

COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026



ine, an ITW Company
 155 Harlem Ave
 10th Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com

This item has been digitally signed by Douglas Fleming on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

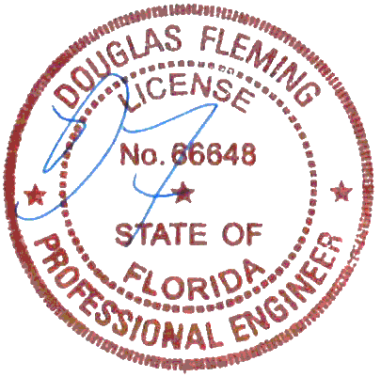
| Site Information: | Page 1: |
|---|---------------------|
| Customer: W. B. Howland Company, Inc. | Job Number: 25-3040 |
| Job Description: ANDERSON PROJECT | |
| Address: SW MEADOWLANDS DR, Lake City, FL 32024 | |

| Job Engineering Criteria: | |
|---|---|
| Design Code: FBC 8th Ed. 2023 Res. | IntelliVIEW Version: 24.02.00D JRef #: 1YGX2150009 |
| Wind Standard: ASCE 7-22 Wind Speed (mph): 140 | Design Loading (psf): 45 |
| Building Type: Enclosed | |

This package contains general notes pages, 54 truss drawing(s) and 3 detail(s).

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 1 | 019.26.1550.16253 | A1 |
| 3 | 019.26.1550.24770 | A1B |
| 5 | 019.26.1551.31073 | A1E |
| 7 | 019.26.1551.38413 | A3 |
| 9 | 019.26.1551.43203 | A5 |
| 11 | 019.26.1551.48233 | B2 |
| 13 | 019.26.1551.54197 | B4 |
| 15 | 019.26.1552.44760 | B5A |
| 17 | 019.26.1552.56517 | EJ1A |
| 19 | 019.26.1553.09470 | EJ2A |
| 21 | 019.26.1554.13830 | EJ3A |
| 23 | 019.26.1601.16577 | EJ4 |
| 25 | 019.26.1556.24773 | HJ2 |
| 27 | 019.26.1557.22633 | HJ4 |
| 29 | 019.26.1558.18350 | J1 |
| 31 | 019.26.1557.37747 | J01B |
| 33 | 019.26.1557.39127 | J03 |
| 35 | 019.26.1557.55710 | J03A |
| 37 | 019.26.1558.33380 | J5 |
| 39 | 019.26.1558.34547 | J5A |
| 41 | 019.26.1558.10550 | J05B |
| 43 | 019.26.1558.42297 | J5C |
| 45 | 019.26.1558.11780 | J07 |
| 47 | 019.26.1558.14570 | J07A |
| 49 | 019.26.1559.18440 | V1 |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 2 | 019.26.1550.18730 | A1A |
| 4 | 019.26.1551.08063 | A1BE |
| 6 | 019.26.1551.33920 | A2 |
| 8 | 019.26.1551.40547 | A4 |
| 10 | 019.26.1551.45723 | B1 |
| 12 | 019.26.1551.51703 | B3 |
| 14 | 019.26.1552.14907 | B5 |
| 16 | 019.26.1552.47307 | EJ1 |
| 18 | 019.26.1553.07267 | EJ2 |
| 20 | 019.26.1553.57103 | EJ3 |
| 22 | 019.26.1554.24373 | EJ3B |
| 24 | 019.26.1555.48607 | HJ1 |
| 26 | 019.26.1556.41563 | HJ3 |
| 28 | 019.26.1557.32930 | HJ5 |
| 30 | 019.26.1557.36120 | J01A |
| 32 | 019.26.1558.28957 | J3 |
| 34 | 019.26.1558.31153 | J3A |
| 36 | 019.26.1557.57243 | J03B |
| 38 | 019.26.1558.05860 | J05 |
| 40 | 019.26.1558.07473 | J05A |
| 42 | 019.26.1558.36133 | J5B |
| 44 | 019.26.1558.45060 | J7 |
| 46 | 019.26.1558.46657 | J7A |
| 48 | 019.26.1559.15627 | J7B |
| 50 | 019.26.1559.32160 | V2 |



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| Site Information: | Page 2: |
|---|---------------------|
| Customer: W. B. Howland Company, Inc. | Job Number: 25-3040 |
| Job Description: ANDERSON PROJECT | |
| Address: SW MEADOWLANDS DR, Lake City, FL 32024 | |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 51 | 019.26.1559.36463 | V3 |
| 53 | 019.26.1559.39190 | V5 |
| 55 | BRCLBSUB0119 | |
| 57 | VALTN220723 | |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 52 | 019.26.1559.37987 | V4 |
| 54 | 019.26.1559.40390 | V6 |
| 56 | VAL180220723 | |
| | | |

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Borafire Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

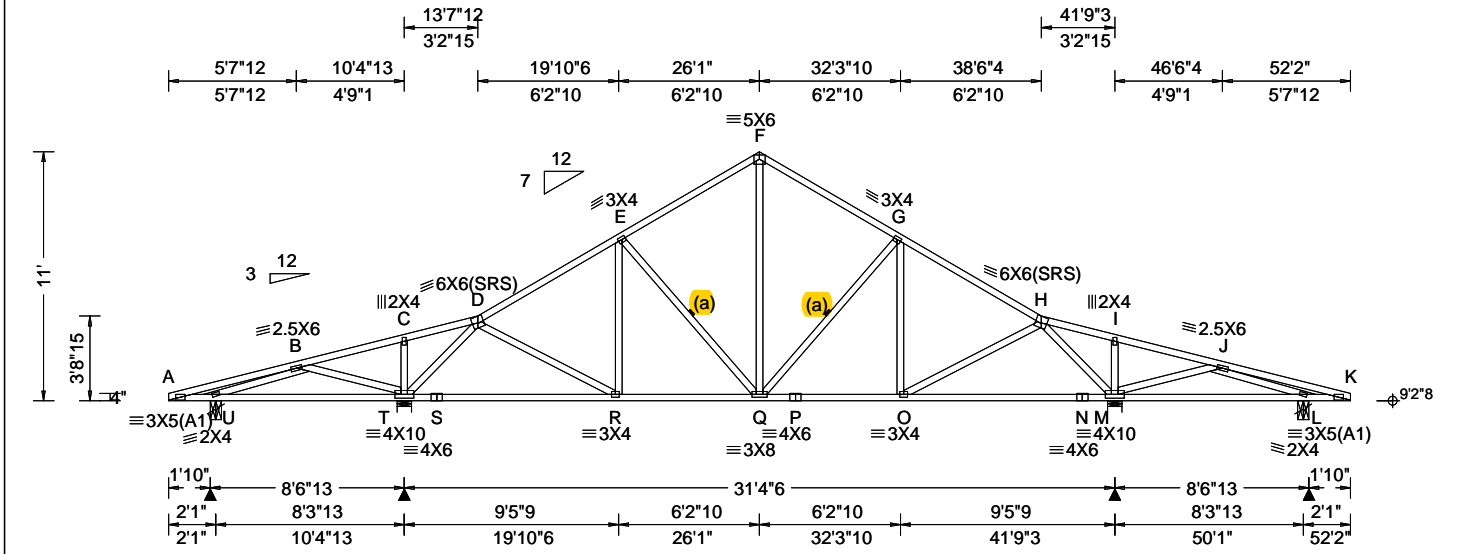
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com



| | | | | |
|--|--|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 5.22 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.052 Q 999 240 VERT(CL): 0.115 Q 999 180 HORZ(LL): 0.019 M - - HORZ(TL): 0.043 M - - Creep Factor: 2.0 Max TC CSI: 0.571 Max BC CSI: 0.691 Max Web CSI: 1.000 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity U 362 - / - / /153 /116 /289 T 2267 - / - / /1398 /601 - M 2267 - / - / /1346 /601 - L 362 - / - / /153 /98 - Wind reactions based on MWFRS U Brg Wid = 6.0 Min Req = 1.5 (Truss) T Brg Wid = 7.6 Min Req = 2.7 (Truss) M Brg Wid = 7.6 Min Req = 2.7 (Truss) L Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings U, T, M, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 474 -465 F - G 524 -1275 B - C 978 -367 G - H 482 -1548 C - D 976 -336 H - I 977 -339 D - E 481 -1548 I - J 979 -370 E - F 523 -1275 J - K 474 -494 |
|--|--|--|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

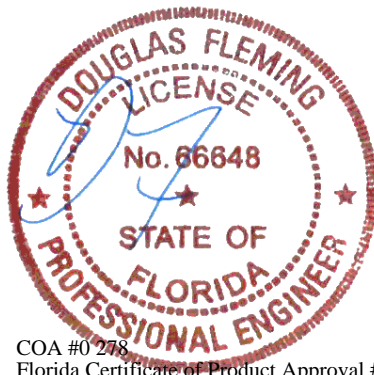
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss excluding overhang is 11'-0".



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Maximum Bot Chord Forces Per Ply (lbs)

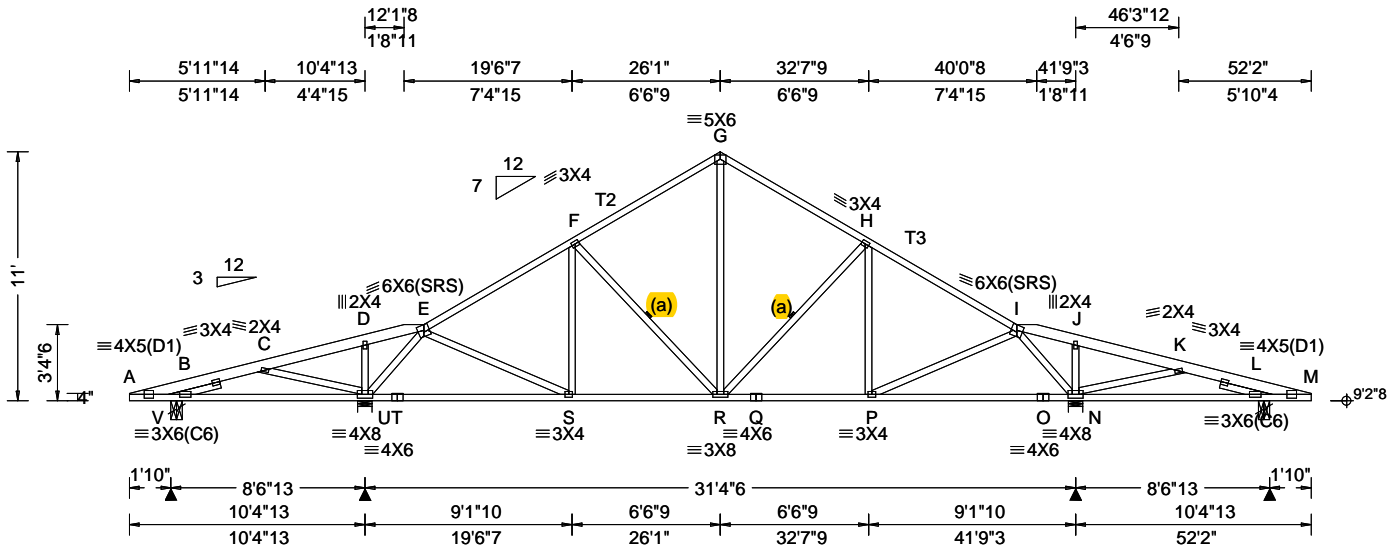
| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - U | 487 -410 | P - O | 1246 -187 |
| U - T | 165 -395 | O - N | 705 -116 |
| T - S | 705 -125 | N - M | 705 -116 |
| S - R | 705 -125 | M - L | 111 -395 |
| R - Q | 1246 -188 | L - K | 518 -410 |
| Q - P | 1246 -187 | | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| U - B | 454 -432 | Q - G | 219 -382 |
| B - T | 423 -732 | O - H | 698 -98 |
| T - D | 773 -2234 | H - M | 777 -2234 |
| D - R | 698 -105 | M - J | 439 -732 |
| E - Q | 222 -382 | J - L | 494 -432 |
| F - Q | 727 -298 | | |

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





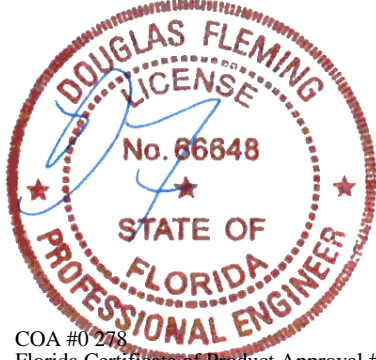
| | | | | |
|--|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 5.22 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.042 R 999 240 VERT(CL): 0.100 R 999 180 HORZ(LL): 0.014 N - - HORZ(TL): 0.035 N - - Creep Factor: 2.0 Max TC CSI: 0.566 Max BC CSI: 0.641 Max Web CSI: 0.683 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity V 369 - / - / /162 /106 /296 U 2150 - / - / /1359 /578 - N 2157 - / - / /1315 /581 - L 365 - / - / /159 /89 - Wind reactions based on MWFRS V Brg Wid = 6.0 Min Req = 1.5 (Truss) U Brg Wid = 7.6 Min Req = 2.5 (Truss) N Brg Wid = 7.6 Min Req = 2.5 (Truss) L Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings V, U, N, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 585 -589 G - H 496 -1172 C - D 941 -420 H - I 441 -1422 D - E 932 -395 I - J 943 -397 E - F 440 -1423 J - K 955 -424 F - G 495 -1172 L - M 574 -581 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - V 602 -543 Q - P 1137 -179 U - T 530 -111 P - O 524 -37 T - S 530 -111 O - N 524 -37 S - R 1139 -181 L - M 594 -530 R - Q 1137 -179 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - U 394 -669 P - I 805 -184 U - E 743 -1988 I - N 743 -1991 E - S 796 -181 N - K 401 -681 G - R 620 -254 |
|--|---|--|---|--|

Lumber
Top chord: 2x6 SP #2; T2,T3 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 2.333'
Rt Slider: 2x4 SP #3; block length = 2.333'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

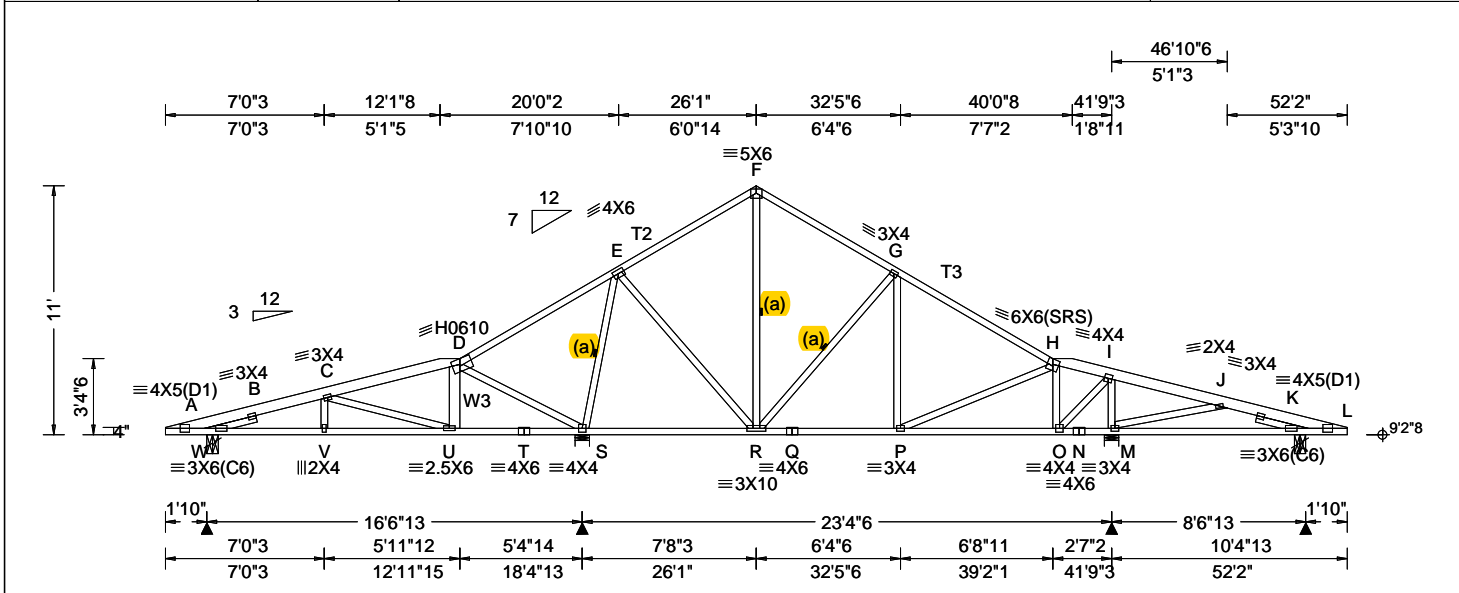
Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss excluding overhang is 11'-0".



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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





| Loading Criteria (psf) TCCL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.22 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.031 V 999 240 VERT(CL): 0.075 V 999 180 HORZ(LL): 0.007 O - - HORZ(TL): 0.018 O - - Creep Factor: 2.0 Max TC CSI: 0.772 Max BC CSI: 0.483 Max Web CSI: 0.641 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>W</td> <td>676</td> <td>-</td> <td>-</td> <td>/362</td> <td>/167</td> <td>/296</td> </tr> <tr> <td>S</td> <td>2283</td> <td>-</td> <td>-</td> <td>/1456</td> <td>/593</td> <td>-</td> </tr> <tr> <td>M</td> <td>1626</td> <td>-</td> <td>-</td> <td>/1000</td> <td>/432</td> <td>-</td> </tr> <tr> <td>K</td> <td>414</td> <td>-</td> <td>-</td> <td>/212</td> <td>/95</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS W Brg Wid = 6.0 Min Req = 1.5 (Truss) S Brg Wid = 7.6 Min Req = 2.7 (Truss) M Brg Wid = 7.6 Min Req = 1.9 (Truss) K Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings W, S, M, & K are a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | W | 676 | - | - | /362 | /167 | /296 | S | 2283 | - | - | /1456 | /593 | - | M | 1626 | - | - | /1000 | /432 | - | K | 414 | - | - | /212 | /95 | - |
|--|--|--|---|--|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|------|------|---|------|---|---|-------|------|---|---|------|---|---|-------|------|---|---|-----|---|---|------|-----|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| W | 676 | - | - | /362 | /167 | /296 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| S | 2283 | - | - | /1456 | /593 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| M | 1626 | - | - | /1000 | /432 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| K | 414 | - | - | /212 | /95 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| Lumber Top chord: 2x6 SP #2; T2,T3 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; W3 2x6 SP #2; Lt Slider: 2x4 SP #3; block length = 2.333' Rt Slider: 2x4 SP #3; block length = 2.333' | Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>530</td> <td>-510</td> <td>F - G</td> <td>319</td> <td>-513</td> </tr> <tr> <td>B - C</td> <td>423</td> <td>-753</td> <td>G - H</td> <td>317</td> <td>-857</td> </tr> <tr> <td>D - E</td> <td>965</td> <td>-402</td> <td>I - J</td> <td>690</td> <td>-411</td> </tr> <tr> <td>E - F</td> <td>319</td> <td>-507</td> <td>K - L</td> <td>513</td> <td>-552</td> </tr> </tbody> </table> | Chords | Tens. | Comp. | Chords | Tens. | Comp. | A - B | 530 | -510 | F - G | 319 | -513 | B - C | 423 | -753 | G - H | 317 | -857 | D - E | 965 | -402 | I - J | 690 | -411 | E - F | 319 | -507 | K - L | 513 | -552 |
|--|---|--------|--------|-------|--------|-------|-------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|
| Chords | Tens. | Comp. | Chords | Tens. | Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 530 | -510 | F - G | 319 | -513 | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - C | 423 | -753 | G - H | 317 | -857 | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - E | 965 | -402 | I - J | 690 | -411 | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E - F | 319 | -507 | K - L | 513 | -552 | | | | | | | | | | | | | | | | | | | | | | | | | | |

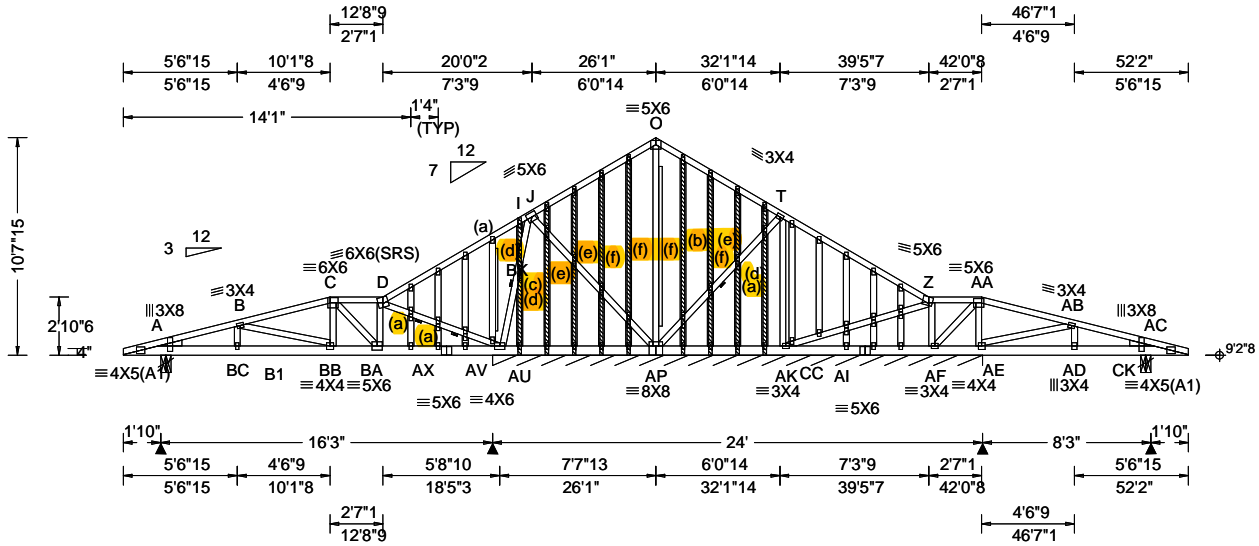
| Bracing (a) Continuous lateral restraint equally spaced on member. | Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>W - V</td> <td>708</td> <td>-382</td> <td>Q - P</td> <td>635</td> <td>0</td> </tr> <tr> <td>A - W</td> <td>524</td> <td>-481</td> <td>P - O</td> <td>424</td> <td>0</td> </tr> <tr> <td>V - U</td> <td>705</td> <td>-385</td> <td>O - N</td> <td>476</td> <td>-508</td> </tr> <tr> <td>S - R</td> <td>431</td> <td>-430</td> <td>N - M</td> <td>476</td> <td>-508</td> </tr> <tr> <td>R - Q</td> <td>635</td> <td>0</td> <td>K - L</td> <td>563</td> <td>-469</td> </tr> </tbody> </table> | Chords | Tens. | Comp. | Chords | Tens. | Comp. | W - V | 708 | -382 | Q - P | 635 | 0 | A - W | 524 | -481 | P - O | 424 | 0 | V - U | 705 | -385 | O - N | 476 | -508 | S - R | 431 | -430 | N - M | 476 | -508 | R - Q | 635 | 0 | K - L | 563 | -469 |
|--|---|--------|--------|-------|--------|-------|-------|-------|-----|------|-------|-----|---|-------|-----|------|-------|-----|---|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|---|-------|-----|------|
| Chords | Tens. | Comp. | Chords | Tens. | Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| W - V | 708 | -382 | Q - P | 635 | 0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - W | 524 | -481 | P - O | 424 | 0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| V - U | 705 | -385 | O - N | 476 | -508 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| S - R | 431 | -430 | N - M | 476 | -508 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R - Q | 635 | 0 | K - L | 563 | -469 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| Additional Notes WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. The overall height of this truss excluding overhang is 11-0-0. | Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.</th> <th>Comp.</th> <th>Webs</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>W - B</td> <td>893</td> <td>-1278</td> <td>R - G</td> <td>312</td> <td>-517</td> </tr> <tr> <td>C - U</td> <td>519</td> <td>-696</td> <td>H - O</td> <td>354</td> <td>-827</td> </tr> <tr> <td>U - D</td> <td>380</td> <td>-133</td> <td>O - I</td> <td>1183</td> <td>-440</td> </tr> <tr> <td>D - S</td> <td>534</td> <td>-816</td> <td>I - M</td> <td>685</td> <td>-1337</td> </tr> <tr> <td>S - E</td> <td>800</td> <td>-1780</td> <td>M - J</td> <td>542</td> <td>-644</td> </tr> <tr> <td>E - R</td> <td>893</td> <td>-251</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> | Webs | Tens. | Comp. | Webs | Tens. | Comp. | W - B | 893 | -1278 | R - G | 312 | -517 | C - U | 519 | -696 | H - O | 354 | -827 | U - D | 380 | -133 | O - I | 1183 | -440 | D - S | 534 | -816 | I - M | 685 | -1337 | S - E | 800 | -1780 | M - J | 542 | -644 | E - R | 893 | -251 | | | |
|--|---|-------|-------|-------|-------|-------|-------|-------|-----|-------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|------|------|-------|-----|------|-------|-----|-------|-------|-----|-------|-------|-----|------|-------|-----|------|--|--|--|
| Webs | Tens. | Comp. | Webs | Tens. | Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| W - B | 893 | -1278 | R - G | 312 | -517 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C - U | 519 | -696 | H - O | 354 | -827 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| U - D | 380 | -133 | O - I | 1183 | -440 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - S | 534 | -816 | I - M | 685 | -1337 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| S - E | 800 | -1780 | M - J | 542 | -644 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E - R | 893 | -251 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



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| Loading Criteria (psf) | |
|------------------------|--------|
| TCLL: | 20.00 |
| TCDL: | 15.00 |
| BCLL: | 0.00 |
| BCDL: | 10.00 |
| Des Ld: | 45.00 |
| NCBCLL: | 10.00 |
| Soffit: | 2.00 |
| Load Duration: | 1.25 |
| Spacing: | 24.0 " |

| Wind Criteria | |
|----------------------|-----------|
| Wind Std: | ASCE 7-22 |
| Speed: | 140 mph |
| Enclosure: | Enclosed |
| Risk Category: | II |
| EXP: | C Kzt: NA |
| Mean Height: | 15.00 ft |
| TCDL: | 4.2 psf |
| BCDL: | 3.0 psf |
| MWFRS Parallel Dist: | 0 to h/2 |
| C&C Dist a: | 5.22 ft |
| Loc. from endwall: | Any |
| GCp1: | 0.18 |
| Wind Duration: | 1.60 |

| Snow Criteria (Pg,Pf in PSF) | |
|------------------------------|-------------------|
| Pg: | NA Ct: NA CAT: NA |
| Pf: | NA Ce: NA |
| Lu: | NA Cs: NA |
| Snow Duration: | NA |
| Building Code: | |
| FBC 8th Ed. 2023 Res. | |
| TPI Std: 2014 | |
| Rep Fac: Varies by Ld Case | |
| FT/RT:20(0)/10(0) | |
| Plate Type(s): | |
| WAVE | |

| Defl/CSI Criteria | |
|---------------------------------|-----------------|
| PP Deflection in loc L/defl L/# | |
| VERT(LL): | 0.088 B 999 240 |
| VERT(CL): | 0.208 B 943 180 |
| HORZ(LL): | -0.020 I - - |
| HORZ(TL): | 0.048 I - - |
| Creep Factor: | 2.0 |
| Max TC CSI: | 0.862 |
| Max BC CSI: | 0.819 |
| Max Web CSI: | 0.965 |
| VIEW Ver: 24.02.00D.0114.10 | |

| ▲ Maximum Reactions (lbs), or *PLF | | | | | |
|---|---------|---------|-------------|-----|-------|
| Gravity | | | Non-Gravity | | |
| Loc | R+ / R- | / Rh | / Rw | / U | / RL |
| A | 1528 | - / - | - / - | 400 | - / - |
| AV* | 344 | - / - | - / - | 79 | - / - |
| CK | 656 | - / - | - / - | 150 | - / - |
| AU | | - / 160 | | | |
| CC | | - / 179 | | | |
| AF | | - / 676 | | | |
| Wind reactions based on MWFRS | | | | | |
| A Brg Wid = 6.0 Min Req = 1.5 (Truss) | | | | | |
| AV Brg Wid = 288 Min Req = - | | | | | |
| CK Brg Wid = 6.0 Min Req = 1.5 (Truss) | | | | | |
| Bearings A, AV, & CK are a rigid surface. | | | | | |
| Members not listed have forces less than 375# | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP #2; B1 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3;
 Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 71 plf at 0.00 to 71 plf at 12.72
 TC: From 75 plf at 12.72 to 75 plf at 39.45
 TC: From 71 plf at 39.45 to 71 plf at 52.17
 BC: From 20 plf at 0.00 to 20 plf at 52.17
 BC: 317 lb Conc. Load at 5.06
 BC: 352 lb Conc. Load at 7.06, 9.06, 11.06, 13.06
 15.06, 17.06, 43.10
 BC: 648 lb Conc. Load at 45.10

Wind
 Wind loads and reactions based on MWFRS.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/134.

| Maximum Top Chord Forces Per Ply (lbs) | | | |
|--|------------|--------|-------------|
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| A - B | 625 -2241 | O - T | 875 -239 |
| B - C | 437 -1555 | T - Z | 620 -169 |
| C - D | 139 -490 | Z - AA | 656 -168 |
| D - I | 2602 -719 | AA-AB | 1150 -285 |
| I - J | 2412 -667 | AB-AC | 125 -509 |
| J - O | 875 -239 | | |

| Maximum Bot Chord Forces Per Ply (lbs) | | | |
|--|------------|---------|-------------|
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| A - BC | 2136 -595 | AK - AI | 165 -646 |
| BC - BB | 2135 -596 | AI - AF | 165 -646 |
| BB - BA | 1454 -408 | AF - AE | 262 -1063 |
| AV - AU | 495 -1783 | AE - AD | 435 -107 |
| AU - AP | 495 -1783 | AD - AC | 473 -113 |
| AP - AK | 131 -470 | | |

Plating Notes
 All plates are 2X4 except as noted.

Loading
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 4.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24' oc.



| Maximum Web Forces Per Ply (lbs) | | | |
|----------------------------------|------------|---------|-------------|
| Webs | Tens.Comp. | Webs | Tens. Comp. |
| B - BB | 196 -713 | J - AP | 1616 -448 |
| C - BB | 999 -273 | O - AP | 253 -957 |
| C - BA | 398 -1421 | AF - AA | 600 -139 |
| BA - D | 1896 -530 | AA - AE | 268 -1062 |
| D - AV | 759 -2738 | AE - AB | 387 -1563 |
| AV - J | 500 -1823 | AB - AD | 515 -74 |

| Maximum Gable Forces Per Ply (lbs) | | | |
|------------------------------------|------------|--------|-------------|
| Gables | Tens.Comp. | Gables | Tens. Comp. |
| BK - AU | 127 -487 | I - BK | 104 -401 |

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| | | | | |
|--------------|------|--------|---------------------|----------------------------------|
| SEQN: 722272 | GABL | Ply: 1 | Job Number: 25-3040 | Cust: R215 JRef: 1YGX2150009 T23 |
| FROM: RFG | | Qty: 1 | ANDERSON PROJECT | DrwNo: 019.26.1551.08063 |
| Page 2 of 2 | | | Truss Label: A1BE | NW / DF 01/19/2026 |

Gable Reinforcement

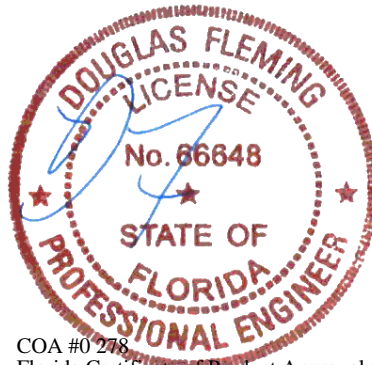
- (b) 2x6 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (c) 1x4 SP/DF #2 or better "L" reinforcement. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (d) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (e) 2x3 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (f) 2x4 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

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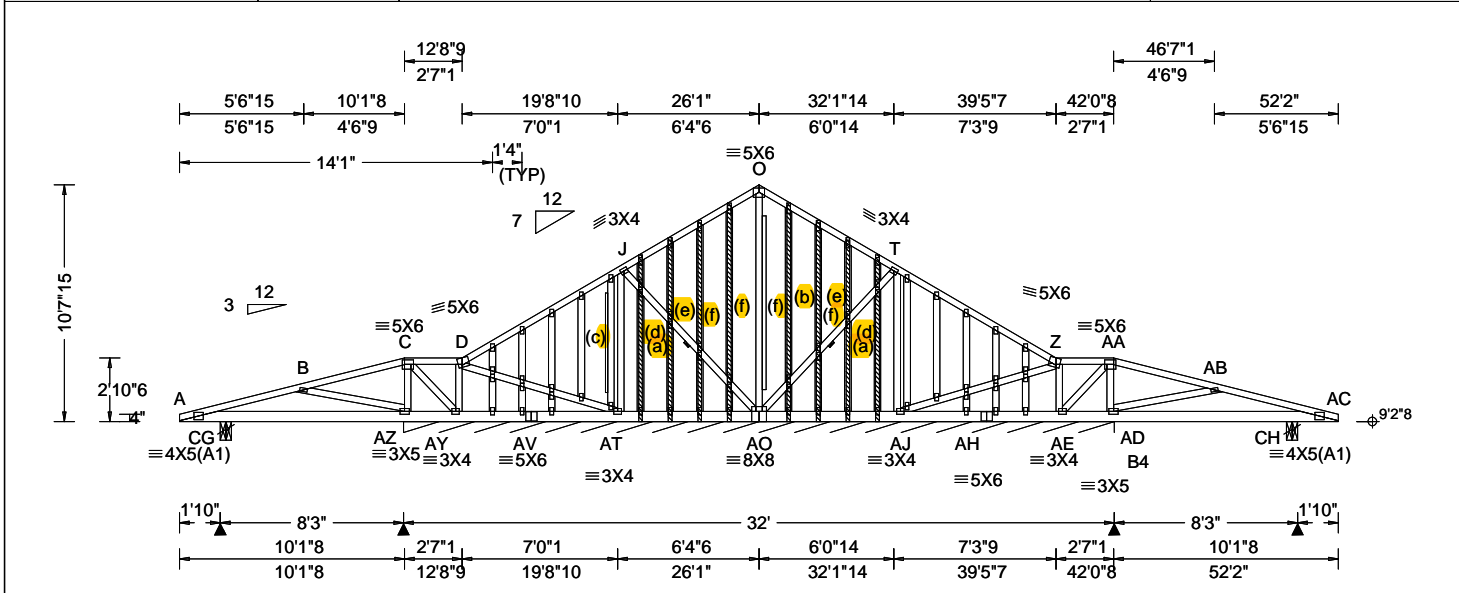
The overall height of this truss excluding overhang is 10-7-15.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| | | | | |
|--|---|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.22 ft Loc. from endwall: not in 6.06 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.027 B 999 240 VERT(CL): 0.065 B 999 180 HORZ(LL): -0.006 C - - HORZ(TL): 0.013 C - - Creep Factor: 2.0 Max TC CSI: 0.900 Max BC CSI: 0.791 Max Web CSI: 0.963 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL CG 516 /- /- /- /96 /- AZ* 245 /- /- /- /53 /- CH 723 /- /- /- /149 /- AY /-527 AE /-462 Wind reactions based on MWFRS CG Brg Wid = 6.0 Min Req = 1.5 (Truss) AZ Brg Wid = 384 Min Req = - CH Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings CG, AZ, & CH are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1126 -358 AA-AB 814 -275 C - D 531 -169 AB-AC 126 -614 Z -AA 400 -135 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A -AZ 643 -45 AV-AT 157 -486 AZ-AY 334 -1046 AE-AD 253 -744 AY-AV 157 -486 AD-AC 1149 -210 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B -AZ 370 -1261 D -AT 447 -138 C -AZ 355 -1148 AE-AA 508 -175 C -AY 760 -243 AA-AD 284 -887 AY -D 105 -378 AD-AB 372 -1270 |
|--|---|--|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2; B4 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 71 plf at 0.00 to 71 plf at 12.72
TC: From 75 plf at 12.72 to 75 plf at 39.45
TC: From 71 plf at 39.45 to 71 plf at 52.17
BC: From 20 plf at 0.00 to 20 plf at 52.17
BC: 1052 lb Conc. Load at 8.90
BC: 352 lb Conc. Load at 43.10
BC: 648 lb Conc. Load at 45.10

Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/134.

Plating Notes
All plates are 2X4 except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 4.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.



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| | | | | | | |
|--------------|------|--------|---------------------|--------------------------|-------------------|------------|
| SEQN: 722257 | GABL | Ply: 1 | Job Number: 25-3040 | Cust: R215 | JRef: 1YGX2150009 | T34 |
| FROM: RFG | | Qty: 1 | ANDERSON PROJECT | DrwNo: 019.26.1551.31073 | | |
| Page 2 of 2 | | | Truss Label: A1E | NW / DF | | 01/19/2026 |

Gable Reinforcement

(b) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

(c) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

(d) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.

(e) 2x3 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.

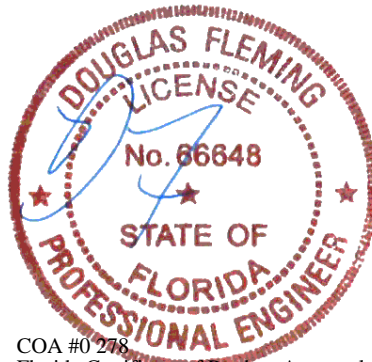
(f) 2x4 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

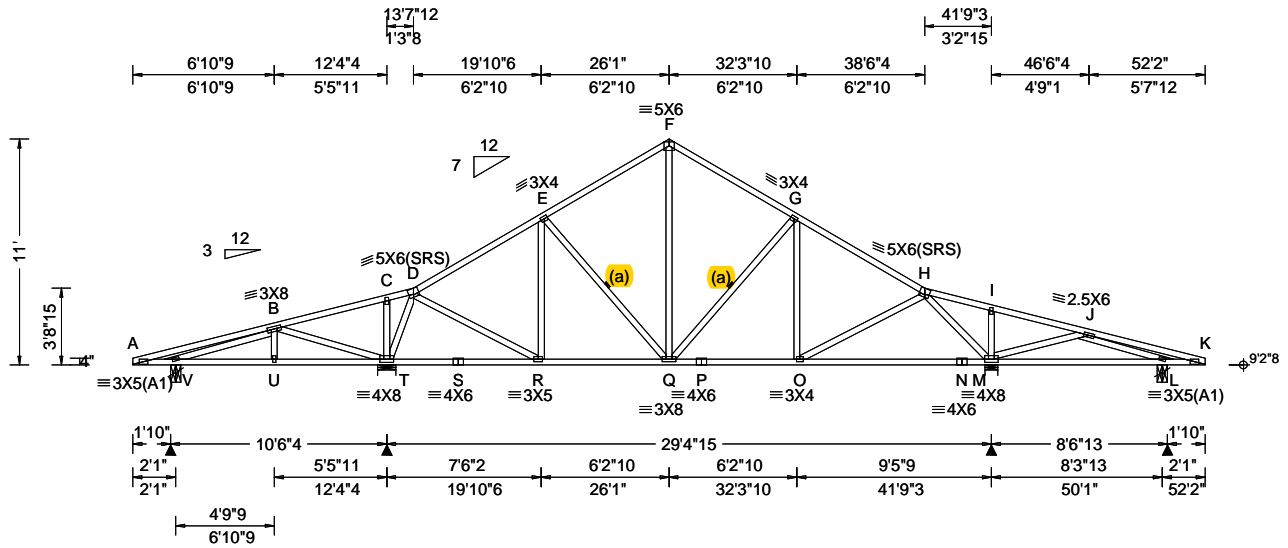
The overall height of this truss excluding overhang is 10-7-15.



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| Loading Criteria (psf) TCCL: 20.00 TCCL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.22 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.034 G 999 240 VERT(CL): 0.080 G 999 180 HORZ(LL): 0.012 M - - HORZ(TL): 0.029 M - - Creep Factor: 2.0 Max TC CSI: 0.696 Max BC CSI: 0.688 Max Web CSI: 0.848 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>V</td> <td>481</td> <td>-</td> <td>-</td> <td>/230</td> <td>/131</td> <td>/289</td> </tr> <tr> <td>T</td> <td>2106</td> <td>-</td> <td>-</td> <td>/1378</td> <td>/252</td> <td>-</td> </tr> <tr> <td>M</td> <td>2011</td> <td>-</td> <td>-</td> <td>/1277</td> <td>/245</td> <td>-</td> </tr> <tr> <td>L</td> <td>401</td> <td>-</td> <td>-</td> <td>/173</td> <td>/102</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS V Brg Wid = 6.0 Min Req = 1.5 (Truss) T Brg Wid = 10.7 Min Req = 2.5 (Truss) M Brg Wid = 7.6 Min Req = 2.4 (Truss) L Brg Wid = 6.0 Min Req = 1.5 (Truss)</p> <p>Bearings V, T, M, & L are a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | V | 481 | - | - | /230 | /131 | /289 | T | 2106 | - | - | /1378 | /252 | - | M | 2011 | - | - | /1277 | /245 | - | L | 401 | - | - | /173 | /102 | - |
|--|--|--|---|--|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|------|------|---|------|---|---|-------|------|---|---|------|---|---|-------|------|---|---|-----|---|---|------|------|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| V | 481 | - | - | /230 | /131 | /289 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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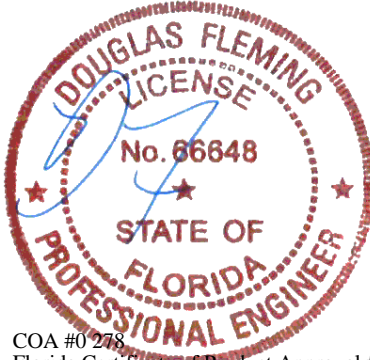
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 11-0-0.



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Maximum Top Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - B | 546 -416 | F - G | 474 -1029 |
| B - C | 801 -287 | G - H | 443 -1291 |
| C - D | 780 -270 | H - I | 802 -302 |
| D - E | 375 -1100 | I - J | 804 -332 |
| E - F | 475 -1029 | J - K | 475 -493 |

Maximum Bot Chord Forces Per Ply (lbs)

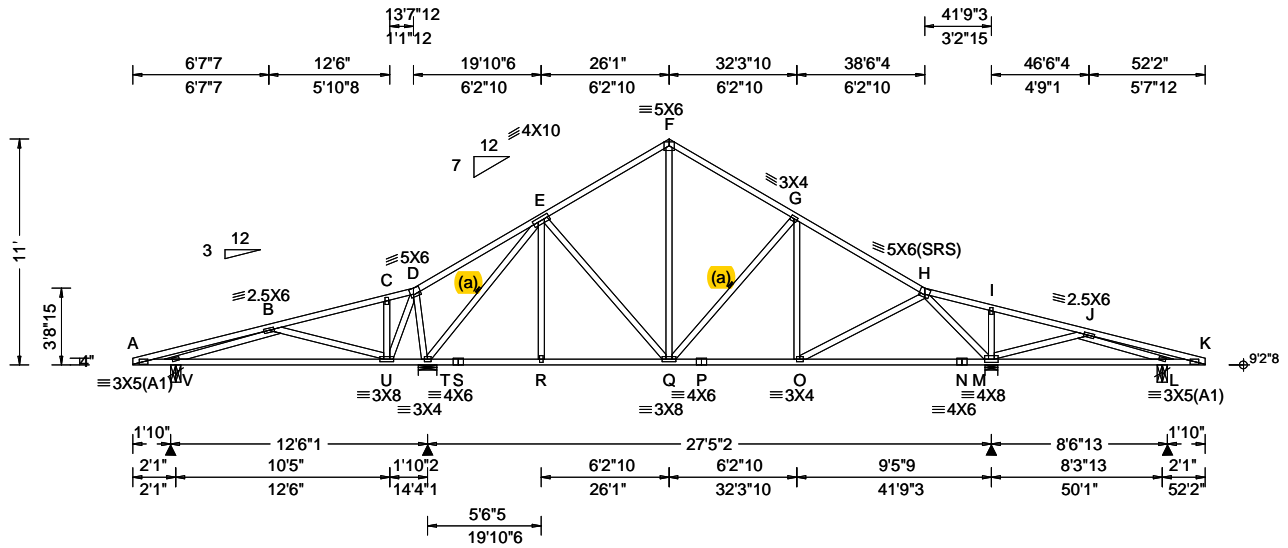
| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - V | 447 -470 | O - N | 635 -113 |
| R - Q | 868 -122 | N - M | 635 -113 |
| Q - P | 1024 -164 | L - K | 517 -411 |
| P - O | 1024 -164 | | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| V - B | 443 -761 | Q - G | 232 -388 |
| B - T | 407 -910 | O - H | 529 -66 |
| T - D | 629 -1598 | H - M | 716 -1895 |
| D - R | 1132 -274 | M - J | 430 -692 |
| R - E | 181 -420 | J - L | 524 -540 |
| F - Q | 499 -248 | | |

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|--|--|--|---|--|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|------|------|---|------|---|---|-------|------|---|---|------|---|---|-------|------|---|---|-----|---|---|------|------|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| V | 627 | - | - | /345 | /135 | /289 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| T | 1990 | - | - | /1287 | /256 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| M | 1931 | - | - | /1231 | /227 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| L | 408 | - | - | /180 | /105 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
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 The overall height of this truss excluding overhang is 11-0-0.

Maximum Top Chord Forces Per Ply (lbs)

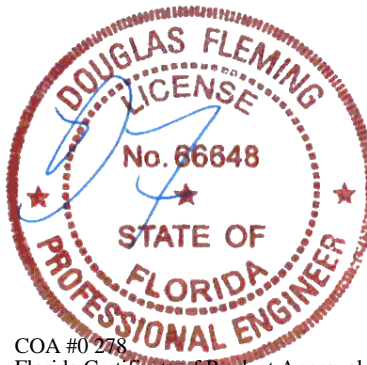
| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - B | 369 -516 | G - H | 399 -1199 |
| D - E | 635 -161 | H - I | 754 -279 |
| E - F | 422 -935 | I - J | 755 -310 |
| F - G | 421 -933 | J - K | 474 -493 |

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - V | 542 -300 | Q - P | 945 -136 |
| V - U | 618 -263 | P - O | 945 -136 |
| T - S | 676 -84 | O - N | 600 -103 |
| S - R | 676 -84 | N - M | 600 -103 |
| R - Q | 677 -84 | L - K | 516 -410 |

Maximum Web Forces Per Ply (lbs)

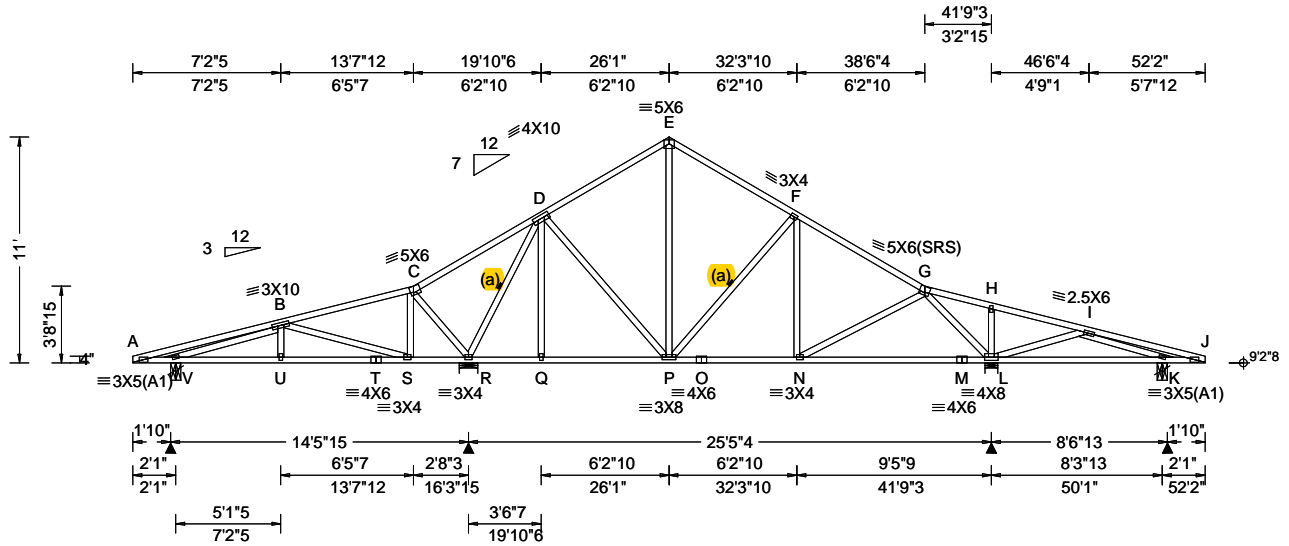
| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| V - B | 730 -972 | Q - G | 243 -407 |
| B - U | 447 -733 | O - H | 478 -44 |
| U - D | 646 -302 | H - M | 661 -1791 |
| D - T | 323 -637 | M - J | 426 -683 |
| T - E | 532 -1698 | J - L | 542 -559 |
| F - Q | 420 -191 | | |



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| | | | | |
|--|--|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.22 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.027 U 999 240 VERT(CL): 0.064 U 999 180 HORZ(LL): 0.012 K - - HORZ(TL): 0.030 L - - Creep Factor: 2.0 Max TC CSI: 0.756 Max BC CSI: 0.667 Max Web CSI: 0.859 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity V 649 -/ - / - /350 /146 /289 R 2127 -/ - / - /1384 /259 -/ L 1770 -/ - / - /1134 /210 -/ K 425 -/ - / - /198 /107 -/ Wind reactions based on MWFRS V Brg Wid = 6.0 Min Req = 1.5 (Truss) R Brg Wid = 10.7 Min Req = 2.5 (Truss) L Brg Wid = 7.6 Min Req = 2.1 (Truss) K Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings V, R, L, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 516 -408 F - G 335 -1030 C - D 854 -226 G - H 666 -252 D - E 354 -732 H - I 667 -282 E - F 346 -731 I - J 472 -492 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - V 442 -438 O - N 798 -97 V - U 713 -241 N - M 524 -87 U - T 714 -240 M - L 524 -87 T - S 714 -240 K - J 515 -408 P - O 798 -97 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. V - B 605 -1217 P - F 257 -454 B - S 461 -949 N - G 379 -18 S - C 411 -102 G - L 585 -1579 C - R 367 -634 L - I 420 -663 R - D 556 -1766 I - K 564 -601 D - P 483 -62 |
|--|--|--|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

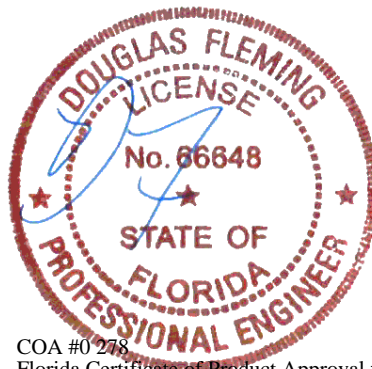
Left and right cantilevers are exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

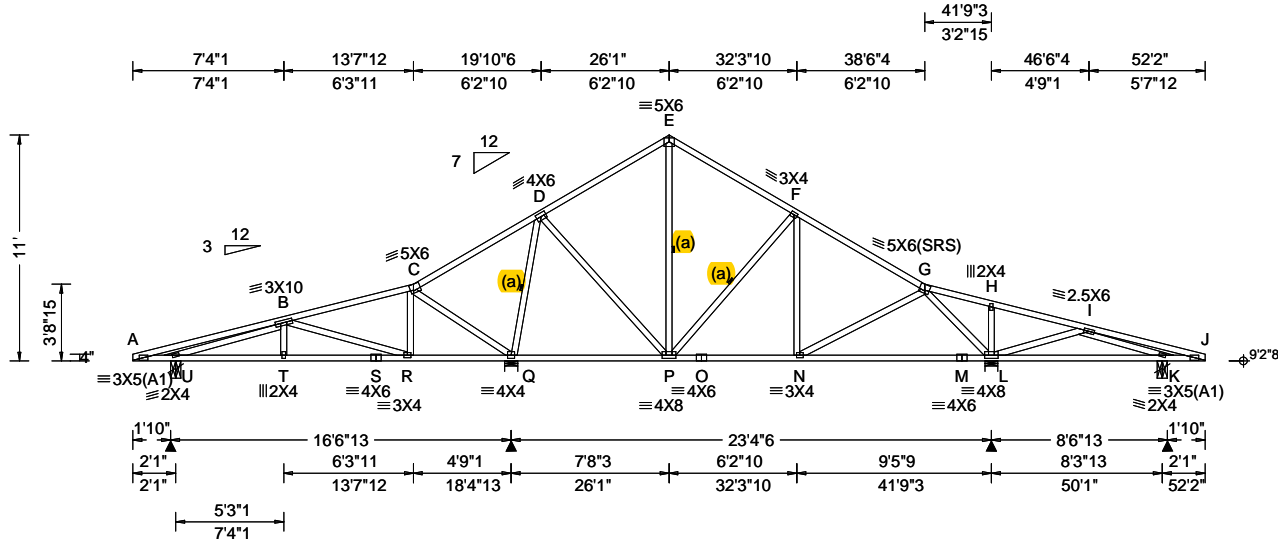
The overall height of this truss excluding overhang is 11-0-0.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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|--|--|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.22 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.034 T 999 240 VERT(CL): 0.075 T 999 180 HORZ(LL): 0.011 K - - HORZ(TL): 0.024 L - - Creep Factor: 2.0 Max TC CSI: 0.769 Max BC CSI: 0.654 Max Web CSI: 0.984 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL U 693 - / - / - /381 /172 /289 Q 2405 - / - / - /1456 /624 -/ L 1700 - / - / - /1042 /438 -/ K 426 - / - / - /210 /108 -/ Wind reactions based on MWFRS U Brg Wid = 6.0 Min Req = 1.5 (Truss) Q Brg Wid = 7.6 Min Req = 2.8 (Truss) L Brg Wid = 7.6 Min Req = 2.0 (Truss) K Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings U, Q, L, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 510 -540 F - G 328 -955 C - D 887 -346 G - H 638 -257 D - E 309 -591 H - I 638 -317 E - F 311 -593 I - J 467 -484 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - U 589 -431 P - O 730 -2 U - T 835 -414 O - N 730 -2 T - S 836 -413 N - M 468 -19 S - R 836 -413 M - L 468 -19 Q - P 457 -389 K - J 506 -403 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. U - B 893 -1327 D - P 938 -247 B - R 634 -923 P - F 331 -539 R - C 407 -165 G - L 590 -1493 C - Q 490 -713 L - I 563 -658 Q - D 767 -1788 I - K 627 -600 |
|--|--|---|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

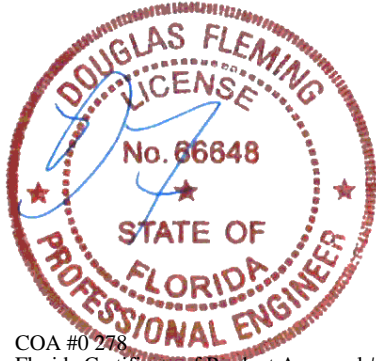
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

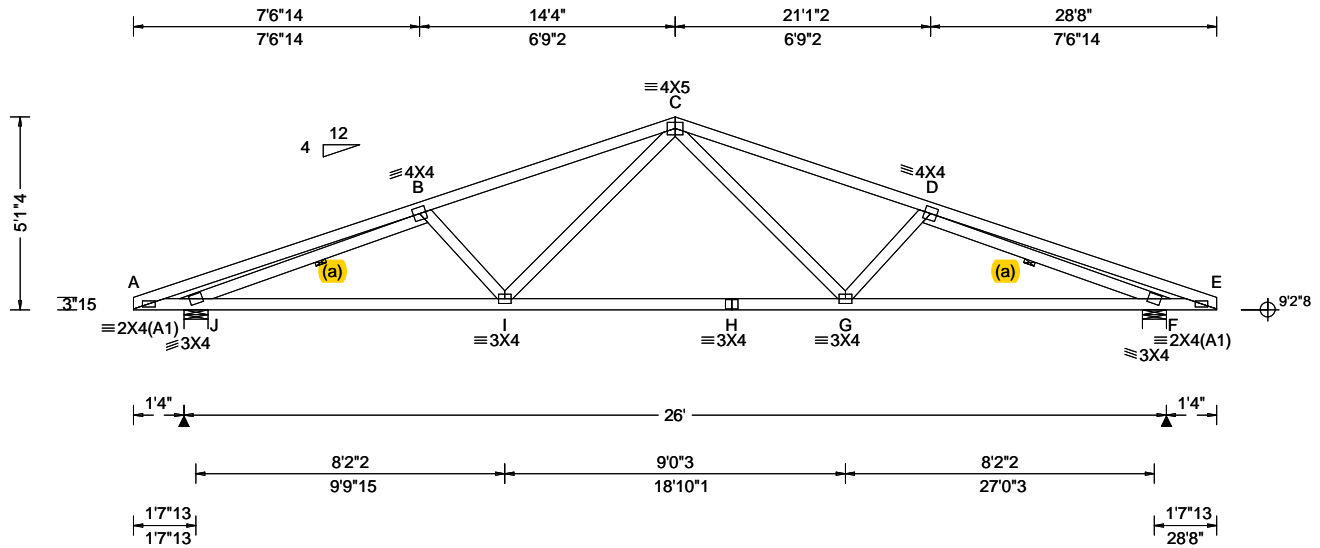
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss excluding overhang is 11'-0".



COA #0278
Florida Certificate of Product Approval #FL1999
01/19/2026

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|---|---|---|---|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.099 C 999 240 VERT(CL): 0.226 C 999 180 HORZ(LL): 0.035 F - - HORZ(TL): 0.080 F - - Creep Factor: 2.0 Max TC CSI: 0.751 Max BC CSI: 0.818 Max Web CSI: 0.741 VIEW Ver: 24.02.00D.0114.10 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 1315 /- /- /747 /365 /90 F 1315 /- /- /747 /365 /- Wind reactions based on MWFRS J Brg Wid = 7.6 Min Req = 1.6 (Truss) F Brg Wid = 7.6 Min Req = 1.6 (Truss) Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 773 -2116 C - D 773 -2117 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

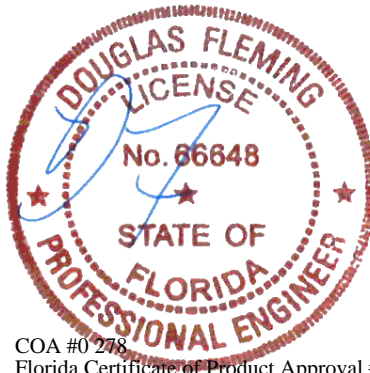
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

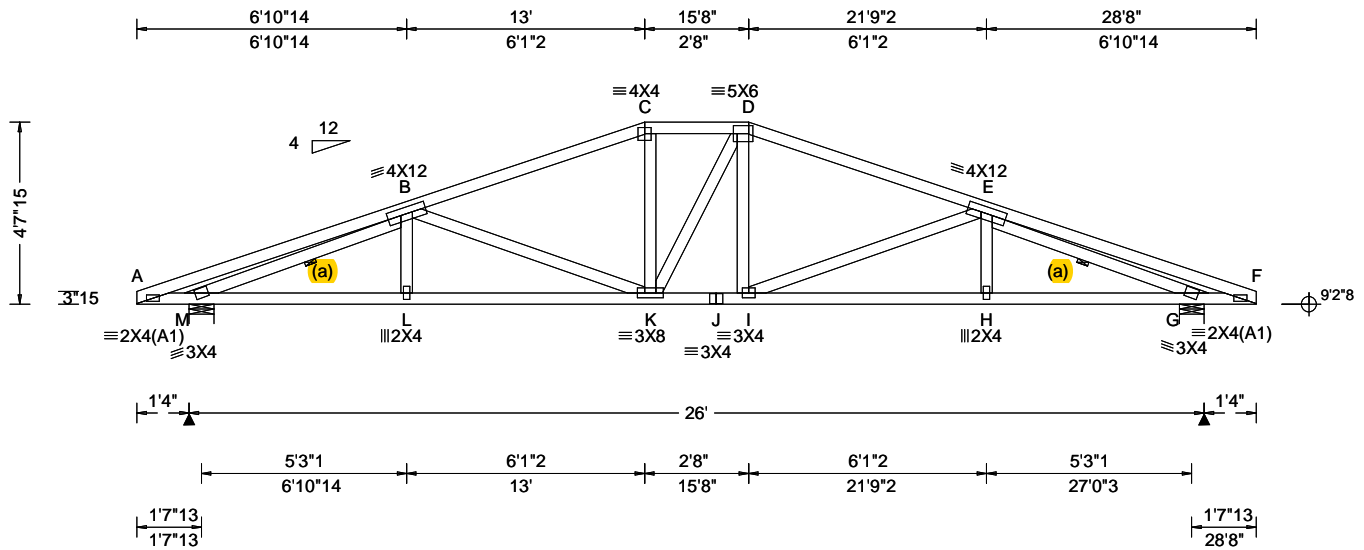
The overall height of this truss excluding overhang is 5-1-4.



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|--|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.097 I 999 240 VERT(CL): 0.222 I 999 180 HORZ(LL): 0.038 G - - HORZ(TL): 0.088 G - - Creep Factor: 2.0 Max TC CSI: 0.634 Max BC CSI: 0.854 Max Web CSI: 0.644 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1315 /- /- /745 /366 /82 G 1315 /- /- /745 /366 /- Wind reactions based on MWFRS M Brg Wid = 7.6 Min Req = 1.6 (Truss) G Brg Wid = 7.6 Min Req = 1.6 (Truss) Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 829 -1848 D - E 831 -1855 C - D 834 -1687 |
|--|---|--|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

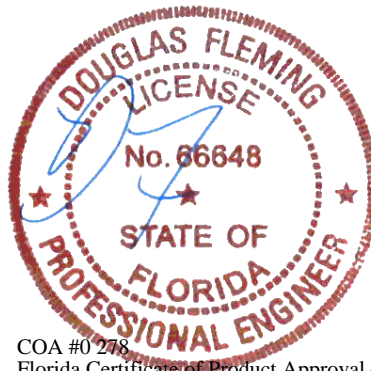
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

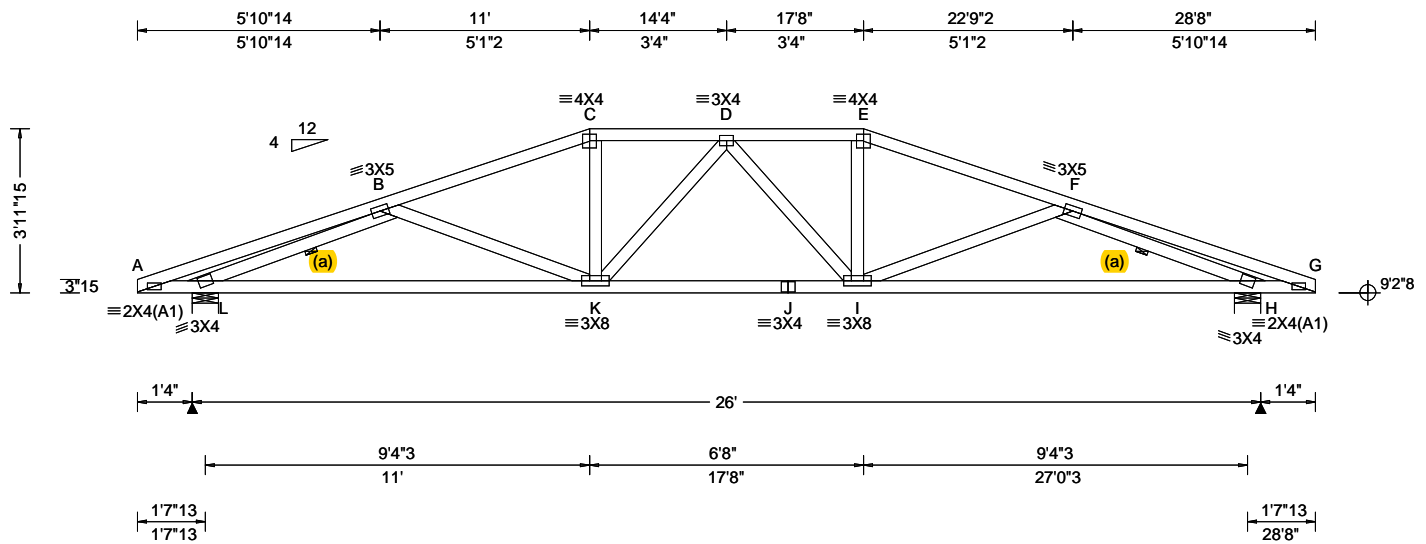
The overall height of this truss excluding overhang is 4-7-15.



COA #0278
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|---|---|---|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.098 D 999 240 VERT(CL): 0.224 D 999 180 HORZ(LL): 0.037 H - - HORZ(TL): 0.085 H - - Creep Factor: 2.0 Max TC CSI: 0.440 Max BC CSI: 0.955 Max Web CSI: 0.533 VIEW Ver: 24.02.00D.0114.10 | Gravity Loc R+ / R- / Rh / Rw / U / RL L 1315 /- /- /741 /367 /69 H 1315 /- /- /741 /367 /- Non-Gravity Wind reactions based on MWFRS L Brg Wid = 7.6 Min Req = 1.6 (Truss) H Brg Wid = 7.6 Min Req = 1.6 (Truss) Bearings L & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 160 -400 D - E 1067 -1884 B - C 1077 -2037 E - F 1077 -2037 C - D 1067 -1884 F - G 160 -400 |

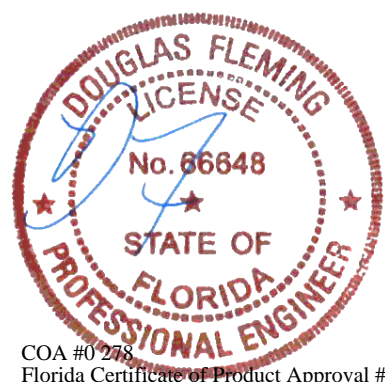
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

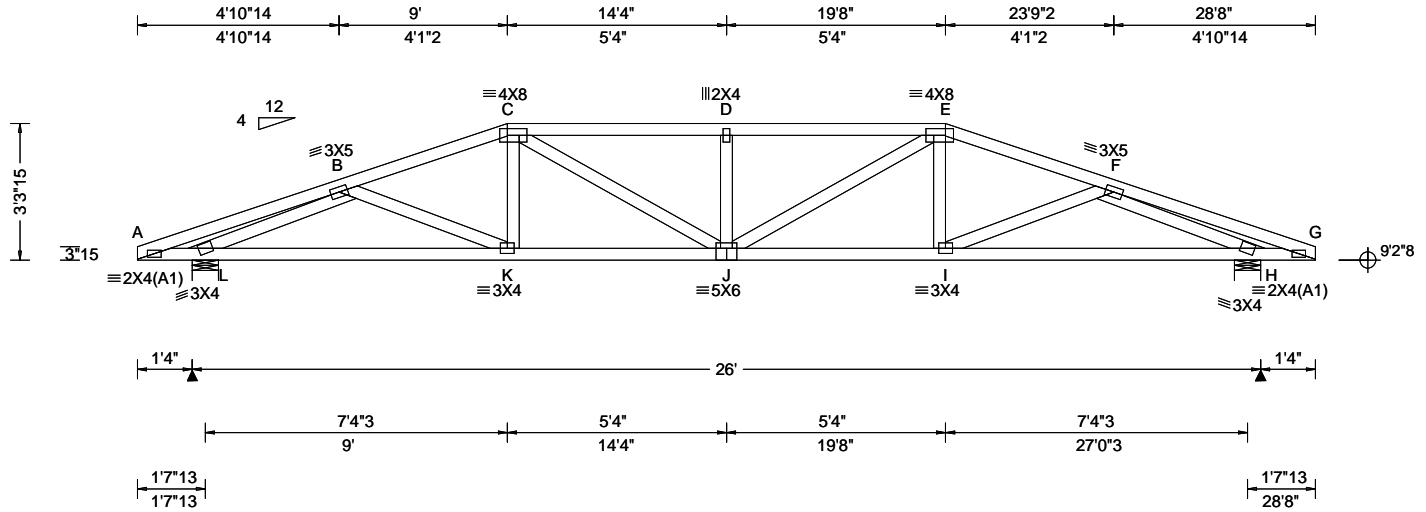
Additional Notes
The overall height of this truss excluding overhang is 3-11-15.



COA #0278
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01/19/2026

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| | | | | |
|---|---|---|---|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.115 D 999 240 VERT(CL): 0.263 D 999 180 HORZ(LL): 0.036 H - - HORZ(TL): 0.082 H - - Creep Factor: 2.0 Max TC CSI: 0.491 Max BC CSI: 0.659 Max Web CSI: 0.880 VIEW Ver: 24.02.00D.0114.10 | Gravity Loc R+ / R- / Rh / Rw / U / RL L 1315 /- /- /738 /368 /57 H 1315 /- /- /738 /368 /- Non-Gravity Wind reactions based on MWFRS L Brg Wid = 7.6 Min Req = 1.6 (Truss) H Brg Wid = 7.6 Min Req = 1.6 (Truss) Bearings L & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1291 -2179 D - E 1584 -2461 C - D 1584 -2461 E - F 1291 -2179 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

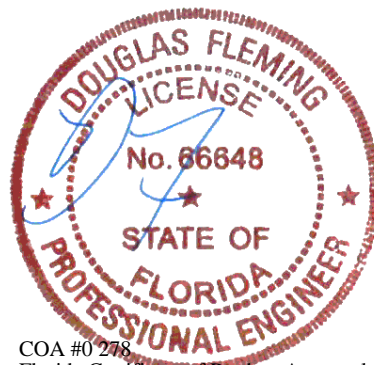
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

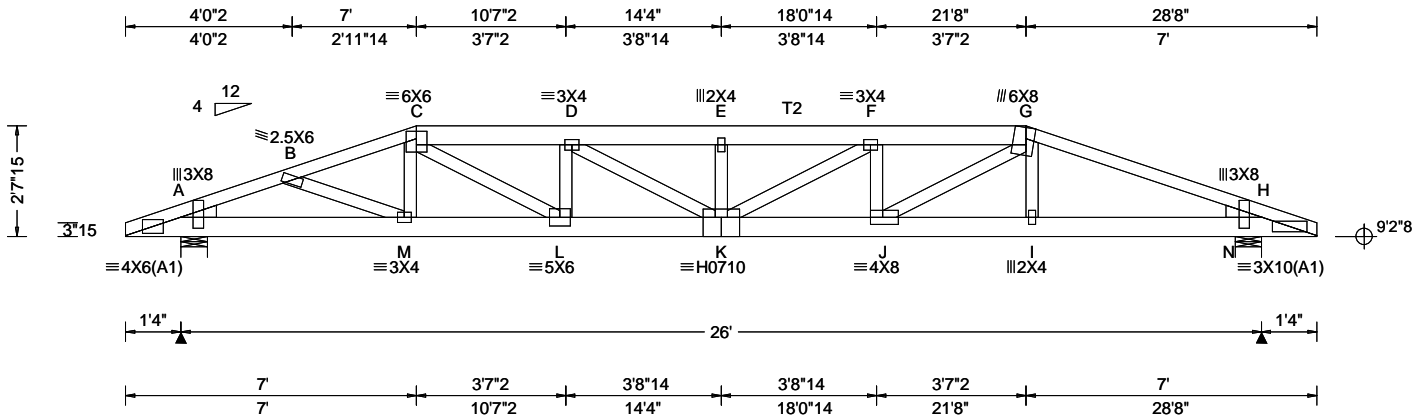
The overall height of this truss excluding overhang is 3-3-15.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





| | | | | |
|--|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.589 E 516 240 VERT(CL): 0.712 E 427 180 HORZ(LL): 0.115 H - - HORZ(TL): 0.139 H - - Creep Factor: 2.0 Max TC CSI: 0.777 Max BC CSI: 0.813 Max Web CSI: 0.989 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 2346 /- /- /- /742 /- N 2346 /- /- /- /742 /- Wind reactions based on MWFRS A Brg Wid = 7.6 Min Req = 1.9 (Truss) N Brg Wid = 7.6 Min Req = 1.9 (Truss) Bearings A & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1264 -3916 E - F 2241 -6953 B - C 1456 -4560 F - G 2033 -6322 C - D 2025 -6295 G - H 1423 -4428 D - E 2241 -6953 |
|--|---|--|---|--|

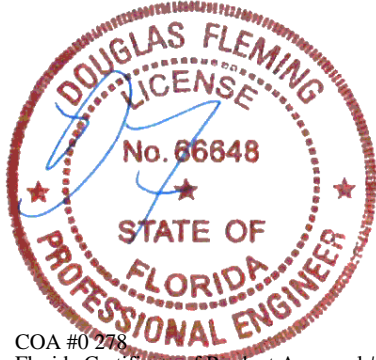
Lumber
Top chord: 2x4 SP M-31; T2 2x6 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 72 plf at 0.00 to 72 plf at 7.00
TC: From 36 plf at 7.00 to 36 plf at 21.67
TC: From 72 plf at 21.67 to 72 plf at 28.67
BC: From 20 plf at 0.00 to 20 plf at 7.03
BC: From 10 plf at 7.03 to 10 plf at 21.64
BC: From 20 plf at 21.64 to 20 plf at 28.67
TC: 211 lb Conc. Load at 7.03,21.64
TC: 161 lb Conc. Load at 9.06,11.06,13.06,14.33
15.60,17.60,19.60
BC: 283 lb Conc. Load at 7.03,21.64
BC: 88 lb Conc. Load at 9.06,11.06,13.06,14.33
15.60,17.60,19.60

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

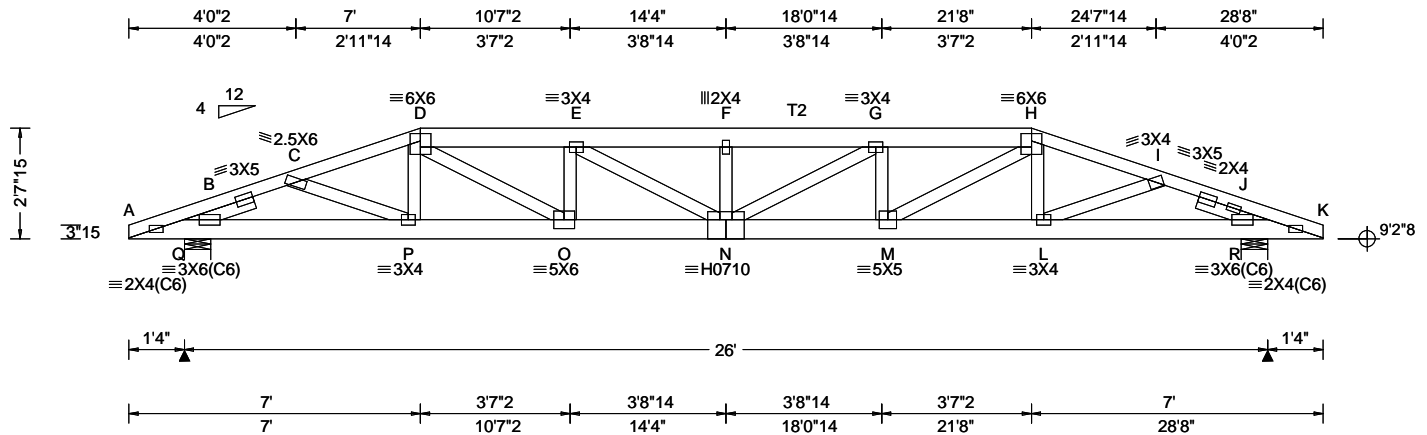
Additional Notes
The overall height of this truss excluding overhang is 2-7-15.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| | | | | |
|--|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.449 F 679 240 VERT(CL): 0.604 F 505 180 HORZ(LL): 0.076 D - - HORZ(TL): 0.109 D - - Creep Factor: 2.0 Max TC CSI: 0.823 Max BC CSI: 0.533 Max Web CSI: 0.986 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Q 2393 /- /- /- /728 /- R 2495 /- /- /- /700 /- Wind reactions based on MWFRS Q Brg Wid = 7.6 Min Req = 2.0 (Truss) R Brg Wid = 7.6 Min Req = 2.1 (Truss) Bearings Q & R are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 304 -1166 F - G 2177 -7253 B - C 1287 -4142 G - H 1927 -6664 C - D 1465 -4802 H - I 1397 -5049 D - E 1976 -6495 I - J 1232 -4343 E - F 2177 -7253 J - K 288 -1227 |
|--|---|--|---|--|

Lumber
Top chord: 2x4 SP M-31; T2 2x6 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.750'
Rt Slider: 2x4 SP #3; block length = 1.750'

Additional Notes
The overall height of this truss excluding overhang is 2-7-15.

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| Q - P | 3744 -1163 | N - M | 6765 -1972 |
| A - Q | 1131 -288 | M - L | 4860 -1353 |
| P - O | 4626 -1417 | L - R | 3925 -1113 |
| O - N | 6606 -2018 | J - K | 1192 -272 |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| C - B | 1052 -3226 | N - G | 580 -240 |
| C - P | 930 -255 | G - M | 354 -793 |
| D - O | 2239 -666 | M - H | 2161 -684 |
| O - E | 330 -865 | L - I | 992 -238 |
| E - N | 767 -186 | J - I | 1011 -3371 |
| F - N | 265 -517 | | |

Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 72 plf at 0.00 to 72 plf at 7.00
TC: From 36 plf at 7.00 to 36 plf at 21.67
TC: From 72 plf at 21.67 to 72 plf at 28.67
BC: From 20 plf at 0.00 to 20 plf at 7.03
BC: From 10 plf at 7.03 to 10 plf at 21.64
BC: From 20 plf at 21.64 to 20 plf at 28.67
TC: 211 lb Conc. Load at 7.03
TC: 161 lb Conc. Load at 9.06,11.06,13.06,14.33
15.60,17.60
TC: 201 lb Conc. Load at 19.60
TC: 277 lb Conc. Load at 21.64
BC: 283 lb Conc. Load at 7.03
BC: 88 lb Conc. Load at 9.06,11.06,13.06,14.33
15.60,17.60
BC: 112 lb Conc. Load at 19.60
BC: 348 lb Conc. Load at 21.64

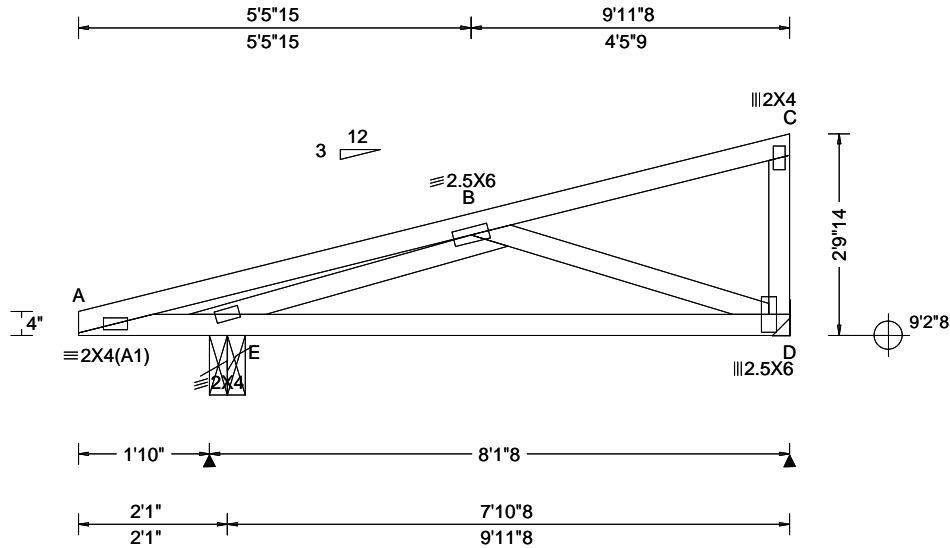
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.



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| | | | | |
|--|---|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 B 999 240 VERT(CL): 0.024 B 999 180 HORZ(LL): 0.003 D - - HORZ(TL): 0.007 D - - Creep Factor: 2.0 Max TC CSI: 0.453 Max BC CSI: 0.550 Max Web CSI: 0.358 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E 572 -/- /- /311 /158 /102 D 344 -/- /- /201 /116 -/ Wind reactions based on MWFRS E Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 390 -703 |
|--|---|--|---|---|

Lumber

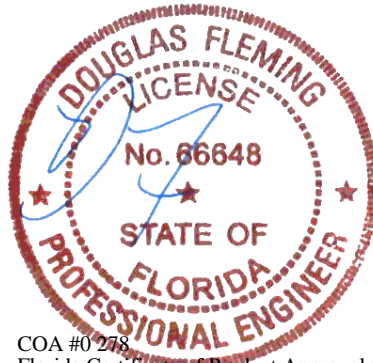
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

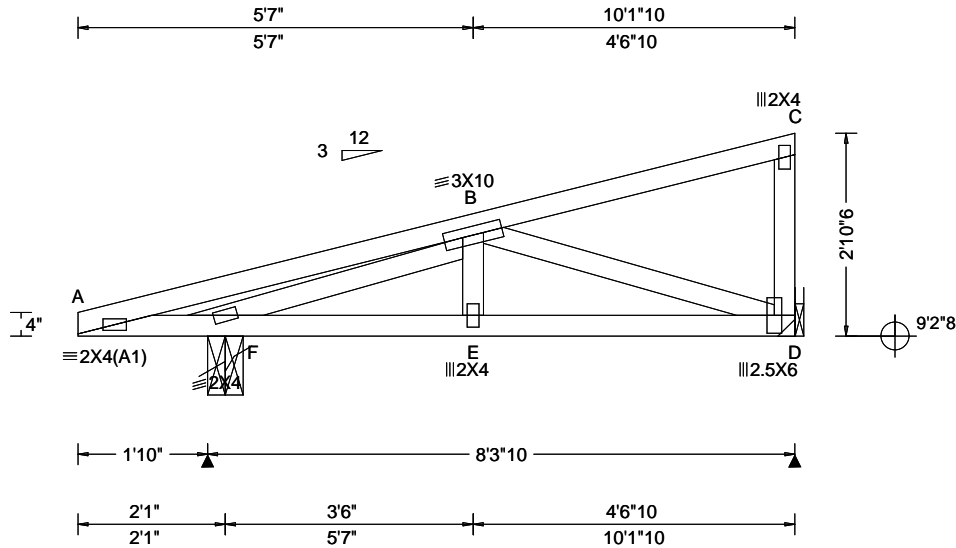
The overall height of this truss excluding overhang is 2-9-14.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| | | | | | | | | | |
|--|---|--|---|--|--|--|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.012 E 999 240 VERT(CL): 0.027 E 999 180 HORZ(LL): 0.003 D - - HORZ(TL): 0.008 D - - Creep Factor: 2.0 Max TC CSI: 0.487 Max BC CSI: 0.347 Max Web CSI: 0.414 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 580 /- /- /316 /160 /104 D 352 /- /- /206 /119 /- Wind reactions based on MWFRS F Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. | | | | | |
| | | | | A - B 485 -567 | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties

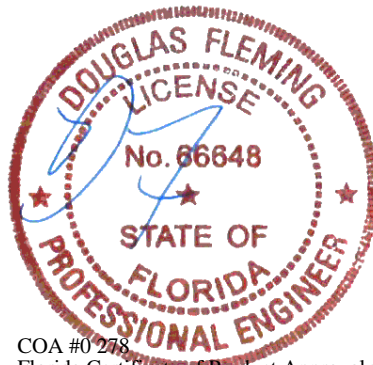
Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

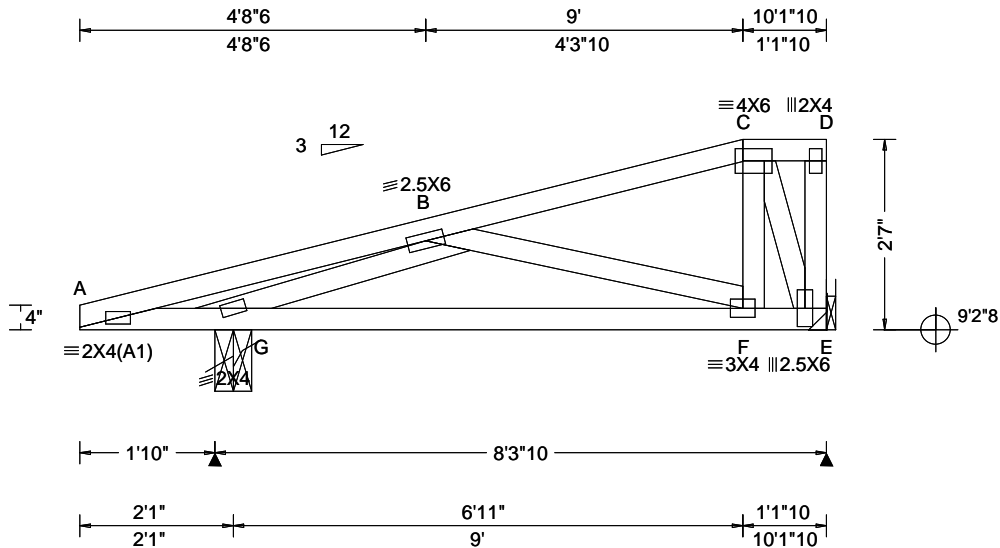
The overall height of this truss excluding overhang is 2-10-7.



COA #0278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| Loading Criteria (psf) TCLL: 20.00 TC DL: 15.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 45.00 NCBC LL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BC DL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 B 999 240 VERT(CL): 0.021 B 999 180 HORZ(LL): 0.003 E - - HORZ(TL): 0.007 E - - Creep Factor: 2.0 Max TC CSI: 0.389 Max BC CSI: 0.356 Max Web CSI: 0.298 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>580</td> <td>-</td> <td>-</td> <td>/316</td> <td>/163</td> <td>/92</td> </tr> <tr> <td>E</td> <td>352</td> <td>-</td> <td>-</td> <td>/205</td> <td>/116</td> <td>-</td> </tr> </tbody> </table> | | | | | | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | G | 580 | - | - | /316 | /163 | /92 | E | 352 | - | - | /205 | /116 | - |
|--|---|--|---|--|------------|--------|------------|-------------|-----|------|---------|---|--|-------------|------------|--------|-------------|-------|-----|------|----------------|-----|---|-----|---|---|------|------|-----|---|-----|---|---|------|------|---|
| | | | | Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 580 | - | - | /316 | /163 | /92 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 352 | - | - | /205 | /116 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>404</td> <td>-615</td> <td></td> </tr> </tbody> </table> | | | | Chords | Tens.Comp. | Chords | Tens.Comp. | A - B | 404 | -615 | | Maximum Bot Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - G</td> <td>641</td> <td>-349</td> <td>G - F 555 -571</td> </tr> </tbody> </table> | | Chords | Tens.Comp. | Chords | Tens. Comp. | A - G | 641 | -349 | G - F 555 -571 | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens.Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 404 | -615 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - G | 641 | -349 | G - F 555 -571 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties

Hanger Support Required, by others

Purlins

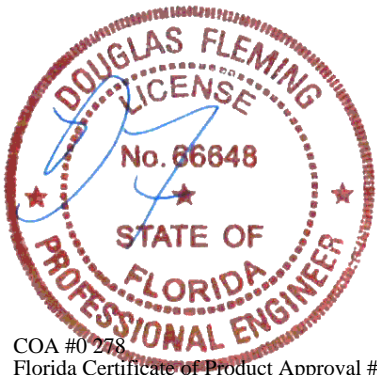
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

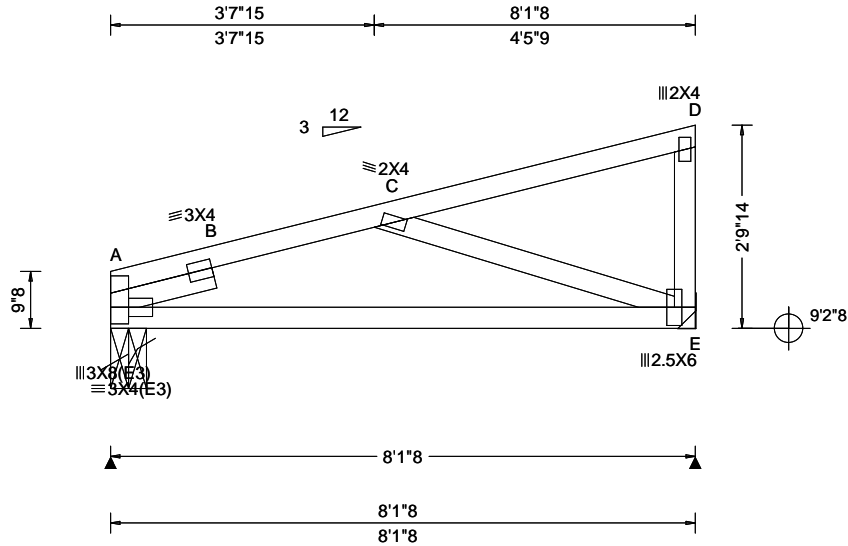
The overall height of this truss excluding overhang is 2'-7-0.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| | | | | |
|--|---|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.011 B 999 240 VERT(CL): 0.026 B 999 180 HORZ(LL): 0.004 B - - HORZ(TL): 0.012 B - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.636 Max Web CSI: 0.297 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 369 -/ - /211 /92 /83 E 369 -/ - /212 /122 /- Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 493 -710 B - C 467 -563 |
|--|---|--|---|---|

Lumber

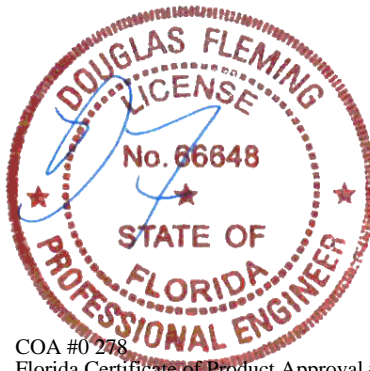
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

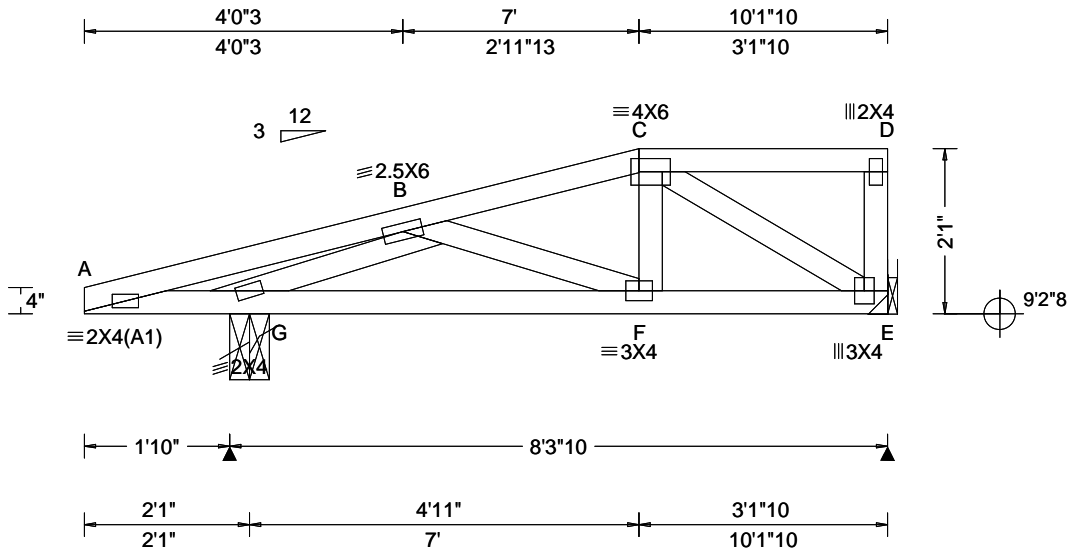
The overall height of this truss excluding overhang is 2-9-14.



COA #0278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| | | | | |
|--|--|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCCL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.011 F 999 240 VERT(CL): 0.024 F 999 180 HORZ(LL): 0.004 E - - HORZ(TL): 0.009 E - - Creep Factor: 2.0 Max TC CSI: 0.409 Max BC CSI: 0.314 Max Web CSI: 0.337 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 863 /- /- /- /216 /- E 648 /- /- /- /166 /- Wind reactions based on MWFRS G Brg Wid = 6.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. |
| | | | | A - B 757 -225 B - C 185 -814 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - G 202 -699 F - E 758 -164 G - F 638 -197 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - B 454 -1479 C - E 184 -859 |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 71 plf at 0.00 to 71 plf at 7.00
 TC: From 35 plf at 7.00 to 35 plf at 10.14
 BC: From 20 plf at 0.00 to 20 plf at 7.03
 BC: From 10 plf at 7.03 to 10 plf at 10.14
 TC: 101 lb Conc. Load at 0.00
 TC: 166 lb Conc. Load at 7.03
 TC: 145 lb Conc. Load at 9.06
 BC: 233 lb Conc. Load at 7.03
 BC: 75 lb Conc. Load at 9.06

Hangers / Ties

Hanger Support Required, by others

Purlins

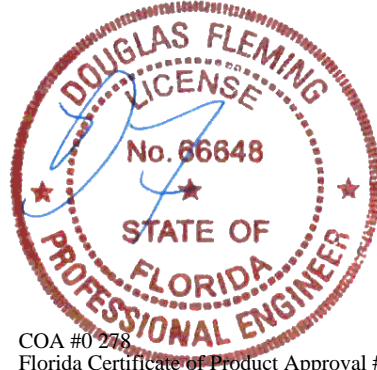
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

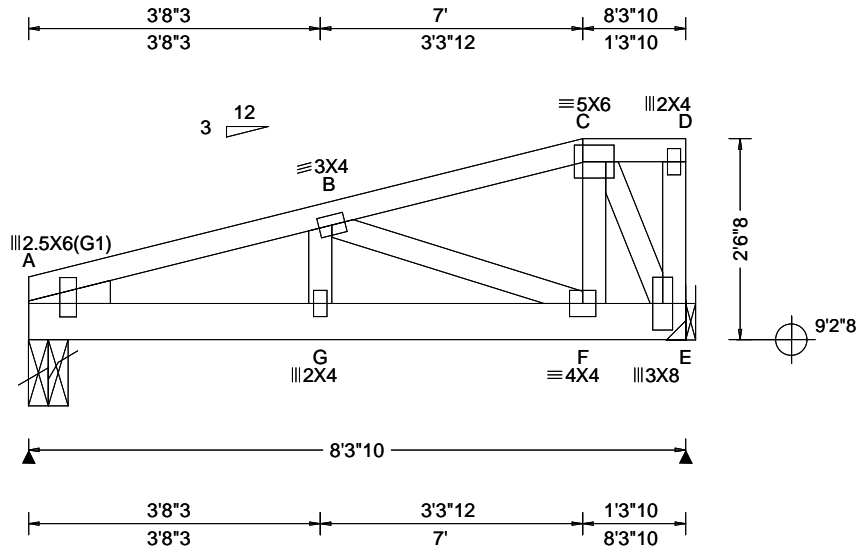
The overall height of this truss excluding overhang is 2-1-0.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| Loading Criteria (psf) TCCL: 20.00 TCCL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.018 B 999 240 VERT(CL): 0.028 G 999 180 HORZ(LL): 0.004 E - - HORZ(TL): 0.006 E - - Creep Factor: 2.0 Max TC CSI: 0.191 Max BC CSI: 0.234 Max Web CSI: 0.359 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>499</td> <td>-</td> <td>-</td> <td>-</td> <td>/145</td> <td>-</td> </tr> <tr> <td>E</td> <td>1052</td> <td>-</td> <td>-</td> <td>-</td> <td>/306</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#</p> | | | | | | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 499 | - | - | - | /145 | - | E | 1052 | - | - | - | /306 | - |
|--|--|--|---|--|---------|-----|--|-------------|--|--------|------------|--------|-------------|-------------|-----|-------|-----|-----|-----|-----|----|-----|---|-----|---|---|---|------|---|---|------|---|---|---|------|---|
| | | | | Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 499 | - | - | - | /145 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 1052 | - | - | - | /306 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>296</td> <td>B - C</td> <td>182</td> </tr> </tbody> </table> | | | | | | Chords | Tens.Comp. | Chords | Tens. Comp. | A - B | 296 | B - C | 182 | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 296 | B - C | 182 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

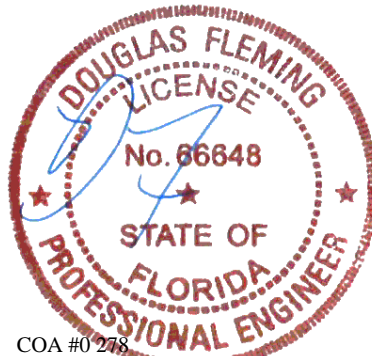
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP #2;
 Webs: 2x4 SP #3;
 Lt Stub Wedge: 2x4 SP #3;

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 71 plf at 0.00 to 71 plf at 8.30
 BC: From 20 plf at 0.00 to 20 plf at 8.30
 BC: 796 lb Conc. Load at 7.03

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 2-6-8.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| | | | |
|--|--------------------------|--|---|
| SEQN: 722231 FROM: RFG Page 2 of 2 | HIPM Ply: 1 Qty: 1 | Job Number: 25-3040 ANDERSON PROJECT Truss Label: EJ3A | Cust: R215 JRef:1YGX2150009 T29 DrwNo: 019.26.1554.13830 NW / DF 01/19/2026 |
|--|--------------------------|--|---|

Hangers / Ties

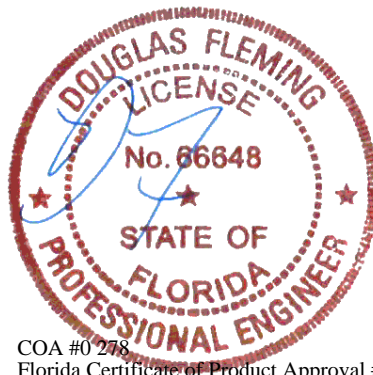
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=8'0"10 uses the following support conditions: 8'0"10

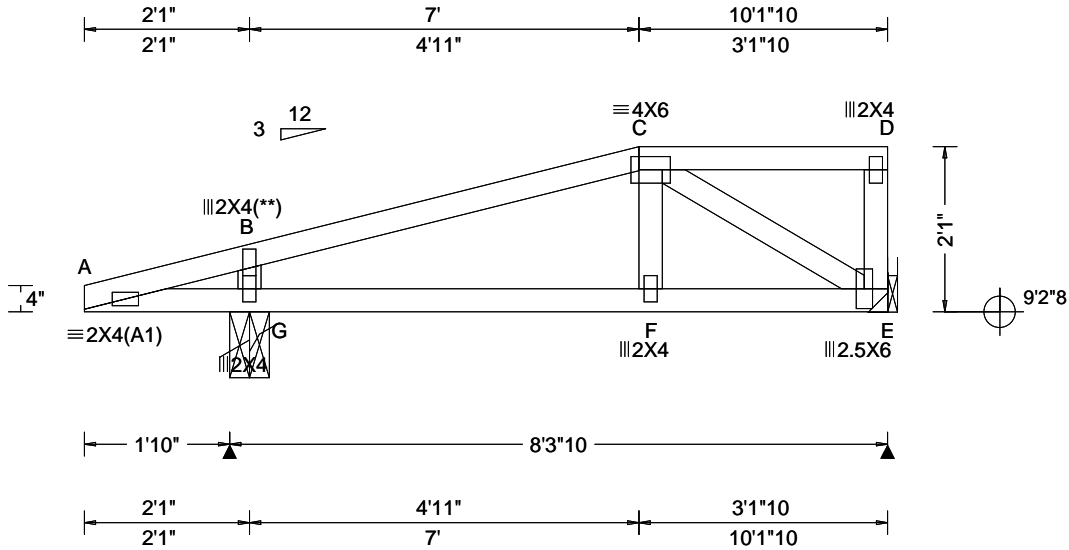
- Bearing E (8'0"10, 9'2"8) HUS26
- Supporting Member: (1)2x6 SP #2
- (14) 0.148"x3" nails into supporting member,
- (4) 0.148"x3" nails into supported member.



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|--|--|--|--|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|------|-----|---|-----|---|---|------|------|---|--------|------------|--------|-------------|-------|----------|-------|----------|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 580 | - | - | /315 | /171 | /72 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 352 | - | - | /203 | /111 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 257 -375 | B - C | 338 -422 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

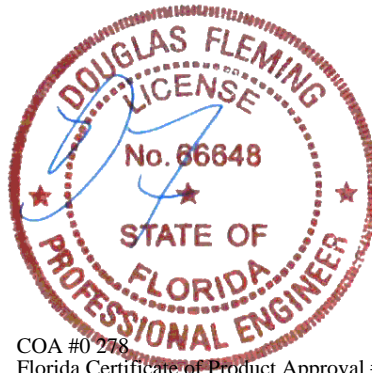
Right end vertical not exposed to wind pressure.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

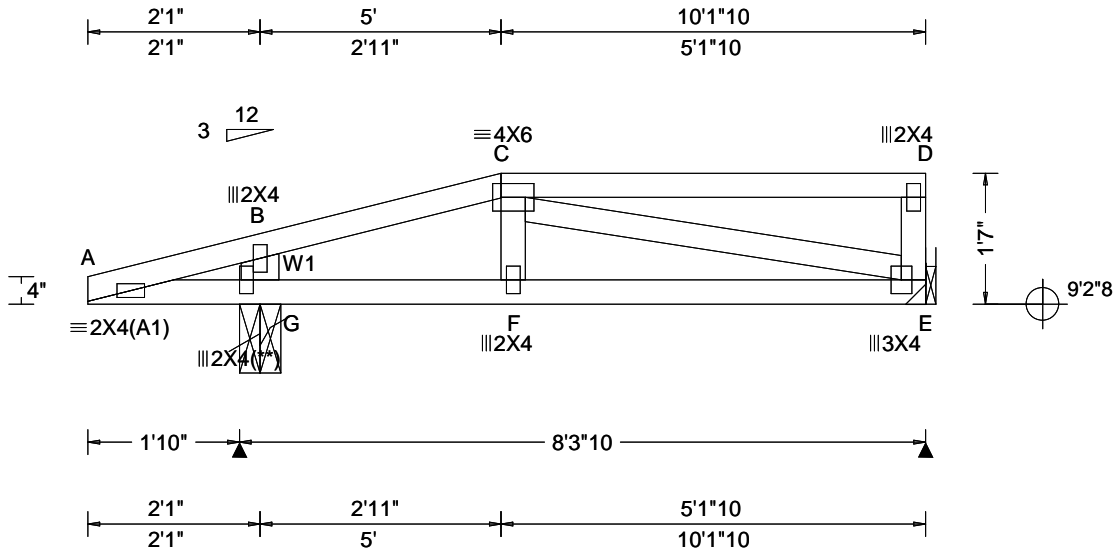
The overall height of this truss excluding overhang is 2'-1-0.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| | | | | |
|--|---|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.055 C 999 240 VERT(CL): 0.104 C 929 180 HORZ(LL): -0.012 D - - HORZ(TL): 0.021 D - - Creep Factor: 2.0 Max TC CSI: 0.787 Max BC CSI: 0.776 Max Web CSI: 0.158 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL G 746 /- /- /- /219 /- E 317 /- /- /- /101 /- Wind reactions based on MWFRS G Brg Wid = 6.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 94 -408 |
|--|---|--|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x6 SP #2;

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp.
B - G 158 -428

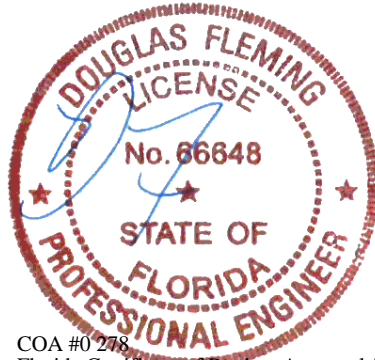
Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 71 plf at 0.00 to 71 plf at 5.00
TC: From 35 plf at 5.00 to 35 plf at 10.14
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 10.14
TC: 113 lb Conc. Load at -0.00
TC: 28 lb Conc. Load at 5.03
TC: 64 lb Conc. Load at 7.06, 9.06
BC: 60 lb Conc. Load at 5.03
BC: 17 lb Conc. Load at 7.06, 9.06

Plating Notes
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

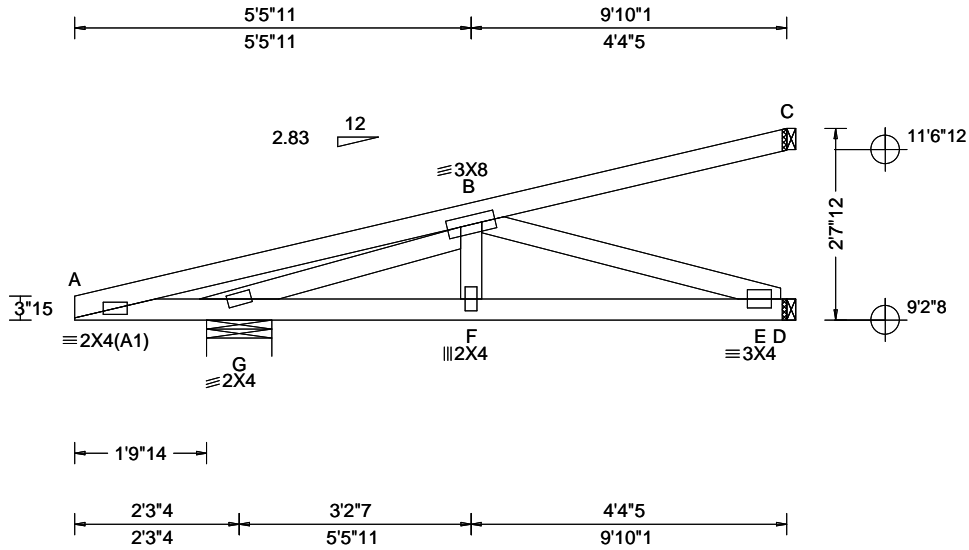
Additional Notes
The overall height of this truss excluding overhang is 1-7-0.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| Loading Criteria (psf) TCLL: 20.00 TC DL: 15.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 45.00 NCBC LL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BC DL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.014 F 999 240 VERT(CL): 0.020 F 999 180 HORZ(LL): 0.003 E - - HORZ(TL): 0.005 E - - Creep Factor: 2.0 Max TC CSI: 0.353 Max BC CSI: 0.272 Max Web CSI: 0.255 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>453</td> <td>-</td> <td>-</td> <td>-</td> <td>150</td> <td>-</td> </tr> <tr> <td>D</td> <td>195</td> <td>-</td> <td>-</td> <td>7</td> <td>64</td> <td>-</td> </tr> <tr> <td>C</td> <td>49</td> <td>-</td> <td>-</td> <td>11</td> <td>21</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS G Brg Wid = 10.8 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>G - F</td> <td>390 -222</td> <td>F - E</td> <td>394 -223</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | G | 453 | - | - | - | 150 | - | D | 195 | - | - | 7 | 64 | - | C | 49 | - | - | 11 | 21 | - | Chords | Tens.Comp. | Chords | Tens. Comp. | G - F | 390 -222 | F - E | 394 -223 |
|--|---|--|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|-----|---|---|---|----|---|---|----|---|---|----|----|---|--------|------------|--------|-------------|-------|----------|-------|----------|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 453 | - | - | - | 150 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 195 | - | - | 7 | 64 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 49 | - | - | 11 | 21 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G - F | 390 -222 | F - E | 394 -223 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

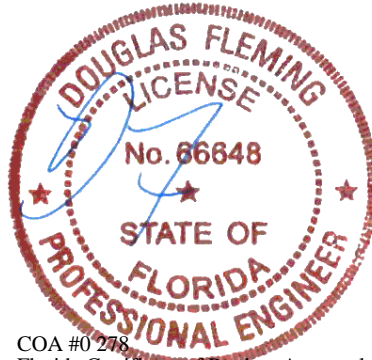
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 2 plf at 0.00 to 2 plf at 9.84
 BC: From 2 plf at 0.00 to 2 plf at 9.84
 TC: 45 lb Conc. Load at 0.06
 TC: 69 lb Conc. Load at 1.38
 TC: -15 lb Conc. Load at 4.21
 TC: 178 lb Conc. Load at 7.03
 BC: 39 lb Conc. Load at 1.38
 BC: -17 lb Conc. Load at 4.21
 BC: 80 lb Conc. Load at 7.03

Wind

Wind loads and reactions based on MWFRS.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

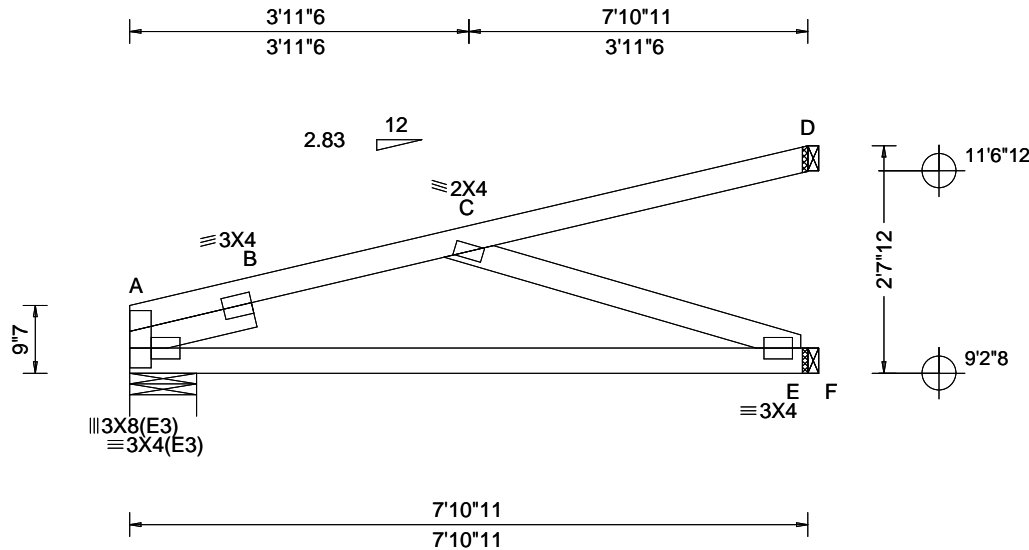
The overall height of this truss excluding overhang is 2-7-12.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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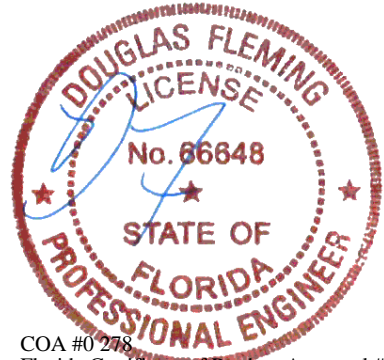
| | | | | |
|--|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.012 C 999 240 VERT(CL): 0.030 B 999 180 HORZ(LL): 0.006 B - - HORZ(TL): 0.014 B - - Creep Factor: 2.0 Max TC CSI: 0.542 Max BC CSI: 0.668 Max Web CSI: 0.303 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 279 - / - / - /81 - E 236 - / - / - /60 - D 75 - / - / - /15 - Wind reactions based on MWFRS A Brg Wid = 9.3 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 198 -762 B - C 196 -520 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. A - F 495 -181 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - F 193 -526 |
|--|---|--|---|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 2 plf at 0.00 to 2 plf at 7.89
BC: From 2 plf at 0.00 to 2 plf at 7.89
TC: 50 lb Conc. Load at 2.26
TC: 218 lb Conc. Load at 5.09
BC: 23 lb Conc. Load at 2.26
BC: 112 lb Conc. Load at 5.09

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

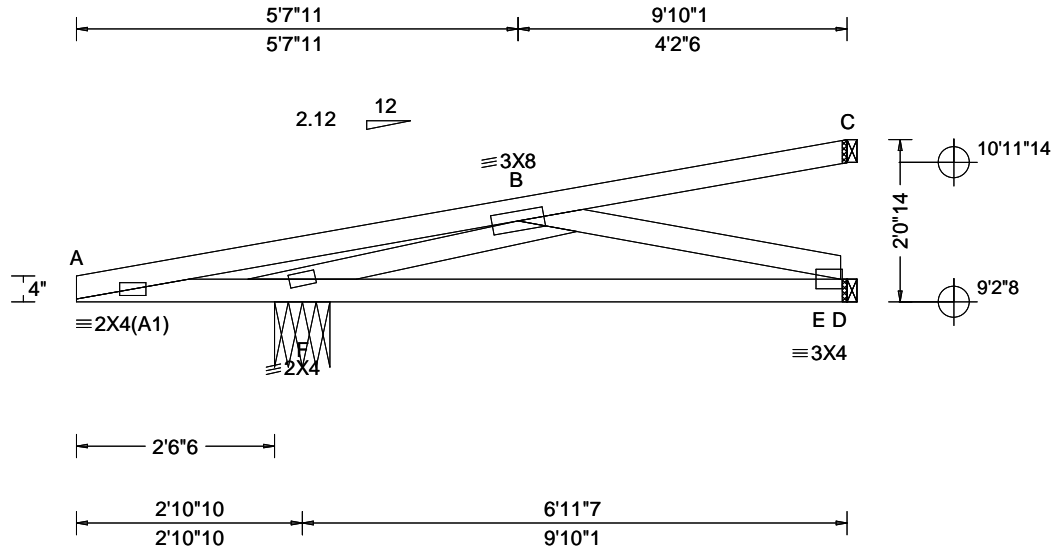
Additional Notes
The overall height of this truss excluding overhang is 2-7-12.



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| | | | | |
|--|--|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCCL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 B 999 240 VERT(CL): 0.012 B 999 180 HORZ(LL): 0.003 B - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.352 Max BC CSI: 0.533 Max Web CSI: 0.257 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL |
| | | | | F 490 /- /- /- /46 /- D 159 /- /- /- /12 /- C 44 /- /- /- /18 /- Wind reactions based on MWFRS F Brg Wid = 8.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

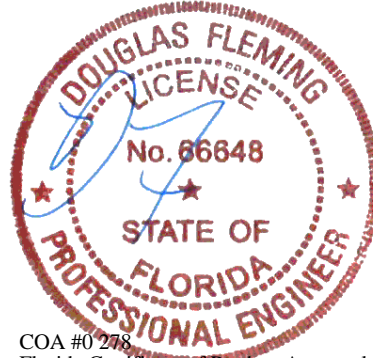
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 2 plf at 0.00 to 2 plf at 9.84
 BC: From 20 plf at 0.00 to 20 plf at 4.21
 BC: From 2 plf at 4.21 to 2 plf at 9.84
 TC: 156 lb Conc. Load at 0.00
 TC: 29 lb Conc. Load at 4.21
 TC: 163 lb Conc. Load at 7.03
 BC: -2 lb Conc. Load at 4.21
 BC: 89 lb Conc. Load at 7.03

Wind

Wind loads and reactions based on MWFRS.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

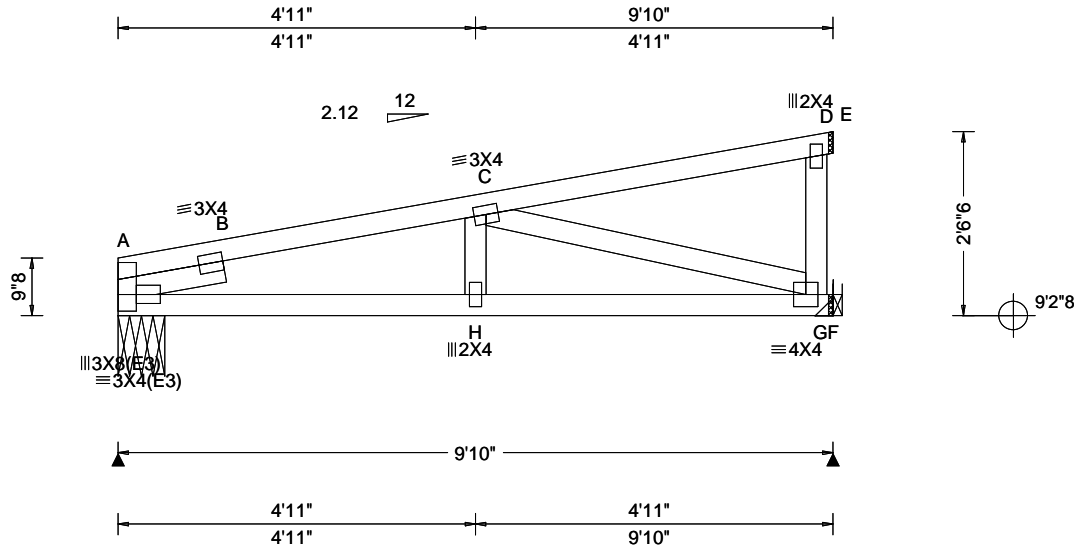
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|---|---|--|---|--|------------|--------|-------------|-------------|------------|-------|------------|---|--|-------------|------------|--------|-------------|-------|-----------|-------|-----------|-----|---|-----|---|---|---|------|---|---|-----|---|---|---|------|---|
| | | | | Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 344 | - | - | - | /225 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| F | 506 | - | - | - | /161 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>427 - 1029</td> <td>B - C</td> <td>424 - 1020</td> </tr> </tbody> </table> | | | | Chords | Tens.Comp. | Chords | Tens. Comp. | A - B | 427 - 1029 | B - C | 424 - 1020 | Maximum Bot Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - H</td> <td>990 - 402</td> <td>H - G</td> <td>977 - 400</td> </tr> </tbody> </table> | | Chords | Tens.Comp. | Chords | Tens. Comp. | A - H | 990 - 402 | H - G | 977 - 400 | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 427 - 1029 | B - C | 424 - 1020 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - H | 990 - 402 | H - G | 977 - 400 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Lt Slider: 2x4 SP #3; block length = 1.500'

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 2 plf at 0.00 to 2 plf at 9.83
 BC: From 2 plf at 0.00 to 2 plf at 9.83
 TC: 40 lb Conc. Load at 1.61
 TC: -27 lb Conc. Load at 1.61
 TC: 177 lb Conc. Load at 4.44
 TC: 317 lb Conc. Load at 7.27
 BC: 23 lb Conc. Load at 1.61
 BC: -27 lb Conc. Load at 1.61
 BC: 82 lb Conc. Load at 4.44
 BC: 175 lb Conc. Load at 7.27

Hangers / Ties

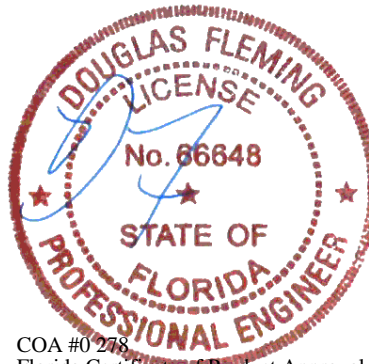
Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

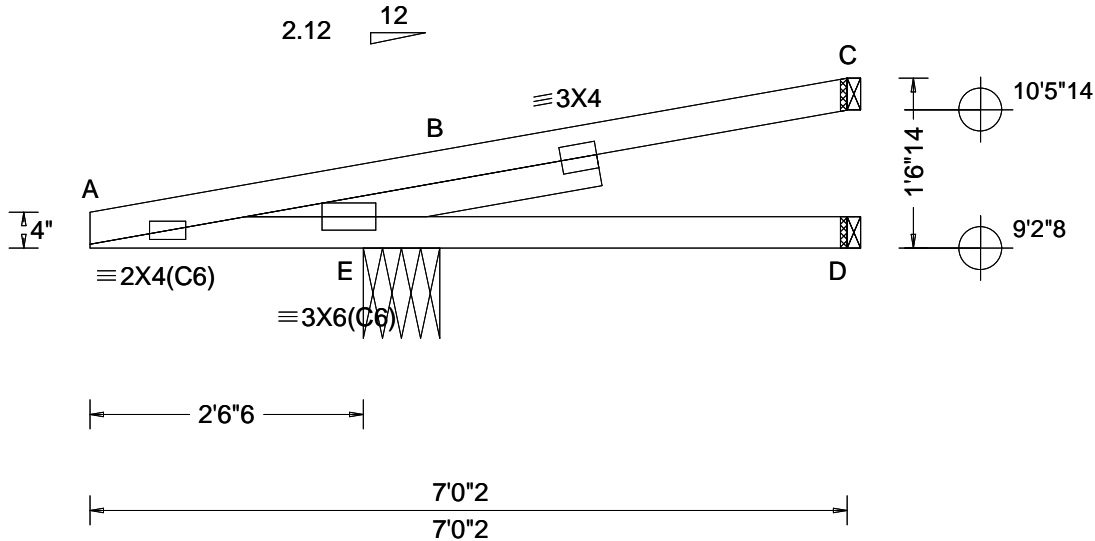
The overall height of this truss excluding overhang is 2-6-6.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| Loading Criteria (psf) TCLL: 20.00 TCCL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): -0.009 B 999 240 VERT(CL): 0.035 B 999 180 HORZ(LL): -0.006 B - - HORZ(TL): 0.015 B - - Creep Factor: 2.0 Max TC CSI: 0.334 Max BC CSI: 0.228 Max Web CSI: 0.245 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>413</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/31</td> <td>/-</td> </tr> <tr> <td>D</td> <td>60</td> <td>/-11</td> <td>/-</td> <td>/2</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>28</td> <td>/-27</td> <td>/-</td> <td>/5</td> <td>/-</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | E | 413 | /- | /- | /- | /31 | /- | D | 60 | /-11 | /- | /2 | /- | /- | C | 28 | /-27 | /- | /5 | /- | /- |
|--|---|--|---|--|-------|---------|-------|-----|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|----|-----|----|---|----|------|----|----|----|----|---|----|------|----|----|----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 413 | /- | /- | /- | /31 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 60 | /-11 | /- | /2 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 28 | /-27 | /- | /5 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Wind reactions based on MWFRS E Brg Wid = 8.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>477</td> <td>-53</td> </tr> </tbody> </table> | | | | Chords | Tens. | Comp. | A - B | 477 | -53 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens. | Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 477 | -53 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Lt Slider: 2x4 SP #3; block length = 3.300'

Special Loads

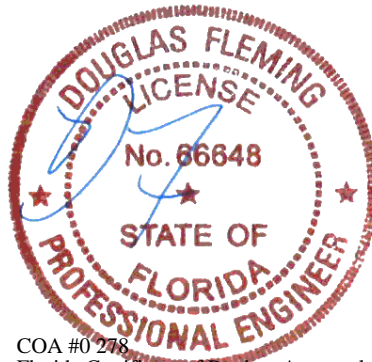
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 2 plf at 0.00 to 2 plf at 7.01
 BC: From 2 plf at 0.00 to 2 plf at 7.01
 TC: 75 lb Conc. Load at -0.00
 TC: 29 lb Conc. Load at 4.21
 BC: -2 lb Conc. Load at 4.21

Wind

Wind loads and reactions based on MWFRS.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

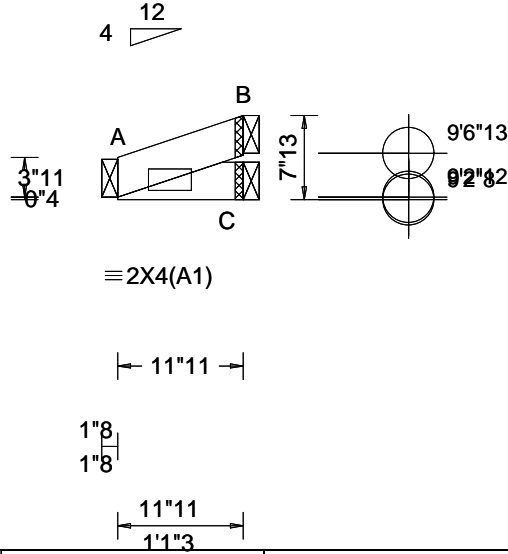
The overall height of this truss excluding overhang is 1-6-14.



COA #0 278
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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|----|-----|-------------|-----|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): NA | A | 45 | /- | /- | /27 | /9 | /13 |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): NA | C | 19 | /- | /- | /10 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 A - - | B | 34 | /- | /- | /18 | /17 | /- |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.000 A - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | A Brg Wid = 1.5 Min Req = - | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.029 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.009 | B Brg Wid = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Members not listed have forces less than 375# | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | | | | | | | |
| | Loc. from endwall: Any | WAVE | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

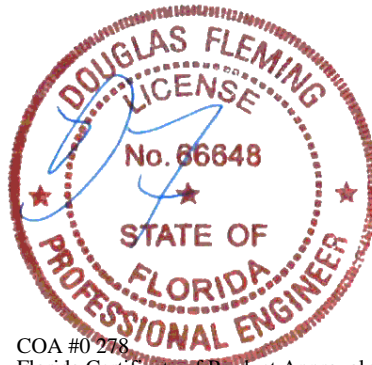
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

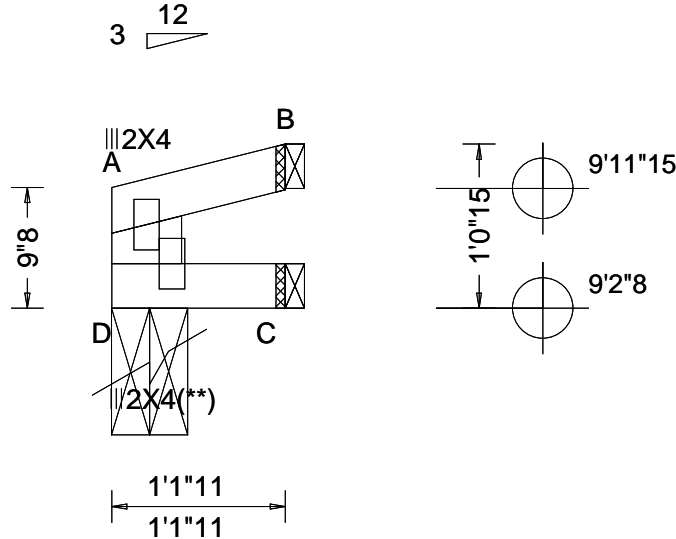
The overall height of this truss excluding overhang is 0-7-13.



COA #0 278
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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|----|-----|-------------|-----|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.000 A 999 240 | D | 52 | /- | /- | /30 | /15 | /- |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.000 A 999 180 | C | 23 | /- | /- | /11 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 A - - | B | 40 | /- | /- | /18 | /17 | /11 |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.000 A - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | D Brg Wid = 6.0 Min Req = 1.5 (Truss) | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.040 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.012 | B Brg Wid = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.011 | Bearing D is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: Any | WAVE | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

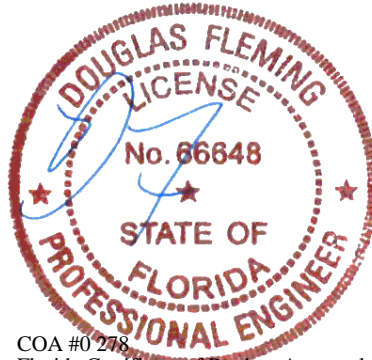
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

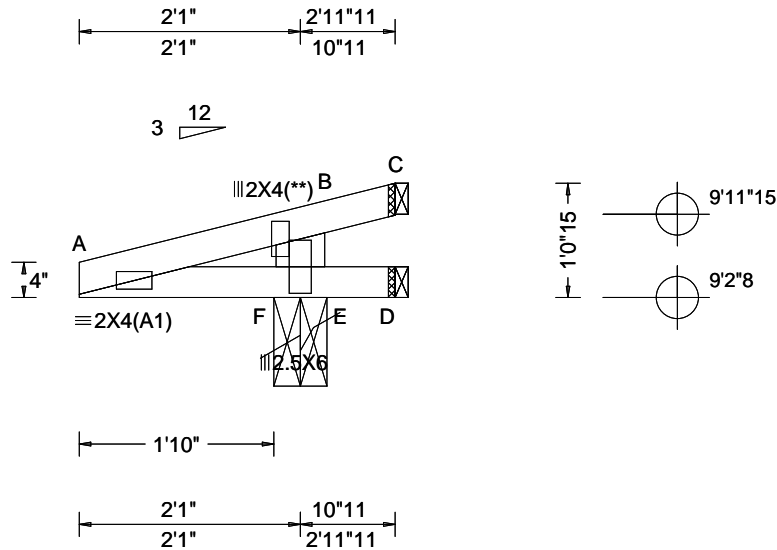
Additional Notes

The overall height of this truss excluding overhang is 1-0-15.



COA #0 278
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01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-----------------------------------|------------------------------|---------------------------------|--|------------|-------|-------------|------|------|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.000 B 999 240 | F | 607 | /- | /- | /303 | /236 | /26 |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.000 B 999 180 | D | - | /-166 | /- | /81 | /89 | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.007 C - - | C | - | /-182 | /- | /67 | /83 | /- |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.015 C - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | F Brg Wid = 6.0 Min Req = 1.5 (Truss) | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.265 | D Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.233 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: > 2h | FT/RT:20(0)/10(0) | Max Web CSI: 0.090 | Bearing F is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: not in 9.00 ft | WAVE | | Maximum Web Forces Per Ply (lbs) | | | | | | |
| | GCp: 0.18 | | | Webs | Tens.Comp. | | | | | |
| | Wind Duration: 1.60 | | | B - E | 532 | -362 | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Plating Notes

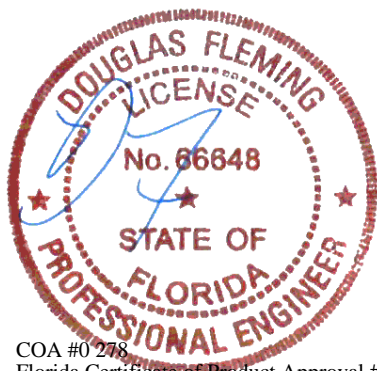
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

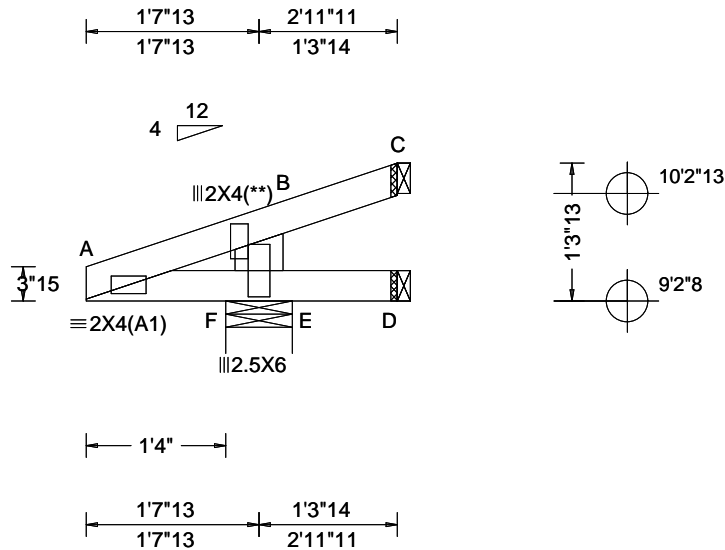
Negative reaction(s) of -182# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
The overall height of this truss excluding overhang is 1-0-15.



COA #0276
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01/19/2026

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| Loading Criteria (psf) TCLL: 20.00 TC DL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): -0.006 C - - HORZ(TL): 0.014 C - - Creep Factor: 2.0 Max TC CSI: 0.233 Max BC CSI: 0.215 Max Web CSI: 0.060 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>F</td> <td>432</td> <td>/-</td> <td>/-</td> <td>/192</td> <td>/125</td> <td>/40</td> </tr> <tr> <td>D</td> <td>-</td> <td>/-79</td> <td>/-</td> <td>/26</td> <td>/30</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-78</td> <td>/-</td> <td>/16</td> <td>/29</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | F | 432 | /- | /- | /192 | /125 | /40 | D | - | /-79 | /- | /26 | /30 | /- | C | - | /-78 | /- | /16 | /29 | /- |
|---|---|--|--|--|------|------------|-------|----------|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|------|-----|---|---|------|----|-----|-----|----|---|---|------|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| F | 432 | /- | /- | /192 | /125 | /40 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | - | /-79 | /- | /26 | /30 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | /-78 | /- | /16 | /29 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | Wind reactions based on MWFRS F Brg Wid = 7.6 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - E</td> <td>407 -269</td> </tr> </tbody> </table> | Webs | Tens.Comp. | B - E | 407 -269 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - E | 407 -269 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x6 SP #2;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 72 plf at 0.00 to 72 plf at 2.97
 BC: From 20 plf at 0.00 to 20 plf at 2.97
 TC: 23 lb Conc. Load at 0.06

Plating Notes

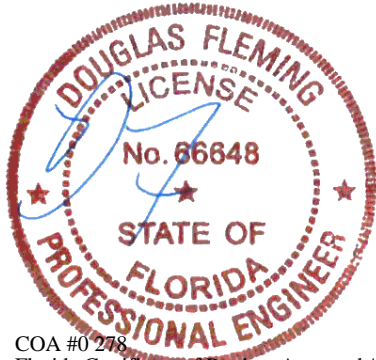
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

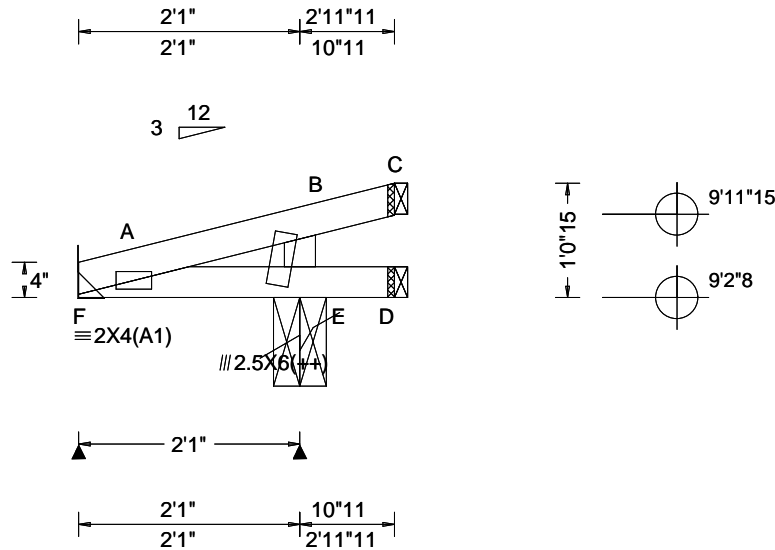
The overall height of this truss excluding overhang is 1-3-13.



COA #0 276
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|---------------|-----------------------|-------------|------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.002 A 999 240 | F | 94 | /- | /- | /53 | /20 | /30 |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.003 A 999 180 | E | 182 | /- | /- | /105 | /59 | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 A - - | D | - | /-10 | /- | /6 | /3 | /- |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.001 A - - | C | 14 | /-9 | /- | /0 | /4 | /- |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | Wind reactions based on MWFRS | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.058 | F | Brg Wid = - | Min Req = - | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.060 | E | Brg Wid = 6.0 | Min Req = 1.5 (Truss) | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.081 | D | Brg Wid = 1.5 | Min Req = - | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | C | Brg Wid = 1.5 | Min Req = - | | | | |
| | Loc. from endwall: Any | WAVE | | Bearing E is a rigid surface. | | | | | | |
| | GCp: 0.18 | | | Members not listed have forces less than 375# | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(++) - This plate works for both joints covered.

Hangers / Ties

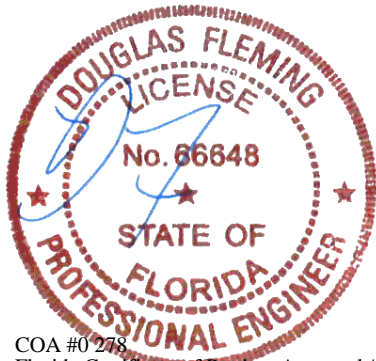
Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

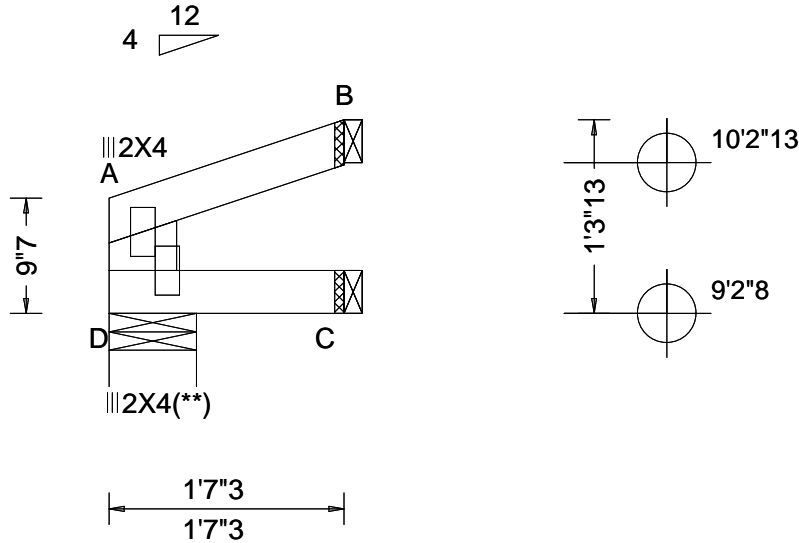
The overall height of this truss excluding overhang is 1-0-15.



COA #0248
Florida Certificate of Product Approval #FL1999
01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|----|-----|-------------|-----|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.000 A 999 240 | D | 73 | /- | /- | /46 | /23 | /- |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.000 A 999 180 | C | 32 | /- | /- | /16 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 A - - | B | 57 | /- | /- | /29 | /22 | /21 |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.000 A - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | D Brg Wid = 7.1 Min Req = 1.5 (Truss) | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.085 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.025 | B Brg Wid = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.017 | Bearing D is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: Any | WAVE | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

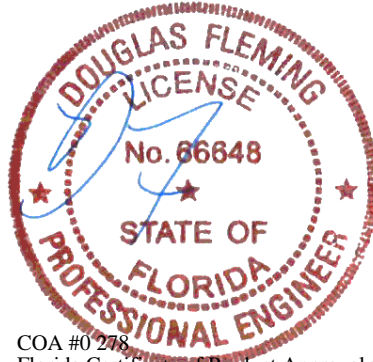
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

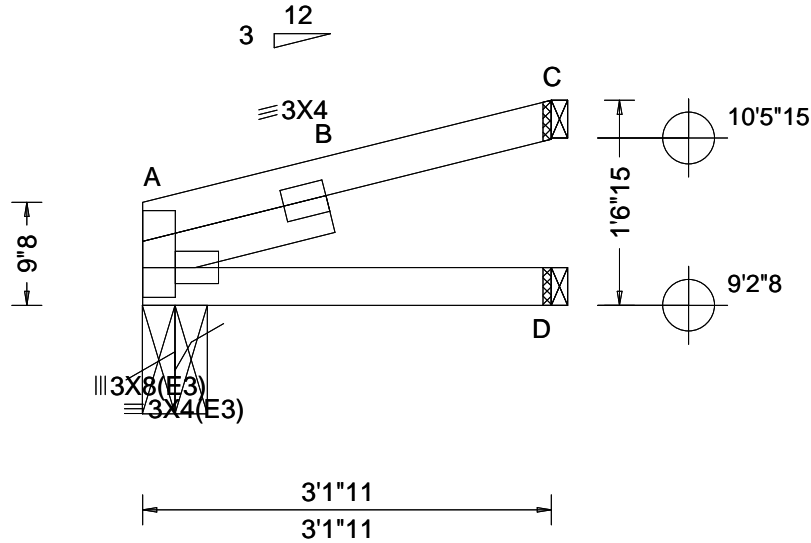
Additional Notes

The overall height of this truss excluding overhang is 1-3-13.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| Loading Criteria (psf) TCCL: 20.00 TCDL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.274 Max BC CSI: 0.102 Max Web CSI: 0.056 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>143</td> <td>/-</td> <td>/-</td> <td>/82</td> <td>/32</td> <td>/32</td> </tr> <tr> <td>D</td> <td>61</td> <td>/-</td> <td>/-</td> <td>/32</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>108</td> <td>/-</td> <td>/-</td> <td>/50</td> <td>/55</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 143 | /- | /- | /82 | /32 | /32 | D | 61 | /- | /- | /32 | /- | /- | C | 108 | /- | /- | /50 | /55 | /- |
|--|---|--|---|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|-----|-----|-----|---|----|----|----|-----|----|----|---|-----|----|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 143 | /- | /- | /82 | /32 | /32 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 61 | /- | /- | /32 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 108 | /- | /- | /50 | /55 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

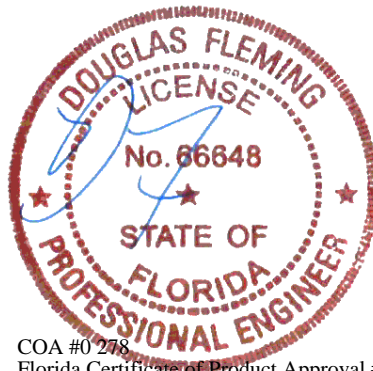
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Lt Slider: 2x4 SP #3; block length = 1.50'

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

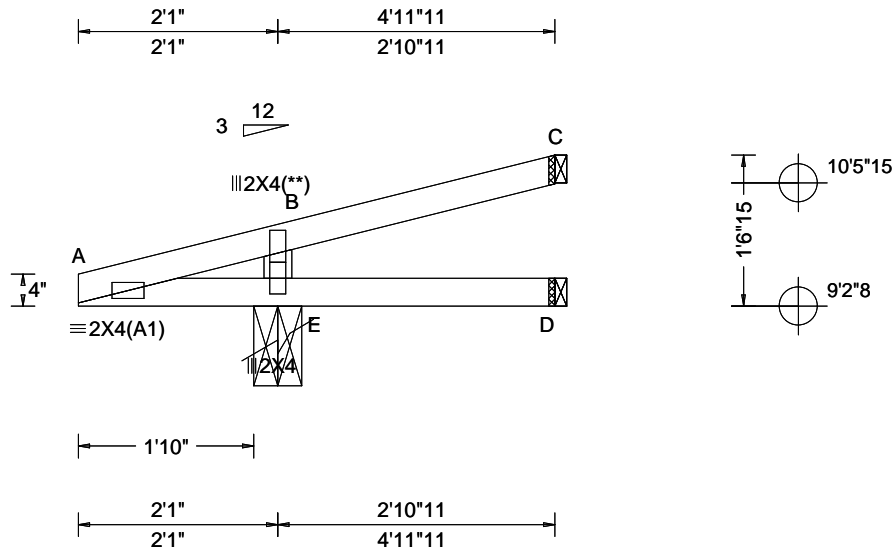
The overall height of this truss excluding overhang is 1-6-15.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-----------------------------------|------------------------------|---------------------------------|---|------------|------|-------------|------|------|------|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.000 B 999 240 | E | 410 | /- | /- | /214 | /138 | /43 |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.000 B 999 180 | D | 21 | /-11 | /- | /23 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.008 C - - | C | 69 | /-1 | /- | /34 | /24 | /- |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.021 C - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | E Brg Wid = 6.0 Min Req = 1.5 (Truss) | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.293 | D Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.202 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: > 2h | FT/RT:20(0)/10(0) | Max Web CSI: 0.145 | Bearing E is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: not in 9.00 ft | WAVE | | Maximum Web Forces Per Ply (lbs) | | | | | | |
| | GCp: 0.18 | | | Webs | Tens.Comp. | | | | | |
| | Wind Duration: 1.60 | | | B - E | 414 | | | | | -284 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

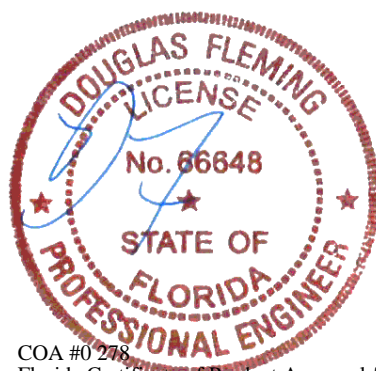
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

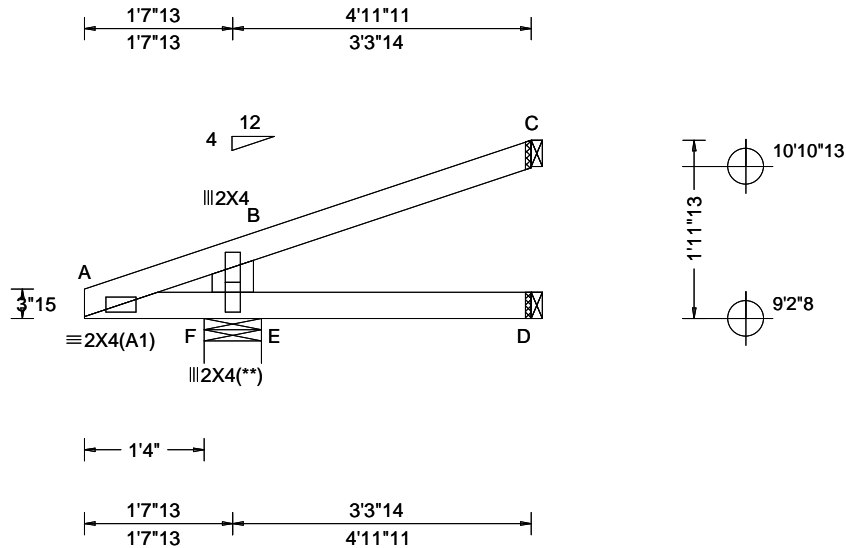
The overall height of this truss excluding overhang is 1-6-15.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|------------|------|-------------|------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCCL: 20.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.000 B 999 240 | F | 366 | /- | /- | /197 | /70 | /68 |
| BCCL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.000 B 999 180 | D | 40 | /- | /- | /28 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.004 C - - | C | 89 | /- | /- | /49 | /52 | /- |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.014 C - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | F Brg Wid = 7.6 Min Req = 1.5 (Truss) | | | | | | |
| Soffit: 2.00 | TCCL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.281 | D Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.163 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.071 | Bearing F is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: Any | WAVE | | Maximum Web Forces Per Ply (lbs) | | | | | | |
| | GCp: 0.18 | | | Webs | Tens.Comp. | | | | | |
| | Wind Duration: 1.60 | | | B - E | 458 | -270 | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Plating Notes

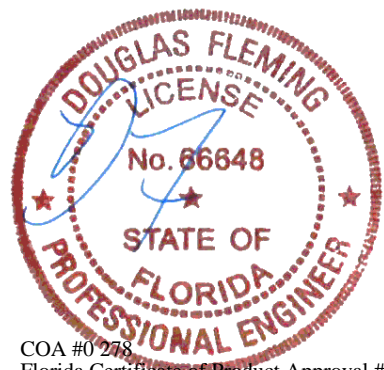
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

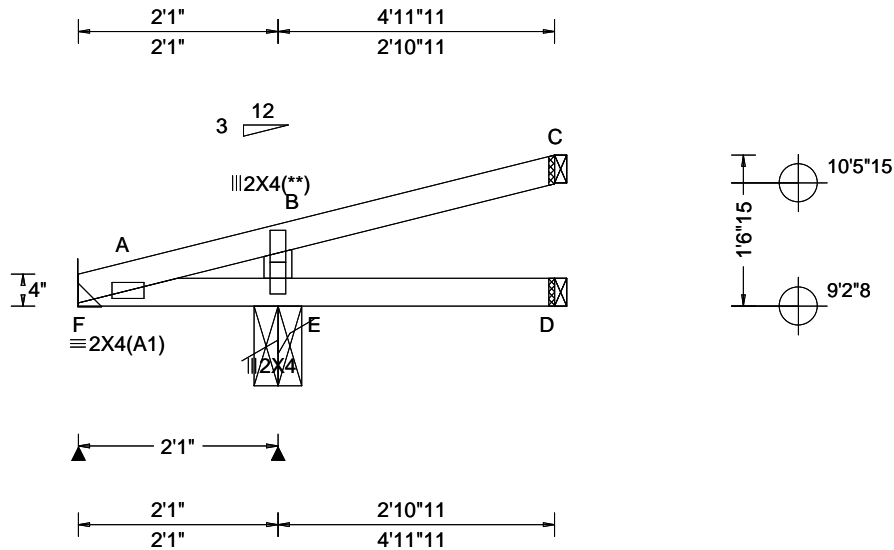
The overall height of this truss excluding overhang is 1-11-13.



COA #0278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| Loading Criteria (psf) TCCL: 20.00 TCCL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 240 VERT(CL): 0.002 A 999 180 HORZ(LL): 0.001 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.137 Max BC CSI: 0.062 Max Web CSI: 0.107 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>F</td> <td>86</td> <td>/-</td> <td>/-</td> <td>/43</td> <td>/9</td> <td>/51</td> </tr> <tr> <td>E</td> <td>275</td> <td>/-</td> <td>/-</td> <td>/158</td> <td>/90</td> <td>/-</td> </tr> <tr> <td>D</td> <td>44</td> <td>/-</td> <td>/-</td> <td>/22</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>81</td> <td>/-</td> <td>/-</td> <td>/36</td> <td>/40</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | F | 86 | /- | /- | /43 | /9 | /51 | E | 275 | /- | /- | /158 | /90 | /- | D | 44 | /- | /- | /22 | /- | /- | C | 81 | /- | /- | /36 | /40 | /- |
|--|---|--|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|----|----|----|-----|----|-----|---|-----|----|----|------|-----|----|---|----|----|----|-----|----|----|---|----|----|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| F | 86 | /- | /- | /43 | /9 | /51 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 275 | /- | /- | /158 | /90 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 44 | /- | /- | /22 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 81 | /- | /- | /36 | /40 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | Wind reactions based on MWFRS F Brg Wid = - Min Req = - E Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

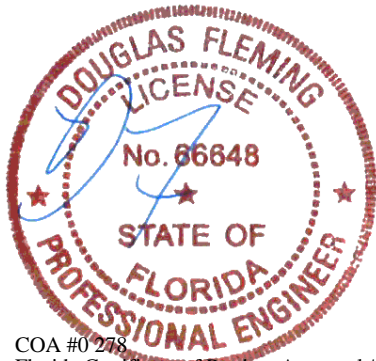
Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

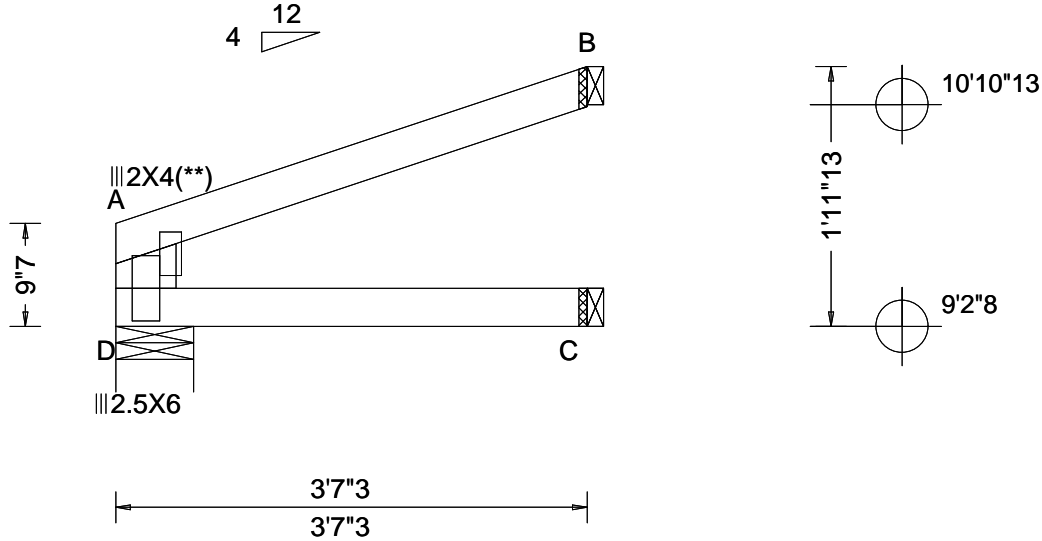
The overall height of this truss excluding overhang is 1-6-15.



COA #0 278
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| Loading Criteria (psf) TCCL: 20.00 TCCL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.000 A 999 180 HORZ(LL): 0.000 A - - HORZ(TL): 0.000 A - - Creep Factor: 2.0 Max TC CSI: 0.369 Max BC CSI: 0.145 Max Web CSI: 0.028 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>D</td> <td>165</td> <td>/-</td> <td>/-</td> <td>/104</td> <td>/53</td> <td>/-</td> </tr> <tr> <td>C</td> <td>72</td> <td>/-</td> <td>/-</td> <td>/36</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>B</td> <td>129</td> <td>/-</td> <td>/-</td> <td>/66</td> <td>/50</td> <td>/49</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | D | 165 | /- | /- | /104 | /53 | /- | C | 72 | /- | /- | /36 | /- | /- | B | 129 | /- | /- | /66 | /50 | /49 |
|--|---|--|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|-----|----|---|----|----|----|-----|----|----|---|-----|----|----|-----|-----|-----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 165 | /- | /- | /104 | /53 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 72 | /- | /- | /36 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 129 | /- | /- | /66 | /50 | /49 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | Wind reactions based on MWFRS D Brg Wid = 7.1 Min Req = 1.5 (Truss) C Brg Wid = 1.5 Min Req = - B Brg Wid = 1.5 Min Req = - Bearing D is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x6 SP #2;

Plating Notes

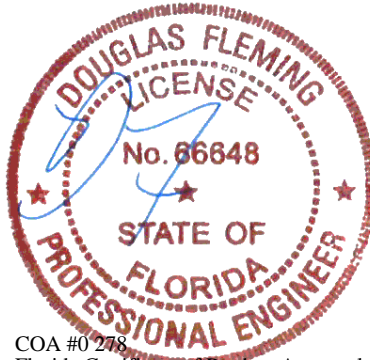
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

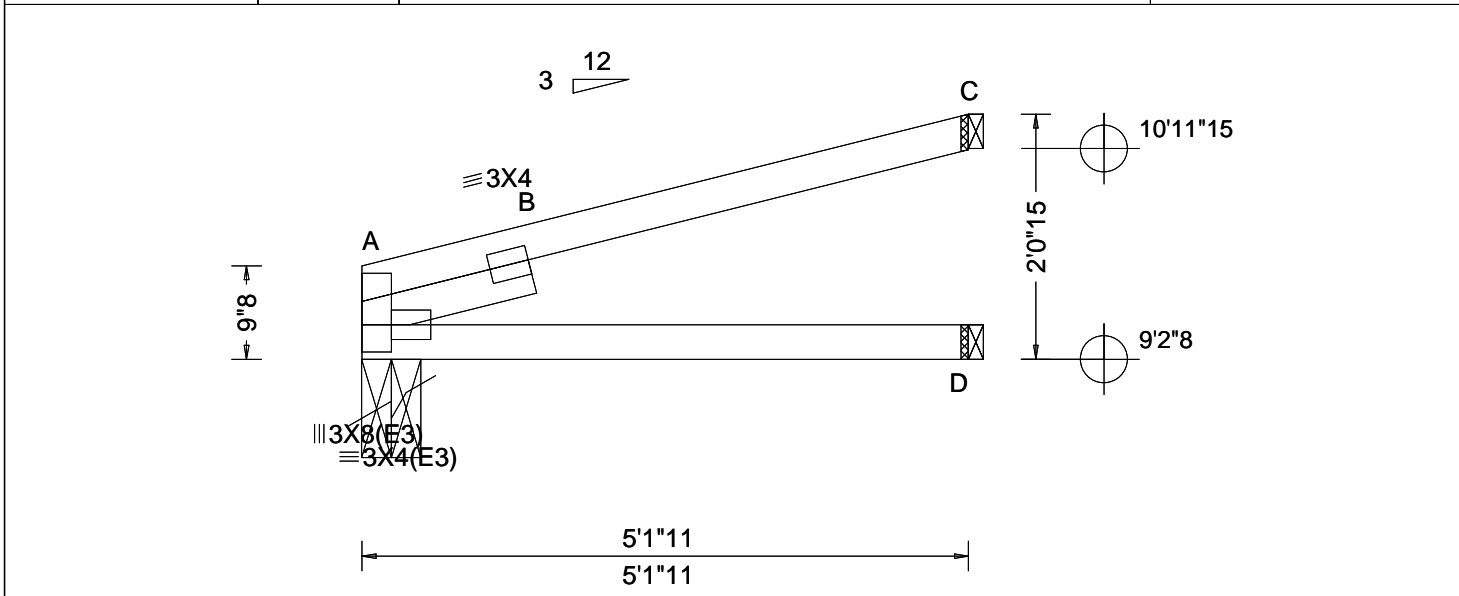
The overall height of this truss excluding overhang is 1-11-13.



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| | | | |
|---------------------------|--------------------------|--|--|
| SEQN: 722210 FROM: RFG | JACK Ply: 1 Qty: 1 | Job Number: 25-3040 ANDERSON PROJECT Truss Label: J05A | Cust: R215 JRef: 1YGX2150009 T46 DrwNo: 019.26.1558.07473 NW / DF 01/19/2026 |
|---------------------------|--------------------------|--|--|

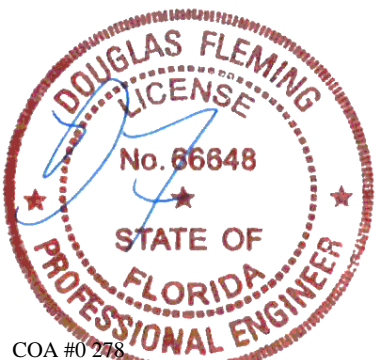


| | | | | |
|--|---|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.020 B - - HORZ(TL): 0.046 B - - Creep Factor: 2.0 Max TC CSI: 0.652 Max BC CSI: 0.300 Max Web CSI: 0.254 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 234 - / - /134 /56 /53 D 100 - / - /54 - / - C 172 - / - /81 /85 - Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |
|--|---|--|---|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Slider: 2x4 SP #3; block length = 1.50'

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

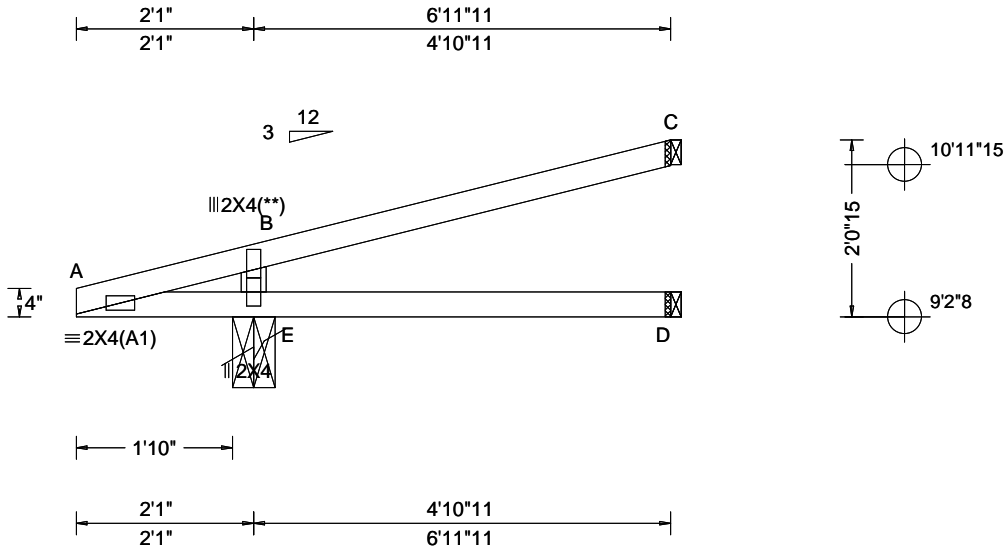
Additional Notes
The overall height of this truss excluding overhang is 2-0-15.



COA #0 278
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| | | | | |
|--|---|--|--|---|
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| | | | | E 466 /- /- /249 /142 /61 D 74 /- /- /47 /- /- C 144 /- /- /69 /54 /- Wind reactions based on MWFRS E Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

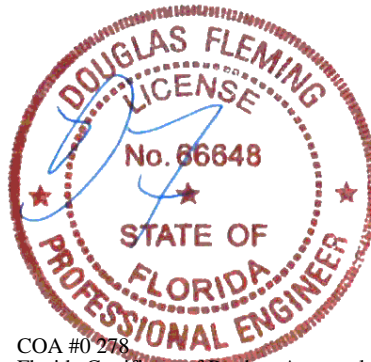
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

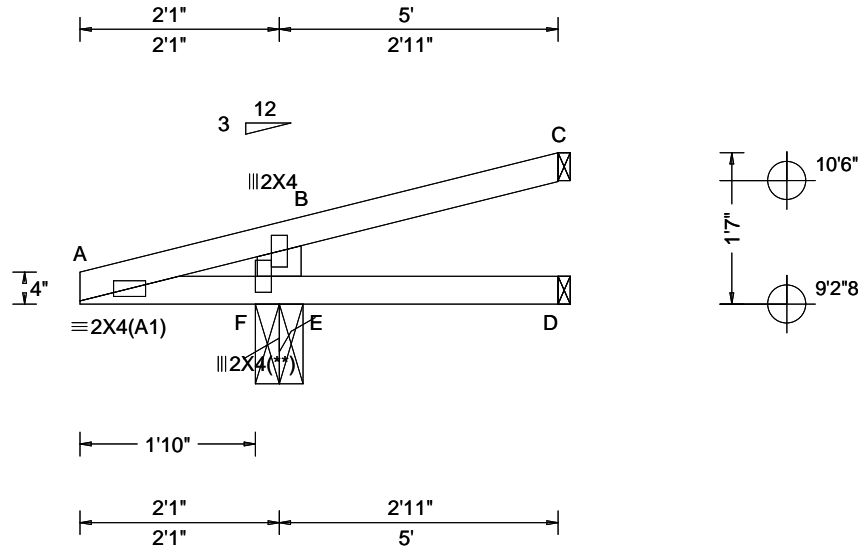
The overall height of this truss excluding overhang is 2-0-15.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|---|--|--|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|------|-----|---|----|------|----|-----|----|----|---|----|-----|----|-----|-----|----|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): -0.008 C - - HORZ(TL): 0.021 C - - Creep Factor: 2.0 Max TC CSI: 0.312 Max BC CSI: 0.218 Max Web CSI: 0.073 VIEW Ver: 24.02.00D.0114.10 | <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>F</td> <td>423</td> <td>/-</td> <td>/-</td> <td>/220</td> <td>/142</td> <td>/51</td> </tr> <tr> <td>D</td> <td>17</td> <td>/-16</td> <td>/-</td> <td>/24</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>64</td> <td>/-6</td> <td>/-</td> <td>/32</td> <td>/37</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS F Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | F | 423 | /- | /- | /220 | /142 | /51 | D | 17 | /-16 | /- | /24 | /- | /- | C | 64 | /-6 | /- | /32 | /37 | /- |
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| F | 423 | /- | /- | /220 | /142 | /51 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 17 | /-16 | /- | /24 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 64 | /-6 | /- | /32 | /37 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x6 SP #2;

Plating Notes

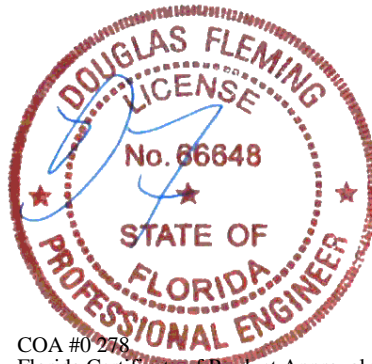
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

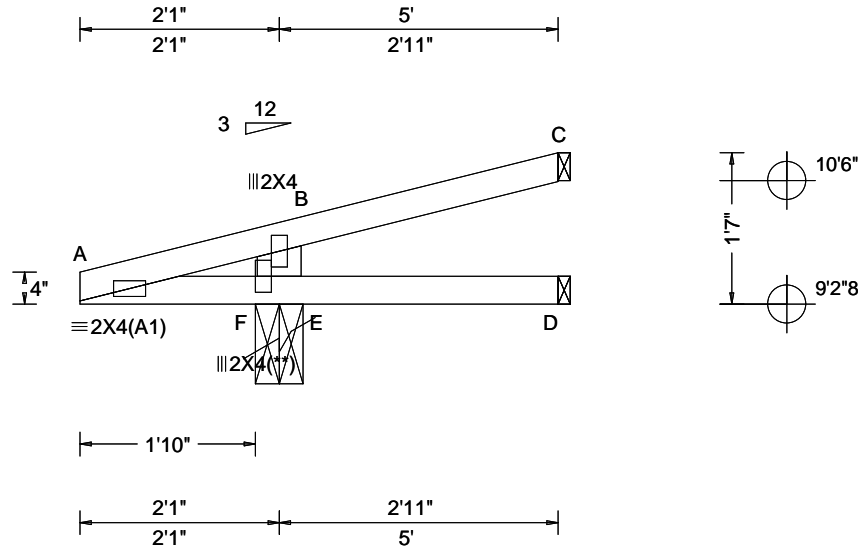
The overall height of this truss excluding overhang is 1-7-0.



COA #0278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| | | | | |
|---|--|---|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): -0.015 C - - HORZ(TL): 0.036 C - - Creep Factor: 2.0 Max TC CSI: 0.505 Max BC CSI: 0.394 Max Web CSI: 0.039 VIEW Ver: 24.02.00D.0114.10 | Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL F 527 /- /- /- /145 /- D 17 /-40 /- /21 /- /- C 41 /-30 /- /- /20 /- Wind reactions based on MWFRS F Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 71 plf at 0.00 to 71 plf at 5.00
BC: From 20 plf at 0.00 to 20 plf at 5.00
TC: 56 lb Conc. Load at -0.00

Plating Notes

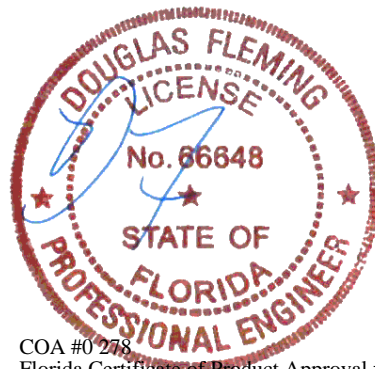
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads and reactions based on MWFRS.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

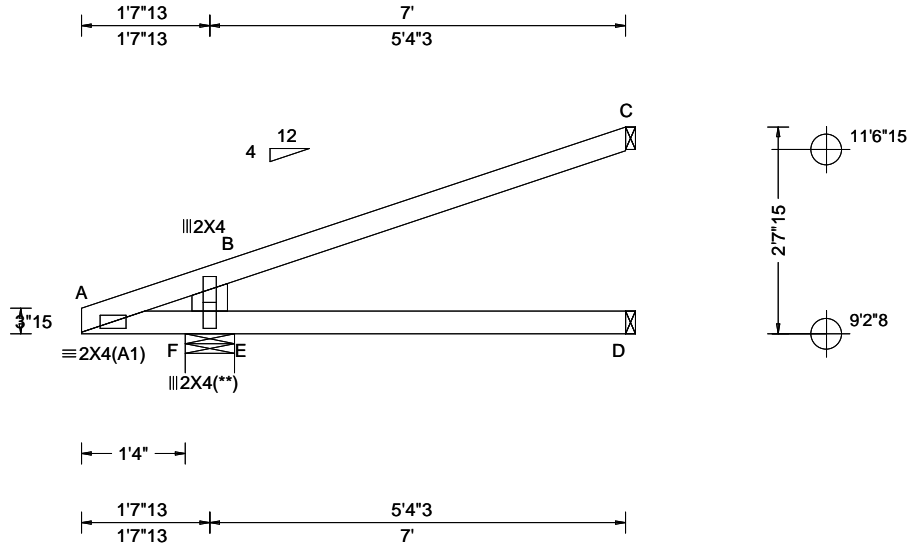
The overall height of this truss excluding overhang is 1-7-0.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| | | | | |
|---|---|---|---|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.009 B - - HORZ(TL): 0.025 B - - Creep Factor: 2.0 Max TC CSI: 0.543 Max BC CSI: 0.332 Max Web CSI: 0.081 VIEW Ver: 24.02.00D.0114.10 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 438 /- /- /240 /89 /95 D 88 /- /- /53 /- /- C 161 /- /- /89 /86 /- Wind reactions based on MWFRS F Brg Wid = 7.6 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

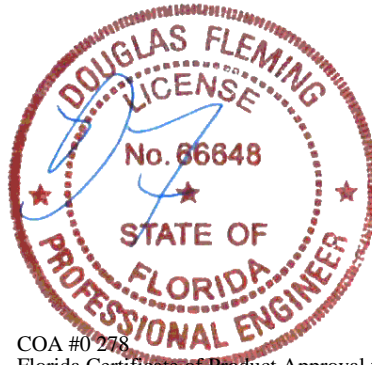
Wind loads based on MWFRS with additional C&C member design.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

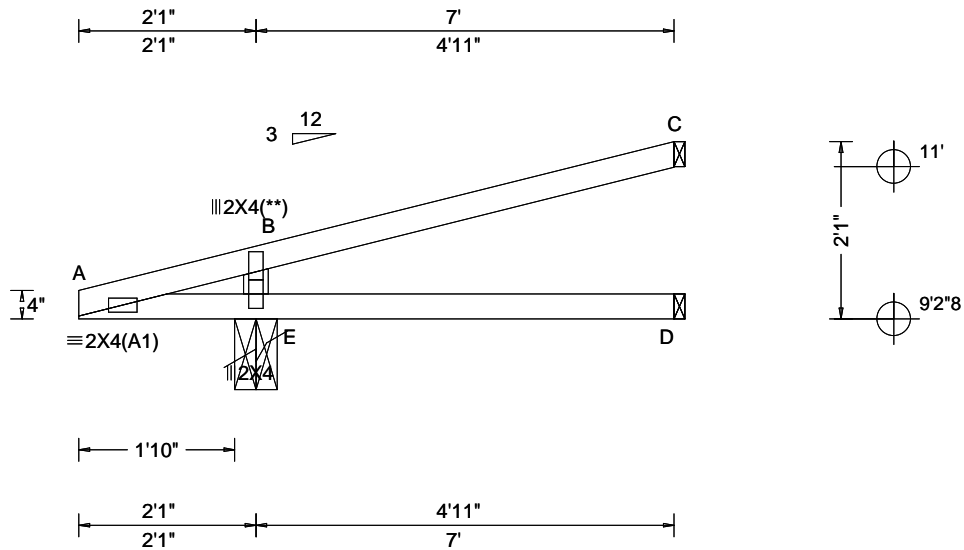
The overall height of this truss excluding overhang is 2-7-15.



COA #0 278
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01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|---|--|--|---------|-----|--|-------------|--|--|-----|----|-----|-----|-----|----|-----|---|-----|----|----|------|------|-----|---|----|----|----|-----|----|----|---|-----|----|----|-----|-----|----|
| TCCL: 20.00 TCDL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.018 C - - Creep Factor: 2.0 Max TC CSI: 0.424 Max BC CSI: 0.234 Max Web CSI: 0.169 VIEW Ver: 24.02.00D.0114.10 | <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>467</td> <td>/-</td> <td>/-</td> <td>/249</td> <td>/142</td> <td>/72</td> </tr> <tr> <td>D</td> <td>75</td> <td>/-</td> <td>/-</td> <td>/47</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>145</td> <td>/-</td> <td>/-</td> <td>/69</td> <td>/74</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS E Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. | Gravity | | | Non-Gravity | | | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | E | 467 | /- | /- | /249 | /142 | /72 | D | 75 | /- | /- | /47 | /- | /- | C | 145 | /- | /- | /69 | /74 | /- |
| Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 467 | /- | /- | /249 | /142 | /72 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 75 | /- | /- | /47 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 145 | /- | /- | /69 | /74 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

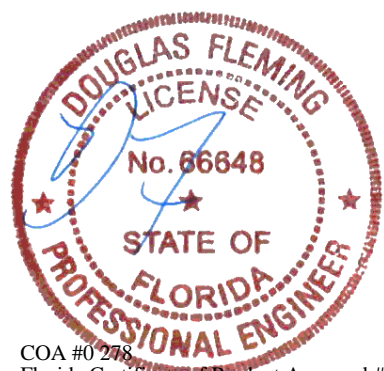
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

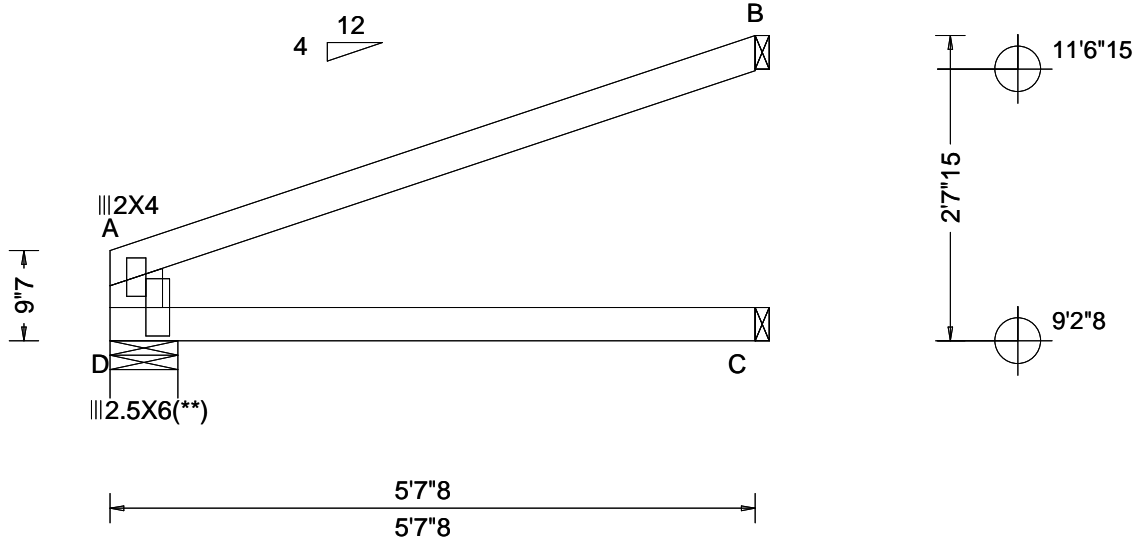
The overall height of this truss excluding overhang is 2-1-0.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-----------------------------------|------------------------------|---------------------------------|---|-----|-----|-------------|------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | D | 258 | /- | /- | /163 | /85 | /- |
| TCDL: 15.00 | Speed: 140 mph | Pf: NA Ce: NA | VERT(LL): 0.000 A 999 240 | C | 112 | /- | /- | /56 | /- | /- |
| BCLL: 0.00 | Enclosure: Enclosed | Lu: NA Cs: NA | VERT(CL): 0.000 A 999 180 | B | 201 | /- | /- | /103 | /78 | /77 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 A - - | Wind reactions based on MWFRS | | | | | | |
| Des Ld: 45.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.000 A - - | D Brg Wid = 7.1 Min Req = 1.5 (Truss) | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 | C Brg Wid = 1.5 Min Req = - | | | | | | |
| Soffit: 2.00 | TCDL: 4.2 psf | TPI Std: 2014 | Max TC CSI: 0.866 | B Brg Wid = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 3.0 psf | Rep Fac: Yes | Max BC CSI: 0.384 | Bearing D is a rigid surface. | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.041 | Members not listed have forces less than 375# | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 24.02.00D.0114.10 | | | | | | | |
| | Loc. from endwall: not in 4.50 ft | WAVE | | | | | | | | |
| | GCp1: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x6 SP #2;

Plating Notes

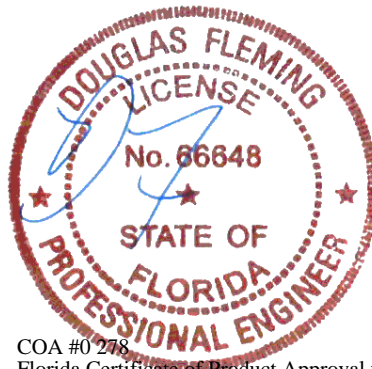
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

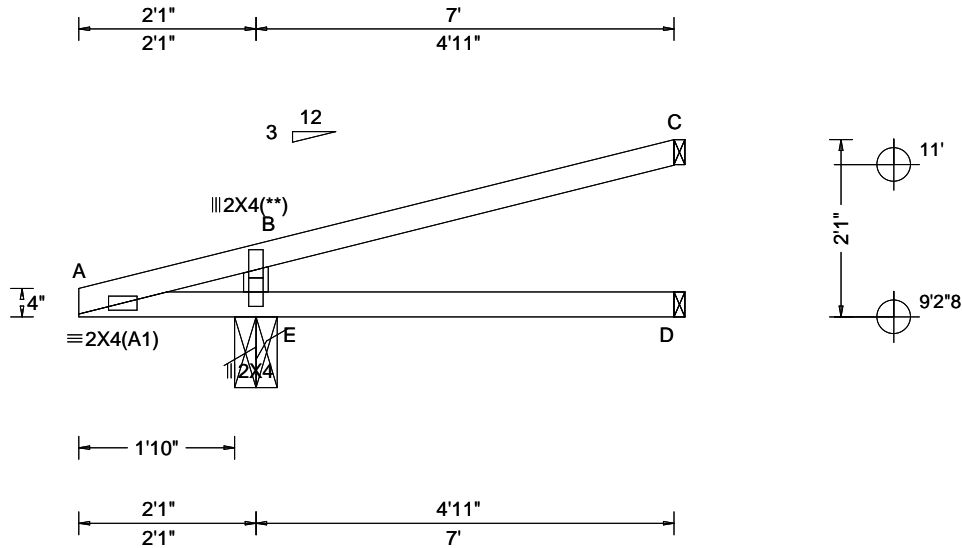
The overall height of this truss excluding overhang is 2-7-15.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/19/2026

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| Loading Criteria (psf) TCCL: 20.00 TCDL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): -0.052 C - - HORZ(TL): 0.055 C - - Creep Factor: 2.0 Max TC CSI: 0.758 Max BC CSI: 0.580 Max Web CSI: 0.076 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>615</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/158</td> <td>/-</td> </tr> <tr> <td>D</td> <td>75</td> <td>/-</td> <td>/-</td> <td>/16</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>122</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/58</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | E | 615 | /- | /- | /- | /158 | /- | D | 75 | /- | /- | /16 | /- | /- | C | 122 | /- | /- | /- | /58 | /- |
|--|---|--|--|--|------------|---------|----------|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|----|------|----|---|----|----|----|-----|----|----|---|-----|----|----|----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 615 | /- | /- | /- | /158 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 75 | /- | /- | /16 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 122 | /- | /- | /- | /58 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Wind reactions based on MWFRS E Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - E</td> <td>172 -436</td> </tr> </tbody> </table> | | | | Webs | Tens.Comp. | B - E | 172 -436 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - E | 172 -436 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 71 plf at 0.00 to 71 plf at 7.00
 BC: From 20 plf at 0.00 to 20 plf at 7.00
 TC: 101 lb Conc. Load at 0.00

Plating Notes

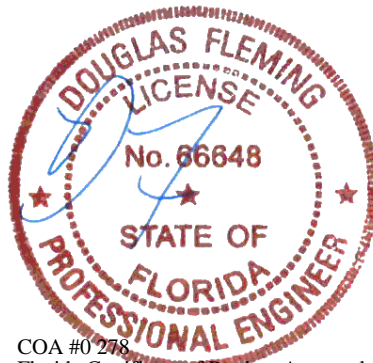
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads and reactions based on MWFRS.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

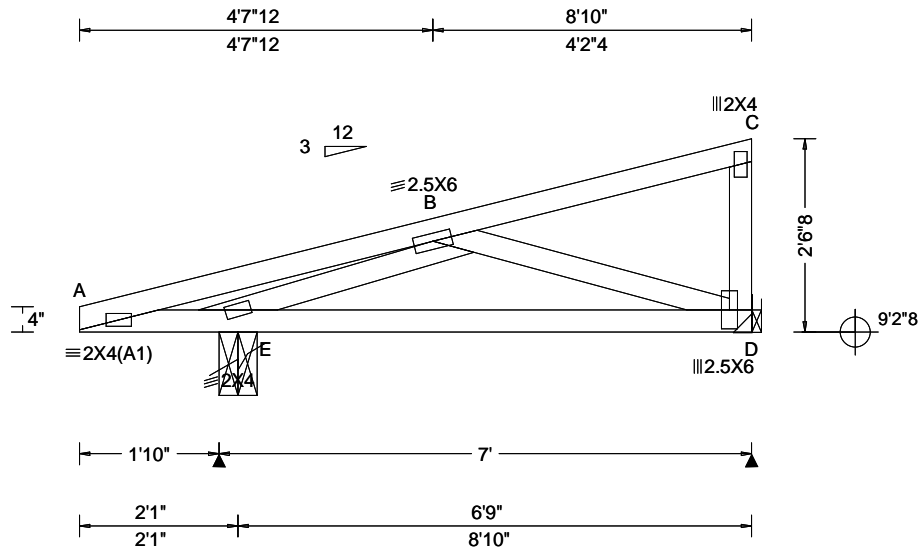
The overall height of this truss excluding overhang is 2-1-0.



COA #0 278
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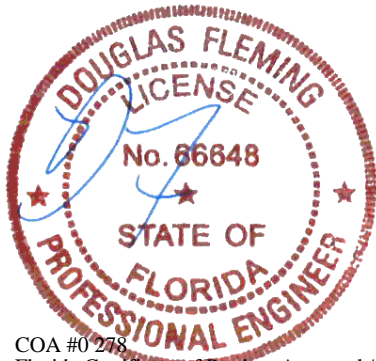
| | | | | |
|--|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.008 B 999 240 VERT(CL): 0.016 B 999 180 HORZ(LL): 0.003 D - - HORZ(TL): 0.005 D - - Creep Factor: 2.0 Max TC CSI: 0.372 Max BC CSI: 0.400 Max Web CSI: 0.293 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 526 /- /- /284 /150 /77 D 290 /- /- /171 /69 /- Wind reactions based on MWFRS E Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 386 -664 |
|--|---|--|---|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 2'-6-8.

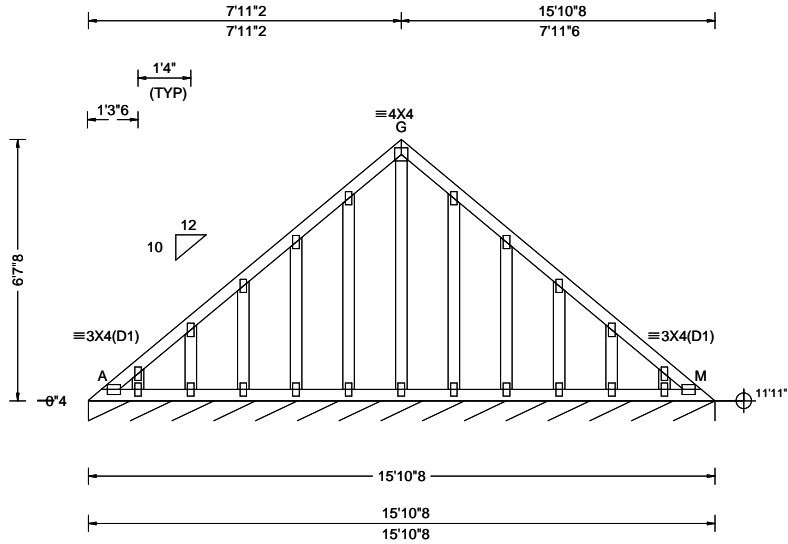


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| | | | |
|---------------------------|--------------------------|--|--|
| SEQN: 704948 FROM: RFG | GABL Ply: 1 Qty: 1 | Job Number: 25-3040 ANDERSON PROJECT Truss Label: V1 | Cust: R215 JRef: 1YGX2150009 T48 DrwNo: 019.26.1559.18440 NW / DF 01/19/2026 |
|---------------------------|--------------------------|--|--|



| | | | | |
|--|--|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.38 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 5.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.002 H 999 180 HORZ(LL): -0.000 B - - HORZ(TL): 0.004 F - - Creep Factor: 2.0 Max TC CSI: 0.048 Max BC CSI: 0.034 Max Web CSI: 0.835 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A* 116 /- /- /65 /26 /16 Wind reactions based on MWFRS A Brg Wid = 190 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |
|--|--|--|--|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 4.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

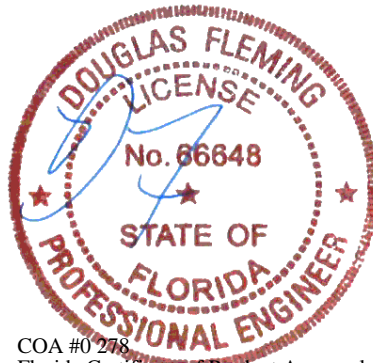
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/302.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
The overall height of this truss excluding overhang is 10-6-8.

See DWGS VALTN220723 and VAL180220723 for valley details.

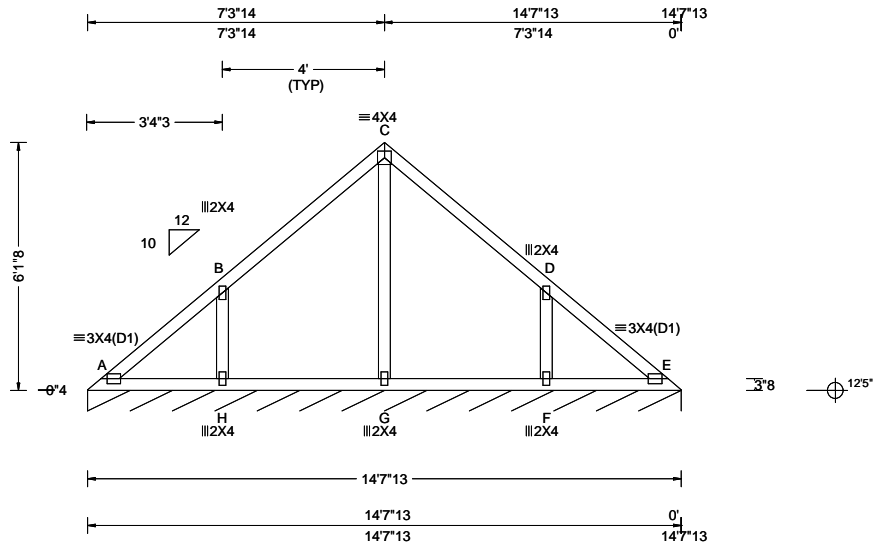


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| | | | |
|---------------------------|--------------------------|--|--|
| SEQN: 704946 FROM: RFG | COMN Ply: 1 Qty: 1 | Job Number: 25-3040 ANDERSON PROJECT Truss Label: V2 | Cust: R215 JRef: 1YGX2150009 T41 DrwNo: 019.26.1559.32160 NW / DF 01/19/2026 |
|---------------------------|--------------------------|--|--|



| | | | | |
|--|---|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.62 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.004 E 999 240 VERT(CL): 0.007 E 999 180 HORZ(LL): -0.002 E - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.336 Max BC CSI: 0.191 Max Web CSI: 0.161 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 127 /- /- /60 /22 /14 Wind reactions based on MWFRS A Brg Wid = 175 Min Req = - Bearing A Fcperp = 565psi. Members not listed have forces less than 375# |
|--|---|--|--|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

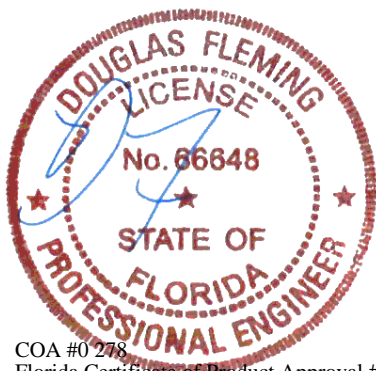
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10-6-8.

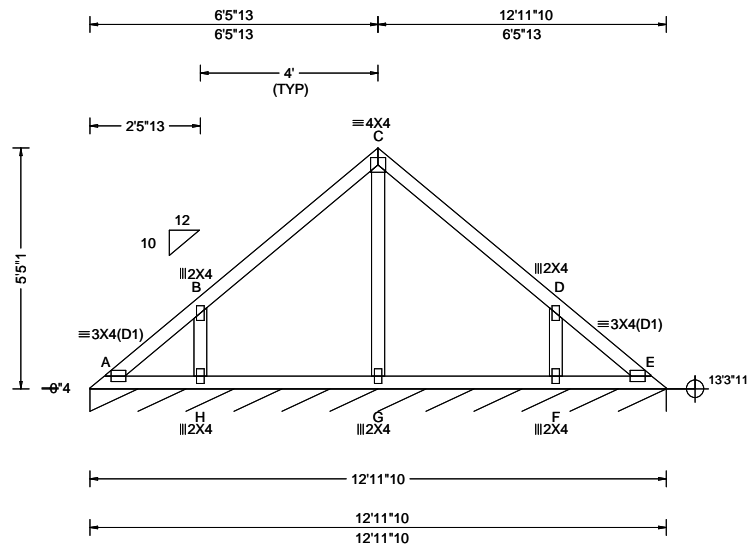
See DWGS VALTN220723 and VAL180220723 for valley details.



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| | | | | |
|---|---|---|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCCL: 20.00 TCCL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.17 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.264 Max BC CSI: 0.117 Max Web CSI: 0.104 VIEW Ver: 24.02.00D.0114.10 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 99 /- /- /60 /22 /14 Wind reactions based on MWFRS E Brg Wid = 155 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |

Lumber

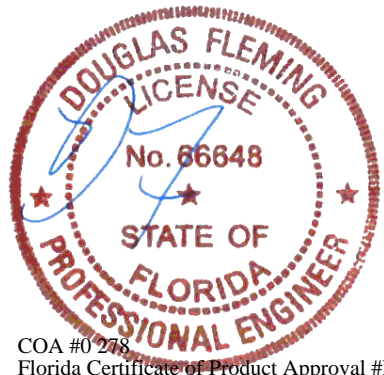
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.
 The overall height of this truss excluding overhang is 5-5-1.

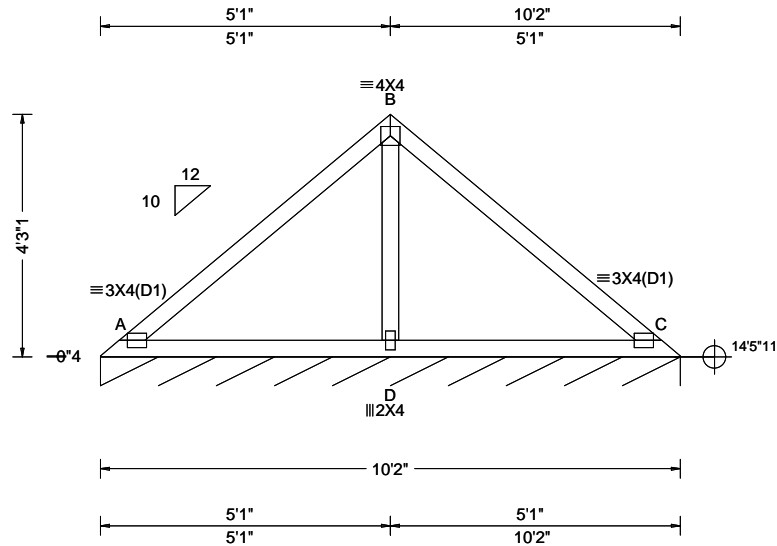


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| | | | |
|---------------------------|-------------------------|--|--|
| SEQN: 722259 FROM: RFG | VAL Ply: 1 Qty: 1 | Job Number: 25-3040 ANDERSON PROJECT Truss Label: V4 | Cust: R215 JRef: 1YGX2150009 T51 DrwNo: 019.26.1559.37987 NW / DF 01/19/2026 |
|---------------------------|-------------------------|--|--|



| Loading Criteria (psf) TCCL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.75 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.016 A 999 240 VERT(CL): 0.028 A 999 180 HORZ(LL): -0.009 C - - HORZ(TL): 0.024 C - - Creep Factor: 2.0 Max TC CSI: 0.445 Max BC CSI: 0.358 Max Web CSI: 0.213 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>98</td> <td>/-</td> <td>/-</td> <td>/59</td> <td>/21</td> <td>/13</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 121 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - D</td> <td>500 - 657</td> </tr> </tbody> </table> | Gravity | | | Non-Gravity | | | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | C* | 98 | /- | /- | /59 | /21 | /13 | Webs | Tens.Comp. | B - D | 500 - 657 |
|--|---|--|--|---|---------|-----|--|-------------|--|--|-----|----|-----|-----|-----|----|-----|----|----|----|----|-----|-----|-----|------|------------|-------|-----------|
| Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | |
| C* | 98 | /- | /- | /59 | /21 | /13 | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - D | 500 - 657 | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

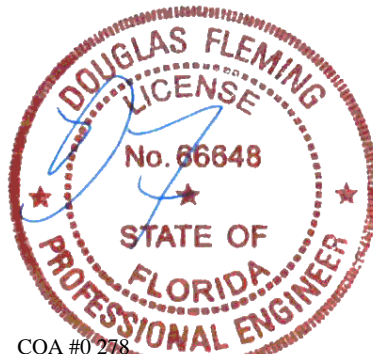
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

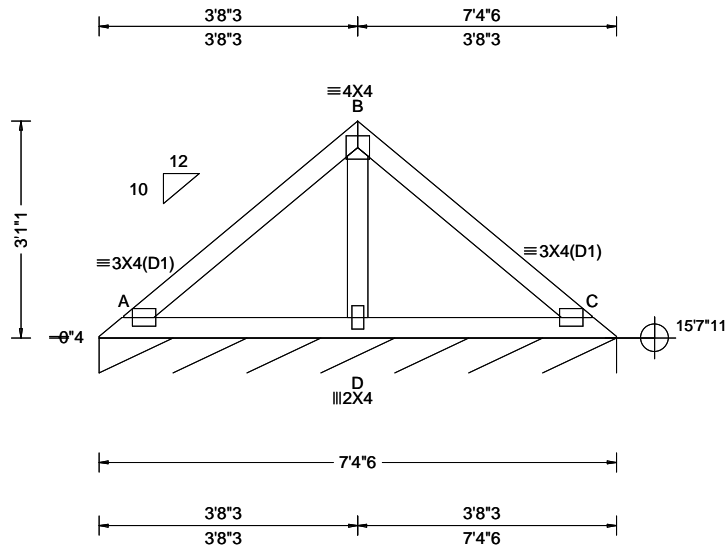
See DWGS VALTN220723 and VAL180220723 for valley details.
 The overall height of this truss excluding overhang is 4-3-1.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





| Loading Criteria (psf) TCLL: 20.00 TCCL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 17.34 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.004 C 999 240 VERT(CL): 0.011 C 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.256 Max BC CSI: 0.172 Max Web CSI: 0.112 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>98</td> <td>/-</td> <td>/-</td> <td>/58</td> <td>/9</td> <td>/13</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 88.4 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - D</td> <td>376 -396</td> </tr> </tbody> </table> | Gravity | | | Non-Gravity | | | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | C* | 98 | /- | /- | /58 | /9 | /13 | Webs | Tens.Comp. | B - D | 376 -396 |
|--|--|--|--|--|---------|-----|--|-------------|--|--|-----|----|-----|-----|-----|----|-----|----|----|----|----|-----|----|-----|------|------------|-------|----------|
| Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | |
| C* | 98 | /- | /- | /58 | /9 | /13 | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - D | 376 -396 | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

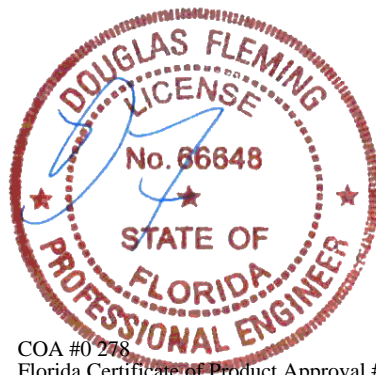
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.
 The overall height of this truss excluding overhang is 3-1-1.

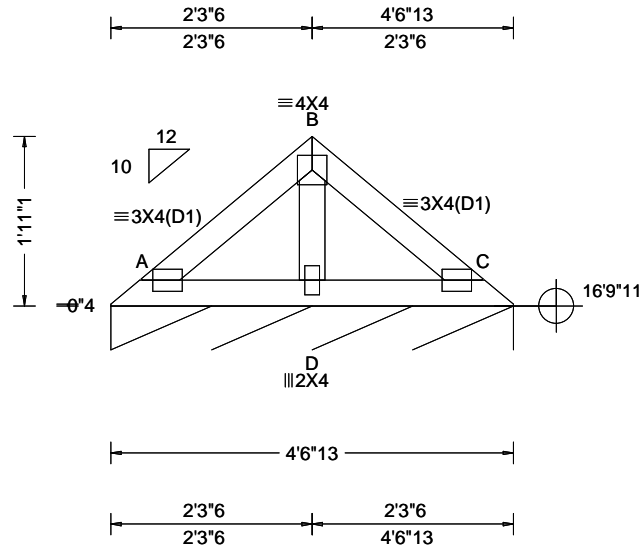


COA #0 278
 Florida Certificate of Product Approval #FL1999
 01/19/2026

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| | | | |
|---------------------------|-------------------------|--|--|
| SEQN: 722261 FROM: RFG | VAL Ply: 1 Qty: 1 | Job Number: 25-3040 ANDERSON PROJECT Truss Label: V6 | Cust: R215 JRef: 1YGX2150009 T54 DrwNo: 019.26.1559.40390 NW / DF 01/19/2026 |
|---------------------------|-------------------------|--|--|



| | | | | |
|--|--|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 140 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 17.92 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 240 VERT(CL): 0.002 A 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.089 Max BC CSI: 0.061 Max Web CSI: 0.058 VIEW Ver: 24.02.00D.0114.10 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 98 /- /- /56 /8 /12 Wind reactions based on MWFRS C Brg Wid = 54.8 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |
|--|--|--|--|--|

Lumber

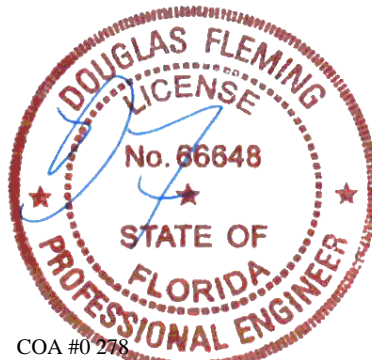
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.
The overall height of this truss excluding overhang is 1-11-1.



COA #0 278
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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

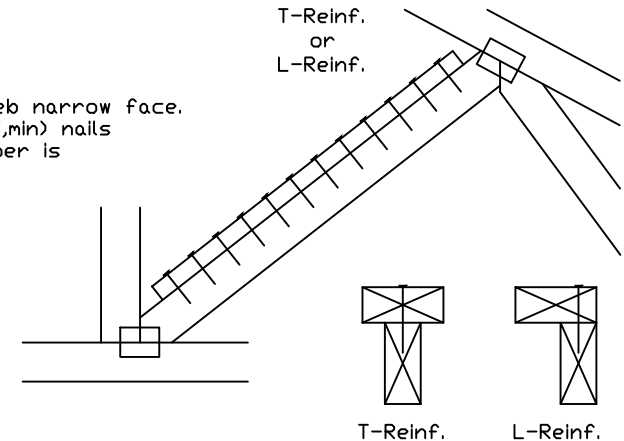
| Web Member Size | Specified CLR Restraint | Alternative Reinforcement T- or L- Reinf. | Scab Reinf. |
|-----------------|-------------------------|---|-------------|
| 2x3 or 2x4 | 1 row | 2x4 | 1-2x4 |
| 2x3 or 2x4 | 2 rows | 2x6 or 2x4 | 2-2x4 |
| 2x6 | 1 row | 2x4 | 1-2x6 |
| 2x6 | 2 rows | 2x6 | 2-2x4(*) |
| 2x8 | 1 row | 2x6 | 1-2x8 |
| 2x8 | 2 rows | 2x6 | 2-2x6(*) |

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

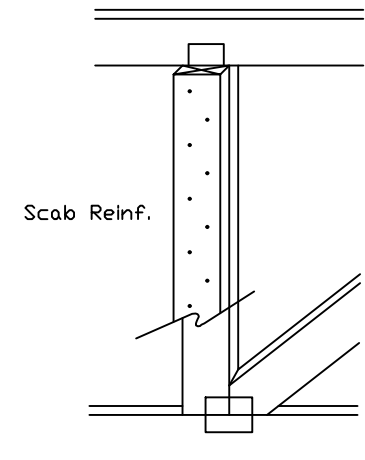
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



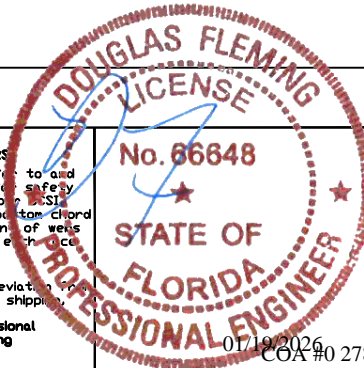
Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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| | | |
|-----------|-----|-------------------|
| TC LL | PSF | REF CLR Subst. |
| TC DL | PSF | DATE 01/02/19 |
| BC DL | PSF | DRWG BRCLBSUB0119 |
| BC LL | PSF | |
| TOT. LD. | PSF | |
| DUR. FAC. | | |
| SPACING | | |

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

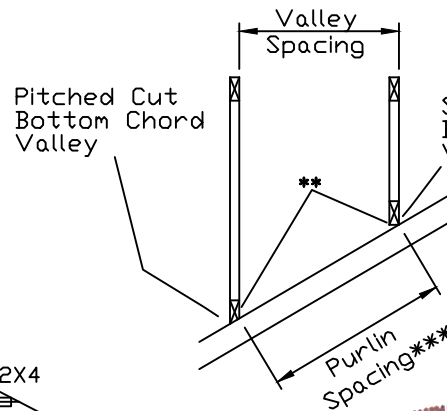
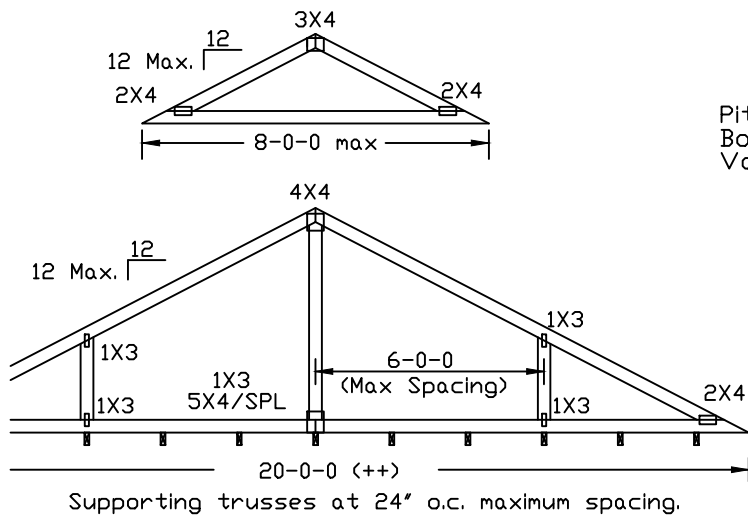
Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Or
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

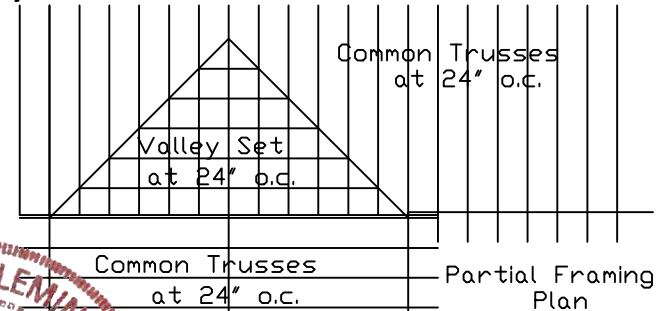
++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



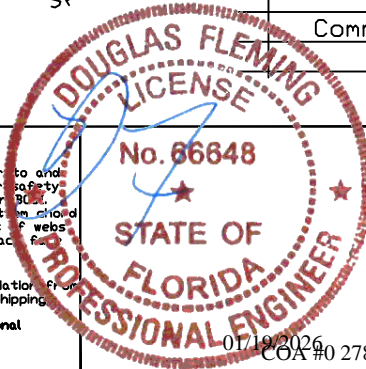
Square Cut Bottom Chord Valley

Stubbed Valley End Detail

Optional Hip Joint Detail



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| | | | | | |
|-------------------|-------|------|--------|------|---------------|
| TC LL | 30 | 30 | 40PSF | REF | VALLEY DETAIL |
| TC DL | 20 | 15 | 7PSF | DATE | 07/03/2023 |
| BC DL | 10 | 10 | 10 PSF | DRWG | VAL180220723 |
| BC LL | 0 | 0 | 0PSF | | |
| TOT. LD. | 60 | 55 | 57PSF | | |
| DUR.FAC.1.25/1.33 | 1.15 | 1.15 | | | |
| SPACING | 24.0" | | | | |

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 140 mph for SP (G = 0.55, min.),
 125 mph for DF-L (G = 0.50, min.), or
 105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

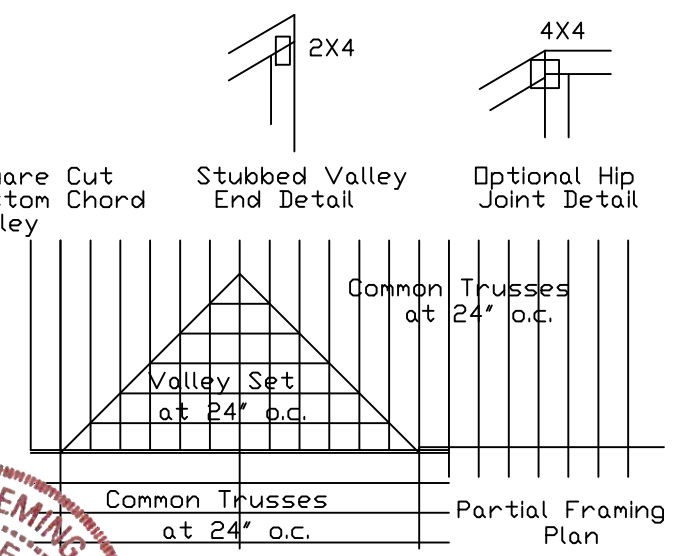
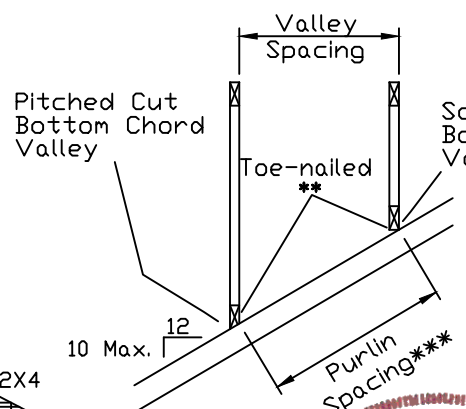
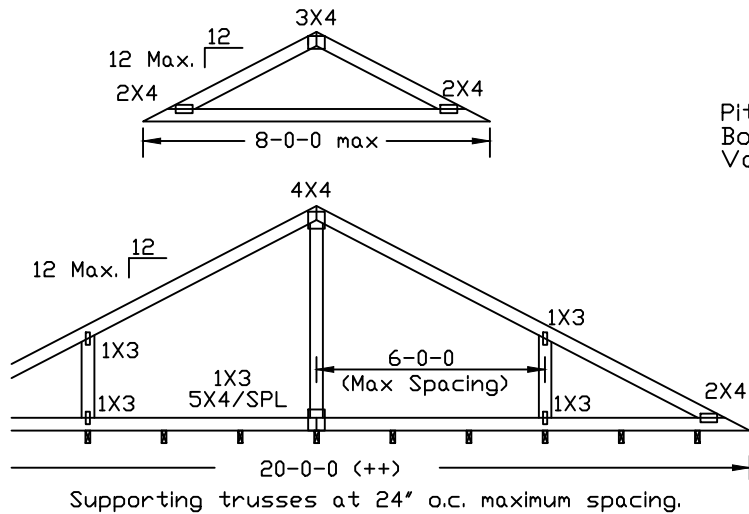
Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

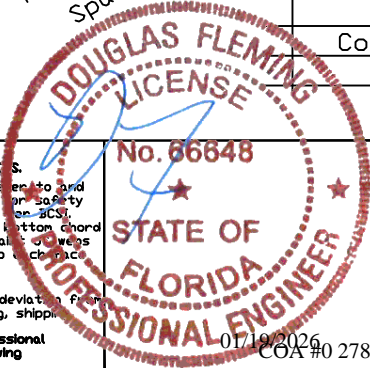
- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER.
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.
 Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



| | | | | | |
|----------|-----------|------|--------|------|---------------|
| TC LL | 30 | 30 | 40PSF | REF | VALLEY DETAIL |
| TC DL | 20 | 15 | 7PSF | DATE | 07/03/2023 |
| BC DL | 10 | 10 | 10 PSF | DRWG | VALTN220723 |
| BC LL | 0 | 0 | 0 PSF | | |
| TOT. LD. | 60 | 55 | 57PSF | | |
| DUR.FAC. | 1.25/1.33 | 1.15 | 1.15 | | |
| SPACING | 24.0" | | | | |