



ENGINEERING • INSPECTIONS
CERTIFICATIONS • TESTING

1 SET !!

May 26, 2021

Champion Home Builders, Inc.
P. O. Box 2097
Lake City, FL 32025

RE: Manufacturer: Champion Home Builders, Inc.
S/N Size & Occupancy: C-MR9673A, 29' X 66'-8"; SFD
HWC Plan#: 2425-0826F

To Whom It May Concern:

This is to certify that the plans for the referenced manufactured building have been reviewed and approved as being in compliance with the 2020 Florida Codes as noted on the approved drawings, subject to the following limitations:

1. Approval covers factory-built structure only. (Note: Any alterations to factory built structure on site voids state approval)
2. Items installed at the site are subject to review, approval, and inspection by the local authority having jurisdiction.
3. The Chapter 633 Plan Review and Inspection shall be conducted by the local fire safety inspector.
4. Signed and sealed plans shall be on file with HWC Engineering.
5. NOT Approved for High Velocity Hurricane Zone (i.e. Broward and Dade Counties)

Sincerely,
HILBORN, WERNER, CARTER & ASSOCIATES, INC.

Plan Reviewer

HILBORN, WERNER, CARTER AND ASSOCIATES, INC.
1827 SOUTH MYRTLE AVENUE CLEARWATER, FLORIDA 33756
(727) 584-8151
FAX: (727) 588-3343 / (727) 585-2392 / (727) 587-0447
Modular Dept Inspection

CHAMPION

MANUFACTURED BEAUTIFULLY™

CHAMPION HOME BUILDERS, INC.
P.O. BOX 2097
(1915 SE STATE ROAD 100)
LAKE CITY, FL 32056

SHEET INDEX

- C-101 COVER SHEET
- GE-101 GENERAL NOTES
- GE-102 ELECTRIC PANEL
- EV-101 ELEVATIONS
- AE-101 FLOOR PLAN
- IM-101 DUCT PLAN
- D-101 DWY PLAN
- W-101 WATER LINE PLAN
- SE-101 TYPICAL CROSS SECTION
- F-101 PIER FOUNDATION PLAN

SHEET INDEX CONT.

- SW-101 ROOF DIAPHRAGM-SHEARWALL LAYOUT
- SW-102 SHEARWALL DETAILS
- SW-103 CONNECTION DETAIL

C-MR9673A
1,933 Sq.Ft.

CHAMPION

MANUFACTURED BEAUTIFULLY™

P.O. BOX 2097 HWY 100 EAST LAKE CITY, FL 32056

CODE SUMMARY:

STATE	RESIDENTIAL	ELECTRICAL	MECHANICAL	PLUMBING	ENERGY	FIRE PREVENTION	FUEL	ACCESSIBILITY
FLORIDA	2020 Florida Residential Building Code 7th Edition	2017 National Electrical Code	2020 Florida Building Code 7th Edition	2020 Florida Building Code Energy Conservation 7th Edition	2020 Florida Residential Building Code 7th Edition	2020 Florida Fire Prevention Code 7th Edition	2020 Florida Residential Building Code 7th Edition	2020 Florida Accessibility 7th Edition (Incorporates 2010 ADA)

D-5126 Foundation No. 2425-0826 F
Approved By SCOTT S. FRANCIS

Scott S. Francis
Professional Engineer
Florida License No. SWP-42

1-STORY TRUSS PACKAGE UNIVERSAL FOREST PRODUCTS 14'-6" WIDE MODULES	
TRUSS # / FINISH	ATTC ACCESS 86270
HMS8467 (612 Fin)	Yes
HMS9458 (Trey)	Yes

NOTES:

ALL MATERIALS COVERED BY THE FLORIDA BUILDING COMMISSION 61 G20-3.006 RULES SHALL HAVE CURRENT FLORIDA PRODUCT APPROVAL. DATA PLATE AND STATE INSIGNIA'S ARE LOCATED IN OR ON THE PANEL BOX OF THE HOME. SEALED PRINTS ARE ON FILE IN THE OFFICE OF HVC, INC.

PLAN REVIEW AND INSPECTION REQUIRED BY CHAPTER 633 F.S., TO BE HANDLED BY LOCAL FIRE SAFETY INSPECTOR. THIS BUILDING IS ON A PERMANENT FOUNDATION AND IS NOT INTENDED TO BE MOVED ONCE SO INSTALLED.

SEE THE STATE APPROVED CONSTRUCTION PACKAGE FOR ROOF CONSTRUCTION DETAILS AND TYPICAL CONSTRUCTION DETAILS.

ADDITIONAL HINGED ROOF DETAILS MUST BE INCLUDED WHEN HOME IS BUILT WITH HINGED ROOF. ACTUAL CONSTRUCTION METHOD AND PRODUCTS MAY VARY FROM DETAILS CONTAINED IN THIS DOCUMENT PROVIDED THE METHOD OF CONSTRUCTION AND PRODUCTS ARE ADDRESSED IN THE DBPR QUALITY ASSURANCE AND BUILDING SYSTEMS MANUAL LOCATED AT THE MANUFACTURING FACILITY.

NOTE:

THIS LIST OF ITEMS MUST NECESSARILY QUOTE THE TRADE WORK AND MATERIALS THAT MAY BE REQUIRED FOR A COMPLETE INSTALLATION. ALL SITE RELATED ITEMS ARE SUBJECT TO LOCAL JURISDICTION APPROVAL.

1. THE COMPLETE FOUNDATION SUPPORT AND THE DOWN SYSTEM.
2. RAMPS, STAIRS AND GENERAL ACCESS TO THE BUILDING.
3. PORTABLE FIRE EXTINGUISHER(S).
4. BUILDING DRAIN, CLEANOUTS, AND HOOK-UP TO PLUMBING SYSTEM.
5. THE MAIN ELECTRICAL PANEL AND ALL ELECTRICAL WIRING.
6. CONNECTION OF ELECTRICAL CIRCUITS CROSSING OVER MODULE MATING LINE(S) - (MULTI-UNITS ONLY).
7. STRUCTURAL AND AESTHETIC INTERCONNECTIONS BETWEEN MODULES (MULTI-UNITS ONLY).
8. DORMERS, AND ANY OTHER AESTHETIC CONNECTIONS.
9. FLOOR INSULATION MUST BE ANCHORED TO THE PERMANENT FOUNDATION.
10. POTABLE WATER SERVICE, MAIN SHUT OFF VALVE.
11. OPENING PROTECTION IN WIND DEBRIS REGIONS (I.E. WINDOWS, DOORS, SHUTTERS).
12. PLAN REVIEW AND INSPECTION REQUIRED BY CHAPTER 633 F.S., TO BE HANDLED BY LOCAL FIRE SAFETY INSPECTOR.
13. ROOF INSULATION FINISHING.
14. AIR UNIT.
15. GAS LINES WILL BE STUBBED OUT. CONNECTIONS AND INSTALLATION TO BE DONE BY OTHERS ON SITE. (WHERE APPLICABLE)
16. A.A.V. (AIR ADMITTANCE VALVE TEST) AFTER DWY TEST
17. COMMUNICATION OUTLET PER NEC 900-156
18. ALL PORTFOLIO ARE TO BE COMPLETED ON SITE PER INCLUDED DETAILS AND LOCAL CODES. (WHERE APPLICABLE)
19. BLOWER FAN TO BE COMPLETED ON SITE PER INCLUDED DETAILS AND LOCAL CODES. (WHERE APPLICABLE)
20. DRYER VENT AND/OR 20A-120 JANUARY RECEIPT PER NEC 410.34(F).
21. DRYER VENT AND/OR 20A-120 JANUARY RECEIPT PER NEC 410.34(F).

BUILDING INSULATION VALUES:

WALLS: R-19
FLOOR: R-11
CEILING: R-30
WINDOWS: U = 0.30 / SHGC = 0.25

12.89 SQUARE FEET NET FREE AREA OF ATTIC VENTILATION TO BE PROVIDED BY SOFFIT AND RIDGE VENTS/ROOF VENTS.

12.89 SQUARE FEET NET FREE AREA OF CRAWL SPACE VENTILATION TO BE PROVIDED BY FOUNDATION CONTRACTOR. 1st-FLOOR IS 1,933 SQ. FT. - CONDITIONED FLOOR AREA.

NOTE: COMPLETION OF THIS BUILDING TO BE IN COMPLIANCE WITH ALL STATE AND LOCAL DESIGN CODES. ALL WORK TO BE COMPLETED BY A LICENSED CONTRACTOR AND INSPECTED BY A LOCAL BUILDING OFFICIAL.

NOTE: STRUCTURE HAS BEEN DESIGNED FOR INSTALLATION ON SITE - BUILT PERMANENT FOUNDATION AND IS NOT INTENDED TO BE MOVED, ONCE SO INSTALLED.

APPROVER'S SEAL

AGENCY APPROVAL

UNIVERSAL FOREST PRODUCTS

14'-6" WIDE MODULES

TRUSS # / FINISH

ATTC ACCESS 86270

HMS8467 (612 Fin)

HMS9458 (Trey)

05/21/2021

MODIFICATIONS

PROJECT: 261-C-MR9673A

66'-8" x 29'-0"

3 BD 2 BT

TITLE: COVER SHEET

DRAWN BY: BOB

SCALE: 1/2"=1'-0"

FILE NAME: C-H6573A

SHEET: C-101

2425-0826F

APPROVER'S SEAL

STATE OF FLORIDA

PROFESSIONAL ENGINEER

SCOTT S. FRANCIS

No. 77466

05/21/2021

PROPRIETARY AND CONFIDENTIAL

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2020 FLORIDA RESIDENTIAL BUILDING CODE 7TH EDITION

FLORIDA STRUCTURAL LOAD LIMITATIONS: EXPOSURE C

- MEAN ROOF HEIGHT: 10 FT 0 IN
- FLOOR SLAB LOAD: 40 PSF
- ROOF LIVE LOAD: 1.50 PSF MIN
- WIND LOAD: 1.50 PSF MIN
2. MIN 1.0
3. C
4. D.M.P.F. FOR GC (W.D) PSF
- PH = ROOF COMPONENT CLADDING LOAD (PSF)
- ZONE 1 = 20 PSF
- ZONE 2 = 25 PSF
- ZONE 3 = 30 PSF
- ROOF OVERHANG: 2.0 FT
- ZONE 1 = 41.1 PSF
- ZONE 2 = 46.1 PSF
- ZONE 3 = 51.1 PSF
- PH = WALL COMPONENT & CLADDING LOAD (PSF)
- WALL ZONE 1 = 31.2 PSF
- WALL ZONE 2 = 36.2 PSF
- WALL ZONE 3 = 41.2 PSF
- PH = WALL CLADDING LOAD (PSF)
- WALL ZONE 1 = 38.8 PSF
- WALL ZONE 2 = 43.8 PSF
- WALL ZONE 3 = 48.8 PSF
6. THIS BUILDING IS NOT OCCUPIED FOR PLACEMENT OR THE OF A HELL OR ESCAPATOR EXCEEDING 15 FEET IN HEIGHT.
7. ELEVATION SHALL NOT BE INSTALLED BEYOND THE BASE FLOOD ELEVATION IN ANY FLOOD HAZARD AREA OR ZONE, BUT THE FOUNDATION SHALL BE DESIGNED AND CONSTRUCTED TO WITHSTAND THE DESIGN WIND SPEED AND PRESSURE LOADS APPLIED TO THE FOUNDATION SHALL BE TRANSFERRED TO THE HOME CONSTRUCTION CONTROL LINE.

2020 FLORIDA RESIDENTIAL BUILDING CODE 7TH EDITION

1. ALL GLAZING WITHIN 24 INCH ARC OF DOORS, WHOSE BOTTOM EDGE IS LESS THAN 50 INCHES ABOVE THE FLOOR, AND ALL GLAZING IN DOORS SHALL BE SAFETY, TEMPERED OR ACRYLIC PLASTIC SHEET.
2. OCCUPANT LOAD IS BASED ON 1 PERSON PER 200 SQUARE FEET OF FLOOR AREA.
3. MINIMUM CORRIDOR WIDTH IS 36 INCHES.
4. WINDOWS, GLASS, DOORS, SHALL COMPLY WITH AAMA / NWDA 101 / LS.2.
5. ALL MATERIALS USE IN THIS CONSTRUCTION OF THE BUILDING WHICH ARE COVERED BY THE FLORIDA BUILDING COMMISSION CHAPTER 61 G20-3.005 RULES SHALL HAVE CURRENT FLORIDA PRODUCT APPROVAL SPECIFIED ON THE DRAWINGS.
6. ALL CONSTRUCTION, MATERIALS AND INSTALLATION SHALL BE IN ACCORDANCE WITH THE CODES SPECIFIED ON THE DRAWINGS.

2020 FLORIDA RESIDENTIAL BUILDING CODE ENERGY CONSERVATION 7TH EDITION

1. LIGHTING - (R404.1) 90% OF PERMANENTLY INSTALLED LIGHTING FIXTURES OR LAMPS SHALL BE HIGH EFFICACY LAMPS

MECHANICAL NOTES: 2020 FLORIDA RESIDENTIAL BUILDING CODE 7TH EDITION

1. ALL SUPPLY AIR REGISTERS ARE ADJUSTABLE EXCEPT WHERE OTHERWISE SPECIFIED.
2. INTERIOR DOORS SHALL BE UNDERCUT 1.5 INCHES ABOVE FINISHED FLOOR FOR AIR RETURN AND/OR AS NOTED ON FLOOR PLAN. PRESSURE DIFFERENTIAL ACROSS CLOSED DOORS SHALL BE LIMITED TO .01 INCH WC (2.5 PASCAIS) OR LESS.
3. RESTROOM VENT FANS SHALL PROVIDE 50 CFM MINIMUM OF VENTILATION INTERMITTENT OR 20 CFM CONTINUOUS.
4. VENT FANS SHALL BE DUCTED TO THE EXTERIOR AND TERMINATE AT AN APPROVED VENT CAP.
5. IF THIS BUILDING IS LOCATED IN A JURISDICTION THAT HAS ADOPTED THE ORDINANCE FOR RADON-RESISTANT CONSTRUCTION, RULE 98-52, FLORIDA STATUTE 553.98, RETURN DUCTS AND PLENUMS SHALL NOT BE LOCATED IN THE CRAWL SPACES.
6. MECHANICAL VENTILATION SHALL BE PROVIDED PER R403.6 OF THE RES. ENERGY CODE.

RIDGE BEAM CONSTRUCTION NOTES

1. LV. F # = 2800 PSI, MOE = 2,000,000 PSI., MICROCOLLUM 2.0 SP OR BETTER.
2. LV. OR OTHER SIMILAR MATERIAL MUST BE CONTINUOUS OVER CLEARSPAN(S).
3. BEAMS SUPPORTED BY ENDWALL COLUMNS MUST EXTEND CONTINUOUS OVER COLUMNS TO EXTERIOR FACE OF ENDWALL.
4. INSTALL (2M) x 20" SPF #3 RIDGE BEAM BEARING STIFFENER OVER SUPPORT COLUMNS WHEN SPECIFIED ON FLOOR PLAN; FASTEN THE FACE OF THE STIFFENER TO THE RIDGE BEAM WITH 100% GLUE COVERAGE AND 9-15 OR X 2 1/2 STAPLES.

ELECTRICAL NOTES: 2017 NEC

1. ALL CIRCUITS AND EQUIPMENT SHALL BE GROUNDED IN ACCORDANCE WITH THE APPROPRIATE ARTICLES OF THE NATIONAL ELECTRICAL CODE (NEC).
2. WHEN LIGHT FIXTURES ARE INSTALLED IN CLOSETS THEY SHALL BE SURFACE MOUNTED OR RECESSED. INCANDESCENT FIXTURES SHALL HAVE COMPLETELY ENCLOSED LAMPS. SURFACE MOUNTED INCANDESCENT FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 12 INCHES AND ALL OTHER FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 6 INCHES FROM "STORAGE AREA" AS DEFINED BY NEC 410.2.
3. WHEN WATER HEATERS ARE INSTALLED THEY SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATERS SERVED. THE BRANCH CIRCUIT SWITCH OR CIRCUIT BREAKER SHALL BE PERMITTED TO SERVE AS THE DISCONNECTING MEANS ONLY WHERE THE SWITCH OR CIRCUIT BREAKER IS WITHIN SIGHT FROM THE WATER HEATER OR IS CAPABLE OF BEING LOCKED IN THE OPEN POSITION.
4. OVERCURRENT PROTECTION SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATERS. DISCONNECTS ON THE LINE AS PART OF THE HVAC EQUIPMENT AND DISCONNECTS ALL UNGROUNDING CONDUCTORS SHALL BE PROVIDED AS ACCESSIBLE DISCONNECTING MEANS WHERE OTHER DISCONNECTING MEANS ARE ALSO PROVIDED BY A READILY ACCESSIBLE CIRCUIT BREAKER.
5. PRIOR TO ENERGIZING THE ELECTRICAL SYSTEM THE INTERRUPTING RATING OF THE MAIN BREAKER MUST BE DESIGNED AND VERIFIED AS BEING IN COMPLIANCE WITH SECTION 110-9 OF THE NEC BY LOCAL ELECTRICAL CONSULTANT.
6. THE MAIN ELECTRICAL PANEL AND FEEDERS ARE DESIGNED BY OTHERS, SITE INSTALLED AND SUBJECT TO LOCAL JURISDICTION APPROVAL.
7. ALL CIRCUITS CROSSING OVER MODULE MOUNTING LINES(S) SHALL BE SITE CONNECTED WITH APPROVED ACCESSIBLE JUNCTION BOXES, OR CABLE CONNECTORS.
8. ALL OUTLETS LOCATED WITHIN 6 FEET OF A SINK OR BASIN SHALL BE EQUIPPED WITH GFCI PROTECTION.
9. SMOKE DETECTORS SHALL BE WIRED SO THAT THE OPERATION OF ANY ONE SMOKE DETECTOR WILL CAUSE SIMULTANEOUS ACTIVATION OF ALL OTHERS (IN ANY ONE DWELLING UNIT).
10. PROVIDE COMBINATION SMOKE/CARBON MONOXIDE DETECTORS WHEN ANY FOSSIL FUEL APPLIANCES, FIRE PLACES OR ATTACHED GARAGE ARE PROVIDED. (FLORIDA 98-3.0472) (OUTSIDE OF SLEEPING AREAS)
11. ALL RECEPTACLES INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER PROOF (WP) ENCLOSURES THE INTEGRITY OF WHICH IS NOT AFFECTED WHEN AN ATTACHMENT PLUG CAP IS INSERTED OR REMOVED.
12. BATHROOMS ARE PROVIDED WITH 20 AMPERE OUTLETS IN ALL AREAS EXCEPT BATHROOMS ARE PROTECTED BY AN AFCI-FAULT CIRCUIT INTERRUPTER IN ACCORDANCE WITH SECTION 210.12.2017 NEC.
13. PROVIDE TAMPER RESISTANT RECEPTACLES IN ACCORDANCE WITH SECTION 406.12. 2017 NEC

PLUMBING NOTES: 2020 FLORIDA RESIDENTIAL BUILDING CODE 7TH EDITION

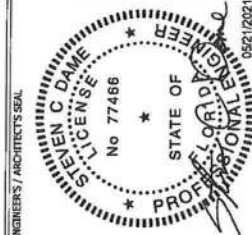
1. ALL PLUMBING FIXTURES SHALL HAVE SEPARATE SHUT-OFF VALVES.
2. WATER HEATER SHALL HAVE SAFETY PAN WITH 1 INCH DRAIN TO EXTERIOR, T & P RELIEF VALVE WITH DRAIN TO EXTERIOR, AND A SHUT OFF VALVE WITHIN 3 FEET ON A COLD WATER SUPPLY LINE.
3. WATER PIPES INSTALLED IN A WALL EXPOSED TO THE EXTERIOR SHALL BE LOCATED ON THE HEATED SIDE OF THE WALL INSULATION. WATER PIPING INSTALLED IN AN UNCONDITIONED ATTIC SHALL BE INSULATED.
4. DWV SYSTEM SHALL BE PVC - DWV.
5. WATER SUPPLY LINES SHALL BE PEX WITH THE MAXIMUM WATER HEATER TEMPERATURE SETTING IS 180° F. THE PEX PIPE SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS LIMITATIONS AND INSTRUCTIONS.
6. TUB DRAIN, SINK AND CLEANSERS ARE DESIGNED AND SITE INSTALLED BY OTHERS, SUBJECT TO LOCAL JURISDICTION APPROVAL.
7. TUB ACCESS PROVIDED UNDER HOME, UNLESS OTHERWISE NOTED.
8. SHOWER STALLS SHALL BE COVERED WITH NON-ABSORBENT MATERIAL TO A HEIGHT OF 72 INCHES.
9. SHOWERS SHALL BE CONTROLLED BY AN APPROVED MIXING VALVE WITH A MAXIMUM WATER OUTLET TEMPERATURE OF 120° F. (48.8° C)
10. THERMO EXPANSION DEVICE, IF REQUIRED BY WATER HEATER MANUFACTURER SHALL BE SITE INSTALLED BY OTHERS, SUBJECT TO LOCAL APPROVAL.
11. CHECK VALVES FOR WATER PRESSOR SHALL BE INSTALLED WHERE QUICK-CLOSING VALVES ARE UTILIZED, UNLESS THE MANUFACTURER'S SPECIFICATIONS.
12. THIS UNIT MUST BE CONNECTED TO PUBLIC WATER SUPPLY AND SEWER SYSTEM IF THESE ARE AVAILABLE.
13. BACKFLOW PREVENTERS ARE TYPICAL ON ALL EXTERIOR HOSE BIBS
14. HOT WATER PIPE WHEN REQUIRED BY R403.5.3 OF THE RES. ENERGY CODE SHALL BE INSULATED WITH R3 INSULATION
15. HEAT TRAPS TO BE PROVIDED - RES. ENERGY CODE (R-403.5.5)

FOUNDATION:

FOUNDATION IS DESIGNED BY OTHERS. DETAILS CONTAINED IN THESE TYPICAL DRAWINGS ARE SUPPLEMENTAL AND MUST BE EVALUATED BY FOUNDATION DESIGNER FOR COMPATIBILITY WITH THE FOUNDATION DESIGN.

CHAMPION

MANUFACTURED BEAUTIFULLY™
P.O. BOX 2097 HWY 100 EAST LAKE CITY, FL 32856



APPROVER'S SEAL

AGENCY APPROVAL
THIS SEAL IS VALID FOR THE PROJECT AND SITE ONLY.
IT IS NOT VALID FOR ANY OTHER PROJECT OR SITE.

PROJECT NO. 261-C-MR9673A
DATE: 03-28-21
SCALE: AS SHOWN
DRAWN BY: BOB
DATE: 03-28-21
SCALE: AS SHOWN
FILE NAME: C-MR9673A

THIRD PARTY
1527 SOUTH WINTER AVE
CLEARWATER, FLORIDA 33755

NO. 77486
STATE OF FLORIDA
EXPIRES 12/31/2021

MODIFICATIONS

PROJECT: 261-C-MR9673A
66'-8" x 29'-0"
3 BD 2 BT

TITLE: GENERAL NOTES

PROJECT: 261-C-MR9673A
66'-8" x 29'-0"
3 BD 2 BT

TITLE: GENERAL NOTES

DRAWN BY: BOB
DATE: 03-28-21
SCALE: AS SHOWN
FILE NAME: C-MR9673A

SHEET: GE-101
2425-0826F

PROFESSIONAL ENGINEER
STEVEN C. DAME
LICENSE NO. 77486
EXPIRES 12/31/2021

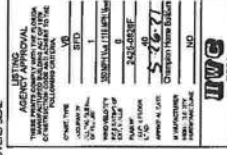
CHAMPION

MANUFACTURED BEAUTIFULLY™
P.O. BOX 2087 HWY 100 EAST LAKE CITY, FL 32068

ENGINEERS / ARCHITECTS SEAL



APPROVERS SEAL



THIRD PARTY:
HEBORN, WENZEL, CARTER
AND ASSOCIATES (P) INC.
1522 SOUTH BRIDGE RD.
CLEARWATER, FLORIDA 33756

MODIFICATIONS

PROJECT: 261-C-MR9673A
66'-8" X 29'-0"
3 BD 2 BT

TITLE: ELECTRICAL PANEL

DRAWN BY: BOB
DATE: 02-22-21
SCALE:
ELEMENT: C-MR9673A

SHEET:

GE-102
2425-0826F

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ELECTRICAL SCHEDULE

PANEL SIZING

BRKR.	NO.	NOMENCLATURE	VOLTS	WIRE CU. NM	DESCRIPTION	KVA
20	1	PORTABLE APPLIANCE	120	12/2 GFCI, AFCI	FLOOR AREA 1,933 SF X 3 VA + 1000 =	5.8
20	2	PORTABLE APPLIANCE	120	12/2 GFCI, AFCI	2 SMALL APPLIANCES AT 1500 VA. + 1000 =	3.0
20	3	GFCI RECEPTACLES	120	12/2 GFCI	BUILT-IN OVEN AT 8.8 KW. DOUBLE =	8.8
15	4	GENERAL LIGHTING	120	14/2 AFCI	COUNTER MOUNTED COOK TOP AT 7.4 KW =	7.4
15	5	GENERAL LIGHTING	120	14/2 AFCI	WATER HEATER AT 5.5 KW =	5.5
15	6	GENERAL LIGHTING	120	14/2 AFCI	DISHWASHER AT 1.4 KW. =	1.4
15	7	GENERAL LIGHTING	120	14/2 AFCI	WASHER AT 1500 VA./1000 =	1.5
15	8	GENERAL LIGHTING	120	14/2 AFCI	DRYER AT 5.0 KW. =	5.0
	9	SPARE			MICROWAVE AT 1.5 KW. =	1.5
20	10	GFCI RECEPTACLES	120	12/2 GFCI, AFCI		
30 (2P)	11	WATER HEATER	240	10/2		
20	12	WASHER	120	12/2 GFCI, AFCI		
30 (2P)	13	DRYER	240	10/3		
20	14	DISHWASHER	120	12/2 GFCI, AFCI		
	15	SPARE				
40 (2P)	16	COOK TOP	240	8/3		
20 (2P)	17	BUILT-IN OVEN (SINGLE)	240	12/3	TOTAL LOAD	39.90
		SPARE			FIRST 10 KVA AT 100%	10.0
	18	SPARE			REMAINDER AT 40%	11.96
	19	HVAC COMPRESSOR	SIZED PER MFR. SPECS.		HVAC AT 100%	15.8
	20	HVAC BLOWER	SIZED PER MFR. SPECS.			
15	21	SMOKE DETECTOR	120	14/3 AFCI	TOTAL	37.76
	22	SPARE			TOTAL X 1000	157.33
	23	SPARE			240	
20	24	MICROWAVE	120	12/2 AFCI	INSTALL 200 AMP PANEL WITH 200 AMP MAIN BREAKER	

CHAMPION
 MANUFACTURED BEAUTIFULLY™
 P.O. BOX 2097 HWY 100 EAST LAKE CITY, FL 32066

ENGINEERS / ARCHITECT'S SEAL
 STEVEN C. DAME
 LICENSE No 77486
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 0821/2021

APPROVER'S SEAL
 LISTING AGENCY APPROVAL
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 0821/2021

THIRD PARTY INSPECTION
 WPC
 1527 SOUTH WINTER AVE
 DEERBEEK, FLORIDA 32926

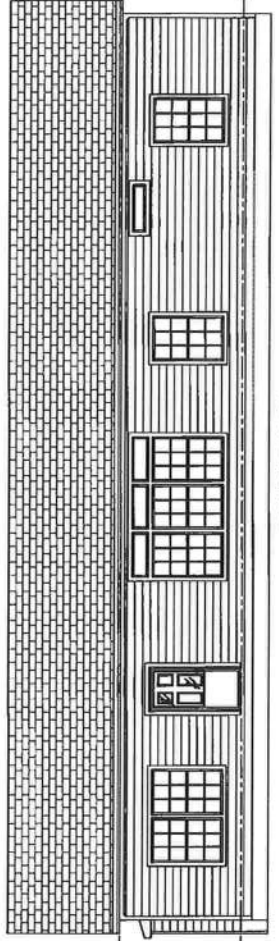
MODIFICATIONS

PROJECT: 201-C-MR9673A
 66'-8" x 29'-0"
 3 BD 2 BT

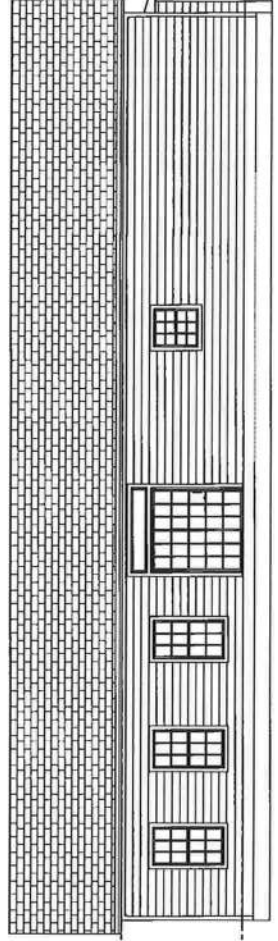
TITLE: ELEVATIONS

DRAWN BY: IOS
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 SCALE:
 PLENOME C-MR9673A
 SHEET:
 EV-101
 2425-0826F

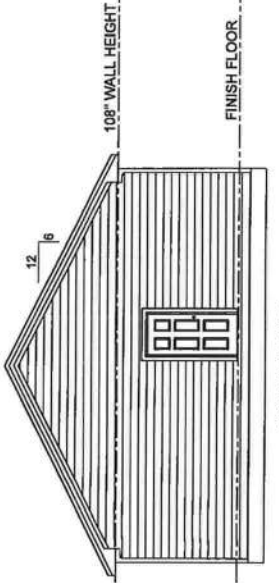
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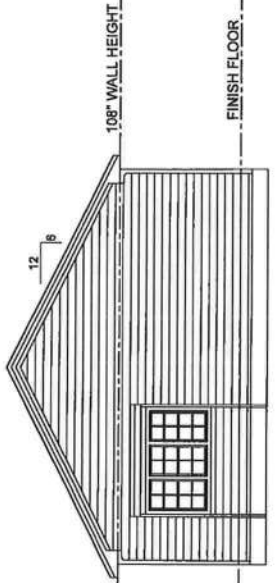
FRONT ELEVATION



REAR ELEVATION



RIGHT ELEVATION



LEFT ELEVATION

108" WALL HEIGHT

FINISH FLOOR

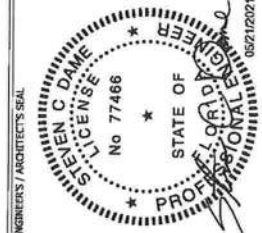
108" WALL HEIGHT

FINISH FLOOR

ELEVATION NOTES:
 NOTE: UNDERPINNING AND STOOPS ON SITE BY OTHERS TYPICAL ONLY
 FOUNDATION ENCLOSURE MUST HAVE 1-SQUARE FOOT NET VENT AREA 1/10TH OF THE FLOOR AREA, AND A 1P/2P MINIMUM CRAWL SPACE ACCESS. SITE INSTALLED BY OTHERS AND SUBJECT TO LOCAL JURISDICTION.
 DRAWINGS MAY NOT DEPICT ACTUAL PRODUCT - OPTIONS MAY VARY.
 IF AREA UNDER THE HOME IS VENTILATED, FLOORING AND OPENINGS IN FLOOR DOORS MUST MEET R302.5, R302.6, R302.7
 **IF WINDOW SILLS ARE LESS THAN 34" ABOVE FINISHED FLOOR AND OVER 7" ABOVE GRADE WINDOW GUARDS ARE REQUIRED. (R312.2.1)

CHAMPION

MANUFACTURED BEAUTIFULLY™
P.O. BOX 2067 HWY 100 EAST LAKE CITY, FL 32056

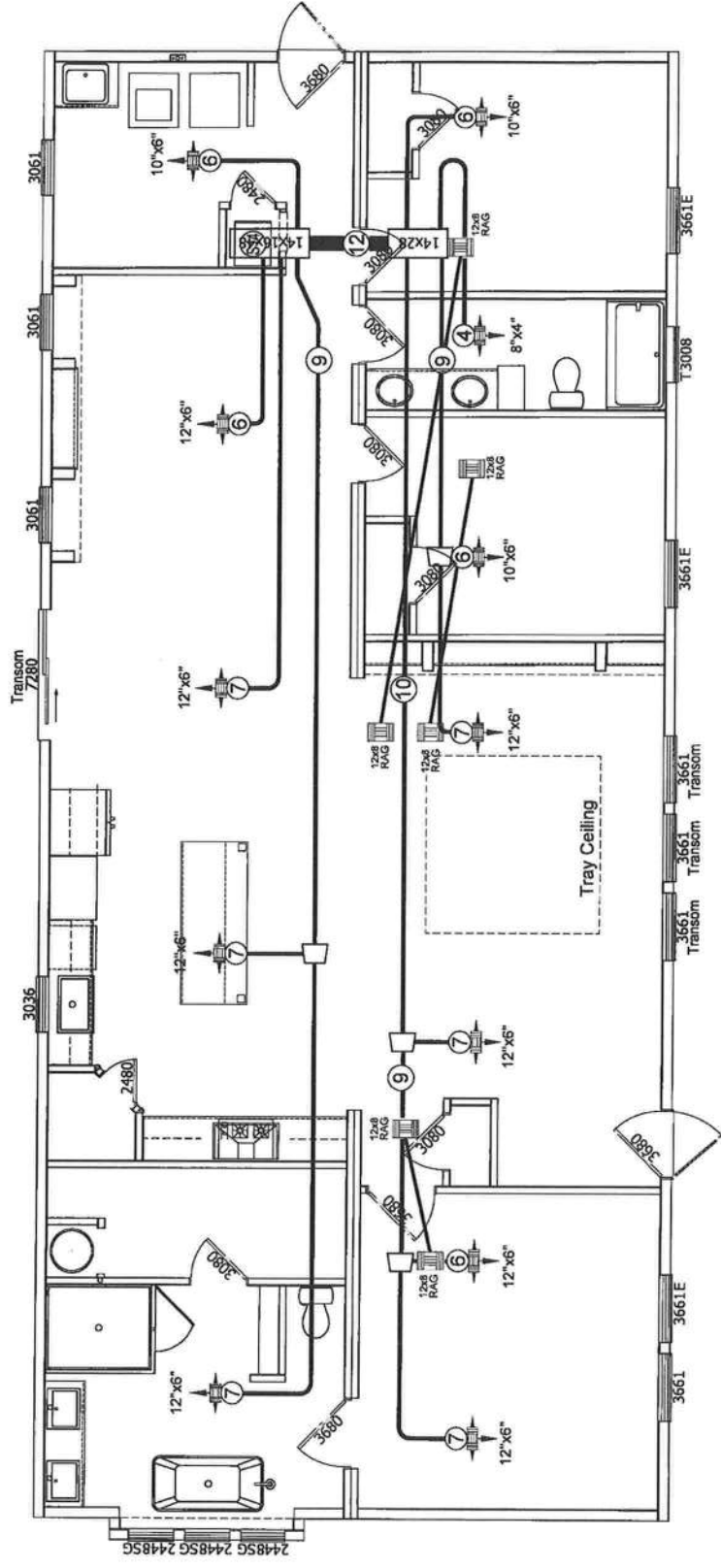


APPROVER'S SEAL
AGENCY APPROVAL
I HAVE REVIEWED THE QUALITY OF THIS PROJECT AND I AM NOT PROVIDING A GUARANTEE.
DATE: 05/21/2021
PROJECT: 261-C-MR9673A
SHEET: 3 BD 2 BT
DRAWN BY: EOB
DATE: 03-24-21
SCALE:
FILENAME: C-MR9673A



THIRD PARTY:
FISHER, WALKER, CARTER
1527 SOUTH MERILE AVE.
CLEARWATER, FLORIDA 33756
DATE: 03-24-21
PROJECT: 261-C-MR9673A
SHEET: 3 BD 2 BT
DRAWN BY: EOB
DATE: 03-24-21
SCALE:
FILENAME: C-MR9673A

PROJECT: 261-C-MR9673A
66'-8" x 29'-0"
3 BD 2 BT
TITLE: MECHANICAL PLAN
1ST FLOOR

DRAWN BY: EOB
DATE: 03-24-21
SCALE:
FILENAME: C-MR9673A
SHEET: M-101
2425-0826F
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NOTICE
CROSSOVER DUCTS MAY BE LOCATED IN THE CEILING.
IF THIS BUILDING IS LOCATED IN A JURISDICTION THAT HAS ADOPTED THE ORDINANCE FOR
RADON-RESISTANT CONSTRUCTION, RULE -9B-52, FLORIDA STATUTE 553.98, RETURN DUCTS
AND PLENUMS SHALL NOT BE LOCATED IN THE CRAWL SPACES

CHAMPION	MANUFACTURED BEAUTIFULLY™ P.O. BOX 2897 HWY 100 EAST LAKE CITY, UT 84056	ENGINEER'S SEAL 	APPROVER'S SEAL 	PROJECT: 261-C-MR9673A 66'-8" X 29'-0" 3 BD 2 BT	TITLE: DRAIN LINE 1ST FLOOR
				DRAWN BY: BOB DATE: 03-24-21 SCALE: FILENAME: C-082673A	
				SHEET: D-101 2425-0826F	PROPRIETARY AND CONFIDENTIAL THESE DRAWINGS AND SPECIFICATIONS ARE OWNED BY HILBORN, WEHNER, CAPRETT & ASSOCIATES, INC. COPYRIGHT © 1979-2021 BY COMPANY

SHIP LOOSE

- | | |
|--------|--------------------|
| 45'-0" | 3" PIPE |
| 25'-0" | 2" PIPE |
| 20'-0" | 1 1/2" PIPE |
| 1 | 3" CLEAN-OUT |
| 1 | 3" COUPLER |
| 1 | 3" LTTY |
| 3 | 3"x3"x3" SAN TEE |
| 1 | 3"x 1 1/2" REDUCER |
| 1 | 1 1/2" LT 90 |
| 1 | 2" COUPLER |
| 1 | 3" X 2" REDUCER |
| 1 | 2" CLEAN-OUT |
| 1 | 2" LTTY |

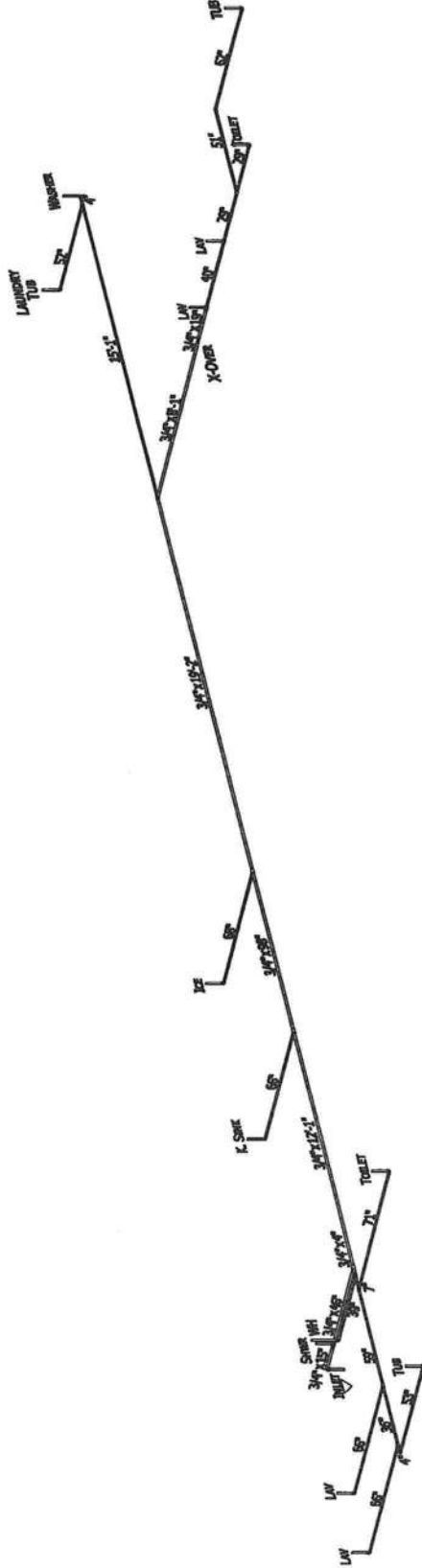


DWV PLUMBING PLAN

NOTES:
 -ALL PIPE SIZES ARE 1/2" UNLESS OTHERWISE SPECIFIED
 -ALL TUBS HAVE OVERFLOWS

COLD WATER LINE

** Hose Bibbs Require Backflow Preventers



**HOT WATER LINE
WATER SUPPLY PLAN**

NOTES:
-ALL PIPE SIZES ARE 1/2" UNLESS OTHERWISE SPECIFIED
-BASED ON 48 PSI

CHAMPION

MANUFACTURED BEAUTIFULLY™
P.O. BOX 2087 HWY 100 EAST LAKE CITY, FL 32085



APPROVER'S SEAL
AGENCY APPROVAL
I HEREBY APPROVE THE WORK AND PROFESSIONAL LIABILITY OF THE ENGINEER AND ARCHITECT FOR THE FOLLOWING PROJECT:
PROJECT: 261-C-MR9673A
DATE: 03-24-21
SCALE: 1/8"=1'-0"
DRAWN BY: EDR
DATE: 03-24-21
SCALE: 1/8"=1'-0"
FILENAME: C-MR9673A

THIRD PARTY
REVISIONS
NO. 1
DATE: 03-24-21
BY: EDR
DESCRIPTION: 3 BD 2 BT

PROJECT: 261-C-MR9673A
66'-8" x 29'-0"
3 BD 2 BT
TITLE: WATER LINES
1ST FLOOR

DRWN BY: EDR
DATE: 03-24-21
SCALE: 1/8"=1'-0"
FILENAME: C-MR9673A

SHEET:
W-101
2425-0826F
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P.O. BOX 2087 HWY 100 EAST LAKE CITY, FL 32055

ENGINEER'S ARCHITECT'S SEAL



APPROVER'S SEAL

AGENCY APPROVAL	
I HAVE REVIEWED THIS PROJECT AND APPROVE THE PROVISIONS OF THE	
DATE	05/21/2021
BY	STEVEN C. DAME
PROFESSIONAL ENGINEER	05212021
NO. 77466	
STATE OF FLORIDA	
THIRD PARTY:	
REGISTERED: STEVEN C. DAME, CHIEF ENGINEER	
ADDRESS: 1627 SOUTH HIGHLAND AVE.	
ANN ARBOR, MI 48106	
CLANWATER, FLORIDA 33756	

MODIFICATIONS

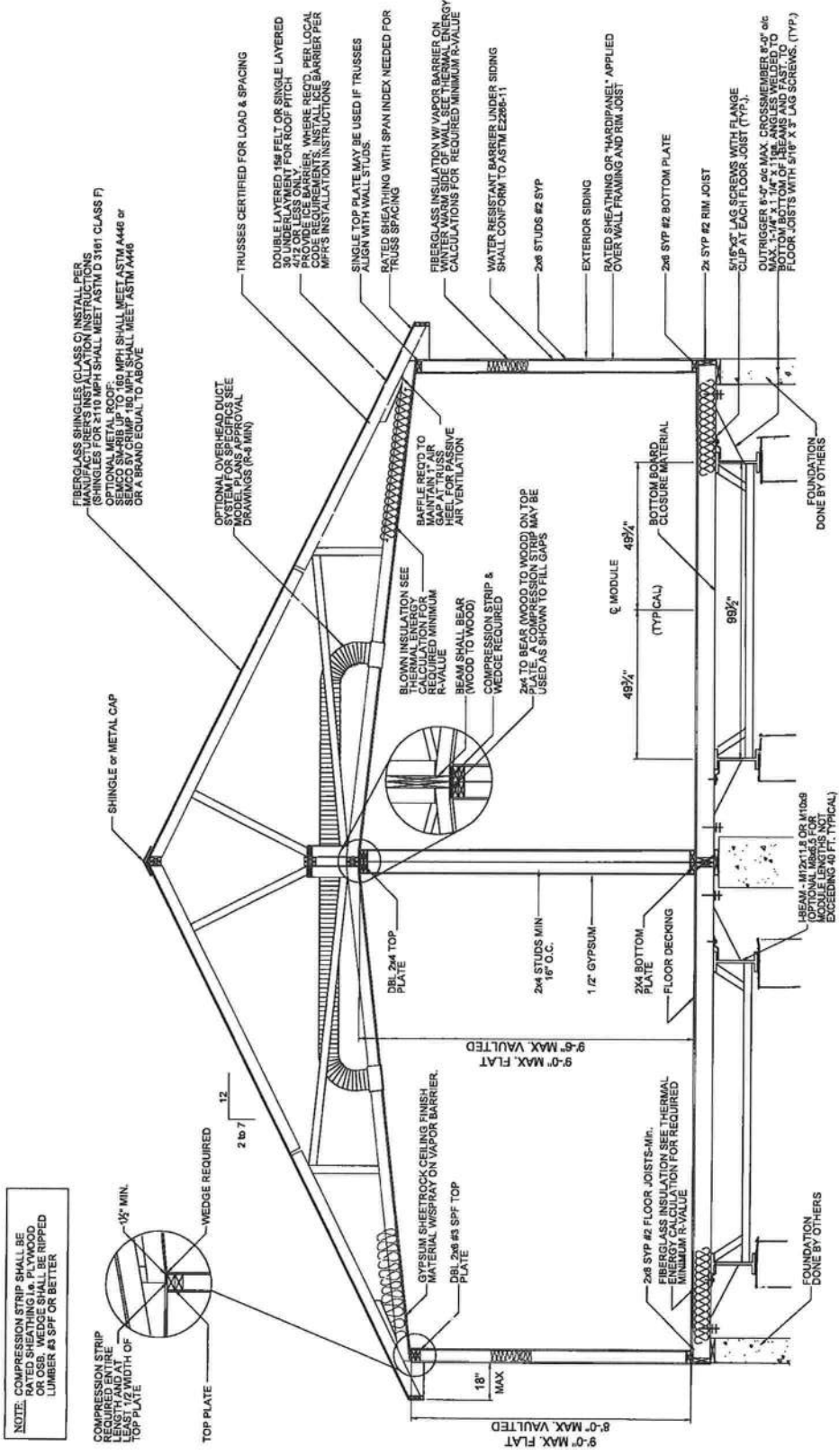
PROJECT: 261-C-MR9673A
66'-8" x 29'-0"
3 BD 2 BT

TITLE: TYPICAL
CROSS-SECTION

DRAWN BY: BOB
DATE: 03-24-21
SCALE:
FILENAME: C-MR9673A

SHEET:
SE-101
2425-0826F

PROJECT AND CONTRACTUAL
THIRD PARTY AND ARCHITECTURAL
PROVISIONS AND SPECIFICATIONS AND GENERAL
CONDITIONS APPLY TO THIS SHEET.



COMPLETE FOUNDATION DESIGN
IS TO BE DONE BY A LICENSED ENGINEER
SEE F-101 FOR REFERENCE

CHAMPION

MANUFACTURED BEAUTIFULLY™

P.O. BOX 2867 HWY 100 EAST LAKE CITY, R. 32056

ENGINEER'S / ARCHITECT'S SEAL

APPROVER'S SEAL

MODIFICATIONS

PROJECT: 261-C-MR9673A
66'-8" x 29'-0"
3 BD 2 BT

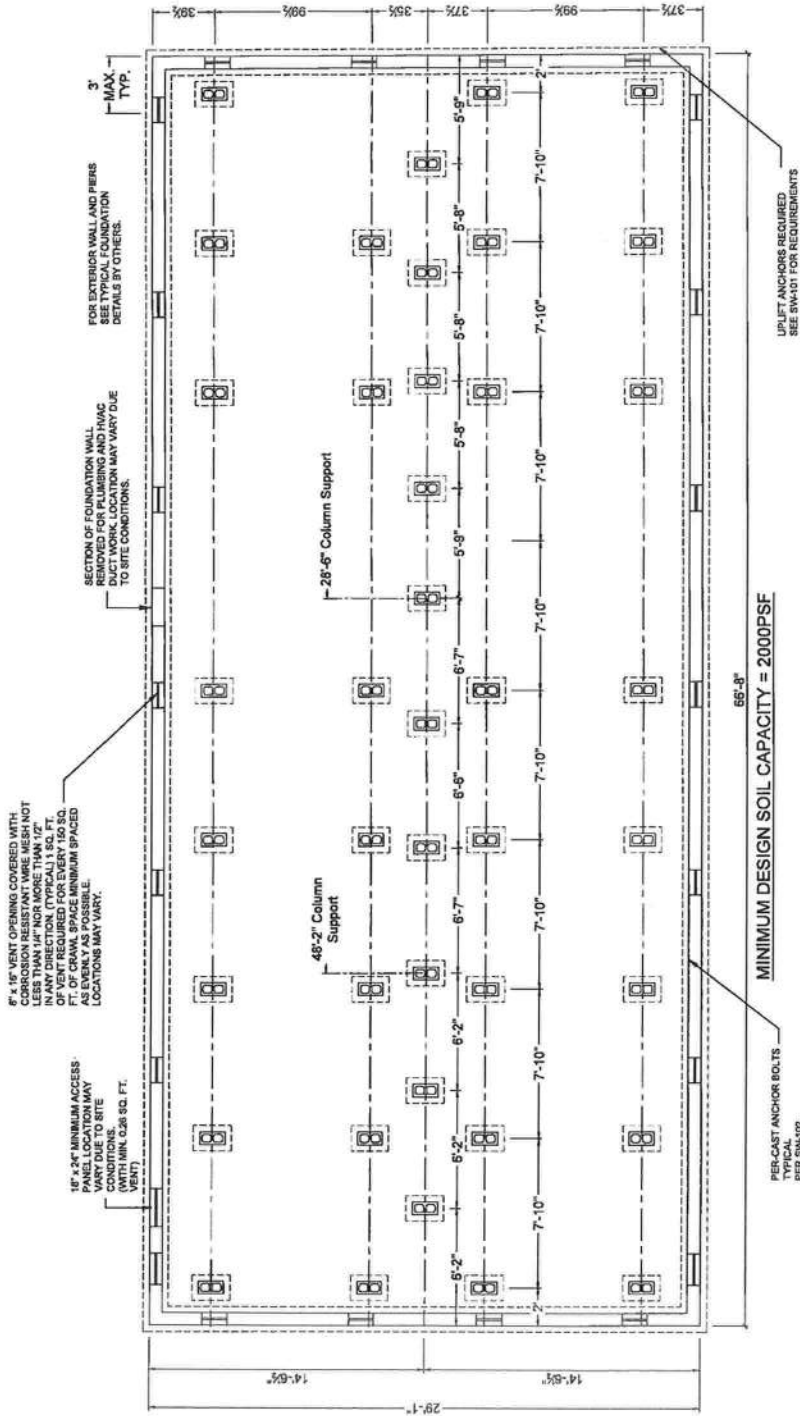
TITLE: PIER
FOUNDATION

DRAWN BY: BDB
DATE: 03-24-21
SCALE:
FILE NAME: C-RR6873A

SHEET:

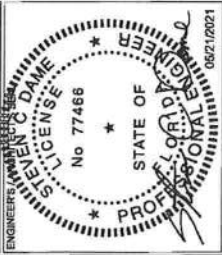
F-101
2425-0826F

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- FOUNDATION NOTES:**
- THIS FOUNDATION PLAN IS PROVIDED FOR REFERENCE AS A TYPICAL STANDARD. ACTUAL FOUNDATION CONDITIONS MUST BE EVALUATED FOR APPLICABILITY OF THIS PLAN IS TO BE USED. ALTERNATE FOUNDATION PLANS MAY BE REQUIRED BY OTHERS IN ACCORDANCE WITH THE REQUIREMENTS OF THE JURISDICTION HAVING AUTHORITY. IF FOUNDATION CONSTRUCTION MATERIALS AND INSTALLATION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE STATE AND LOCAL CODES.
 - EXCAVATE AN ADDITIONAL 1 TO 2 INCHES AT BOTTOM AND SIDES OF ALL FOOTINGS THAT ARE POURED DIRECTLY AGAINST EARTH.
 - ALL PIERS SHALL BE CONSTRUCTED OF 7" X 8" X 16" NOMINAL STANDARD WEIGHT CONCRETE MASONRY UNITS LAD IN RUNNING BOND PATTERN AND CONFORMING TO ASTM C90 HAVING A UNIT COMPRESSIVE STRENGTH OF 1500 PSI (f_m = 1500 PSI). MASONRY UNITS SHALL BE FULLY LAD IN TYPE M OR S MORTAR OR COVERED WITH CONCRETE SHALL BE STANDARD WEIGHT (160 Pcf) WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. MORTAR SHALL COMPLY WITH ASTM C270. GROUT SHALL COMPLY WITH ASTM C930 AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 7500 PSI.
 - ALL REINFORCEMENT BARS SHALL COMPLY WITH ASTM A615 GRADE 60. REINFORCEMENT BARS SHALL BE UNCOATED DEFORMED BARS (NO EPOXY). REINFORCEMENT BARS SHALL BE EQUALLY SPACED AND PLACED WITH 3" CLEARANCE FROM BOTTOM AND SIDES OF THE FOOTING. AT SPACES UP ALL #4 BARS IN INCHES MINIMUM AND LAP ALL #6 BARS 30 INCHES MINIMUM. OFF SET ALL SPLICES 30 INCHES MINIMUM.
 - ALL PIERS SHALL BE CAPPED WITH 4 INCHES OF SOLID MASONRY OR CONCRETE OR THE CAVITIES OF THE TOP COURSE SHALL BE FILLED WITH CONCRETE OR GROUT. PIERS SHALL PROVIDE A TRUE AND EVEN BEARING SURFACE.
 - THE CENTERLINE OF EACH PIER SHALL BE LOCATED DIRECTLY BELOW THE MATE LINE CENTERLINE WITH 1/8" MAXIMUM TOLERANCE.
 - CONCRETE CAPACITY IS ASSUMED TO BE 2000 PSF MINIMUM. IF THE ACTUAL SOIL BEARING CAPACITY IS LESS THAN 2000 PSF, THE ENGINEER MUST BE CONSULTED FOR REQUIRED ALTERNATE FOUNDATION DESIGN. FOOTINGS SHALL BE PLACED ON NON-EXPANSIVE SOILS ONLY. IT IS THE RESPONSIBILITY OF THE SITE OR AHI TO DETERMINE THE ACTUAL SOIL BEARING CAPACITY.
 - THE PERIMETER GRADE SHALL BE SLOPED AWAY FROM THE BUILDING TO PROVIDE POSITIVE DRAINAGE. THE GRADE OF THE GROUND UNDER THE BUILDING SHALL NOT BE LOWER THAN THE LOWEST SURROUNDING FINISHED LOT AREA GRADE IN ORDER TO PREVENT THE ACCUMULATION AND STANDING OF WATER UNDER THE BUILDING.
 - ALL STAIRS, RAMPS, DECKS AND OTHER SITE WORK NOT SHOWN ON THESE DRAWINGS ARE DESIGNED BY OTHERS AND SUBJECT TO THE APPROVAL OF THE JURISDICTION HAVING AUTHORITY.
 - TERMITE PROTECTION SHALL BE PROVIDED IN ACCORDANCE WITH THE APPLICABLE CODES WHEN REQUIRED BY SUCH CODES.
 - THE FOUNDATION DIMENSIONS SHOWN INCLUDE AN INCREASE IN MODULE WIDTH DUE TO MODULE EXPANSION, SETTING TOLERANCES, ETC. THE FOUNDATION CONTRACTOR SHOULD CONSULT WITH THE MANUFACTURER OF THE MODULES PRIOR CONSTRUCTION OF THE FOUNDATION TO DETERMINE THE APPROPRIATE AMOUNT OF GROUND SURFACE IS NOT DESIRED FOR PROTECTIVE COVERING WITH AN APPROVED VAPOR BARRIER.
 - SEE MODEL PLAN FOR SHEAR WALL, DECK AND FOUNDATION UPLIFT DETAILS AND SEE TYPICAL PACKAGE FOR GENERAL TIE DOWN REQUIREMENTS AND ADDITIONAL CONSTRUCTION REQUIREMENTS.
 - THIS FOUNDATION PLAN IS ONLY APPLICABLE TO THE MODEL NUMBER SPECIFIED IN THE TITLE BLOCK. FOUNDATION PLAN IS NOT VALID FOR ANY MODEL CHANGES MADE AFTER THE CERTIFICATION DATE OF THIS FOUNDATION DESIGN. SEE MODEL PLAN COVER SHEET FOR STRUCTURAL LOAD LIMITATIONS.
 - THIS FOUNDATION DESIGN MUST INCORPORATE ALL CAST IN PLACE STRAPS AND ANCHOR BOLTS AS SHOWN ON ATTACHED SHEARWALL CALCULATIONS AND DRAWINGS IN ORDER TO COMPLETE THE LOAD PATH ASSOCIATED WITH THE MAIN WIND FORCE RESISTING SYSTEM OF THIS HOME. ANY VARIATION IN RECOMMENDED CONNECTIONS MUST BE DESIGNED BY AN ENGINEERING PROFESSIONAL.

MANUFACTURED BEAUTIFULLY™
 750 W. 300 BROADWAY
 ALBUQUERQUE, NM 87102, USA
 PHONE: 505.263.1000 FAX: 505.263.1001



APPROVER'S SEAL



MODIFICATIONS

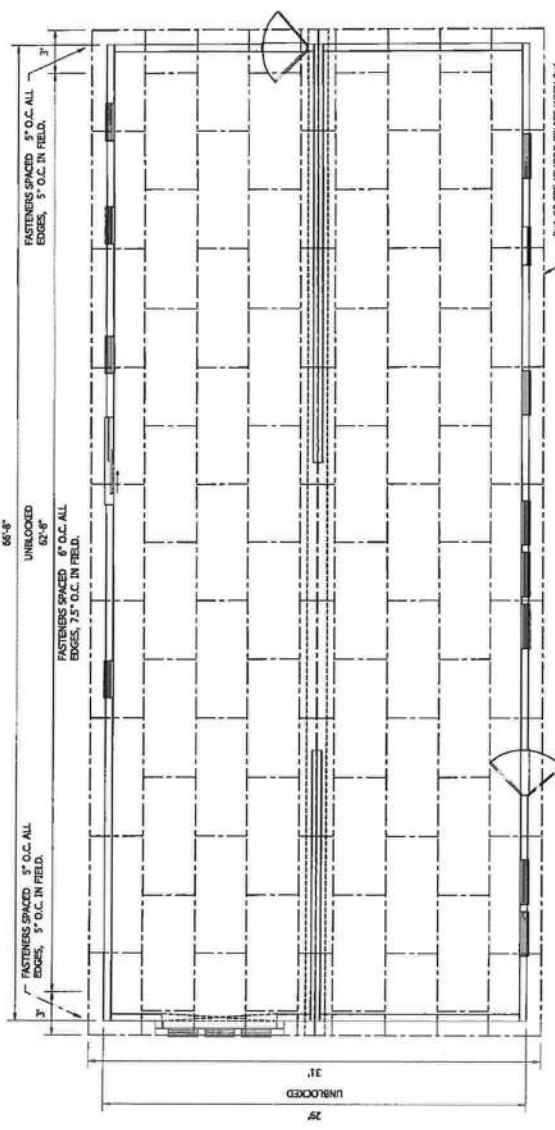
TITLE: **ROOF DIAPHRAGM SHEARWALL LAYOUT**
 MWERS
 MODEL: **261-C-MR9673A**
 Model Description

DATE: 05/13/21 SCALE:
 DRAWN BY: DOB CHECKED BY:
 FILENAME: 2020 - C-MR9673A (150MPH, EXP C)
 SHEET NO.:
SW-101
 PAGE: 1 OF 1
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DESIGN PROFILE	
DESIGN WIND SPEED CATEGORY:	C
WIND SPEED DESIGN CATEGORY:	150 MPH
WIND SPEED DESIGN CATEGORY:	116 MPH
EXPOSURE CATEGORY:	C
PEAK ROOF HEIGHT:	25'-0"
ROOF FIT:	6/12
WALL HEIGHT:	108 IN.

DIAPHRAGM LAYOUT

- NOTES:
- USE 1/4" RATED SHEATHING FOR TRUSSES @ 16" O.C.
 - & 3/8" RATED SHEATHING FOR TRUSSES @ 24" O.C.
 - FASTENERS SHALL BE 0.131 x 2" NAILS (MIN.) SPACED AS INDICATED ABOVE
 - DIAPHRAGM WITH SPF FRAMING RATED FOR:
 - 204 RFL WITH 40% INCREASE PER IBC 2018 / 2020 IRC WIND LOAD ONLY.
 - 204 RFL BLOCKED PER ESR-1359 WITH 40% INCREASE PER IBC 2018 / 2020 IRC WIND LOAD ONLY.
 - CALCULATIONS OUTLINE THE MWERS (PER ASCE7-16) ONLY. REFERENCE STATE APPROVED DESIGN MANUAL FOR ADDITIONAL STRUCTURAL INFORMATION
 - FOUNDATION DESIGN PERFORMED BY OTHERS BASED ON SITE CONDITIONS AND MUST ACCOMMODATE ALL UPLIFT AND LATERAL FORCES AS NOTED.



SHEAR WALL	SHEAR WALL CAPACITY
END WALL 1	378 RFL 16" O.C. EDGED
END WALL 2	293 RFL 16" O.C. EDGED
SIDEWALL 1	127 RFL 16" O.C. EDGED
SIDEWALL 2	127 RFL 16" O.C. EDGED

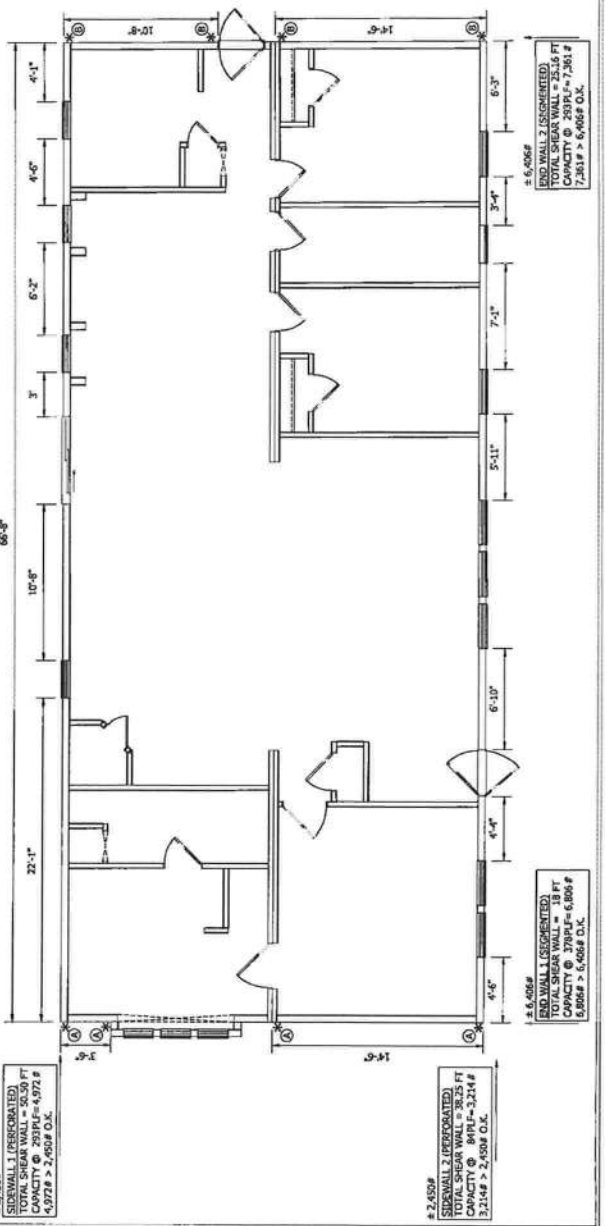
THESE WALLS ARE CONSIDERED TO BE UNBLOCKED UNLESS OTHERWISE NOTED.

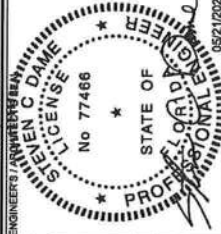
SHEAR WALL LAYOUT

- NOTES:
- SHEAR WALLS SHALL BE CONSTRUCTED OF RATED SHEATHING WITH SPF #2 FRAMING AT 16" O.C. WITH FASTENING AS FOLLOWS:
 - 3/8" RFL: 0.131" x 2" NAILS (MIN.), 3" O.C. AT PANEL EDGES AND 12" O.C. IN FIELD. STANDARD DETAIL PAGE 02.
 - 2x4 RFL: 0.131" x 2" NAILS (MIN.), 3" O.C. AT PANEL EDGES AND 12" O.C. IN FIELD. STANDARD DETAIL PAGE 02.
 - 127 RFL: 3/4" x 1/2" GALV. STAPLES, 6" O.C. AT PANEL EDGES AND 18" O.C. IN FIELD. STANDARD DETAIL PAGE 02.

UPLIFT MARK	UPLIFT LOAD
①	2,558 #
②	1,386 #

UPLIFT CONNECTION REQUIREMENTS AS INDICATED BY "U" ARE TO BE USED FOR UPLIFT VALUES UPLIFT CONNECTION DETAILS ARE PAGES 14 & 15.





APPROVER'S SEAL



MODIFICATIONS

TITLE: **SHEAR WALL END WALL DETAILS**

MODEL: **261-C-MR9673A**
 Model Description

DATE: 05-24-21 SCALE: DRAWN BY: RJB CHECKED BY:

FILENAME: 2002-C-MR9673A (100MPH_EXP.C)
 SHEET NO.:

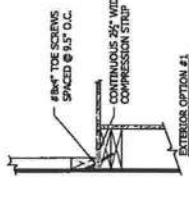
PAGE: **SW-102**
 1 OF 1

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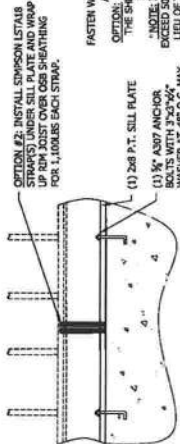
GENERAL NOTES

1. SHEATHING SHALL BE 3/4" OSB SHEATHING. THE FOLLOWING LOADS ARE WIND ONLY RATED.
 END WALL: FASTENING (28 PL):
 0.131 x 2" NAILS, 3" O.C. AT PANEL EDGES AND 12" O.C. FIELD. FRAMING AT ADJOINING
 PANEL EDGES SHALL BE MINIMUM 3" NOMINAL. THIS IS ACHIEVED BY ATTACHING (2) 2x5 WITH
 90% GLUED SURFACES AND (2) ROWS OF 0.131x2" NAILS AT 6" O.C.
 END WALL: FASTENING (28 PL):
 0.131 x 2" NAILS, 4" O.C. AT PANEL EDGES AND 12" O.C. FIELD. FRAMING AT ADJOINING
 PANEL EDGES SHALL BE MINIMUM 2" NOMINAL.
 2. ANY MODIFICATION TO THE DETAILS SPECIFIED MUST HAVE PRIOR ENGINEERING APPROVAL.
 3. STRENGTH DATA FOR SHEAR WALL CAPACITY TAKEN FROM ICC EVALUATION SERVICE, INC. ESR-1539
 REISSUED JULY 1, 2016 WITH A 40% INCREASE PER IBC 2018 / 2020 IBC.

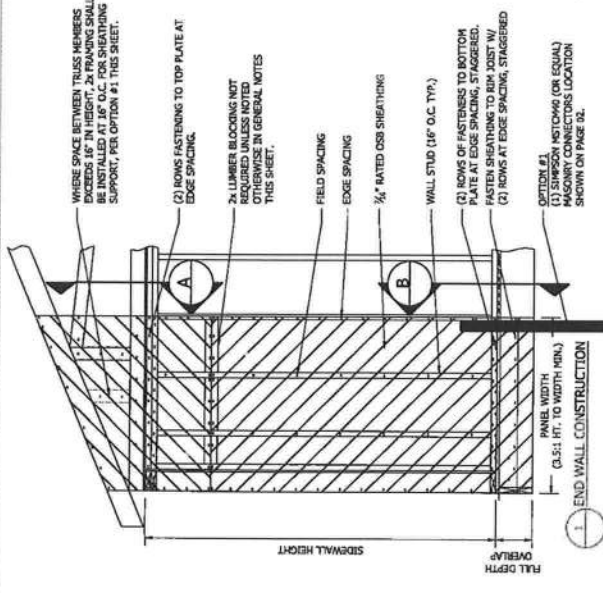
END WALL SHEAR WALLS



A TRUSSES TO TOP PLATE
 EXTERIOR OPTION #2: SHEATHING IS INSTALLED



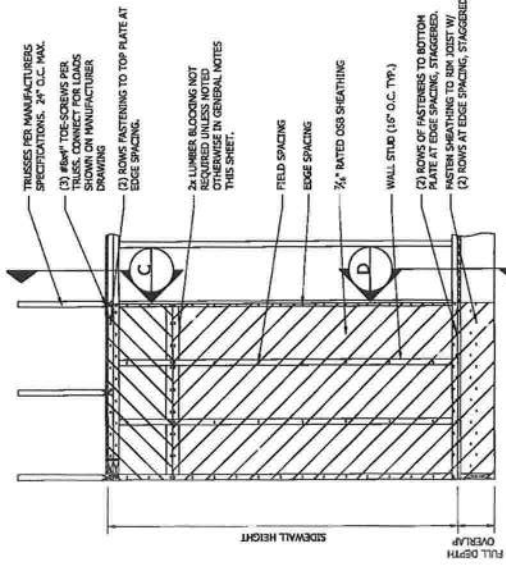
B OPTION #2: FOUNDATION CONNECTION
 SEE MULTI-LOAD CONNECTION LOCATION ON PAGE 02.



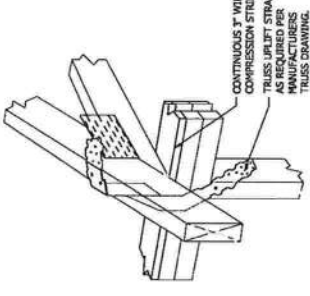
1 END WALL CONSTRUCTION
 (CL-S): HT, TO WIDTH MIN.)

SIDEWALL SHEAR WALLS

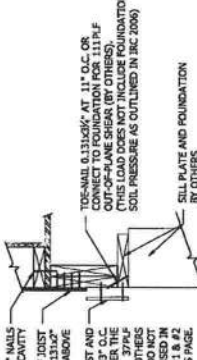
1. SHEATHING SHALL BE 3/4" OSB RATED SHEATHING. THE FOLLOWING LOADS ARE WIND ONLY RATED.
 SIDE WALL: FASTENING (27 PL):
 0.131 x 2" NAILS, 3" O.C. AT PANEL EDGES AND 12" O.C. FIELD. FRAMING AT ADJOINING
 PANEL EDGES SHALL BE MINIMUM 3" NOMINAL.
 2. ANY MODIFICATION TO THE DETAILS SPECIFIED MUST HAVE PRIOR ENGINEERING APPROVAL.
 3. STRENGTH DATA FOR SHEAR WALL CAPACITY TAKEN FROM ICC EVALUATION SERVICE, INC. ESR-1539
 REISSUED JULY 1, 2016 WITH A 40% INCREASE PER IBC 2018 / 2020 IBC.



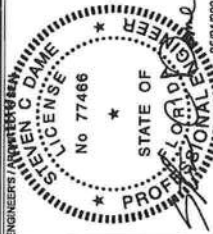
2 SIDEWALL CONSTRUCTION



C TRUSSES TO TOP PLATE
 TRUSSES TO TOP PLATE CONNECTION PER APPROVED DESIGN MANUAL



D BOTTOM PLATE TO RIM JOIST
 BY OTHERS



APPROVERS SEAL
05/21/2021



MODIFICATIONS

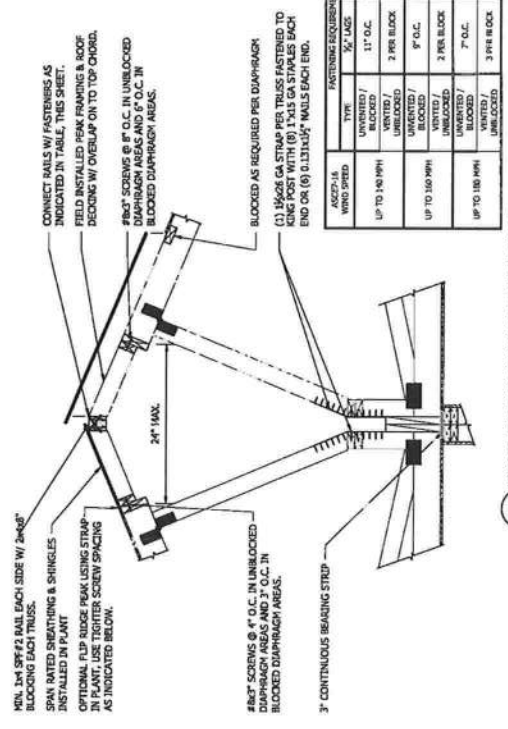
TITLE: **CONNECTION DETAILS**
MWFERS

MODEL: **261-C-MR9673A**
Model Description

DATE: 05-12-21 SCALE:
DRAWN BY: EOB CHECKED BY:

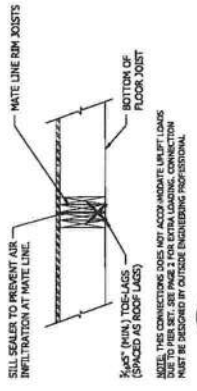
FILENAME: 2020 - CAMR9673A (150MPH, EXP. 0)
SHEET NO.: **SW-103**
PAGE: 1 OF 1

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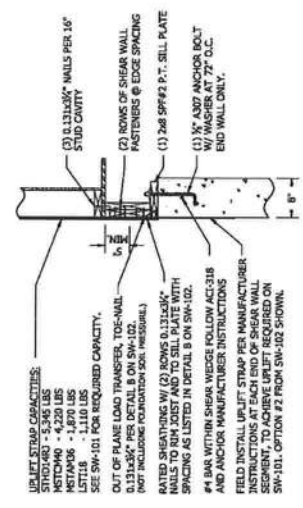


3 HINGED ROOF CONNECTION W/ RIDGE VENT
RIDGE VENT MUST NOT OCCUR IN BLOCKED DIAPHRAGM AREAS

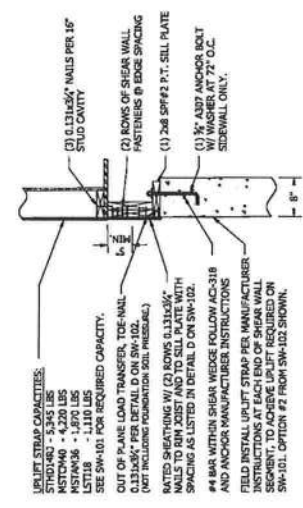
THIS MWFERS PACKAGE ASSUMES A PERIMETER FOUNDATION SET, FOR ALL OTHER FOUNDATION TYPES, DESIGN BY OTHERS.



4 MULTI-SECTION FLOOR CONNECTION



5 END WALL UPLIFT CONNECTION



6 SIDEWALL UPLIFT CONNECTION

Unblocked, 7/16" Wood Roof Diaphragm

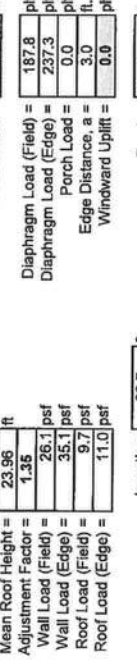
Model: C-MR9673A

Spacing at 6 in. o.c. at diaphragm perimeter, 6 in. o.c. at other panel edges, 12 in. o.c. field

Loading Type	Wind	Code Reference	ESR-1639
Diaphragm Type	Unblocked	Diaphragm Perimeter Spacing	6 in
Sheathing Layout	Case 1	Panel Edge Spacing	6 in
Fastener Type	0.131 in. dia nails	Max L/W Ratio	3:1
Minimum 2" fastener length			

Components & Cladding:	Corner Field	Fasten at: 5" o.c.	in the field
	Field	Fasten at: 6" o.c.	in the field

Roof Diaphragm Data:	Diaphragm Capacity = 320 pif	Reduction Factor = 0.92 for SPF	Max. Allowable Shear = 294 pif
Wind Speed (mph) = 150	ASCE7-16	ASCE7-05	Wind Speed = 119
Exposure = C	Roof Pitch = 6.08/12	Overhang, OH = 12 in	Wall Height, H = 108 in
Mean Roof Height = 23.96 ft	Floor Width, W = 14.58 ft	Number of Boxes = 2	Porch Pitch = No Porch / 12
Adjustment Factor = 1.35	Length = 66.67 ft	Porch Length = 0 ft.	
Wall Load (Field) = 26.1 psf	Diaphragm Load (Field) = 187.8 pif	Diaphragm Load (Edge) = 237.3 pif	Porch Load = 0.0 pif
Roof Load (Field) = 9.7 psf	Edge Distance, a = 3.0 ft	Windward Uplift = 0.9 pif	
Roof Load (Edge) = 11.0 psf			



Diaphragm Shear Load:	Main Roof Shear = 6,406 lbs	Porch Shear = 0 lbs	Allowable Shear = 9,126 lbs
Diaphragm Moment Load:	Diaphragm Chord: #2	2x4 (SPF)	Actual Moment = 104,801 lb-ft
			Allowable Moment = 175,770 lb-ft

Change from Blocked to Unblocked Diaphragm Not Required

Diaphragm Capacity = 320 pif	Diaphragm Capacity = 294 pif
Max. Shear for Unblocked Diaphragm = 294 pif (for SPF)	Maximum Roof Shear Wind = 6,406 lbs
	Maximum Roof Shear Seismic = N/A lbs
	Length from Blocked to Unblocked = -11.41 ft. (From Roof Edge)

Blocking Not Required

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Unblocked, 7/16" Wood Roof Diaphragm

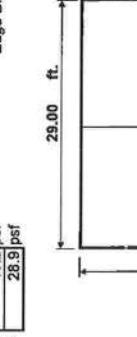
Model: C-MR9673A

Spacing at 6 in. o.c. at diaphragm perimeter, 6 in. o.c. at other panel edges, 12 in. o.c. field

Loading Type	Wind	Code Reference	ESR-1639
Diaphragm Type	Unblocked	Diaphragm Perimeter Spacing	6 in
Sheathing Layout	Case 3	Panel Edge Spacing	6 in
Fastener Type	0.131 in. dia nails	Max L/W Ratio	3:1
Minimum 2" fastener length			

Roof Diaphragm Data:	Diaphragm Capacity = 235 pif	Reduction Factor = 0.92 for SPF	Max. Shear = 216 pif
Wind Speed (mph) = 150	ASCE7-16	ASCE7-05	Wind Speed = 119
Exposure = C	Roof Pitch = 6.08/12	Overhang, OH = 12 in	Wall Height, H = 108 in
Mean Roof Height = 23.96 ft	Floor Width, W = 14.58 ft	Number of Boxes = 2	Porch Pitch = No Porch / 12
Adjustment Factor = 1.35	Length = 66.67 ft	Porch Length = 0 ft.	
Wall Load (Field) = 19.2 psf	Diaphragm Load (Field) = 166.3 pif	Diaphragm Load (Edge) = 173.4 pif	Porch Load = 0.0 pif
Wall Load (Edge) = 28.9 psf	Edge Distance, a = 3.0 ft	Windward Uplift = 0.9 pif	

House Layout:	Roof Pitch = 6/12	Overhang, OH = 12 in	Wall Height, H = 108 in
	Floor Width, W = 14.58 ft	Number of floors = 2	Length = 66.67 ft
2020 Florida Building Code	Wind Speed = 119	ASCE7-05	Wind Speed = 116
ASCE7-16	Exposure = C	Roof Pitch = 6.08/12	Overhang, OH = 12 in
ASCE7-05	Mean Roof Height = 23.96 ft	Floor Width, W = 14.58 ft	Number of Boxes = 2
ASCE7-16	Adjustment Factor = 1.35	Length = 66.67 ft	Porch Pitch = No Porch / 12
ASCE7-05	Wall Load (Field) = 19.2 psf	Diaphragm Load (Field) = 166.3 pif	Diaphragm Load (Edge) = 173.4 pif
ASCE7-16	Wall Load (Edge) = 28.9 psf	Edge Distance, a = 3.0 ft	Windward Uplift = 0.9 pif



Span Calculation:	Allowable Shear Load = 14,414 lbs	Maximum Span = 183.2 ft.
Chord Force = 5,670 lbs	Moment due to Chord = 378,019 ft-lbs	Maximum Span = 134.8 ft.

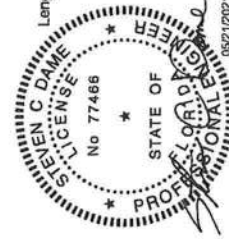
Diaphragm Chord: #2 2x4 (SPF)

Change from Blocked to Unblocked Diaphragm Not Required

Diaphragm Capacity = 235 pif	Diaphragm Capacity = 216 pif
Max. Shear for Unblocked Diaphragm = 216 pif (for SPF)	Maximum Reaction Wind = 2,450 lbs
	Maximum Reaction Seismic = N/A lbs
	Length from Blocked to Unblocked = -68.98 ft. (From Roof Edge)

Blocking Not Required

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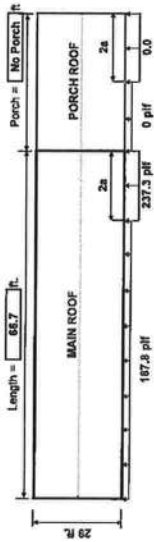
7/16" Rated Shearwall at Endwall

Model: C-MR9673A

Connected by: **3.353** dia. Nails spaced @ **4** in. o.c. (edge), 12 in. o.c. (field)

Loading Type = **Wind**

Shearwall Data:	Shearwall Capacity = 530 pf	Stud Spacing = 16 in. o.c.	Components & Cladding
	Reduction Factor = 0.92 for SPF	Umbilicated	Edge
	Max. Shear = 293 pf	0.8	Field
		0.8	Corner
		0.8	Field
		0.8	Field



Total Shear at Endwall = **6,408** lbs
 Shear Wall Required = **21,90** ft.
 Available Wall = **35.72** ft.
 Actual Shear Value = **25.5** pf.
 Shear Capacity = **7,351** lbs

Overturning Moment: Anchor Capacity = **4,220** lbs
 Anchor Uplift Force = **1,395** lbs
 (1) - Simpson (or EQ.)
 MSTCMA10 @ 4,220#
 tension ea.

Shearwall to Roof Truss Connection:

(Out of Plane) Fasteners = #8x4" Toe-Screws (NDS)
 Fastener Capacity = **103.6** lbs/screw
 Shear Load = **125.9** pf
 Fasteners req'd @ **9.5** in. o.c.

In Plane and Out of Plane Shear Option:

Without OSB overlap (OSB seen at BC of truss)
 Shear Load = **346.8** pf
 Fasteners req'd @ **3.5** in. o.c.

Bottom Plates to Rim-Joint Connection:

(Out of Plane) Fasteners = 0.131 x 3.25" Nails (NDS)
 Fastener Capacity = **100.2** lbs/nail
 Shear Load = **130.6** pf
 Nails req'd @ **9.0** in. o.c.
 Number per cavity = **3.0** nails



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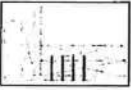
7/16" Rated Shearwall at Endwall

Model: C-MR9673A

Shear wall to Rim-Joint connection: (ESR-1539)

(In Plane)

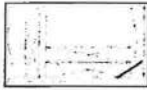
Fastener = **0.131** Nail
 Fastener Capacity = **131.2** lbs/nail
 Shear Load = **254.6** pf
 Fasteners req'd @ **6.0** in. o.c.
 use (2) rows: **6 @ 0** in. o.c. staggered



Bottom Wall Shear Connection:

(Out of Plane) Fastener = 0.131 x 3.25" Toe Nail (ESR-1539)

Fastener Capacity = **108.9** lbs/nail
 Shear Load = **183.9** pf
 Nails req'd @ **8 @ 0** in. o.c.



Shear Transfer from Rim-Joint to Sill Plates:

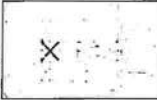
(In Plane) (ESR-1539)

Fastener = **0.131** Nails
 Fastener Capacity = **131.2** lbs/nail
 Lateral Shear Load = **220.9** pf
 Uplift Shear Load = **0.0** pf
 Nails req'd @ **7.0** in. o.c.
 Option: LTPA Clips = **30** in. o.c.



Roof-Joint Connection: (NDS)

Angle = **45** deg.
 Angle = **0.7654** rad.
 Shear Capacity = **5165** lbs Bolt
 Screws = **410** Screws
 Withdrawal Capacity = **131** lbs Bolt
 Combined Capacity = **204** lbs Bolt
 Combined Capacity = **131.5** lbs Bolt



Roof Uplift Connection:

Uplift = **18.9** pf
 Roof Tie = **194.54** in.
 Moment = **1094** lbs-ft
 Tension Force = **17.02** ft
 Spacing = **31** in. o.c.
 #10 Screws = **43** in. o.c.

Roof Shear Load Connection: From Diaphragm

Connect 26 Ga. Metal Strip with 15 Ga. Staples
 Total Load across Joint = **221** pf
 Capacity Parallel to Grain = **92** lbs/staple
 Spacing = **5** in. o.c.



Screw Option: (no metal strip required) (NDS)

Connect With = #10 x 4" Screws
 Total Load = **174** lbs/screw
 Capacity Parallel to Grain = **145** lbs/screw
 Spacing = **7** in. o.c.

Anchor Bolt Shear: (NDS)

Number of Sill Plates = **1**
 Fastener = **5/8** in. dia. Anchor Bolt
 Shear Parallel to Grain = **220.9** pf
 Shear Perpendicular to Grain = **153.9** pf
 Capacity Parallel to Grain = **1532** lbs/bolt
 Capacity Perpendicular to Grain = **72** lbs/bolt
 Spacing = **72** in. o.c.



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5/12/2021

HOME BUILDERS

7/16 in. Thick Rated Shear Wall

Model: C-MR9673A

HOME BUILDERS

Connected by: 15 ga. Staples @ 12 in. o.c. (edge), 12 in. o.c. (field)

Loading Type = Wind Loading Factor = 1.4

Shearwall Data:

Shearwall Capacity = 269 pif
 Reduction Factor = 0.82 for SPF
 Maximum Shear = 127 pif (for SPF)

Wall Height, H = 108 in

Wall Segment Lengths

ft	in	ft	hbs	% Capacity
1	22.08	0.41	100%	127
2	10.67	0.84	100%	127
3	3.00	3.00	100%	127
4	6.17	1.46	100%	127
5	4.50	2.00	100%	127
6	4.08	2.20	100%	127
7	0.00	0.00	0%	127
8	0.00	0.00	0%	127
	50.50			

Total Shear @ Wall = 2,450 lbs
 Actual Shear Wall Segment Lengths = 50.50 ft
 Perforated Shear Wall Length = 96.67 ft
 Actual Shear Value = 62.7 pif

Percent Full Height Sheathing

Lower Bound = 0.76
 Upper Bound = 0.80

	1/2 h	1/2 h	2/3 h	5/6 h	1 h
10%	1.00	0.69	0.53	0.43	0.36
20%	1.00	0.71	0.56	0.45	0.38
30%	1.00	0.74	0.59	0.49	0.42
40%	1.00	0.77	0.63	0.53	0.45
50%	1.00	0.80	0.67	0.57	0.50
60%	1.00	0.83	0.71	0.63	0.56
70%	1.00	0.87	0.77	0.69	0.63
80%	1.00	0.91	0.83	0.77	0.71
90%	1.00	0.95	0.91	0.87	0.83
100%	1.00	1.00	1.00	1.00	1.00

Co = 0.77
 Perforated Capacity = 96 pif
 Allowable Shear Force = 4972 lbs
 Anchor Uplift Force = 0 lbs



PROFESSIONAL ENGINEER
 STATE OF FLORIDA
 No 77466
 STEVEN C. DAME

05/21/2021
 Perforated (Sidewall)
 C-MR9673A SW_56x

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5/12/2021

HOME BUILDERS

7/16 in. Thick Rated Shear Wall

Model: C-MR9673A

HOME BUILDERS

Roof Truss to Top Plate Connection:

Fasteners = 0.131 x 3.25" Top Nails (NDS)
 Fastener Capacity = 108.9 lbs/nail
 Parallel Shear Load = 36.7 pif
 Perpendicular Shear Load = 50.2 pif
 No. of nails req'd = 2 per truss

Bottom Plate to Rim Joist Connection:

Fasteners = 0.131 x 3.25" Nails (NDS)
 Fastener Capacity = 100.2 lbs/nail
 Shear Load = 105.4 pif
 Nails req'd @ 11.0 in. oc.
 Nails req'd @ 3 per stud cavity

**Bottom Half Wall Shear Connection:
 (Out of Plane)**

Fastener = 0.131 x 3.25" Top Nail (ESR-1539)
 Fastener Capacity = 108.9 lbs/nail
 Shear Load = 110.2 pif
 Nails req'd @ 11.0 in. oc.

Shear Transfer from Rim Joist to Sill Plate:

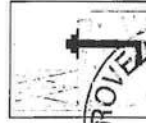
Fastener = 15 Staple
 Fastener Capacity = 72 lbs/each
 Lateral Shear Load = 36.7 pif
 Uplift Shear Load = 0.0 pif
 Nails req'd @ 23.0 in. oc.
 Option: LTP4 Clips = 186.0 in. o.c.

Anchor Bolt Shear and Uplift:

of Sill Fasteners = 1
 Fastener = 5/8 in. dia. Anchor Bolt

Shear Parallel to Grain = 36.7 pif
 Capacity Parallel to Grain = 110.2 pif
 Shear Perpendicular to Grain = 1552 lb/bolt
 Capacity Perpendicular to Grain = 928 lb/bolt
 Required Shear Spacing = 72 in. oc.

Uplift = 0.0 pif
 Washer Area = 2.03 in²
 Cb = 1.21
 Fcp = 425 psi
 Anchor Bolt Uplift Capacity = 1050 lb/bolt
 Required Uplift Spacing = N/A in. oc.



PROFESSIONAL ENGINEER
 STATE OF FLORIDA
 No 77466
 HILBORN, WERNER, C. & P.

05/21/2021
 Perforated (Sidewall)
 C-MR9673A SW_56x

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HOME BUILDERS

C-MR9673A
Sidewall 2

7/16 in. Thick Rated Shear Wall

Model: **6**

Connected by: **15** ga. Staples @ **6** in. o.c. (edge), 12 in. o.c. (field)

Loading Type = **Wind** Loading Factor = **1.4**

Shearwall Data:

Shearwall Capacity = 259 pif
Reduction Factor = 0.82 for SPF
Maximum Shear = 127 pif (for SPF)

Wall Height, H = **108** in

Wall Segment Lengths

ft	in	ft	lbs	% Capacity	Strength (pif)
1	4	4.50	2.00	100%	127
2	4	4.33	2.08	100%	127
3	6	6.83	1.32	100%	127
4	5	5.82	1.52	100%	127
5	7	7.08	1.27	100%	127
6	3	3.33	2.70	100%	127
7	6	6.25	1.44	100%	127
8		0.00	0.00	0%	127
			38.25		

Total Shear @ Wall = 2,450 lbs
Actual Shear Wall Segment Length = 38.25 ft
Perforated Shear Wall Length = 66.67 ft
Actual Shear Value = 96.8 pif.

Percent Full Height Sheathing

Lower Bound = 0.57
Upper Bound = 0.80

	1/3 h	1/2 h	2/3 h	5/6 h	1 h
10%	1.00	0.69	0.53	0.43	0.36
20%	1.00	0.71	0.56	0.45	0.38
30%	1.00	0.74	0.59	0.48	0.42
40%	1.00	0.77	0.63	0.53	0.46
50%	1.00	0.80	0.67	0.57	0.50
60%	1.00	0.83	0.71	0.63	0.56
70%	1.00	0.87	0.77	0.69	0.63
80%	1.00	0.91	0.83	0.77	0.71
90%	1.00	0.95	0.89	0.87	0.83
100%	1.00	1.00	1.00	1.00	1.00

Co = 0.86
Perforated Capacity = 84 pif
Allowable Shear Force = 3214 lbs
Anchor Uplift Force = 0 lbs



PROFESSIONAL ENGINEER
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No. 77466
STATE OF OREGON
LICENSE
05/21/2021

PERFORATED (Sidewall) (2)
C-MR9673A SW.dwg
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7/16 in. Thick Rated Shear Wall

Model: **2**

C-MR9673A
Sidewall 2

Roof Truss to Top Plate Connection:

Fasteners = 0.131 x 3.25" Toe Nails (NDS)
Fastener Capacity = 108.9 lb/nail
Parallel Shear Load = 36.7 pif
Perpendicular Shear Load = 90.2 pif
No. of nails req'd = **2** per truss

Bottom Plate to Rim Joist Connection:

Fasteners = 0.131 x 3.25" Nails (NDS)
Fastener Capacity = 100.2 lb/nail
Shear Load = 105.4 pif
Nails req'd @ **11.0** in. o.c.
Nails req'd @ **3** per stud cavity

Bottom Half Wall Shear Connection:
(Out of Plane)

Fastener = 0.131 x 3.25" Toe Nail (ESR-1539)
Fastener Capacity = 108.9 lb/nail
Shear Load = 110.2 pif
Nails req'd @ **11.0** in. o.c.

Shear Transfer from Rim Joist to Sill Plates:

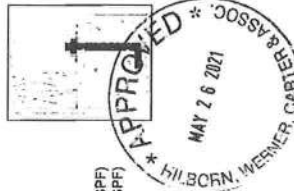
ESR-1539
Minimum 1.75" fastener length
Fastener = 15 Staple
Fastener Capacity = 72 lb/each
Lateral Shear Load = 36.7 pif
Uplift Shear Load = 0.0 pif
Nails req'd @ **23.0** in. o.c.
Option: LTP4 Clips = **186.0** in. o.c. (NDS)

Anchor Bolt Shear and Uplift:

of Sill Plates = **1**
Fastener = **5/8** in. dia. Anchor Bolt

Shear Parallel to Grain = 36.7 pif
Shear Perpendicular to Grain = 110.2 pif
Capacity Parallel to Grain = 1552 lb/bolt
Capacity Perpendicular to Grain = 928 lb/bolt
Required Shear Spacing = **72** in. o.c. (NDS Table 11E SPF)

Uplift = 0.0 pif
Washer Area = 2.03 in²
Cb = 1.21
Fcp = 425 psi
Anchor Bolt Uplift Capacity = 1050 lb/bolt
Required Uplift Spacing = **N/A** in. o.c.



PROFESSIONAL ENGINEER
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CALIFORNIA REGISTERED PROFESSIONAL ENGINEERS ASSOCIATION
MAY 26 2021

PERFORATED (Sidewall) (2)
C-MR9673A SW.dwg
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Job 105569	Truss HM584548	Truss Type HINGE MONO	Qty 1	Ply 1	Champion Homes 407 GA
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 8.420 e Sep 14 2020 MiTek Industries, Inc. Thu Apr 1 11:51:45 2021 Page 1 of 1
 Ref. #10011994

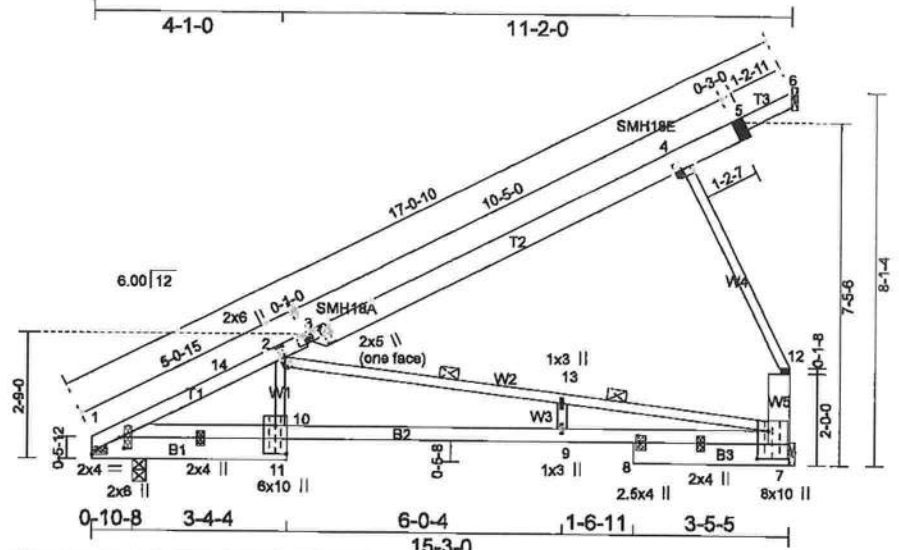


Plate Offsets (X,Y) - [1:0-2-13,0-1-0], [1:Edge,0-4-7], [2:0-3-0,0-0-8], [3:0-2-0,0-4-0], [3:0-0-4,0-0-8], [4:0-1-4,0-1-0], [7:0-7-4,0-3-0], [11:Edge,0-3-0]

SPACING- 2-0-0	SPACING- 1-4-0	SPACING- 2-0-0	CSI.	DEFL.	PLATES	GRIP
LOADING (psf)	LOADING (psf)	Plate Grip DOL 1.25	TC 0.61	in (loc) l/def L/d	MT20 244/190	
TCLL 20.0	TCLL 30.0	Lumber DOL 1.25	BC 0.93	Ver(LL) -0.30 9 >591 240	MT18HS 244/190	
TCDL 7.0	TCDL 10.5	Rep Stress Incr YES	WB 0.93	Ver(CT) -0.50 9 >356 180		
BCLL 0.0 *	BCLL 0.0 *	Code FBC2020/TPI2014	Matrix-R	Horz(CT) 0.07 7 n/a n/a		
BCDL 7.0	BCDL 10.5					Weight: 96 lb FT = 0%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 *Except* T2: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 5-2-13 oc purlins, except and verticals.
BOT CHORD 2x6 SP No.2 *Except* B2: 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x3 SP No.2 *Except* W5: 2x6 SP No.2	WEBS 2 Rows at 1/3 pts 2-7
REACTIONS. (lb/size) 1=530/0-3-8 (min. 0-1-8), 7=490/Mechanical, 6=0/Mechanical Max Horz 1=473(LC 7), 6=B1(LC 7) Max Uplift 1=352(LC 7), 7=588(LC 7) Max Grav 1=530(LC 1), 7=480(LC 1)	
FORCES. (b) - Maximum Compression/Maximum Tension TOP CHORD 1-14=-1185/757, 2-14=-1094/759, 2-3=-278/0, 3-4=-281/50, 4-5=-99/86, 5-6=-55/74, 7-12=-267/447 BOT CHORD 1-11=-1114/974, 10-11=-843/644, 9-10=-1112/968, 8-9=-1112/968, 7-8=-1111/961 WEBS 2-11=0/440, 2-13=-852/914, 7-13=-869/928, 4-12=-295/494, 9-13=-72/101	
REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (b)/ Tension (b)/ Shear (b)/ Moment (b-in) 5=70/71/49/0, 12=295/484/209/0	



- NOTES-**
- This truss has been checked for uniform roof live load only, except as noted.
 - Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=116mph @24in o.c.; TC DL=2.6psf; BCDL=2.8psf; (ALL 180mph @16in o.c.; TC DL=4.2psf; BCDL=4.2psf); h=30ft; Cat. II; Exp C; Encl. GCpl=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 15-2-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - All plates are MT20 plates unless otherwise indicated.
 - See HINGE PLATE DETAILS for plate placement.
 - Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
 - All additional member connections shall be provided by others for forces as indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint 1 and 588 lb uplift at joint 7.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - The presence of holes in the chord(s) plates, any forming or blocking of the longitudinal plate must be reported before the building is put into service.
 - The field-installed members are an integral part of the truss design. Retain a designer, professional or specialty final field connections and temporary supports. All field-installed members must be properly fastened prior to applying any loading to the truss. This does not indicate the final fasteners.
 - Based on UFP 10000
 - Revised/Updated Code

The professional engineering seal indicates that a licensed professional engineer has designed the truss under the standards referenced within this document, not necessarily the current state building code. The engineering seal is not an approval to use in a specific state. The final determination on whether a truss design is acceptable under the locally adopted building code rest with the building official or designated appointee.

WARNING - Verify design parameters and READ NOTES

UFP Industries, Inc. 2801 EAST BELTLINE RD, NE
 PHONE (616)-364-6161 FAX (616)-365-0060 GRAND RAPIDS, MI 49525

This component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCS1 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MiTek\Supp\templates\ufp.tps

Job 105568	Truss HM584547	Truss Type HINGE MONO	Qty 1	Ply 1	Champion Homes 407 GA
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 8.420 e Sep 14 2020 MiTek Industries, Inc. Thu Apr 1 11:32:59 2021 Page 1 of 1
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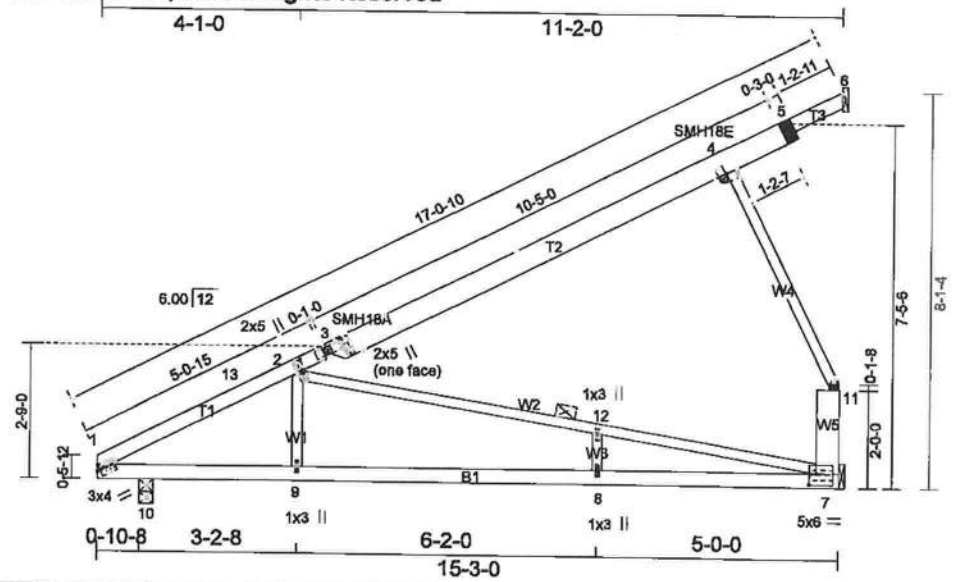


Plate Offsets (X,Y)-- [1:0-0-12,0-1-4], [2:0-2-12,0-0-8], [3:0-2-0,0-4-0], [3:0-0-4,0-0-8], [4:0-1-4,0-1-0], [7:0-1-12,0-2-12]

SPACING--: 2-0-0	SPACING--: 1-4-0	SPACING--: 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
LOADING (psf)	LOADING (psf)	Plate Grip DOL	TC 0.58	Vert(LL)	-0.41	8-9	>408	MT20	244/190
TCLL 20.0	TCLL 30.0	Lumber DOL	BC 0.80	Vert(CT)	-0.67	8-9	>252	MT18HS	244/190
TCDL 7.0	TCDL 10.5	Rep Stress Incr	WB 0.80	Horz(CT)	-0.02	7	n/a		
BCLL 0.0 *	BCLL 0.0 *	Code FBC2020/TPI2014	Matrix-R						
BCDL 7.0	BCDL 10.5							Weight: 81 lb	
								FT = 0%	

LUMBER-
 TOP CHORD 2x4 SP No.2 *Except*
 T2: 2x6 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x3 SP No.2 *Except*
 W5: 2x6 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 3-11-10 oc bracing.
 WEBS 1 Row at midpt 2-7

REACTIONS. (lb/size) 7=456/Mechanical, 6=0/Mechanical, 10=574/0-3-8 (min. 0-1-8)
 Max Horz 6=81(LC 7), 10=479(LC 7)
 Max Uplift 7=546(LC 7), 10=380(LC 7)
 Max Grav 7=456(LC 1), 10=574(LC 1)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-13=754/351, 2-13=660/361, 2-3=278/0, 3-4=281/50, 4-5=99/38, 5-6=55/74, 7-11=267/447
 BOT CHORD 1-10=230/618, 8-10=755/570, 6-9=755/570, 7-8=755/570
 WEBS 2-9=24/261, 2-12=487/558, 7-12=476/555, 4-11=295/494, 6-12=0/43

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Tension (lb)/ Shear (lb)/ Moment (lb-in)
 5=707/149/0, 11=295/494/209/0

- NOTES-**
- This truss has been checked for uniform roof live load only, except as noted.
 - Wind: ASCE 7-16; Vult=150mph (3-second gust) Vead=116mph @24in o.c.; TCCL=2.8psf; BCDL=2.8psf; (All 180mph @16in o.c.; TCCL=4.2psf; BCDL=4.2psf); h=30ft; Cat. II; Exp C; Encl. GCp=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 15-2-4 zone; cantilever left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - All plates are MT20 plates unless otherwise indicated.
 - See HINGE PLATE DETAILS for plate placement.
 - Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
 - All additional member connections shall be provided by others for forces as indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 546 lb uplift at joint 7 and 380 lb uplift at joint 10.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Take precautions to keep the chords in place, by bracing or bolting of the hinge plate must be repaired before the building is put into service.
 - The field-installed members are an integral part of the truss design. Retain a design professional to specify final field connections and temporary supports. All field-installed members must be properly fastened prior to applying any loading to the truss. This design anticipates the final suggestions.
 - Based on HHS 95-10-4
 - Revision: Updated Code



The professional engineering seal indicates that a licensed professional engineer has designed the truss under the standards referenced within this document, not necessarily the current state building code. The engineering seal is not an approval to use in a specific state. The final determination on whether a truss design is acceptable under the locally adopted building code rest with the building official or designated appointee.

WARNING - Verify design parameters and READ NOTES
 UFP Industries, Inc. 2801 EAST BELTLINE RD, NE GRAND RAPIDS, MI 49525
 PHONE (616)-364-6161 FAX (616)-365-0060

Truss shall not be cut or modified without approval of the truss design engineer.
 This component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPIH. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MitekSuppl\templates\upf.tpe



CHAMPION HOMES PROJECT 261-C-MR9673A

PREPARED BY W.J. KALKER, JR., P.E.

BUILDING U_o ANALYSIS

ANALYSIS COMPUTES THE TOTAL BUILDING THERMAL ENVELOPE U_o FOR A PROPOSED BUILDING DESIGN AND THE SAME BUILDING WHICH COMPLIES WITH THE 2020 FLORIDA ENERGY CODE - RESIDENTIAL FOR CLIMATE ZONES 1 AND 2.

NOTE, ALL INPUT VALUES FOR U HAVE THE DIMENSIONS BTUH/(SQ.FT.-F).

TYPE THE GROSS EXTERIOR WALL AREA IN FT**2:
1722
TYPE THE U VALUE FOR THE NET WALL AREA IN BTUH/FT**2-F:
.06
TYPE THE TOTAL STD. EXT. GLAZING AREA IN FT**2:
169
TYPE THE U VALUE FOR THE STD. EXT. GLAZING IN BTUH/FT**2-F:
.3
TYPE THE TOTAL STD. EXT. DOOR AREA IN FT**2:
20
TYPE THE U VALUE FOR THE STD. EXT. DOOR AREA IN BTUH/FT**2-F:
.134
TYPE THE NON-STD. EXT. GLAZING AND/OR DOOR AREA IN FT**2
60
TYPE THE U VALUE FOR THE NON-STD. EXT. GLAZING AND/OR DOOR AREA IN BTUH/FT**2-F:
.33
TYPE THE TOTAL FLOOR AREA IN FT**2:
1933
TYPE THE U VALUE FOR THE FLOOR AREA IN BTUH/FT**2-F:
.072
TYPE THE TOTAL ROOF AREA IN FT**2:
1933
TYPE THE U VALUE FOR THE ROOF AREA IN BTUH/FT**2-F:
.035

PROPOSED BUILDING U_o = .0659 BTUH/SQ.FT.-F
(UA)sum = 368.391

FLORIDA CODE BUILDING U_o = .0725 BTUH/SQ.FT.-F
(UA)sum = 405.034

FLORIDA CODE U VALUES:

U-floor = .064
U-wall = .084
U-roof = .030
U-glass = .400
U-door = .400

PROPOSED BUILDING U_o < FLORIDA CODE BUILDING U_o - OK

ANALYSIS PERFORMED USING BUILDING PROPERTIES PROVIDED BY CLIENT

SEE MANUFACTURER'S CONTRACT WITH FLORIDA

Date _____ Plan No. _____
Approved By SCOTT S. FRANCIS



Florida Building Plans Examiner
License No. SMP-42



05/21/2021