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COA #0 278

Florida Certificate of Product Approval #FL 1999
 02/10/2023



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-8875
Job Description: Archbold	
Address: Lot 32, Village on the Green, LAKE CITY, FL 32025	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 22.02.00 JRef #: 1XN32150005
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 40 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	041.23.1238.26590	A01
3	041.23.1238.30450	A03
5	041.23.1239.01170	A05
7	041.23.1239.48520	C02
9	041.23.1239.53310	C04
11	041.23.1239.57860	C06
13	041.23.1240.03253	C08
15	041.23.1240.13633	C10
17	041.23.1240.21890	C12
19	041.23.1240.29127	C14
21	041.23.1240.53027	FT01
23	041.23.1241.08440	G02
25	041.23.1241.34063	J01
27	041.23.1241.38803	J03
29	041.23.1241.44740	J05
31	041.23.1241.50200	PB02
33	041.23.1241.54367	PB04
35	041.23.1242.21860	PB06
37	041.23.1242.29843	PB08
39	041.23.1242.36877	PB10
41	A14030ENC160118	
43	GBLLETIN0118	
45	PB160160118	

Item	Drawing Number	Truss
2	041.23.1238.28410	A02
4	041.23.1238.58590	A04
6	041.23.1239.28643	C01
8	041.23.1239.50933	C03
10	041.23.1239.55850	C05
12	041.23.1240.00787	C07
14	041.23.1240.05427	C09
16	041.23.1240.19087	C11
18	041.23.1240.26033	C13
20	041.23.1240.31163	C15
22	041.23.1241.00670	G01
24	041.23.1241.31420	G03
26	041.23.1241.36587	J02
28	041.23.1241.41610	J04
30	041.23.1241.47910	PB01
32	041.23.1241.52363	PB03
34	041.23.1241.56037	PB05
36	041.23.1242.26473	PB07
38	041.23.1242.34560	PB09
40	041.23.1242.43643	PB11
42	BRCLBSUB0119	
44	A14015ENC160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

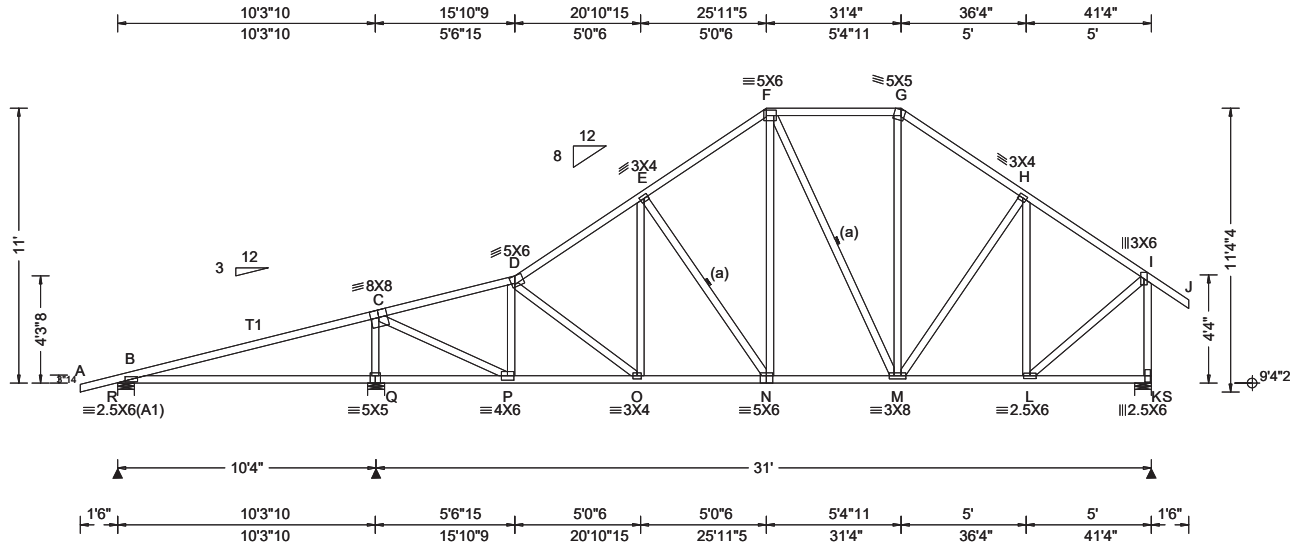
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.13 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.105 B 999 240 VERT(CL): 0.204 B 598 180 HORZ(LL): 0.031 B - - HORZ(TL): 0.060 B - - Creep Factor: 2.0 Max TC CSI: 0.934 Max BC CSI: 0.818 Max Web CSI: 0.704 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity R 458 - / - / 192 / 109 / 327 Q 1984 - / - / 1084 / 244 - S 1540 - / - / 811 / 120 - Wind reactions based on MWFRS R Brg Wid = 8.0 Min Req = 1.5 (Truss) Q Brg Wid = 8.0 Min Req = 2.3 S Brg Wid = 8.0 Min Req = 1.8 (Truss) Bearings R, Q, & S are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber Top chord: 2x4 SP #2; T1 2x4 SP M-31; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 430 -155 F - G 318 -938 C - D 329 -1437 G - H 321 -1213 D - E 362 -1678 H - I 235 -1092 E - F 352 -1359
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Bracing (a) Continuous lateral restraint equally spaced on member.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. Q - C 599 -1736 F - N 616 -202 C - P 1848 -319 H - L 102 -517 P - D 202 -788 L - I 1089 -69 E - N 307 -488 I - K 315 -1507
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Loading Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - Q 22 -387 N - M 1053 0 P - O 1391 -302 M - L 855 -56 O - N 1326 -150
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Purlins In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. Q - C 599 -1736 F - N 616 -202 C - P 1848 -319 H - L 102 -517 P - D 202 -788 L - I 1089 -69 E - N 307 -488 I - K 315 -1507
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Wind Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - Q 22 -387 N - M 1053 0 P - O 1391 -302 M - L 855 -56 O - N 1326 -150
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Additional Notes
The overall height of this truss excluding overhang is 11'-0".

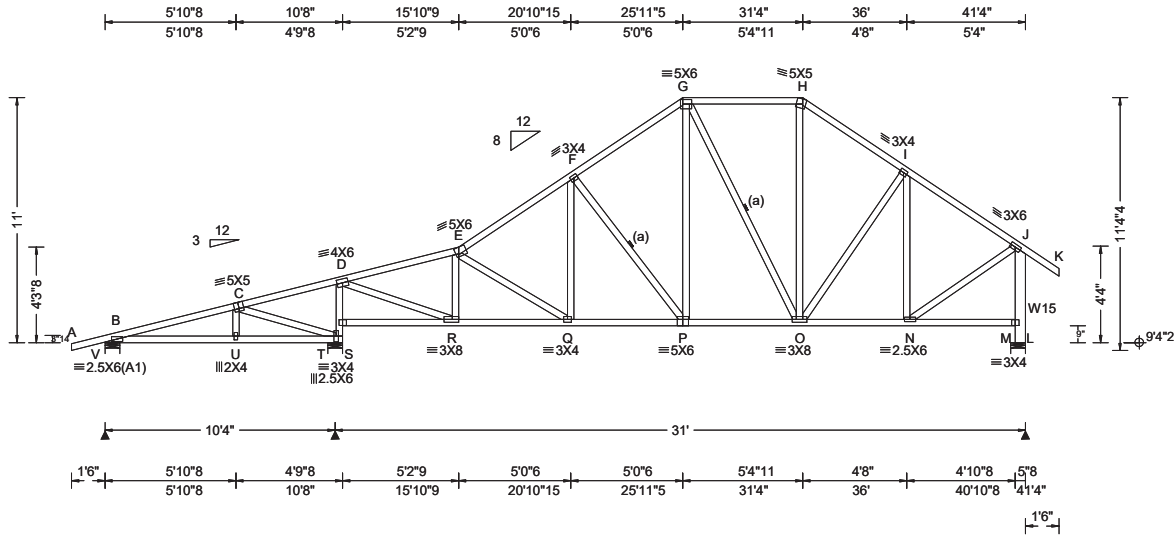
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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.13 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.077 Q 999 240 VERT(CL): 0.150 Q 999 180 HORZ(LL): -0.015 H - - HORZ(TL): 0.028 H - - Creep Factor: 2.0 Max TC CSI: 0.399 Max BC CSI: 0.488 Max Web CSI: 0.793 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>V</td> <td>457</td> <td>-</td> <td>-</td> <td>/192</td> <td>/105</td> <td>/327</td> </tr> <tr> <td>T</td> <td>1915</td> <td>-</td> <td>-</td> <td>/1081</td> <td>-</td> <td>-</td> </tr> <tr> <td>L</td> <td>1470</td> <td>-</td> <td>-</td> <td>/817</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS V Brg Wid = 8.0 Min Req = 1.5 (Truss) T Brg Wid = 8.0 Min Req = 2.3 (Truss) L Brg Wid = 8.0 Min Req = 1.5 (Support) Bearings V, T, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>150 -556</td> <td>F - G</td> <td>225 -1402</td> </tr> <tr> <td>C - D</td> <td>427 -246</td> <td>G - H</td> <td>212 -944</td> </tr> <tr> <td>D - E</td> <td>204 -1784</td> <td>H - I</td> <td>212 -1215</td> </tr> <tr> <td>E - F</td> <td>221 -1825</td> <td>I - J</td> <td>152 -1119</td> </tr> </tbody> </table> </p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	V	457	-	-	/192	/105	/327	T	1915	-	-	/1081	-	-	L	1470	-	-	/817	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	150 -556	F - G	225 -1402	C - D	427 -246	G - H	212 -944	D - E	204 -1784	H - I	212 -1215	E - F	221 -1825	I - J	152 -1119
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W15 2x6 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
 In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

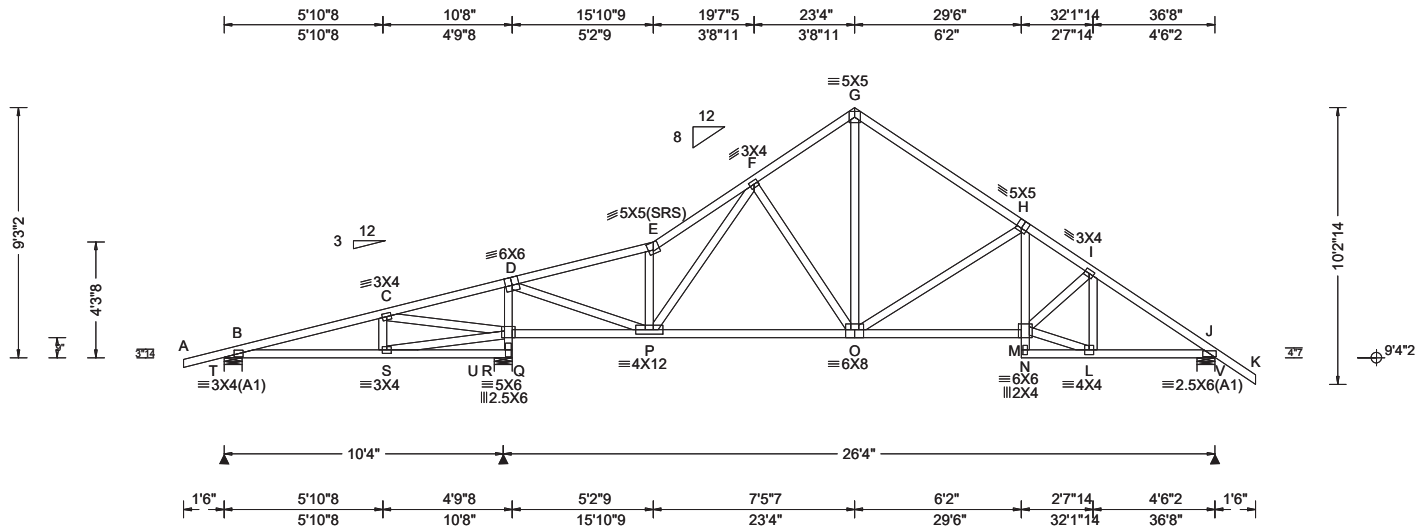
Additional Notes
 The overall height of this truss excluding overhang is 11'-0".



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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.67 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.066 O 999 240 VERT(CL): 0.133 O 999 180 HORZ(LL): 0.023 J - - HORZ(TL): 0.046 J - - Creep Factor: 2.0 Max TC CSI: 0.487 Max BC CSI: 0.685 Max Web CSI: 0.774 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL T 422 - / - / - /179 /107 /284 U 1770 - / - / - /955 /37 - V 1184 - / - / - /741 /13 - Non-Gravity Wind reactions based on MWFRS T Brg Wid = 8.0 Min Req = 1.5 (Truss) U Brg Wid = 8.0 Min Req = 2.1 (Truss) V Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings T, U, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 145 -414 F - G 298 -1080 C - D 891 -165 G - H 280 -1139 D - E 252 -1266 H - I 328 -1751 E - F 400 -1539 I - J 270 -1575
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-3-2.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	210 -779	O - M	1450 -101
P - O	1028 -33	L - J	1233 -112

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - Q	287 -1016	G - O	790 -167
S - Q	438 -139	O - H	215 -704
Q - R	398 -1742	H - M	439 -25
Q - D	363 -1526	M - L	1292 -111
D - P	2031 -289	L - I	84 -462
P - E	284 -792		

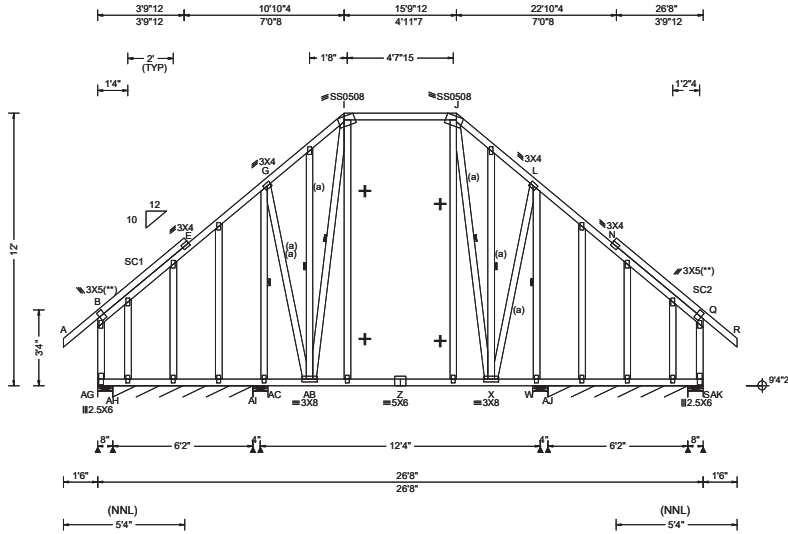


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.063 AA 999 240 VERT(CL): 0.135 AA 999 180 HORZ(LL): 0.224 J - - HORZ(TL): 0.344 J - - Creep Factor: 2.0 Max TC CSI: 0.296 Max BC CSI: 0.464 Max Web CSI: 0.563 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AG 276 /- /- /177 /138 /337 AH*62 /- /- /54 /24 /- AI 612 /- /- /339 /- /- W 612 /- /- /340 /- /- AJ* 62 /- /- /54 /24 /- AK 276 /- /- /174 /138 /- Wind reactions based on MWFRS AG Brg Wid = 8.0 Min Req = 1.5 (Truss) AH Brg Wid = 74.0 Min Req = - AI Brg Wid = 8.0 Min Req = 1.5 (Truss) W Brg Wid = 8.0 Min Req = 1.5 (Truss) AJ Brg Wid = 74.0 Min Req = - AK Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings AG, AH, AI, W, AJ, & AK are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. E - G 408 -100 J - L 517 -216 G - I 517 -215 L - N 408 -101 I - J 427 -115 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G -AB 446 -197 J - X 387 -580 AB - I 386 -580 X - L 446 -198 Maximum Gable Forces Per Ply (lbs) Gables Tens.Comp. Gables Tens. Comp. AC - G 76 -541 L - W 77 -541
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

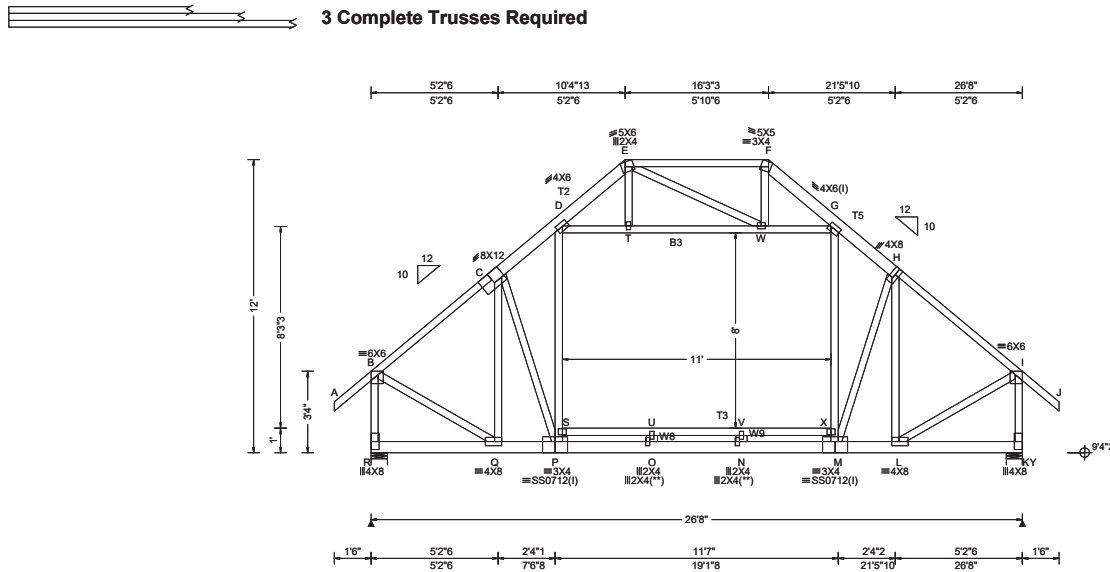
Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 12-0-0.
+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 120.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.84 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.387 U 826 240 VERT(CL): 0.789 U 405 180 HORZ(LL): 0.097 T - - HORZ(TL): 0.234 T - - Creep Factor: 2.0 Max TC CSI: 0.868 Max BC CSI: 0.922 Max Web CSI: 0.979 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL R 10976 /- /- /- /1593 /- Y 10604 /- /- /- /1365 /- Wind reactions based on MWFRS R Brg Wid = 8.0 Min Req = 3.0 (Truss) Y Brg Wid = 8.0 Min Req = 2.9 (Truss) Bearings R & Y are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 441 -3062 F - G 331 -1343 C - D 409 -3479 G - H 346 -3371 D - E 339 -1385 H - I 366 -2939 E - F 224 -972 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. Q - P 2193 -289 N - M 2511 -252 P - O 2511 -252 M - L 2158 -234 O - N 2511 -252 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - R 570 -3563 V - N 19 -662 B - Q 2569 -334 W - F 422 -28 C - Q 133 -1753 W - G 47 -1620 C - P 1392 0 X - G 1190 0 P - S 975 0 X - M 884 0 D - S 1277 0 M - H 1408 -58 D - T 0 -1517 H - L 216 -1914 T - W 0 -1484 L - I 2448 -263 U - O 5 -659 I - K 490 -3433
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Lumber
Top chord: 2x4 SP #2; T2,T5 2x6 SP #2;
T3 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W8,W9 2x6 SP #2;

Nailnote
Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 2.25" o.c.
Bot Chord: 1 Row @ 10.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes
(l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Attic room loading from 7-10-0 to 18-10-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc.

Wind
Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 12-0-0.

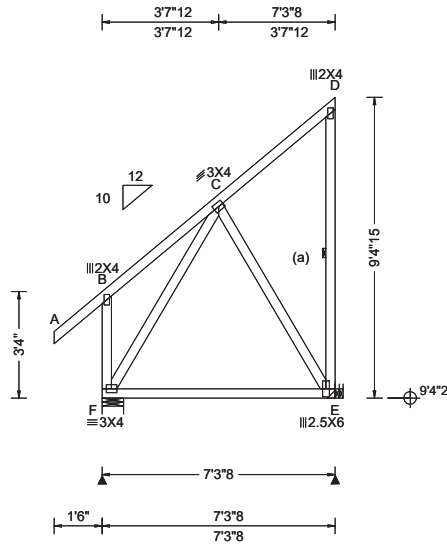


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SEQN: 691050 FROM: CDM	MONO Ply: 1 Qty: 1	Job Number: 23-8875 Archbold Truss Label: C04	Cust: R215 JRef: 1XN32150005 T39 DrwNo: 041.23.1239.53310 KD / DF 02/10/2023
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.09 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.003 C 999 180 HORZ(LL): -0.005 D - - HORZ(TL): 0.006 D - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.605 Max Web CSI: 0.438 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 432 /- /- /249 /- /248 E 303 /- /- /328 /232 /- Wind reactions based on MWFRS F Brg Wid = 8.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. F - C 124 -479 C - E 464 -204
				Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Bracing (a) Continuous lateral restraint equally spaced on member. Hangers / Ties Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information. Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage. Bearing at location x=7'0"8 uses the following support conditions: 7'0"8 Bearing E (7'0"8, 9'4"2) LUS26 Supporting Member: (2)2x6 SP 2400f-2.0E (4) 0.148"x3" nails into supporting member, (3) 0.148"x3" nails into supported member.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

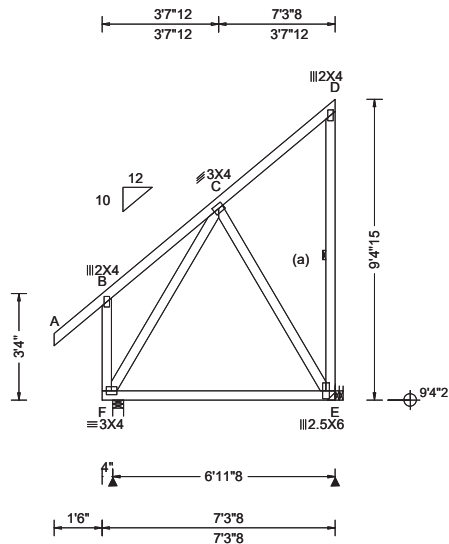
Additional Notes
 The overall height of this truss excluding overhang is 9-4-15.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.09 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.012 B 513 240 VERT(CL): -0.029 F 999 180 HORZ(LL): -0.014 D - - HORZ(TL): 0.036 D - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.460 Max Web CSI: 0.442 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 463 /- /- /279 /- /248 E 271 /- /- /311 /241 /- Wind reactions based on MWFRS F Brg Wid = 4.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. F - C 118 -483 C - E 465 -201

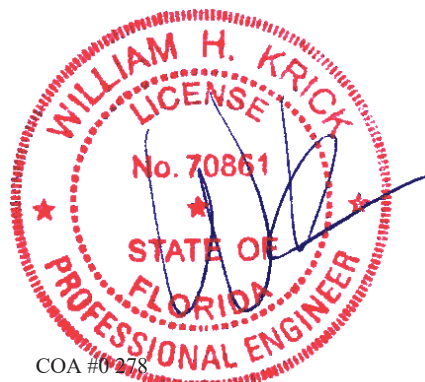
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=7'0"8 uses the following support conditions: 7'0"8
Bearing E (7'0"8, 9'4"2) LUS26
Supporting Member: (2)2x6 SP 2400F-2.0E
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

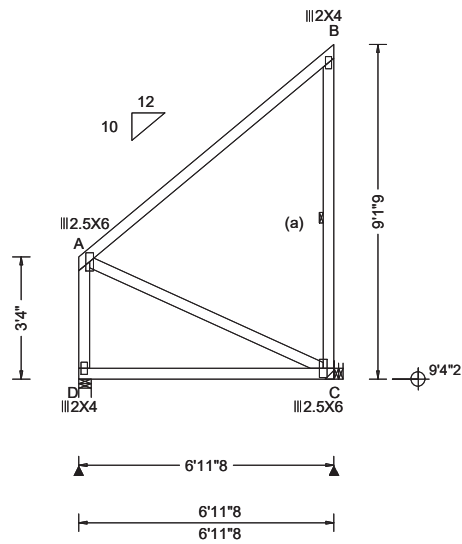
Additional Notes
The overall height of this truss excluding overhang is 9-4-15.



COA #0278
02/10/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.58 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): -0.002 B 999 240 VERT(CL): 0.003 B 999 180 HORZ(LL): -0.007 B - - HORZ(TL): 0.009 B - - Creep Factor: 2.0 Max TC CSI: 0.440 Max BC CSI: 0.549 Max Web CSI: 0.361 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 299 /- /- /159 /- /203 C 299 /- /- /309 /216 /- Wind reactions based on MWFRS D Brg Wid = 4.0 Min Req = 1.5 (Truss) C Brg Wid = - Min Req = - Bearing D is a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. D - C 131 -416
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Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=6'8"8 uses the following support conditions: 6'8"8
Bearing C (6'8"8, 9'4"2) LUS26
Supporting Member: (2)2x6 SP 2400F-2.0E
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

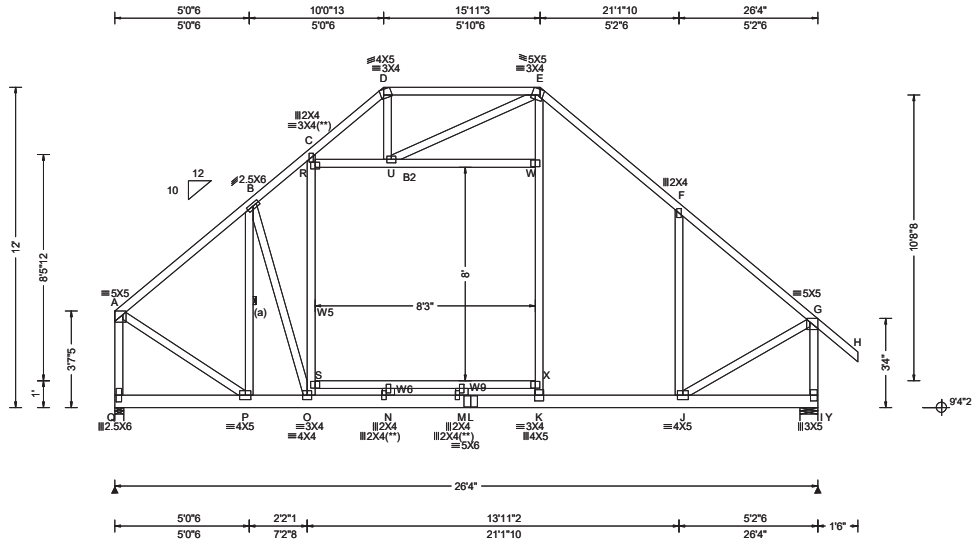
Additional Notes
The overall height of this truss excluding overhang is 9-1-9.

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Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp.
A - C 453 -143

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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.84 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.230 T 999 240 VERT(CL): 0.540 T 584 180 HORZ(LL): 0.249 F - - HORZ(TL): 0.584 F - - Creep Factor: 2.0 Max TC CSI: 0.539 Max BC CSI: 0.464 Max Web CSI: 0.992 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Q 2039 - / - / - / 686 - / / 306 Y 2241 - / - / - / 803 - / - Wind reactions based on MWFRS Q Brg Wid = 4.0 Min Req = 1.7 (Truss) Y Brg Wid = 8.0 Min Req = 1.9 (Truss) Bearings Q & Y are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 97 - 1715 D - E 107 - 910 B - C 178 - 1938 E - F 217 - 1973 C - D 82 - 1150 F - G 6 - 1940 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. P - O 1272 - 141 M - L 1339 0 O - N 1339 0 L - K 1339 0 N - M 1339 0 K - J 1421 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - Q 87 - 1990 U - E 143 - 561 A - P 1476 0 W - X 1044 - 140 P - B 185 - 970 W - E 1083 - 138 B - O 560 - 408 X - K 907 - 161 C - R 449 - 25 F - J 258 - 399 R - S 446 - 26 J - G 1632 0 R - U 142 - 563 G - I 26 - 2252

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E; B2 2x4 SP #2;
 Webs: 2x4 SP #3; W5 2x4 SP #2; W6,W9 2x6 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 (**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.
 Attic room loading from 7-6-0 to 15-9-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF
 Truss supports 400# mech unit; unit centered at 18-6-0; supported by BC; unit width 4-0-0; supported by 2 trusses.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 12-0-0.



COA #0278

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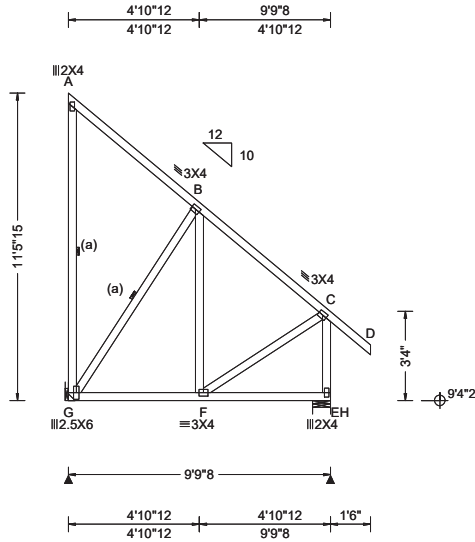
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SEQN: 690960 FROM: CDM	MONO Ply: 1 Qty: 1	Job Number: 23-8875 Archbold Truss Label: C09	Cust: R215 JRef: 1XN32150005 T33 DrwNo: 041.23.1240.05427 KD / DF 02/10/2023
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.13 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.003 F 999 240 VERT(CL): 0.006 F 999 180 HORZ(LL): 0.004 A - - HORZ(TL): 0.006 A - - Creep Factor: 2.0 Max TC CSI: 0.429 Max BC CSI: 0.299 Max Web CSI: 0.135 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 413 /- /- /418 /177 /245 H 536 /- /- /271 /- /- Wind reactions based on MWFRS G Brg Wid = - Min Req = - H Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - B 453 -331 C - E 0 -499
				Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Bracing (a) Continuous lateral restraint equally spaced on member. Hangers / Ties Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information. Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage. Bearing at location x=0' uses the following support conditions: 0' Bearing G (0', 9'4"2) LUS26 Supporting Member: (1)2x6 SP 2400f-2.0E (4) 0.148"x3" nails into supporting member, (3) 0.148"x3" nails into supported member.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 11'-5-1/2".



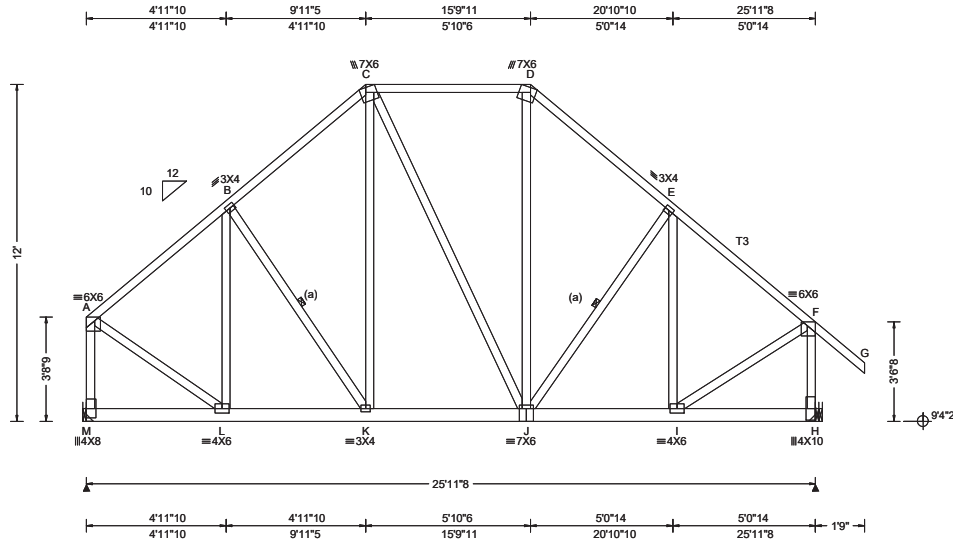
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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 60.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 11.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.047 J 999 240 VERT(CL): 0.099 J 999 180 HORZ(LL): 0.017 B - - HORZ(TL): 0.036 B - - Creep Factor: 2.0 Max TC CSI: 0.951 Max BC CSI: 0.165 Max Web CSI: 0.899 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 2909 /- /- /- /559 /- H 3332 /- /- /- /713 /- Wind reactions based on MWFRS M Brg Wid = - Min Req = - H Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 494 -2340 D - E 550 -2468 B - C 517 -2372 E - F 539 -2492 C - D 341 -1695
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Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint, equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 165 plf at 0.00 to 165 plf at 27.71
BC: From 50 plf at 0.00 to 50 plf at 25.96
BC: From 13 plf at 25.96 to 13 plf at 27.71
BC: 346 lb Conc. Load at 16.35

Purlins

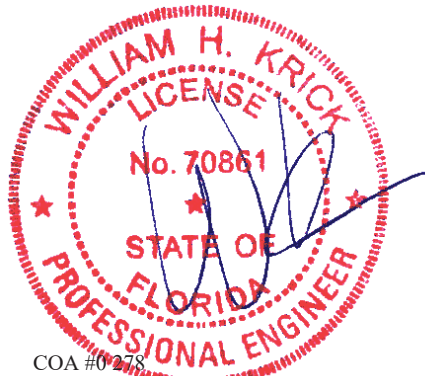
In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 12-0-0.



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SEQN: 691107	COMN	Ply: 1	Job Number: 23-8875	Cust: R215 JRef: 1XN32150005 T38
FROM: CDM		Qty: 1	Archbold	DrwNo: 041.23.1240.13633
Page 2 of 2			Truss Label: C10	KD / DF 02/10/2023

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=25'8"8 uses the following support conditions: 25'8"8

Bearing H (25'8"8, 9'4"2) HGUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(20) 0.148"x3" nails into supporting member,

(8) 0.148"x3" nails into supported member.



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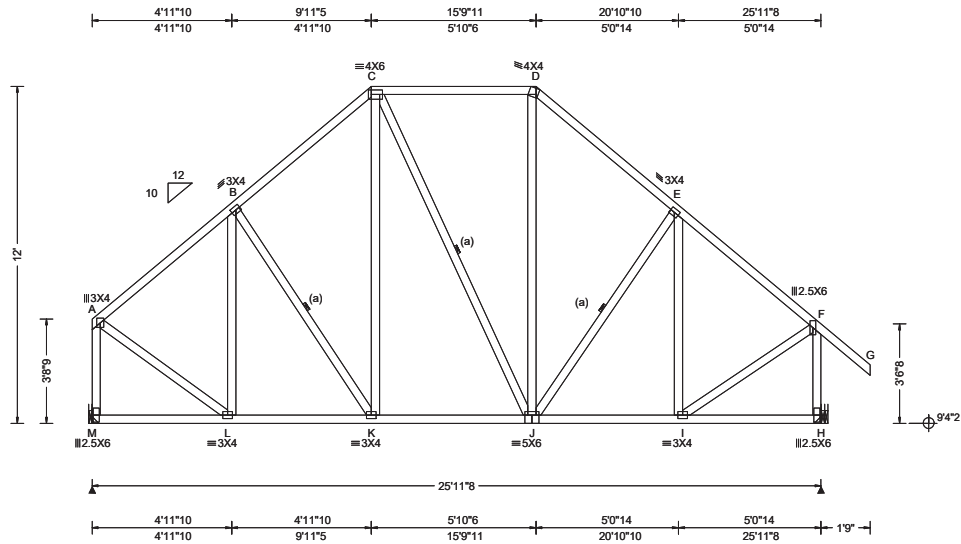
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.020 K 999 240 VERT(CL): 0.042 K 999 180 HORZ(LL): 0.009 B - - HORZ(TL): 0.019 B - - Creep Factor: 2.0 Max TC CSI: 0.354 Max BC CSI: 0.299 Max Web CSI: 0.354 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1112 /- /- /623 /19 /307 H 1246 /- /- /738 /19 /- Wind reactions based on MWFRS M Brg Wid = - Min Req = - H Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 302 -886 D - E 458 -889 B - C 457 -888 E - F 337 -911 C - D 418 -603
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

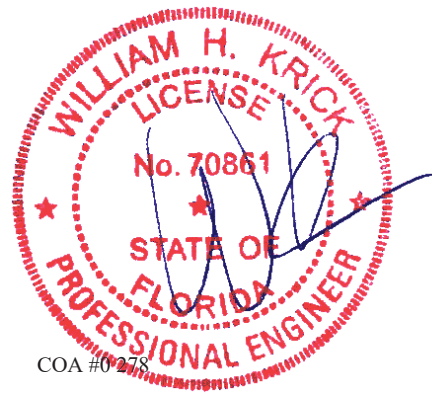
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 12-0-0.



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Hangers / Ties

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Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing M (0', 9'4"2) HUS26
 Supporting Member: (1)2x6 SP #2
 (14) 0.148"x3" nails into supporting member,
 (4) 0.148"x3" nails into supported member.

Bearing H (25'8"8, 9'4"2) HUS26
 Supporting Member: (2)2x6 SP 2400f-2.0E
 (14) 0.148"x3" nails into supporting member,
 (4) 0.148"x3" nails into supported member.



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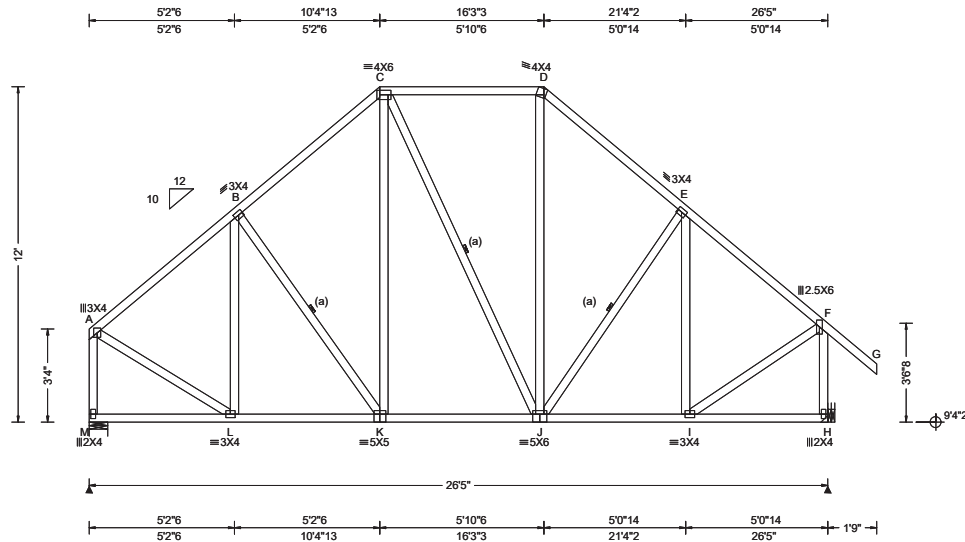
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.022 K 999 240 VERT(CL): 0.046 K 999 180 HORZ(LL): 0.010 B - - HORZ(TL): 0.021 B - - Creep Factor: 2.0 Max TC CSI: 0.362 Max BC CSI: 0.310 Max Web CSI: 0.325 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1132 /- /- /654 /- /317 H 1264 /- /- /762 /- /- Wind reactions based on MWFRS M Brg Wid = 8.0 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing M is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 119 -961 D - E 217 -911 B - C 219 -929 E - F 149 -927 C - D 221 -619 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. L - K 674 -171 J - I 639 0 K - J 625 -68 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - M 110 -1089 I - F 748 0 A - L 761 -3 F - H 182 -1223
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=26'2" uses the following support conditions: 26'2"
Bearing H (26'2", 9'4"2) HUS26
Supporting Member: (2)2x6 SP 2400F-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

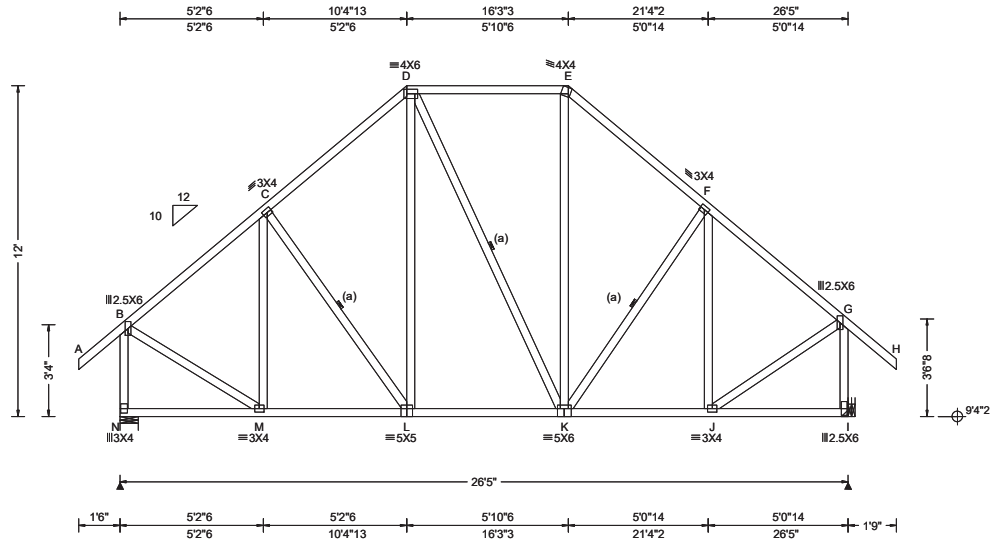
Additional Notes
The overall height of this truss excluding overhang is 12-0-0.



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02/10/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.030 L 999 240 VERT(CL): 0.056 L 999 180 HORZ(LL): 0.014 C - - HORZ(TL): 0.025 C - - Creep Factor: 2.0 Max TC CSI: 0.349 Max BC CSI: 0.403 Max Web CSI: 0.341 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL N 1410 /- /- /747 /81 /342 I 1422 /- /- /760 /84 /- Wind reactions based on MWFRS N Brg Wid = 8.0 Min Req = 1.7 (Truss) I Brg Wid = - Min Req = - Bearing N is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 147 -1115 E - F 222 -1056 C - D 224 -1084 F - G 152 -1070 D - E 224 -732 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. M - L 784 -176 K - J 749 0 L - K 746 -71 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - N 176 -1372 J - G 879 0 B - M 895 0 G - I 186 -1386
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

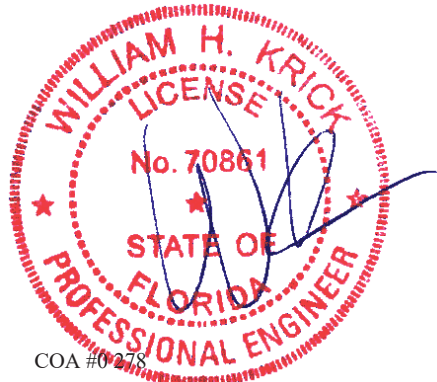
Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=26'2" uses the following support conditions: 26'2"
Bearing I (26'2", 9'4"2) HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0218
02/10/2023
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SEQN: 691094	COMN	Ply: 1	Job Number: 23-8875	Cust: R215 JRef: 1XN32150005 T20
FROM: CDM		Qty: 5	Archbold	DrwNo: 041.23.1240.26033
Page 2 of 2			Truss Label: C13	KD / DF 02/10/2023

Additional Notes

The overall height of this truss excluding overhang is 12-0-0.



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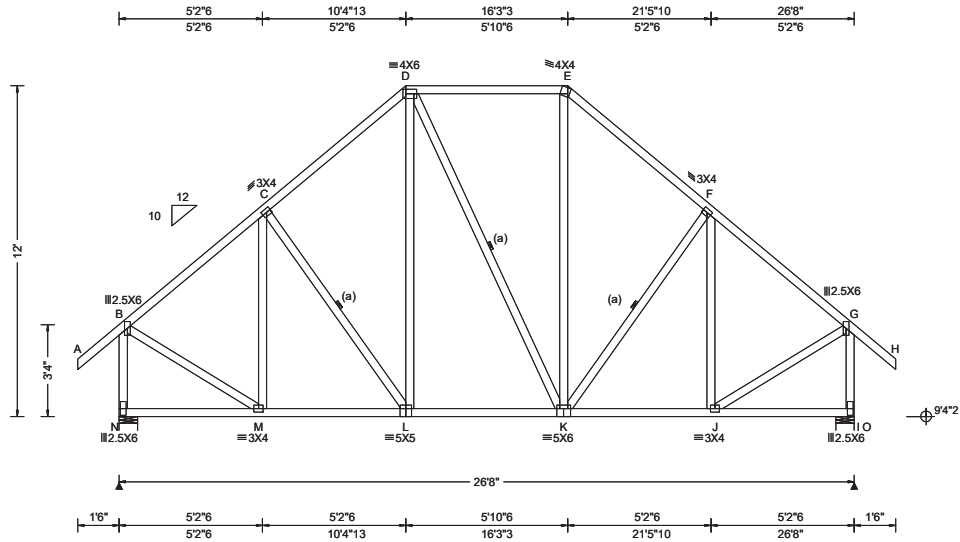
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 12-0-0.



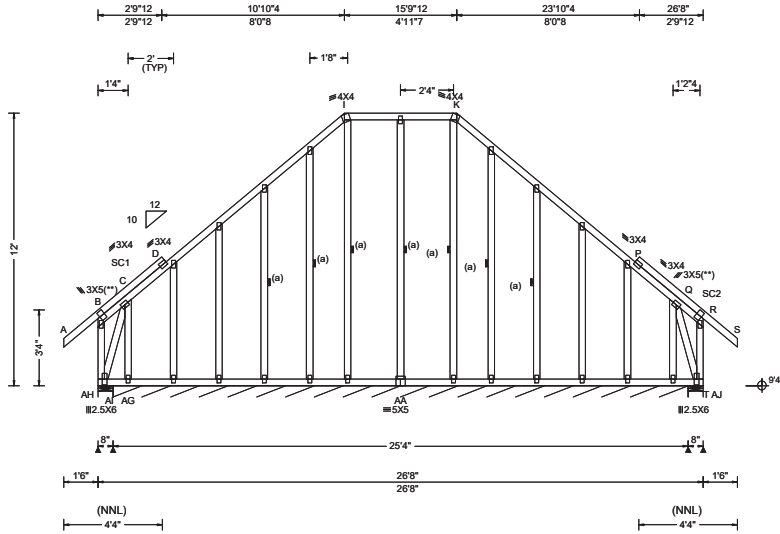
COA #0278

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SEQN: 690925 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 23-8875 Archbold Truss Label: C15	Cust: R215 JRef: 1XN32150005 T17 DrwNo: 041.23.1240.31163 KD / DF 02/10/2023
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 K 999 240 VERT(CL): 0.004 J 999 180 HORZ(LL): 0.011 M - - HORZ(TL): 0.016 M - - Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.040 Max Web CSI: 0.170 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AH 278</td> <td>-</td> <td>-</td> <td>-</td> <td>1570</td> <td>1350</td> <td>1337</td> </tr> <tr> <td>AI* 77</td> <td>-</td> <td>-</td> <td>-</td> <td>149</td> <td>125</td> <td>-</td> </tr> <tr> <td>AJ 278</td> <td>-</td> <td>-</td> <td>-</td> <td>1439</td> <td>1219</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS Bearings AH, AI, & AJ are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>D - I</td> <td>451 -191</td> <td>K - P</td> <td>453 -129</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AH 278	-	-	-	1570	1350	1337	AI* 77	-	-	-	149	125	-	AJ 278	-	-	-	1439	1219	-	Chords	Tens.Comp.	Chords	Tens. Comp.	D - I	451 -191	K - P	453 -129
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D - I	451 -191	K - P	453 -129																																											

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 12-0-0.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
AH- C	565 -609	Q - T	427 -472

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.
C -AG	407 -380

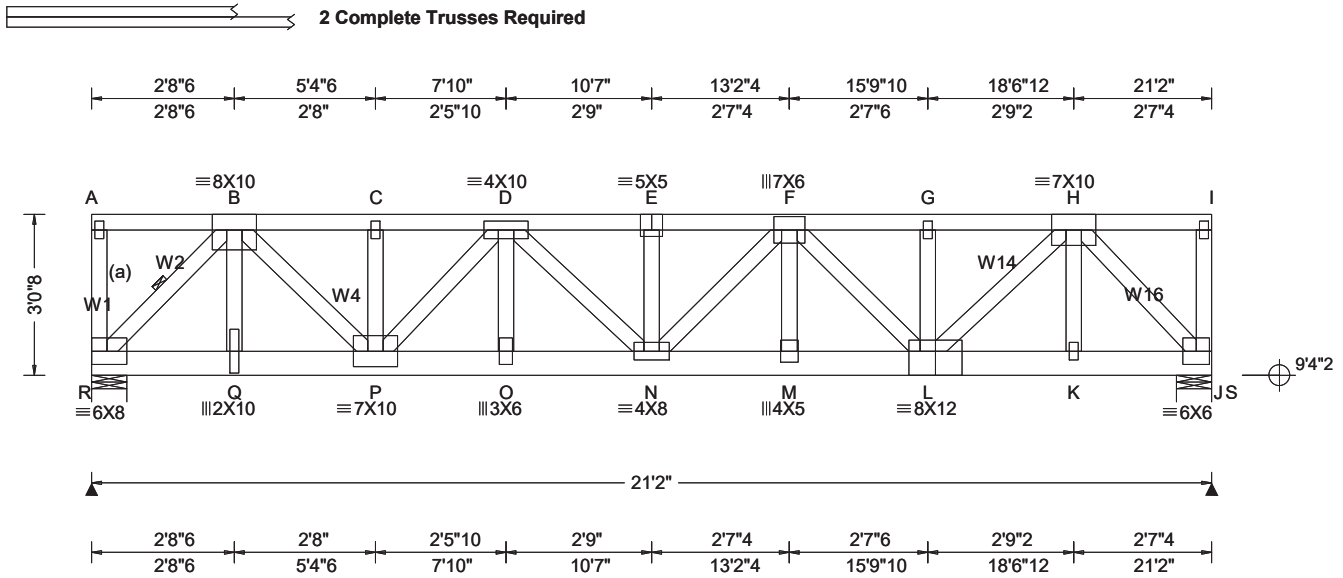


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Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W1,W2,W4,W16 2x4 SP #2;
W14 2x4 SP M-31;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Nailnote
Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.00 to 60 plf at 21.17
BC: From 10 plf at 0.00 to 10 plf at 16.77
BC: From 20 plf at 16.77 to 20 plf at 21.17
BC: 3332 lb Conc. Load at 0.77
BC: 1246 lb Conc. Load at 2.77, 4.77
BC: 1264 lb Conc. Load at 6.77, 8.77
BC: 1422 lb Conc. Load at 10.77,12.77,14.77,16.77

Plating Notes
All plates are 2X4 except as noted.

Purlins
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind
Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.

Additional Notes
Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 3-0-8.
THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	3657 -245	N - M	7101 -440
Q - P	3657 -245	M - L	7101 -440
P - O	7143 -370	L - K	2872 -193
O - N	7143 -370	K - J	2872 -193

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
R - B	343 -5123	N - F	754 0
B - Q	1321 -139	F - M	911 -52
B - P	3105 -92	F - L	113 -2163
P - D	87 -1902	L - H	3776 -234
D - O	826 0	H - J	276 -4103
D - N	675 -90		



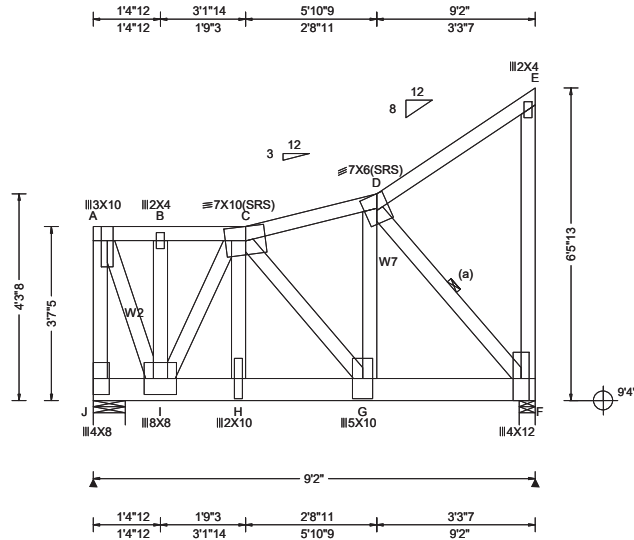
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.040 H 999 240 VERT(CL): 0.079 H 999 180 HORZ(LL): -0.019 E - - HORZ(TL): 0.038 E - - Creep Factor: 2.0 Max TC CSI: 0.203 Max BC CSI: 0.813 Max Web CSI: 0.735 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 3173 -/- /- /- /344 -/ F 2657 -/- /- /- /411 -/ Wind reactions based on MWFRS J Brg Wid = 8.0 Min Req = 3.7 (Truss) F Brg Wid = 4.0 Min Req = 3.1 (Truss) Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 138 - 1293 C - D 382 - 2465 B - C 138 - 1293 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. I - H 2413 - 300 G - F 2162 - 334 H - G 2353 - 297 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - J 335 - 3042 H - C 1325 - 66 A - I 3255 - 347 G - D 2932 - 403 I - C 337 - 2337 D - F 518 - 3355
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #3; W2,W7 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 61 plf at 0.00 to 61 plf at 5.88
TC: From 64 plf at 5.88 to 64 plf at 9.17
BC: From 10 plf at 0.00 to 10 plf at 5.40
BC: From 20 plf at 5.40 to 20 plf at 9.17
BC: 1112 lb Conc. Load at 1.40, 3.40
BC: 2909 lb Conc. Load at 5.40

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 6-5-13.

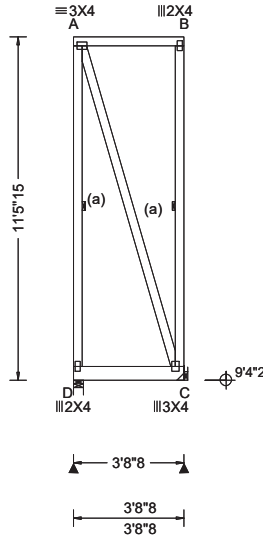


COA #0278

Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 B 999 240	D	364	/-	/-	/-	/136	/-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 B 999 180	C	346	/-	/-	/-	/128	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.003 A - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.007 A - -	D Brg Wid = 4.0 Min Req = 1.5 (Truss)						
NCBCLL: 10.00	Mean Height: 20.84 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	C Brg Wid = - Min Req = -						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.268	Bearing D is a rigid surface.						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Varies by Ld Case	Max BC CSI: 0.187	Members not listed have forces less than 375#						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.070							
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 22.02.00.0914.12							
	GCp: 0.18	WAVE								
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.00 to 60 plf at 3.71
BC: From 20 plf at 0.00 to 20 plf at 3.71
BC: 413 lb Conc. Load at 1.77

Purlins

The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.

Additional Notes

Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 11-5-15.



COA #0278

02/10/2023
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SEQN: 691104	FLAT	Ply: 1	Job Number: 23-8875	Cust: R215 JRef: 1XN32150005 T37
FROM: CDM		Qty: 1	Archbold	DrwNo: 041.23.1241.08440
Page 2 of 2			Truss Label: G02	KD / DF 02/10/2023

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

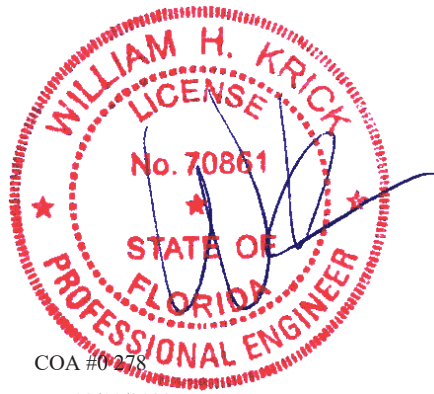
Bearing at location $x=3'5"8$, $y=9'4"2$ uses the following support conditions: 3'5"8

Bearing C (3'5"8, 9'4"2) LUS26

Supporting Member: (1)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting member,

(3) 0.148"x3" nails into supported member.



COA #0278

Florida Certificate of Product Approval #FL 1999

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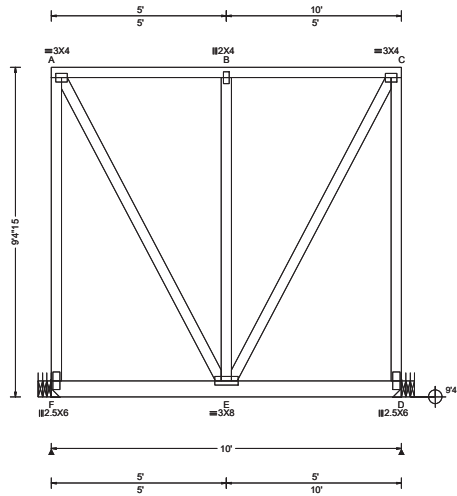
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.75 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 12.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): -0.008 B 999 240 VERT(CL): 0.013 B 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.115 Max BC CSI: 0.095 Max Web CSI: 0.218 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 786 /- /- /- /495 /- D 787 /- /- /- /513 /- Wind reactions based on MWFRS F Brg Wid = - Min Req = - D Brg Wid = - Min Req = - Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 30 plf at 0.00 to 30 plf at 10.00
BC: From 10 plf at 0.00 to 10 plf at 10.00
BC: 299 lb Conc. Load at 2.02, 4.02
BC: 271 lb Conc. Load at 6.02
BC: 303 lb Conc. Load at 8.02

Purlins

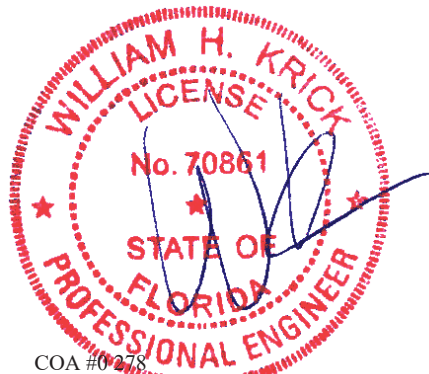
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.

Additional Notes

Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 9-4-15.



COA #0218

02/10/2023 Florida Certificate of Product Approval #FL 1999

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SEQN: 691121	FLAT	Ply: 2	Job Number: 23-8875	Cust: R215	JRef: 1XN32150005	T45
FROM: CDM		Qty: 1	Archbold	DrwNo: 041.23.1241.31420		
Page 2 of 2			Truss Label: G03	KD / DF		02/10/2023

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing F (0', 9'4"2) LUS26-2

Supporting Member: (3)2x6 SP 2400f-2.0E

(4) 0.162"x3.5" nails into supporting member,

(3) 0.162"x3.5" nails into supported member.

Bearing D (9'9", 9'4"2) LUS26-2

Supporting Member: (3)2x6 SP 2400f-2.0E

(4) 0.162"x3.5" nails into supporting member,

(3) 0.162"x3.5" nails into supported member.



COA #0218

02/10/2023 Florida Certificate of Product Approval #FL 1999

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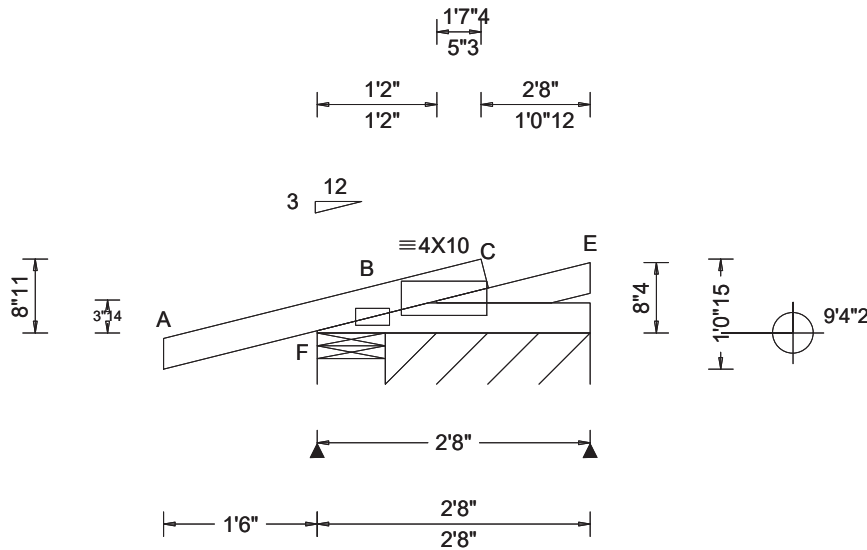
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.004 E 999 240 VERT(CL): 0.007 E 999 180 HORZ(LL): 0.001 E - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.250 Max BC CSI: 0.027 Max Web CSI: 0.000 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL F 203 /- /- /125 /97 /36 E* 55 /- /- /36 /12 /- Non-Gravity Wind reactions based on MWFRS F Brg Wid = 8.0 Min Req = 1.5 (Truss) E Brg Wid = 24.0 Min Req = - Bearings F & B are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Plating Notes

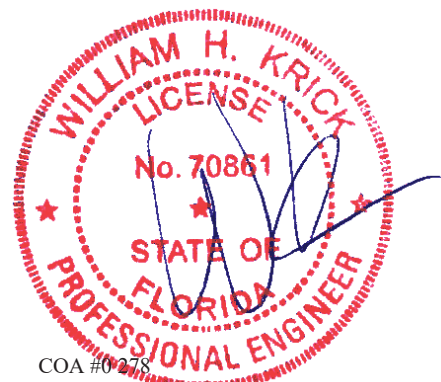
All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

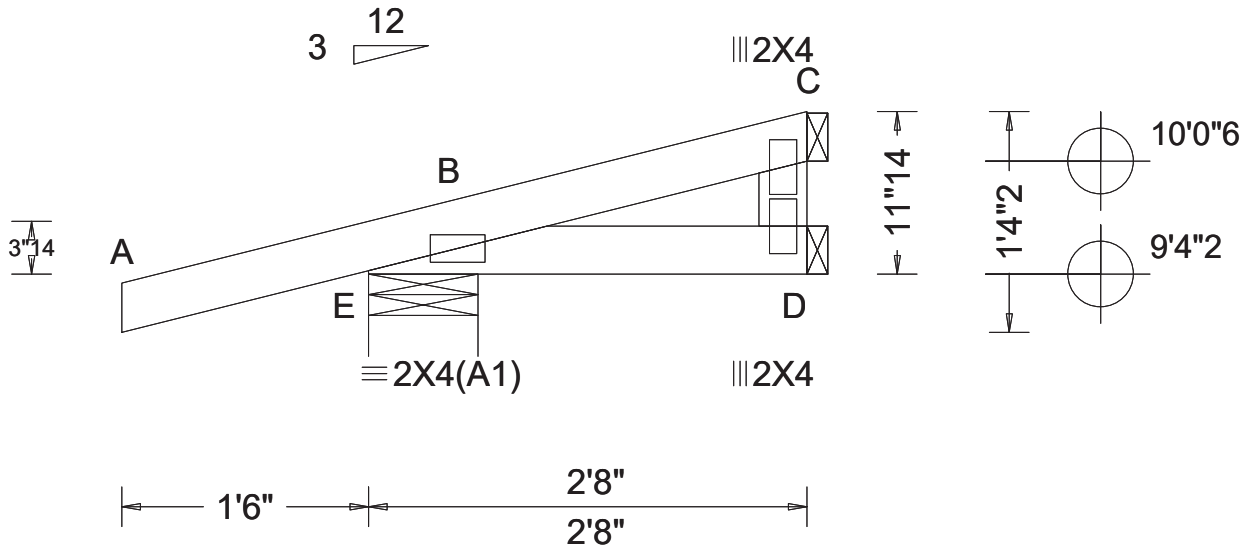
The overall height of this truss excluding overhang is 0-8-11.



COA #0218
02/10/2023
Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.252 Max BC CSI: 0.039 Max Web CSI: 0.008 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>245</td> <td>/-</td> <td>/-</td> <td>/141</td> <td>/89</td> <td>/38</td> </tr> <tr> <td>D</td> <td>39</td> <td>/-</td> <td>/-</td> <td>/22</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>47</td> <td>/-</td> <td>/-</td> <td>/22</td> <td>/19</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	245	/-	/-	/141	/89	/38	D	39	/-	/-	/22	/-	/-	C	47	/-	/-	/22	/19	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
E	245	/-	/-	/141	/89	/38																																
D	39	/-	/-	/22	/-	/-																																
C	47	/-	/-	/22	/19	/-																																
Wind reactions based on MWFRS E Brg Wid = 8.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#																																						

Lumber

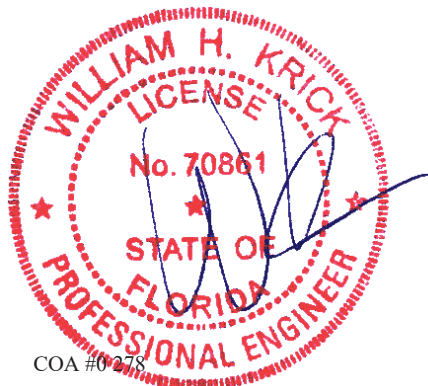
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 0-11-14.

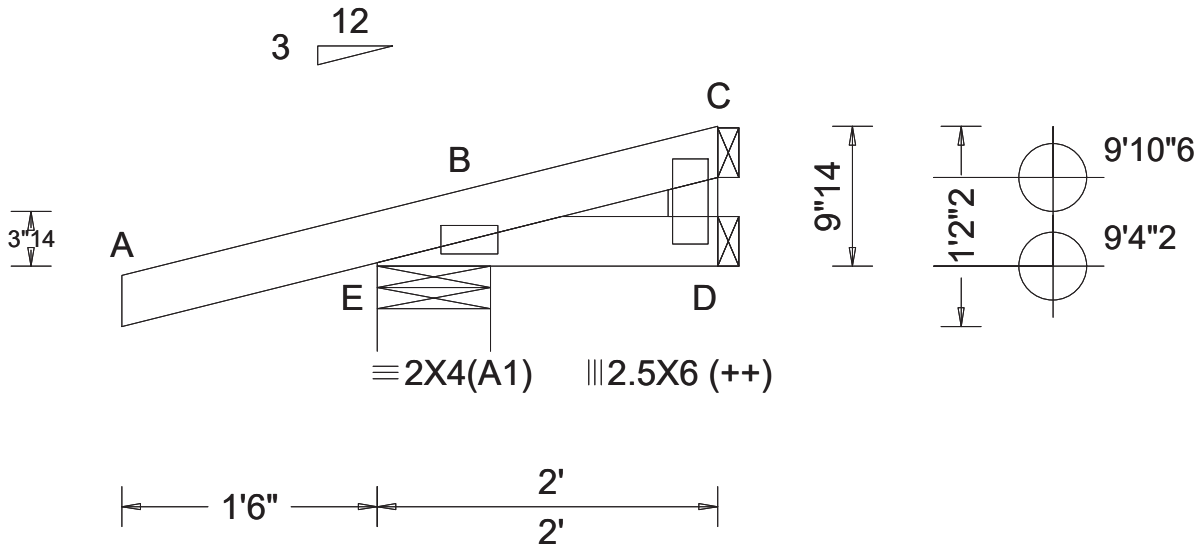


COA #0218

Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.253 Max BC CSI: 0.040 Max Web CSI: 0.005 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>231</td> <td>/-</td> <td>/-</td> <td>/135</td> <td>/94</td> <td>/33</td> </tr> <tr> <td>D</td> <td>24</td> <td>/-</td> <td>/-</td> <td>/19</td> <td>/5</td> <td>/-</td> </tr> <tr> <td>C</td> <td>22</td> <td>/-</td> <td>/-</td> <td>/14</td> <td>/10</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	231	/-	/-	/135	/94	/33	D	24	/-	/-	/19	/5	/-	C	22	/-	/-	/14	/10	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
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C	22	/-	/-	/14	/10	/-																																
Wind reactions based on MWFRS E Brg Wid = 8.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#																																						

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

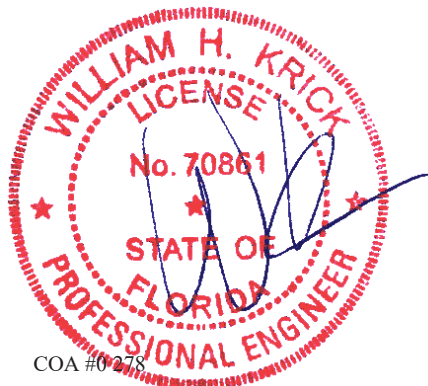
(++) - This plate works for both joints covered.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 0-9-14.



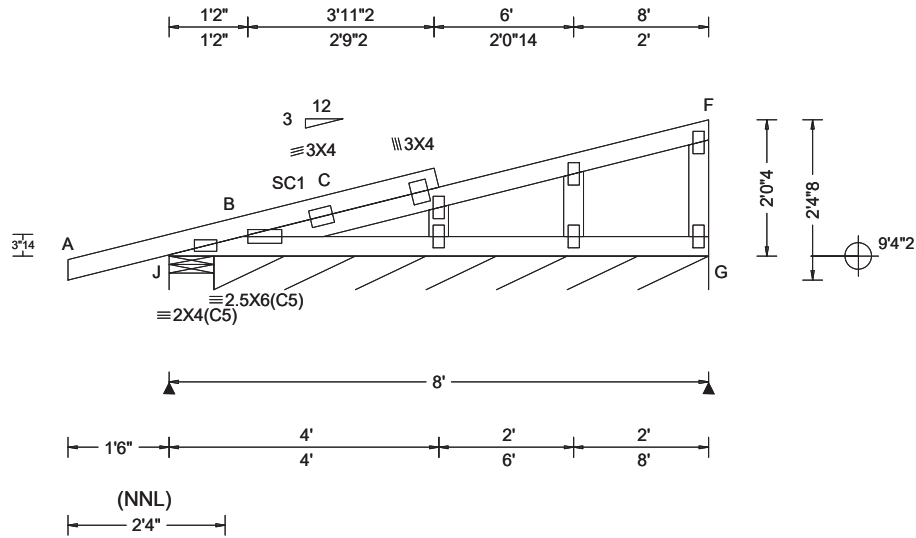
COA #0218

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SEQN: 690910 FROM: CDM	GABL Ply: 1 Qty: 2	Job Number: 23-8875 Archbold Truss Label: J04	Cust: R215 JRef: 1XN32150005 T1 DrwNo: 041.23.1241.41610 KD / DF 02/10/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.010 B 999 180 HORZ(LL): -0.001 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.243 Max BC CSI: 0.090 Max Web CSI: 0.082 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 281 /- /- /158 /82 /65 G* 63 /- /- /34 /5 /- Wind reactions based on MWFRS J Brg Wid = 8.0 Min Req = 1.5 (Truss) G Brg Wid = 88.0 Min Req = - Bearings J & B are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

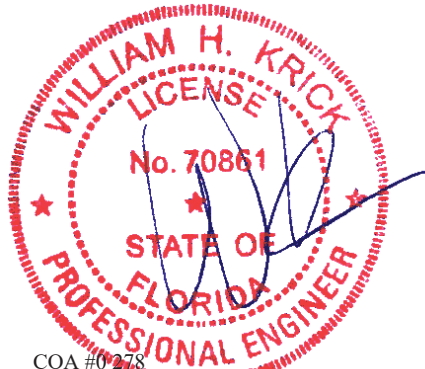
All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

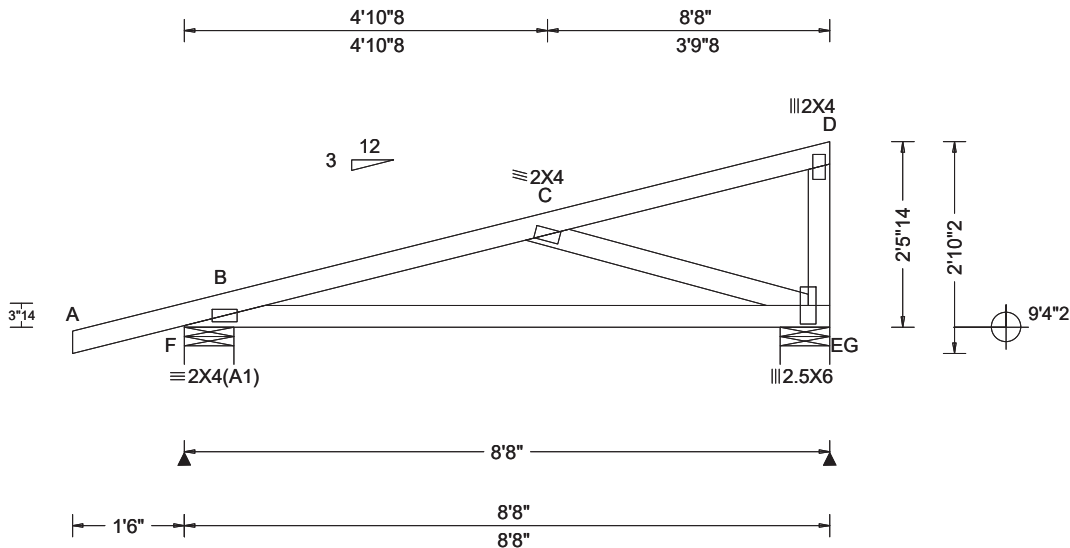
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 2'-0.4.



COA #0278
02/10/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.021 B 999 240 VERT(CL): 0.064 B 999 180 HORZ(LL): 0.006 B - - HORZ(TL): 0.018 B - - Creep Factor: 2.0 Max TC CSI: 0.378 Max BC CSI: 0.591 Max Web CSI: 0.215 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 463 /- /- /250 /98 /87 G 333 /- /- /175 /71 /- Wind reactions based on MWFRS F Brg Wid = 8.0 Min Req = 1.5 (Truss) G Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings F & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 570 -593 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. B - E 560 -691 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - E 724 -573
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Lumber

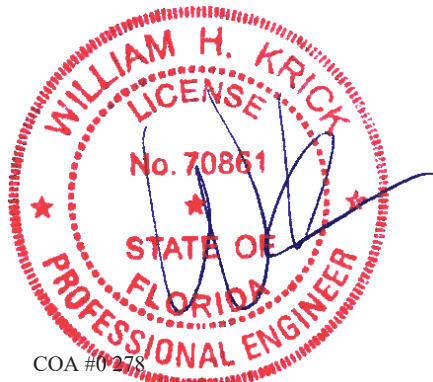
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-5-14.

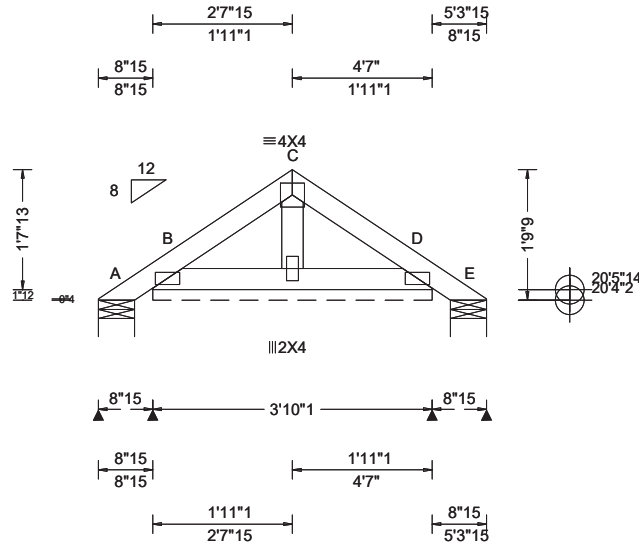


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02/10/2023
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SEQN: 690916 FROM: CDM	COMN Ply: 1 Qty: 17	Job Number: 23-8875 Archbold Truss Label: PB02	Cust: R215 JRef: 1XN32150005 T9 DrwNo: 041.23.1241.50200 KD / DF 02/10/2023
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.25 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.06 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.033 Max BC CSI: 0.017 Max Web CSI: 0.011 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>7</td> <td>/-</td> <td>/-</td> <td>/35</td> <td>/26</td> <td>/48</td> </tr> <tr> <td>B*</td> <td>83</td> <td>/-</td> <td>/-</td> <td>/62</td> <td>/25</td> <td>/-</td> </tr> <tr> <td>E</td> <td>7</td> <td>/-</td> <td>/-</td> <td>/7</td> <td>/0</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	7	/-	/-	/35	/26	/48	B*	83	/-	/-	/62	/25	/-	E	7	/-	/-	/7	/0	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
A	7	/-	/-	/35	/26	/48																																
B*	83	/-	/-	/62	/25	/-																																
E	7	/-	/-	/7	/0	/-																																
				Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 46.1 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#																																		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Refer to DWG PB160160118 for piggyback details.
 The overall height of this truss excluding overhang is 1'-9".



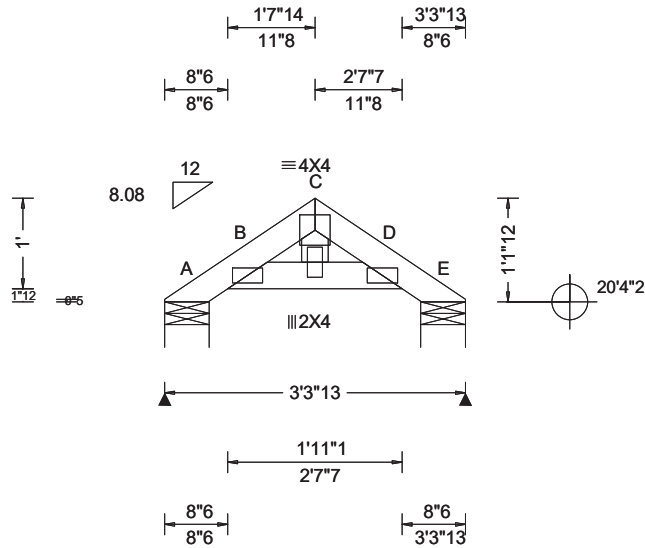
COA #0278

02/10/2023
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SEQN: 690918 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 23-8875 Archbold Truss Label: PB03	Cust: R215 JRef: 1XN32150005 T6 DrwNo: 041.23.1241.52363 KD / DF 02/10/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.93 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.11 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.042 Max BC CSI: 0.021 Max Web CSI: 0.003 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>126</td> <td>/-</td> <td>/-</td> <td>/73</td> <td>/13</td> <td>/31</td> </tr> <tr> <td>E</td> <td>126</td> <td>/-</td> <td>/-</td> <td>/73</td> <td>/13</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	126	/-	/-	/73	/13	/31	E	126	/-	/-	/73	/13	/-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
A	126	/-	/-	/73	/13	/31																									
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Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375#																															

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 1-1-12.

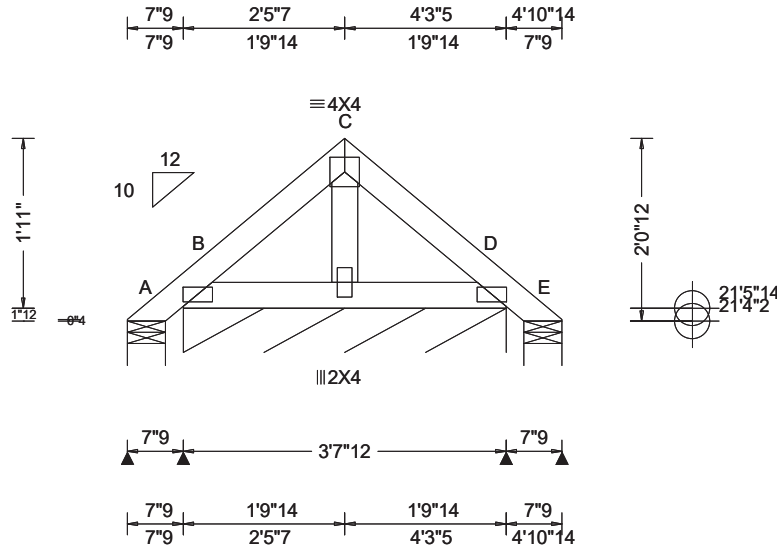


COA #0278

02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF																																		
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.39 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.069 Max BC CSI: 0.025 Max Web CSI: 0.009 VIEW Ver: 22.02.00.0914.12	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>2</td> <td>/0</td> <td>/-</td> <td>/43</td> <td>/42</td> <td>/58</td> </tr> <tr> <td>B*</td> <td>100</td> <td>/-</td> <td>/-</td> <td>/65</td> <td>/32</td> <td>/-</td> </tr> <tr> <td>E</td> <td>2</td> <td>/0</td> <td>/-</td> <td>/4</td> <td>/3</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 43.7 Min Req = - E Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	2	/0	/-	/43	/42	/58	B*	100	/-	/-	/65	/32	/-	E	2	/0	/-	/4	/3	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 2-0-12.

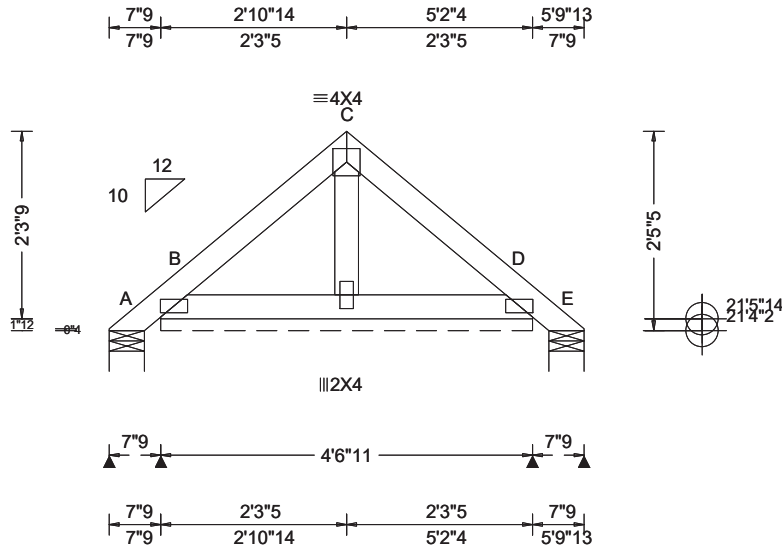


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF																																		
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.58 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.053 Max BC CSI: 0.024 Max Web CSI: 0.012 VIEW Ver: 22.02.00.0914.12	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-10</td> <td>/-</td> <td>/60</td> <td>/59</td> <td>/69</td> </tr> <tr> <td>B*</td> <td>88</td> <td>/-</td> <td>/-</td> <td>/69</td> <td>/32</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-10</td> <td>/-</td> <td>/15</td> <td>/12</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 54.7 Min Req = - E Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-10	/-	/60	/59	/69	B*	88	/-	/-	/69	/32	/-	E	-	/-10	/-	/15	/12	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Refer to DWG PB160160118 for piggyback details.
 The overall height of this truss excluding overhang is 2-5-5.



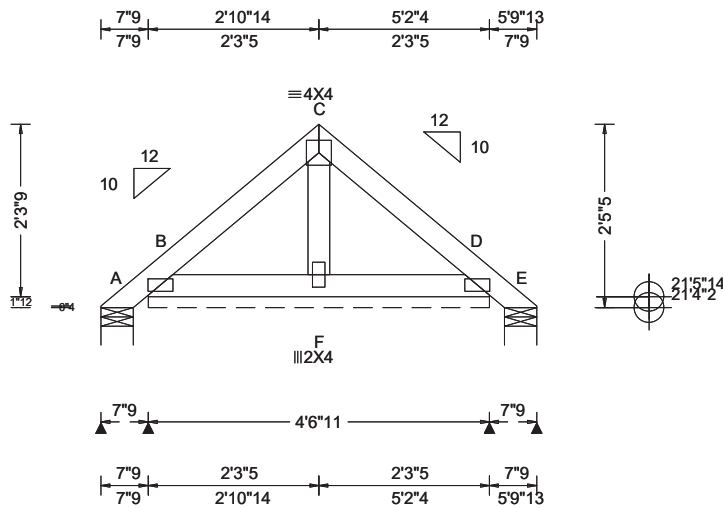
COA #0218

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3 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 120.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.58 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): -0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.117 Max BC CSI: 0.040 Max Web CSI: 0.021 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-63</td> <td>/-</td> <td>/300</td> <td>/320</td> <td>/351</td> </tr> <tr> <td>B*</td> <td>437</td> <td>/-</td> <td>/-</td> <td>/334</td> <td>/212</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-63</td> <td>/-</td> <td>/89</td> <td>/85</td> <td>/-</td> </tr> <tr> <td>D</td> <td></td> <td></td> <td>/-217</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-63	/-	/300	/320	/351	B*	437	/-	/-	/334	/212	/-	E	-	/-63	/-	/89	/85	/-	D			/-217			
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @ 7.25" o.c.
 Bot Chord: 1 Row @ 12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 2X4(A1) except as noted.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2'-5".
 See Detail PB160160118 for piggyback details.



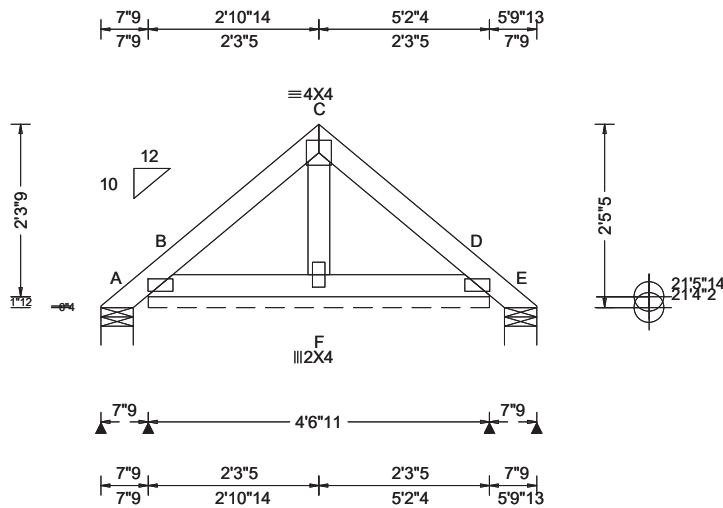
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3 Complete Trusses Required



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Plating Notes

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Purlins

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Wind

Wind loads based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2'-5".
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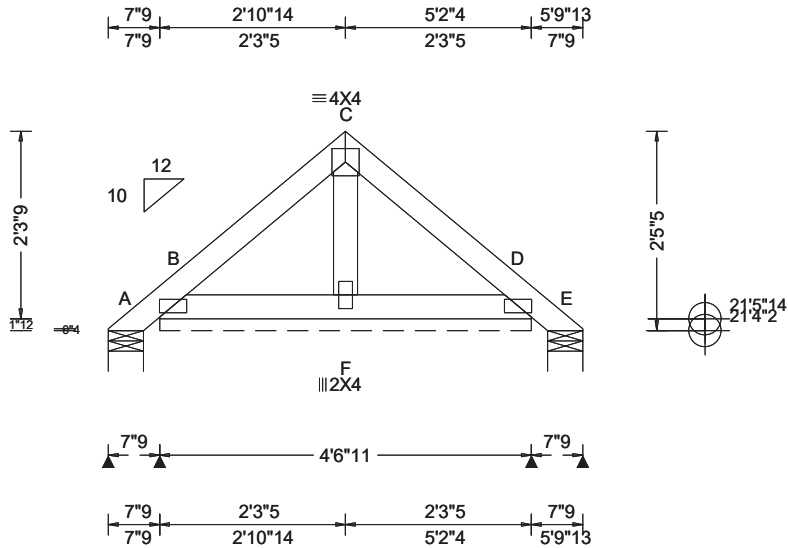


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 60.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.58 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 11.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.186 Max BC CSI: 0.066 Max Web CSI: 0.031 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-31 /- /150 /160 /176 B* 218 /- /- /167 /46 /- E - /-31 /- /32 /42 /- B /-175 D /-108 Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 54.7 Min Req = - E Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

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Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-5-5.
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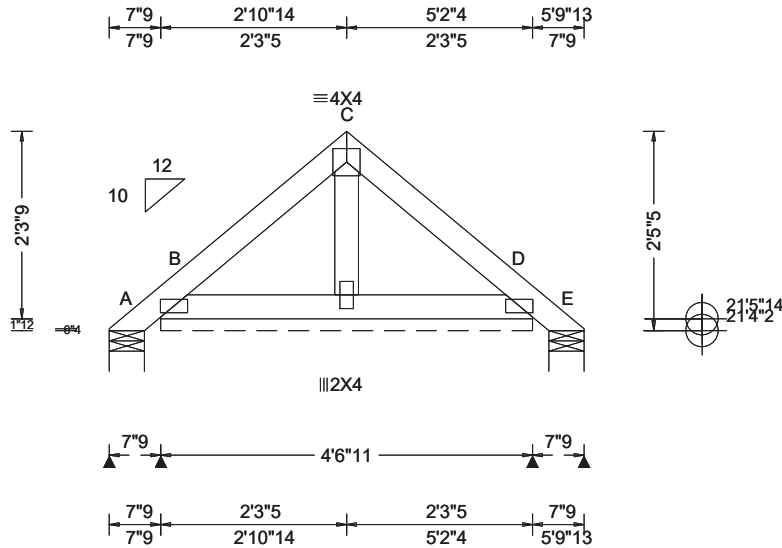


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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.61 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.053 Max BC CSI: 0.024 Max Web CSI: 0.012 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-10</td> <td>/-</td> <td>/60</td> <td>/59</td> <td>/69</td> </tr> <tr> <td>B*</td> <td>88</td> <td>/-</td> <td>/-</td> <td>/69</td> <td>/33</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-10</td> <td>/-</td> <td>/16</td> <td>/12</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-10	/-	/60	/59	/69	B*	88	/-	/-	/69	/33	/-	E	-	/-10	/-	/16	/12	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 2-5-5.

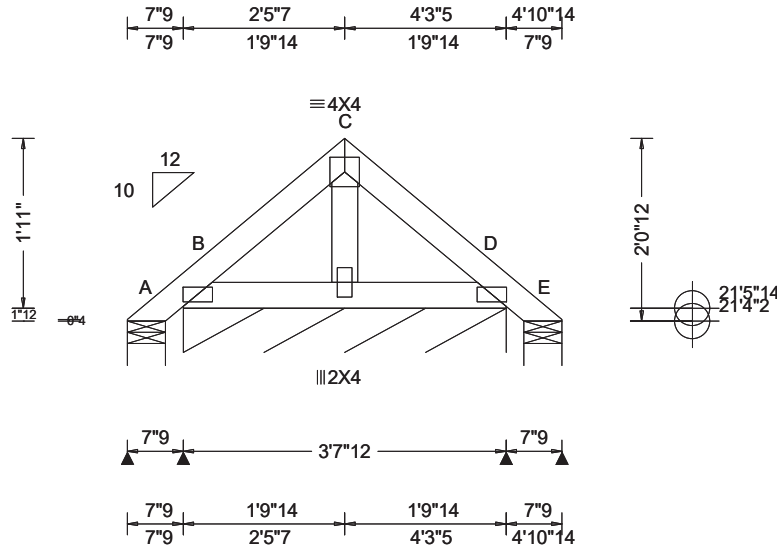


COA #0278

02/10/2023
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.39 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): -0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.069 Max BC CSI: 0.025 Max Web CSI: 0.009 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>2</td> <td>/0</td> <td>/-</td> <td>/43</td> <td>/42</td> <td>/58</td> </tr> <tr> <td>B*</td> <td>100</td> <td>/-</td> <td>/-</td> <td>/65</td> <td>/32</td> <td>/-</td> </tr> <tr> <td>E</td> <td>2</td> <td>/0</td> <td>/-</td> <td>/4</td> <td>/3</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	2	/0	/-	/43	/42	/58	B*	100	/-	/-	/65	/32	/-	E	2	/0	/-	/4	/3	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

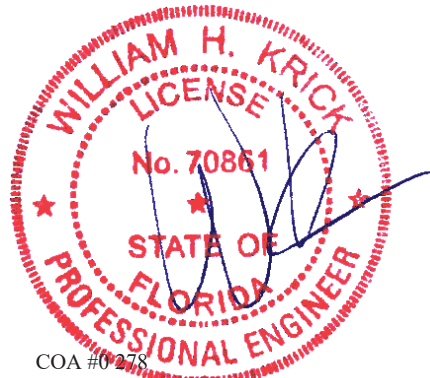
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 The overall height of this truss excluding overhang is 2-0-12.
 See Detail PB160160118 for piggyback details.



COA #0278

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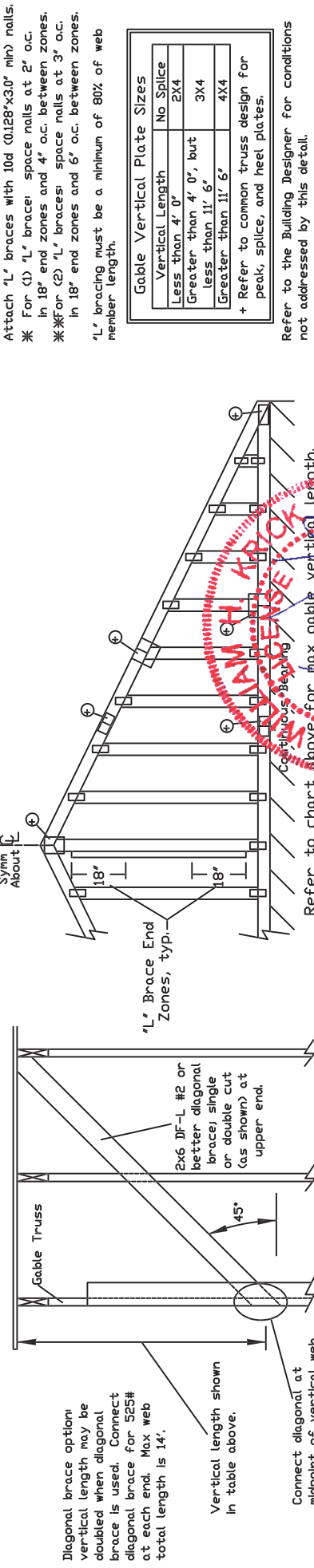
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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00
 Or 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Or 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Or 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

2x4 Gable Vertical Spacing	Gable Vertical Species	No Braces	(1) 1x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace	
			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
12" o.c.	SPF	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SP	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	10' 3"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	9' 6"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	4' 7"	6' 10"	7' 3"	8' 1"	8' 8"	10' 1"	10' 1"	11' 6"	12' 0"	13' 15"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	4' 5"	6' 0"	6' 5"	7' 3"	7' 8"	8' 7"	10' 10"	10' 10"	12' 7"	13' 15"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	4' 2"	5' 9"	6' 1"	7' 1"	7' 6"	8' 9"	10' 4"	10' 4"	12' 9"	13' 15"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	4' 10"	7' 5"	8' 11"	9' 11"	10' 7"	12' 2"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	4' 10"	7' 5"	8' 11"	9' 11"	10' 7"	12' 2"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	4' 10"	7' 5"	8' 11"	9' 11"	10' 7"	12' 2"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"



REF	ASCE7-16-GABI4030
DATE	01/26/2018
DRWG	A14030ENC160118
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

Florida Certificate of Product Approval
 No. 70861
 02/10/2023
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER

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Bracing Group Species and Grades:

Group A:	
Spruce-Pine-Fir	Hem-Fir
#1 / #2 Standard	#2 Stud
#3 Stud	#3 Standard
Douglas Fir-Larch	Southern Pine***
#3 Stud	#3 Stud
Standard	Standard

Group B:	
Hem-Fir	#1
#1 & Btr	#2
Douglas Fir-Larch	Southern Pine***
#1 Stud	#1 Stud
Standard	Standard

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial S5 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 100 plf over continuous bearing (5 psf IC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

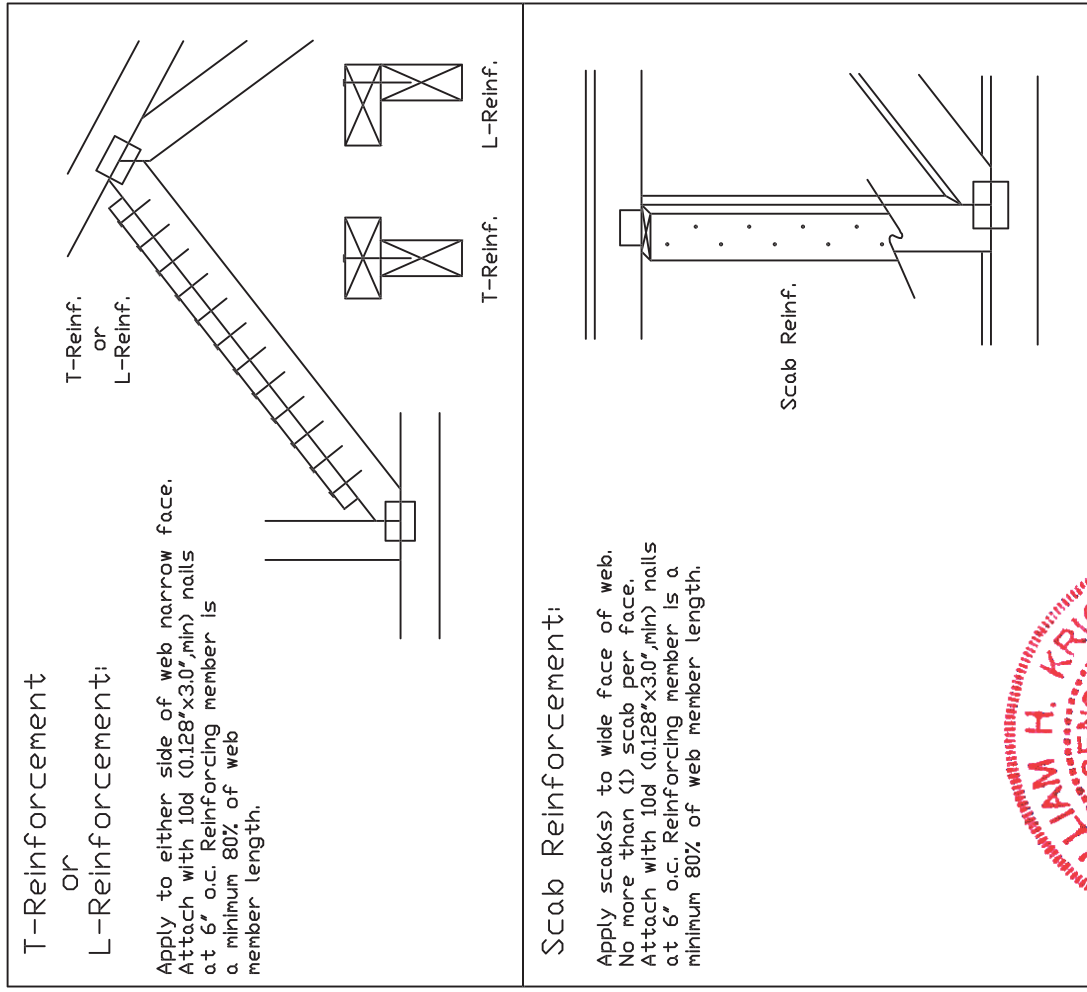
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
	2 rows	2x6	2-2x6(*)

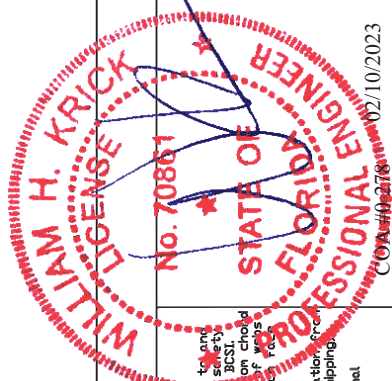
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.



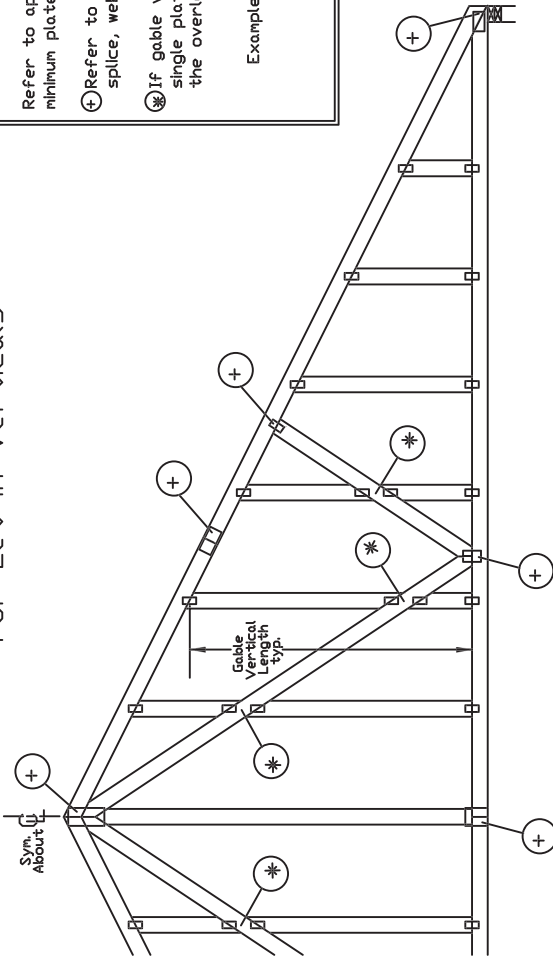
ALPINE
 AN ITW COMPANY
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

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PSF	REF	CLR Subst.
TC DL	DATE	01/02/19
BC DL	DRWG	BRCLBSUB0119
BC LL	PSF	
TOT. L.D.	PSF	
DUR. FAC.		
DATE		

Gable Detail For Let-in Verticals

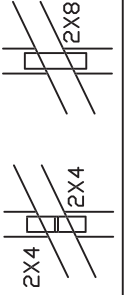


Gable Truss Plate Sizes

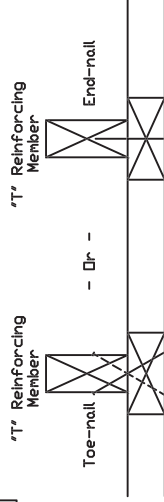
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase %
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x 3", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3", min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE

wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014,

A13030051014, A12030051014, A11030051014, A10030051014,

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,

A18015ENC100118, A20015ENC100118, A20015ENDI00118, A2001SPEDI00118,

A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,

A18030ENC100118, A20030ENC100118, A20030ENDI00118, A20030PEDI00118,

S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,

S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,

S18030ENC100118, S20030ENC100118, S20030PEDI00118

See appropriate Alpine gable detail for maximum allowable gable vertical length.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING THE INSTALLERS
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Components Safety Information, TPI and SBA's for best practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections 83, 87 or 810, as applicable. Apply plates to each end of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1004-2 for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, or bracing of trusses.
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpinetw.com, TPI: www.tpinet.org, SBCA: www.sbcccomponents.com, ICC: www.iccsafe.org

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

MAX. TOT. LD. 60 PSF
 DUR. FAC. ANY
 APPROVAL # 1999
 MAX. SPACING 24.0"

No. 70861
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 COA # 0278
 Florida Certificate of Product Approval # 1999
 02/10/2023

ALPINE
 AN ITW COMPANY
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Gable Stud Reinforcement Detail

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

2x4 Gable Vertical Spacing	Gable Vertical Species	Brace Grade	No Braces		(1) 1x4 "L" Brace		(2) 2x4 "L" Brace		(1) 2x4 "L" Brace		(2) 2x4 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace	
			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
12" o.c.	SPF	#1 / #2	7' 3"	8' 7"	8' 11"	10' 3"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	6' 7"	7' 1"	8' 10"	10' 1"	10' 6"	13' 4"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	6' 7"	7' 0"	8' 10"	10' 1"	10' 6"	13' 4"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	HF	Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	4' 6"	7' 4"	7' 8"	9' 0"	10' 4"	12' 5"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 11"	10' 3"	12' 2"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	DFL	#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	11' 0"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1 / #2	4' 11"	8' 4"	8' 8"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	HF	#3	4' 8"	8' 1"	8' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	8' 1"	8' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 1"	8' 5"	8' 9"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
16" o.c.	SP	#2	4' 11"	8' 4"	8' 8"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	SP	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	DFL	Standard	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 1"	7' 5"	7' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 1"	7' 5"	7' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	

Bracing Group Species and Grades:

Group A:

- Spruce-Pine-Fir: #1 / #2 Standard, #3 Stud
- Hem-Fir: #2 Standard, #3 Stud
- Douglas Fir-Larch: #3 Stud, Standard
- Southern Pine***: #3 Stud, Standard

Group B:

- Hem-Fir: #1 & Btr, #1
- Douglas Fir-Larch: #1, #2
- Southern Pine***: #1, #2

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial S5 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

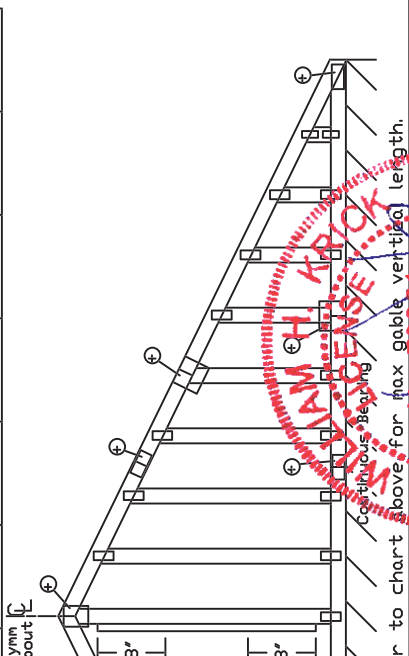
Wind Load deflection criterion is L/240.
 Provide uplift connections for 55 psf over continuous bearing (5 psf IC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



REF	ASCE7-16-GABI4015
DATE	01/26/2018
DRWG	A14015ENC160118

MAX. TOT. LD. 60 PSF

APPROVAL: [Signature]

MAX. SPACING 24.0"

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLER'S

Trusses require extensive care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Components Installation, TPI and SBA for best practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position of standard plate above and on the joint details, unless noted otherwise. Refer to drawings 1049-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the truss design shown. The truss is to be installed in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and web site:
 ALPINE: www.alpinetw.com; TPI: www.tpi.org; SBCA: www.sbcccomponents.com; ICC: www.iccsafe.org

ALPINE
 AN ITW COMPANY

155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

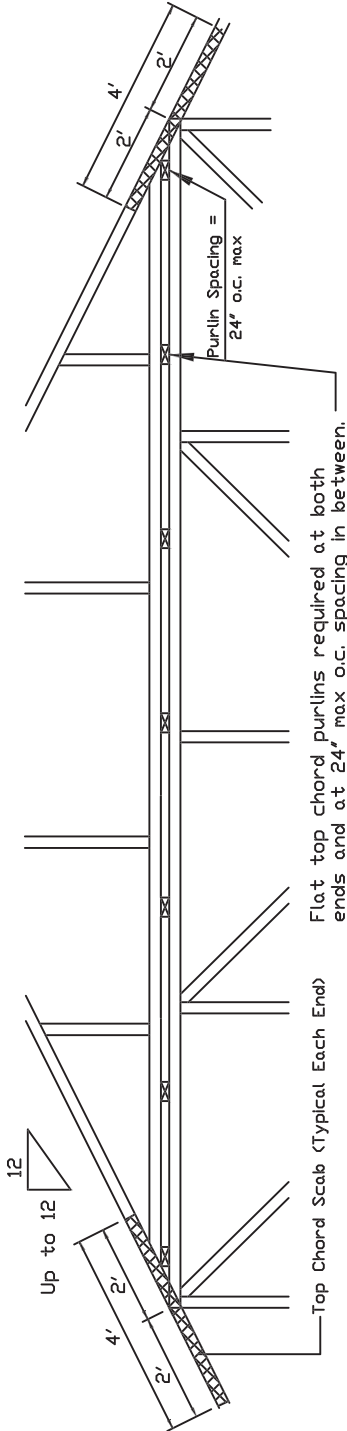
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (min), Kzt=1.00
 Dir 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, wind DL = 5.0 psf (min), Kzt=1.00

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

*** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



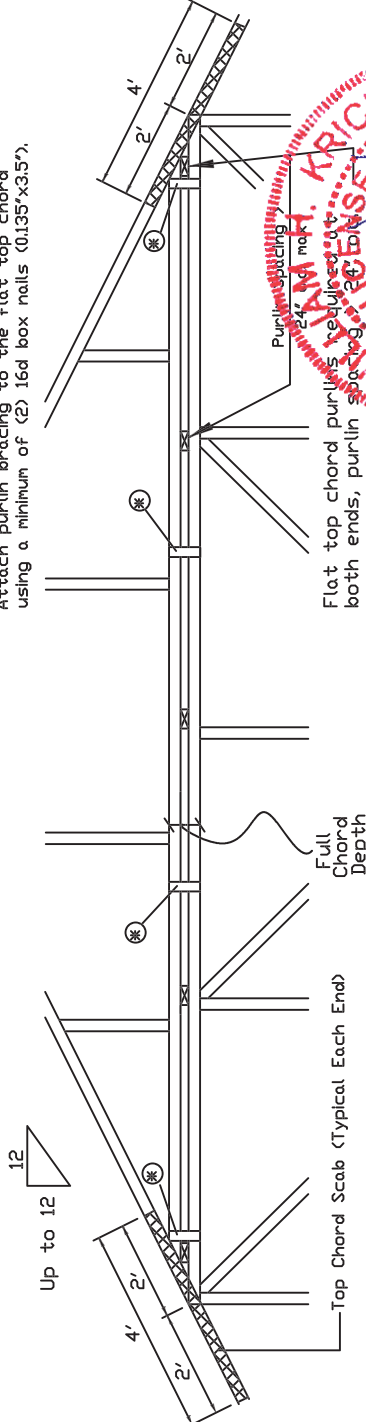
Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the truss, purlins must be installed at 24' o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

Trulox
 Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8" o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4" o.c. front to back faces.

APA Rated Gusset
 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8" o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4" o.c. front to back faces.

2x4 Vertical Scabs
 2x4 SPF #2 Full chord depth scabs (each face). Attach @ 8" o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4" o.c. front to back faces.

28PB Wave Piggyback Plate
 One 28PB wave piggyback plate to each face @ 8" o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4" o.c. front to back faces.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING! CONTRACTORS INCLUDING THE INSTALLERS

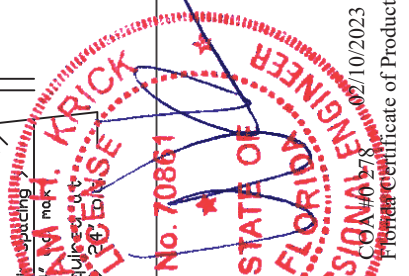
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Components Installation Manual (TPI and SBC) for all details and practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1001-2 for standard plate positions.

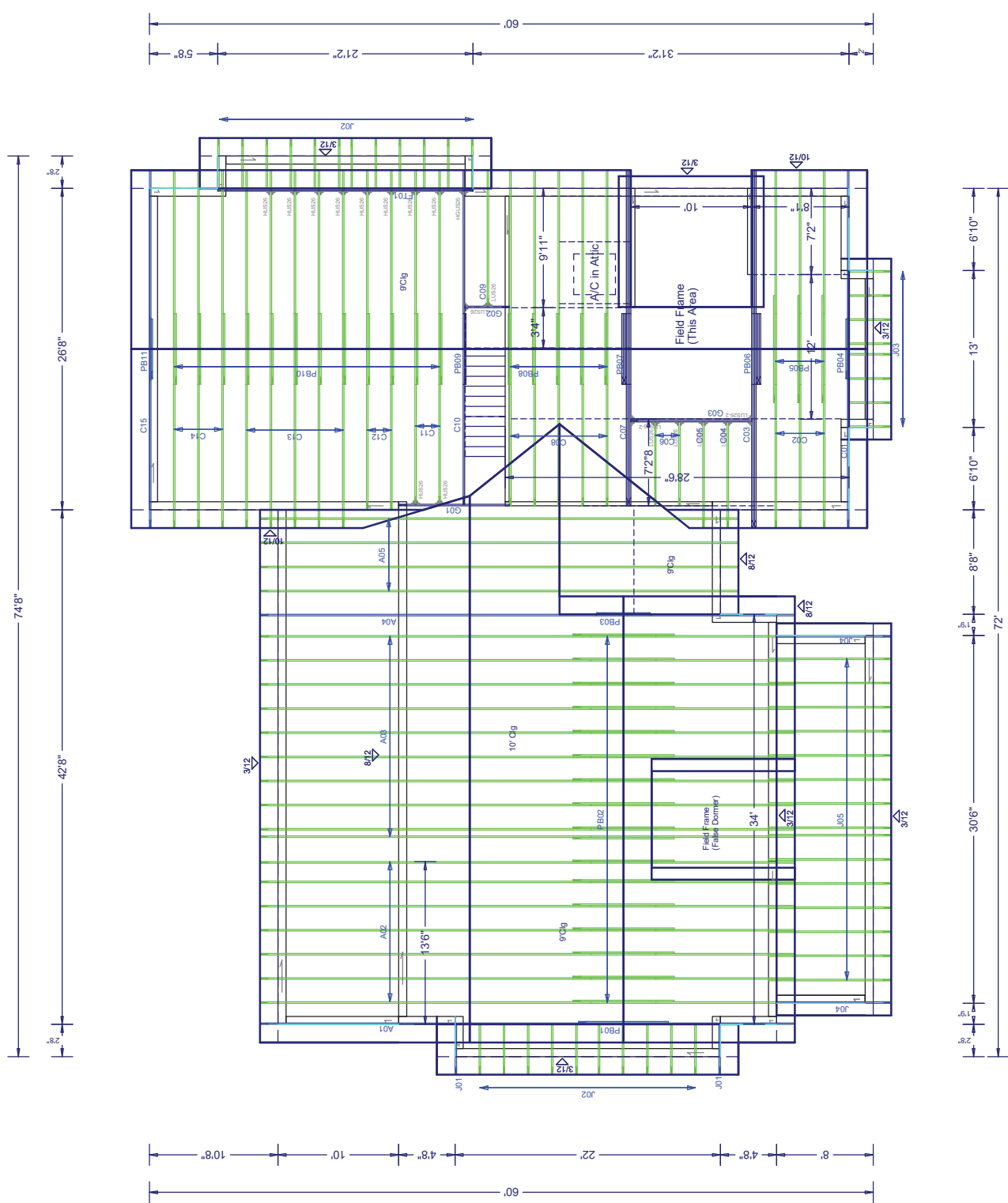
Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from the truss or sheathing installation and bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
 ALPINE: www.alpinetw.com | TPI: www.tpi.net | SBC: www.sbcccomponents.com | ICC: www.iccsafe.org

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
Approval #	1999
SPACING	24.0"





W.B. Howland Truss Co.
 610 11th St. SW
 Live Oak, FL 32064
 (386) 362-1235
 (386) 362-7124 (Fax)
 howlandtruss@gmail.com

ROOF PITCH: 3, 8, 10/12
 OVERRANG: 18"
 CEILING: 9', 10"
 EXT. WALLS: 8" CMU
 FLOORING: 40psf
 WIND LOAD: 150mph
 EXPOSURE: C
 DATE: 2/7/23

Truss to Truss Connectors:
 (6) LUS26
 (2) LUS26-2
 (10) HUS26
 (1) HGUS26

JOB #: 23-8875

Job Name: Archbold Franks
 Customer: BRADLEY FRANKS
 Designer: Fill in later
 ADDRESS: Lot 32, Village on the Green
 SALESMAN: DB
 : <Not Found>

JOB NO:
 23-8875

PAGE NO:
 1 OF 1