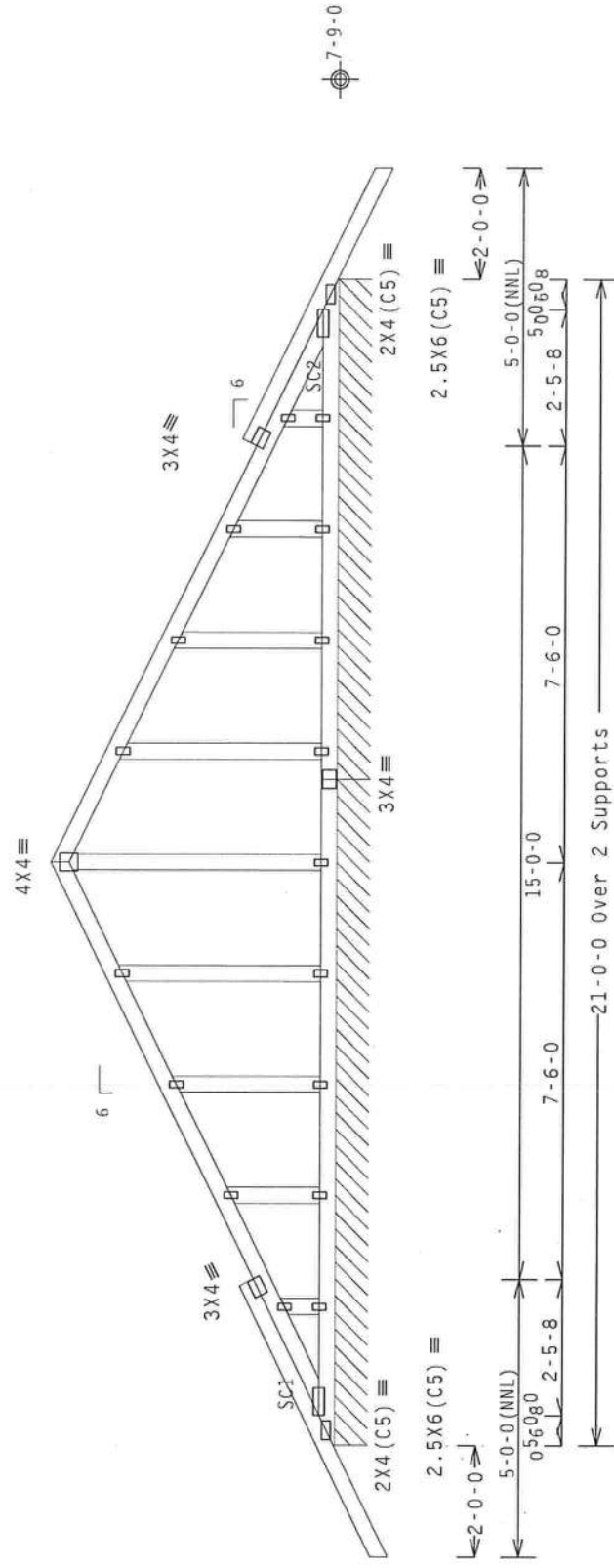
Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.
See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.
In lieu of structural panels use purlins to brace TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpf(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface. plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



R=91 PLF U=1 PLF W=12-0-0
RL=10/-10 PLF

R=101 PLF U=8

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = .3125"/Ft.

ALPINE rw Building Components Group Inc. Haines City, FL 33844 FL COA #0278	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET! FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.		No. 70861	10/11/2012	STATE OF FLORIDA PROFESSIONAL ENGINEER		REF R9114- 39511
	Trusses require extreme care in fabricating, handling, shipping, installation and bracing. Follow the latest edition of RCSI (Building Component Safety Information, by TPI and MICA) practices prior to performing these functions. Installers shall provide temporary bracing per RCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraining bracing shall have bracing installed per RCSI sections B3, B7 or B10, as applicable.				TC LL 20.0 PSF	DATE 10/11/12	
	17th Building Components Group Inc. (17BGC) shall not be responsible for any deviation from the design or any failure to build the Truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or assembly. All plates shall be attached to each face of truss and position as shown above and on the drawings. All bracing shall be installed in accordance with the design and on the drawings. All bracing details, unless noted otherwise, shall be in accordance with the design and on the drawings. The drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: general notes page. 17B-BCG; www.17bdcg.com; TPI: www.tpinet.org; MICA: www.structure.com; RCI: www.rciusa.com				TC DL 10.0 PSF		
					BC DL 10.0 PSF		
					BC LL 0.0 PSF	HC-ENG WHK/WHK	
					TOT.LD. 40.0 PSF	SEQN- 292164	
				DUR.FAC. 1.25			
				SPACING 24.0"	JREF- 1U089114702		

(12-261--Jerry Castagna Constructi Willmann Res. -- Columbia County, FL - E1 4'8"8 Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFG.

Top chord 2x4 SP-#1_12A
Bot chord 2x4 SP-#1_12A

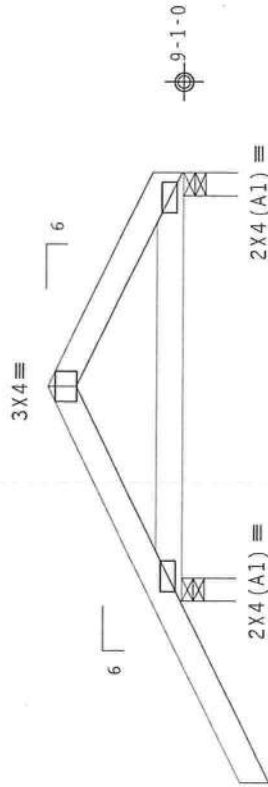
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpf(+/-)=0.18

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



2-0-0

2-4-4

4-8-8 Over 2 Supports

R=361 U=11 W=3"

RL=-41

R=160 U=0 W=3"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = .5"/Ft.

 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012		TC LL 20.0 PSF	REF R9114- 39512
	No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012		TC DL 10.0 PSF	DATE 10/11/12
	No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012		BC DL 10.0 PSF	DRW HCUSR9114 12285006
	No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012		BC LL 0.0 PSF	HC-ENG WHK/WHK
	No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012		TOT.LD. 40.0 PSF	SEQN- 22402
DUR.FAC. 1.25				
			SPACING 24.0"	JREF- 1U089114Z02

(12-261--Jerry Castagna Constructi Willmann Res. -- Columbia County , FL - E3 4'5"8 Common)

Top chord	2x4	SP_#1_12A
Bot chord	2x4	SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

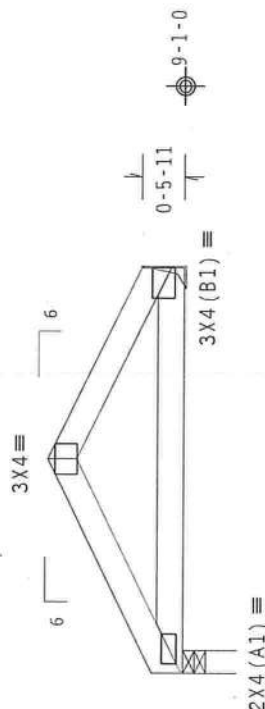
Bottom chord checked for 10.00 psf non-concurrent live load.

MFERS loads based on trusses located at least 30.00 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



$\overbrace{2-4-4}^{2-1-4}$
 $\overbrace{4-5-8}^{2 \text{ Supports}}$
 $R=184 \quad U=0$
 $H=H1$

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

:1 FL/-/5/-/-/R/-/

Scale = .5"/Ft.

****IMPORTANT****
 FURNISH THIS** HEAD AND FOLLOW ALPHABETS OF THIS SHEET
 FURNISH THIS** TO ALL CONTRACTORS INCLUDING INSTALLERS.

INCESS, require extreme care in fabricating, handling, shipping, installing and bracing. The following instructions are to be followed by all installers and fabricators prior to performing these functions. Installers must be properly trained in the practices noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent, lateral restraint of bracing installed per BCSP sections B3, B7 and B10, as applicable.

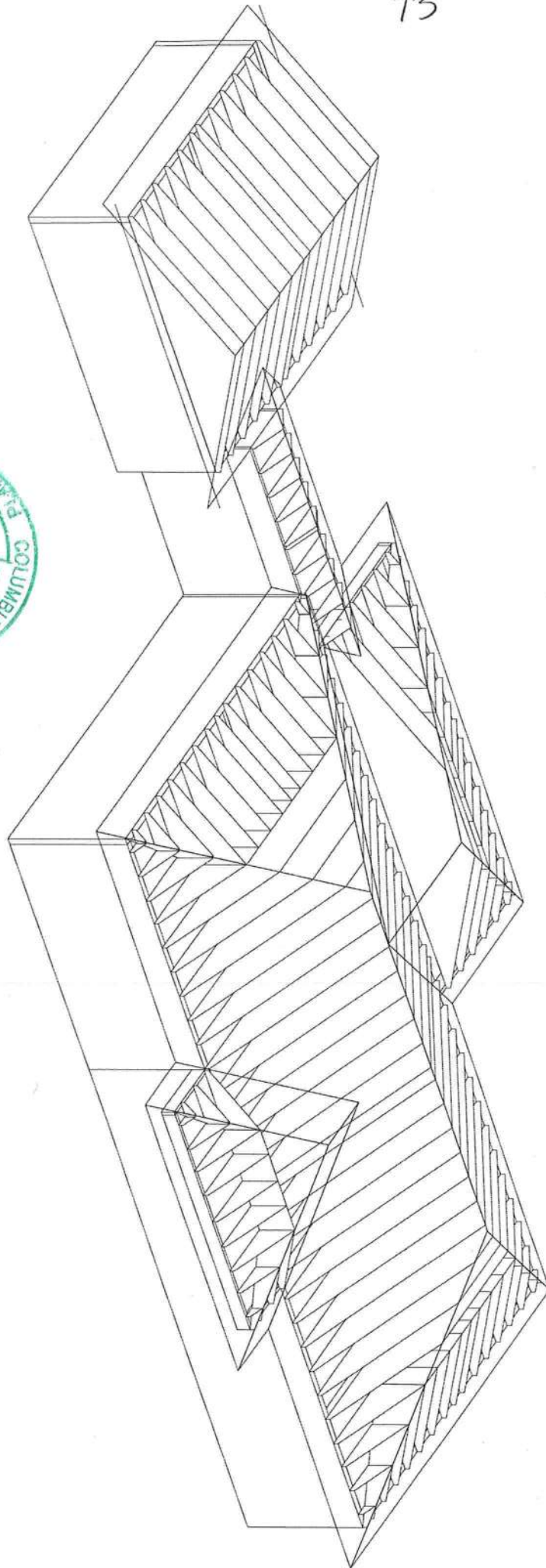
ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

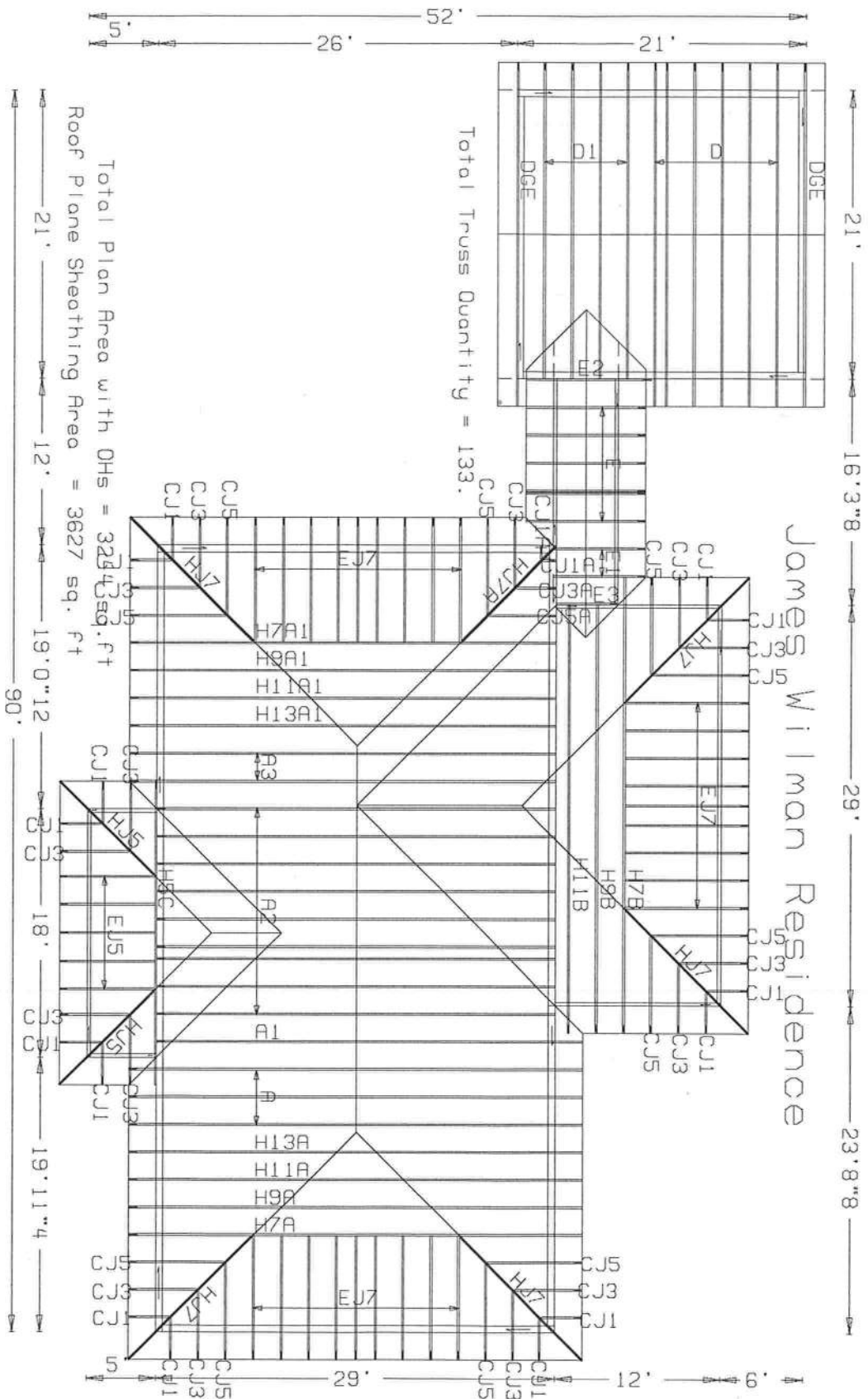
Haines City, FL 33844
FL COA #0 278

10/11/2012

SPACING 24 0"

JREF- 1UQ89114Z02





James William Residence

Total Truss Quantity = 133.

Total Plan Area with DHS = 3243 sq. ft

PAGE NO: 1 OF 1	JOB NO: 12-261	Location: Columbia County JOB DESCRIPTION: Jerry Castagna Constructi /: Willmann Res.	
--------------------	-------------------	---	--

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: IUQ89114Z0211162123



Truss Fabricator: Anderson Truss Company
Job Identification: 12-261--Jerry Castagna Constructi Willmann Res. -- Columbia County
Truss Count: 34
Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Versions 10.03, 12.02.
Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 130 MPH ASCE 7-10 -Closed

Notes:

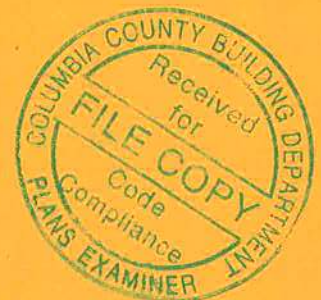
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 14015EC1-GBLLETIN-BRCLBSUB-CNNAILSP-

#	Ref	Description	Drawing#	Date
1	39507--E	4'8"8 Common	12285001	10/11/12
2	39508--E2	4'8"8 Common	12285002	10/11/12
3	39509--D1	21' Common	12285003	10/11/12
4	39510--D	21' Common	12285004	10/11/12
5	39511--DGE	21' Gable	12285005	10/11/12
6	39512--E1	4'8"8 Common	12285006	10/11/12
7	39513--E3	4'5"8 Common	12285007	10/11/12
8	39514--H9B	29' Stepdown	12285008	10/11/12
9	39515--CJ1	1' Jack	12285009	10/11/12
10	39516--EJ5	4'10"8 End J	12285010	10/11/12
11	39517--A2	29' Common	12285011	10/11/12
12	39518--H9A1	29' Stepdown	12285012	10/11/12
13	39519--H11A1	29' Stepdown	12285013	10/11/12
14	39520--H13A1	29' Stepdown	12285014	10/11/12
15	39521--A1	29' Common	12285015	10/11/12
16	39522--A	29' Common	12285016	10/11/12
17	39523--H13A	29' Stepdown	12285017	10/11/12
18	39524--H11A	29' Stepdown	12285018	10/11/12
19	39525--H9A	29' Stepdown	12285019	10/11/12
20	39526--CJ1A	1' Jack	12285020	10/11/12
21	39527--A3	29' Common	12285021	10/11/12
22	39528--CJ3	3' Jack	12285022	10/11/12
23	39529--CJ5	5' Jack	12285023	10/11/12
24	39530--EJ7	7' End Jack	12285024	10/11/12
25	39531--CJ5A	5' Jack	12285025	10/11/12
26	39532--CJ3A	3' Jack	12285026	10/11/12
27	39533--H11B	29' Stepdown	12285027	10/11/12
28	39534--HJ7A	9'8"11 Hip	12285028	10/11/12
29	39535--HJ7	9'10"13 Hip	12285040	10/11/12
30	39536--H7A	29' Stepdown	12285029	10/11/12
31	39537--H7A1	29' Stepdown	12285030	10/11/12
32	39538--HJ5	6'10"12 Hip	12285041	10/11/12
33	39539--H5C	18' Stepdown	12285042	10/11/12
34	39540--H7B	29' Stepdown	12285043	10/11/12



Top chord 2x4 SP-#1-12A
Bot chord 2x4 SP-#1-12A
Webs 2x4 SP-#3-12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

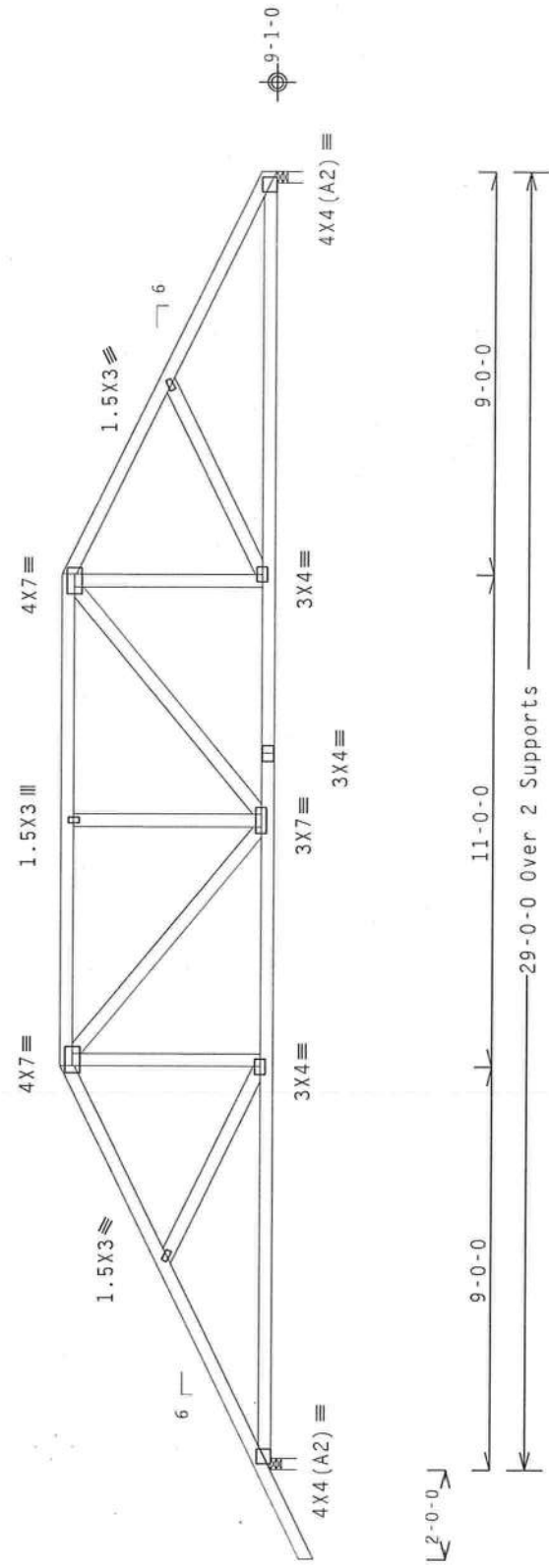
130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

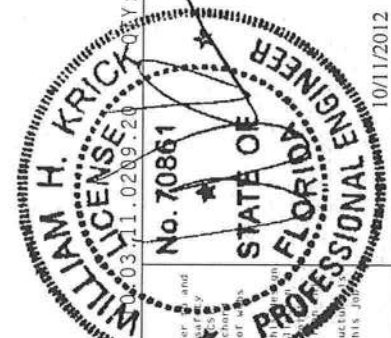


R=1333 U=63 W=3"
RL=90/-99

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

Scale = .25"/ft.



ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. **IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety) information, by TPI and MICA, for details on proper bracing practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral bracing of trusses shall have bracing installed per BCSI Sections B3, B7 or B10, as applicable. The Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design shown in accordance with ANSI/TPI 1, or for handling, shipping, installation or use of this design. The responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: ITWBCG, www.itwbcg.com, TPI: www.tpiinc.org; MICA: www.sdcindustry.com; ICC: www.iccsafe.org No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/11/2012									
	TC LL 20.0 PSF									
	TC DL 10.0 PSF									
	BC DL 10.0 PSF									
	BC LL 0.0 PSF									
TOT.LD. 40.0 PSF										
DUR.FAC. 1.25										
SPACING 24.0"										
JREF- 1UQ89114Z02										

(12-261--Jerry Castagna Constructi Willmann Res. -- Columbia County . FL - CJI 1' Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP-#1_12A
Bot chord 2x4 SP-#1_12A

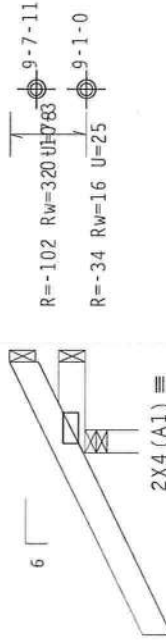
Lumber grades designated with "12A" use design values approved
1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

130 mph wind, 15.00 ft mean hgt. ASCE 7-10, CLOSED bldg. Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member
design.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



1'-0-0 Over 3 Supports

R=353 U=56 W=3"
RL=29/-24

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20% (0%)/10 (0)

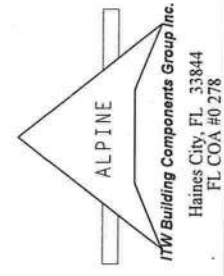
PLT TYP. Wave

Scale = 5"/Ft.

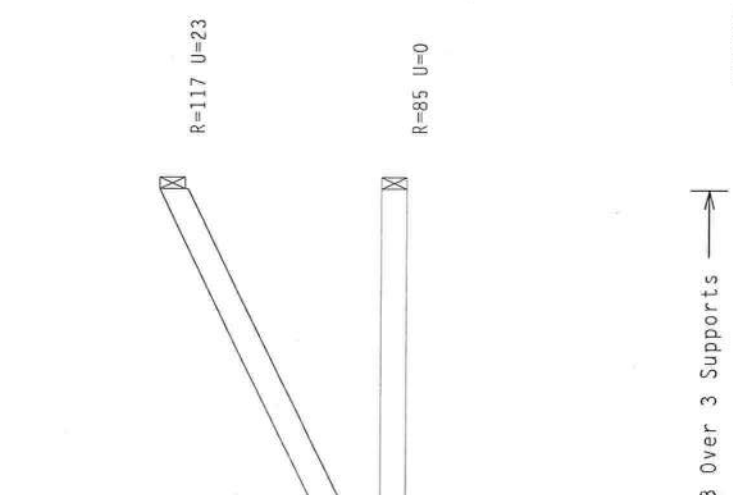


TC LL	20.0 PSF	REF	R9114- 39515
TC DL	10.0 PSF	DATE	10/11/12
BC DL	10.0 PSF	DRW	HCUSR9114 12285009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	22407
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UQ89114Z02

IMPORTANT!! FOLLOW THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and ALSC) for details. The contractor shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of walls shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of trusses. Apply plates to each face of truss and position as shown above and on the details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal, drawing or cover page listing this design shall be provided to the contractor. The contractor shall be responsible for the design shown. The availability and use of this design for any structural application is the responsibility of the contractor. For more information see: this job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; ALSC: www.alscsafe.org; ICC: www.iccsafe.org



Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.
Bottom chord checked for 10.00 psf non-concurrent live load.
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 6Cpf (+/-)=0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20% (0%) / 10 (0)

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

10/11/2012

Professional Engineer
STATE OF FLORIDA
No. 70861
WILLIAM H. KRICK
LICENSE

Scale = .5" / Ft.

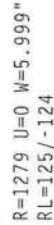
IC LL	20.0 PSF	REF	R9114- 39516
TC DL	10.0 PSF	DATE	10/11/12
BC DL	10.0 PSF	DRW	HCUSR9114 12285010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEON-	22410
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UQ89114Z02

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



9 FL/-/5/-/-/R/-

Scale = .25"/Ft.

DATE	TIME	LOCATION	REMARKS
1/1/77	17N	4TEN	1TCC

DATE 10/11/12

HC-ENG WHK/WHK

CCON 00410

SEC- 22412

JREF- 111089114702

207475000000 13110

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind loads and reactions based on MMFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 6cpi (+/-)=0.18

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Scale = .25" / Ft.

REF R9114- 39518

DATE 10/11/12

DRW HCUSR9114 12285012

HC-ENG WHK /WHK

SEQN - 22414

JREF - 1U089114Z02

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)

FT/RT=20%(0%)/10(0)

FL/-/5/-/R/-

TC LL 20.0 PSF

TC DL 10.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

TOT.LD. 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"

10/11/2012

William H. Krick

No. 70861

FLORIDA

PROFESSIONAL ENGINEER

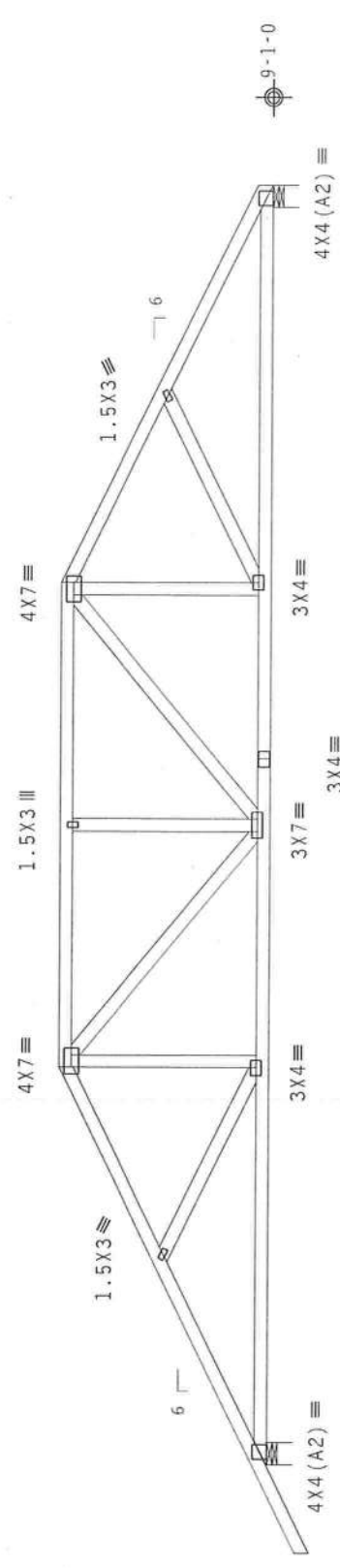
10/11/2012

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278



L2-0-0

9-0-0

11-0-0

29-0-0 Over 2 Supports

9-0-0

9-0-0

9-0-0

9-0-0

9-0-0

9-0-0

9-0-0

9-0-0

9-0-0

9-0-0

R=1189 U=44 W=6"

R=1333 U=63 W=5.999"

RL=91/-99

Top chord 2x4 SP-#1-12A
Bot chord 2x4 SP-#1-12A
Webs 2x4 SP-#3-12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

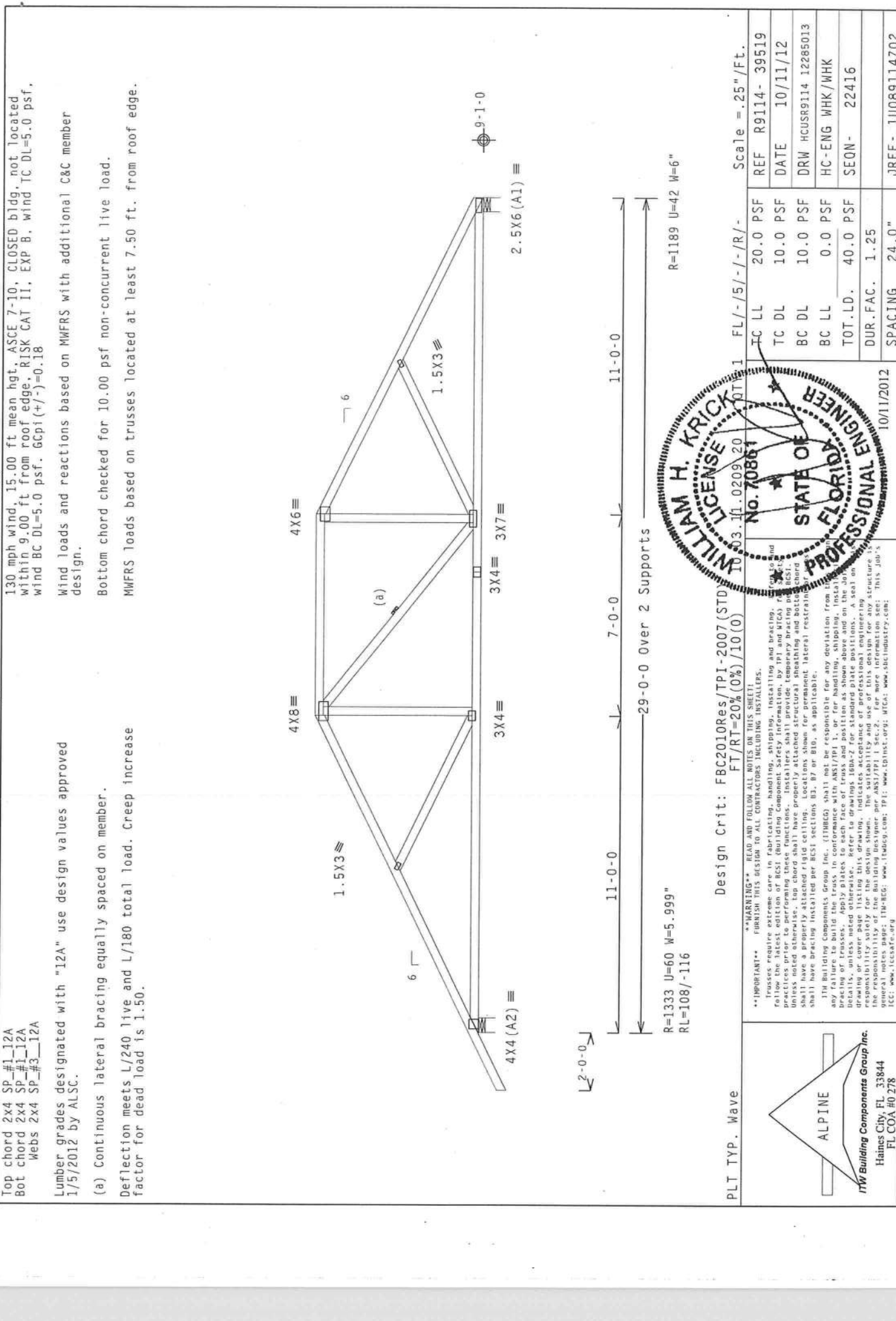
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



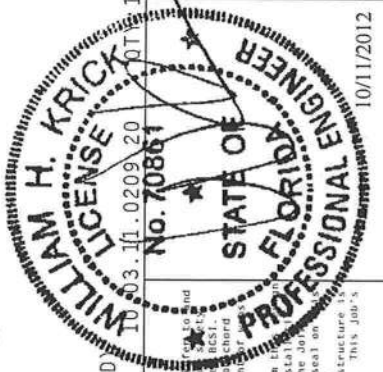
R=1333 U=60 W=5.999"
RL=108/-116

29'-0'-0 Over 2 Supports

R=1333 U=60 W=5.999"
RL=108/-116

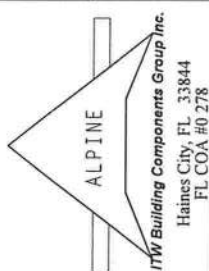
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = .25" / Ft.



****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI Building Component Safety Information by BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of this truss. Apply plates to each face of truss and position as shown above and on the job. The responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's general notes page: ITW-BCG; www.itwbcg.com; TPI: www.tpiinc.org; WCA: www.steelfabrication.com; ICC: www.iccsafe.org

REF	R9114-	39519
DATE	10/11/12	
DRW	HCUSR9114	12285013
HC-ENG	WHK/WHK	
SEQN-	22416	
DUR.FAC.	1.25	
SPACING	24.0"	
JREF-	1U089114202	



PLT TYP. Wave

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Design Crit: $\text{FBC2010Res/TPI-2007(STD)}$
 $\text{FT/RT}=20\%(0\%)/10(0)$

Scale = .25"/Ft.

[illegible]