Septic Tank 07-0375

Columbia County Building Permit Application

For Office Use Only Application # 0701-103 Date Received 1/30 By JW Permit # 25494

Application Approved by - Zoning Official BLK Date 3.01.07 Plans Examiner OK JTH Date 1-30-07

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments

☑ NOC ☑ EH ☐ Deed or PA ☑ Site Plan ☑ State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Charese Norton Phone 386-752-3331 Fax 386-752-6427

Address 3367 S US Hwy 441, Ste 101, Lake City, FL 32025

Owners Name Steve & Dena Cribbs Phone 386-454-4226

911 Address 19498 S US Hwy 441, High Springs, FL 32643

Contractors Name James H. Norton Phone 386-752-3331

Address 3367 S. US. Hwy 441, Ste 101, Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tim Delbene 192 SW Sagewood Gln, Lake City, FL 32024 Mark Disosway POB 868, Lake City, FL 32056

Mortgage Lenders Name & Address 1st Federal, P.O. Box 2029, Lake City, FL 32056

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-6S-17-09834-103 Estimated Cost of Construction 115000.00

Subdivision Name Rumph Farms Lot 3 Block — Unit — Phase —

Driving Directions 441 South to Oleno State Park, 2 lots North of South Entrance to Park.

Type of Construction SFD, New Home Const Number of Existing Dwellings on Property 0

Total Acreage 5.2 Lot Size 3 Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 450' ✓ Side 47' ✓ Side 115' ✓ Rear 514' ✓

Total Building Height 17'6" Number of Stories 1 Heated Floor Area 1740 TOTAL 2416 Roof Pitch 4/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James H Norton
Owner Builder or Authorized Person by Notarized Letter

James H Norton
Contractor Signature
Contractors License Number RB0031780
Competency Card Number 5553
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

PATRICIA T. PEELER
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

Sworn to (or affirmed) and subscribed before me this 25 day of Jan 20 07.

Personally known ✓ or Produced Identification

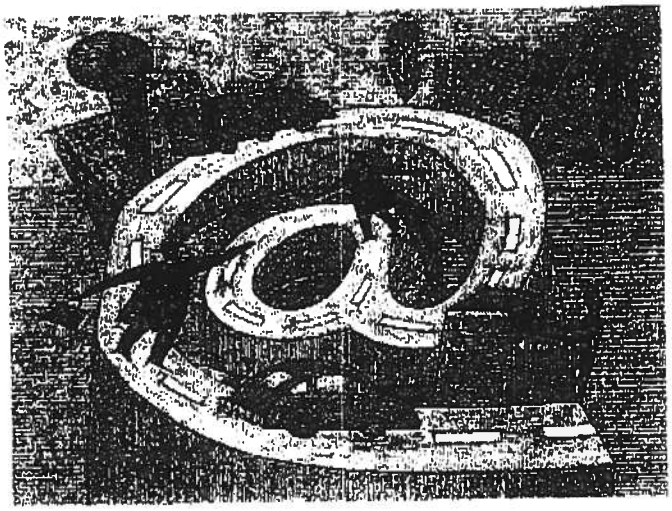
Patricia Peeler
Notary Signature

(Revised Sept. 2006)

Will advised Charese on 1/31/07

FLORIDA DEPARTMENT OF TRANSPORTATION

LAKE CITY MAINTENANCE
PHONE (386) 961-7180
FAX (386) 961-7183



FACISIMILE TRANSMITTAL

DATE: 2-6-07

TO: Col Co Building & Zoning Dept.

ATTN: Janice Williams

FROM: Dale Cray F.D.O.T Permits, Dept

SUBJECT: Mr & Mrs. Cribbs (Res. Dwl)

COMMENTS: This driveway is a joint-use Res. drive between Mr & Mrs. Cribbs and adjacent landowner, and it meet code, if any question please call 961-7146 or 961-7193

FAX MEMORANDUM

MEMORANDUM

FLORIDA DEPARTMENT OF TRANSPORTATION

To: Mr. John Kerce, Dept. Director Columbia Co. Building & Zoning Dept. **Fax No: 386-758-2160**	**From:** Dale L. Cray, FDOT Permits Insp. **Date: 2-06-2007** Fax **No. 386-961-7183** **Attention: Col Co. Building Zoning Dept.**

( ) Sign and return. ( ) For your files. ( ) Please call me. **(XX)** FYI ( ) For Review

REF: Comm. D/W / Inspected On:2- 06-2007
PROJECT: Existing Joint-Use D/W / Res. Access S.R. 25 (S)
PARCEL ID No: N/A Permit No : N/A Sec No : 29030
MILE POST N/A +- Engineer: N/A

Mr. Kerce:

Please accept this as our legal notice of final passing inspection for an Existing Residential Joint Use Driveway for Steve & Deana Cribbs 19498 S US Hwy 441, LAKE CITY, FL.32055

This access has been inspected and meets FDOT Standard Requirements.

If further information is required on this project please do not hesitate to contact this office for additional access permitting information details. My office number is 961-7193 or 961-7146.

Sincerely,

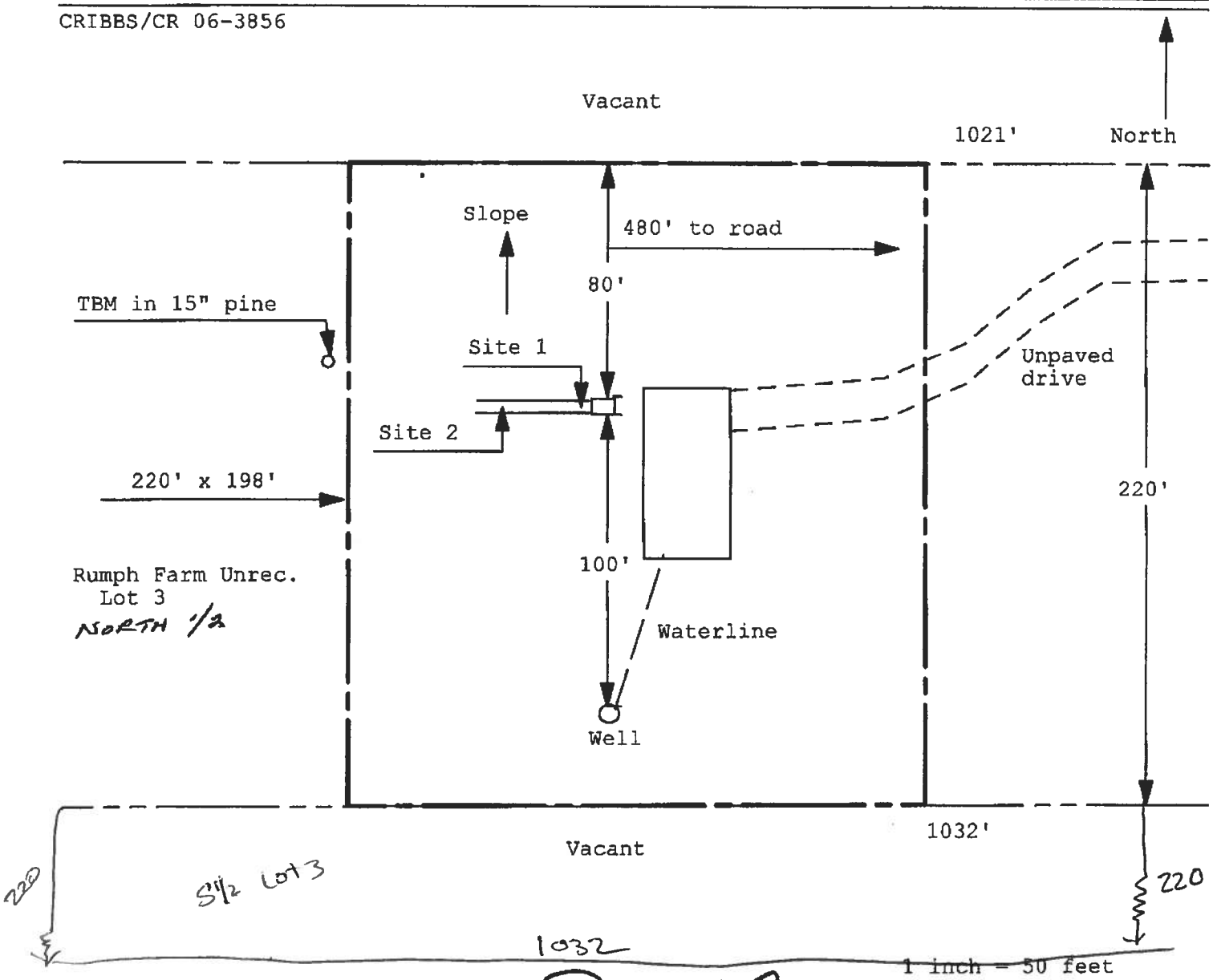
Dale L. Cray
Access Permits Inspector

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-00075N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

CRIBBS/CR 06-3856



Site Plan Submitted By Paul Lloyd **Date** 1/25/07

Plan Approved ✓ **Not Approved** ____ **Date** 2/5/07

By _____ Columbia **CPHU**

Notes: _____

Columbia County Property Appraiser

DB Last Updated: 12/29/2006

Parcel: 33-6S-17-09834-103

2007 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

<< Prev | Search Result: 8 of 10 | Next >>

Owner & Property Info

Owner's Name	CRIBBS STEPHEN J & DENA L		
Site Address	RUMPH FARMS S/D UNREC		
Mailing Address	924 SW HORSESHOE LOOP FT WHITE, FL 32038		
Use Desc. (code)	NO AG ACRE (009900)		
Neighborhood	33617.01	**Tax District**	3
UD Codes	MKTA02	**Market Area**	02
Total Land Area	10.400 ACRES		
Description	COMM INTERS W R/W US-41 & S LINE OF SEC, RUN N ALONG R/W 896.71 FT FOR POB, CONT N 441.17 FT, W 1032.10 FT, S 441.04 FT, E 1021.28 FT FOR POB. (AKA LOT 3 RUMPH FARMS S/D UNREC) ORB 887-1467, 924-1891,		

GIS Aerial

Property & Assessment Values

Mkt Land Value	cnt: (1)	$117,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB	cnt: (0)	$0.00

Just Value	$117,000.00
Class Value	$0.00
Assessed Value	$117,000.00
Exempt Value	$0.00

Value		
Total Appraised Value		$117,000.00

Total Taxable Value	$117,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
4/13/2001	924/1891	WD	V	Q		$33,000.00
5/21/1999	887/1467	WD	V	Q		$31,200.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	10.400 AC	1.00/1.00/1.00/.75	$11,250.00	$117,000.00

Columbia County Property Appraiser DB Last Updated: 12/29/2006

<< Prev 8 of 10 Next >>

Disclaimer

This information was derived from data which was compiled by the Columbia County Property Appraiser's Office solely for the government purpose of property assessment. The information shown is a **work in progress** and should

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Cribbs Residence**	Builder:	**Norton Home Imp.**
Address:	**US Highway 41**	Permitting Office:	**Columbia County**
City, State:	**Lake City, FL 32055-**	Permit Number:	
Owner:	**Cribbs**	Jurisdiction Number:	~~**121000**~~ 24000
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		1740 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	174.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 176.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1171.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1740.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 10.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 14.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.10

Total as-built points: 20054
Total base points: 26471

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 11/20/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **US Highway 41, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1740.0	20.04	6276.5	Double, Clear	N	2.0	3.0	3.0	19.20	0.78	44.8	
				Double, Clear	S	2.0	3.0	6.0	35.87	0.59	127.0	
				Double, Clear	E	2.0	7.0	60.0	42.06	0.89	2235.8	
				Double, Clear	E	10.0	8.0	45.0	42.06	0.46	876.6	
				Double, Clear	W	14.0	7.0	30.0	38.52	0.41	473.0	
				Double, Clear	W	2.0	7.0	30.0	38.52	0.89	1024.8	
				As-Built Total:				**174.0**			**4782.1**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1171.0	1.50		1756.5	
Exterior	1171.0	1.70	1990.7									
Base Total:	**1171.0**		**1990.7**	**As-Built Total:**				**1171.0**			**1756.5**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	21.0	2.40	50.4	Exterior Insulated				21.0	4.10		86.1	
Exterior	63.0	6.10	384.3	Exterior Insulated				21.0	4.10		86.1	
				Adjacent Insulated				21.0	1.60		33.6	
				Exterior Insulated				21.0	4.10		86.1	
Base Total:	**84.0**		**434.7**	**As-Built Total:**				**84.0**			**291.9**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1740.0	1.73	3010.2	Under Attic			30.0	1740.0	1.73 X 1.00		3010.2	
Base Total:	**1740.0**		**3010.2**	**As-Built Total:**				**1740.0**			**3010.2**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	176.0(p)	-37.0	-6512.0	Slab-On-Grade Edge Insulation			0.0	176.0(p	-41.20		-7251.2	
Raised	0.0	0.00	0.0									
Base Total:			**-6512.0**	**As-Built Total:**				**176.0**			**-7251.2**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1740.0	10.21	17765.4					1740.0	10.21		17765.4	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **US Highway 41, Lake City, FL, 32055-**	PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**22965.5**	**Summer As-Built Points:**					**20354.9**
Total Summer Points X	System Multiplier =	Cooling Points	Total Component X	Cap Ratio X	Duct Multiplier (DM x DSM x AHU) X	System Multiplier X	Credit Multiplier =	Cooling Points
22965.5	**0.4266**	**9797.1**	20354.9 **20354.9**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.244 **0.244**	0.902 **0.902**	5095.1 **5095.1**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **US Highway 41, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1740.0	12.74	3990.2	Double, Clear	N	2.0	3.0	3.0	24.58	1.01	74.7	
				Double, Clear	S	2.0	3.0	6.0	13.30	2.06	164.7	
				Double, Clear	E	2.0	7.0	60.0	18.79	1.05	1178.8	
				Double, Clear	E	10.0	8.0	45.0	18.79	1.35	1139.7	
				Double, Clear	W	14.0	7.0	30.0	20.73	1.22	760.1	
				Double, Clear	W	2.0	7.0	30.0	20.73	1.03	641.3	
				As-Built Total:				**174.0**			**3959.4**	
WALL TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM	=	Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1171.0	3.40		3981.4	
Exterior	1171.0	3.70	4332.7									
Base Total:	**1171.0**		**4332.7**	**As-Built Total:**				**1171.0**			**3981.4**	
DOOR TYPES	Area X	BWPM	= Points	Type				Area X	WPM	=	Points	
Adjacent	21.0	11.50	241.5	Exterior Insulated				21.0	8.40		176.4	
Exterior	63.0	12.30	774.9	Exterior Insulated				21.0	8.40		176.4	
				Adjacent Insulated				21.0	8.00		168.0	
				Exterior Insulated				21.0	8.40		176.4	
Base Total:	**84.0**		**1016.4**	**As-Built Total:**				**84.0**			**697.2**	
CEILING TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1740.0	2.05	3567.0	Under Attic			30.0	1740.0	2.05 X	1.00	3567.0	
Base Total:	**1740.0**		**3567.0**	**As-Built Total:**				**1740.0**			**3567.0**	
FLOOR TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM	=	Points	
Slab	176.0(p)	8.9	1566.4	Slab-On-Grade Edge Insulation			0.0	176.0(p	18.80		3308.8	
Raised	0.0	0.00	0.0									
Base Total:			**1566.4**	**As-Built Total:**				**176.0**			**3308.8**	
INFILTRATION	Area X	BWPM	= Points					Area X	WPM	=	Points	
	1740.0	-0.59	-1026.6					1740.0	-0.59		-1026.6	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **US Highway 41, Lake City, FL, 32055-**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**13446.1**	**Winter As-Built Points:**					**14487.2**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
13446.1	**0.6274**	**8436.1**	14487.2 **14487.2**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.432 **0.432**	0.950 **0.950**	6904.1 **6904.1**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **US Highway 41, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2746.00	8238.0	30.0	0.90	3		1.00	2684.98	1.00	8054.9
				As-Built Total:							**8054.9**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
9797	**8436**	**8238**	**26471**	**5095**	**6904**	**8055**	**20054**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

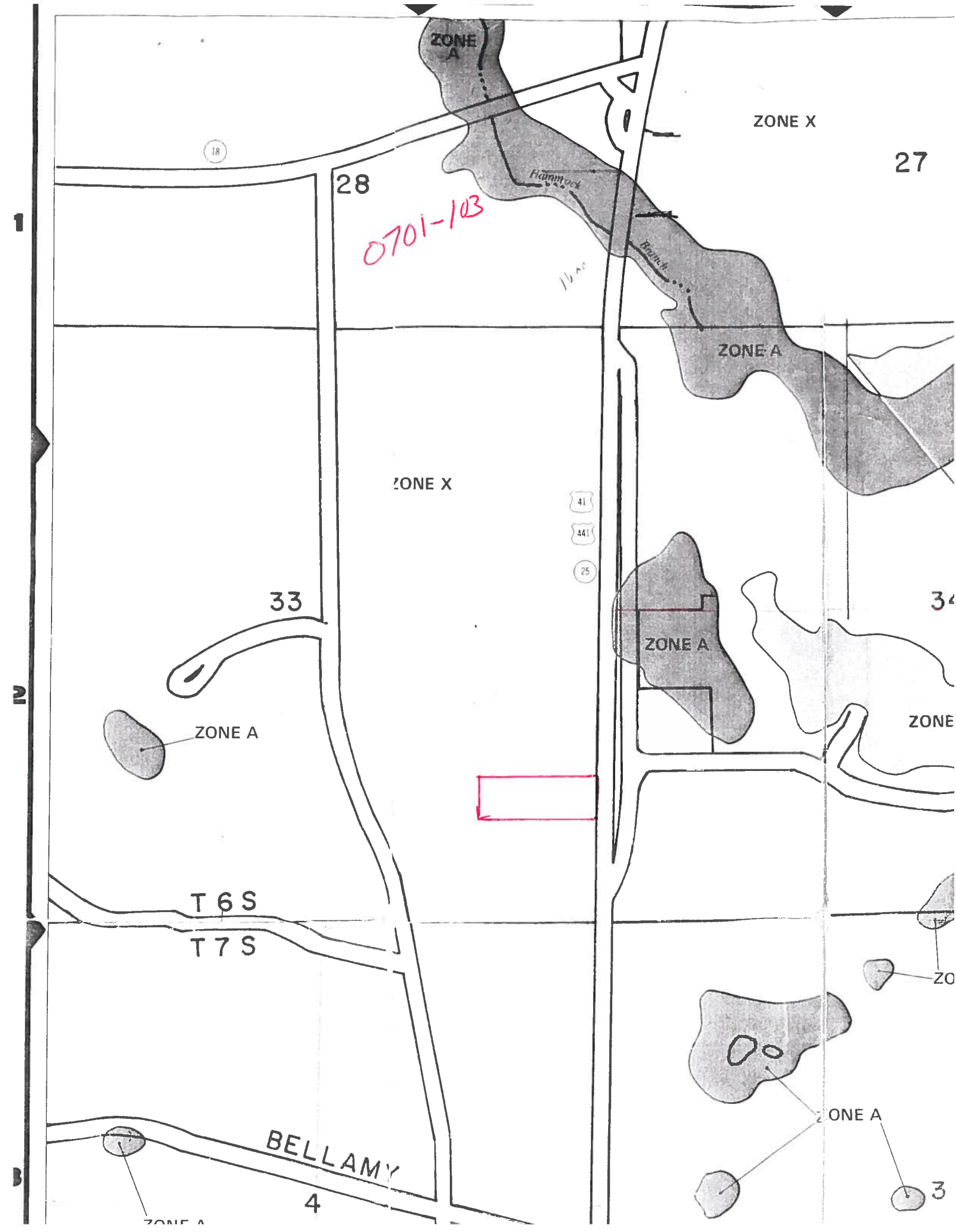
ADDRESS: **US Highway 41, Lake City, FL, 32055-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓


ZONE A
ZONE X
27
28
0701-103
Hammock
Branch
ZONE A
ZONE X
41
441
25
33
34
ZONE A
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ZONE A
BELLAMY
4
3

OFFICIAL RECORDS

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328
01-173
Grantee #1 S.S. No. [redacted]

Grantee #2 S.S. No. [redacted]

Property Appraiser's
Parcel Identification No.
09834-103

Recording Fee $ 10.50
Documentary [illegible] PUBLIC
RECORDS OF COLUMBIA COUNTY, FL

01-06833

'01 APR 17 PM 3:31

Documentary Stamp # 231.00
Intangible Tax 0
P. DeWitt Cason
Clerk of Court
By MK D.C.

COLUMBIA COUNTY CLERK CIRCUIT

WARRANTY DEED

THIS INDENTURE, made this 13th day of April, 2001, BETWEEN GARY D. MANNING, SR., and his wife, TERRIE A. MANNING, whose post office address is 6004 N.W. 111th Place, Alachua, FL 32615, of the County of Alachua, State of Florida, grantor*, and STEPHEN J. CRIBBS and his wife, DENA L. CRIBBS, whose post office address is Route 1, Box 1310, Ft. White, FL 32038, of the County of Columbia, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 3, RUMPH FARMS, described as follows:

That certain piece, parcel or tract of land situate, lying and being in the County of Columbia and State of Florida, known as Lot 3, a part of Section 33, Township 6 South, Range 17 East; described as: A part of the SE 1/4 of Section 33, Township 6 South, Range 17 East more particularly described as follows: Commence at a concrete monument marking the intersection of the West right-of-way of U.S. #41 and the South line of said Section 33 and run North 00°05'15" West along said West right-of-way, 896.71 feet for a point of beginning. Thence continue North 00°05'15" West, 441.17 feet, thence South 88°30'48" West, 1032.10 feet; thence South 01°29'35" East, 441.04 feet; thence North 88°30'48" East, 1021.28 feet to the Point of Beginning. Columbia County, Florida.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

Documentary Stamp # 99.75
Intangible Tax
P. DeWitt Cason
Clerk of Court
By MK D.C.

SUBJECT TO: Mortgage given to J. Quinton Rumph and his wife, Ann S. Rumph, dated May 21, 1999, and filed in Official Records Book 887, Page 1470, Columbia County public records, Florida. By accepting this deed, the grantees hereby assume and agree to pay the outstanding balance owed on said mortgage in the amount of $28,448.66.

0924 PG 1892

OFFICIAL RECORDS

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in our presence:

(First Witness)
Terry McDavid
Printed Name

GARY D. MANNING, SR. (SEAL)

(Second Witness)
DeEtte F. Brown
Printed Name

TERRIE A. MANNING (SEAL)

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 13th day of April, 2001, by GARY D. MANNING, SR., and his wife, TERRIE A. MANNING, who are personally known to me and who did not take an oath.

My Commission Expires:

Notary Public

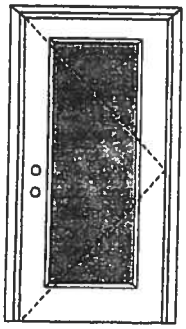
Terry McDavid
MY COMMISSION EXPIRES
JANUARY 16, 2002
#CC 680627
Bonded thru Troy Fain Insurance
NOTARY PUBLIC, STATE OF FLORIDA

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


160 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:

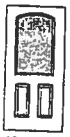

105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

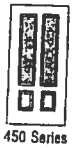
WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

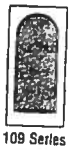
3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
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PREMDOR Collection Premium Quality Doors

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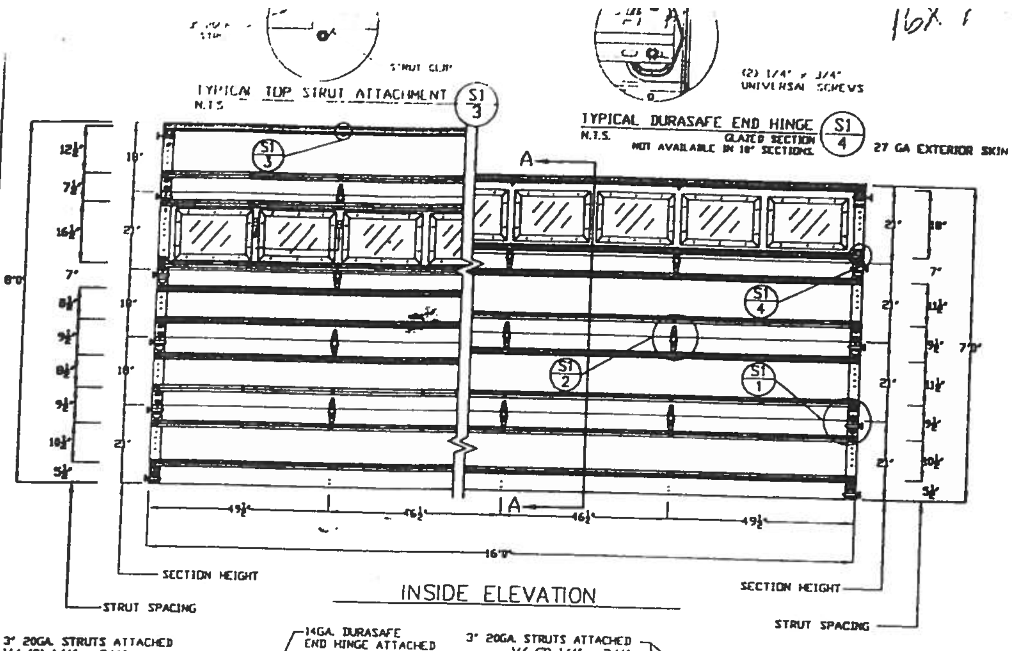
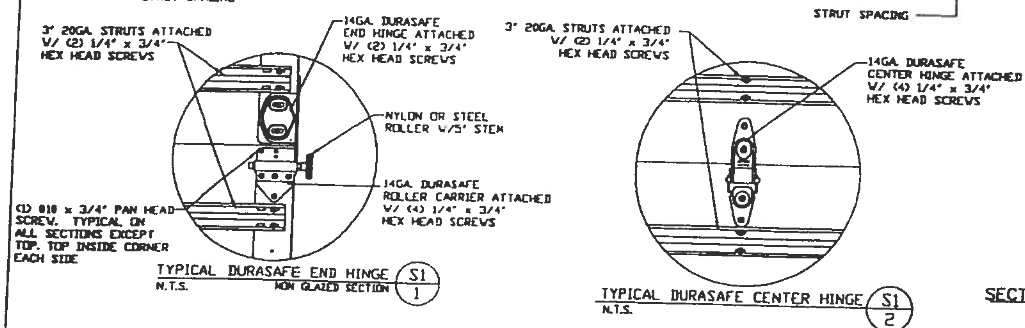
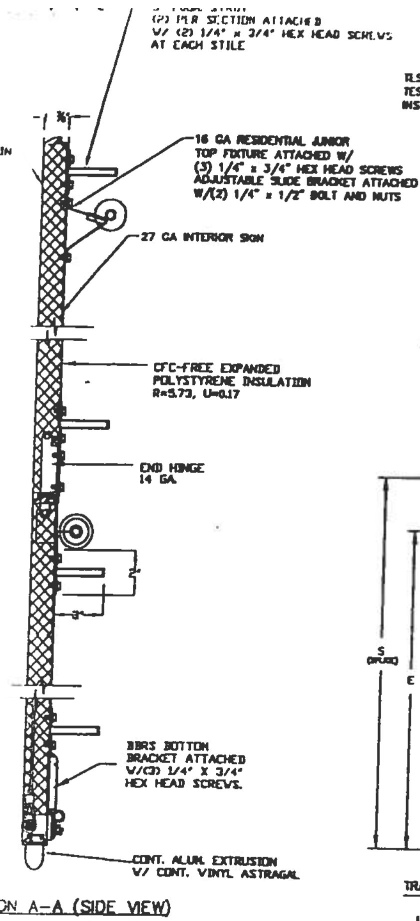
Masonite International Corporation

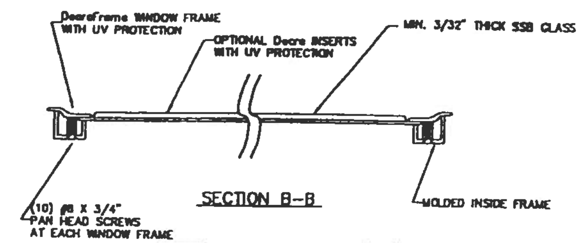


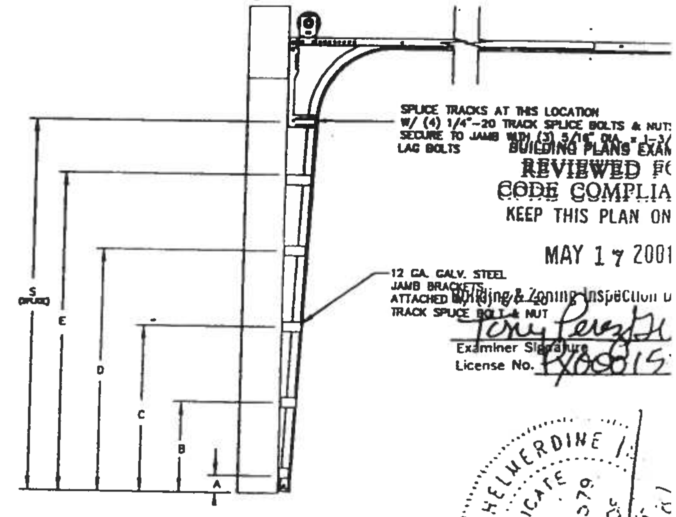




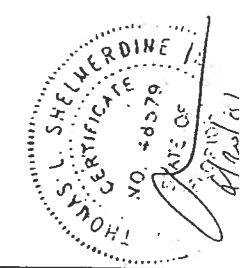
GLAZING OPTION CROSS SECTION

TEST No. SBC-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +48.5 PSF AND -51.9 PSF. BY COMPARISON, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' X 7' AND 16' X 8' MODEL 1500-D DOORS.



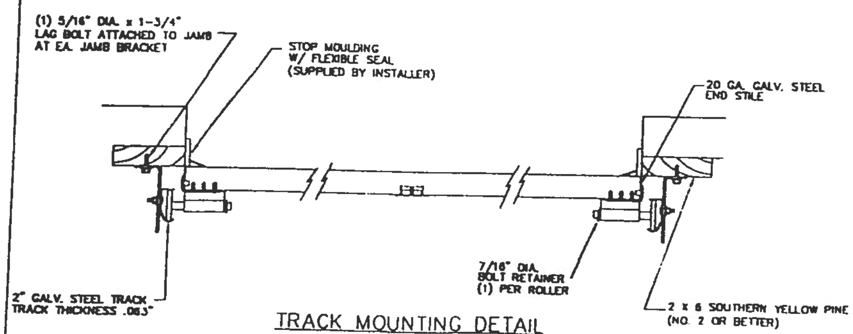


TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS



JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 18" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.016") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

MAX SIZE 16' x 8'
DESIGN LOADS +29.5 PSF -29.5 PSF
TEST LOADS +45.3 PSF -45.3 PSF

Amarr

MODEL #1500 WeatherGuard

SBC-580-019-J

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Rumph Farms Lot 3

Building Permit #_____ Owner's Name Steve + Dena Cribbs

Well Depth_____ Ft. Casing Depth _____Ft Water Level _____Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model 520-100 HP 1

System Pressure (PSI) _____On 40 Off 60 Average Pressure 50

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: (Bladder)/Galvanized Make Challenger
Model PC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

1/15/07
Date

** LAMAR BOOZER **		PROJECT:	CUSTOM
900 EAST PUTNAM STREET		CLIENT:	NORTON (CRIBS
LAKE CITY, FL 32055		DATE:	1 12 07
RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS		DESIGNER:	LAMAR BOOZER

CLIENT INFORMATION:

NAME: NORTON (CRIBS
ADDRESS:
CITY, STATE: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	85	2,773	0	2,964	2,964
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,219	4,389	0	2,400	2,400
11-C DOOR METAL POLYSTYRENE CORE	40	846	0	462	462
16-G CEILING R-30 INSULATION	1,740	2,627	0	2,627	2,627
22-A SLAB ON GRADE NO EDGE INSUL	111	4,046	0	0	0
SUBTOTALS FOR STRUCTURE:	3,195	14,681	0	8,453	8,453
PEOPLE	10	0	0	3,000	3,000
APPLIANCES	0	0	800	1,500	2,300
DUCTWORK	0	734	0	1,841	1,841
INFILTRATION W.CFM: 0.0 S.CFM: 235.9	0	0	7,859	5,449	13,308
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				20,243	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		15,415	8,659	20,243	28,902

SUPPLY CFM AT 20 DEG DT:	920	CFM PER SQUARE FOOT:	0.520
SQUARE FT. OF ROOM AREA:	1,740	SQUARE FOOT PER TON:	734.482

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 15.415 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.409 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8540
TYPE: PVC Casement Window

Title of Test	Results
AAMA/WDMA Rating	C-R40 (36 x 72)
Uniform Load Deflection Test Pressure	±40.0 psf
Air Infiltration	0.08 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Forced Entry Resistance	Pass Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48974.01
Report Date: 02/06/04
Expiration Date: 02/06/08

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3835
fax: 651.636.3843
www.architest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48974.01
Test Dates:	01/13/04
Thru:	02/06/04
Report Date:	02/12/04
Expiration Date:	02/06/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8540 Casement Window. The sample tested successfully met the performance requirements for a C-R40 36 x 72 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8540

Type: PVC Casement Window

Overall Size: 3' 0" wide by 6' 0" high

Sash Size: 2' 10-1/4" wide by 5' 10-1/4" high

Finish: All PVC was white.

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two double-strength annealed sheets and a desiccant-filled metal spacer system. The glass was set from the exterior against a bed of silicone with PVC stops used on the exterior.

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3635
fax: 651.636.3843
www.architest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.460" high pile with center fin	1 Row	Perimeter of sash exterior
Foam-filled vinyl bulb gasket	1 Row	Perimeter of sash interior
1/4" EPDM rubber bulb	1 Row	Perimeter of frame

Frame Construction: Frame corners were miter-cut and welded.

Sash Construction: Sash corners were miter-cut and welded.

Hardware:

Dual arm roto-operator	1	Sill
4-point lock with keepers on the sash	1	Locking jamb
Casement hinges	2	Top and bottom corner of sash on hinge side
Metal snubbers	2	24" from top and bottom on hinge side

Installation: The unit was installed into a grade 2 SPF 2" by 8" wood test buck and secured with 1-5/8" screws through the nail fin spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results:

The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-01 (See Note #1)		
	@ 1.57 psf (25 mph)	0.08 cfm/ft^2	0.3 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	0.13 cfm/ft^2	--

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA101/I.S.2-97 for air infiltration.

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.5.6.1	Vertical Deflection Test @ 45lbs	0.09"	0.71"
2.2.5.6.2	Hardware Load Test @ 5lbs/ft²	No damage	No damage
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97 Type B Grade 10 Lock Manipulation Test Tests B1 through B3 Lock Manipulation Test	 No entry No entry No entry	 No entry No entry No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the top rail) (Loads were held for 60 seconds) @ 40.0 psf (positive) @ 40.0 psf (negative)	 0.10" 0.30"	 (See Note #3) (See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the top rail) (Loads were held for 10 seconds) @ 60.0 psf (positive) @ 60.0 psf (negative)	 0.01" 0.01"	 0.136" max. 0.136" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

PLS/jb
02-48974.01

DOCUMENT CONTROL ADDENDUM 02-48974.00

Current Issue Date: 02/12/04

Report No. 02-48974.01

Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 testing on a Jordan 8540 Casement
Issue Date: 02/12/04
Comments: Reports and drawings forwarded to ALI for AAMA certification.

Project Information for:	L218265		
Builder:	NORTON HOME	Date:	12/12/2006
Lot:	N/A	Start Number:	**1200**
Subdivision:	19498 S. US HWY 441	SEI Ref:	L218265
County or City:	ALACHUA COUNTY		
Truss Page Count:	11		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORTON, JAMES H. RB 0031780
Address: 3367 S US HWY 441, SUITE 101
LAKE CITY, FL 32025 Designer: **124**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

DEC 12 2006

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	T01	1212061200	12/12/2006				
2	T01G	1212061201	12/12/2006				
3	T02	1212061202	12/12/2006				
4	T02G	1212061203	12/12/2006				
5	T03	1212061204	12/12/2006				
6	T03G	1212061205	12/12/2006				
7	T04	1212061206	12/12/2006				
8	T05	1212061207	12/12/2006				
9	T05G	1212061208	12/12/2006				
10	T06	1212061209	12/12/2006				
11	T06G	1212061210	12/12/2006				

Dwg.#1212061199

Licensee Details

Licensee Information

Name:	**NORTON, JAMES H** (Primary Name) **NORTON HOME IMPROVEMENT COMPANY INC** (DBA Name)
Main Address:	**3367 S US HWY 441, SUITE 101** **LAKE CITY Florida 32025**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**RT 28 BOX 388A** **LAKE CITY FL 32025**
County:	**COLUMBIA**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB0031780**
Status:	**Current,Active**
Licensure Date:	**02/16/1978**
Expires:	**08/31/2007**

Special Qualifications	Qualification Effective
Bldg Code Core Course Credit	
Qualified Business License Required	**02/20/2004**

View Related License Information

View License Complaint

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061200

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T01	COMMON	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 21 09:18:50 2006 Page 1

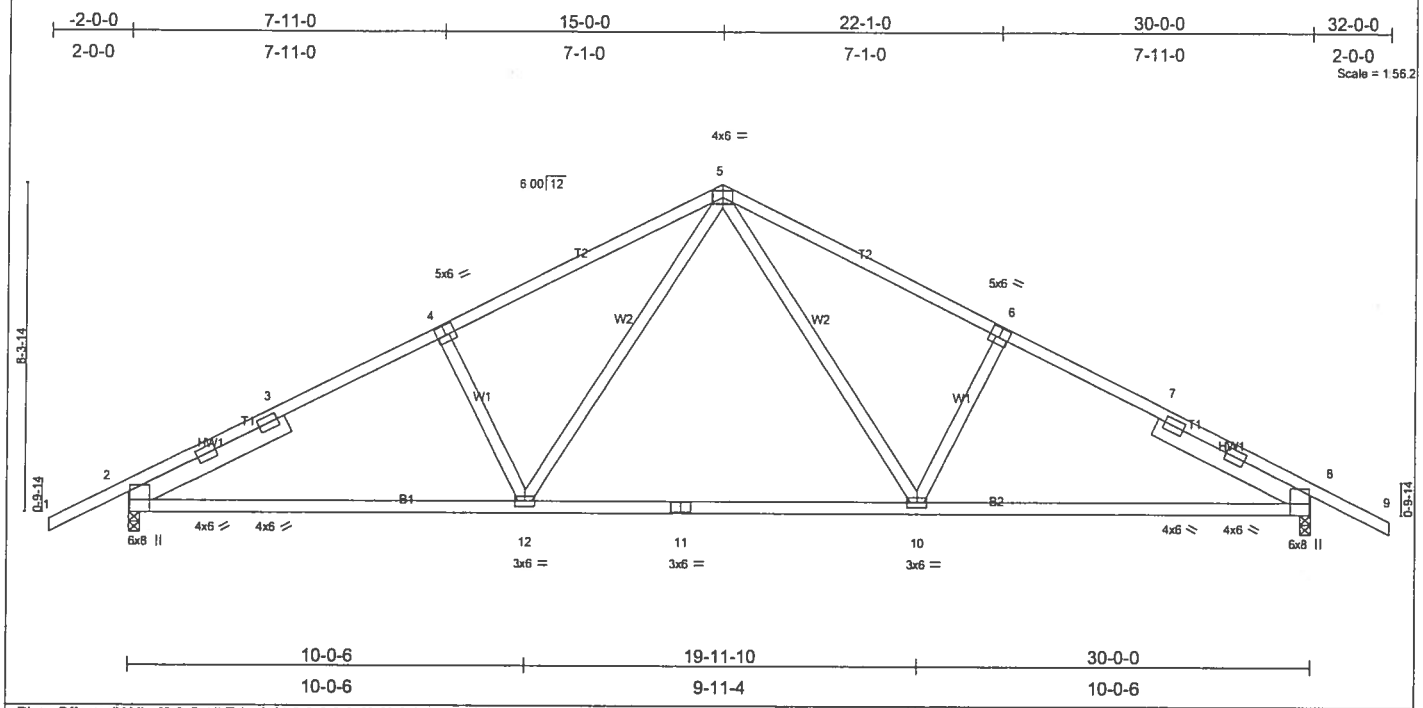


Plate Offsets (X,Y): [2:0-5-15,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.20	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.34	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 4-5-5, Right 2 X 6 SYP No.1D 4-5-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-8-5 oc bracing.

REACTIONS (lb/size) 2=1368/0-3-8, 8=1368/0-3-8
Max Horz 2=-131(load case 6)
Max Uplift2=-523(load case 5), 8=-523(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1967/606, 3-4=-1802/625, 4-5=-1788/658, 5-6=-1788/658, 6-7=-1802/625, 7-8=-1966/606, 8-9=0/26
BOT CHORD 2-12=-520/1651, 11-12=-250/1183, 10-11=-250/1183, 8-10=-392/1651
WEBS 4-12=-322/309, 5-12=-261/696, 5-10=-261/696, 6-10=-322/310

JOINT STRESS INDEX
2 = 0.72, 2 = 0.37, 2 = 0.37, 3 = 0.00, 4 = 0.64, 5 = 0.72, 6 = 0.64, 7 = 0.00, 8 = 0.72, 8 = 0.37, 8 = 0.37, 10 = 0.54, 11 = 0.63 and 12 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 523 lb uplift at joint 2 and 523 lb uplift at joint 8.

LOAD CASE(S) Standard

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061201

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T01G	GABLE	1	1	Job Reference (optional)

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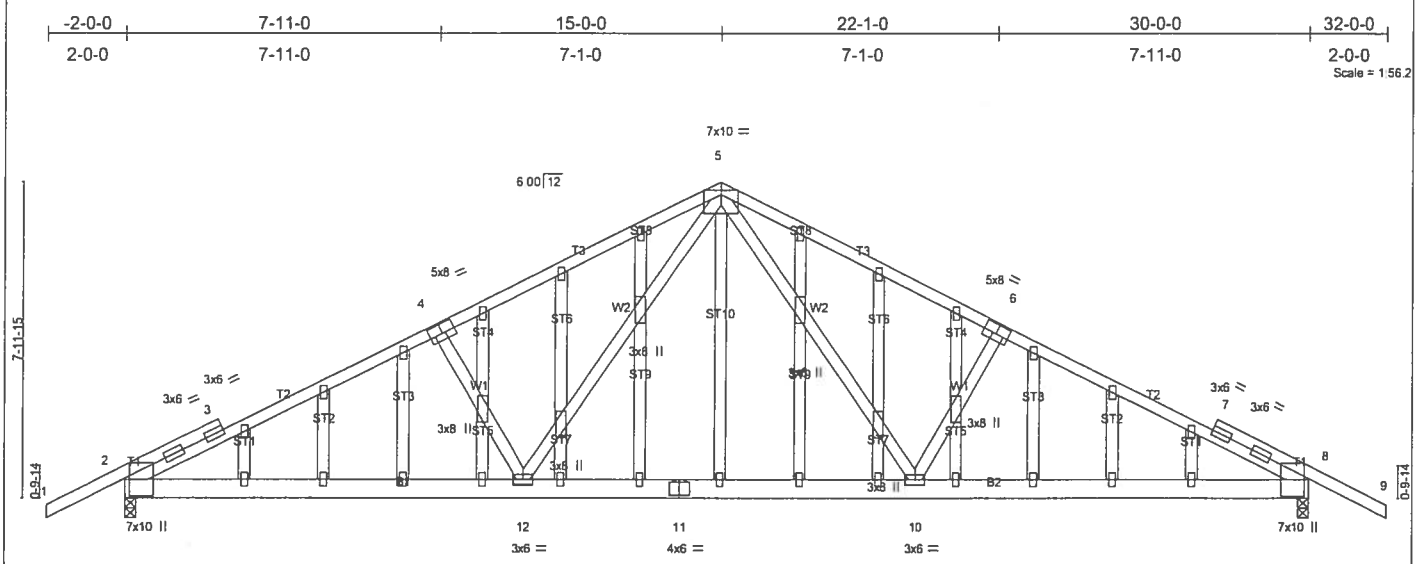

Plate Offsets (X,Y): [2:0-5-0,0-1-8], [4:0-4-0,0-3-0], [6:0-4-0,0-3-0], [8:0-5-0,0-1-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.99	Vert(LL)	-0.20	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.32	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.65	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-9-10 oc bracing.

REACTIONS (lb/size) 2=2388/0-3-8, 8=2388/0-3-8
Max Horz 2=-129(load case 6)
Max Uplift2=-910(load case 5), 8=-910(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/63, 2-3=-3746/1247, 3-4=-3517/1215, 4-5=-3317/1184, 5-6=-3317/1184, 6-7=-3517/1216, 7-8=-3746/1247, 8-9=-8/63
BOT CHORD 2-12=-1076/3146, 11-12=-614/2152, 10-11=-614/2152, 8-10=-947/3146
WEBS 4-12=-756/467, 5-12=-447/1219, 5-10=-447/1219, 6-10=-756/467

JOINT STRESS INDEX
2 = 0.95, 3 = 0.00, 3 = 0.53, 3 = 0.67, 4 = 0.99, 5 = 0.76, 6 = 0.99, 7 = 0.00, 7 = 0.67, 7 = 0.53, 8 = 0.95, 10 = 0.92, 11 = 0.60, 12 = 0.92, 13 = 0.34, 14 = 0.64, 15 = 0.34, 16 = 0.34, 17 = 0.64, 18 = 0.34, 19 = 0.34, 20 = 0.84, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.64, 32 = 0.34, 33 = 0.34, 34 = 0.64, 35 = 0.34, 36 = 0.34, 37 = 0.84, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34 and 43 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 910 lb uplift at joint 2 and 910 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061202

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T02	COMMON	8	1	Job Reference (optional)

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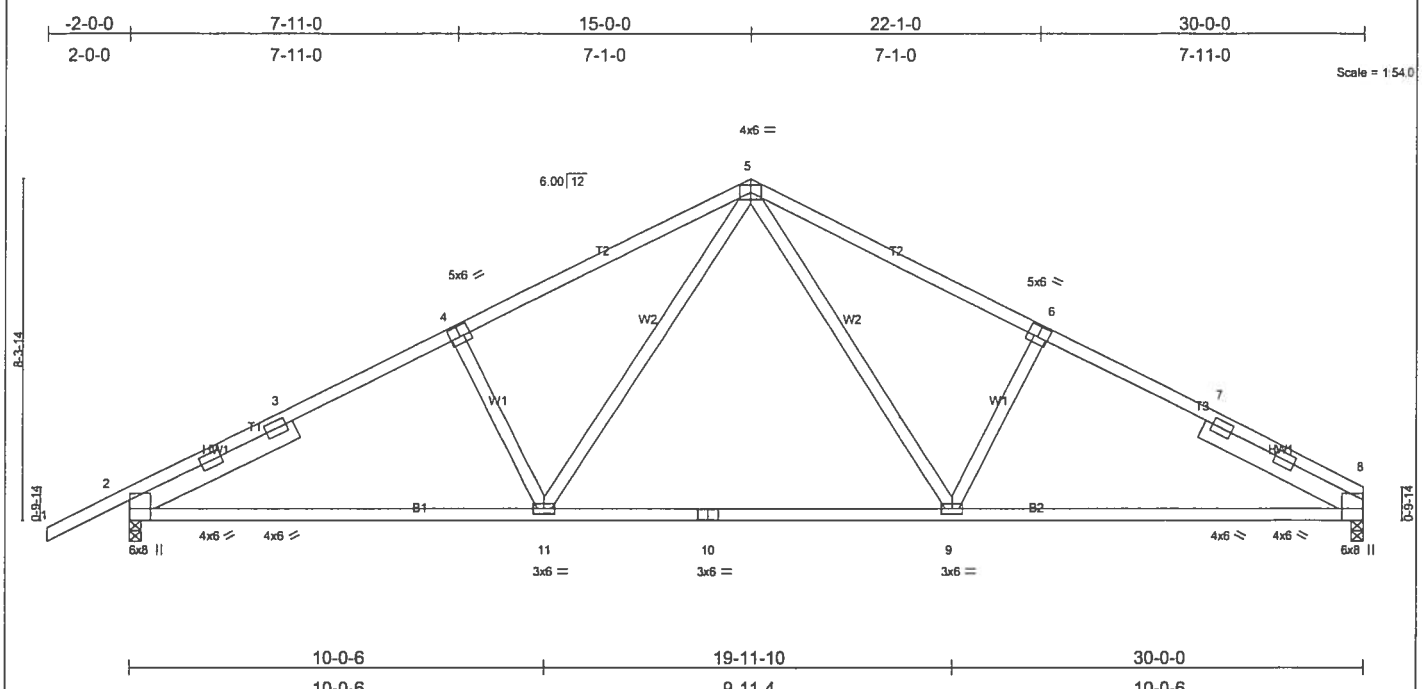

10-0-6	19-11-10	30-0-0
10-0-6	9-11-4	10-0-6

Plate Offsets (X,Y): [2:0-5-15,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.20	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.33	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 4-5-5, Right 2 X 6 SYP No.1D 4-5-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-0 oc bracing.

REACTIONS (lb/size) 2=1372/0-3-8, 8=1256/0-3-8
Max Horz 2=144(load case 5)
Max Uplift2=-524(load case 5), 8=-407(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1973/609, 3-4=-1809/627, 4-5=-1795/660, 5-6=-1806/678, 6-7=-1819/645, 7-8=-1983/624
BOT CHORD 2-11=-534/1657, 10-11=-264/1189, 9-10=-264/1189, 8-9=-443/1672
WEBS 4-11=-321/309, 5-11=-261/696, 5-9=-280/713, 6-9=-335/324

JOINT STRESS INDEX
2 = 0.72, 2 = 0.37, 2 = 0.37, 3 = 0.00, 4 = 0.69, 5 = 0.72, 6 = 0.69, 7 = 0.00, 8 = 0.72, 8 = 0.37, 8 = 0.37, 9 = 0.55, 10 = 0.63 and 11 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 524 lb uplift at joint 2 and 407 lb uplift at joint 8.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061203

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T02G	GABLE	1	1	Job Reference (optional)

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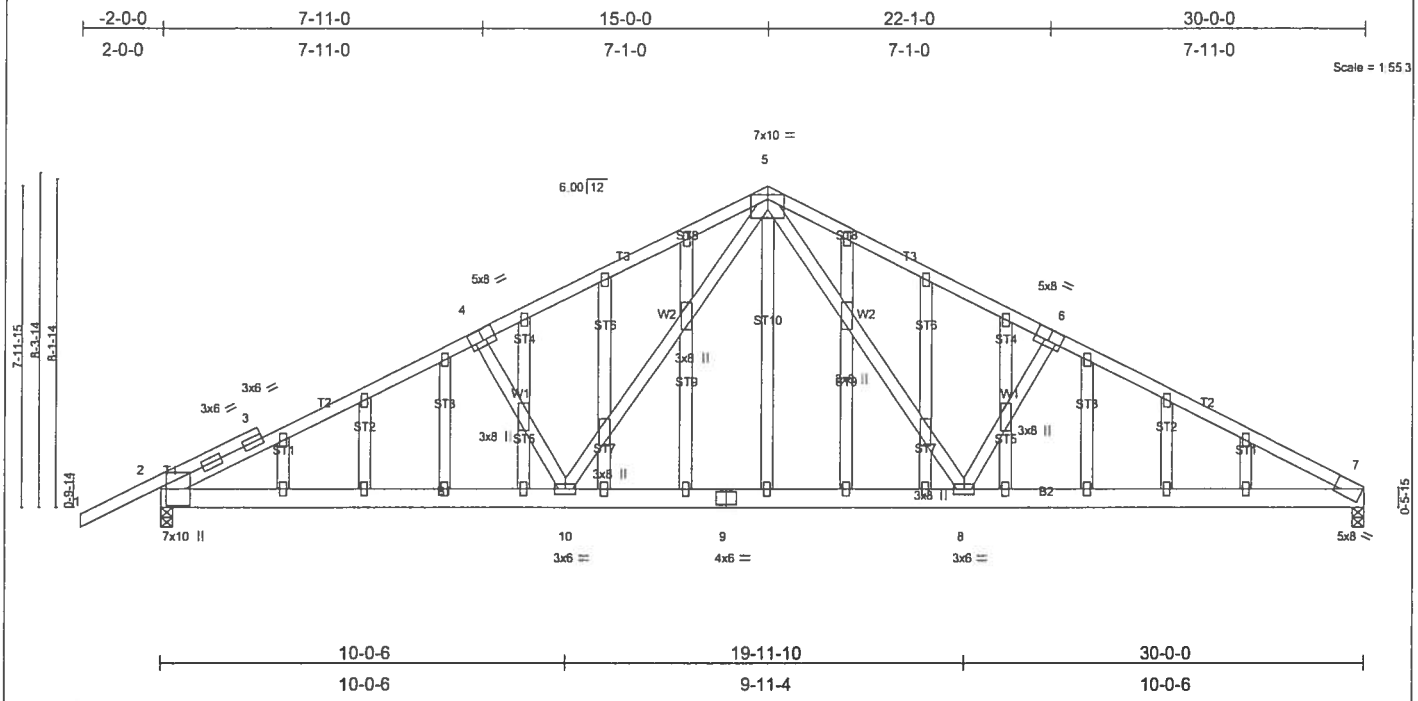

Plate Offsets (X,Y): [2:0-5-0,0-1-8], [4:0-4-0,0-3-4], [6:0-4-0,0-3-4], [7:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.23	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.37	7-8	>979	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.68	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 243 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D, T2 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-9-4 oc bracing.

REACTIONS (lb/size) 2=2387/0-3-8, 7=2144/0-3-8
Max Horz 2=143(load case 5)
Max Uplift2=-910(load case 5), 7=-743(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/63, 2-3=-3742/1247, 3-4=-3515/1216, 4-5=-3313/1184, 5-6=-3335/1207, 6-7=-3752/1263
BOT CHORD 2-10=-1091/3144, 9-10=-628/2150, 8-9=-628/2150, 7-8=-1006/3153
WEBS 4-10=-762/470, 5-10=-447/1215, 5-8=-472/1246, 6-8=-748/477

JOINT STRESS INDEX
2 = 0.95, 3 = 0.00, 3 = 0.55, 3 = 0.66, 4 = 0.87, 5 = 0.73, 6 = 0.95, 7 = 0.65, 8 = 0.94, 9 = 0.59, 10 = 0.91, 11 = 0.34, 12 = 0.64, 13 = 0.34, 14 = 0.34, 15 = 0.64, 16 = 0.34, 17 = 0.34, 18 = 0.84, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.64, 30 = 0.34, 31 = 0.34, 32 = 0.64, 33 = 0.34, 34 = 0.34, 35 = 0.84, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34 and 41 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 910 lb uplift at joint 2 and 743 lb uplift at joint 7.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-7=-114(F=-60), 2-7=-30

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061204

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T03	SPECIAL	10	1	Job Reference (optional)

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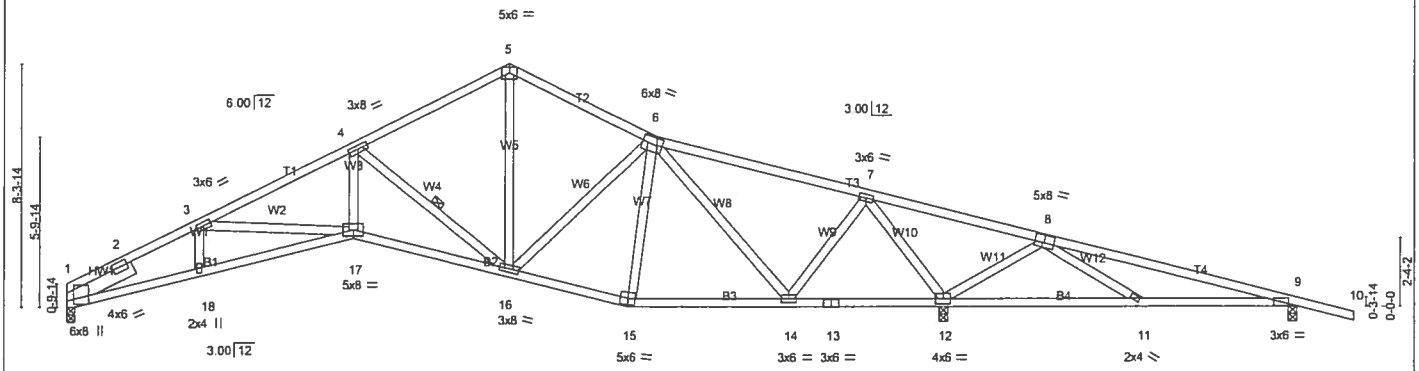
4-5-9	9-8-0	15-0-0	20-0-0	27-2-0	33-3-5	42-0-0	44-0-0
4-5-9	5-2-7	5-4-0	5-0-0	7-2-0	6-1-4	8-8-11	2-0-0

Scale = 1:75.9



4-5-9	9-8-0	15-0-0	19-0-8	24-6-6	29-10-4	36-5-0	42-0-0
4-5-9	5-2-7	5-4-0	4-0-8	5-5-14	5-3-14	6-6-12	5-7-0

Plate Offsets (X,Y): [1:0-1-6,Edge], [8:0-4-0,0-3-0], [9:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.23	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.37	17-18	>964	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(TL)	0.19	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 225 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-6-6

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-16

REACTIONS (lb/size) 1=1089/0-3-8, 12=2308/0-3-8, 9=228/0-3-8
Max Horz 1=-141(load case 6)
Max Uplift1=-345(load case 5), 12=-931(load case 6), 9=-359(load case 4)
Max Grav 1=1089(load case 1), 12=2308(load case 1), 9=305(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2713/865, 2-3=-2648/876, 3-4=-2565/787, 4-5=-1197/410, 5-6=-1194/429, 6-7=-385/168, 7-8=-545/1628, 8-9=-28/615, 9-10=0/25
BOT CHORD 1-18=-792/2320, 17-18=-798/2344, 16-17=-640/2360, 15-16=-132/968, 14-15=-135/941, 13-14=-317/291, 12-13=-317/291, 11-12=-908/302, 9-11=-538/66
WEBS 3-18=0/93, 3-17=-63/153, 4-17=-329/1308, 4-16=-1626/607, 5-16=-215/755, 6-16=-38/213, 6-15=-94/57, 6-14=-967/359, 7-14=-280/1107, 7-12=-2113/722, 8-12=-758/536, 8-11=-440/442

JOINT STRESS INDEX
1 = 0.91, 1 = 0.99, 2 = 0.00, 3 = 0.41, 4 = 0.77, 5 = 0.26, 6 = 0.70, 7 = 0.88, 8 = 0.80, 9 = 0.74, 11 = 0.34, 12 = 0.51, 13 = 0.15, 14 = 0.80, 15 = 0.38, 16 = 0.85, 17 = 0.77 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 1, 931 lb uplift at joint 12 and 359 lb uplift at joint 9.

LOAD CASE(S) Standard

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061205

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T03G	GABLE	1	1	Job Reference (optional)

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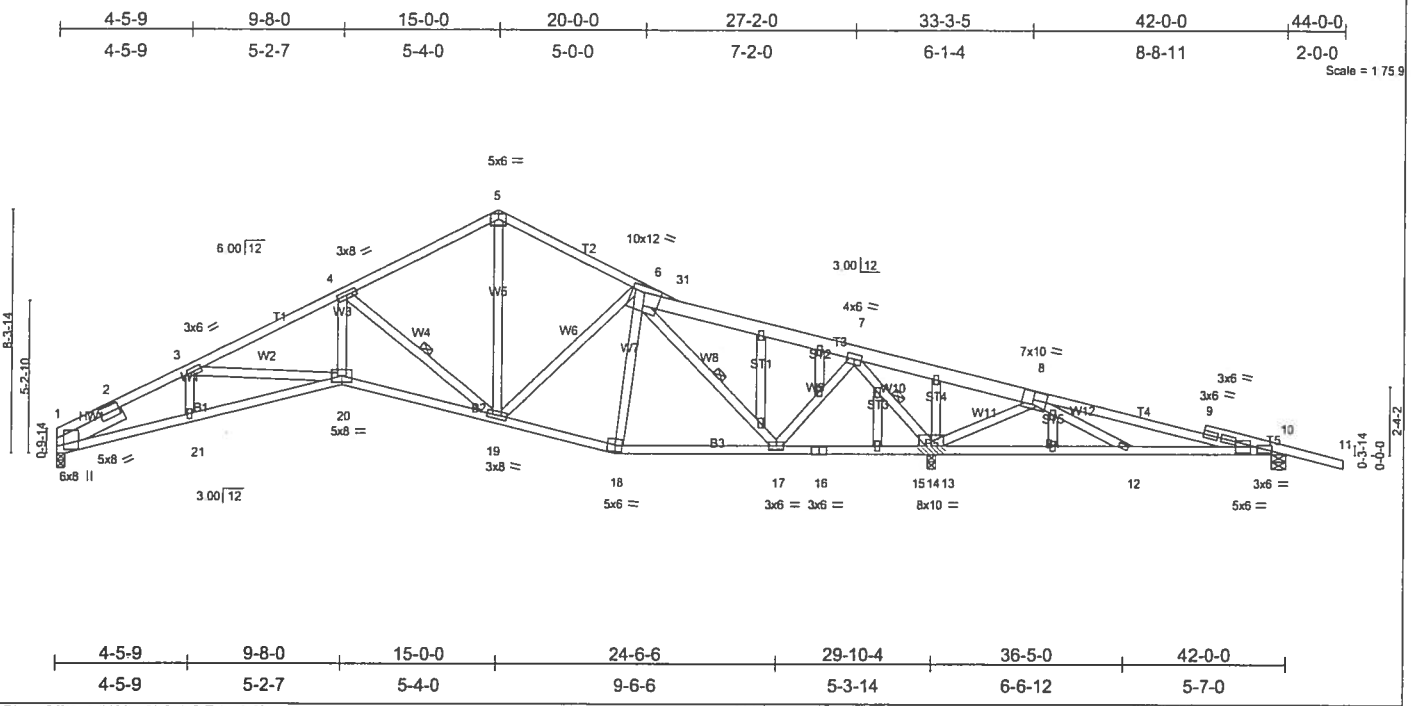

Plate Offsets (X,Y): [1:0-1-6,Edge], [6:0-3-12,0-5-0], [8:0-4-8,0-3-4], [9:0-2-9,0-1-8], [10:1-2-12,0-0-12], [10:0-5-12,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.24	20-21	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.38	20-21	>929	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.20	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 256 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T3 2 X 6 SYP No.1D, T4 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-6-6

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-3 oc bracing.
WEBS 1 Row at midpt 4-19, 6-17, 7-14

REACTIONS (lb/size) 1=1111/0-3-8, 14=3409/0-4-0 (0-3-8 + bearing block), 10=542/0-6-0
Max Horz 1=-141(load case 6)
Max Uplift1=-358(load case 5), 14=-1701(load case 6), 10=-552(load case 4)
Max Grav 1=1111(load case 1), 14=3409(load case 1), 10=606(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2773/899, 2-3=-2709/911, 3-4=-2641/830, 4-5=-1250/440, 5-31=-1268/462, 6-31=-1296/450, 6-7=-414/97, 7-8=-1332/2765, 8-9=-211/972, 9-10=-174/831, 10-11=-17/53
BOT CHORD 1-21=-822/2373, 20-21=-828/2397, 19-20=-679/2430, 18-19=-187/1072, 17-18=-199/1060, 16-17=-558/498, 15-16=-558/498, 14-15=-558/498, 13-14=-1315/524, 12-13=-1315/524, 10-12=-840/212
WEBS 3-21=0/91, 3-20=-34/143, 4-20=-345/1337, 4-19=-1650/621, 5-19=-250/830, 6-19=-97/159, 6-18=-122/72, 6-17=-1207/543, 7-17=-474/1394, 7-14=-3274/1492, 8-14=-1474/1045, 8-12=-556/540

JOINT STRESS INDEX
1 = 0.93, 1 = 0.76, 2 = 0.00, 3 = 0.41, 4 = 0.79, 5 = 0.29, 6 = 0.25, 7 = 0.93, 8 = 0.83, 9 = 0.00, 9 = 0.76, 9 = 0.76, 10 = 0.84, 10 = 0.93, 12 = 0.34, 13 = 0.00, 13 = 0.00, 14 = 0.24, 14 = 0.00, 15 = 0.00, 15 = 0.00, 16 = 0.18, 17 = 0.94, 18 = 0.40, 19 = 0.86, 20 = 0.79, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34 and 30 = 0.34

NOTES
1) 2 X 4 SYP No.2 bearing block 12" long at jt. 14 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 358 lb uplift at joint 1, 1701 lb uplift at joint 14 and 552 lb uplift at joint 10.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-31=-54, 6-31=-114(F=-60), 6-11=-114(F=-60), 1-20=-30, 18-20=-30, 10-18=-30

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061206

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T04	SPECIAL	2	1	Job Reference (optional)

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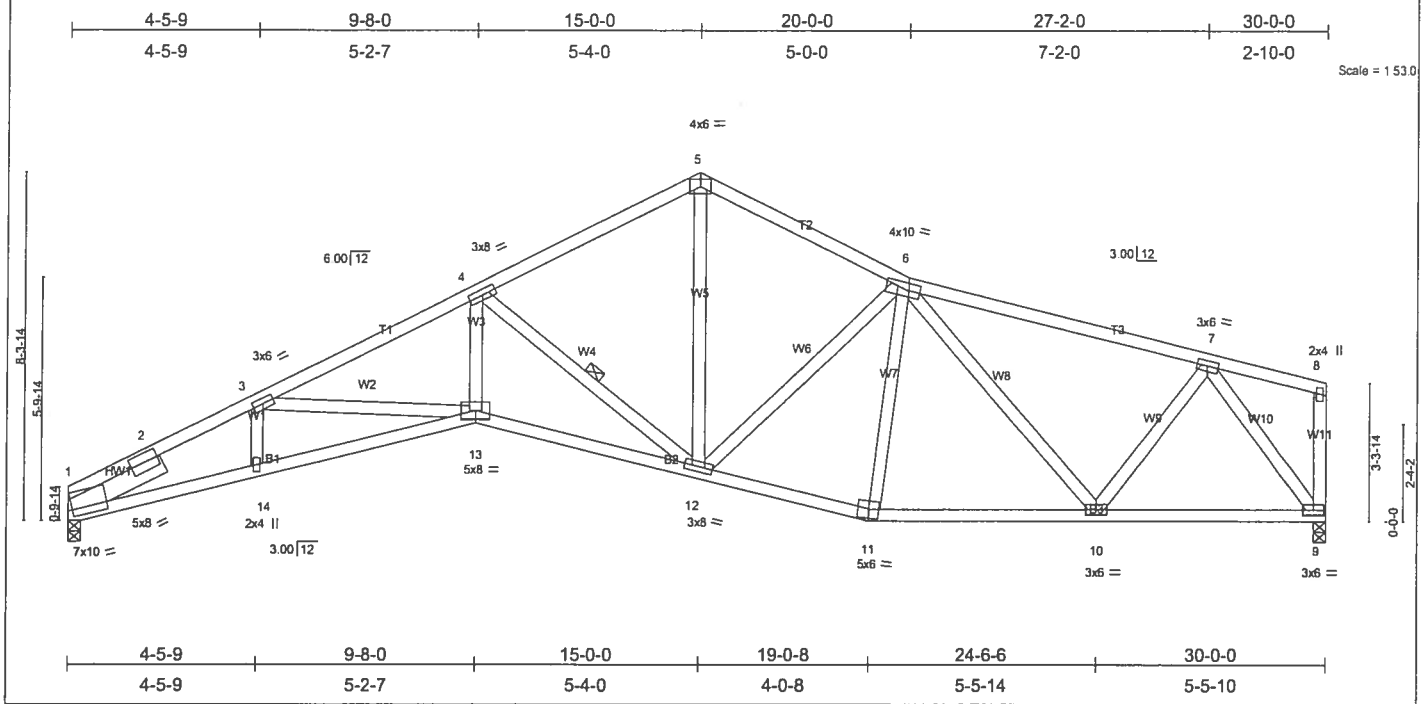

4-5-9	9-8-0	15-0-0	19-0-8	24-6-6	30-0-0
4-5-9	5-2-7	5-4-0	4-0-8	5-5-14	5-5-10

Plate Offsets (X,Y): [1:0-1-5,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.28	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.44	13-14	>806	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.25	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 179 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-6-6

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-12

REACTIONS (lb/size) 1=1251/0-3-8, 9=1251/0-3-8
Max Horz 1=213(load case 5)
Max Uplift1=-400(load case 5), 9=-412(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3169/1095, 2-3=-3104/1105, 3-4=-3141/1079, 4-5=-1583/559, 5-6=-1580/578, 6-7=-1316/454, 7-8=-23/15, 8-9=-13/0
BOT CHORD 1-14=-1105/2718, 13-14=-1113/2743, 12-13=-1024/2894, 11-12=-451/1545, 10-11=-459/1524, 9-10=-303/858
WEBS 3-14=0/81, 3-13=-7/197, 4-13=-494/1525, 4-12=-1848/773, 5-12=-347/1099, 6-12=-271/177, 6-11=-239/139, 6-10=-462/185, 7-10=-131/648, 7-9=-1436/529

JOINT STRESS INDEX
1 = 0.65, 1 = 0.86, 2 = 0.00, 3 = 0.41, 4 = 0.90, 5 = 0.53, 6 = 0.86, 7 = 0.52, 8 = 0.34, 9 = 0.53, 10 = 0.47, 11 = 0.48, 12 = 0.96, 13 = 0.91 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 1 and 412 lb uplift at joint 9.

LOAD CASE(S) Standard

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061207

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T05	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 21 09:18:59 2006 Page 1

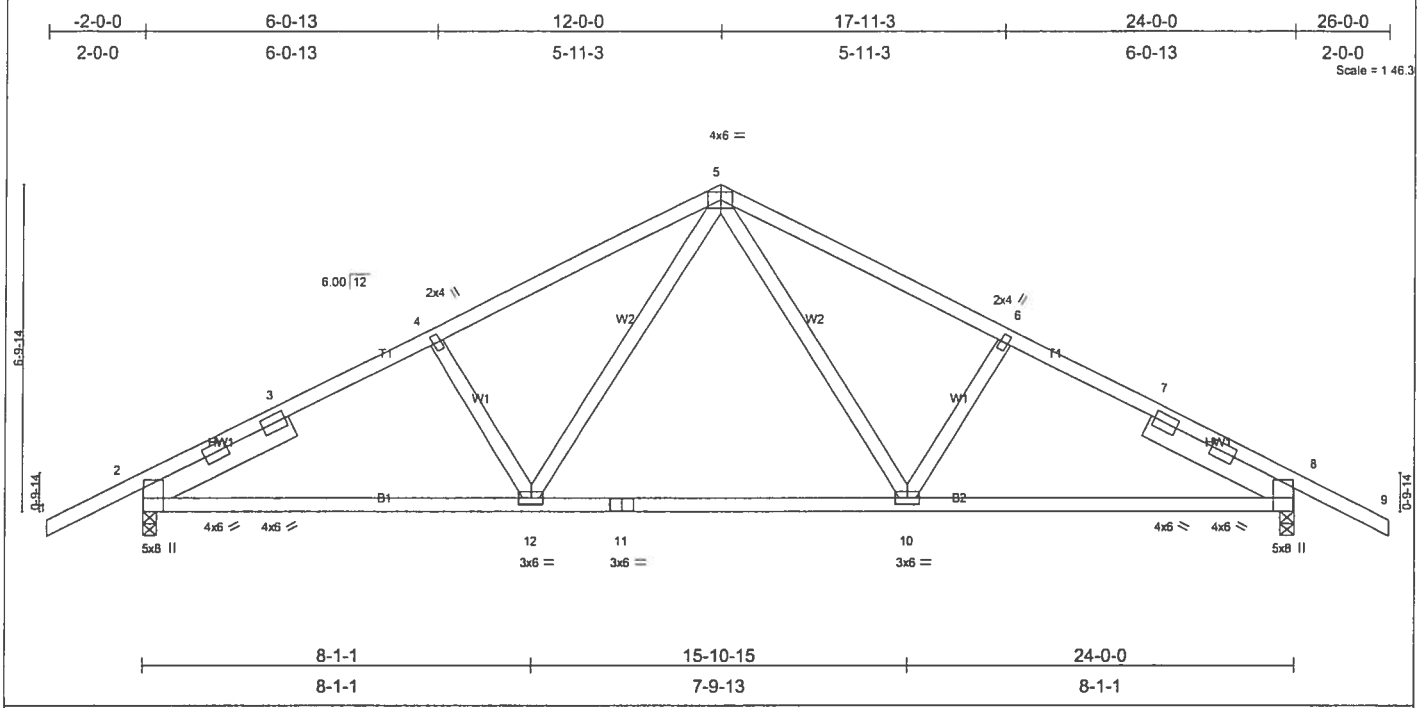

Plate Offsets (X,Y): [2:0-5-15,Edge], [8:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.20	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.18	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-5-7, Right 2 X 6 SYP No.1D 3-5-7

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-14 oc bracing.

REACTIONS (lb/size) 2=1116/0-3-8, 8=1116/0-3-8
Max Horz 2=-110(load case 6)
Max Uplift2=-743(load case 5), 8=-743(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1533/1233, 3-4=-1460/1259, 4-5=-1372/1272, 5-6=-1372/1272, 6-7=-1460/1259, 7-8=-1533/1233, 8-9=0/26
BOT CHORD 2-12=-978/1271, 11-12=-634/931, 10-11=-634/931, 8-10=-978/1271
WEBS 4-12=-229/217, 5-12=-544/503, 5-10=-544/503, 6-10=-229/217

JOINT STRESS INDEX
2 = 0.71, 2 = 0.29, 2 = 0.29, 3 = 0.00, 4 = 0.34, 5 = 0.64, 6 = 0.34, 7 = 0.00, 8 = 0.71, 8 = 0.29, 8 = 0.29, 10 = 0.44, 11 = 0.35 and 12 = 0.44

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 743 lb uplift at joint 2 and 743 lb uplift at joint 8.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061208

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T05G	GABLE	1	1	Job Reference (optional)

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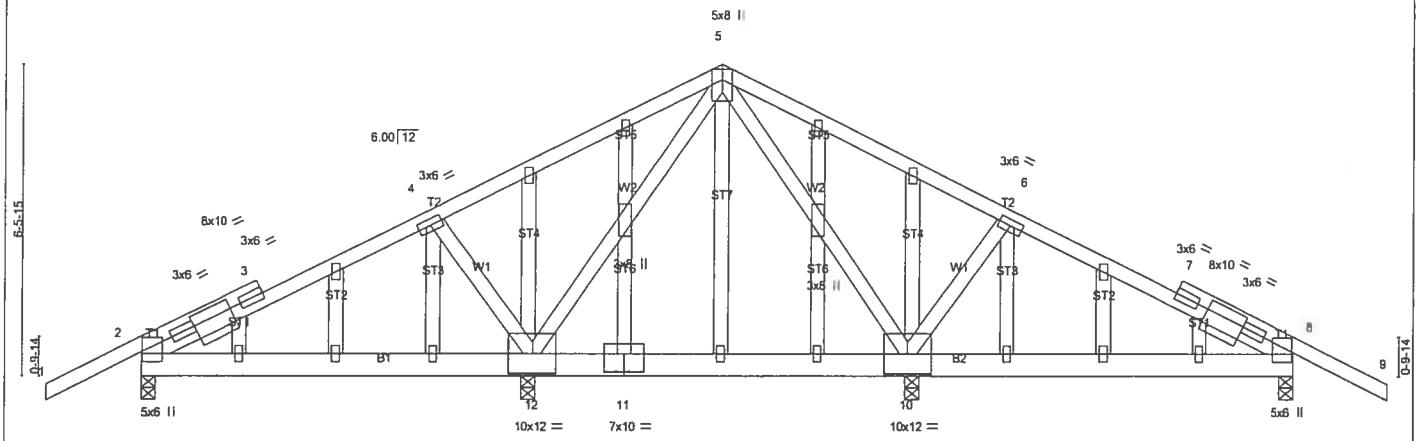
-2-0-0 | 6-0-13 | 12-0-0 | 17-11-3 | 24-0-0 | 26-0-0
2-0-0 | 6-0-13 | 5-11-3 | 5-11-3 | 6-0-13 | 2-0-0
Scale = 1:46.3



8-1-1 | 15-10-15 | 24-0-0
8-1-1 | 7-9-13 | 8-1-1

Plate Offsets (X,Y): [2:0-2-0,0-0-4], [3:0-2-4,0-1-8], [7:0-2-4,0-1-8], [8:0-2-0,0-1-12], [11:0-5-0,0-4-8], [21:1-2-5,0-2-8], [30:1-2-5,0-2-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.08	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.06	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=449/0-3-8, 8=449/0-3-8, 12=667/0-3-8, 10=667/0-3-8
Max Horz 2=-108(load case 6)
Max Uplift2=-331(load case 5), 8=-345(load case 6), 12=-343(load case 5), 10=-329(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-3=-271/122, 3-4=-199/135, 4-5=-81/134, 5-6=-81/120, 6-7=-199/129, 7-8=-271/116, 8-9=0/30
BOT CHORD 2-12=-79/178, 11-12=0/134, 10-11=0/134, 8-10=-22/178
WEBS 4-12=-319/283, 5-12=-187/66, 5-10=-187/64, 6-10=-319/282

JOINT STRESS INDEX
2 = 0.74, 3 = 0.00, 3 = 0.21, 3 = 0.21, 4 = 0.63, 5 = 0.83, 6 = 0.63, 7 = 0.00, 7 = 0.21, 7 = 0.21, 8 = 0.74, 10 = 0.56, 11 = 0.16, 12 = 0.56, 13 = 0.34, 14 = 0.64, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.51, 22 = 0.34, 23 = 0.34, 24 = 0.64, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34 and 30 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2, 345 lb uplift at joint 8, 343 lb uplift at joint 12 and 329 lb uplift at joint 10.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061209

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T06	COMMON	11	1	Job Reference (optional)

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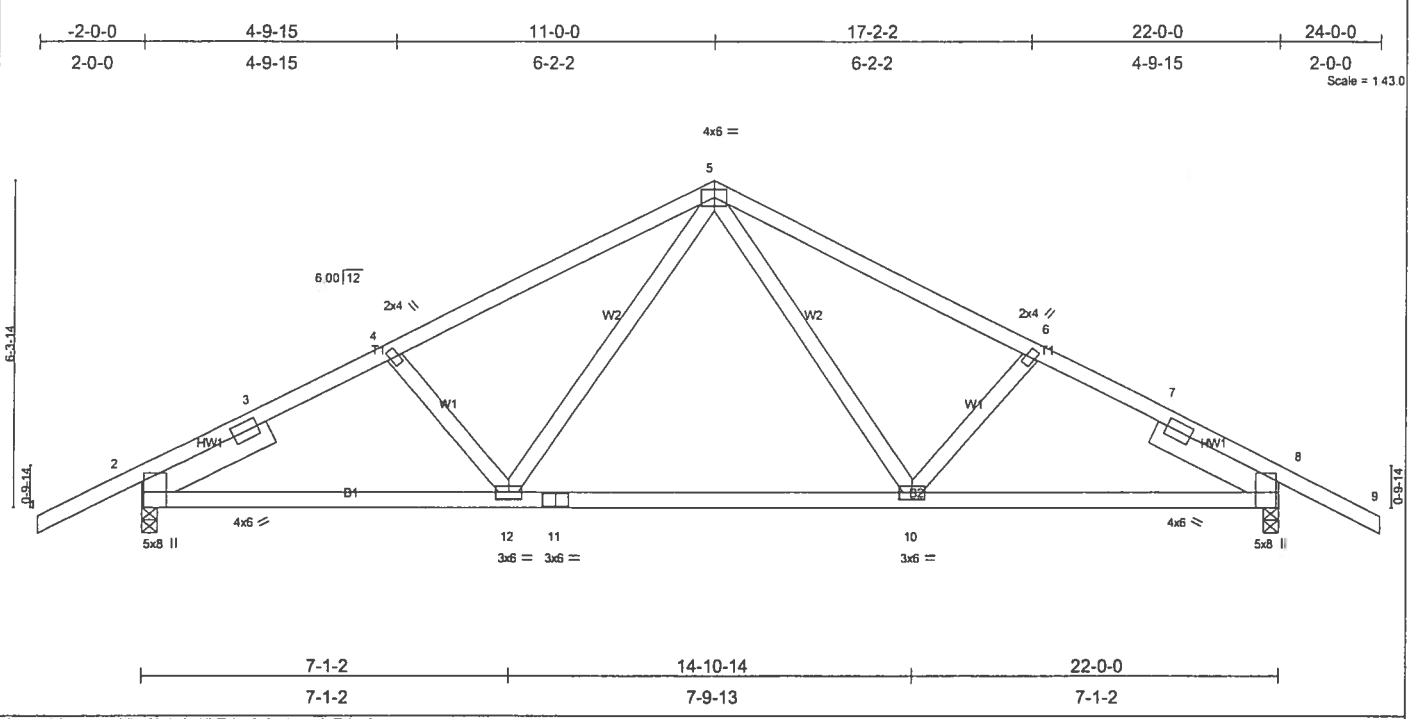

Plate Offsets (X,Y): [2:0-5-15,Edge], [8:0-5-15,Edge]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.31	10-12	>855	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.50	10-12	>528	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 120 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-8-9, Right 2 X 6 SYP No.1D 2-8-9

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-9-7 oc bracing.

REACTIONS (lb/size) 2=1227/0-3-8, 8=1227/0-3-8
Max Horz 2=103(load case 5)
Max Uplift2=-489(load case 5), 8=-489(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1812/589, 3-4=-1753/605, 4-5=-1660/589, 5-6=-1660/589, 6-7=-1753/606, 7-8=-1812/589, 8-9=0/26
BOT CHORD 2-12=-503/1500, 11-12=-276/1112, 10-11=-276/1112, 8-10=-401/1500
WEBS 4-12=-126/180, 5-12=-208/649, 5-10=-208/649, 6-10=-126/180

JOINT STRESS INDEX
2 = 0.64, 2 = 0.67, 3 = 0.00, 4 = 0.34, 5 = 0.69, 6 = 0.34, 7 = 0.00, 8 = 0.64, 8 = 0.67, 10 = 0.49, 11 = 0.46 and 12 = 0.49

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 489 lb uplift at joint 2 and 489 lb uplift at joint 8.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-9=-54, 2-12=-30, 10-12=-80(F=-50), 8-10=-30

DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1212061210

Job	Truss	Truss Type	Qty	Ply	NORTON HOME - CRIBBS RES.
L218265	T06G	GABLE	1	1	Job Reference (optional)

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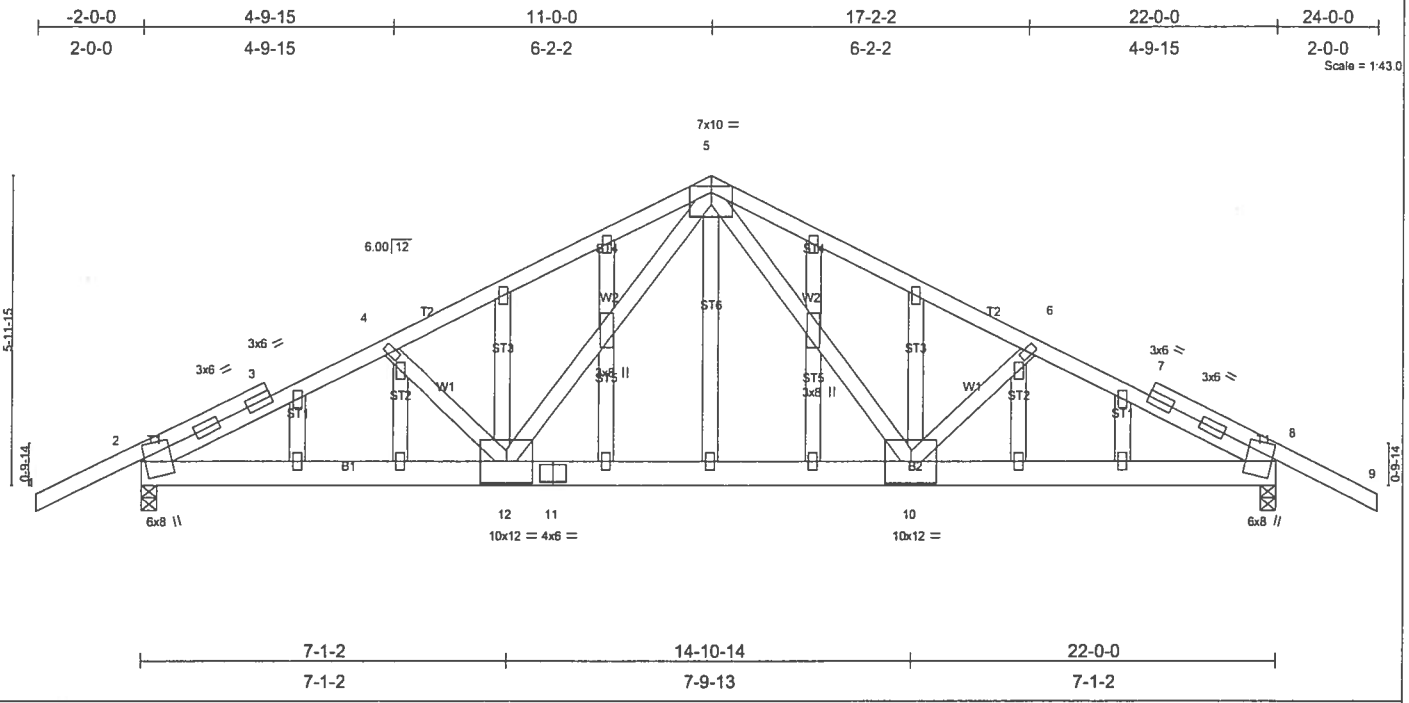


Plate Offsets (X,Y): [2:0-3-11,0-0-14], [8:0-2-11,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.11	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.18	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-13 oc bracing.

REACTIONS (lb/size) 2=1812/0-3-8, 8=1812/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-711(load case 5), 8=-711(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/63, 2-3=-2778/935, 3-4=-2555/908, 4-5=-2369/799, 5-6=-2369/799, 6-7=-2555/908, 7-8=-2778/935, 8-9=-8/63
BOT CHORD 2-12=-824/2362, 11-12=-446/1567, 10-11=-446/1567, 8-10=-723/2362
WEBS 4-12=-565/351, 5-12=-234/754, 5-10=-234/754, 6-10=-565/351

JOINT STRESS INDEX
2 = 0.87, 3 = 0.00, 3 = 0.29, 3 = 0.42, 4 = 0.34, 5 = 0.62, 6 = 0.34, 7 = 0.00, 7 = 0.42, 7 = 0.29, 8 = 0.87, 10 = 0.31, 11 = 0.39, 12 = 0.31, 13 = 0.34, 14 = 0.57, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.57, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34 and 29 = 0.34

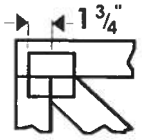
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 711 lb uplift at joint 2 and 711 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

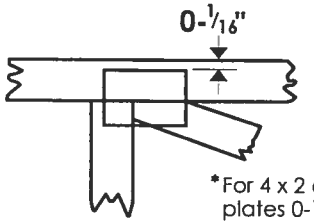
~~DECEMBER 12, 2006 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

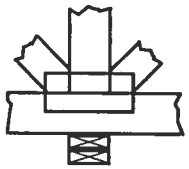
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

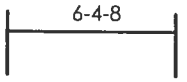


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

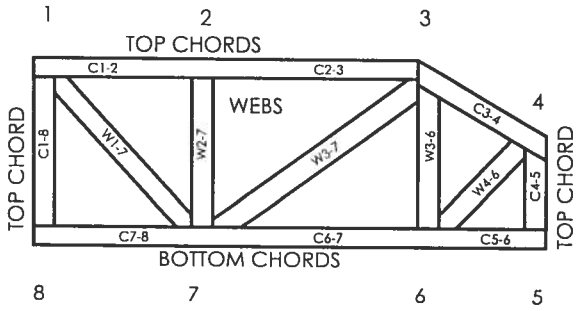
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 33-6S-17-09834-103 **Building permit No.** 000025494

Use Classification SFD/UTILITY **Fire:** 22.32

Permit Holder JAMES H. NORTON **Waste:** 67.00

Owner of Building STEVE & DENA CRIBBS **Total:** 89.32

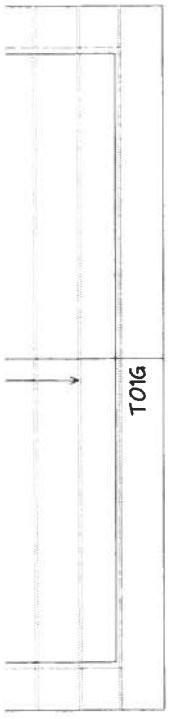
Location: 19498 S US HIGHWAY 441(RUMPH FARMS, LOT 3)

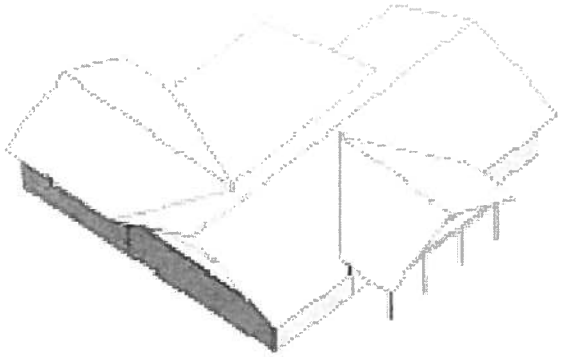
Date: 06/26/2007

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

6/12 PITCH
2'0" O/H





BEARING HEIGHT SCHEDULE

8'-1 1/8"

9'-1 1/8"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU

Expected Delivery Date _____

Approved By _____ Date _____

Builders FirstSource

Bunnell
PHONE 904-437-3349 FAX 904-437-3994

Jacksonville
PHONE 904-772-6100 FAX 904-772-1973

Lake City
PHONE 904-755-6894 FAX 904-755-7973

Sanford
PHONE 407-322-0059 FAX 407-322-5553

BUILDER: NORTON HOME		
LEGAL ADDRESS: CRIBBS RES		
MODEL: CUSTOM		REVISION: SCALE: NTS
DATE: 11-21-06	DESIGNER: K.L.H	JOB #: L218265