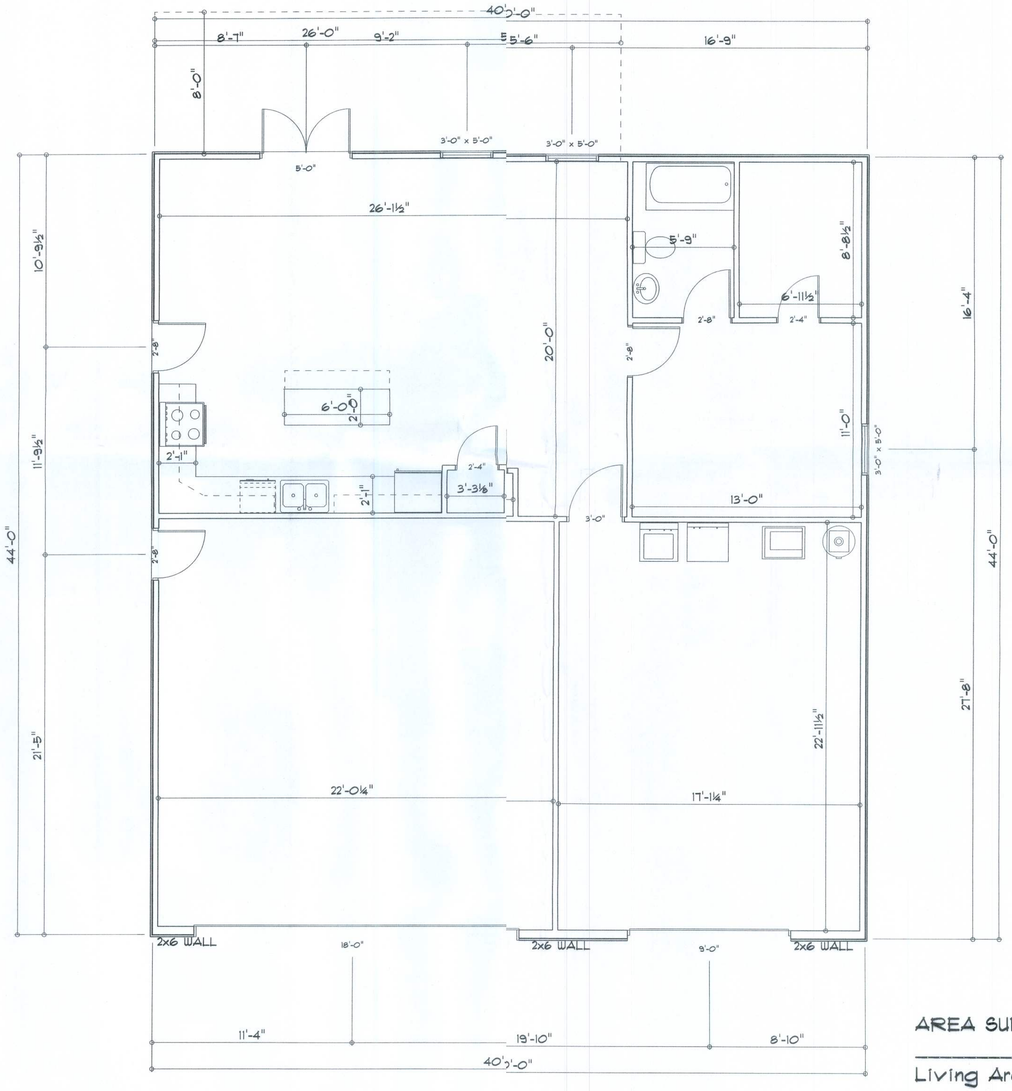
REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



AREA SUMMERY

Living Area	788 S.F.
Garage Area	923 S.F.
Total Area	1711 S.F.

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Codes Compliance
PLANS EXAMINER

Floor Plan

RESIDENCE

Mark Hadox
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Lake City, L 32024

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Woodman Pak Builders, Inc.
Lake City, Florida
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Fax: (386 755-8684
Email:

DESIGNED BY:
Mark Haddox

DRAWING NUMBER
A-1

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

Electrical Plan Notes:

E-1 Wire all appliances, HAC units and other equiptment per manufactures speifications.

E-2 Consult the owner fcthe number or seperate telephone lines to b installed. Owner is responsible for all osrages not noted on plan.

E-3 All installations shall :e per national code 2008.

E-4 All smoke detectorshall be 120v with battery back-up of the photoisctric type, and shall be interlocked togethe install inside and near all bedrooms.

E-5 Telephone, televisic and other low voltage devices or outlets all be as per the owners directions and in acordance with applicable sections of the Natioal Electric Codes latest edition. Owner is resonsible for all overages not noted on plan.

E-6 Electrical contractoshall be responsible for the design and sizin; of slectrical service and circuits.

E-7 Entry of service (unerground or overhead) to to be determined bcontractor agreement.

E-8 All outlets located iresidential to be tamper-resistant paNEC.

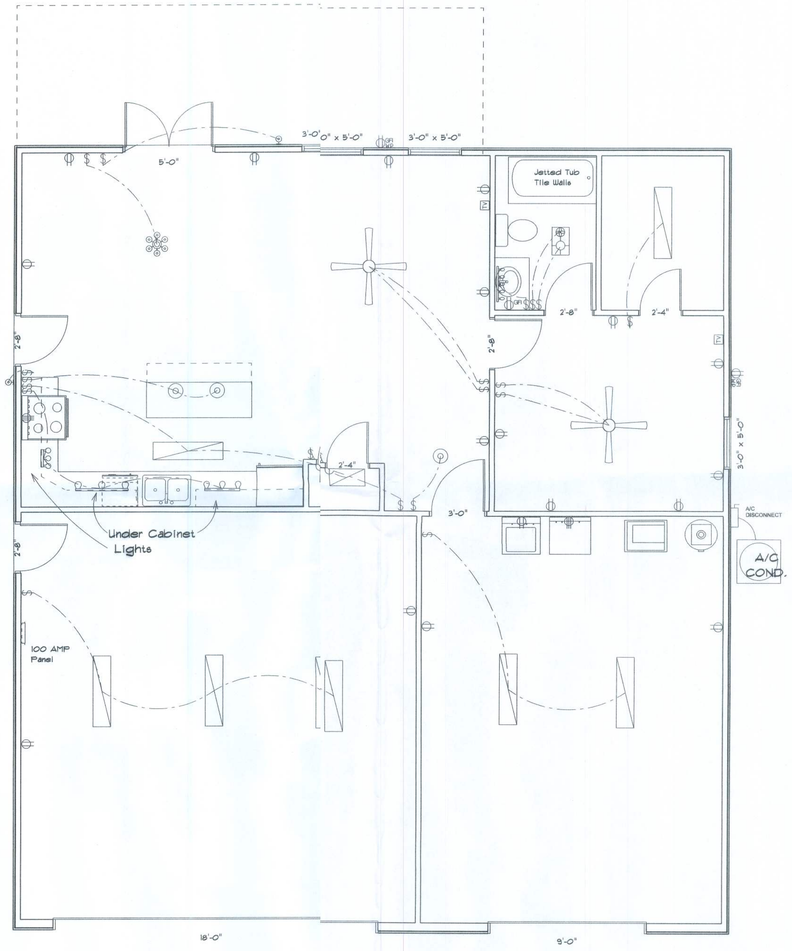
E-9 All outlets to be loated above base flood elevation.

E-10 All exterior GFI outlts shall be weatherproof.

E-11 Overcurrent Protecbn device shall be installed on the exteor of structures on the load side of themeter to serve as a disconnscting mans. Conductors used from the exterior disconnscting means to a panel or sub pansl shall have3ur-wire conductors, of which one conductoshall be used as an equiptment ground.

E-12 All 120-VOLT, single nase, 15 and 20 ampere branch circuits supping outlets installed in dwelling unit family rooms,dinin rooms, living rooms, parlors, libraries, dens, bedrooms, sun rooms, recreation rooms, closets, hallways, or sillar rooms or areas shall be protected by a liste arc-fault circuit interrupter, combination-type instead to provide protection of the branch circuit.

E-13 Carbon Monoxide alms shall be required within 10' of all rooms for sleeping purposes in buildings having a fossil-fuel burning bater or appliance, a fireplace or attached garage.



Electrical Plan

ELECTRICAL	SYMBOL
ceiling fan globe l	
ceiling globe light	
chandelier	
fluorescent fixture	
track light	
vanity bar light	
wall sconce	
electrical panel	
AC Disconnect	A/C DISCONNECT, A/C COND.
Outlet WP GFI	
cable tv outlet	TV
fan	
light	
outlet	
outlet 220v	
outlet gfi	GFCI
switch	$

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DESIGNED BY:
Mark Haddox

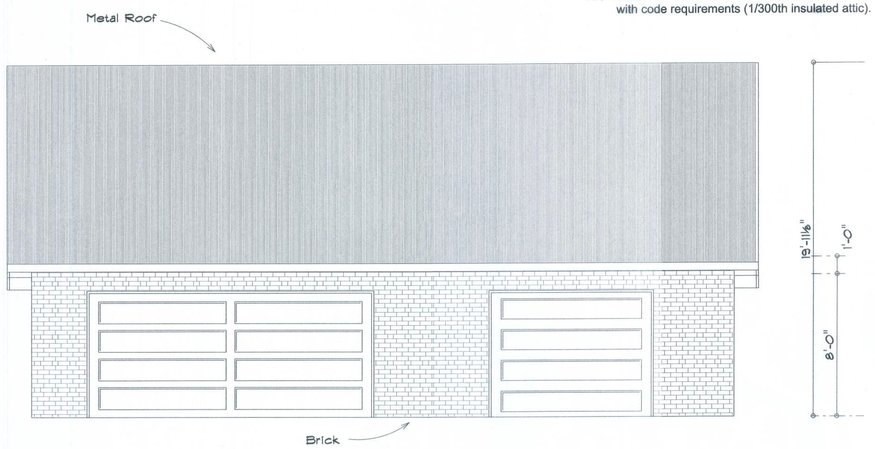
DRAWING NUMBER
A-2

Notes:

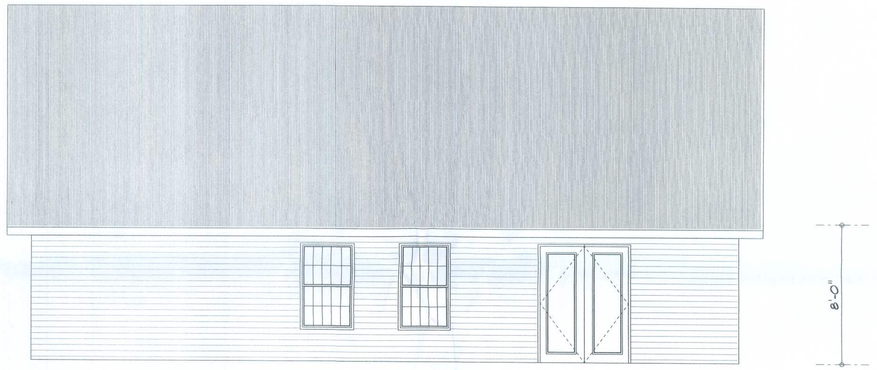
R-1 All roof pitches shall be 6/12 unless otherwise noted.

R-2 All overhangs shall be 24" except on gables 18".

R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).



Front Elevation



Rear Elevation

Right Elevation

Left Elevation

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

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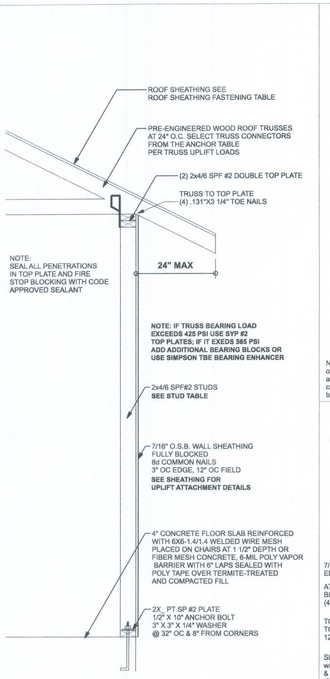
DRAWINGNUMBER
A-3



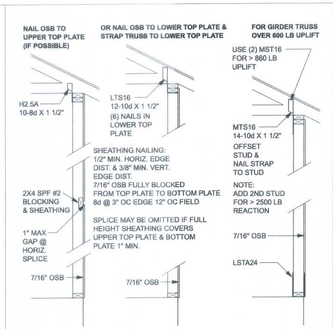
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

ROOF SHEATHING FASTENING TABLE (RAFTER / TRU: SG = 0.49)

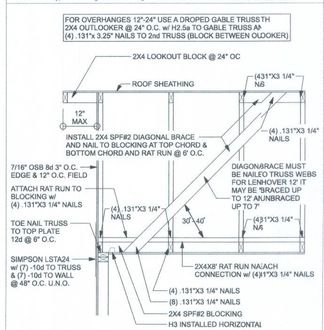
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nailacing alointermediate supports in the panfield
120 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	12"
120 mph Exp. C	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" c
120 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" c
130 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" c
130 mph Exp. C	15/32"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" c
130 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" c
140 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" c
140 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" c
140 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" c
150 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" c
150 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	4" oc	4" c

Note: For sheathing located a minimum of 4 feet from the perimeter edge of theof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on centslong panel edges and 6 inches on center along intermediate supports in the panel field. Note: Thible specifies the code minimum thickness of roof sheathing. The thickness of the sheathing maysd to be increased based in the type of roofing material being used. See manufacturer Florida prod approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS

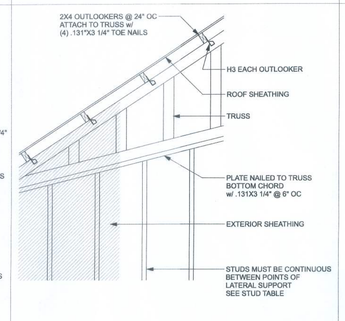
ONE STORY WOOD FRAME



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLSED

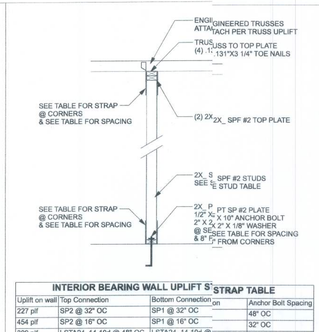
(TYP.) GABLE BRACING DETAIL

WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME



INTERIOR BEARING WALL UPLIFT STRAP TABLE

Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 plf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 plf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
309 plf	LSTA24, 14-10d @ 48" OC Wrap Over Plate	LSTA24, 14-10d @ 48" OC Wrap Over Plate	48" OC
465 plf	LSTA24, 14-10d @ 32" OC Wrap Under Plate	LSTA24, 14-10d @ 32" OC Wrap Over Plate	32" OC

(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"
575	495	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	**Uplift SPF**	**Strap Ties**	**To One Member**	**To Other Member**
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	**Uplift SPF**	**Stud Plate Ties**	**To Stud**	**To Plate**
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	**Uplift SPF**	**Holdowns @ Stemwall**	**To Stud / Post**	**Anchor**
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	**Uplift SPF**	**Holdowns @ Mono**	**To Stud / Post**	**Anchor**
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	**Uplift SPF**	**Post Bases @ Stamwall**	**To Post**	**Anchor**
1900		ABU44Z	12-16d	5/8"x12" Drill & Epoxy
2475		ABU66Z	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	**Uplift SPF**	**Post Bases @ Mono**	**To Post**	**Anchor**
1900		ABU44Z	12-16d	5/8"x7" Drill & Epoxy
2475		ABU66Z	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS). UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

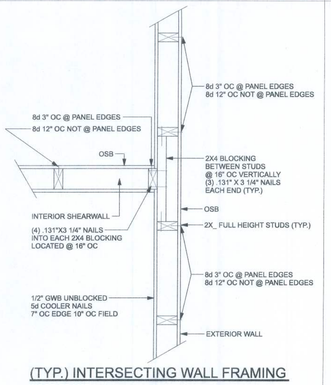
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

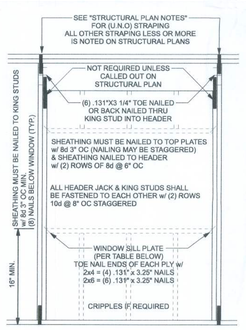
ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



(TYP.) INTERSECTING WALL FRAMING

WOOD FRAME



TYPICAL HEADER STRAPING DETAIL

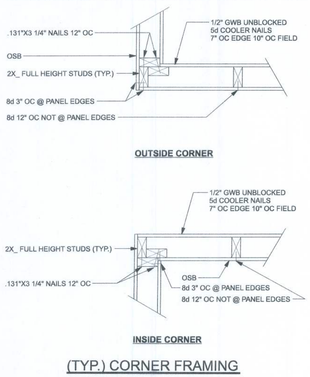
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

HEADER STRAP TABLE

Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
< 1455	MSTA24, 18-10d header to jacks	DTT2Z
< 1800	(2) MSTA24, 18-10d header to jacks	DTT2Z
< 2910	(2) MSTA24, 18-10d header to jacks	HTT4

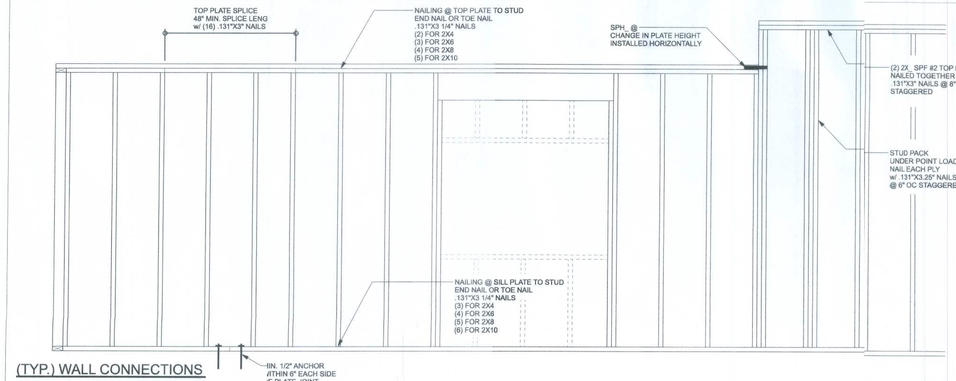
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2 (1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	BASED ON WFCM TABLE A-3.23B FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



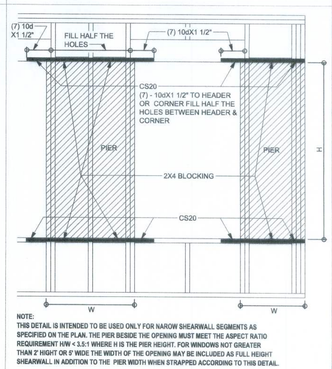
(TYP.) CORNER FRAMING

WOOD FRAME



(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME



NOTE:
THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT H/W < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGHT OR 3' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

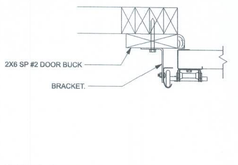
OPENING FORCE TRANSFER

WOOD FRAME

2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW.

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION

WOOD FRAME

DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd)	-34.2(Vasd)
0 - 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult)	-57(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.5(Vasd)	-25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd)	-24.1(Vasd)

Woodman Park Builders, Inc.

Mark Haddox Arden

PROJECT ADDRESS:
372 Sunday Gln
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all quotions to Mark Disoway, P.E. for resolution. Do not proceed without clrrification.

CERTIFICATION: I hereb certify that I have examined this plan, and tat the applicable portions of the plan, relatig to wind engineering comply with the 7th Editio Florida Building Code Residentia(2020) to the best of my knowledge.

LIMITATION: This designs valid for one building, at specified locaon.

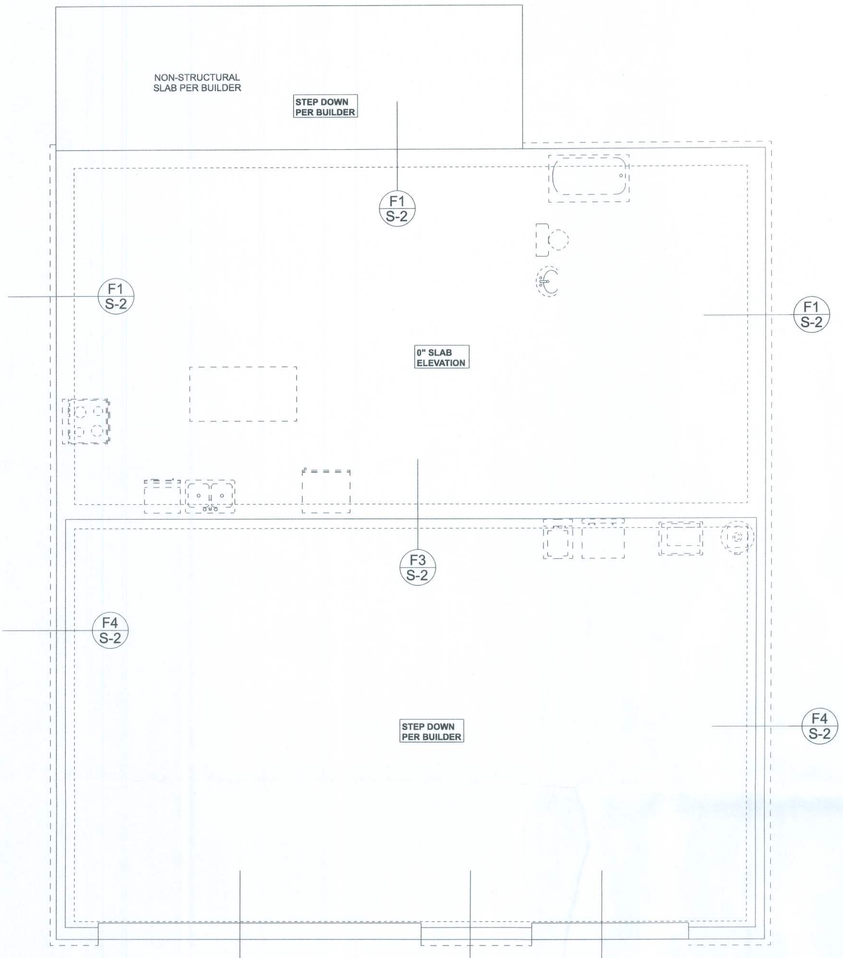
MARK DISOWAY P.E. 53915

Tuesday, September 7, 2021

Mark Disoway P.E.
163 SW Midown Place
Suite103
Lake City, Flrida 32025
386.75-.5419
disoswaydesign@gmail.com

JOB NUMBER:
21145

S-1
OF 2 SHEETS

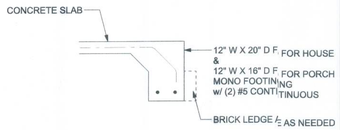


FOUNDATION PLAN

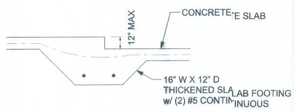
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

BOTTOM OF EXTERIOR FOOTIS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SODR ENGINEERED FILL

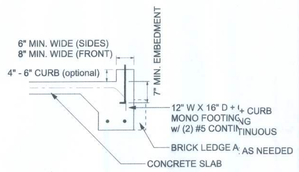


F1 S-2 MONOLITHIC FOOTING



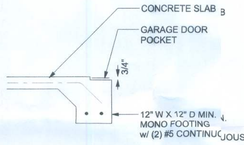
F3 S-2 INTERIOR BEARING STEP FOOTING

SCALE: 1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING

SCALE: 1/2" = 1'-0"

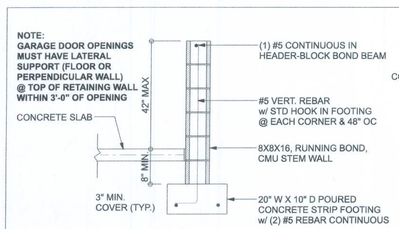


F5 S-2 GARAGE DOOR POCKET FOOTING

SCALE: 1/2" = 1'-0"

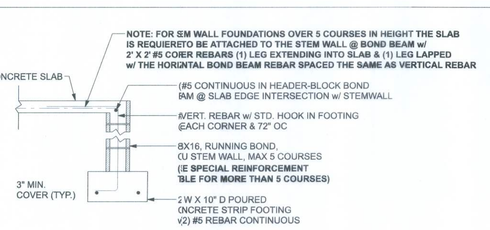
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). BEFORE THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" nominal bond and 12"x12" or 16"x16" running block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or ASTM
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints as if not detailed on project drawings.



F4 S-2 OPTIONAL STEM WALL CURB FOOTING

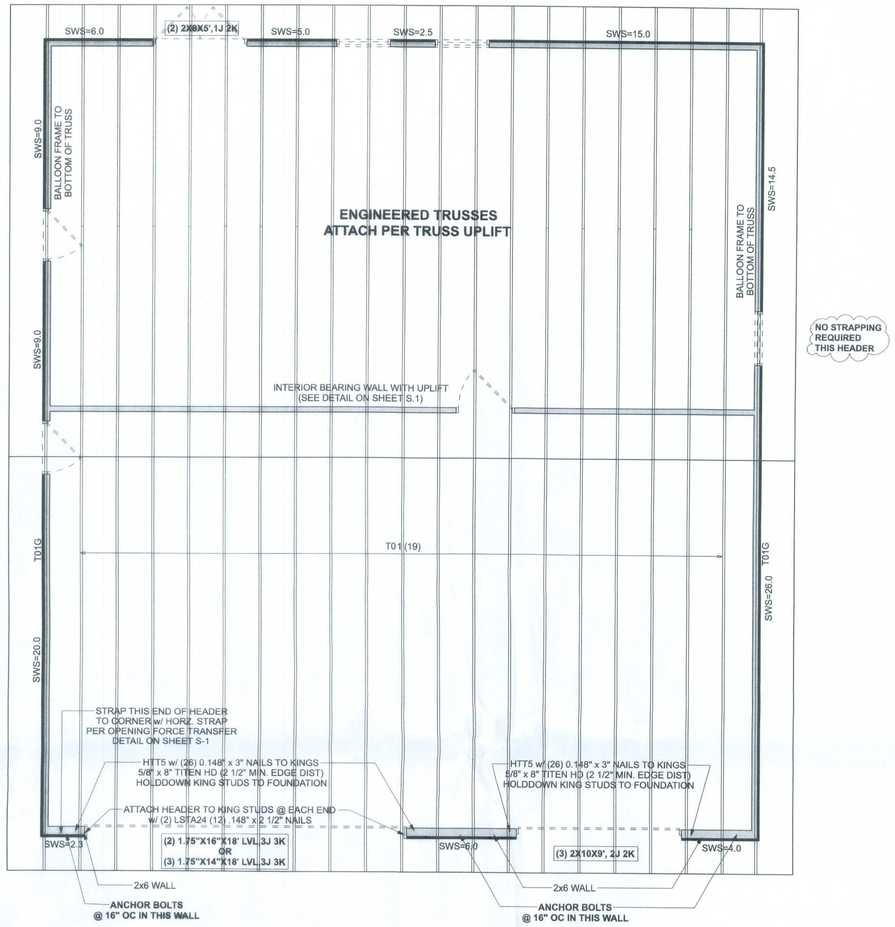
SCALE: 1/2" = 1'-0"



F1 S-2 OPTIONAL STEM WALL FOOTING

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, with a minimum 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2x6 SP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

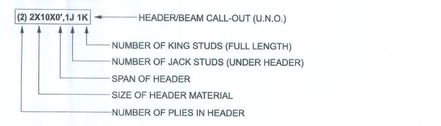
SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)

SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD

SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDUNAL
ACTUAL	31086 LBF	17190 LBF
REQUIRED	14563 LBF	11562 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2918891

Woodman Park Builders, Inc.

Mark Haddox - Arden

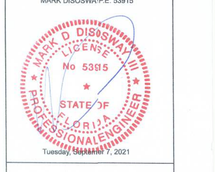
PROJECT ADDRESS:
372 Sunday Glo
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questns to Mark Disoway, P.E. for resolution. Do not proceed without clafication.

CERTIFICATION: I hereby ertify that I have examined this plan, and ths the applicable portions of the plan, relatin to wind engineering comply with the 7th Edition Florida Building Code Residential (020) to the best of my knowledg.

LIMITATION: This design iswalid for one building, at specified locatin.

MARK DISOSWAY P.E. 53915



Tuesday, December 7, 2021

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163 SW Midtwn Place
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Lake City, Florida 32025
386.7545419
disoswaydesigr@gmail.com

JOB NUMBER:
2112-5

S-2

OF 2 SHEETS