ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V3B487-Z0124144926

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

Truss Fabricator	Anderson Truss Company
Job Identification	14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake Cit[illegible], FL
Truss Count	15
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

01/24/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	67186	A 24'3" Common	14024145	01/24/14
2	67187	A1 24'3"8 Mono H	14024146	01/24/14
3	67188	ACE 24'3" Specia	14024147	01/24/14
4	67189	B 35'5" Common	14024148	01/24/14
5	67190	BGE 17' Gable	14024149	01/24/14
6	67191	CJ1 1' Jack	14024150	01/24/14
7	67192	CJ3 3' Jack	14024151	01/24/14
8	67193	CJ5 5' End Jack	14024152	01/24/14
9	67194	EJ7 7' End Jack	14024153	01/24/14
10	67195	H7 24'3"8 Mono H	14024154	01/24/14
11	67196	HJ7 9'10"13 Hip	14024155	01/24/14
12	67197	H9 24'3"8 Mono H	14024156	01/24/14
13	67198	H11 24'3"8 Mono	14024157	01/24/14
14	67199	M2 5'8"8 Common	14024158	01/24/14
15	67200	M1 10'3"8 Mono	14024159	01/24/14

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City FL - A 24 3 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

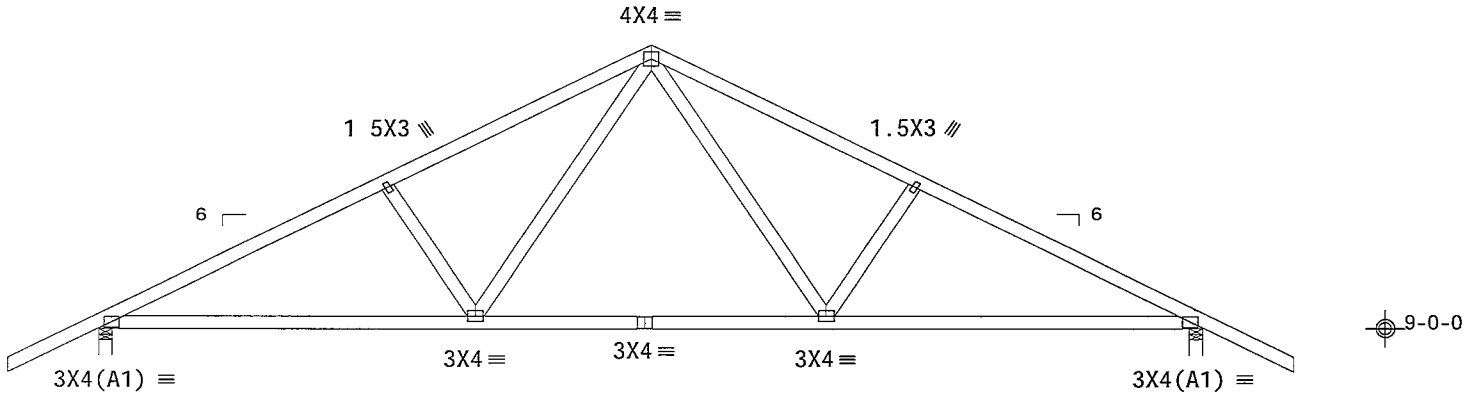
Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24 -wide clearance

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03 04 QTY:3 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabr cating handling sh pp ng nstall ng and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rig d ceiling Locations shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping installat on bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherwise Refer to drawings 160A-Z for standard p ate pos t ons A seal on th s draw ng or cover page listing this draw ng indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitabil ty and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2014

TC LL	20.0 PSF	REF	R9114- 67186
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024145
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	345973
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - A1 24 3 8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24' -wide clearance

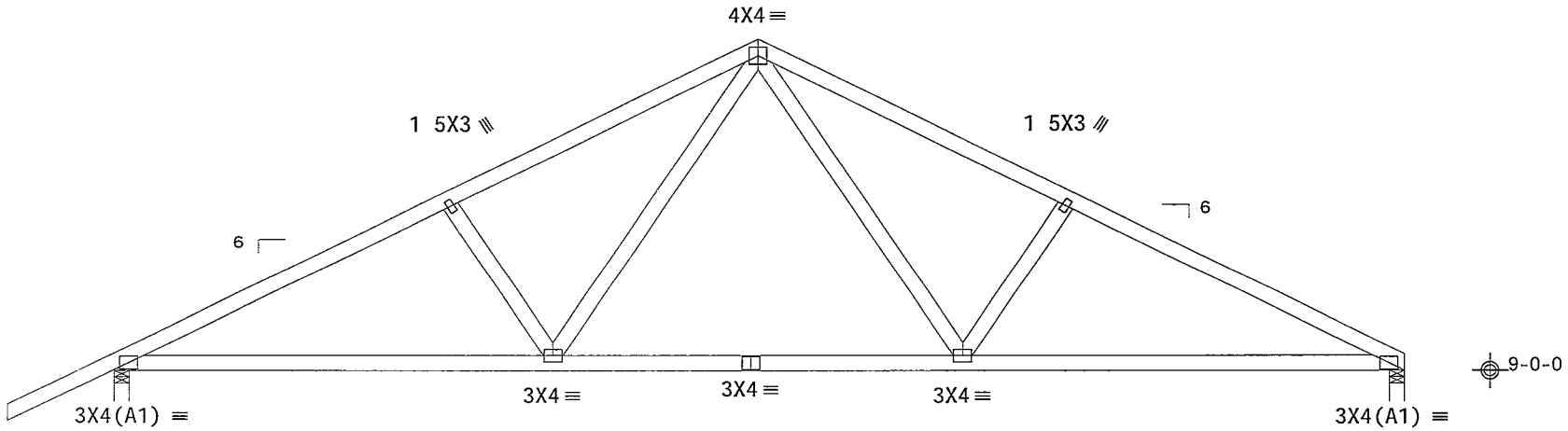
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=1098 U=49 W=3 5' (3 5 min)
RL=100/-107

R=967 U=29 W=3 5" (3 5" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 QTY:8 FL/-/4/-/-/R/- Scale =.3125"/Ft.

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Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this desi any fa lure to build the truss in conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallation brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Dcta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate positions A seal on th s draw ng or cover page l st ng this draw ng nd cates acceptance of professional eng neering responsibility solely for the des gn shown The su tab lity and use of this design for any structure is the responsibil ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

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01/24/2014

TC LL	20 0 PSF	REF	R9114- 67187
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024146
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	345977
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - AGE 24 3 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

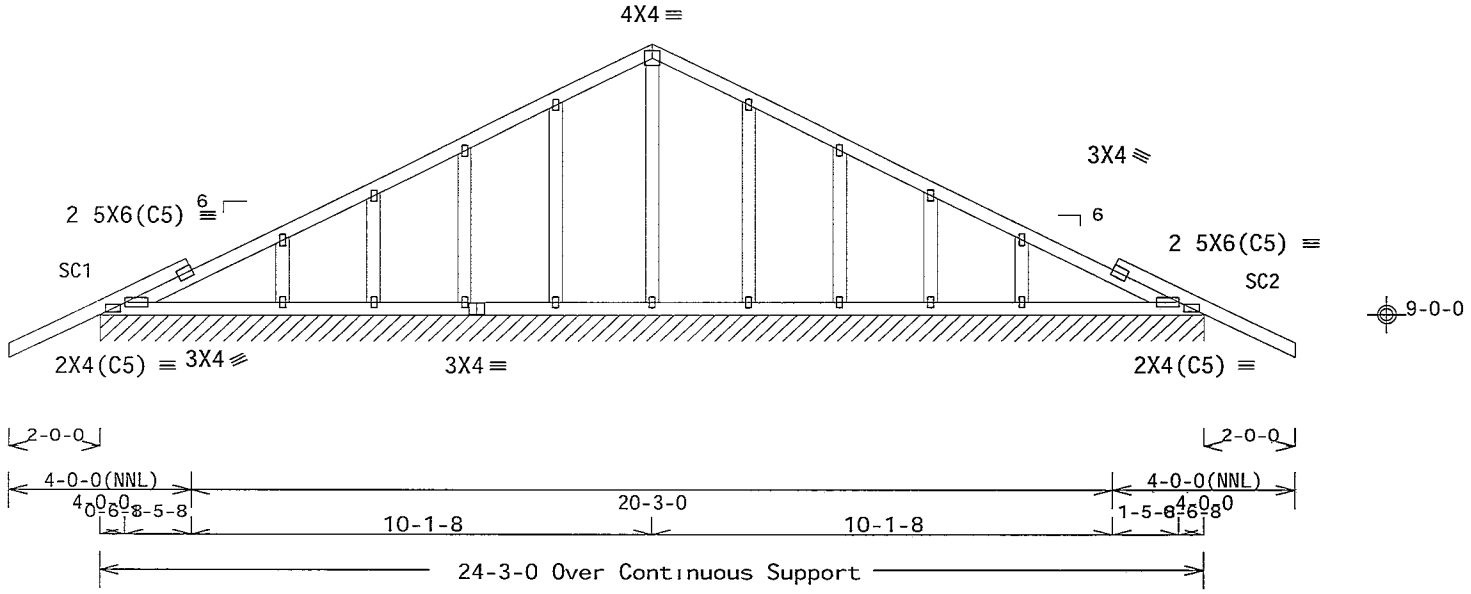
Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=191 PLF U=24 PLF W=24-3-0
RL=11/-11 PLF

Note All Plates Are 1 5X3 Except As Shown.

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]43 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping installation bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherw se Refer to drawings 160A-Z for standard plate pos t ons A seal on th s draw ng or cover page listing this draw ng indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitability and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www ccsafe org

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01/24/2014

TC LL	20.0 PSF	REF R9114- 67188
TC DL	7.0 PSF	DATE 01/24/14
BC DL	10.0 PSF	DRW HCUSR9114 14024147
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN- 346111
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - B 35'5 Common)

Top chord 2x4 SP #1-13B T1 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

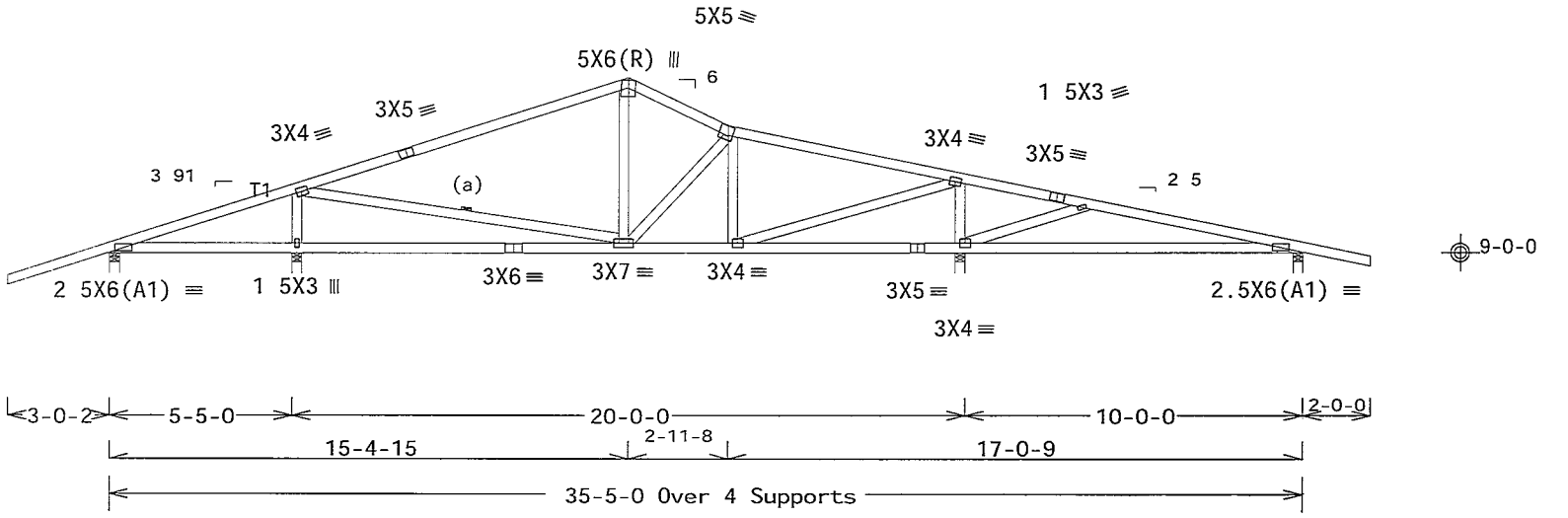
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:8 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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Orlando FL, 32837
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ITW Build ng Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s des g any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handling shipp ng nstallat on & brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page listing this draw ng nd cates acceptance of profess ona engineering responsibility solely for the design shown The suitabil ty and use of th s design for any structure is the respons b lity of the Building Des gner per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

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01/24/2014

TC LL	20.0 PSF	REF	R9114- 67189
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024148
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD.	37.0 PSF	SEQN-	26918
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - BGE 17' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP M-30
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0 OC designed to support 0-6-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

These hangers and support conditions used at bearings indicated
(H1) = Simpson
(H2) = (J) Hanger not calculated

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

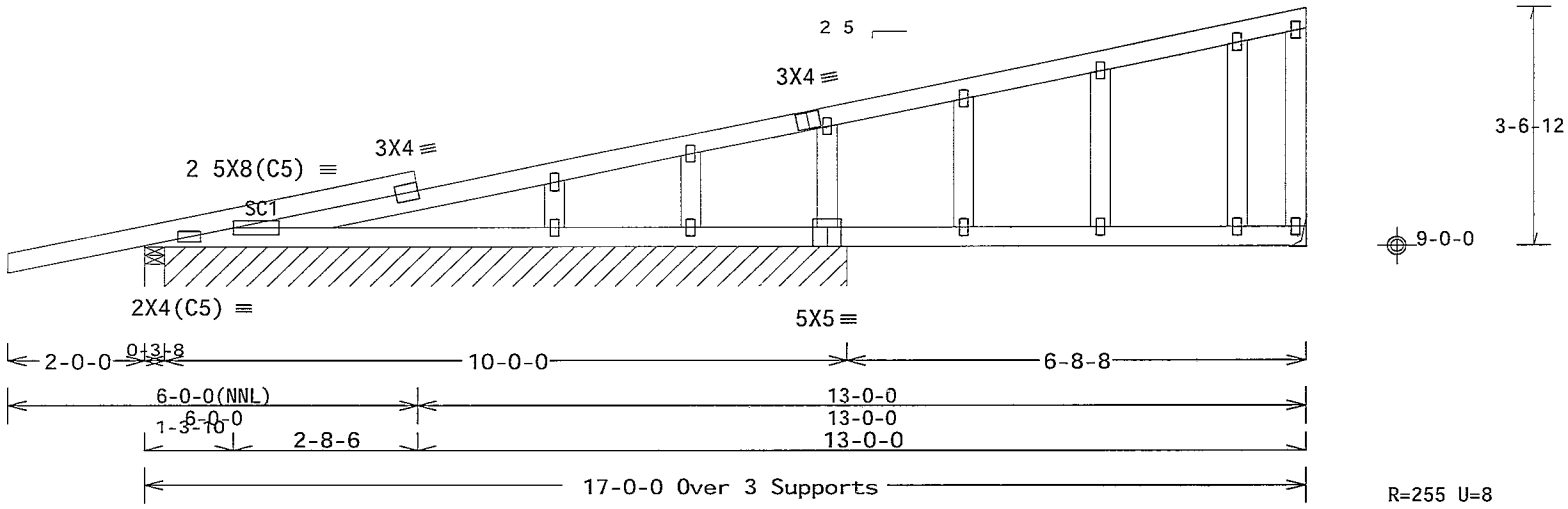
Right end vertical not exposed to wind pressure

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 1 5X3 Except As Shown

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

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Orlando FL, 32837
FL COA #0 278

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01/24/2014

TC LL	20.0 PSF	REF	R9114- 67190
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024149
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	26170
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - CJ1 1 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

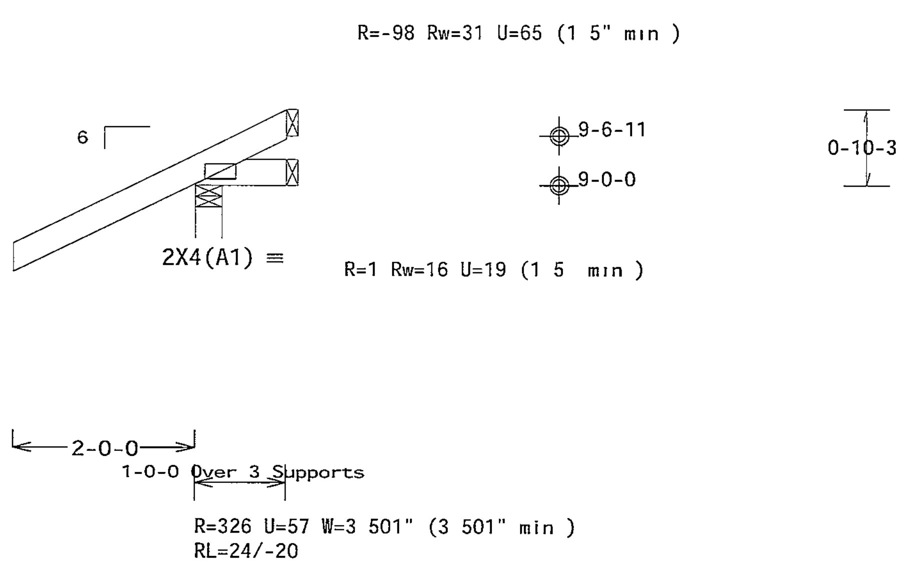
Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50.



Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0) 12.03.04 .26.13 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP Wave

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01/24/2014

TC LL	20.0 PSF	REF	R9114- 67191
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024150
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	346230
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - CJ3 3 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

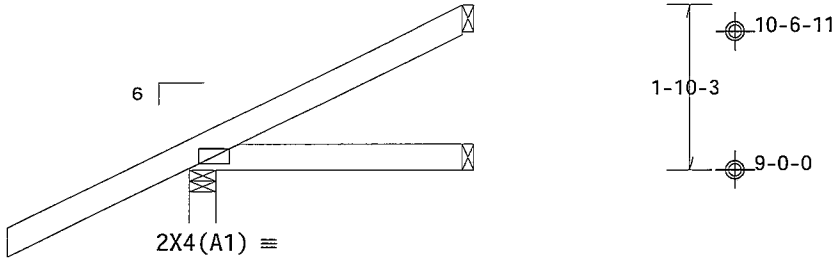
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

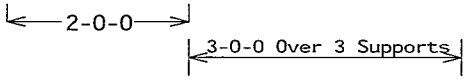
Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

R=44 U=10 (1 5' min)



R=47 U=0 (1 5" min)



R=288 U=23 W=3 501" (3 501' min)
RL=41/-24

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

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Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

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ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this desi any failure to bu ld the truss in conformance with ANSI/TPI 1 or for hand ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this draw ng or cover page l st ng this draw ng nd cates acceptance of profess onal engineering respons b l ty solely for the des gn shown The su tab l ity and use of th s des gn for any structure is the respons bil ty of the Building Des gner per ANSI/TPI 1 Sec 2 For more information see Th s job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2014

TC LL	20.0 PSF	REF	R9114- 67192
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024151
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	346234
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - CJ5 5 End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

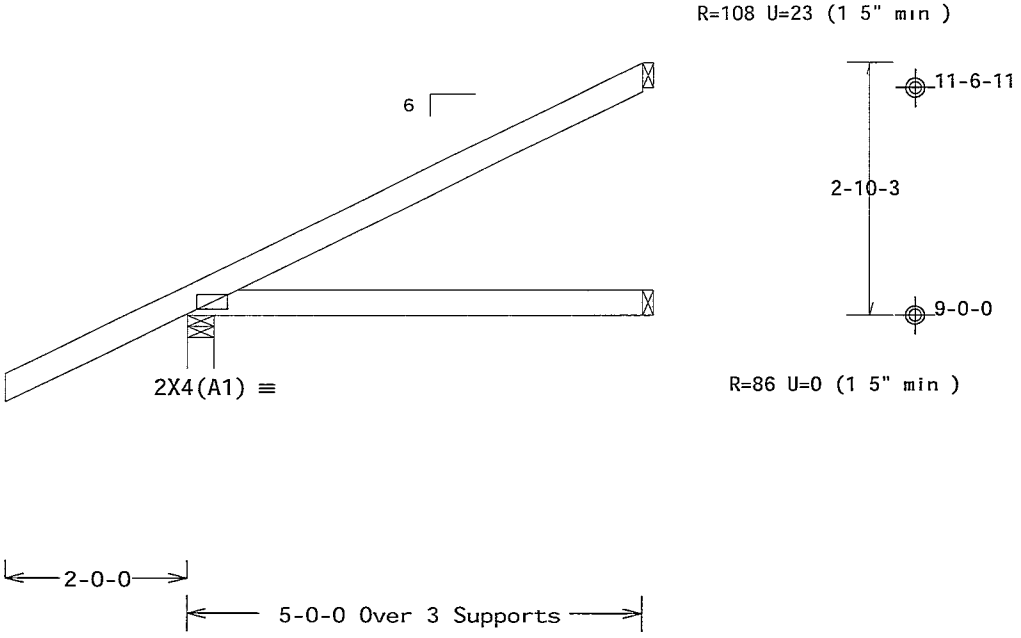
Lumber grades designated with '13B use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 0326 13 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2014

TC LL	20.0 PSF	REF	R9114- 67193
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024152
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	346228
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - EJ7 7 End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

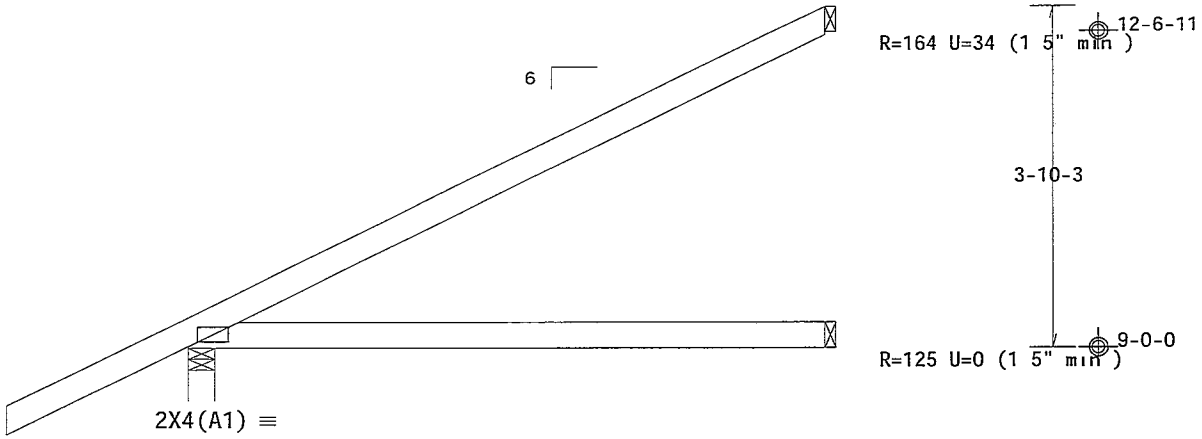
Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 0326 13 QTY:9 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2014

TC LL	20.0 PSF	REF	R9114- 67194
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024153
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	346261
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City FL - H7 24 3'8 Mono Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B W6 2x4 SP #2-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

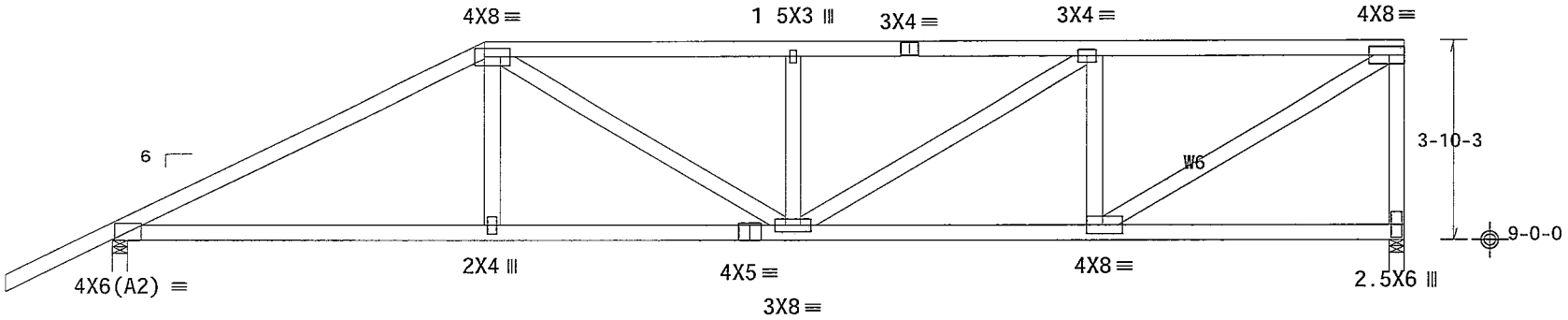
In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at -2 00 to 56 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 24 29
BC- From 4 plf at -2 00 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 24 29
TC- 227 74 lb Conc Load at 7 03
TC- 163 56 lb Conc Load at 9 06,11 06,13 06,15 06 17 06,19 06,21 06,23 06
BC- 417 92 lb Conc Load at 7 03
BC- 125 00 lb Conc Load at 9 06,11 06,13 06,15 06 17 06,19 06,21 06,23 06
BC- 255 44 lb Conc Load at 24 06

Right end vertical not exposed to wind pressure

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 03/26/17 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2014

TC LL	20.0 PSF	REF	R9114- 67195
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024154
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	26173
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - HJ7 9 10 13 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

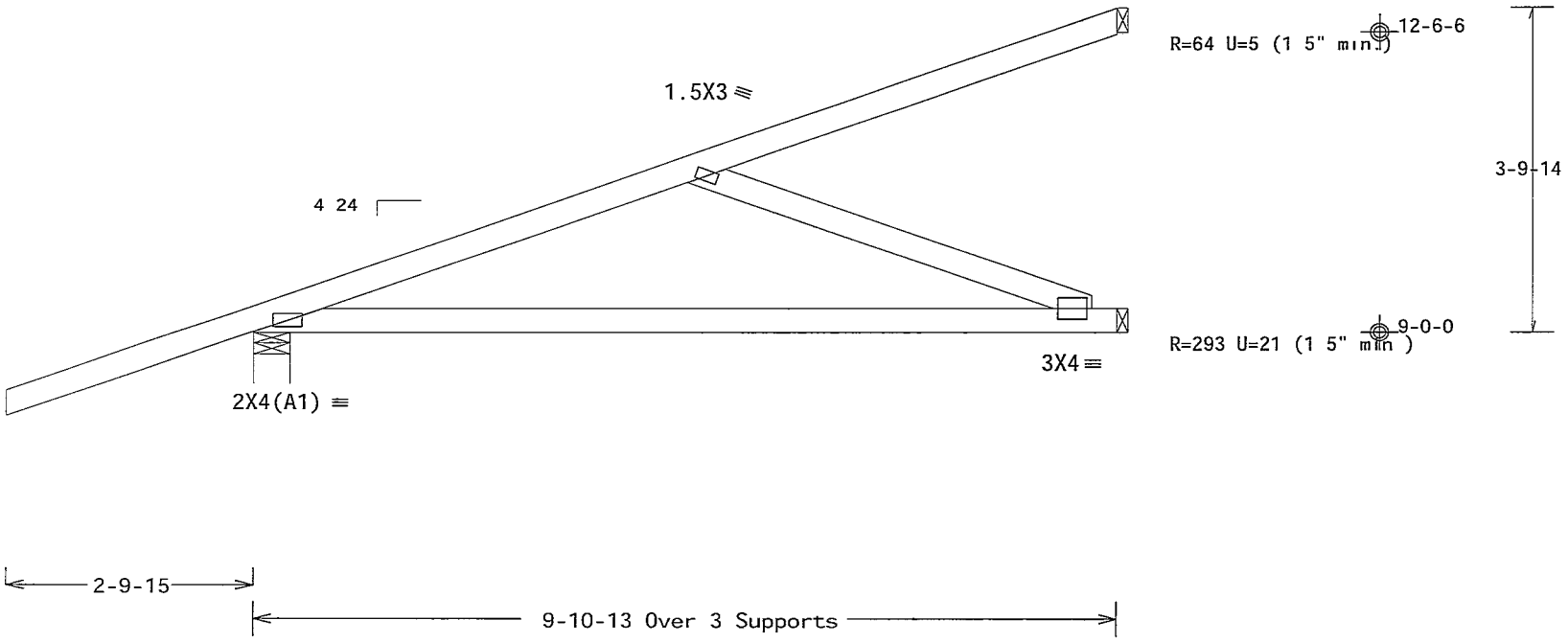
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 83 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -2 83 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 90
TC- -63 44 lb Conc Load at 1 48
TC- 87 81 lb Conc. Load at 4 31
TC- 215 62 lb Conc Load at 7 13
BC- 1 08 lb Conc Load at 1 48
BC- 93 03 lb Conc. Load at 4 31
BC- 172 84 lb Conc Load at 7 13

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 R6 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/24/2014

TC LL	20.0 PSF	REF	R9114- 67196
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024155
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	346263
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City FL - H9 24 3 8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

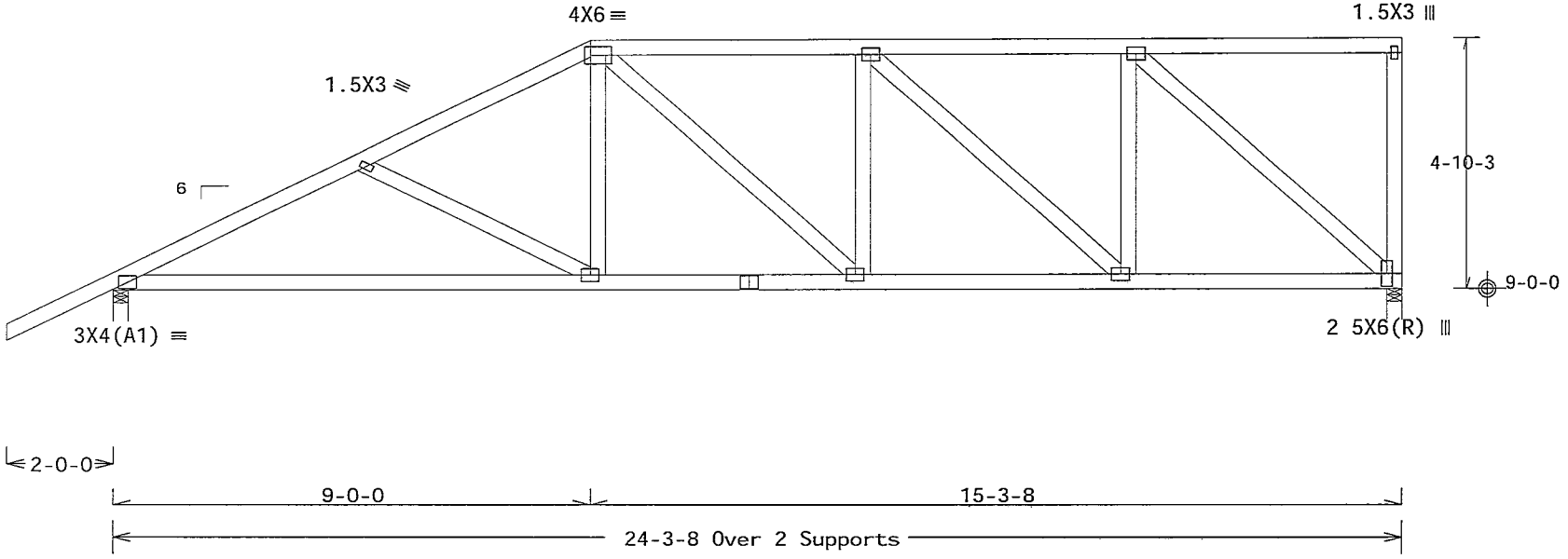
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge



R=1050 U=46 W=3 5" (3 5' min)
RL=91/-35

R=908 U=46 W=3 5" (3 5" min.)

Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/24/2014

TC LL	20.0 PSF	REF	R9114- 67197
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024156
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD.	37.0 PSF	SEQN-	345979
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - H11 24 3'8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

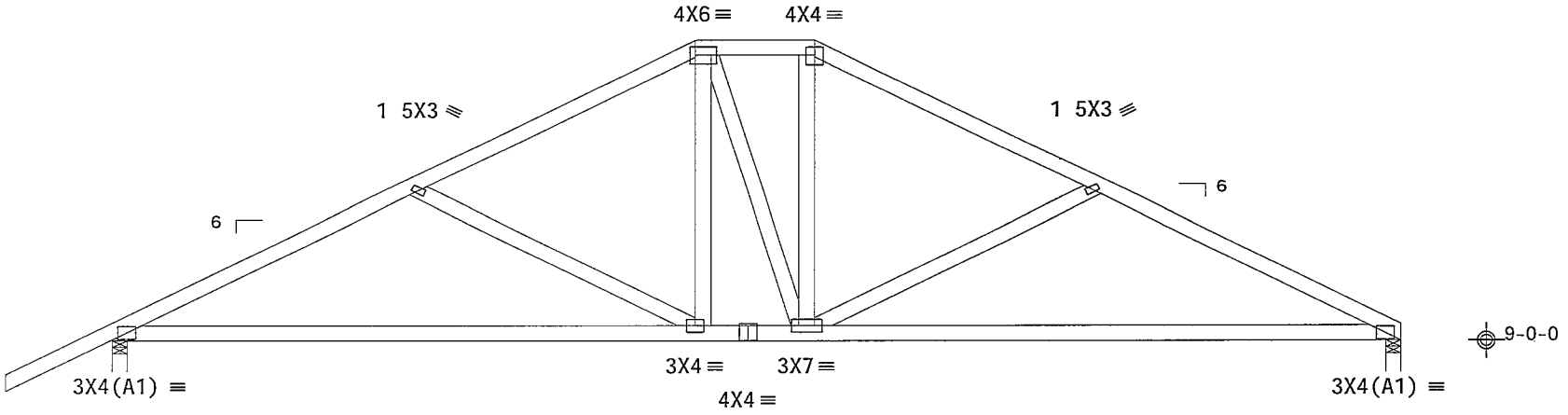
MWFRS loads based on trusses located at least 7 50 ft from roof edge

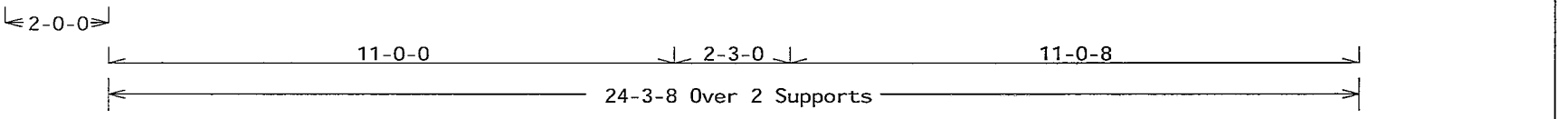
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1045 U=50 W=3 5" (3 5" min)
RL=92/-99

R=913 U=30 W=3 5" (3 5" min.)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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01/24/2014

TC LL	20.0 PSF	REF	R9114- 67198
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024157
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	345961
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - M2 5'8 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

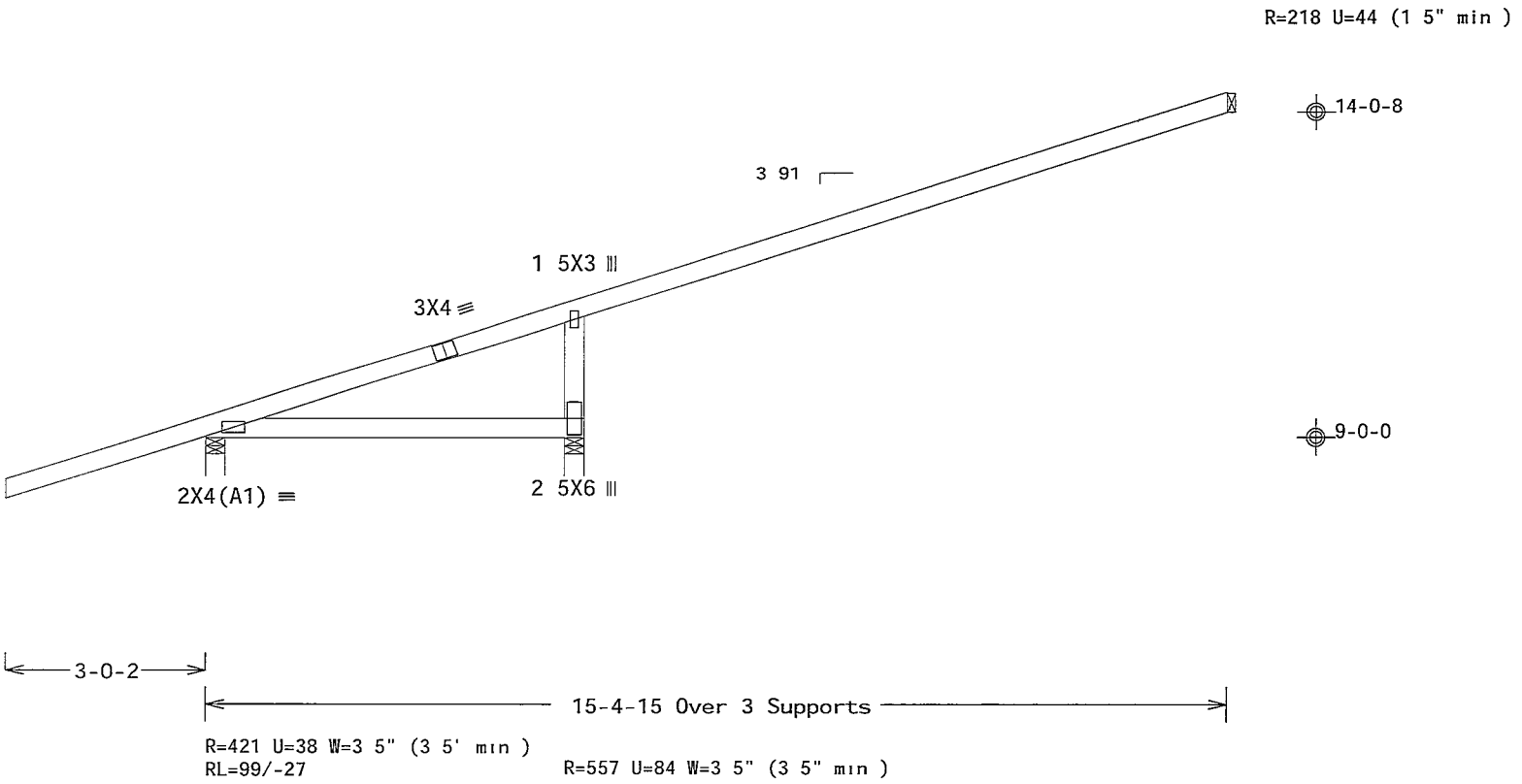
Bottom chord checked for 10 00 psf non-concurrent live load

Top chord overhangs have been checked only for loads as indicated
Overhangs not checked for man loads or long-term deflection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY:10 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2014

TC LL	20.0 PSF	REF	R9114- 67199
TC DL	7.0 PSF	DATE	01/24/14
BC DL	10.0 PSF	DRW	HCUSR9114 14024158
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	26909
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3B487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-011--BRYAN ZECHER /Jim & Karen Lewis Remodel -- Lake City, FL - M1 10'3'8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

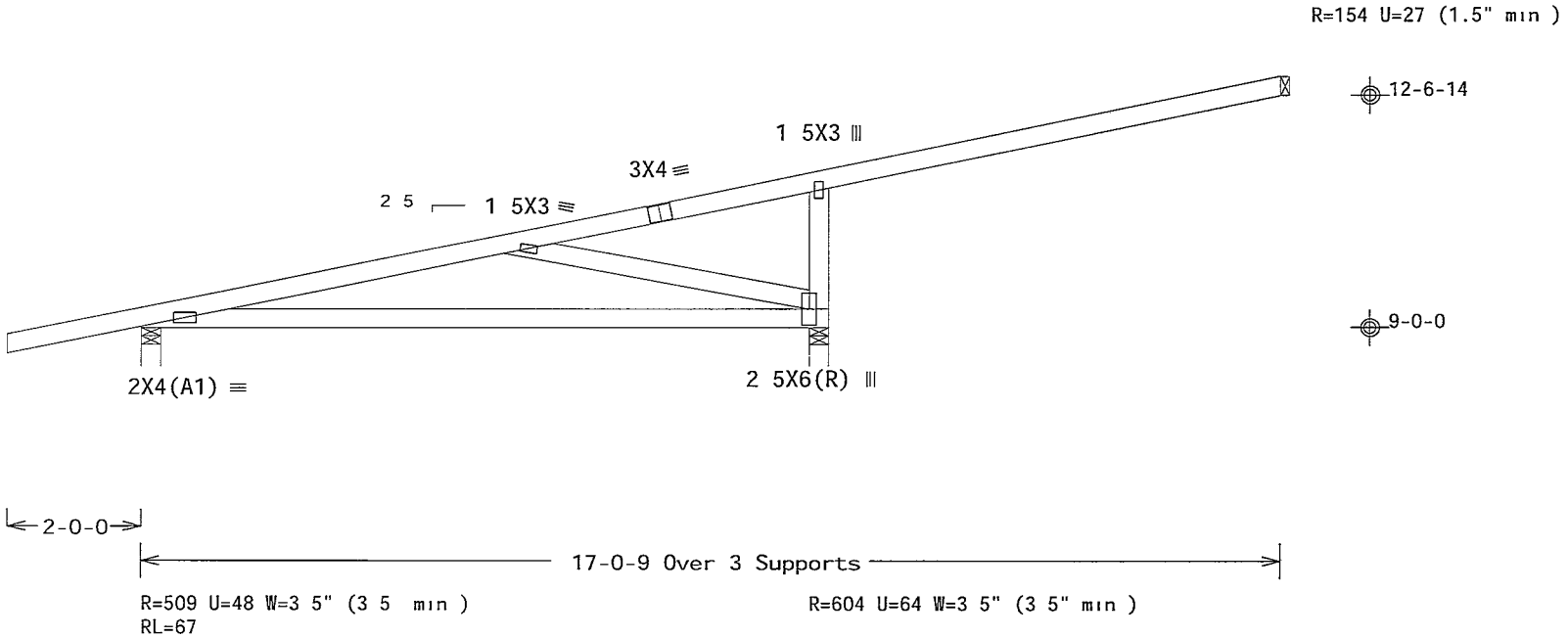
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 0326 13 QTY:10 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

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01/24/2014

TC LL	20.0 PSF	REF R9114- 67200
TC DL	7.0 PSF	DATE 01/24/14
BC DL	10.0 PSF	DRW HCUSR9114 14024159
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT LD	37.0 PSF	SEQN- 26914
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V3B487_Z01

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height, Partially Enclosed, Exposure C Kzt = 1.00
Or 100 mph Wind Speed 15 Mean Height Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
					Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" oc		SPF HF	# / #2	4 10"	8' 2"	8 6"	9 8"	10 1"	11 6"	12' 0"	14 0"	14' 0"	14' 0"	14' 0"
			#3	4 7"	7' 9"	8 3"	9 7"	9 11"	11 5"	11 10"	14 0"	14' 0"	14' 0"	14' 0"
			Stud	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14' 0"	14 0"	14 0"
			Standard	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11' 10"	14 0"	14' 0"	14 0"	14' 0"
		SP DF L	#1	4 11"	8 3"	8 7"	9 9"	10 1"	11 7"	12' 1"	14 0"	14' 0"	14' 0"	14' 0"
			#2	4 10"	8' 2"	8 6"	9 8"	10 1"	11 6"	12' 0"	14 0"	14' 0"	14' 0"	14' 0"
			#3	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11' 10"	14 0"	14' 0"	14 0"	14' 0"
			Stud	4 7"	6 11"	7 4"	9 3"	9 10"	1 5"	11 10"	14 0"	14' 0"	14' 0"	14 0"
			Standard	4 7"	6 0"	6 4"	8 0"	8 6"	10 10"	11 7"	12' 6"	13' 5"	14' 0"	14 0"
16" oc		SPF HF	#1 / #2	5 6"	9' 5"	9 9"	11 1"	11 6"	13' 2"	13 9"	14 0"	14' 0"	14 0"	14' 0"
			#3	5 3"	9 3"	9 9"	10 11"	1 4"	13' 0"	13 7"	14 0"	14' 0"	14 0"	14 0"
			Stud	5 3"	9' 3"	9 7"	10 11"	11 4"	13' 0"	13' 7"	14 0"	14' 0"	14 0"	14' 0"
			Standard	5 3"	9' 3"	9 7"	10 11"	11 4"	13 0"	13 7"	14' 0"	14' 0"	14 0"	14' 0"
		SP DF L	#1	5 8"	9 5"	9 10"	11 2"	11 7"	13 3"	13 10"	14 0"	14' 0"	14' 0"	14' 0"
			#2	5 6"	9 5"	9 9"	11 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5 3"	8' 6"	9 0"	10 11"	11' 4"	13' 0"	13 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5 3"	8 6"	9 0"	10 11"	11 4"	13' 0"	13' 7"	14 0"	14' 0"	14 0"	14 0"
			Standard	5 3"	7 4"	7 10"	9 9"	10 5"	13' 0"	13 7"	14 0"	14' 0"	14' 0"	14' 0"
12" oc		SPF HF	# / #2	6 1"	10 4"	10 8"	12 2"	12 8"	13' 2"	14 0"	14 0"	14' 0"	14' 0"	14' 0"
			#3	5 9"	10 2"	10 7"	12' 0"	12 6"	14 0"	14 0"	14 0"	14' 0"	14' 0"	14 0"
			Stud	5 9"	10 2"	10 7"	12' 0"	12 6"	14 0"	14 0"	14 0"	14' 0"	14 0"	14 0"
			Standard	5 9"	10 2"	10 7"	12' 0"	12' 6"	14 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"
		SP DF L	#1	6 2"	10 5"	10 9"	12' 3"	12' 9"	14 0"	14 0"	14 0"	14' 0"	14 0"	14' 0"
			#2	6 1"	10 4"	10 8"	12 2"	12 8"	14' 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"
			#3	5 9"	9 9"	10 5"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14' 0"	14' 0"
			Stud	5 9"	9 9"	10 5"	12 0"	12' 6"	14 0"	14 0"	14 0"	14 0"	14' 0"	14 0"
			Standard	5 9"	8' 6"	9 0"	11 3"	12' 1"	14 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

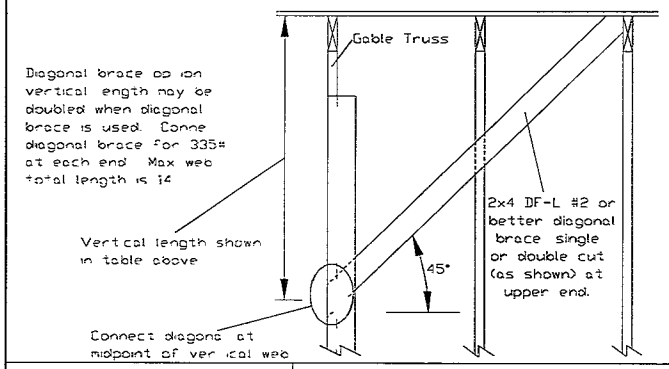
Hem-Fir
#1 & Btr
#1

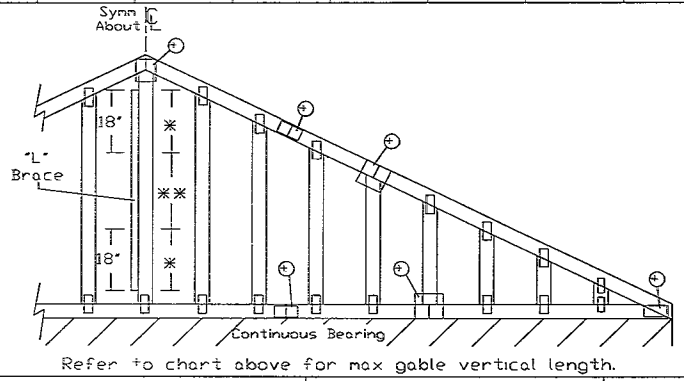
Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.





Refer to chart above for max gable vertical length.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c in 18" end zones and 4" o.c between zones.

** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4 0"	1X4 or 2X3
Greater than 4' 0" but less than 11 6"	2X4
Greater than 11 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail

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Earth City MO 63045

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14

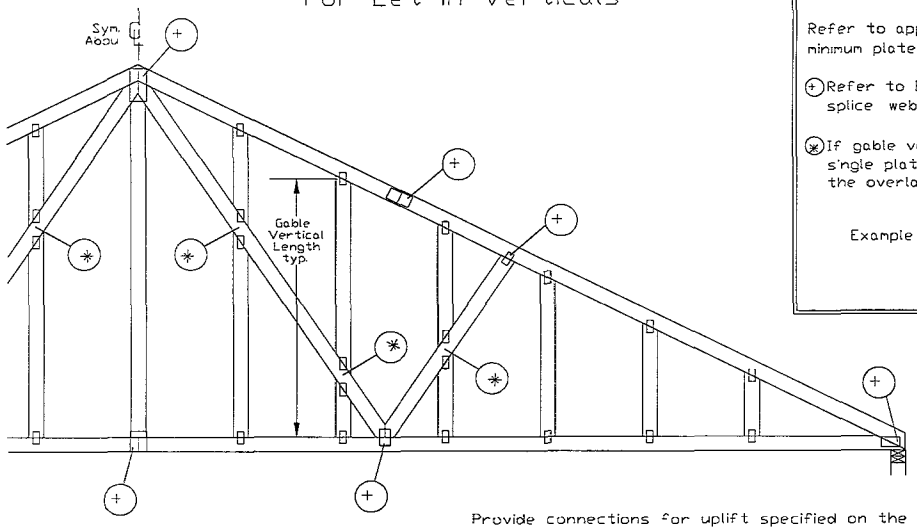
MAX TOT LD 60 PSF

MAX SPACING 24 0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

01/24/2014

Gable Detail
For Let-in Verticals



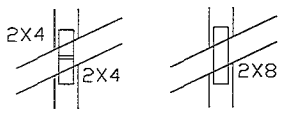
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

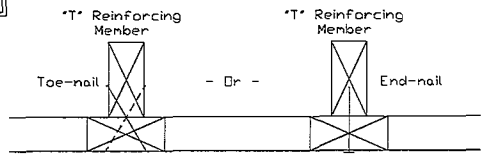
⊕ Refer to Engineered truss design for peak splice web and heel plates

⊛ If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web.

Example



"T" Reinforcement Attachment Detail

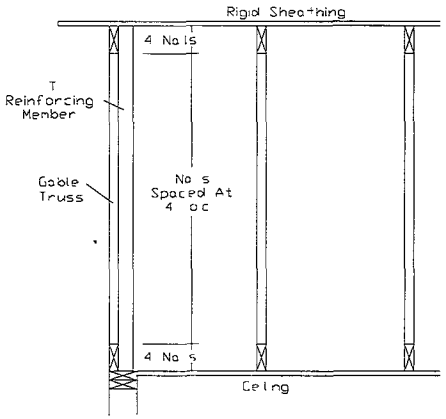


Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3" min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords.



This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212, A20015PED100212,
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord

"T" reinforcing member material must match size specie, and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"oc SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"

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14

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

01/24/2014

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00 Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber plates, and other information not shown on this detail

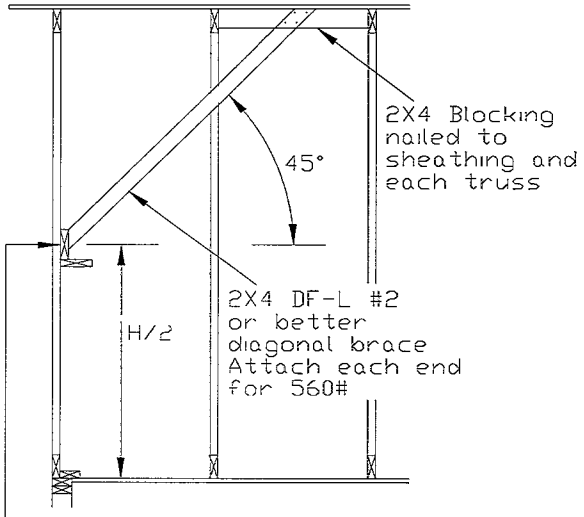
Nails 10d box or gun (0.128"x3",min) nails

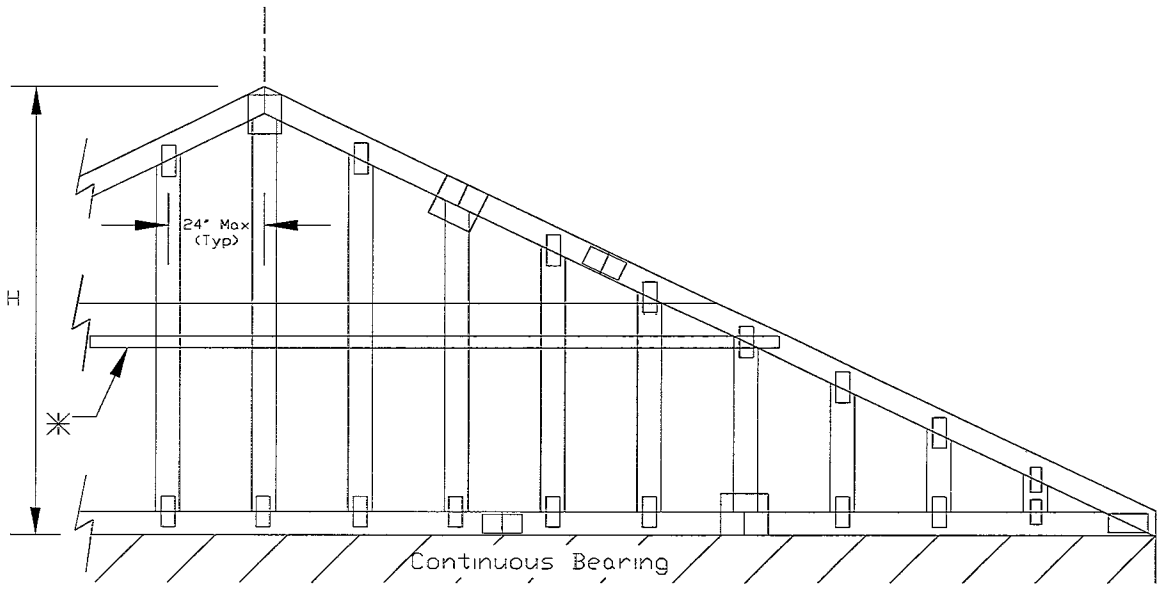
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.





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WALTER P. FINN
LICENSE
No. 22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER

1/24/14

MAX TOT LD 60 PSF

MAX SPACING

REF GE WHALER
DATE 2/14/12
DRWG GABRST100212

01/24/2014

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a t uss design but an alternative web reinforcement method is desired

Notes

This detail s only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecemen or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum a lernative reinforcement, re-run design with appropriate reinforcement type

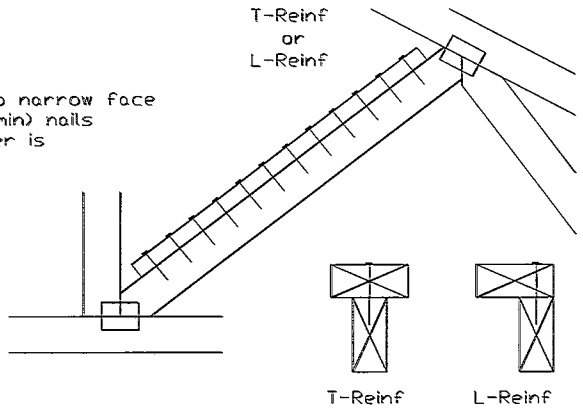
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T reinforcement l reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Enginee s sealed design

(*) Center scab on wide face of web Apply (1) scab to each face of web

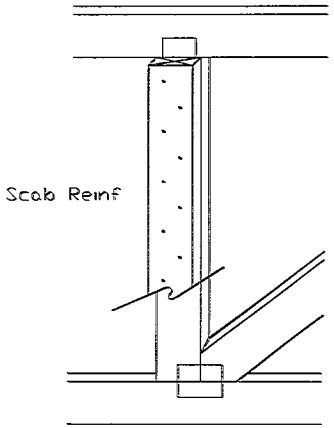
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0128"x3 0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0128"x3 0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Re-er to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.
Refer to drawings 160A-Z for standard plate positions.
ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
A seal on this drawing or cover page listing this drawing, indicates acceptance o- professional engineering responsibility solely for the design shown. The suitability and use o- this drawing or any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
For nore information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org, ICC: www.iccsafe.org

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TOT LD	PSF		
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01/24/2014