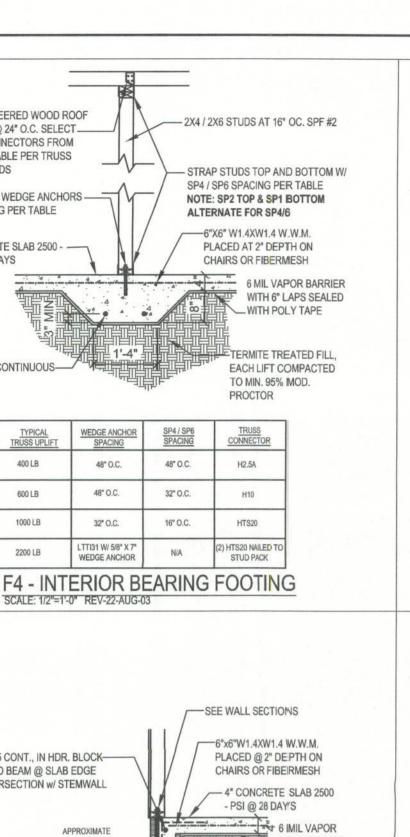


THREADED ROD WITH %" COUPLE! THRU COLUMN & HEADER WITH 3"

WASHER & NUT TOP

W12 - PORCH HEADER ANCHORS

2300 LB



PRE-ENGINEERED WOOD ROOF

TRUSSES @ 24" O.C. SELECT ____

1/2" X 6" WEDGE ANCHORS ---

TRUSS CONNECTORS FROM

ANCHOR TABLE PER TRUSS

SPACING PER TABLE

4" CONCRETE SLAB 2500 - -

(2) #5 CONTINUOUS-

600 LB

1000 LB

2200 LB

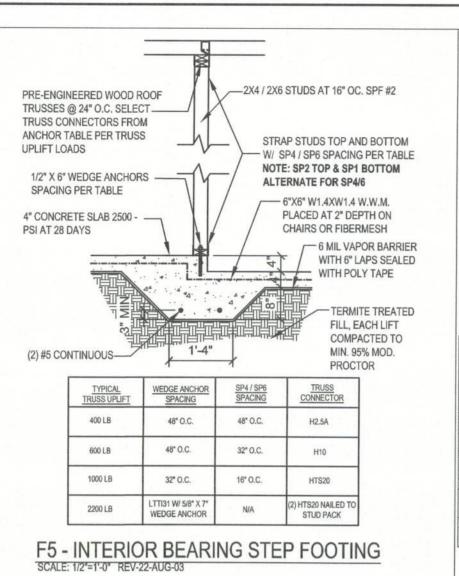
48" O.C.

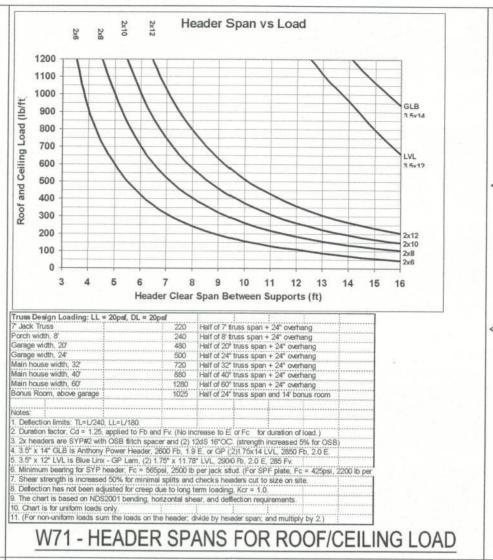
48" O.C.

32" O.C.

UPLIFT LOADS

PSI AT 28 DAYS





- 6"x6"W1.4XW1.4 W.W.M. PLACED @ 2"

-4" CONCRETE SLAB 2500 - PSI

HOUSE SLAB

- 6 MIL VAPOR BARRIER W/ 6" LAPS SEALED

DEPTH ON CHAIRS OR FIBERMESH

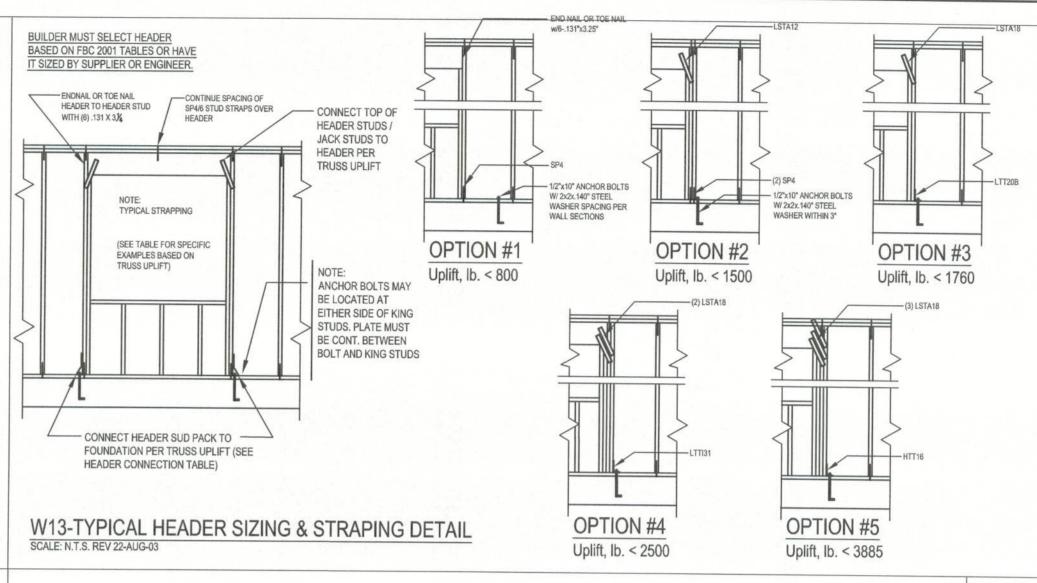
€ @ 28 DAYS

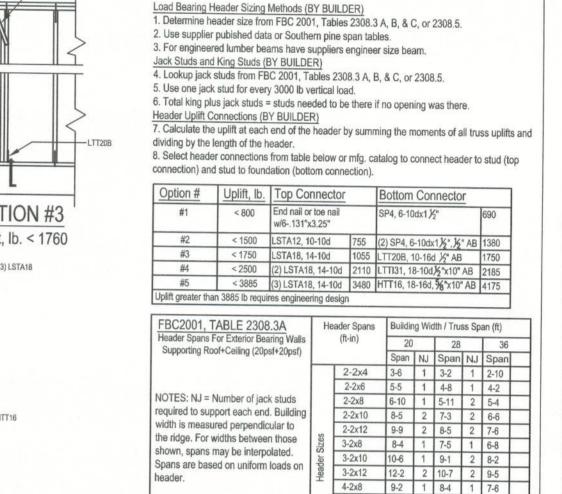
TERMITE TREATED FILL, EA. LIFT

COMPACTED TO MIN 95% MOD.

PROCTOR

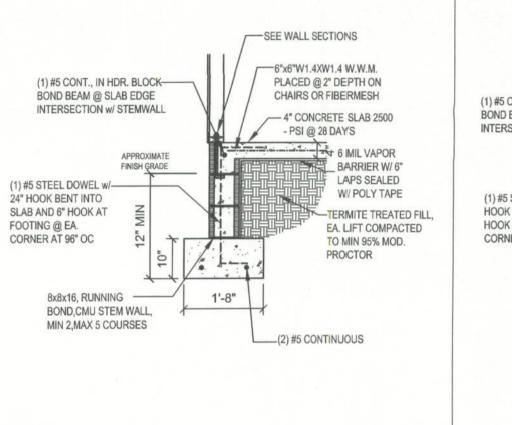
-(1) #5 CONTINUOUS





FOUNDATION: FOR POINT LOADS GRATER THAN 5000 Ib OR REPETITIVE TRUSS LOADS GRATER THAN

2000 Ib PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0"D X 1 sq ft. FOR EVERY 1000 Ib OF



F1 - STEM WALL FOUNDATION

2X4 OUTRIGGER @ 48" OC.

SCALE: 1/2"=1'-0" REV-27-MAY-03

BLOCKING REQUIRED BETWEEN OUTRIGGERS -

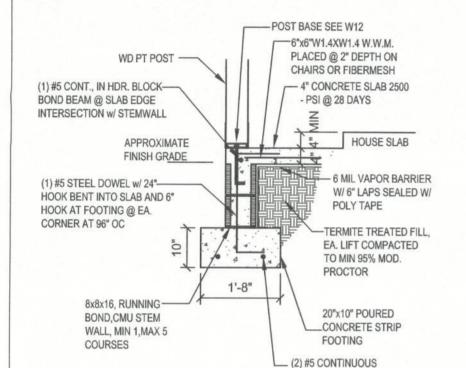
2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END -

4 - 10d NAILS OR 4 -

ALL CONNECTIONS

2X4 SCAB IF VERT.

WEB IS NOT PRESENT



F10 - STEM WALL PORCH FOOTING

BOTTOM CHORD OF-

TOE NAIL TRUSS TO -

DOUBLE PLATE w/ 16d

GABLE END TRUSS

COM @8" OC.

-2X6 SYP #2 STRONG

BACK (8) .131 X 3 1/4"

-H5A 6-8d TO EACH GABLE

TRUSS VERTICLE WEB

-(2) - 2X4 TOP PLATE

- SIMPSON LSTA

24 @ 48" OC.

SEE DETAIL W1

WITH TRAY CEILING

TO RAT RUN

-7/16" STRUCTURAL ROOF

HURRICANE

-CLIP H-2.5 OR

-TOP CHORD

OF GABLE END

TRUSS DROP 3

TOE NAIL TRUSS TO

COM @8" OC.

DOUBLE PLATE w/ 16d

BOTTOM CHORD OF

GABLE END TRUSS

- SIMPSON LSTA

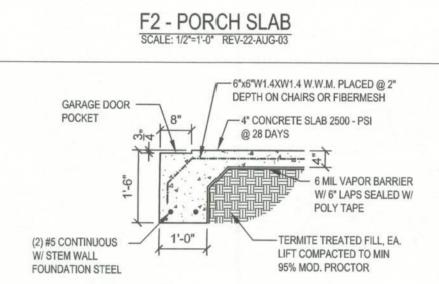
24 @ 48" OC.

SEE DETAIL W1

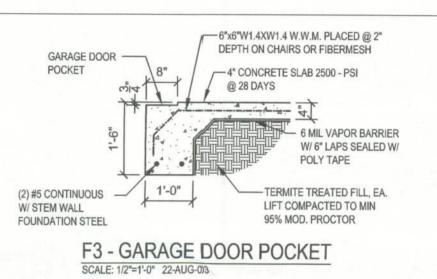
EQUAL 48" OC.

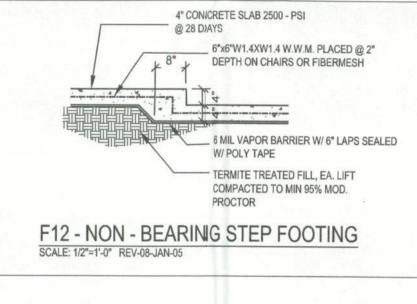
2X4 BARGE

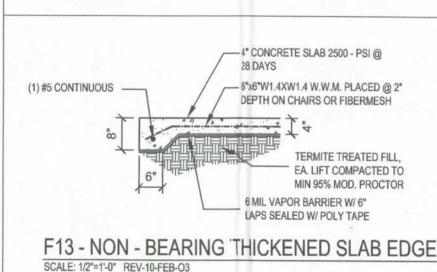
RAFTER CONT.



SLAB TO DRAIN







N5 - TRUSS UPLIFT CONNECTOR TABLE

8'-115" 6-812" 5'-915" 5'-912" INGLEHUNG DINING 13'-0" 9'-3" LIVING ROOM 10' CLG HGT 11'-31/2" 16'-31/2" BEARING WAL 2-CAR GARAGE BEDROOM * 5'-315" 5'-914" 5'-912" 6'-0" 10'-315" * FLOOR PLAN * A/C LIVING AREA = 1500 S.F. FRONT ENTRY = 44 S.F. 2-CAR GARAGE & MECH.CL = 430 6.F.

TOTAL AREA UNDER ROOF = 1974 6.F.

47'-6"

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'C = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL. CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.) REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAPS SPLICES 48 * db (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30 STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS

N2-GENERAL NOTES:

BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE

SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

W10 - TYPICAL GABLE END (X-BRACING)

(3) .131 X 3 4" -

CONT 2X4 SCAB FROM TOP

TO BOTTOM CHORD @

VERTICAL IF HIGHER THAN

X-BRACING (PROVIDE

ADDITIONAL 2X4'S @

48' TO FORM AN "L"

GUN NAILS

2X4 X-BRACE @ 6'-0" OC.

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter	
320	455	H3	4-8d	4-8d	
245	350	H5A	3-8d	3-8d	
535	600	H2.5A	5-8d	5-8d	
620	720	H10	6-10dx11/2"	6-10dx 1½"	
850	990	LTS12	8-8dx11½"	8-8dx 11/5"	
1245	1450	HTS20	10-10dd or 12-10dx11/2"	10-10d or 12-10dx 1½"	
1265	1470	H16, H16-2	10-10cdx1 1/2"	2-10dx 1½"	
1785	2050	LGT2	14-10cd Sinker	16-16d Sinker	
3655	4200	MGT	%" Thid Rod	22-10d	
SPF	SYP	Strap Connector	To) One Member	To Other Member	
760	885	SP4	6-10dx1½"	N/A	
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d	
1085	1265	LSTA18-24	7-10d	7-10d	
1170	1360	SPH4	12-10dix1½"	N/A	
420	455	SSP	4-10d	3-10d to double plate or 1-10d to single	
600	825	DSP	8-10d	6-10d to double plate or 2-10d to single	
1420	1650	CS16	14-8d (or 11-10d	14-8d or 11-10d	
SPF	SYP	Column Anchor	TωFoundation	To Column / Truss	
1160	1350	LTT19	%"x 166" AB	8-16d Sinkers	
1985	2310	LTTI31	%"x 166" AB	18-10dx 1½"	
2385	2775	HD2A	%"x 166" AB	2-%" Bolts	
3590	4175	HTT16	%"x 165" AB	18-16d	
1975 porting Trus	2300	ABU66	%"x 16" AB	12-16d	

hieve rated loads. All connections exposed directly to the weather shall be not dipped galvanized after fabrication. Loads are increased for wind

duration. Strap uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes (10d=.84*16d, 10dx1½*=.80*10d,

REV-18-NOV-04

N4-WIND LO	AD DESIGN DATA				N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engine structure to wind load requirements of FBC 2004, Section 1609. If trusses
(Wind loads are per FBC 2004, Section 1609 for eless than 60' or the least horizontal dimension; not >10% slope.)	nclosed simple diaphragm but sited on the upper half of an	uildings with m unobstructed	ean roof 60' high	height hill with	engineer of record for the trusses and did not design the trusses or dele BUILDER'S RESPONSIBILITY: The builder and owner are responsible of the wind load engineer's scope of work.
Basic Wind Speed	110	110 MPH			* Confirm that the foundation design & site conditions meet gravity load
Wind Exposure	В			capacity unless visual observation or soils test proves otherwise * Provide materials and construction techniques, which comply with FB	
Wind Importance Factor	1.0	1.0			velocity and design pressures. * Provide a continuous load path from roof to foundation. If you believe connection, call the wind load engineer immediately.
Building Category		II and the second secon			
Internal pressure Coefficient	N/A	N/A (Enclosed)			
Building not in the high velocity hurricane	zone	* Verify the truss engineering includes truss design, placement plans, te			
Building not in the wind-borne debris region	n	truss-to-truss connections, and load reactions for all bearing locations.			
Mean Roof Height	< 30	< 30 ft			* Select uplift connections, walls, columns, and footings based on truss including interior bearing walls.
Roof Angle	10-	10-45 degrees			* Size headers for gravity loads; headers sized by the builder for gravity
Components And Cladding Wind Pressure	es (FBC Table1609 B&C)				
5 2 2 2 1	Zone 4 5 Total	10 21.8 -23.6 21.8 -29.1 Shear Wall S	18.5	100 -20.4 -22.6	DOCUMENT CONTIROL and PRIORITY: Structural requirements on S- architectural sheets lhave more stringent requirements. Non-structural re Specific requirements take precedence over general requirements. Rev is the responsibility of the builder. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby its common law copyrights and property right in these instruments of set
1	2'-4"min for	2'-4"min for 8'-0"H wall 2'-10"min for 10'-0"H wall			not to be reproduced, altered or copied in any form or manner without
\ \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\				itudinal	of Mark Disosway.
55	Required		15.4'		DIMENSIONS:
The state of the s	ACTUAL	89.0' or walls are typ SHEAR WAL	length i	s the total	Stated dimensions supercede scaled dimensions. Refer all questions to proceed without clarification.
5 2 3	sheathing	of all wall segments with full height sheathing and width to height ratio greater than 1: 3.5 (plus special shear wall			WINDLOAD ENGINEER: Mark Disosway, PE No.53915
1 1	segments WALL len	if noted.) RE	QUIRED FCM-200	SHEAR	CERTIFICATION: The attached plans and "Windload Engineering", she wind loads, to the best of my knowledge.
515	adjustme	3.17A & 3.17B with table 3.17E adjustment for type II shear wall (or equivalent calculation)			LIMITATION: This design is valid for one building, at specified location. This drawing is not valid for construction unless raised seal is affixed.

NDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the are to wind load requirements of FBC 2004, Section 1609. If trusses are used, the wind load engineer is not neer of record for the trusses and did not design the trusses or delegate to the truss designer.

ER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part wind load englineer's scope of work. nfirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing

ride materials and construction techniques, which comply with FBC 2004 requirements for the stated wind ty and design pressures. ride a continuous load path from roof to foundation. If you believe the plan omits a continuous load path ction, call the wind load engineer immediately

the truss engineering includes truss design, placement plans, temporary and permanent bracing details, o-truss connections, and load reactions for all bearing locations.

ct uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions; ling interior bearing walls. headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads.

MENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or ctural sheets lhave more stringent requirements. Non-structural requirements on architectural sheets control. c requirements take precedence over general requirements. Revision control is by the latest signature date and esponsibility of the builder.

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dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not without clarification.

TICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2004, Section 1609 ads, to the best of my knowledge.

Sheet S-1 of 2 Sheet Windload Engineering

REV-27-Jul-04 WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disosway P.E. POB 868, Lake City, FL 32056 Phone: (386) 754-5419 Fax: (386) 754-6749 Email: windloadengineer@bellsouth.net

Location: Lot # 96 Emerald Cove S/D, Lake City, Florida

The Samuel Model

Builder: Cornerstone Development

Designer: Teena Ruffo



Job # 510318