

DESIGN CRITERIA

- STRUCTURE HAS BEEN DESIGNED TO COMPLY WITH 2023 FLORIDA BUILDING CODE AND SUBSEQUENT REFERENCE STANDARDS.
- RISK CATEGORY: II
- SUPERIMPOSED DEAD LOADS:

ROOF	TYPICAL	20 PSF
SUPERIMPOSED LIVE LOADS: LIVE LOAD REDUCTION USED AS ALLOWED PER CODE		
ROOF	TYPICAL	20 PSF (REDUCIBLE)
	MECHANICAL	SEE ROOF PLAN NOTES

- MECHANICAL UNITS: THE CONTRACTOR SHALL CONFIRM THE ACTUAL MECHANICAL UNITS PURCHASED ARE OF WEIGHT AND SIZE SUCH THAT:
 - the UNIT WEIGHT IS LESS THAN OR EQUAL TO WEIGHT LISTED ON PLAN.

TOTAL SUPERIMPOSED LOAD UNDER UNIT =	TOTAL EQUIPMENT WEIGHT / UNIT FOOTPRINT AREA =	INDICATED SERVICE LEVEL DESIGN LIVE LOAD

TOTAL EQUIPMENT WEIGHT =	OPERATING WEIGHT OF UNIT + CONCRETE HOUSEKEEPING PAD + INERTIA BLOCK + OTHER ANCILLARY ITEMS ASSOCIATED WITH THE EQUIPMENT

UNIT FOOTPRINT AREA =	EFFECTIVE AREA OF UNIT OR HOUSEKEEPING PAD, WHICHEVER IS LARGER

EFFECTIVE AREA =	(LENGTH + 6 FEET) X (WIDTH + 6 FEET)

CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION ON THE LOCATION OF ANY MECHANICAL UNITS WHERE THE SUPERIMPOSED LOAD UNDER THE UNIT EXCEEDS THE INDICATED DESIGN SIZE AND LOAD FOR THE ROOF AND/OR LIVE LOAD FOR THE FLOOR AS INDICATED ON THE PLAN.

- WIND:

BASIC WIND SPEED	VULT = 120 MPH VASO = 93 MPH
EXPOSURE CLASS	C
INTERNAL PRESSURE COEFFICIENT, GCpi	±0.18

SEE S0.2 FOR COMPONENTS & CLADDING WIND PRESSURE AND ZONE DIAGRAM.

RAIN:	
RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

SEE S0.2 FOR COMPONENTS & CLADDING WIND PRESSURE AND ZONE DIAGRAM.	

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

RAIN LOADING, R	30 PSF
RAIN INTENSITY, I (15-MIN INTENSITY)	8.29 IN/HR

- FIRE PROTECTION FOR ALL STRUCTURAL PARTS SHALL BE PROVIDED AS SPECIFIED BY THE ARCHITECTURAL DRAWINGS. UL FIRE RESISTANCE RATING RESTRAINT CLASSIFICATION AS FOLLOWS:
 - PRIMARY STRUCTURAL FRAME" AS DEFINED BY IBC SECTION 202 IS "RESTRAINED"
 - EXCEPT AS NOTED IN FOLLOWING NOTE:
 - THE FOLLOWING FRAMING IS "UNRESTRAINED":
 - FRAMING SUPPORTED BY BEARING WALLS
 - CONCRETE WALLS IN END BAYS AND OTHER LOCATIONS WHERE THE END OF THE FRAMING IS NOT ABUTTING FRAMING IN AN ADJACENT BAY
 - HOLLOW CORE SLABS IN END BAYS AND OTHER LOCATIONS WHERE THE END OF THE SLAB IS NOT ATTING SLABS IN AN ADJACENT BAY
 - STEEL JOISTS IN END BAYS ON ROOFS SUPPORTING STEEL DECK
 - ALL WOOD CONSTRUCTION

- STRUCTURAL ENGINEER'S APPROVAL. BELOW GRADE UTILITY OR PIPE ELEVATIONS, WHERE SHOWN, ARE INDICATED FOR REFERENCE ONLY. REQUIRED ELEVATIONS SHALL BE DETERMINED BY OTHERS AND COORDINATED WITH THE FOUNDATIONS.
- WHERE GRADE ELEVATIONS ARE APPROXIMATELY EQUAL ON BOTH SIDES OF WALLS, BACKFILL SHALL BE PLACED SO THAT IT IS NOT UNBALANCED BY MORE THAN 2 FEET ON EITHER SIDE.
- ALL REQUIRED BACKFILL AND UTILITY TRENCH BACKFILL WITHIN THE BUILDING AREA SHALL BE COMPACTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.

- SHALLOW FOUNDATIONS

- SEE THE GEOTECHNICAL REPORT FOR SHALLOW FOUNDATION REQUIREMENTS.
- FOUNDATION ELEVATIONS SHOWN INDICATE LOCATIONS WHERE ADEQUATE SOIL BEARING PRESSURE IS ANTICIPATED. IF INADEQUATE BEARING CAPACITY IS ENCOUNTERED, CONTACT STRUCTURAL ENGINEER FOR RESOLUTION. BEARING ELEVATIONS ARE ESTIMATED FROM SOIL BORING DATA INDICATED IN THE GEOTECHNICAL REPORT. DETERMINATION OF FINAL BEARING ELEVATIONS AND FIELD VERIFICATION OF ALLOWABLE BEARING PRESSURE SHALL BE MADE BY AN EXPERIENCED, QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO PLACING FOUNDATIONS.
- FOUNDATIONS SHALL BE PLACED ON UNDISTURBED SOIL OR COMPACTED STRUCTURAL FILL AND CLEAN AND FREE OF LOOSE DEBRIS AND STANDING WATER AT TIME OF CONCRETE PLACEMENT.
- THE SLOPE BETWEEN THE LOWER EDGES OF ADJACENT FOOTINGS SHALL NOT EXCEED 45 DEGREES WITH THE HORIZONTAL UNON IN THE GEOTECHNICAL REPORT. CONTACT STRUCTURAL ENGINEER WHERE ADEQUATE SLOPE IS NOT ACHIEVED.

- REINFORCING STEEL

- ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH ACI 318, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AND ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE", UNON.
- REINFORCING STEEL SHALL BE REVIEWED AND APPROVED PRIOR TO CONSTRUCTION. THE ARCHITECT, EACH SHOP DRAWING SUBMITTED SHALL BE STAMPED INDICATING REVIEW BY THE CONSTRUCTION MANAGER/GENERAL CONTRACTOR AND REVIEW BY THE ARCHITECT SHALL NOT BEGIN UNTIL THIS IS COMPLETE. WORK SHALL NOT BEGIN WITHOUT REVIEW BY THE DESIGN PROFESSIONALS.
- SUBMITTALS SHALL BE REVIEWED BY THE DESIGN PROFESSIONALS FOR GENERAL CONFORMANCE WITH DESIGN CONCEPT ONLY. NOTATIONS MADE BY THE DESIGN PROFESSIONALS ON THE SHOP DRAWINGS DO NOT RELIEVE THE CONTRACTOR FROM COMPLYING WITH THE REQUIREMENTS OF THE DRAWINGS.
- FOR ADDITIONAL INFORMATION ON REQUIRED SUBMITTALS, SEE INDIVIDUAL MATERIAL SECTIONS.

- DEFERRED SUBMITTALS

- DEFERRED SUBMITTALS PER SECTION 107.3.4.1 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND THE DESIGN PROFESSIONALS AND REVIEWED PRIOR TO INSTALLATION.
- DEFERRED SUBMITTALS ARE:
 - SITE-CAST TILT-UP CONCRETE ERECTION DRAWINGS
 - STEEL JOISTS AND JOIST GIRDERS
 - STRUCTURAL, STEEL CONNECTIONS
 - EXTERIOR WALL SYSTEMS
 - CURTAIN WALL AND STOREFRONT SYSTEMS
 - COLD FORMED STEEL FRAMING AND CONNECTIONS
 - ROOF TOP EQUIPMENT ANCHORAGE AND CURBS
 - GRAVITY SUPPORT AND ANCHORAGE FOR MECHANICAL, ELECTRICAL, PLUMBING, AND FIRE PROTECTION EQUIPMENT AND SYSTEMS
 - PRE-MANUF CANOPY

- DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE QUALIFIED PROFESSIONAL ENGINEER, REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, RESPONSIBLE FOR THE PREPARATION OF THESE DOCUMENTS.
- ACADE:

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE TO THE GIRTS.

- ACADE: DESIGN AND DETAILING OF THE FACADE SYSTEM AND CONNECTIONS TO THE STRUCTURE SHALL TAKE INTO CONSIDERATION THE FOLLOWING MOVEMENTS:
 - ±1/2" VERTICAL DIFFERENTIAL DEFLECTION OF SLAB EDGES ON ADJACENT FLOORS
 - ±1/2" HORIZONTAL DIFFERENTIAL DEFLECTION BETWEEN ADJACENT FLOORS
 - DIFFERENTIAL THERMAL EXPANSION / CONTRACTION BETWEEN FACADE SYSTEM AND SUPPORTING PRIMARY STRUCTURAL SYSTEM
 - THE FACADE SHALL NOT BE INSTALLED UNTIL AFTER THE SUPERSTRUCTURE HAS BEEN PLUMBED, ALL FRAMING CONNECTIONS HAVE BEEN INSTALLED, AND ALL SHORES AND RESHORES HAVE BEEN REMOVED
 - WHERE STRUCTURAL STEEL HORIZONTAL GIRTS ARE PROVIDED, THOSE GIRTS MAY BE USED ONLY TO RESIST LATERAL LOADS FROM THE FACADE. VERTICAL LOADS MAY NOT BE IMPOSED BY THE FACADE UPON THE GIRT FRAMING UNLESS THE SECTIONS AND DETAILS ON THE STRUCTURAL DRAWINGS SPECIFICALLY INDICATE THAT FACADE GRAVITY LOAD CONNECTIONS MAY BE MADE

6. DELEGATED STRUCTURAL STEEL CONNECTIONS:

- a. ALL STRUCTURAL STEEL CONNECTIONS (OTHER THAN THOSE CONNECTIONS IDENTIFIED AS "FULLY DESIGNED") AND MEMBER REINFORCEMENT AT THE CONNECTIONS SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER WORKING FOR THE FABRICATOR IN ACCORDANCE WITH AISC 360, OPTION 3. THAT ENGINEER SHALL BE LICENSED IN THE JURISDICTION IN WHICH THE PROJECT IS LOCATED.
- b. CONNECTIONS AND MEMBER REINFORCEMENT AT THE CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE REACTIONS SPECIFIED OR INDICATED ON THE DRAWINGS.
- c. WHERE SPECIFIC CONNECTION TYPES, DETAILS, CONFIGURATIONS, OR CONNECTION DESIGN LIMITATIONS ARE INDICATED, THE CONNECTIONS SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THOSE REQUIREMENTS. WHERE SPECIFIC SIZES, QUANTITIES AND DIMENSIONS OF BOLTS, WELDS, AND PLATES ARE INDICATED, THOSE ARE MINIMUM PERMITTED SIZES, QUANTITIES, AND DIMENSIONS. THE CONNECTION STRENGTH OF ALL CONNECTIONS SHALL BE DETERMINED BY THE FABRICATOR'S CONNECTION DESIGN ENGINEER, AND THE CONNECTION DETAIL SHALL BE MODIFIED IF REQUIRED TO PROVIDE THE REQUIRED STRENGTH.
- d. CONNECTION DESIGN RESPONSIBILITY SHALL INCLUDE CALCULATION OF MEMBER STRENGTH AT CONNECTIONS CONSIDERING THE EFFECTS OF COPIES, BOLT HOLES, CONNECTION ECCENTRICITY, AND CONNECTION GEOMETRY AND SHALL CONSIDER ALL LIMIT STATES INCLUDING BUT NOT LIMITED TO REVIEW OF GROSS SHEAR, NET SHEAR, BLOCK SHEAR, WEB TEAR-OUT, BEARING, FLEXURAL STRENGTH, LOCAL BUCKLING, TENSILE STRENGTH THROUGH BOTH THE NET AND GROSS SECTIONS, COMPRESSIVE STRENGTH, AND DUCTILITY. DESIGN OF CONNECTIONS SHALL BE IN ACCORDANCE WITH RECOGNIZED PUBLISHED METHODS SUCH AS THOSE PUBLISHED IN THE AISC "ENGINEERING JOURNAL", THE AISC 15TH EDITION STEEL CONSTRUCTION MANUAL, AND THE AISC STEEL CONSTRUCTION MANUAL DESIGN EXAMPLES. CONNECTION DESIGN SHALL CONSIDER TRANSFER OF STRESSES THROUGH CONNECTED AND CONNECTING MEMBERS. CONNECTION DESIGN RESPONSIBILITY SHALL INCLUDE ANALYSIS AND DESIGN OF PLATES, BRACKETS, STRUTS, STIFFENER PLATES, GUSSET PLATES, AND OTHER ELEMENTS TO TRANSFER FORCES INTO AND BETWEEN MEMBERS. CALCULATIONS SHALL SHOW FULL DETAILED CODE EQUATIONS AND METHODOLOGY FOR A REPRESENTATIVE SAMPLE OF EACH CONNECTION TYPE.
- e. AT ENGINEER OR RECORDS REQUEST, MANUAL CALCULATIONS SHALL BE SUBMITTED FOR EACH CONNECTION TYPE TO VERIFY THAT CONNECTIONS DESIGNED USING COMPUTER SOFTWARE CONSIDER ALL LIMIT STATES AND PRODUCE RESULTS IDENTICAL TO THE MANUAL CALCULATIONS.
- f. ALL BOLTED BEAM SHEAR CONNECTIONS SHALL HAVE A MINIMUM OF 2 BOLTS. ALL BEAM SHEAR CONNECTIONS SHALL BE DETAILED SO THAT THE LENGTH OF THE CONNECTION IS GREATER THAN OR EQUAL TO 50% OF THE "T" DIMENSION OF THE SUPPORTED MEMBER.
- g. BEAM SHEAR CONNECTIONS SHALL BE DESIGNED AS SIMPLE SHEAR CONNECTIONS PER AISC 360 PART 10 TO SUPPORT LRFD REACTIONS, EXCEPT WHERE INDICATED AS FULLY DESIGNED ON THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DESIGNED TO SUPPORT A MINIMUM OF 15 KIPS, WHERE LOADS ARE NOT PROVIDED ON CONTRACT DOCUMENTS. CONNECTIONS SHALL SUPPORT 60% OF THE TOTAL UNIFORM LOAD CAPACITY FOR NON-COMPOSITE BEAMS. FOR COMPOSITE BEAMS, CONNECTIONS SHALL SUPPORT 80% OF THE TOTAL UNIFORM LOAD CAPACITY.
- h. MOMENT CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE LRFD MOMENTS INDICATED ON THE DRAWINGS. WHERE NO MOMENT REACTION IS INDICATED, THE CONNECTION SHALL BE DESIGNED TO SUPPORT 15% OF THE FULL PLASTIC MOMENT STRENGTH OF THE MEMBER. MOMENT CONNECTIONS SHALL BE DESIGNED AS FULLY RESTRAINED MOMENT CONNECTIONS.
7. WELD LENGTHS INDICATED ON THE DRAWINGS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE WELD LENGTH IS NOT SPECIFIED, PROVIDE WELD ALONG ENTIRE INTERSECTION OF THE JOINED PARTS. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM WELD SIZE AS SPECIFIED IN AISC 360, TABLE J2.4.
8. SHOP CONNECTIONS SHALL BE WELDED UN. WELDS INDICATED WITH A SHOP WELD SYMBOL MAY BE MADE IN THE FIELD WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. LOCATIONS OF ALL FIELD WELDS SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS.
9. ALL WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED BY CERTIFIED WELDERS WITH EXPERIENCE AND CERTIFICATION IN THE TYPES OF WELDING INDICATED. WELDERS SHALL HAVE BEEN RECENTLY QUALIFIED AS PRESCRIBED IN "QUALIFICATION PROCEDURES" OF THE AMERICAN WELDING SOCIETY (AWS).
10. HEADED STUD ANCHORS (HSA) SHALL BE INSTALLED IN ACCORDANCE WITH AWS D1.1 AND SHALL BE AUTOMATICALLY END WELDED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS IN SUCH A MANNER AS TO PROVIDE COMPLETE FUSION BETWEEN THE END OF THE HSA AND THE STEEL SHAPE. THERE SHOULD BE NO POROSITY OR EVIDENCE OF LACK OF FUSION BETWEEN THE WELDED END OF THE HSA AND THE STEEL SHAPE. THE HSA SHALL DECREASE IN LENGTH DURING WELDING. APPROXIMATELY 1/8" FOR 5/8"Ø AND SMALLER AND 3/16" FOR LARGER THAN 5/8"Ø.
11. MINIMUM HORIZONTAL CONCRETE COVER FOR HSA TO BE 2".
12. BEAMS SHALL BE CAMBERED UPWARD WHERE SHOWN ON THE DRAWINGS. WHERE NO UPWARD CAMBER IS INDICATED, ANY MILL CAMBER SHALL BE DETAILED UPWARD IN THE BEAMS.
13. SPLICING OF STEEL MEMBERS WHERE NOT DETAILED ON THE DRAWINGS IS PROHIBITED WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION, TYPE OF SPLICE, AND CONNECTION TO BE MADE.
14. PROVIDE ONE SHOP COAT OF PAINT ON ALL STRUCTURAL STEEL NOT COVERED WITH CONCRETE, FIREPROOFING, MASONRY, OR AT CONTACT SURFACES AT HIGH STRENGTH BOLTS.
15. ALL STEEL EXPOSED TO WEATHER OR AS NOTED ON PLAN SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 G60. ABRADED AREAS TO BE TOUCHED UP WITH COLD GALVANIZING COMPOUND IN ACCORDANCE WITH ASTM A790.
16. ALL GALVANIZED HOLLOW SECTIONS SHALL HAVE WELDED CAP PLATES TO SEAL EXPOSED ENDS.
17. CUTS, HOLES, OPENINGS, ETC. REQUIRED IN STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWINGS. BURNING OR TORCHING OF HOLES, CUTS, AND OTHER FIELD MODIFICATIONS SHALL NOT BE ALLOWED, EXCEPT BY WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER.
18. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, ETC. FOR MISCELLANEOUS STEEL NOT DETAILED SPECIFICALLY ON THE STRUCTURAL DRAWINGS.
19. GROUT FOR BASE AND BEARING PLATES SHALL BE A NON-SHRINK, NON-METALLIC PRODUCT. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 7000 PSI. INSTALL GROUT PRIOR TO APPLYING SIGNIFICANT LOADING TO MEMBER.
20. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS OF ALL STRUCTURAL STEEL FOR REVIEW AND APPROVAL BEFORE FABRICATION.
21. ALL WELDING ELECTRODES SHALL BE E70XX UN. WELDING ELECTRODES FOR ASTM A913, GR65 MEMBERS AND THEIR CONNECTIONS SHALL BE E80XX.

STEEL JOISTS

1. DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE (SJI) SPECIFICATION BY A MEMBER OF THE SJI, APPROVED FOR THE TYPE OF JOIST BEING USED.
2. ATTACH STEEL JOIST TO SUPPORT AS FOLLOWS:

JOIST TYPE/SERIES	DETAILS WITH WELD INFORMATION		DETAILS WITH BOLT INFORMATION		MINIMUM END BEARING (IN)	
	FILLET WELD SIZE	WELD LENGTH (IN)	BOLT DIAMETER (IN)	BOLT MATERIAL	STEEL	MASONRY
K	1/8	2 1/2	1/2	A307	2 1/2	4
LH/DLH 02-06	3/16	2 1/2	3/4	A307	2 1/2	6
LH/DLH 07-17	1/4	2 1/2	3/4	A307	4	6
LH/DLH 18-25	1/4	4	3/4	A325	6	6
JOIST GIRDER	1/4	4	3/4	A325	6	6

WHERE WELDS OR BOLTS ARE INDICATED, WELD/BOLT TO BE INSTALLED ON BOTH SIDES OF JOIST SEAT UN.

3. DESIGN JOIST SEAT FOR 1500 LBS ROLLOVER LOAD FOR K-SERIES JOISTS ONLY.
4. ALL ROOF JOISTS SHALL BE DESIGNED FOR A 65 PSF NET WIND UPLIFT (LRFD).
5. SPECIAL JOIST AND JOIST GIRDER DEFLECTION REQUIREMENTS:
- a. ROOF JOISTS: LIVE LOAD = L160 & TOTAL LOAD = L240
- b. FLOOR JOISTS: LIVE LOAD = L480 & TOTAL LOAD = L360
6. PROVIDE SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR SPECIAL JOIST AND JOIST GIRDER DESIGNS. JOIST DESIGNER SHALL UTILIZE LOADINGS INDICATED ON THE STRUCTURAL DRAWINGS.
7. PROVIDE BRIDGING PER SJI SPECIFICATIONS. DESIGN AND PROVIDE UPLIFT BRIDGING TO WITHSTAND A NET UPLIFT PRESSURE AS INDICATED ABOVE. WHERE BRIDGING INTERFERES WITH MECHANICAL OR OTHER TRADE INSTALLATIONS, THE JOIST MANUFACTURER SHALL PROVIDE REMOVAL AND REPLACEMENT OF ANY BRIDGING.
8. HANGING OF MISCELLANEOUS ITEMS IS NOT PERMITTED FROM BRIDGING / BRACING.
9. PROVIDE ANCHORS AT EACH END OF EACH ROW OF BRIDGING TOP AND BOTTOM CHORDS EXCEPT AT EXPANSION JOINTS.
10. ALL JOIST HEADERS AND ACCESSORIES SHALL BE DESIGNED AND FURNISHED BY THE JOIST FABRICATOR.
11. STEEL JOISTS SHALL BE TOP CHORD BEARING UN.
12. THE JOIST FABRICATOR SHALL FURNISH SHOP DRAWINGS OF ALL BAR JOIST MATERIAL AND ACCESSORIES FOR REVIEW AND APPROVAL BEFORE FABRICATION.
13. PROVIDE SLOPED JOIST SEATS AS REQUIRED TO MEET ROOF SLOPE REQUIREMENTS. COORDINATE WITH ARCHITECTURAL DRAWINGS.

STEEL DECK

1. MATERIAL, DETAILING, DESIGN, MANUFACTURE, AND ERECTION OF STEEL DECKS SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE (SDI) SPECIFICATION.
2. DECK SIZE AND GAUGE INDICATED ON THE DRAWINGS ARE BASED ON THE FOLLOWING:
- a. CURRENT VERSION OF VULCRAFT VERO CATALOG FOR GRAVITY DESIGN LOADS
- b. SDI DIAPHRAGM DESIGN MANUAL 4 FOR DIAPHRAGM LOADS
3. PROVIDE MINIMUM DECK BEARING AND LAP LENGTHS PER MANUFACTURER'S RECOMMENDATIONS.
4. FOR PENETRATIONS LESS THAN OR EQUAL TO 12"Ø OR 12"x12", FRAMING OF OPENINGS FOR OTHER TRADES SHALL BE THE RESPONSIBILITY OF THE TRADES INSTALLING. HOLES LOCATED AND DIMENSIONED ON THE DRAWINGS SHALL BE THE RESPONSIBILITY OF THE DECK ERECTOR. SEE STRUCTURAL FRAMING DETAILS FOR ANY OPENING EXCEEDING LIMITS NOTED.
5. SUBMIT SHOP DRAWINGS SHOWING DECKING LAYOUT, DECK GAUGE, DECK DIRECTION, FASTENERS, AND FASTENING PATTERN FOR REVIEW AND APPROVAL.
6. FURNISH DECKING IN MINIMUM LENGTHS TO ACHIEVE 3-SPAN CONDITION UN.
7. ROOF DECK:
- a. ALL ROOF DECK SHALL BE GALVANIZED TO CONFORM TO ASTM A653 WITH A MINIMUM COATING OF G60.
- b. ALL ROOF DECK SHALL BE PRIME PAINTED GRAY TO CONFORM TO ASTM A1008, GRADE C.
- c. DO NOT EXCEED 25 LBS PER HANGER AND A MINIMUM SPACING OF 2'-0" ON CENTER WHEN ATTACHING TO ROOF DECK. THIS 25 LBS LOAD AND 2'-0" SPACING INCLUDE ADJACENT MECHANICAL, ELECTRICAL, AND ARCHITECTURAL ITEMS HANGING FROM THE DECK. IF THE HANGER RESTRICTIONS CANNOT BE ACHIEVED, SUPPLEMENTAL FRAMING SUPPORTED OFF STEEL FRAMING WILL NEED TO BE ADDED. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING LOCATION AND WEIGHT OF ALL THE ELEMENTS BEING HUNG WITH STRUCTURAL ENGINEER UN.
- d. DECK MANUFACTURER SHALL FURNISH ALL RIDGE AND VALLEY PLATES, SUMP PANS, DRAIN PLATES, AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE INSTALLATION.

COLD-FORMED STEEL (CFS) DELEGATED

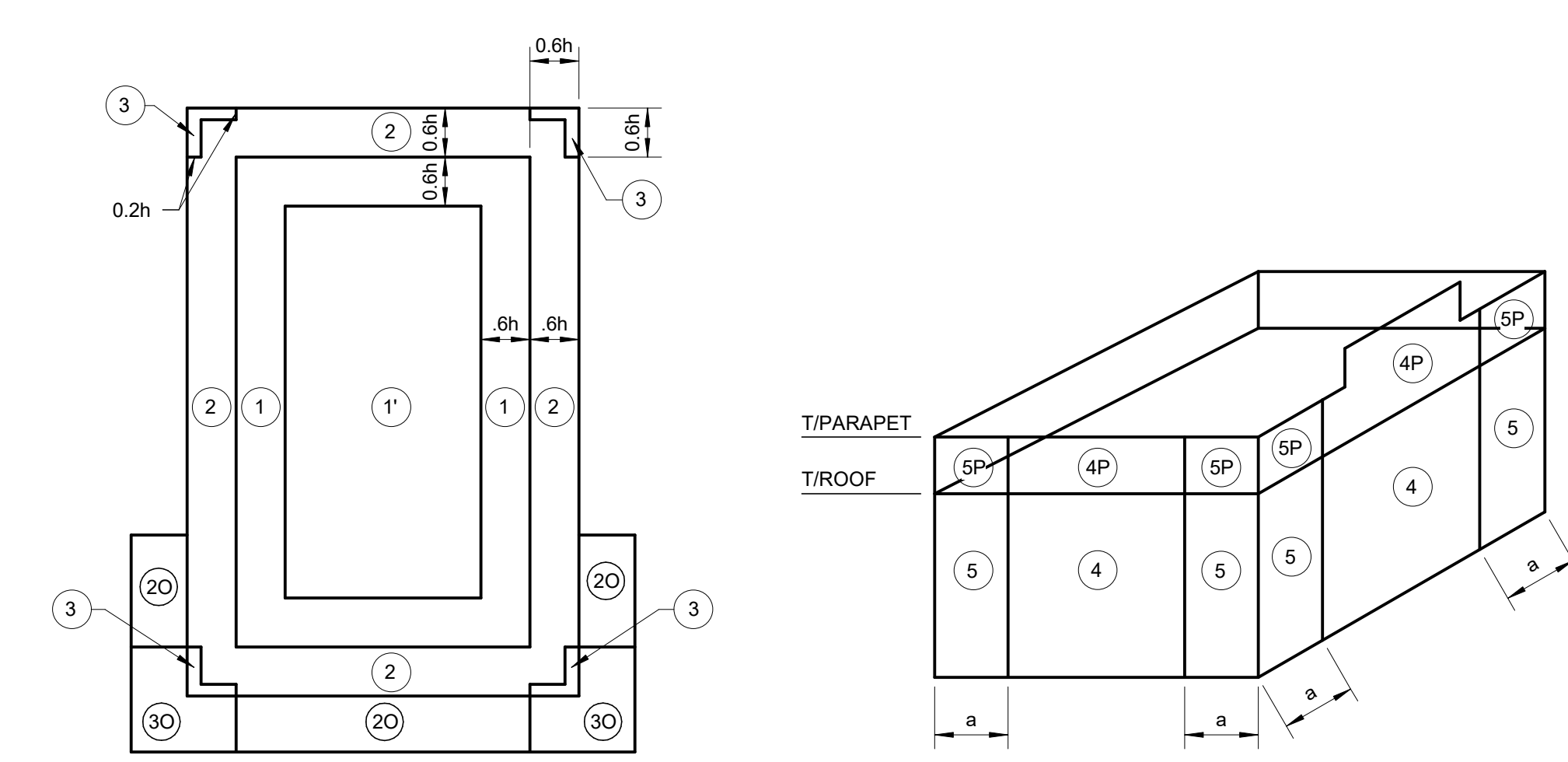
1. ALL COLD-FORMED STEEL FRAMING AND CONNECTIONS SHALL CONFORM TO THE AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURAL METALS, AISI S100.
2. THE STRUCTURAL DRAWINGS DO NOT REFLECT THE ENTIRE SCOPE OF WORK REQUIRED FOR COLD-FORMED STEEL FRAMING AND CONNECTIONS. COLD-FORMED STEEL FRAMING SHALL BE PROVIDED FOR AND COORDINATED WITH ARCHITECTURAL, MEP, AND OTHER DRAWINGS.
3. COLD-FORMED STEEL FRAMING AND CONNECTIONS INDICATED OR REFERENCED IN THE CONTRACT DOCUMENTS ARE FOR PURPOSES OF CONVEYING DESIGN INTENT ONLY. THE COMPLETE DESIGN OF COLD-FORMED STEEL SYSTEMS THAT SUPPORT GRAVITY OR LATERAL LOADS SHALL BE THE RESPONSIBILITY OF THE COLD-FORMED STEEL SUBCONTRACTOR AND THEIR STRUCTURAL ENGINEER. THE DESIGN OF COLD-FORMED STEEL SYSTEMS SHALL CONFORM TO THE GOVERNING BUILDING CODE AND REGULATIONS, AS WELL AS ANY SPECIAL CONDITIONS AND REQUIREMENTS AS INDICATED IN THE CONTRACT DOCUMENTS.
4. STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) MEMBER DESIGNATIONS ARE SPECIFIED ON THE CONTRACT DOCUMENTS UN. WHERE SPECIFIC MANUFACTURERS ARE SPECIFIED AND PROPRIETARY FRAMING DESIGNATIONS INDICATED, FRAMING BY OTHER MANUFACTURERS MAY BE USED PROVIDED THOSE FRAMING MEMBERS HAVE DIMENSIONS EQUIVALENT TO THE SPECIFIED FRAMING AND ENGINEERING PRIORITIES EQUIVALENT TO OR GREATER THAN THE SPECIFIED FRAMING. SUCH PROPOSED SUBSTITUTIONS ARE PERMITTED ONLY IF APPROVED IN WRITING BY THE DESIGN PROFESSIONALS.
5. STRUCTURAL COLD-FORMED STEEL IS DEFINED AS THE FOLLOWING:
- a. ANY COLD-FORMED STEEL THICKER THAN 20GA (33 MIL)
- b. ANY EXTERIOR COLD-FORMED FRAMING AND CONNECTIONS
- c. ANY LOAD BEARING MEMBER
- d. ANY MEMBER CALLED OUT AND SPECIFIED IN THE STRUCTURAL DRAWINGS
- e. ALL OTHER COLD-FORMED STEEL IS NON-STRUCTURAL AND NOT A PART OF THE STRUCTURAL PACKAGE
6. STRUCTURAL COLD-FORMED STEEL (ROLLED SECTIONS, CONNECTION MATERIAL, AND STIFFENER PLATES) SHALL CONFORM TO THE FOLLOWING STANDARDS:
- | | | |
|-----------------------------|------------------|-----------|
| 16GA (43 MIL) AND THINNER | ASTM A653, GR 33 | Fy=33 KSI |
| 16GA (54 MIL) AND THICKER | ASTM A653, GR 50 | Fy=50 KSI |
| CONNECTION MATERIAL > 3/16" | ASTM A36 | Fy=36 KSI |
| HOT-DIP COATING | ASTM A924, G60 | |
| ELECTRO-PLATE COATING | ASTM A591 | |
| ALUMINUM-ZINC COATING | ASTM A792, GR 40 | |
| ELECTRODES FOR ARC WELDING | AWS 5.1 E60XX | |
7. WIND LOADS PERPENDICULAR TO THE PLANE OF EXTERIOR WALLS SHALL BE COMPUTED BASED UPON THE COMPONENT AND CLADDING WIND PRESSURES INDICATED IN THE GENERAL NOTES.
8. DESIGN SHALL INCLUDE DESIGN AND DETAILING OF FRAMING, BRIDGING, TEMPORARY AND PERMANENT BRACING, AND ALL FRAMING CONNECTIONS.
9. COLD-FORMED STEEL COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR A FULL ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE SECURED IN PLACE UNTIL PROPERLY ATTACHED.
10. SPLICING OF COLD-FORMED MEMBERS IS NOT PERMITTED.
11. CUTOUTS, HOLES, OR NOTCHES ARE NOT PERMITTED IN COLD-FORMED STEEL, ROOF AND FLOOR JOISTS, HEADERS, OR BEAMS WITHOUT PRIOR WRITTEN APPROVAL OF EITHER THE STRUCTURAL ENGINEER OR THE COLD-FORMED STEEL ENGINEER.
12. CUTTING OF COLD-FORMED STEEL MEMBERS SHALL BE ACCOMPLISHED WITH A SAW OR SHEARS. TORCH CUTTING OF SUCH MEMBERS IS NOT PERMITTED. THE CUTTING OF ANY LOAD BEARING MEMBER IS PROHIBITED.
13. HOLES FIELD CUT THROUGH COLD-FORMED STUD FRAMING SHALL BE MADE WITHIN THE LIMITATIONS OF THE PRODUCT AND THE PRODUCT DESIGN. HOLES SHALL BE REINFORCED AS RECOMMENDED BY THE MANUFACTURER.
14. ALL WELDS SHALL COMPLY WITH THE REQUIREMENTS OF THE NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AISI S100, AND THE STRUCTURAL WELDING CODE - SHEET STEEL, AWS D1.3.
15. CONNECTIONS SHALL BE MADE BY WELDS, SCREWS, BOLTS, OR OTHER APPROVED FASTENING DEVICES OR METHODS PROVIDING POSITIVE ATTACHMENT AND RESISTANCE TO LOOSENING. FASTENERS SHALL BE MATERIAL COMPATIBLE WITH THE COLD-FORMED STEEL.
16. LIMIT HORIZONTAL COLD-FORMED STUD OUT-OF-PLANE DEFLECTIONS DUE TO WIND LOADS TO THE FOLLOWING:
- a. STUDS BACKING UP BRICK, MASONRY, AND STONE: L/600 OR 0.3" MAX
- b. STUDS BACKING UP ALL OTHER FACADE MATERIALS: L/360 UN
17. TEMPORARY BRACING SHALL BE PROVIDED WHERE REQUIRED TO ENSURE ALIGNMENT UNTIL WALL PANELS ARE INSTALLED AND CONSTRUCTION IS COMPLETED.
18. CONSTRUCTION SHALL NOT BEGIN UNTIL SHOP DRAWINGS AND CALCULATIONS HAVE BEEN REVIEWED BY THE DESIGN PROFESSIONALS. ALL COLD-FORMED STEEL FRAMING AND CONNECTIONS INCLUDING CONNECTIONS TO THE SUPPORTING STRUCTURE SHALL BE DESIGNED BY THE COLD-FORMED STEEL FRAMING CONTRACTORS ENGINEER LICENSED IN THE PROJECT'S JURISDICTION. ALL SUBMITTALS, INCLUDING DESIGN CALCULATIONS AND SHOP DRAWINGS, SHALL BE SIGNED AND SEALED BY THAT ENGINEER.

POST-INSTALLED ANCHORS

1. BASIS OF DESIGN ANCHORS:
- | INSTALLATION CONDITION | ANCHOR TYPE |
|--|---|
| EXPANSION ANCHORS INTO CONCRETE | SIMPSON STRONG-BOLT 2 (ESR-3037) |
| SCREW ANCHORS > 1/4"Ø INTO CONCRETE | SIMPSON TITEN HD (ESR-2713) |
| ADHESIVE ANCHORS INTO CONCRETE, NEW AND EXISTING | SIMPSON SET-3G w/ GR55 THREADED ROD (ESR-4057) |
| EXPANSION ANCHORS INTO GROUTED CMU | SIMPSON STRONG-BOLT 2 (IAPMO UES ESR-240) |
| SCREW ANCHORS > 1/4"Ø INTO GROUTED CMU | SIMPSON TITEN HD (ESR-1056) |
| SCREW ANCHORS > 1/4"Ø INTO CONCRETE OR GROUTED CMU | SIMPSON TITEN TURBO (CONCRETE: IAPMO UES ESR-172) (MASONRY: IAPMO UES ER-716) |
| ADHESIVE ANCHORS IN GROUTED CMU OR SOLID BRICK | SIMPSON SET-3G w/ GR55 THREADED ROD (ESR-4844) |
| ADHESIVE ANCHORS INTO HOLLOW CMU, BRICK OR MULTI-WYTHE BRICK WALLS | SIMPSON SET-3G w/ GR55 THREADED ROD (ESR-4844) |
| POWDER-ACTUATED FASTENERS (PAFs) IN CONCRETE | SIMPSON POPA (ESR-2138) |

BAR SIZE	CONCRETE COMPRESSIVE STRENGTH				
	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI
#3	22"	19"	17"	15"	14"
#4	29"	25"	22"	20"	19"
#5	36"	31"	28"	26"	24"
#6	43"	38"	34"	31"	28"
#7	62"	54"	49"	44"	41"
#8	70"	62"	56"	51"	47"
#9	76"	70"	62"	57"	53"
#10	84"	78"	70"	63"	59"
#11	96"	87"	76"	70"	64"

NOTES:
1. THE SPlice LENGTH BY THE FOLLOWING (INCREASED LENGTHS ARE CUMULATIVE):
1. HORIZONTAL TOP BARS WITH GREATER THAN 12" OF CONCRETE BELOW +30%
2. BAR SPACING LESS THAN 2 BAR DIAMETERS +50%
3. EPOXY COATED BARS WHEN CLEAR COVER IS LESS THAN 3 BAR DIAMETERS OR CLEAR SPACING IS LESS THAN 6 BAR DIAMETERS +50%
OTHER EPOXY COATED BARS +20%



ROOF PLAN (GENERIC BUILDING SHOWN)

WALLS (GENERIC BUILDING SHOWN)

COMPONENT & CLADDING DESIGN WIND PRESSURES (PSF)							
ROOF							
ZONE	10 SF	20 SF	50 SF	100 SF	200 SF	350 SF	500 SF
NEGATIVE 1	-55.2	-51.6	-46.8	-43.1	-39.5	-36.5	-34.7
NEGATIVE 1'	-31.7	-31.7	-31.7	-31.7	-27.3	-23.7	-21.5
NEGATIVE 2	-72.8	-68.1	-62.0	-57.3	-52.6	-48.8	-46.4
NEGATIVE 3	-99.3	-89.9	-77.5	-68.1	-58.8	-51.2	-46.4
POSITIVE ALL ZONES	+16.0	+16.0	+16.0	+16.0	+16.0	+16.0	+16.0
OVERHANG 1 & 1'	-49.9	-49.0	-47.9	-39.4	-33.3	-29.4	
OVERHANG 2	-67.5	-61.3	-53.0	-46.8	-40.6	-35.5	-32.3
OVERHANG 3	-94.0	-83.0	-68.6	-57.7	-46.7	-37.9	-32.3
PARAPET							
ZONE	10 SF	20 SF	50 SF	100 SF	200 SF	500 SF	
CASE A:	ZONE 2	+100.2	+93.7	+85.1	+78.6	+72.1	+63.5
	ZONE 3	+128.3	+116.9	+101.7	+90.2	+78.7	+63.5
CASE B:	INTERIOR	-59.2	-56.2	-52.2	-49.2	-46.2	-42.3
	CORNER	-67.6	-63.1	-57.2	-52.7	-48.2	-42.3
WALL							
ZONE	10 SF	100 SF	200 SF	500 SF			
POSITIVE ZONES 4 & 5	+31.7	+27.0	+25.6	+23.8			
NEGATIVE 4	-34.4	-29.7	-28.3	-26.4			
NEGATIVE 5	-42.3	-33.0	-30.1	-26.4			

NOTES:
1) PRESSURES ARE FOR THE SQUARE FOOT (SF) TRIBUTARY AREA SHOWN. FOR OTHER TRIBUTARY AREAS, LINEARLY INTERPOLATE BETWEEN VALUES SHOWN ABOVE.
2) POSITIVE PRESSURES ACT TOWARD THE BUILDING. NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
3) SEE DIAGRAMS FOR LOCATION OF ZONES.
4) PRESSURES SHOWN ARE ULTIMATE PRESSURES. MULTIPLY VALUES BY 0.6 FOR NOMINAL PRESSURES.

STRUCTURAL ABBREVIATIONS			
#	NUMBER OR POUND	GA	GAGE
Ø	ROUND OR DIAMETER	GB	GRAVITIZED
□	SQUARE	GC	GENERAL CONTRACTOR
Δ	AND	GT	GIRDER TRUSS
@	AT	HGT	HEIGHT
ALUM	ALUMINUM	NS	HORIZONTAL
AR	ANCHOR ROD	HSA	HEADED STUD ANCHOR
ARCH	ARCHITECTURE	HT	HIP TRUSS
ASSY	ASSEMBLY	IF	INSIDE FACE
B/	BOTTOM OF	INFO	INFORMATION
BETWN	BETWEEN	INT	INTERIOR
BULDG	BUILDING	JNT	JOINT
BM	BEAM	JST	JOIST
BOT	BOTTOM	LB	POUND
BPL	BASE PLATE	LG	LONG
BRG	BEARING	LH	LONG LEG HORIZONTAL
C TO C	CENTER TO CENTER	LLV	LONG LEG VERTICAL
CANT	CANTILEVER	LH	LONG SIDE HORIZONTAL
CFS	COLD FORMED STEEL	LSV	LONG SIDE VERTICAL
CJ	CONSTRUCTION JOINT	MANUF	MANUFACTURER
CL	CENTER LINE	MATL	MATERIAL
CLR	CLEAR	MEXH	MECHANICAL
CMU	CONCRETE MASONRY UNIT	MIN	MINIMUM
COL	COLUMN	MISC	MISCELLANEOUS
CONC	CONCRETE	NIC	NOT IN CONTRACT
CONN	CONNECTION	No or No	NUMBER
CONST	CONSTRUCTION	NS	NEAR SIDE
CONTR	CONTRACT	NTS	NOT TO SCALE
CTR	CENTER	OC	ON CENTER
CTRD	CENTERED	OD	OUTSIDE DIAMETER
DBA	DEFORMED BAR ANCHOR	OFF	OUTSIDE FACE
DEFL	DEFLECTION	OPNG	OPENING
DET	DETAIL	OPP	OPPOSITE
DIA	DIAMETER	PERB	PRE-ENGINEERED METAL BUILDING
DIAG	DIAGONAL	PL	PLATE
DIFF	DIFFERENT	PANEL	PANEL
DIM	DIMENSION	PREFAB	PRE-FABRICATED
DWG	DRAWING	PSF	POUNDS PER SQUARE FOOT
EA	EACH	QTY	QUANTITY
EF	EACH FACE	R or RAD	RADIUS
EJ	EXPANSION JOINT	ROUND	ROUND
ELEV	ELEVATION	REF	REFERENCE
ELEC	ELECTRICAL	REIN	REINFORCEMENT
ELEV	ELEVATOR	REQ	REQUIRED
ENGR	ENGINEER	RECD	RECORD
ENR	ENGINEER OF RECORD	RET	RETAINING
EQ	EQUAL	REV	REVISION
EQUIP	EQUIPMENT		
EXIST	EXISTING		
EXP	EXPANSION		
EXT	EXTERIOR		
EW	EACH WAY		
FB	FLAT BAR		
FD	FLOOR DRAIN		
FDN	FOUNDATION		
FF	FINISH FLOOR		
FLR	FLOOR		
FS	FAR SIDE		
FTG	FOOTING		

GENERAL SYMBOLS

	PLAN, SECTION OR DETAIL NO SHEET NUMBER
	NORTH ARROW
	KEYED NOTE TO PLAN
	REVISION NUMBER
	FOOTING STEP



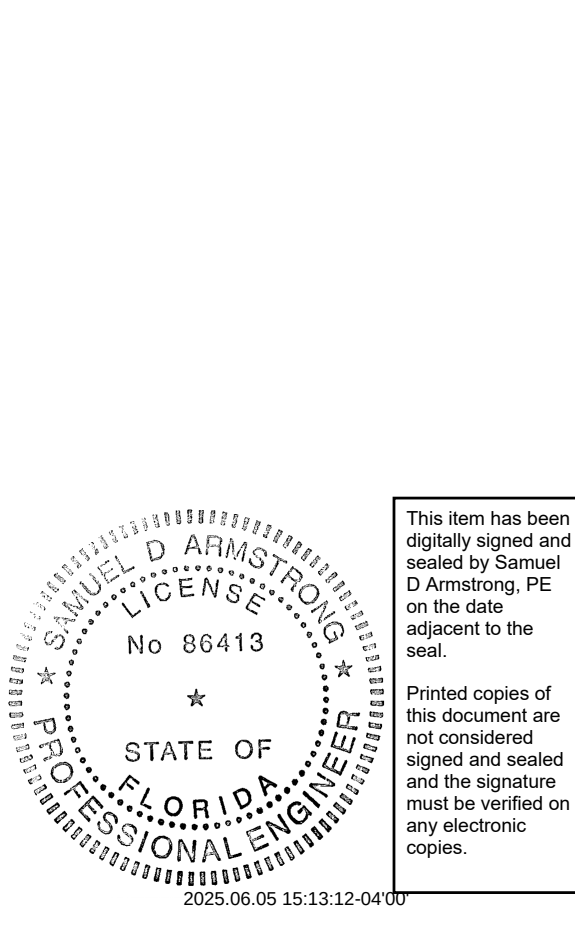
www.kasperarch.com
FL License No.: A2A2000802



9133 R.C. SKINNER PARKWAY
SUITE 1
JACKSONVILLE, FL 32256
P: 904.483.5200
PROJECT #23000902.00
IMEG RESERVES PROPRIETARY RIGHTS, INCLUDING COMPENSITY TO THIS DRAWING AND THE DATA SHOWN THEREON. SUCH DRAWINGS AND DATA ARE THE EXCLUSIVE PROPERTY OF IMEG AND SHALL NOT BE USED OR REPRODUCED WITHOUT THE EXPRESS WRITTEN APPROVAL AND PARTICIPATION OF IMEG. ©2025 IMEG CONSULTANTS CORP.

UNLESS OTHERWISE NOTED
VER VERIFY
VERT VERTICAL
WA WHERE APPLICABLE
w/ or w/ WITH
w/o WITHOUT
WIP WORK POINT
WS WATERSTOP
WWF WELDED WIRE FABRIC

GENERAL SYMBOLS	
	PLAN, SECTION OR DETAIL NO SHEET NUMBER
	NORTH ARROW
	KEYED NOTE TO PLAN
	REVISION NUMBER
	FOOTING STEP



ERIK C. KASPER AR#0017162

BUSY BEE ELLISVILLE

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 220224

DATE: 06/02/2025

PRIOR ISSUES:

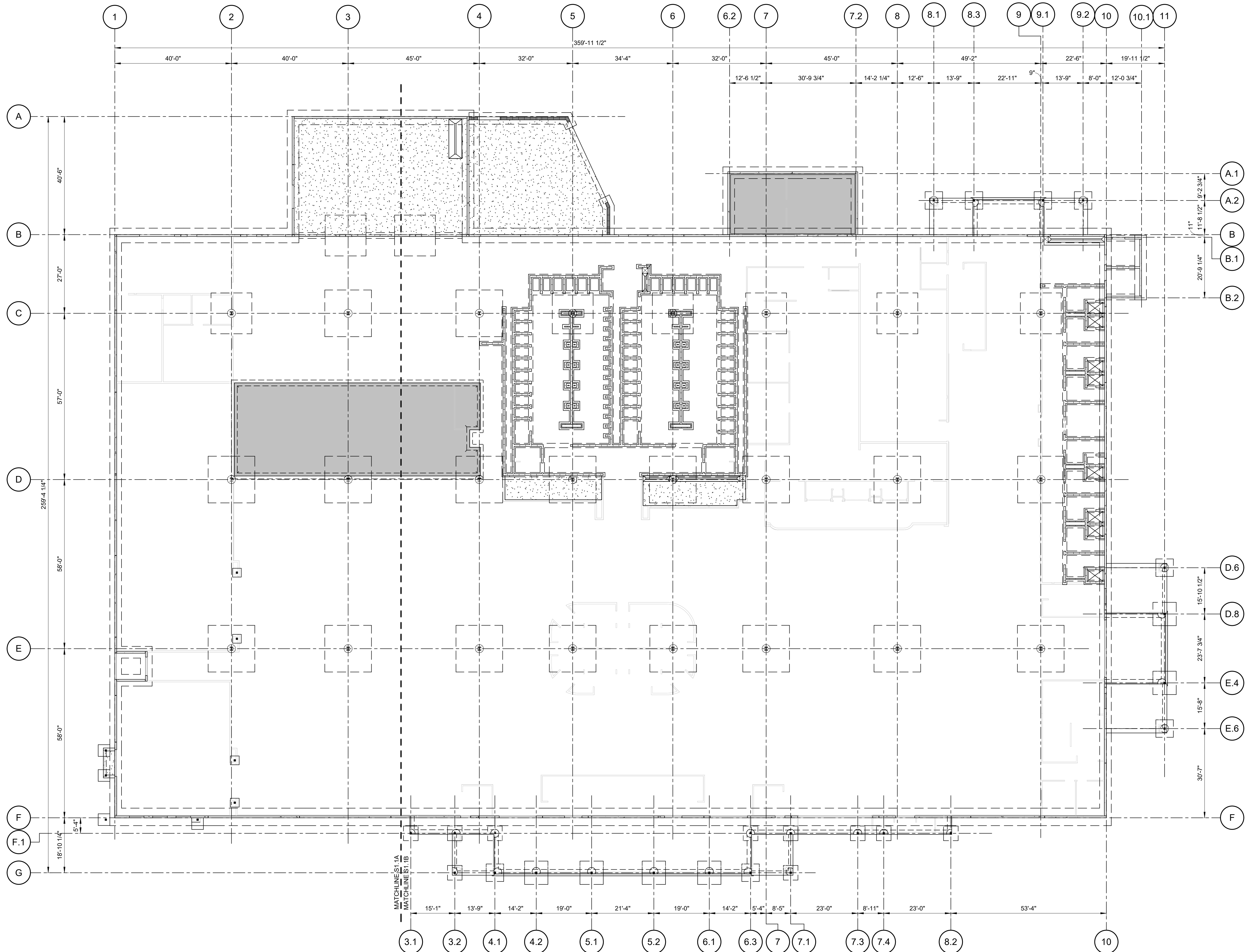
GENERAL NOTES

S0.2

DRAWN BY: Author
CHECKED BY: Checker

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.

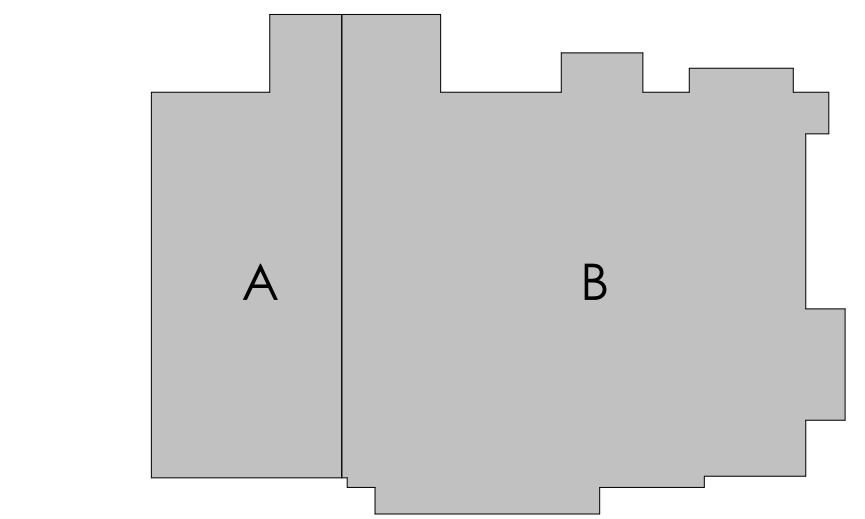
100%
CONSTRUCTION DOCUMENTS



1 OVERALL FOUNDATION PLAN
SCALE: 1/16" = 1'-0"

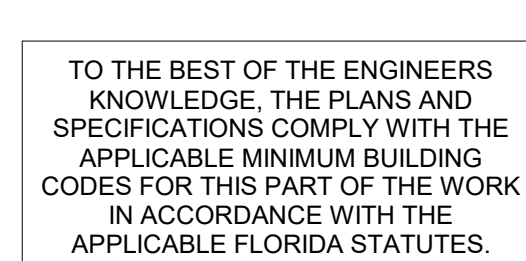
- FOUNDATION PLAN NOTES:
- 1) FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEET S0.2
 - 2) FOR DIMENSIONS, SECTIONS & ELEVATIONS NOT SHOWN, SEE ARCH.
 - 3) ELEVATIONS ARE BASED ON T/SLAB DATUM EL 0'-0". SEE CIVIL DWGS FOR ACTUAL ELEVATION.
 - 4) TOP OF SPREAD FOOTINGS EL = -2'-0", TYP UON.
 - 5) TOP OF TILT PANEL FOOTINGS EL = -4'-0", TYP UON.
 - 6) F# & W## DENOTES FOOTING TYPES, SEE SCHEDULE THIS SHEET.
 - 7) SEE 2/S2.1 FOR PIPE PENETRATIONS THROUGH BUILDING PERIMETER.
 - 8) SEE DETAIL 3/S2.1 FOR REINF @ WALL FOOTING CORNER.
 - 9) PROVIDE 2#x3'-0" MID SLAB @ ALL RE-ENTRANT CORNERS.
 - 10) FOR TYPICAL COLUMN BASE PLATES, SEE DETAIL 5/S2.1.
 - 11) DENOTES NON-LOAD BEARING 8" CMU WALL, FOR TYP CMU WALL DETAILS AND REINFORCEMENT, SEE GENERAL NOTES & SHEET S3.1. AT LOCATIONS WHERE ARCH SPECIFIES 4" CMU, CMU TO BE UNREINFORCED & FULLY GROUTED.
 - 12) DENOTES 12" CMU, SEE PLAN FOR LOCATION & REINF.
 - 13) DENOTES TILT UP CONCRETE PANEL, 7 1/4" TOTAL THICKNESS (6 1/2" STRUCTURAL + 3/4" REVEAL). SEE SHEETS S7.1-S7.4 FOR INFO.
 - 14) DENOTES 8" NW CONCRETE SLAB ON GRADE w/ #5 @ 12" EA WAY CT'D IN SLAB.
 - 15) DENOTES 6" NW CONCRETE SLAB ON GRADE w/ #5 @ 12" OC EA WAY CT'D IN SLAB, ON 6" OF RIGID INSULATION w/ 4" MID SLAB.
 - 16) DENOTES RECESS IN SLAB, COORD DEPTH w/ ARCH, SEE DETAIL 10/S2.1 FOR INFO.
 - 17) DENOTES SLOPE IN SLAB, COORD SLOPE w/ ARCH SEE DETAIL 9/S2.1 FOR INFO.
 - 18) PROVIDE 3'-0" SQ x 1'-6" DEEP THICKENED SLAB w/ (4)#5 BOT EA WAY, CONNECT HSB TO SLAB w/ EMBED PL 1/2"x14"x14" w/ (4) 1/2"x8" LG HAS.
 - 19) AT DRAIN, SEE DETAILS 6/S2.2, 4/S6.2 & 8/S6.2.

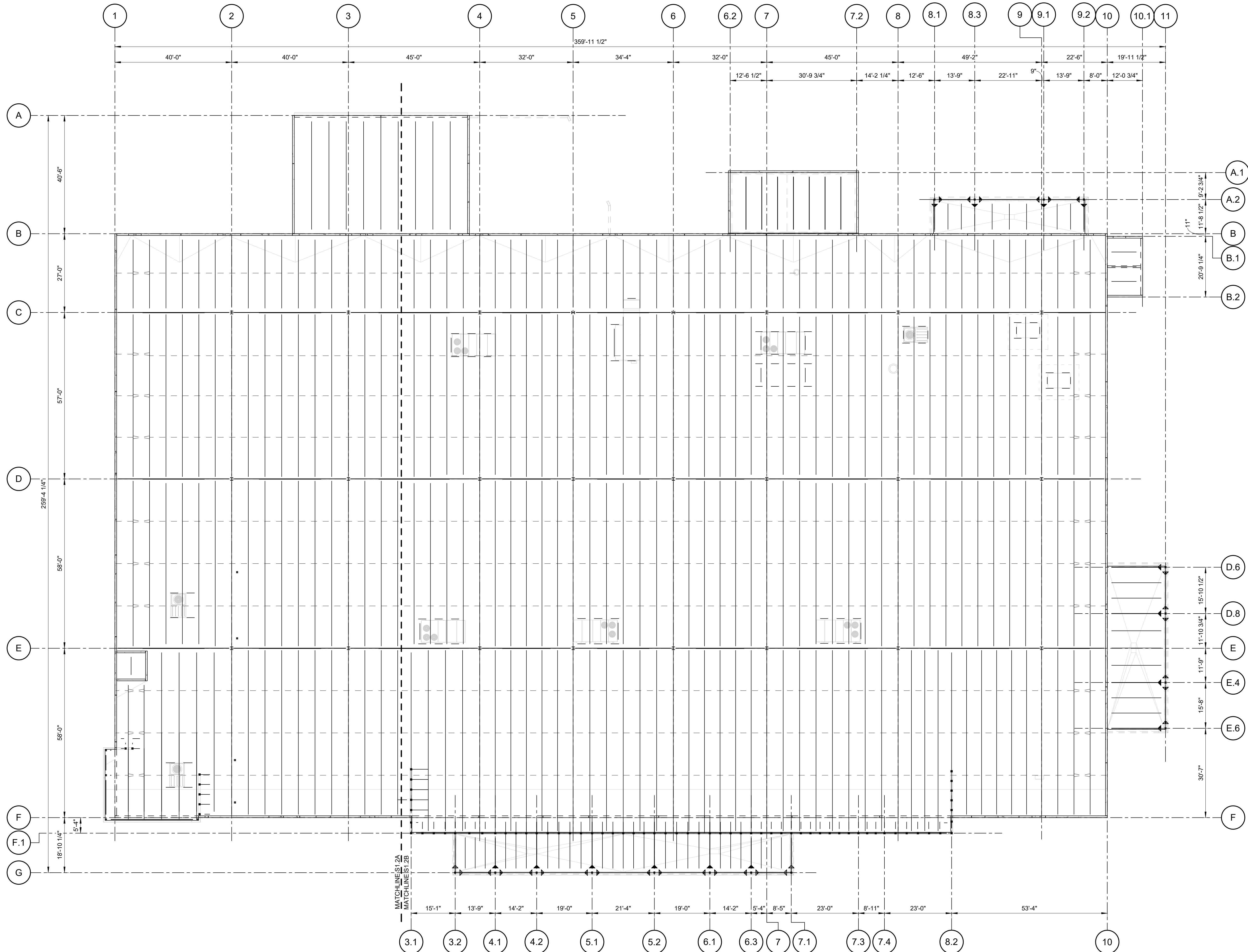
FOOTING SCHEDULE							
TYPE	LENGTH	WIDTH	THICKNESS	BOTTOM REINF (LONG WAY)	BOTTOM REINF (SHORT WAY)	TOP REINF (LONG WAY)	TOP REINF (SHORT WAY)
F4.0	4'-0"	4'-0"	1'-0"	5#5	5#5		
F5.0	5'-0"	5'-0"	1'-0"	6#5	6#5	6#5	6#5
F6.0	6'-0"	6'-0"	2'-0"	7#5	7#5	7#5	7#5
F8.0	8'-0"	8'-0"	2'-0"	12#5	12#5	12#5	12#5
F14.0	14'-0"	14'-0"	2'-0"	18#5	18#5	18#5	18#5
F16.0	16'-0"	16'-0"	2'-0"	22#5	22#5	22#5	22#5
WF2.0	CONT	2'-0"	1'-0"	3#5	#5 @ 30" OC		
WF4.0	CONT	4'-0"	2'-0"	5#5	#5 @ 12" OC		
WF5.0	CONT	5'-0"	2'-0"	6#5	#5 @ 12" OC		
WF6.0	CONT	6'-0"	2'-0"	7#5	#5 @ 12" OC		



KEY PLAN

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.

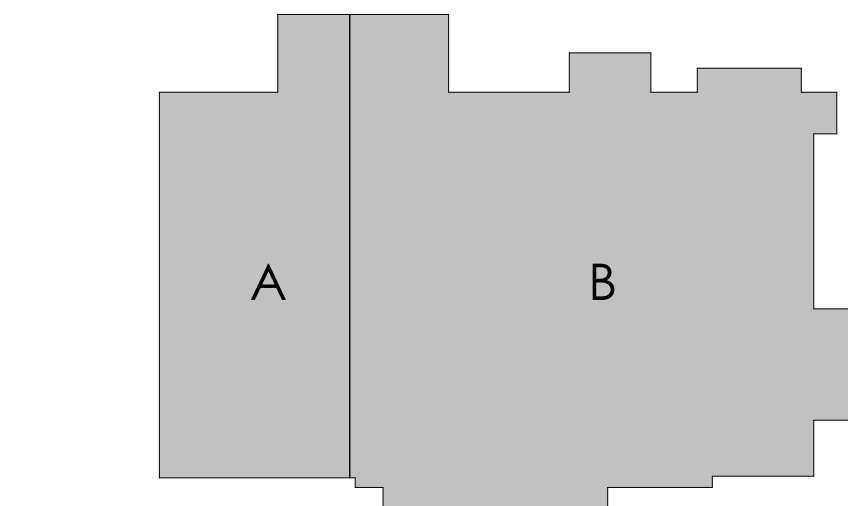




OVERALL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0"

- ROOF FRAMING PLAN NOTES:
- 1) FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEET S0.2.
 - 2) SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, SECTIONS, AND ELEVATIONS NOT SHOWN.
 - 3) 1/8" ELEVATIONS VARY. SEE PLAN AND ARCH.
 - 4) ELEVATIONS ARE BASED ON T&B DATUM EL. 0'-0". SEE CIVIL DWGS FOR ACTUAL ELEVATION.
 - 5) ———> INDICATES SPAN DIRECTION OF METAL DECK. SEE PLAN FOR SIZE AND GAUGE.
 - 6) ROOF DECK ATTACHMENT SHALL BE PER DETAIL 1/S5.1.
 - 7) FOR BEAM TO BEAM & BEAM TO COLUMN CONNECTION, SEE DETAILS 1/S4.1 2/S4.1 7/S4.1.
 - 8) ———> INDICATES MOMENT CONNECTION. SEE 3/S4.1.
 - 9) VERIFY ALL DIMENSIONS AFFECTED BY MECHANICAL WITH ACTUAL EQUIPMENT PURCHASED PRIOR TO FABRICATION OF STEEL FRAMING.
 - 10) BEAMS & OTHER FRAMING MEMBERS WERE DESIGNED TO ACCOMMODATE A MISCELLANEOUS HANGING LOAD (INCLUDING DUCTWORK, SPRINKLER SYSTEM, PLUMBING, CEILING & OTHER MISCELLANEOUS HANGING COMPONENTS) OF 8 PSF. HANGING LOADS SHALL BE DISTRIBUTED AS TO NOT EXCEED THIS VALUE.
 - 11) FOR ROOF SLOPE, SEE ARCH.
 - 12) FOR ADDITIONAL FRAMING AT MECHANICAL UNITS AND ROOF PENETRATIONS, SEE DETAILS 2/S5.1, 3/S5.1 & 4/S5.1. SOME PENETRATIONS MAY NOT BE SHOWN. COORDINATE AND VERIFY OPENINGS WITH ARCH & MECH DWGS.
 - 13) FOR FRAMING FUTURE HOODS OVER RESTAURANT, SEE DETAIL 2/S5.1.
 - 14) FOR EDGE ANGLE SPLICE, SEE DETAIL 9/S4.1.
 - 15) ALL EXTERIOR STEEL TO BE GALVANIZED.
 - 16) ALL EXPOSED STEEL TO BE FINISHED IN ACCORDANCE W/ AESS SPECS.
 - 17) ALL STEEL BEAMS TO HAVE L2x2x1/4 T&B BRACING LOCATED AT THIRD POINTS ALONG THE LENGTH OF THE BEAM.
 - 18) JOISTS & BEAMS TO BE SPACED EVENLY BETWEEN COLUMNS AND NOT TO EXCEED 6'-0".
 - 19) FOR FIRE SPRINKLER SUPPORT SEE DETAIL 10/S6.1.
 - 20) JOIST SEATS SHALL BE DESIGNED TO RESIST A LATERAL LOAD APPLIED TO THE TOP CHORD, PERPENDICULAR TO THE SPAN (ROLL-OVER), PROVIDE A MINIMUM ULTIMATE ROLLOVER FORCE OF 1500 LB.
 - 21) JOIST MANUF. TO DESIGN JOISTS AT RTU'S FOR RTU LOADING AS SHOWN ON PLAN. THE WEIGHTS AS SHOWN INCLUDE A 40# CURB.

- 22) JOISTS DENOTED 'SP' ARE SPECIAL DESIGNED JOISTS BY JOIST MANUFACTURER SPECIAL LOADING REQUIREMENTS (IN ADDITION TO TYPICAL LOADING) ARE NOTED ON PLAN AND IN DETAILS.
- 23) JOIST MANUF. TO COORD BRIDGING REQUIREMENTS W/ DUCTWORK BETWEEN JOIST. SEE MECH FOR DUCT LAYOUT.
- 24) THE JOIST SIZING BENEATH THE RTU'S ARE BASED ON THE RTU WEIGHTS SHOWN. NOTIFY ENGINEER IF ACTUAL EQUIPMENT EXCEEDS THIS WEIGHT.
- 25) AT LOCATIONS WHERE LH JOISTS AND K-SERIES JOISTS BEAR ON THE SAME ELEMENT, THE K-SERIES JOISTS SHALL BE DESIGNED FOR A 5' JOIST SEAT (BY JOIST ENGINEER).
- 26) SCREENWALL LOCATED OVER STARBUCKS CONSISTS OF HSS4x4x3/8 SPACED AT 4'-0" MAX, SEE ARCH FOR LOCATION. SEE 9/S6.2 AND 10/S6.2 FOR ANGLE SUPPORT INFORMATION.
- 27) FOR CONCENTRATED LOADS AT JOIST, SEE 11/S5.1.
- 28) FOR HSS WIND GIRT CONNECTION, SEE DETAIL 6/S4.1.



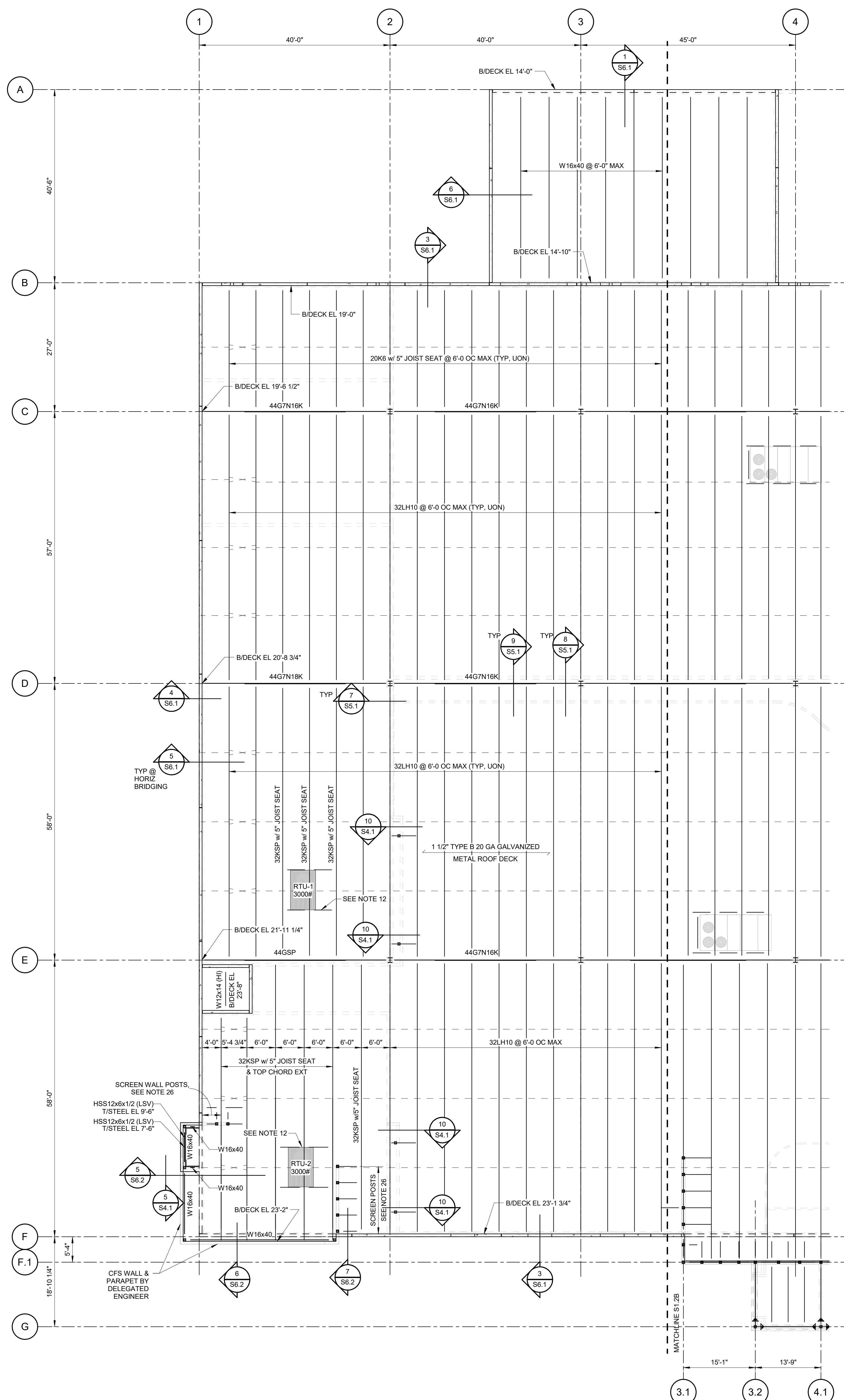
KEY PLAN

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.

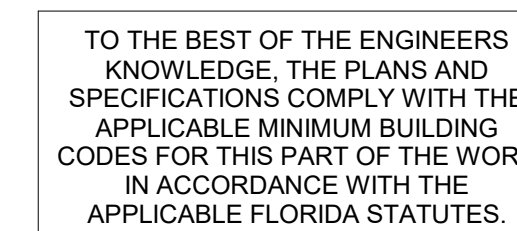


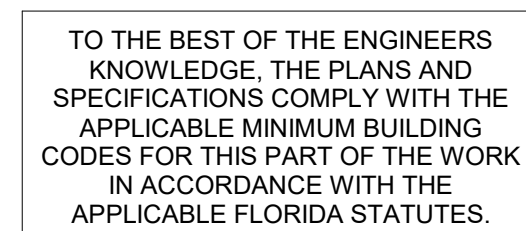
PRIOR ISSUES:

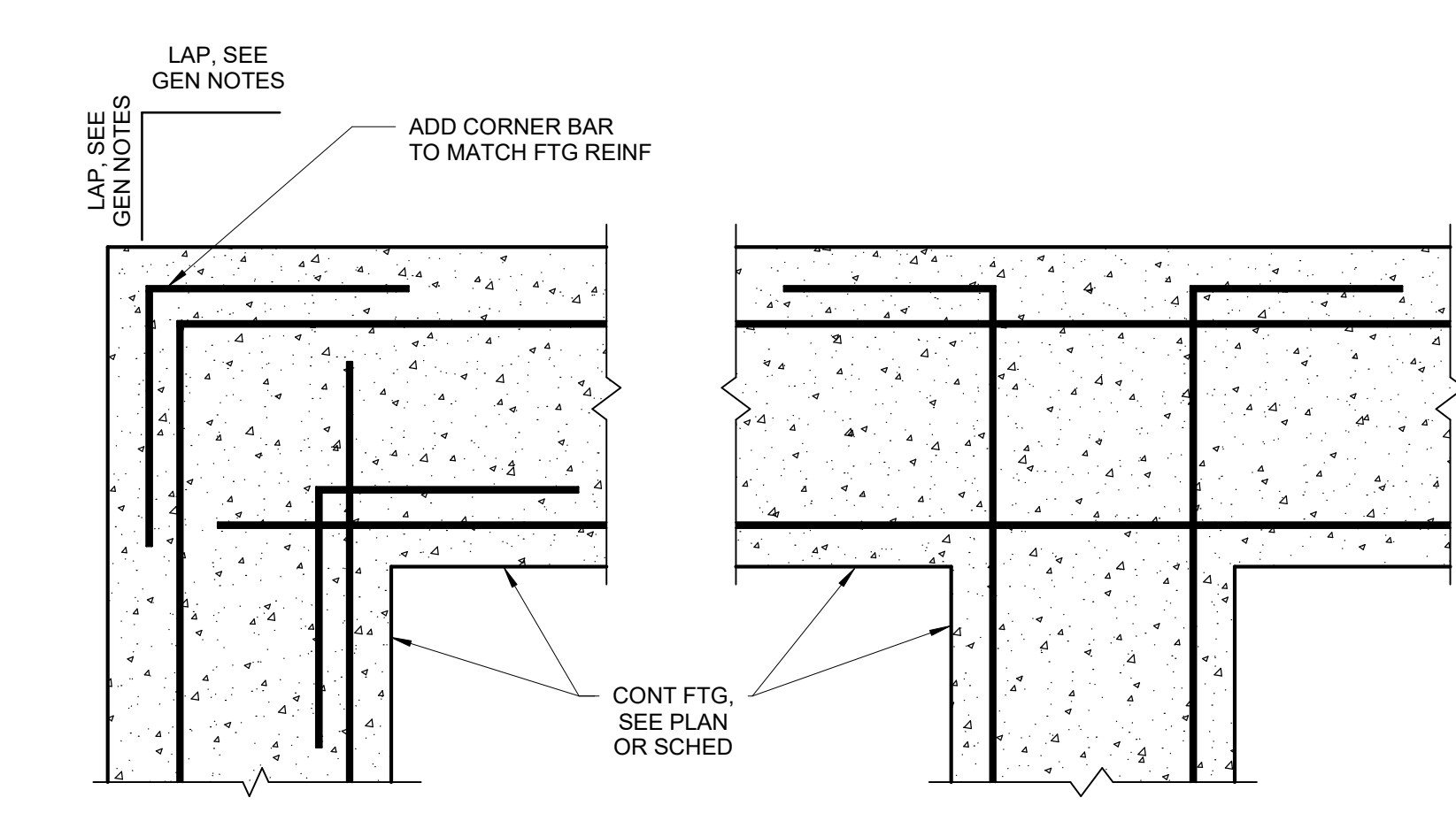
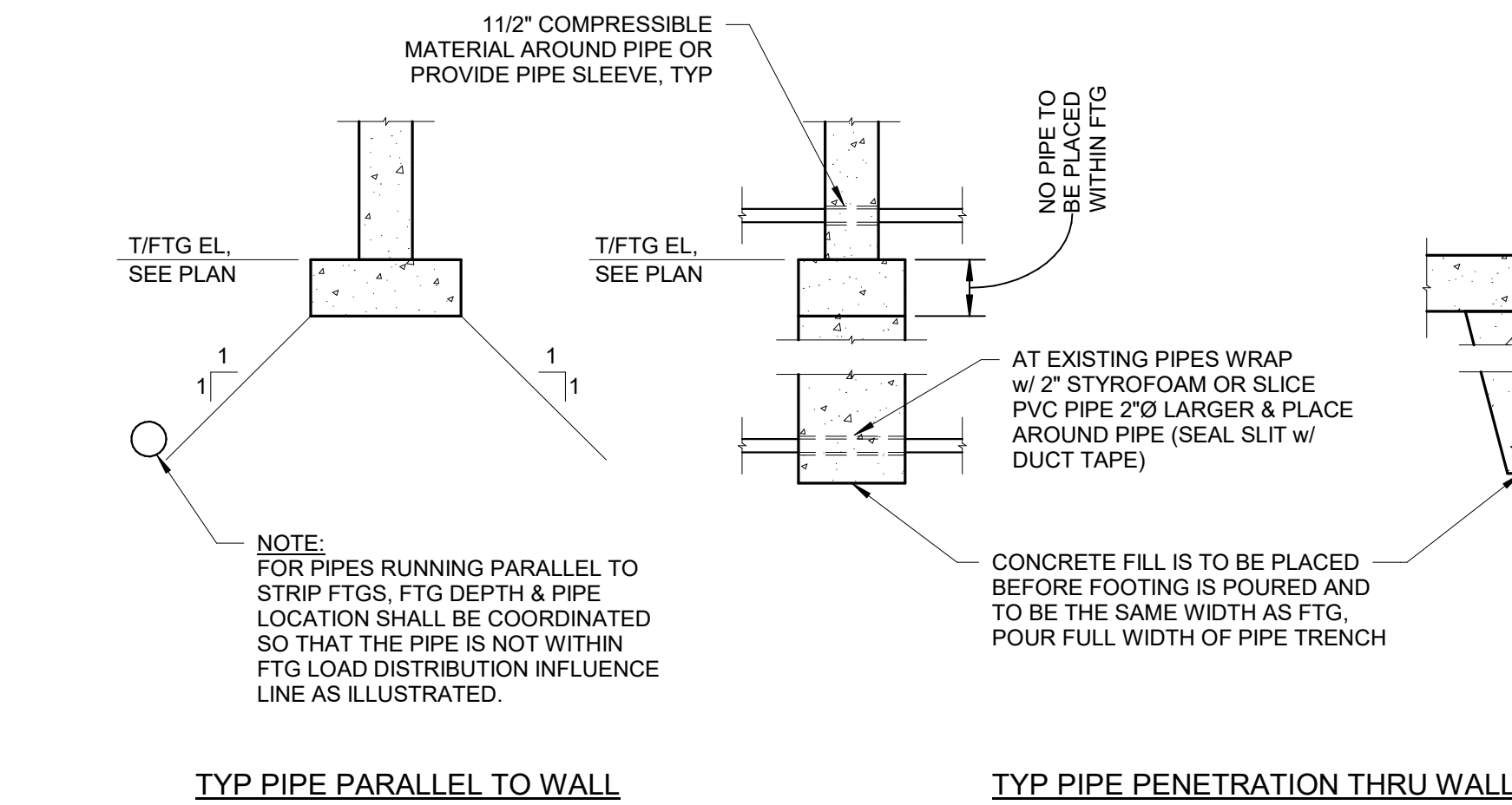
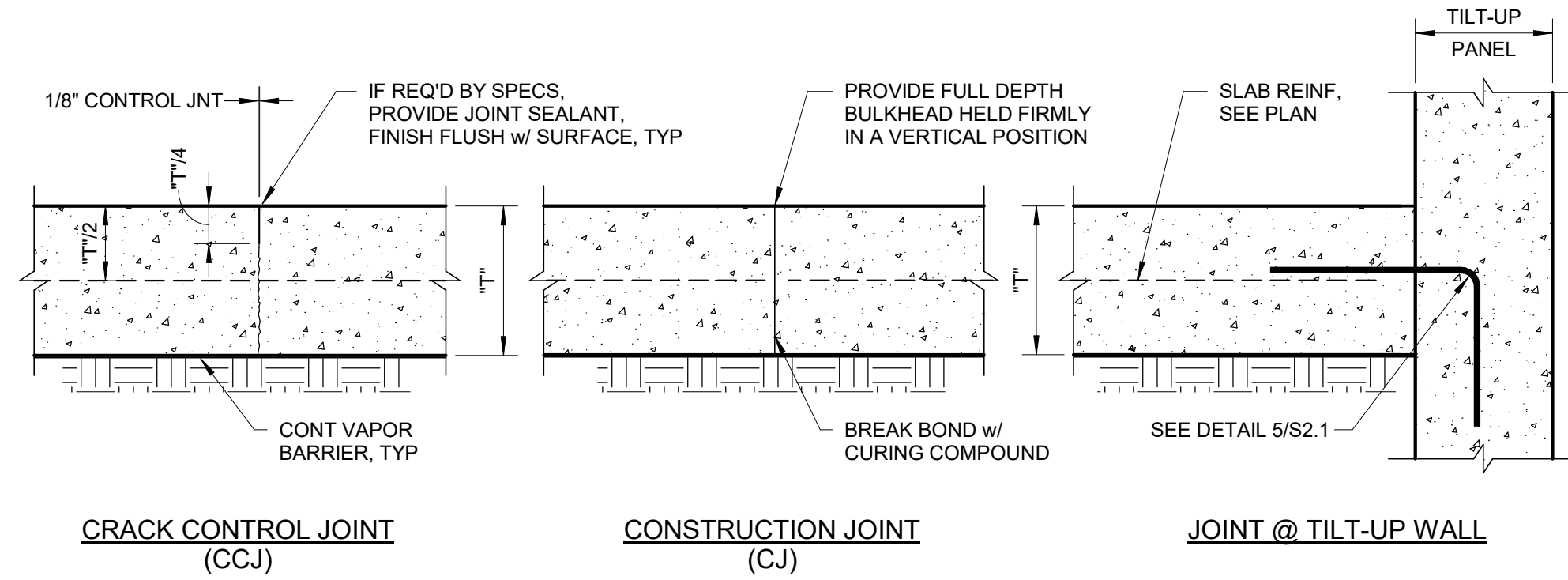
100%
CONSTRUCTION
DOCUMENTS



ROOF FRAMING PLAN NOTES:
1) SEE S1.2 FOR PLAN NOTES



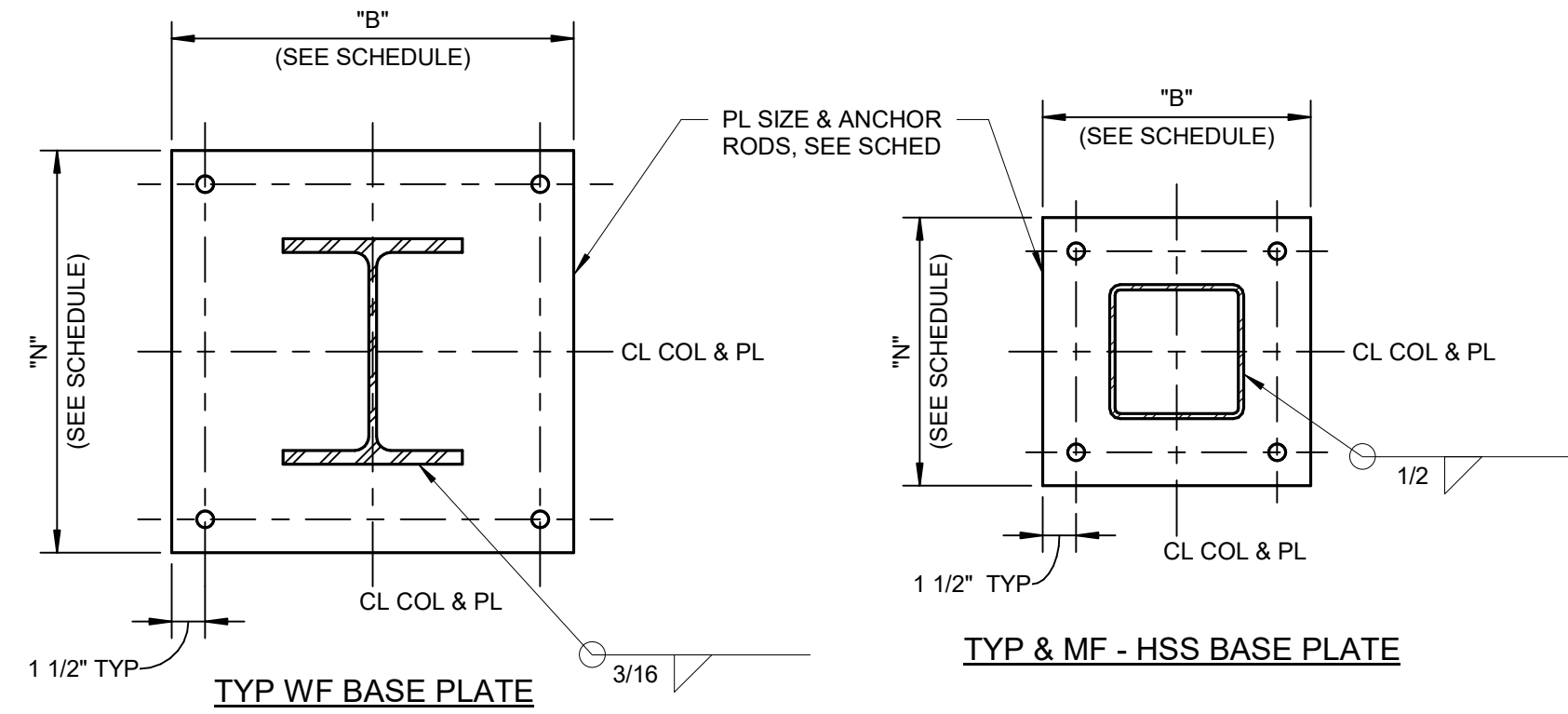
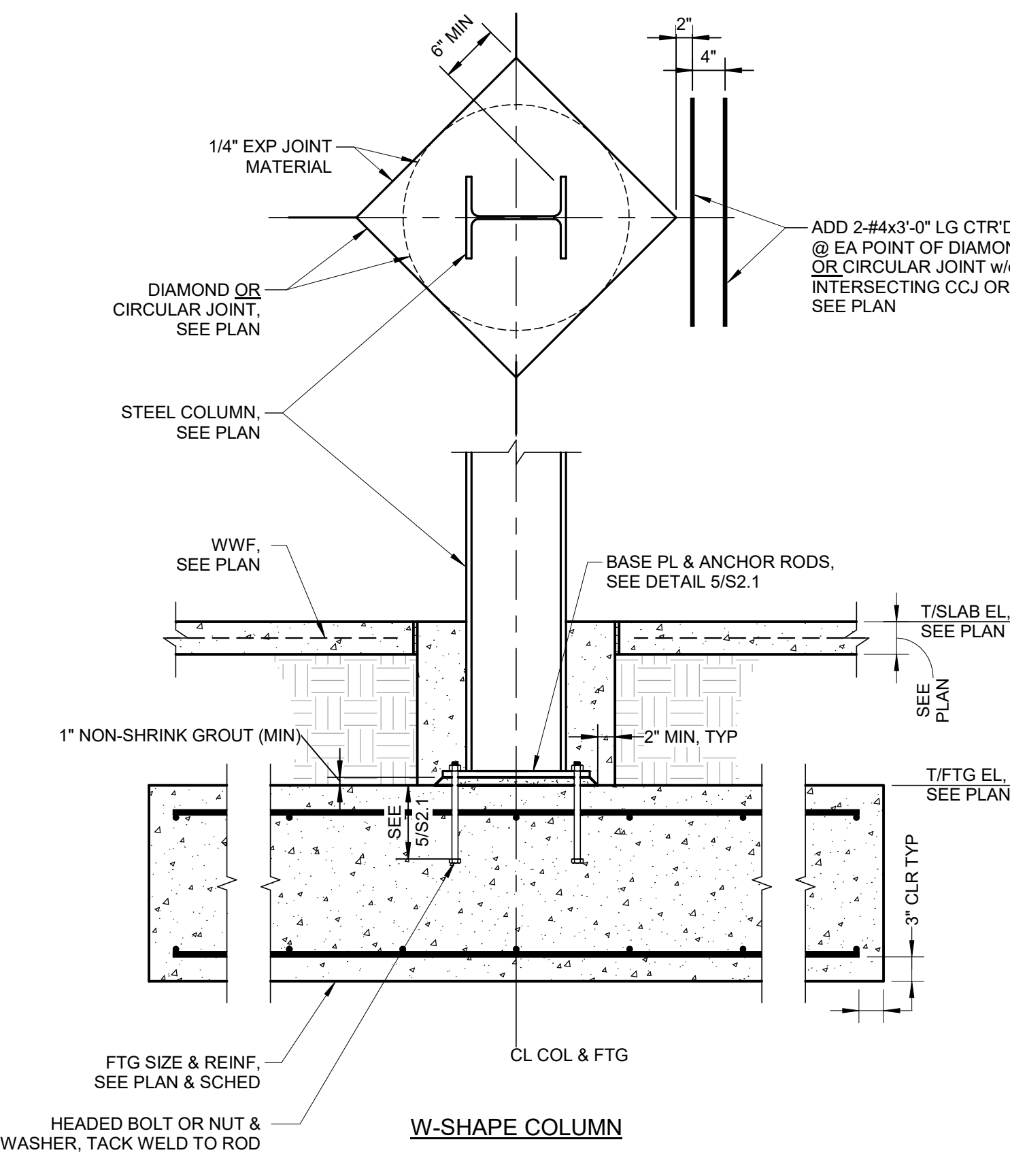
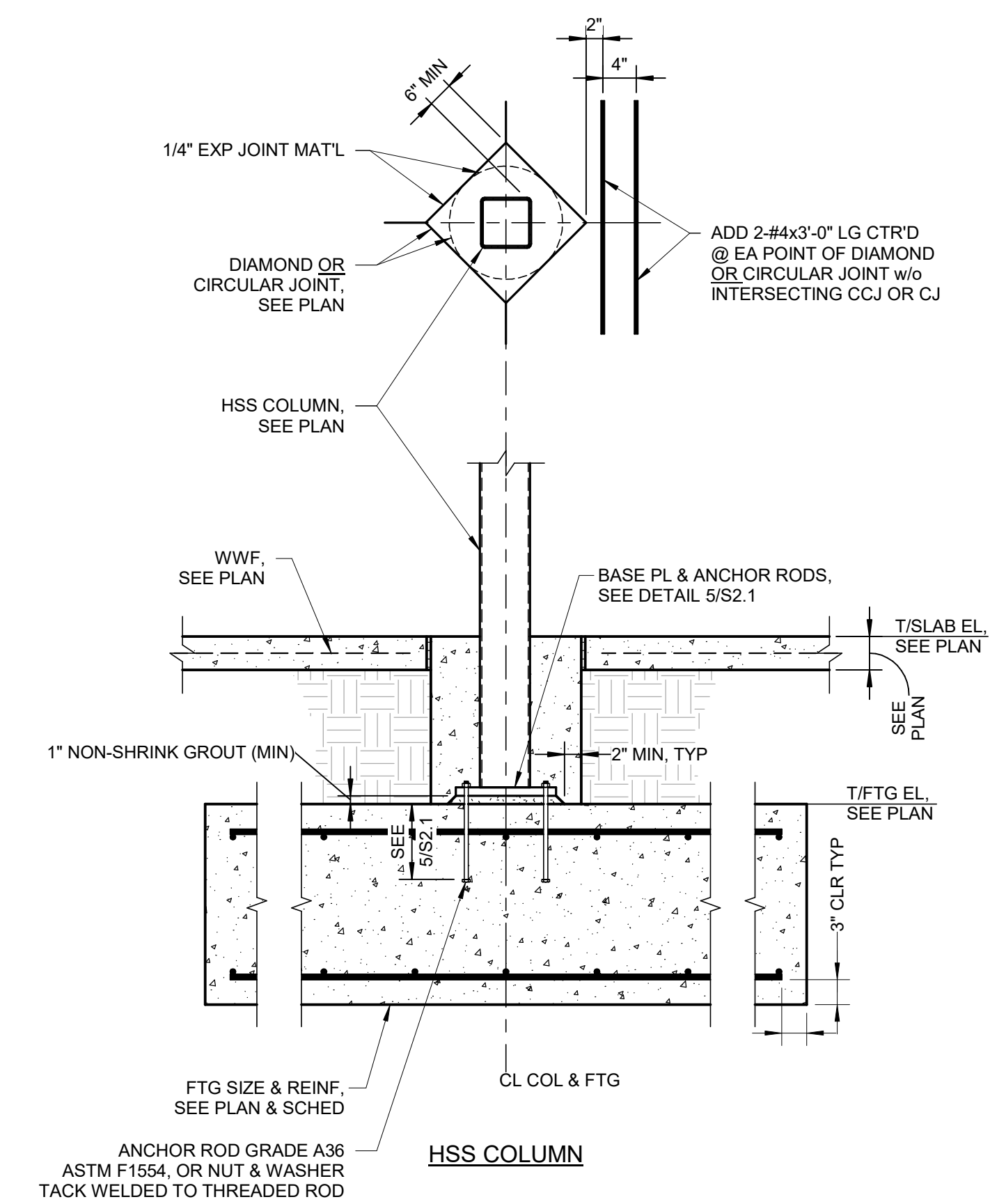




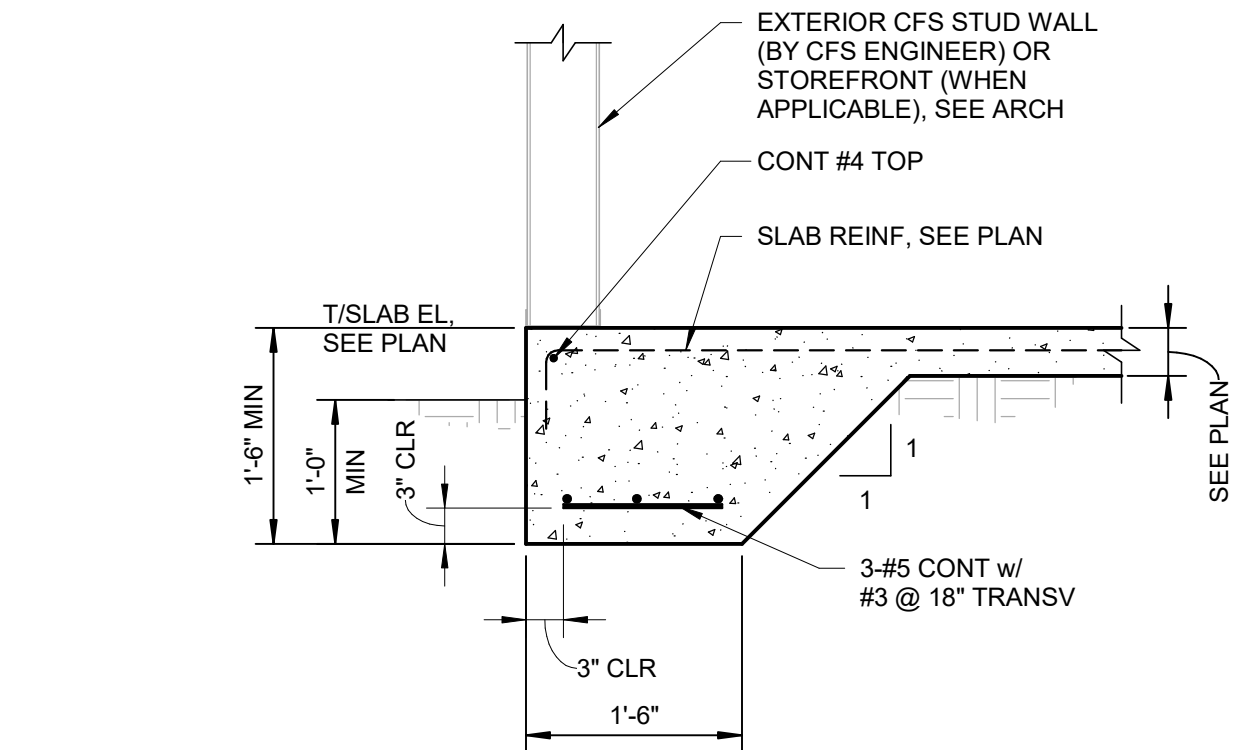
1 TYP SLAB ON GRADE JOINTS
SCALE: NTS

2 TYP PIPE PENETRATION THRU FOOTING
SCALE: NTS

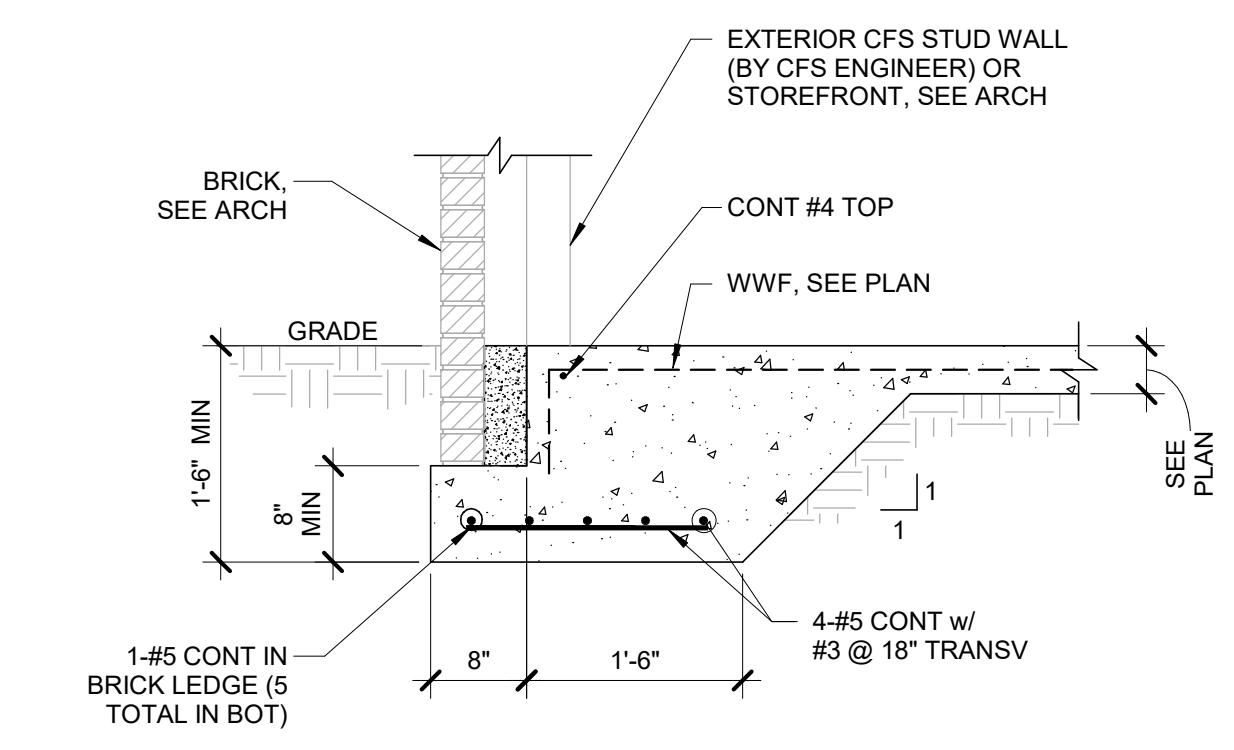
3 TYP CORNER FTG REINF
SCALE: NTS



BASE PLATE SCHEDULE			
COLUMN	PLATE (bXh)	ANCHOR RODS	EMBED (MIN)
WF	3/4"x18"x1'-6"	(4) 3/4"	9"
HSS	3/4"x12"x1'-0"	(4) 3/4"	9"



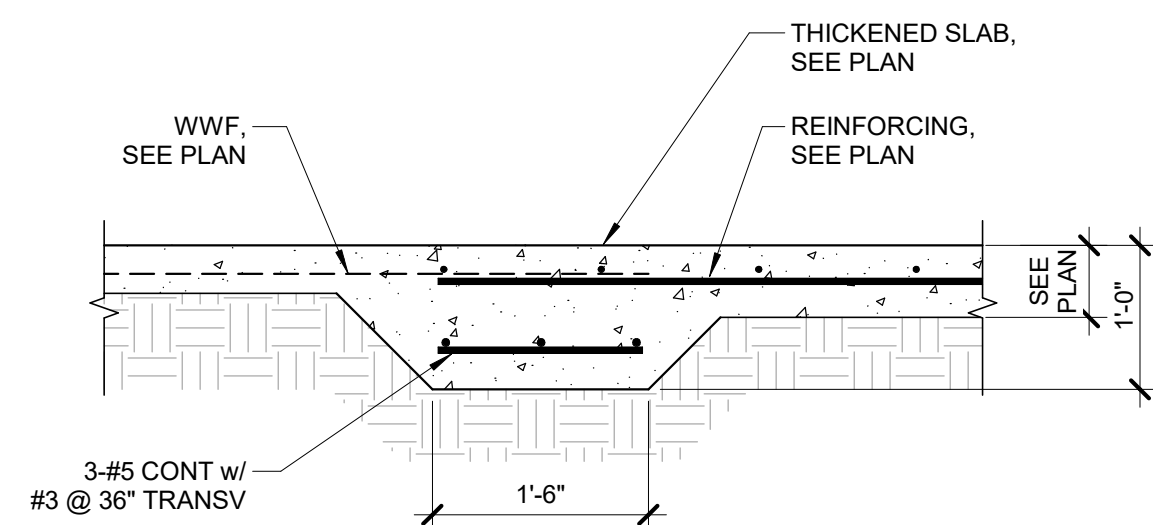
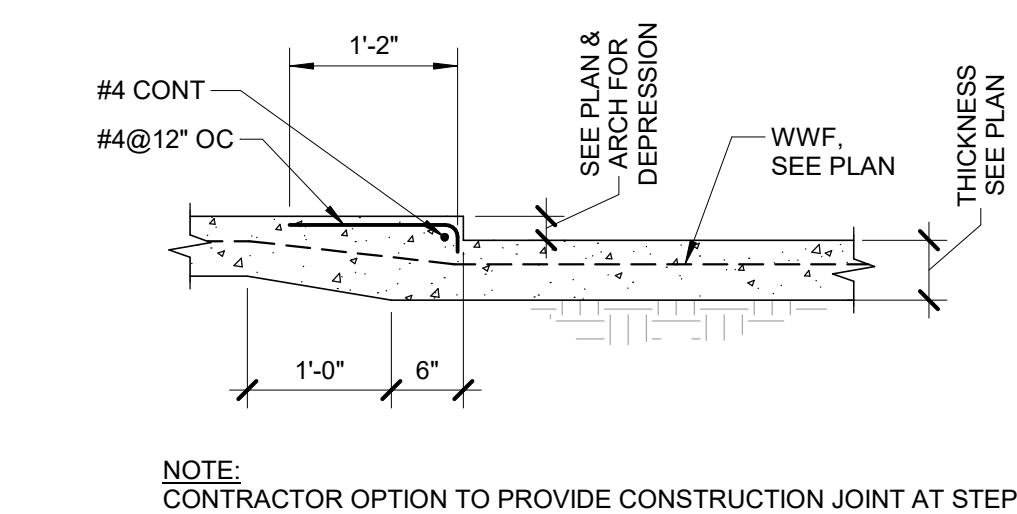
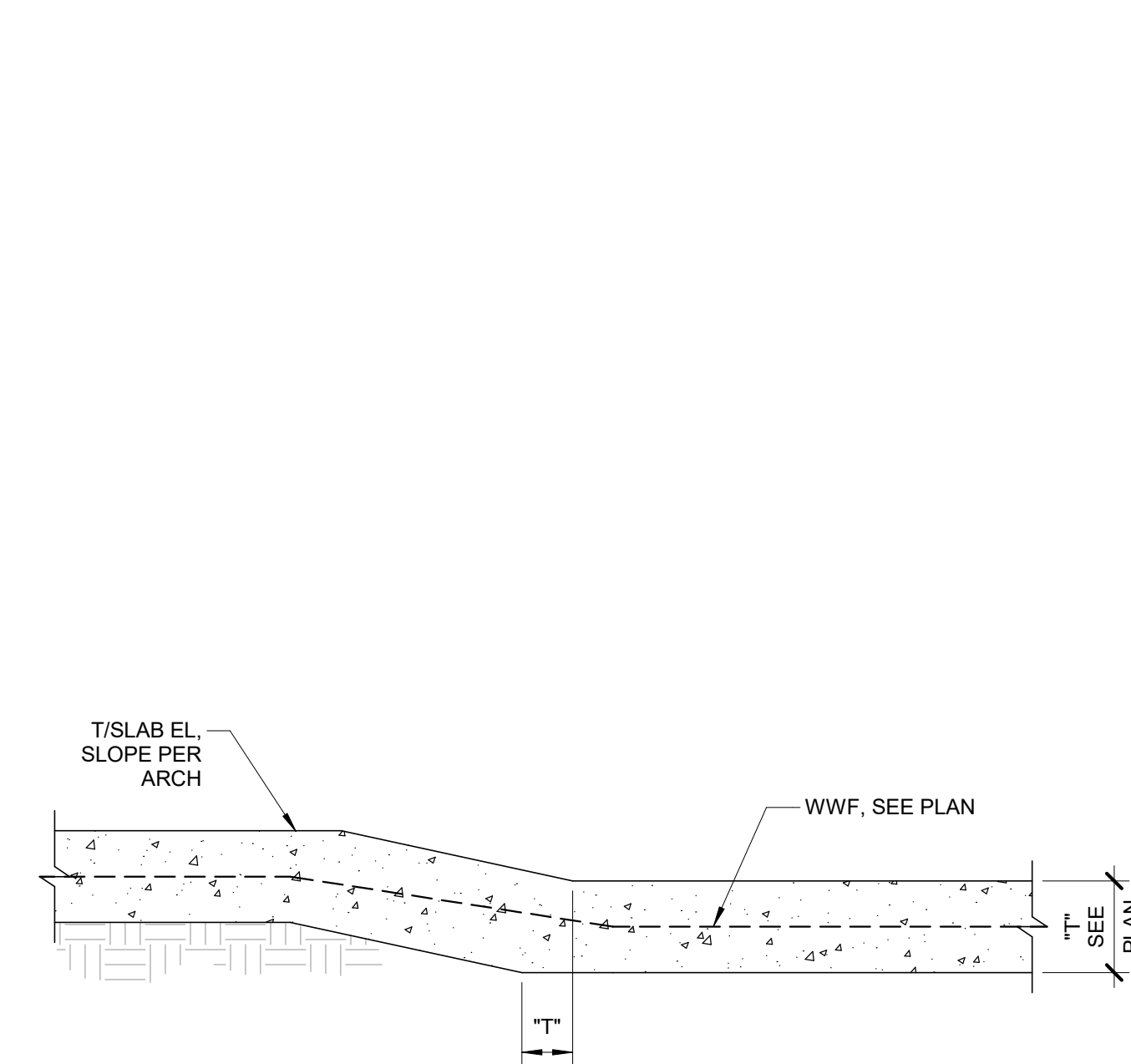
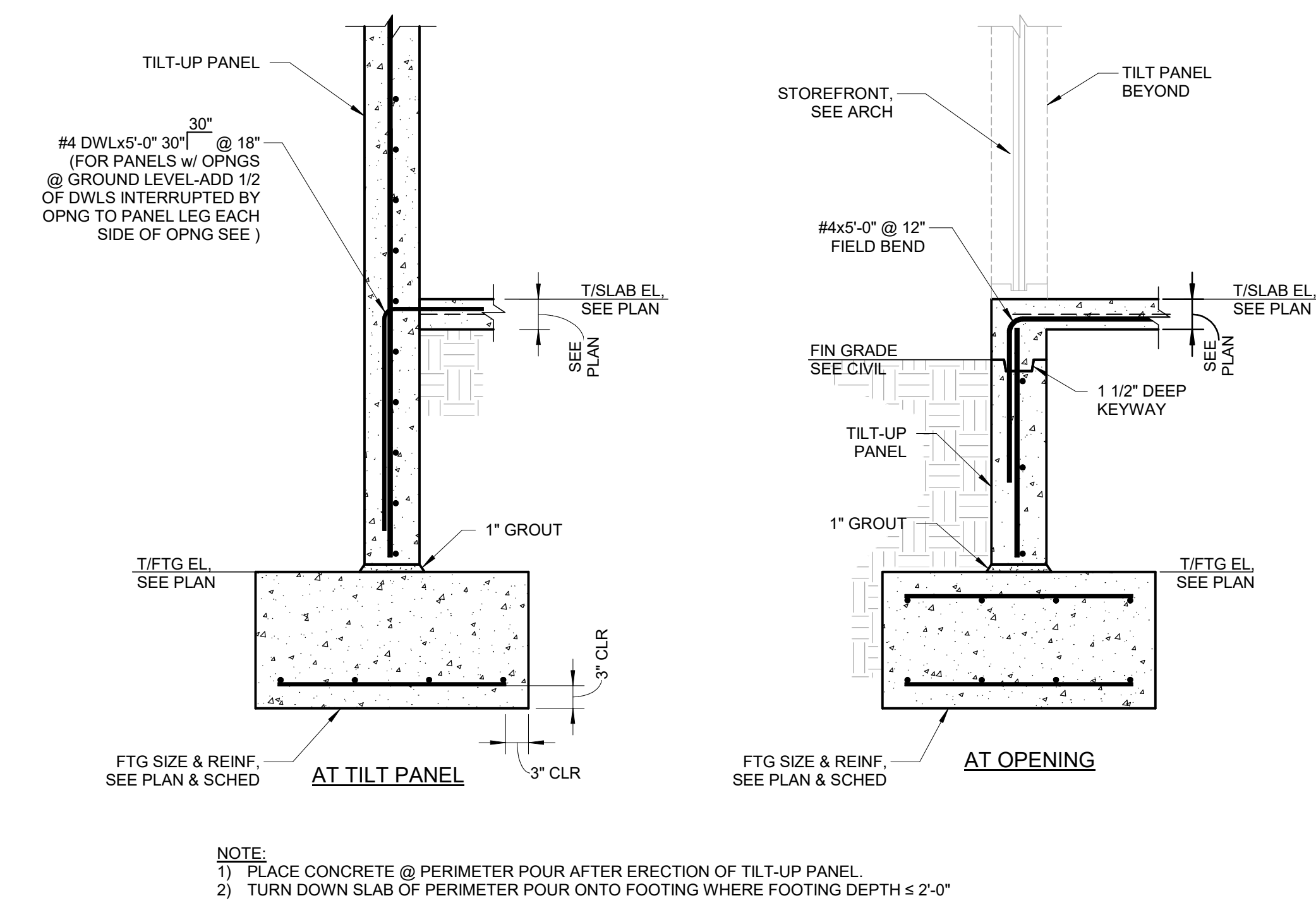
6 TYP TURNDOWN @ METAL STUD WALL
SCALE: NTS



7 TYP TURNDOWN W/ BRICK LEDGE
SCALE: NTS

4 TYPICAL COLUMN FOOTINGS
SCALE: NTS

5 COLUMN BASE PL
SCALE: NTS



8 PERIMETER TILT WALL FOUNDATION
SCALE: NTS

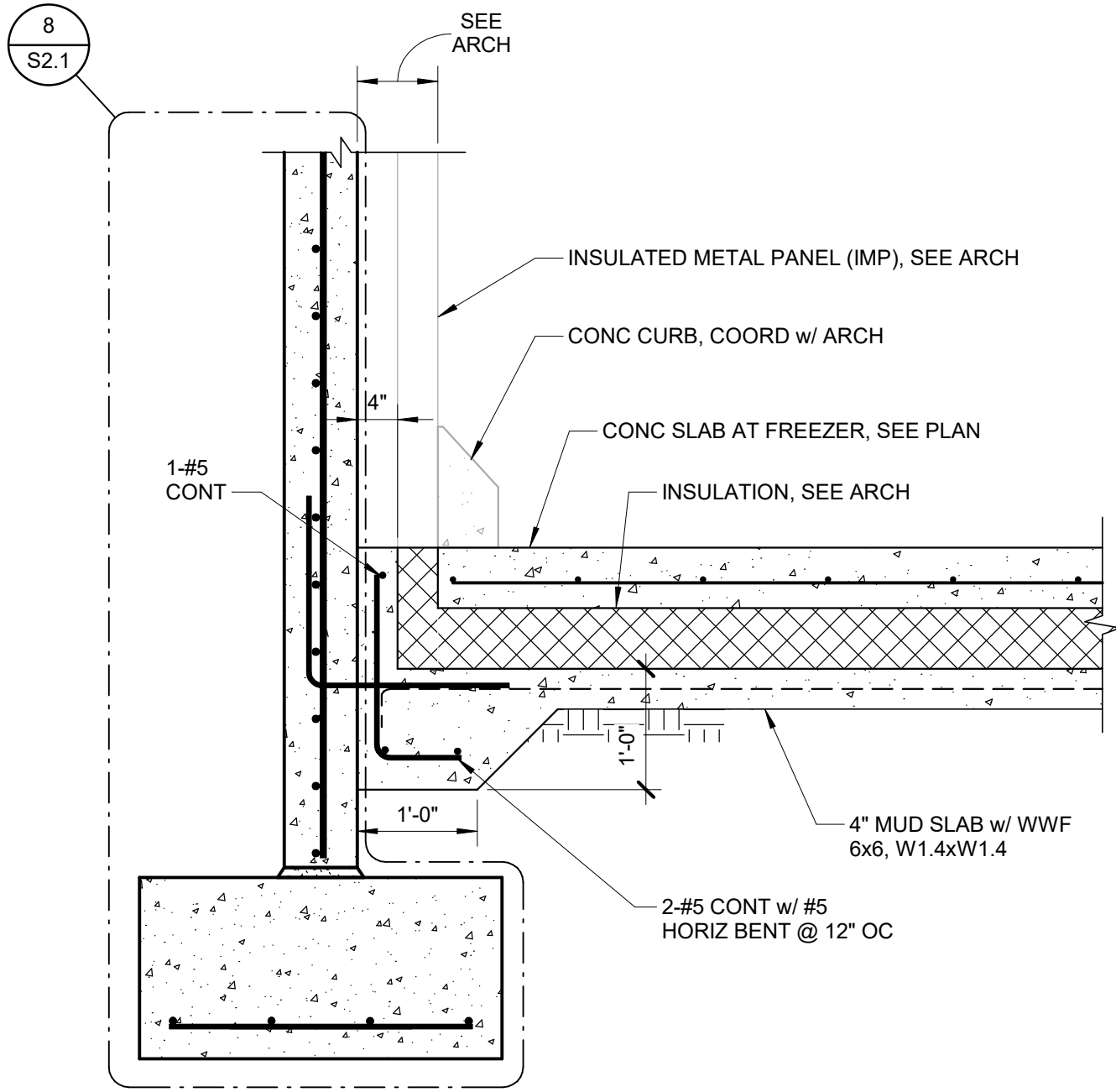
9 TYP SLOPED SLAB
SCALE: NTS

10 TYPICAL SLAB DEPRESSION
SCALE: NTS

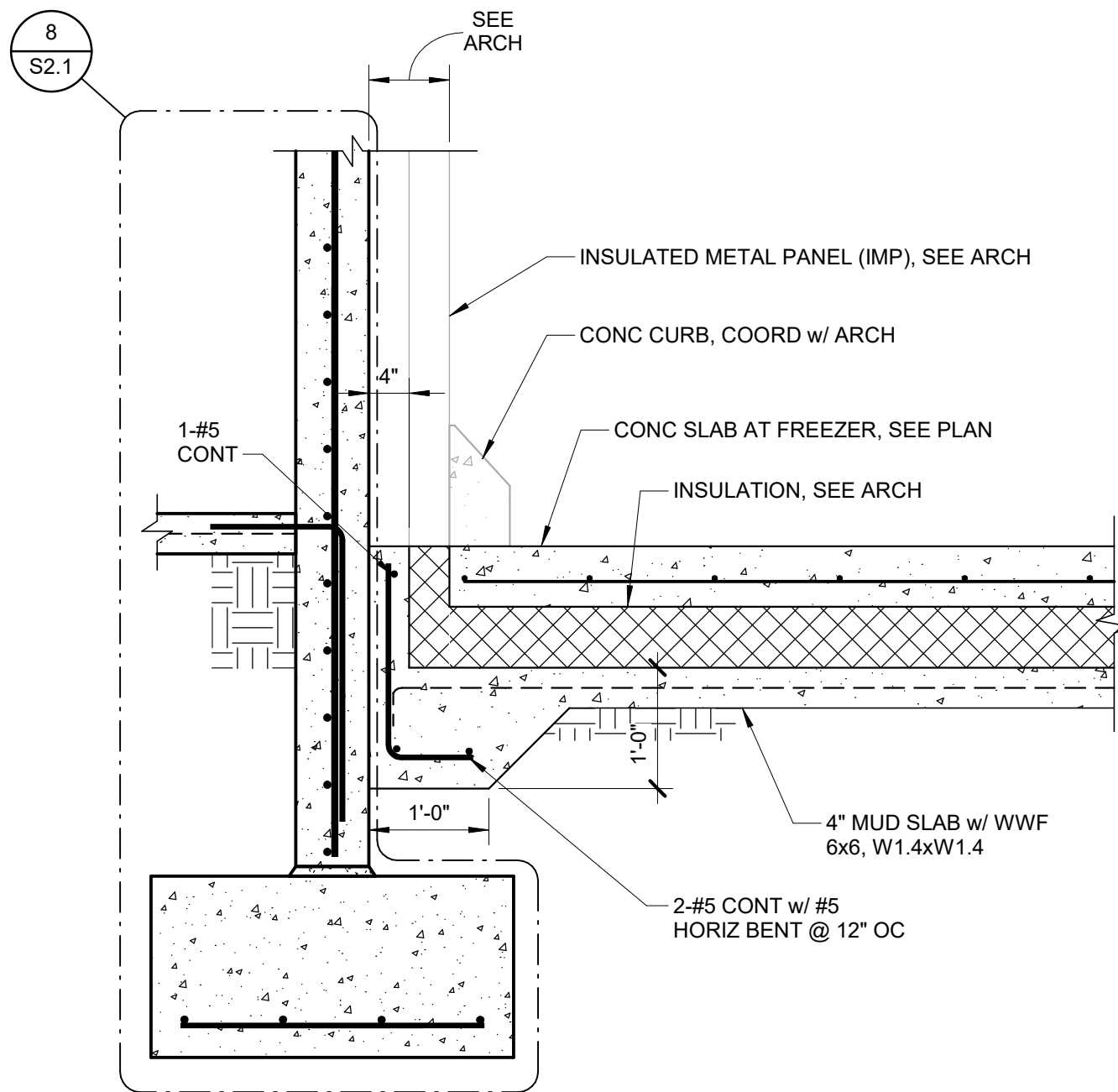
11 TRANSITION IN SLAB THICKNESS
SCALE: NTS

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.

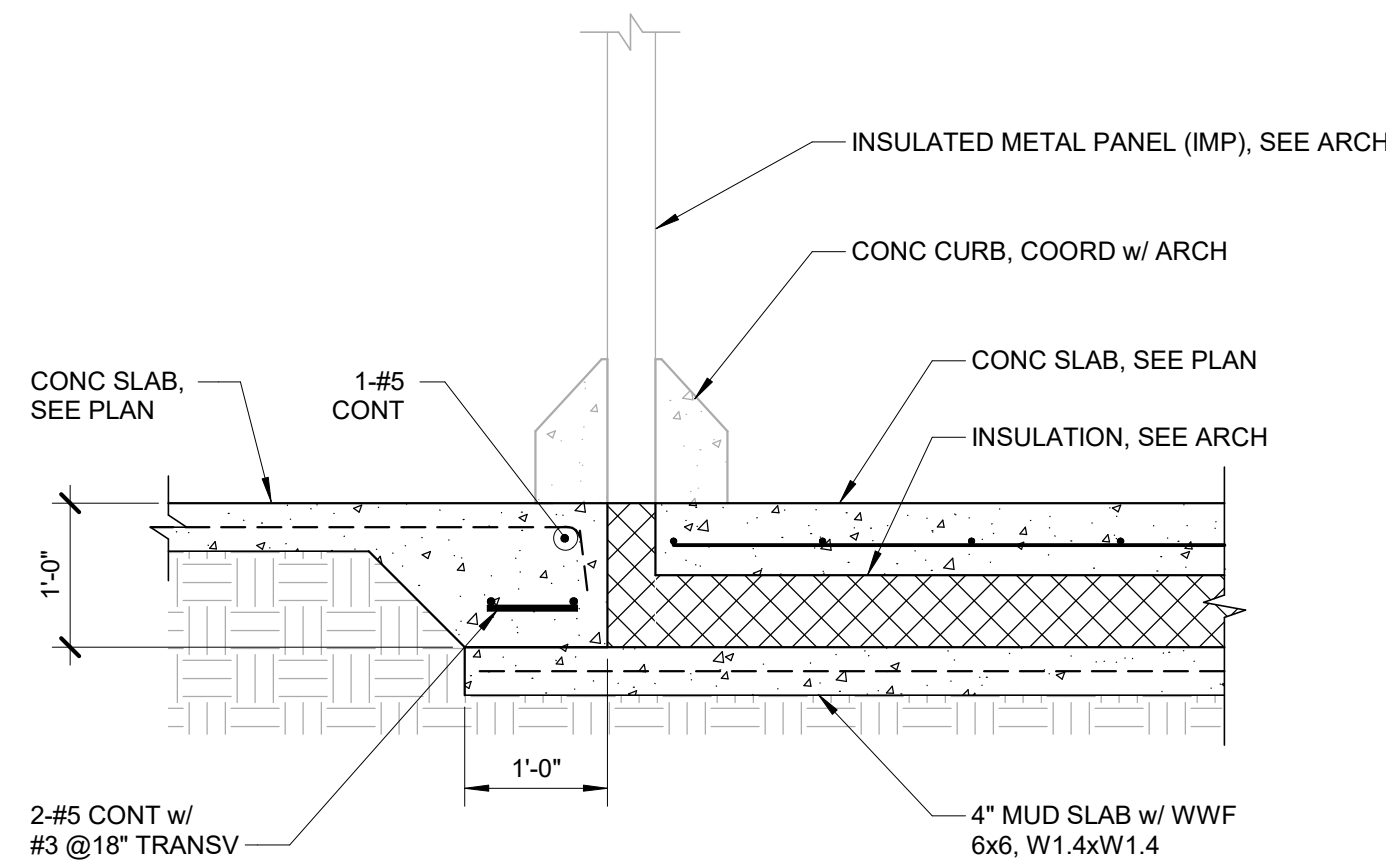
ALL RECORD SPECIFICATIONS, COMPUTER FILES, FIELD DATA AND OTHER DOCUMENTATION PREPARED BY KASPER ARCHITECTS & ASSOCIATES, INC. (KASPER) IS IN FULL COMPLIANCE WITH THE PROFESSIONAL ENGINEERING STATUTE OF THE STATE OF FLORIDA. NO PART OF THIS DOCUMENT SHALL BE REPRODUCED IN WHOLE OR IN PART WITHOUT THE EXPRESS WRITTEN PERMISSION OF KASPER.



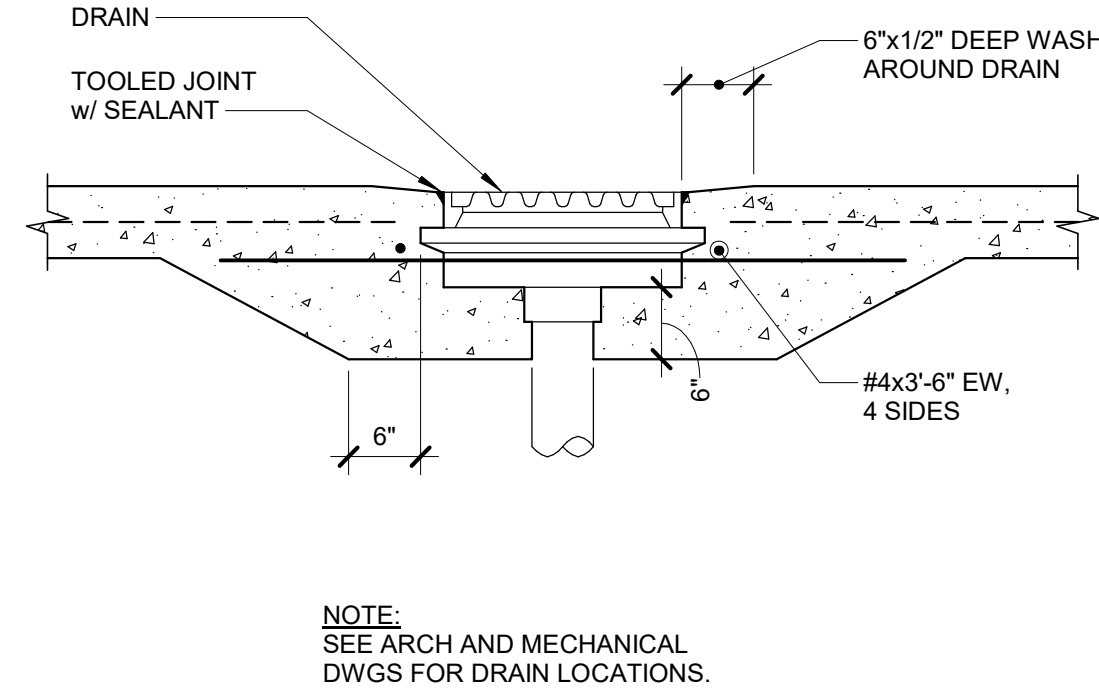
1 SECTION AT FREEZER WALL
SCALE: NTS



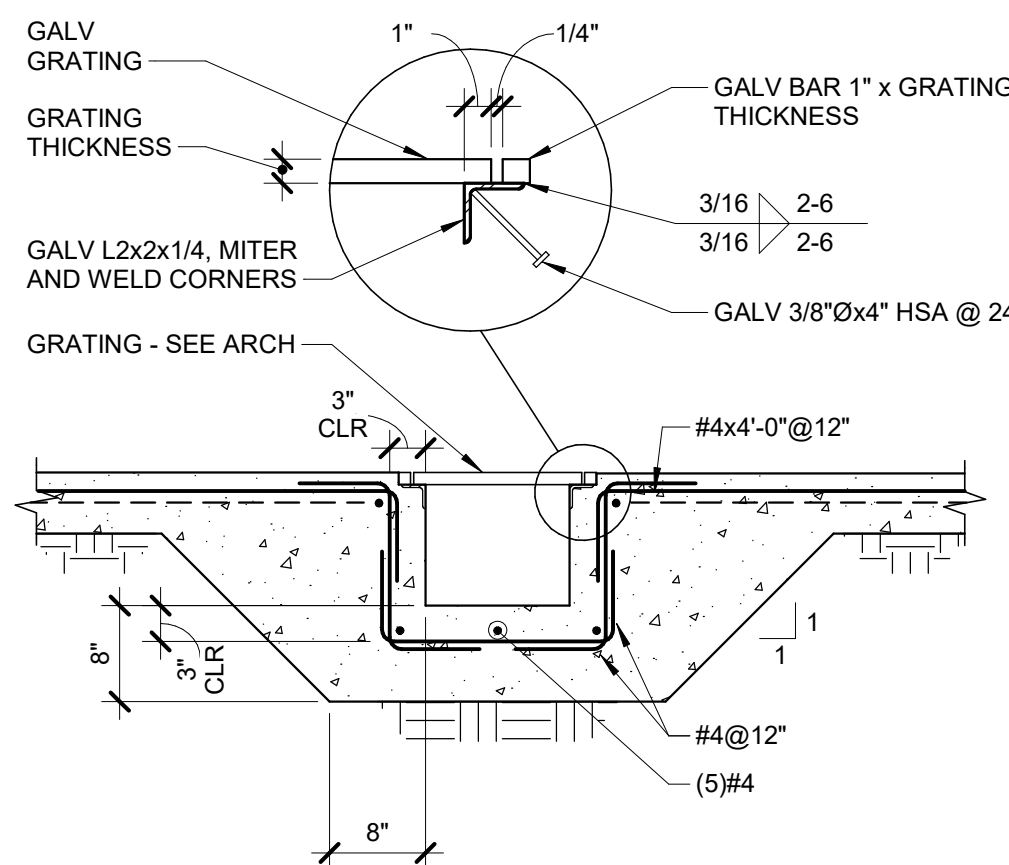
2 SECTION AT FREEZER EXPANSION JOINT
SCALE: NTS



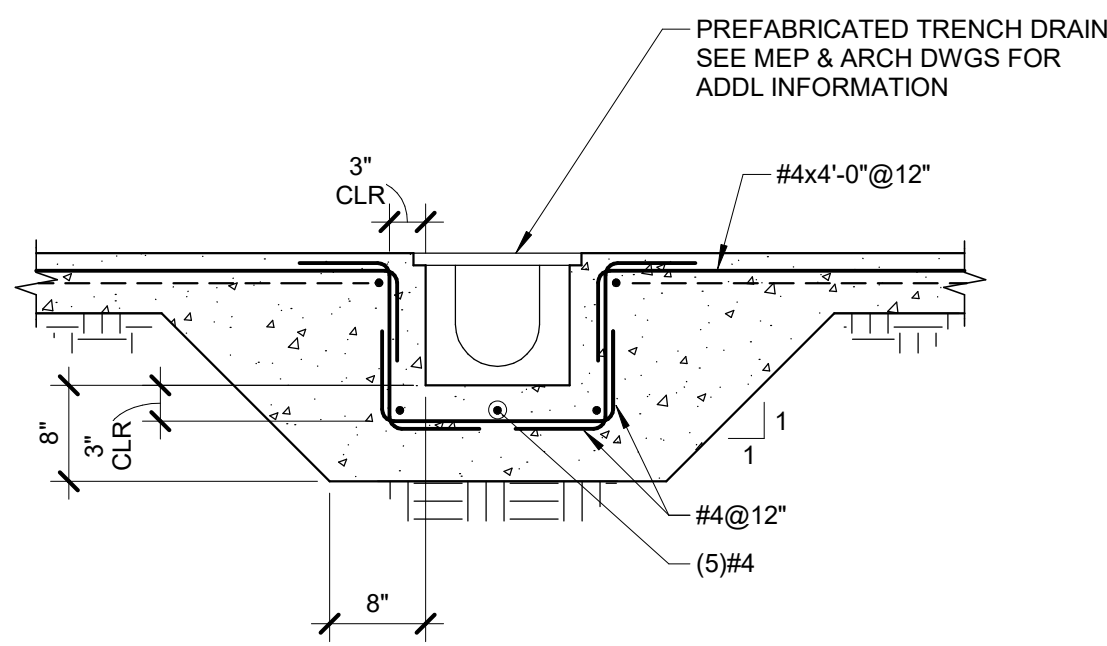
3 SECTION AT FREEZER EXPANSION JOINT
SCALE: NTS



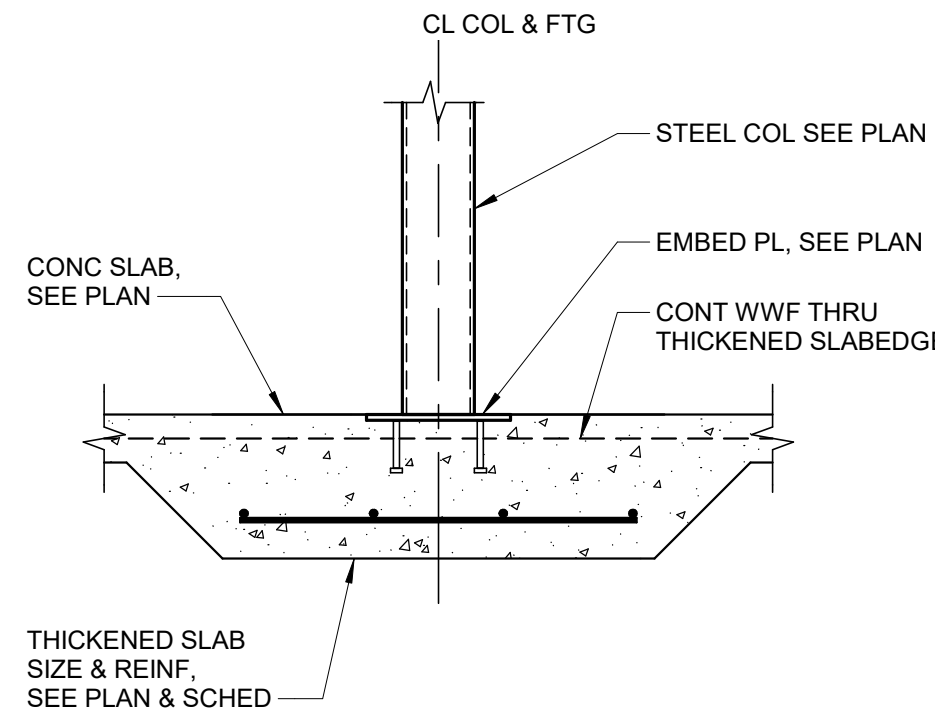
4 FLOOR DRAIN IN SLAB-ON-GRADE
SCALE: NTS



5 CIP TRENCH DRAIN
SCALE: NTS



6 PREFAB TRENCH DRAIN
SCALE: NTS



7 THICKENED SLAB FTG AT COLUMN
SCALE: NTS

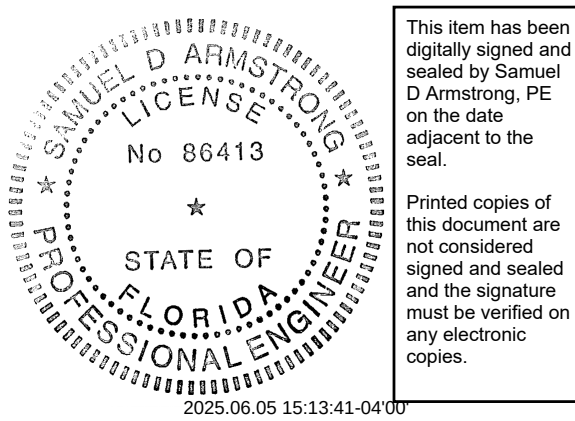


10175 Fortune Parkway, Suite 701
Jacksonville, FL 32256
p 904.483.9201

www.kasperarch.com
FL License No.: AA2600802



9133 R.C. SKINNER PARKWAY
SUITE 1
JACKSONVILLE, FL 32256
P: 904.483.5200
PROJECT #23000902.00
IMEG RESERVES PROPRIETARY RIGHTS, INCLUDING COPYRIGHTS TO THIS DRAWING AND THE DATA SHOWN THEREON. SAID DRAWINGS AND/OR DATA ARE THE EXCLUSIVE PROPERTY OF IMEG AND SHALL NOT BE USED OR REPRODUCED FOR ANY OTHER PROJECT WITHOUT THE EXPRESS WRITTEN APPROVAL AND PARTICIPATION OF IMEG. ©2025 IMEG CONSULTANTS CORP.



ERIK C. KASPER AR#0017162

BUSY BEE
ELLISVILLE

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

PRIOR ISSUES:

FOUNDATION
SECTIONS &
DETAILS

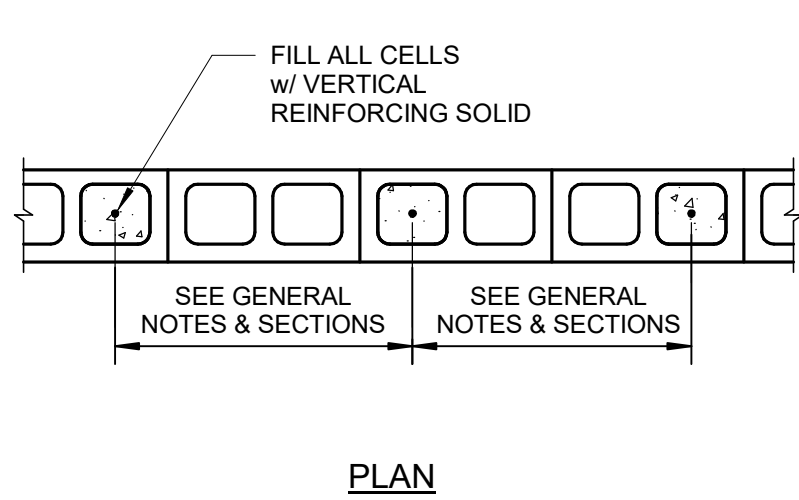
S2.2

DRAWN BY: ES
CHECKED BY: CF

100%
CONSTRUCTION
DOCUMENTS

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.

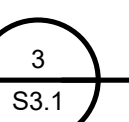
© 2023



1
S3.1



2
S3.1



3
S3.1

CM
SCALE

(PA
SCALE: 1

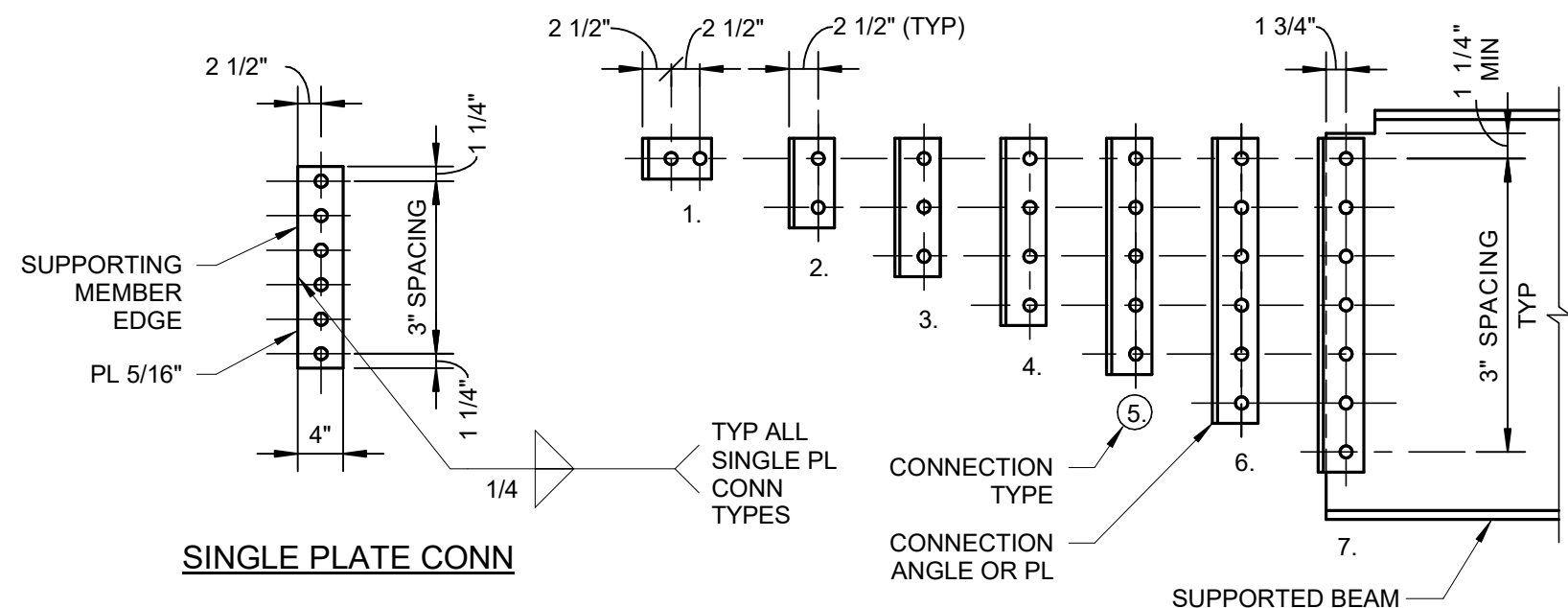
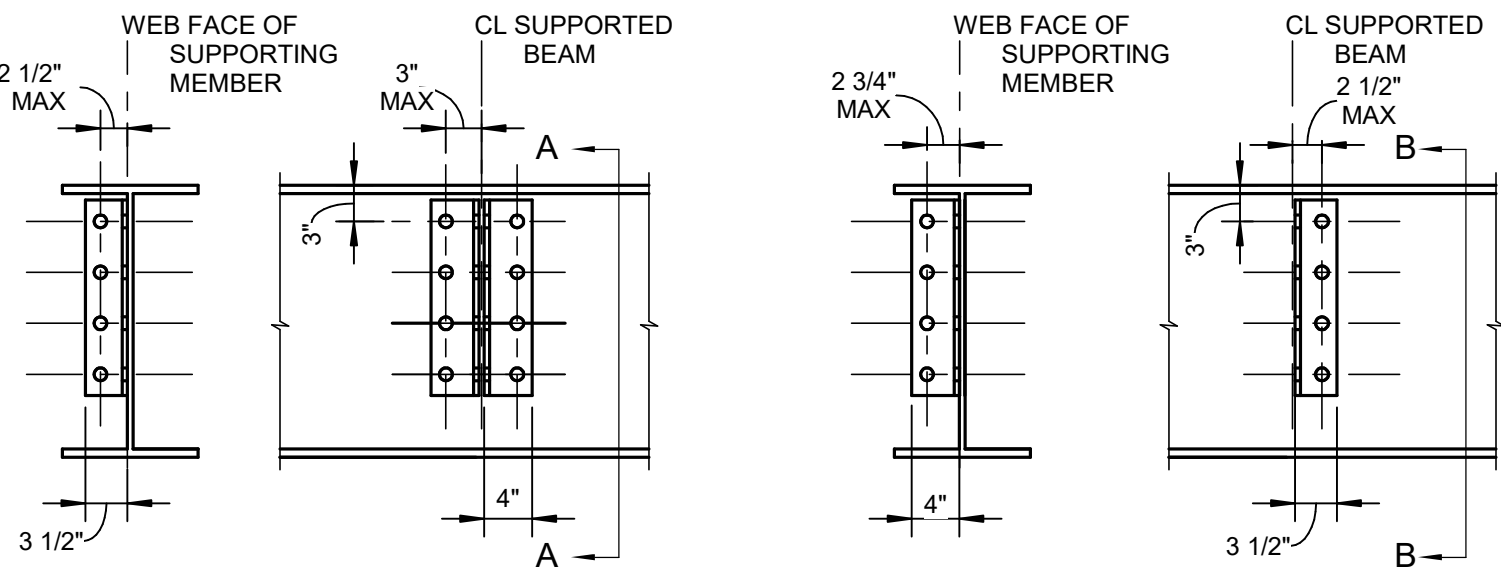


(PE
SCALE:

DRAWN BY:	ES
CHECKED BY:	CF

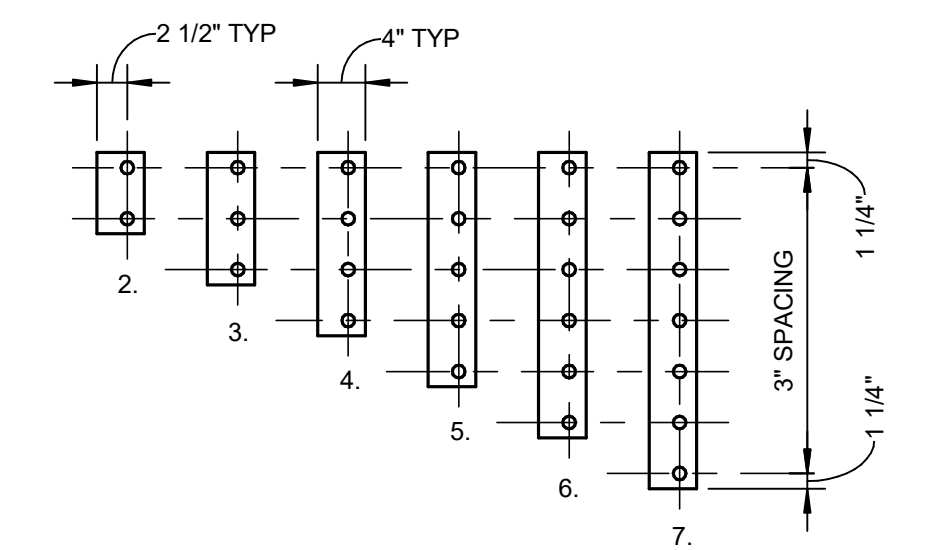
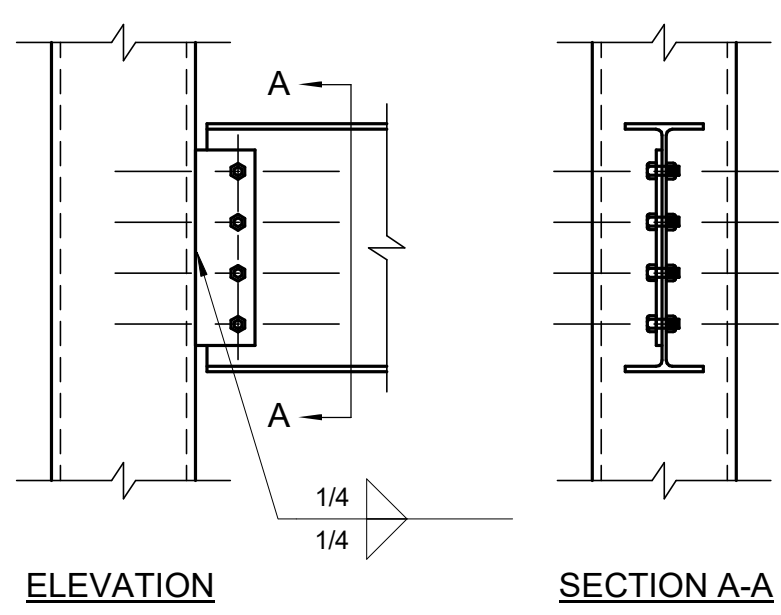
100%
CONSTRUCTION
DOCUMENTS

ALL REPORT, SPECIFICATIONS, COMPUTER FILES, FIELD DATA AND OTHER DOCUMENTATION PREPARED BY KASPER ARCHITECTURAL & DEVELOPMENT, INC. (KAD) IS INSTRUMENTS OF SERVICE SHALL REMAIN THE PROPERTY OF KAD. NO PART SHALL BE REPRODUCED IN WHOLE OR IN PART WITHOUT THE EXPRESS WRITTEN PERMISSION OF KAD.



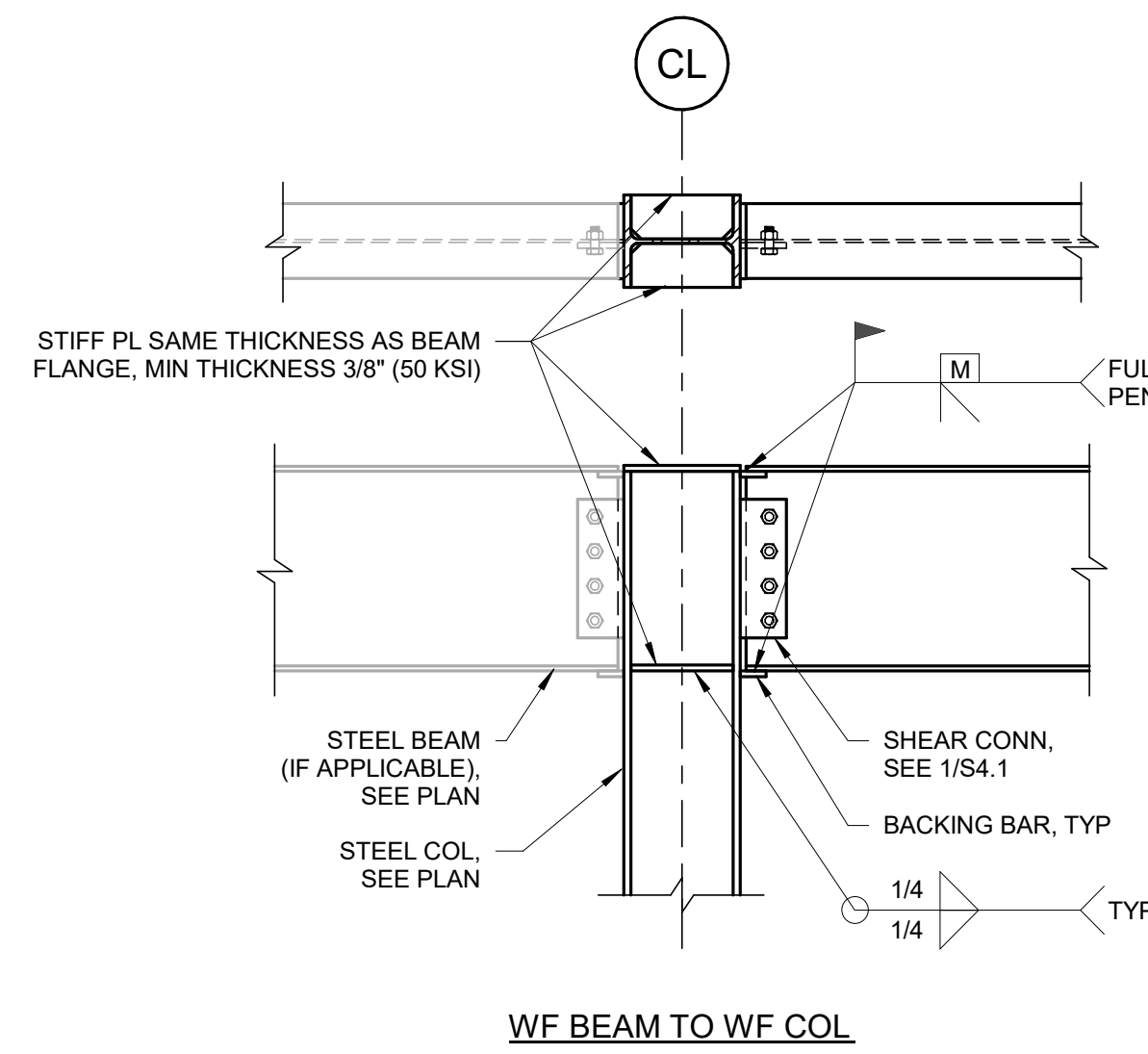
CONNECTION TABLE							
CONN TYPE	DOUBLE & SINGLE ANGLE CONNECTION		SINGLE PLATE CONNECTION		FACTORED LOAD CAPACITY		MINIMUM SUPPORTED MEMBER SIZE
	SIZE	LENGTH	SIZE	LENGTH	DOUBLE ANGLE	SINGLE ANGLE	
7	L4x3 1/2	1' - 8 1/2"	PL 5/16"x4"	1' - 8 1/2"	217k	112k	108k
6	L4x3 1/2	1' - 5 1/2"	PL 5/16"x4"	1' - 5 1/2"	176k	93k	89k
5	L4x3 1/2	1' - 2 1/2"	PL 5/16"x4"	1' - 2 1/2"	112k	74k	81k
4	L4x3 1/2	11 1/2"	PL 5/16"x4"	11 1/2"	64k	55k	62k
3	L4x3 1/2	8 1/2"	PL 5/16"x4"	8 1/2"	39k	35k	39k
2	L4x3 1/2	5 1/2"	PL 5/16"x4"	5 1/2"	23k	18k	23k
1	L6x4x3/8	2 1/2"	-	-	16k	-	-

- NOTES:
- 1) CONNECTION BOLTS SHALL BE 3/4"Ø A325-N BEARING TYPE. BOLT LIMIT STATE IN SINGLE SHEAR = 17.9 KIPS.
 - 2) CONNECTION ANGLE THICKNESS: 5/16" FOR DOUBLE ANGLE CONNECTIONS TYPE 2 THRU 7. 3/8" FOR SINGLE ANGLE CONNECTIONS TYPE 2 THRU 7.
 - 3) SINGLE PLATE CONNECTIONS ARE BASED ON FLEXIBLE CONNECTIONS WITH STANDARD HOLES.
 - 4) ANY CONNECTION THAT IS OUTSIDE THE LIMITS OF THE CONNECTION TABLE SHALL BE DESIGNED BY THE SPECIALTY ENGINEER LICENSED IN THE STATE OF PROJECT AND SHALL BE SUBMITTED AS SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR APPROVAL.
 - 5) ALL CONNECTIONS WITH AXIAL LOADS SHOWN ON PLAN SHALL BE DESIGNED BY THE SPECIALTY ENGINEER LICENSED IN THE STATE OF THE PROJECT AND SHALL BE SUBMITTED AS SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR APPROVAL.
 - 6) BOLTED/WELDED SINGLE AND DOUBLE ANGLE CONNECTIONS MAY BE USED PROVIDED THAT THE WELDS ARE SIZED TO RESIST THE LOADS SHOWN IN ACCORDANCE WITH AISC STANDARDS.

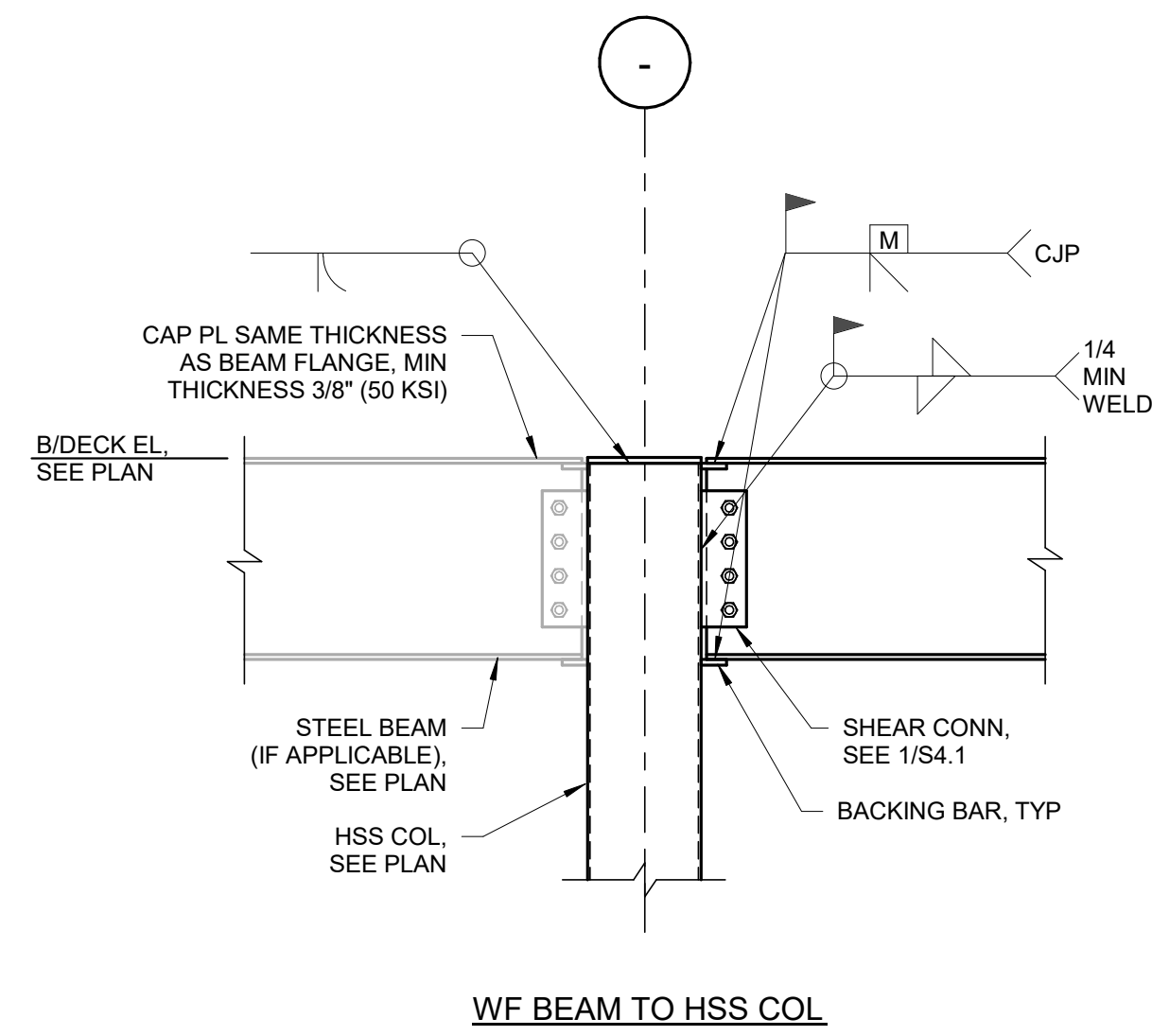


CONN TYPE	FACTORED LOAD CAPACITY	SINGLE PLATE SIZE	MINIMUM BEAM SIZE
7	108K	PL 5/16"x4"x20 1/2"	W36, W33
6	89K	PL 5/16"x4"x17 1/2"	W30, W27
5	81K	PL 5/16"x4"x14 1/2"	W24, W21
4	62K	PL 5/16"x4"x11 1/2"	W18, W16
3	43K	PL 5/16"x4"x8 1/2"	W14, C15, W12, C12
2	24K	PL 5/16"x4"x5 1/2"	W10, C10, W8, C8

- NOTES:
- 1) BOLTS SHALL BE 3/4"Ø A325-N BEARING TYPE. BOLT LIMIT STATE IN SINGLE SHEAR = 17.9 KIPS.
 - 2) WELDING MAY BE USED IN LIEU OF BOLTING. WELDS SHALL BE CAPABLE OF RESISTING LOADS SHOWN ABOVE FOR THIS SCHEDULE TO BE VALID.
 - 3) COLUMN THICKNESS SHALL BE THE MINIMUM TABULATED ABOVE FOR THIS SCHEDULE TO BE VALID.
 - 4) TABLE VALUES ARE FOR UNCOUPED BEAMS.

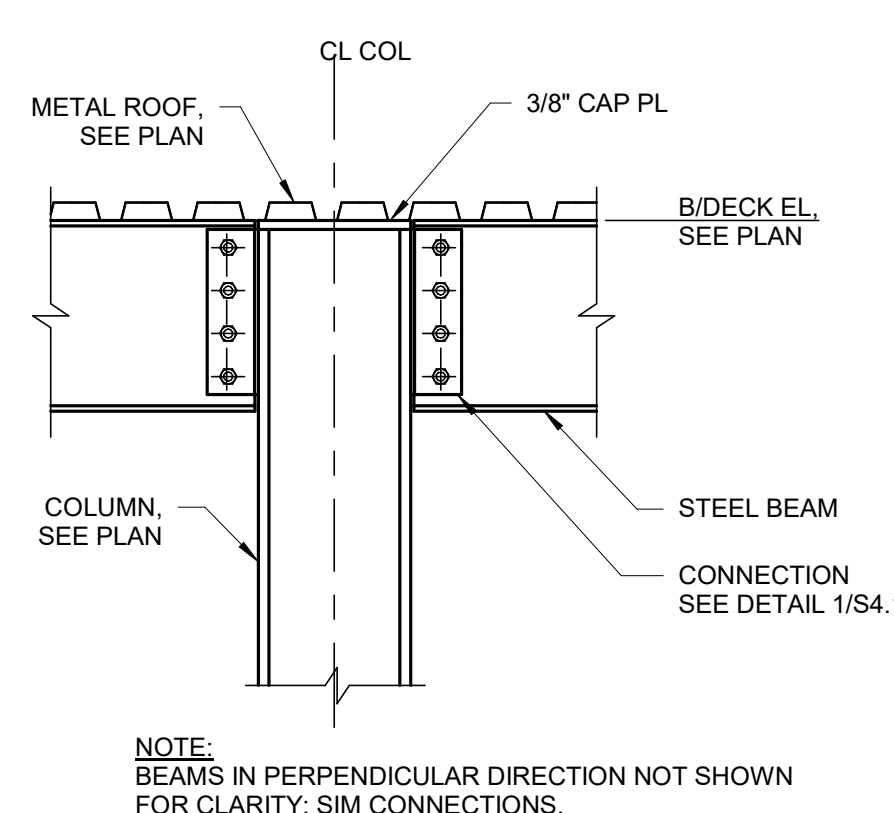
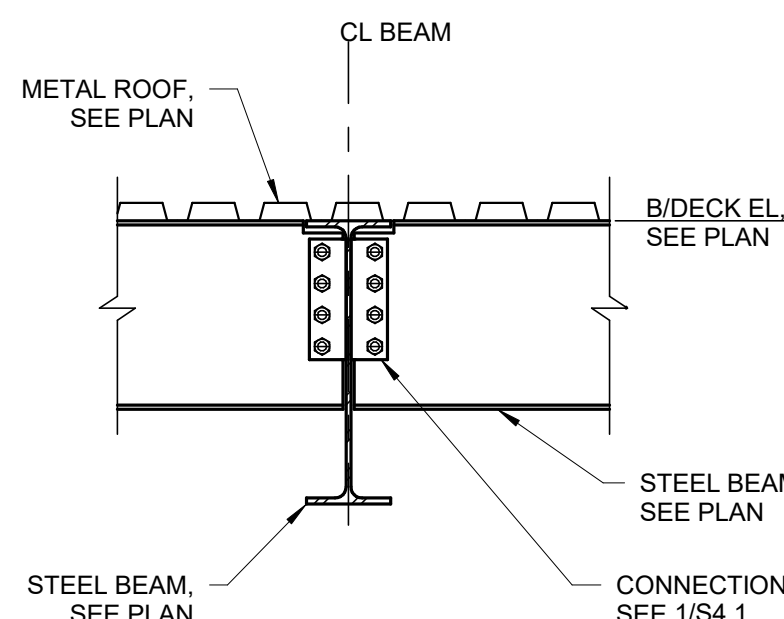


NOTE: MOMENT CONNECTIONS SHOWN ARE FULL PENETRATION WELDS. IF ALTERNATE MOMENT CONNECTION DETAILS ARE DESIRED THEY SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT AND SHALL BE SUBMITTED FOR REVIEW AS SIGNED/SEALED SHOP DRAWINGS & CALCULATIONS.



3 TYP BEAM/COLUMN MOMENT CONN

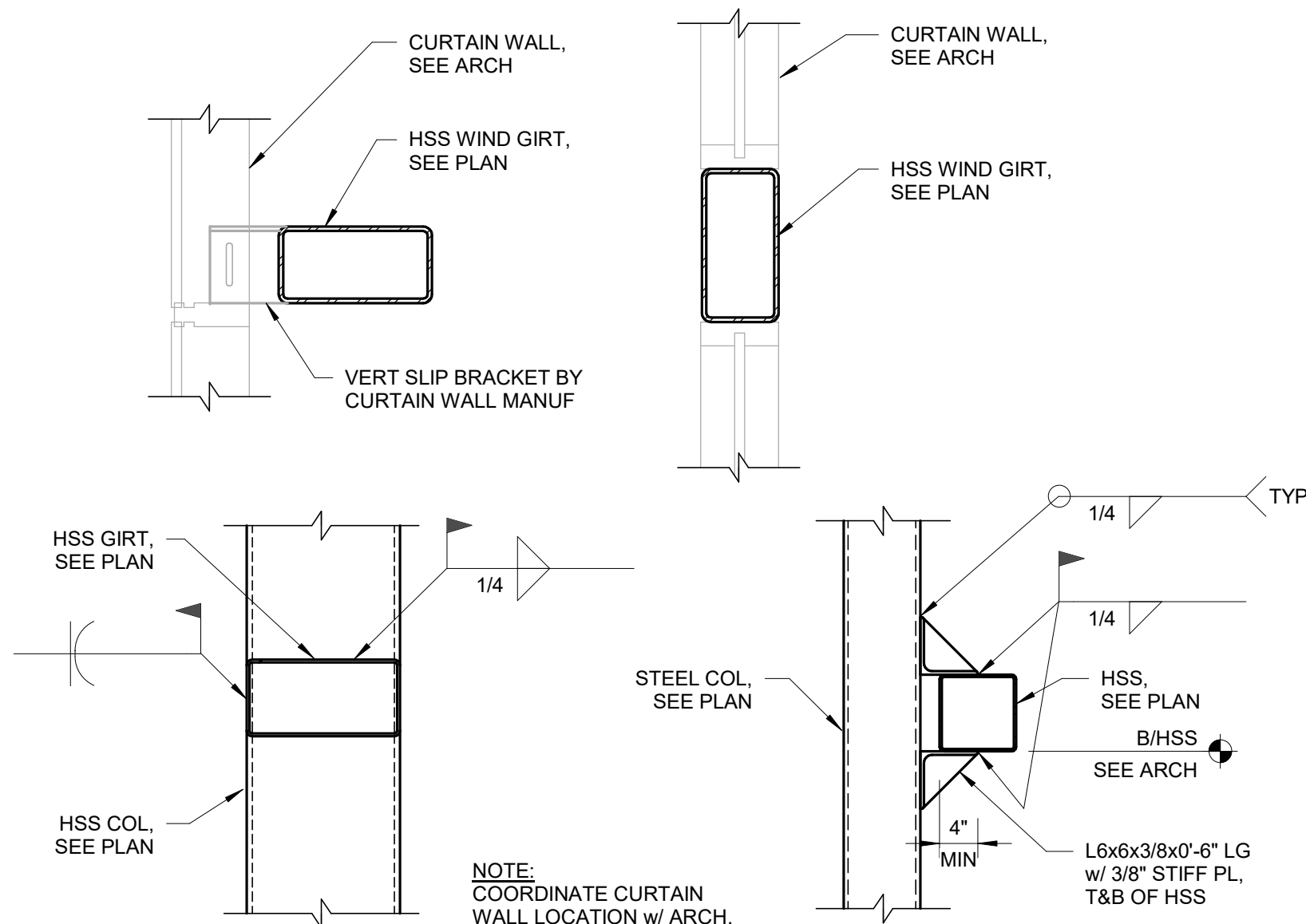
SCALE: NTS



NOTE: BEAMS IN PERPENDICULAR DIRECTION NOT SHOWN FOR CLARITY; SIM CONNECTIONS.

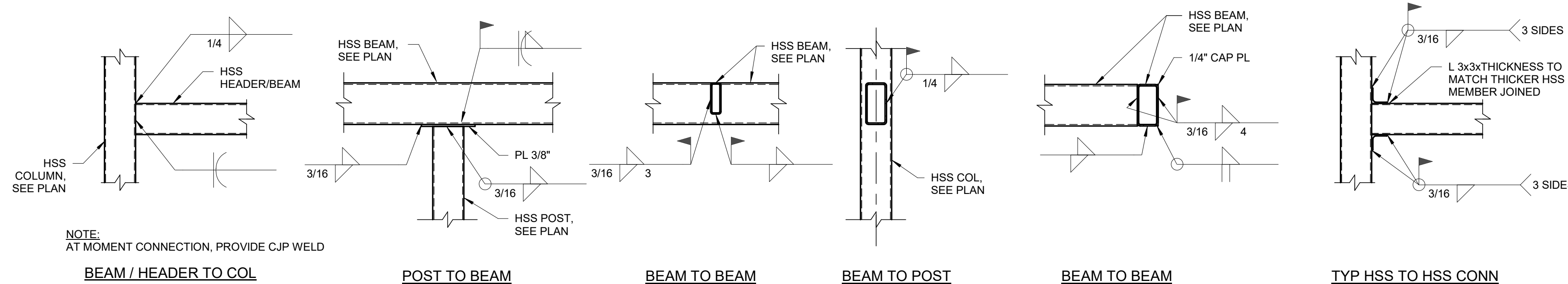
1 BEAM TO GIRDER/WF COL CONNECTIONS

SCALE: NTS



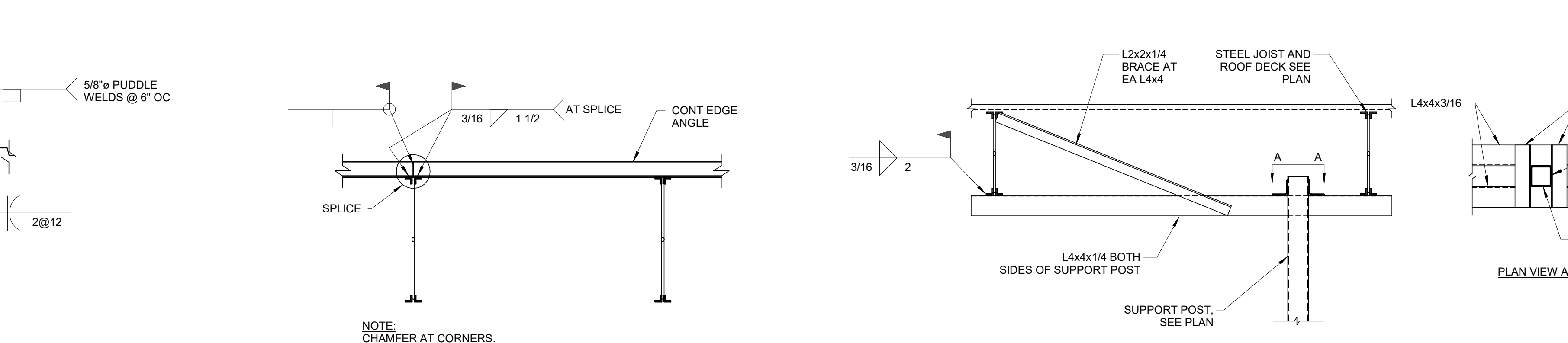
2 BEAM TO SQUARE HSS COL CONN

SCALE: NTS



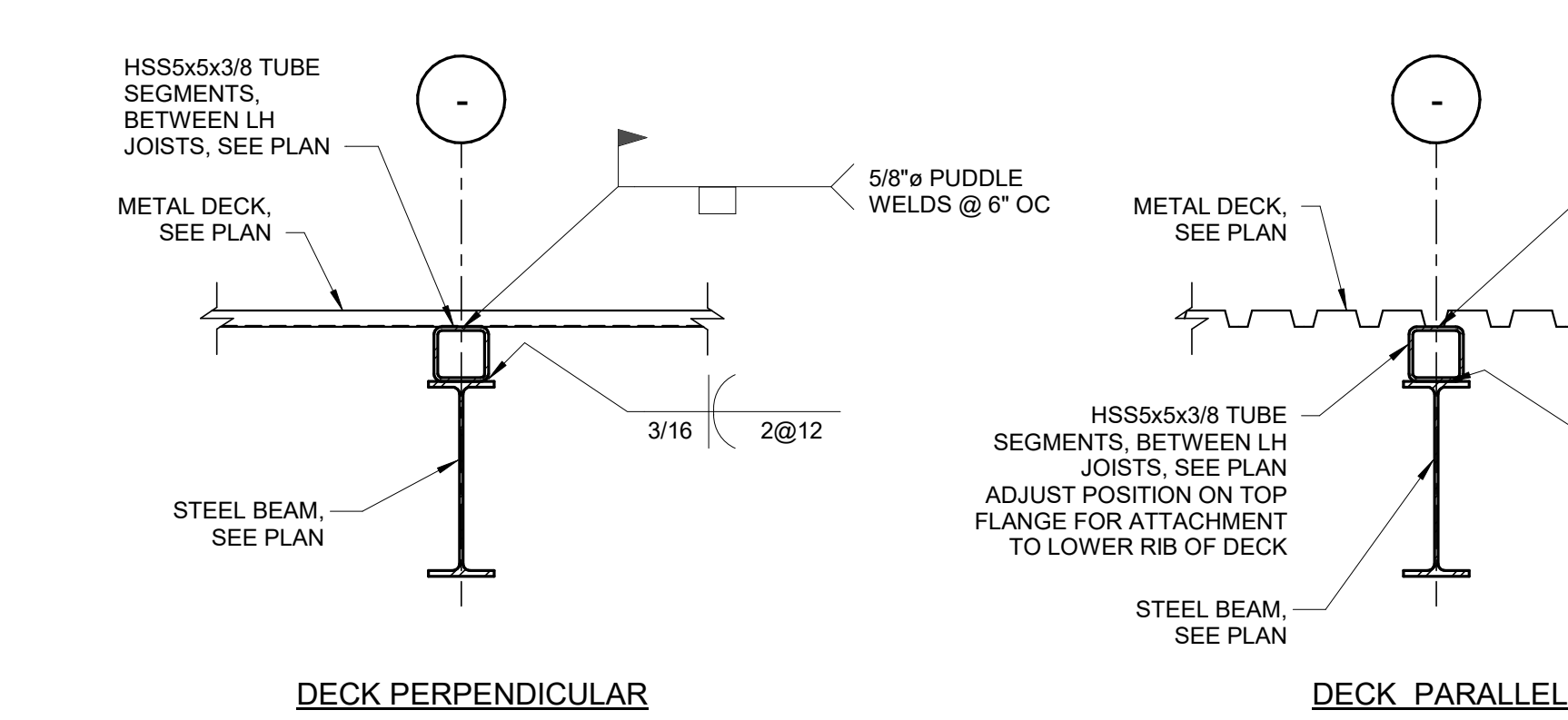
7 TYP HSS TO HSS CONNECTION

SCALE: NTS



6 CURTAIN WALL ATTACHMENT TO HSS WIND GIRT

SCALE: NTS



8 DECK ATTACHMENT @ MOMENT FRAME

SCALE: NTS



9 EDGE ANGLE SPLICE

SCALE: NTS



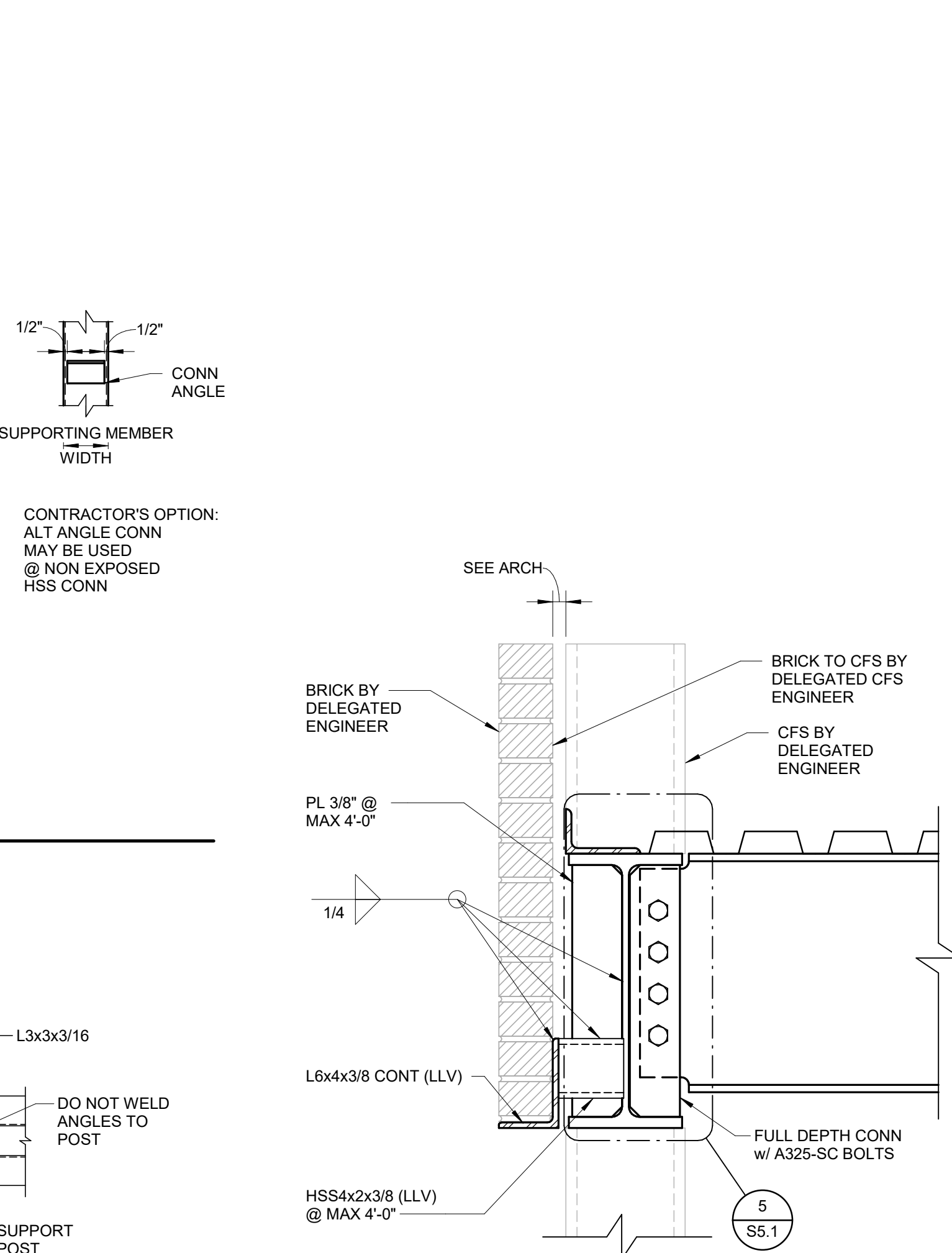
10 SECTION AT COILING DOOR

SCALE: NTS



5 STEEL BEAM TO COLUMN

SCALE: NTS

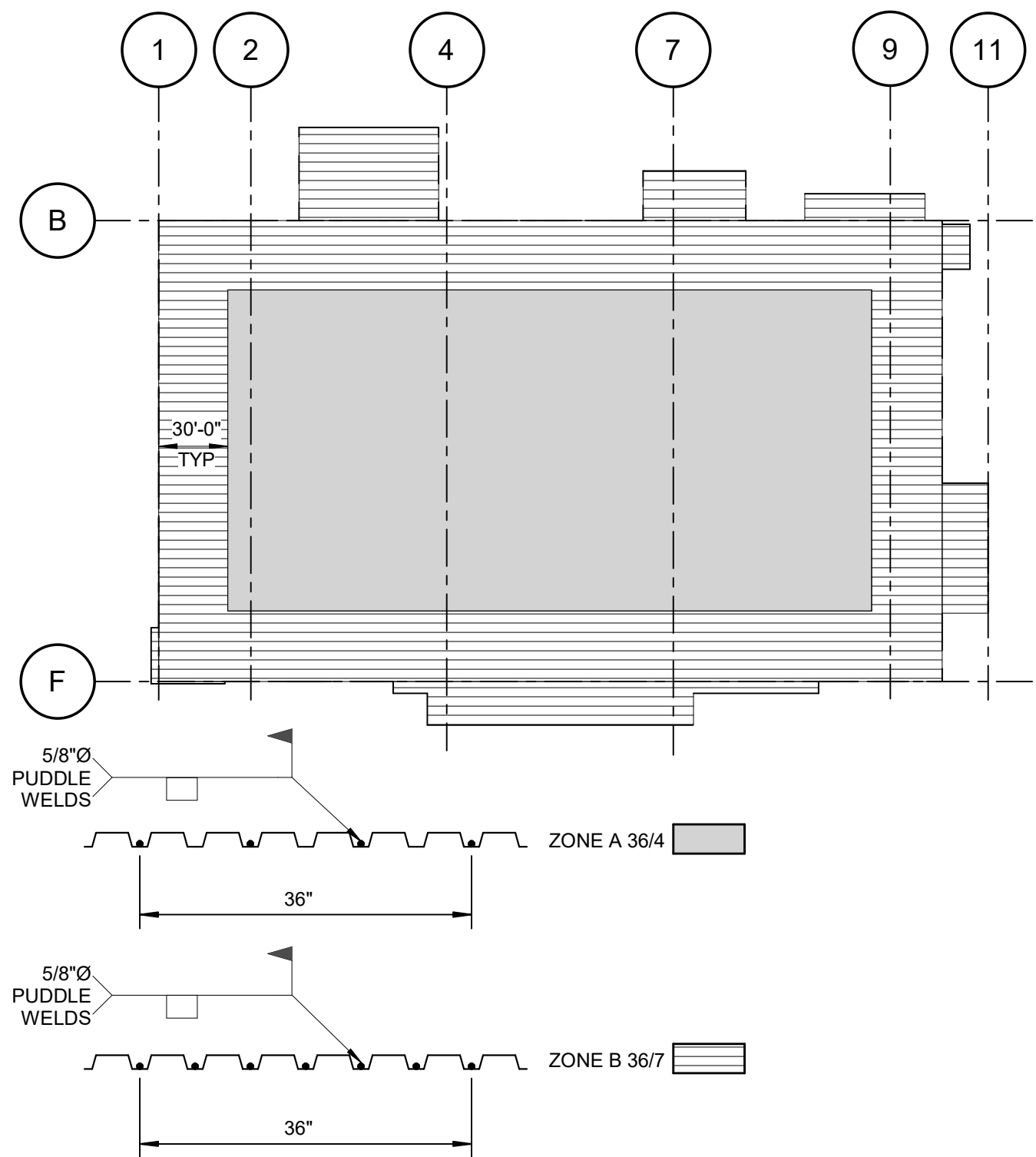


11 ENTRY PARAPET DETAIL

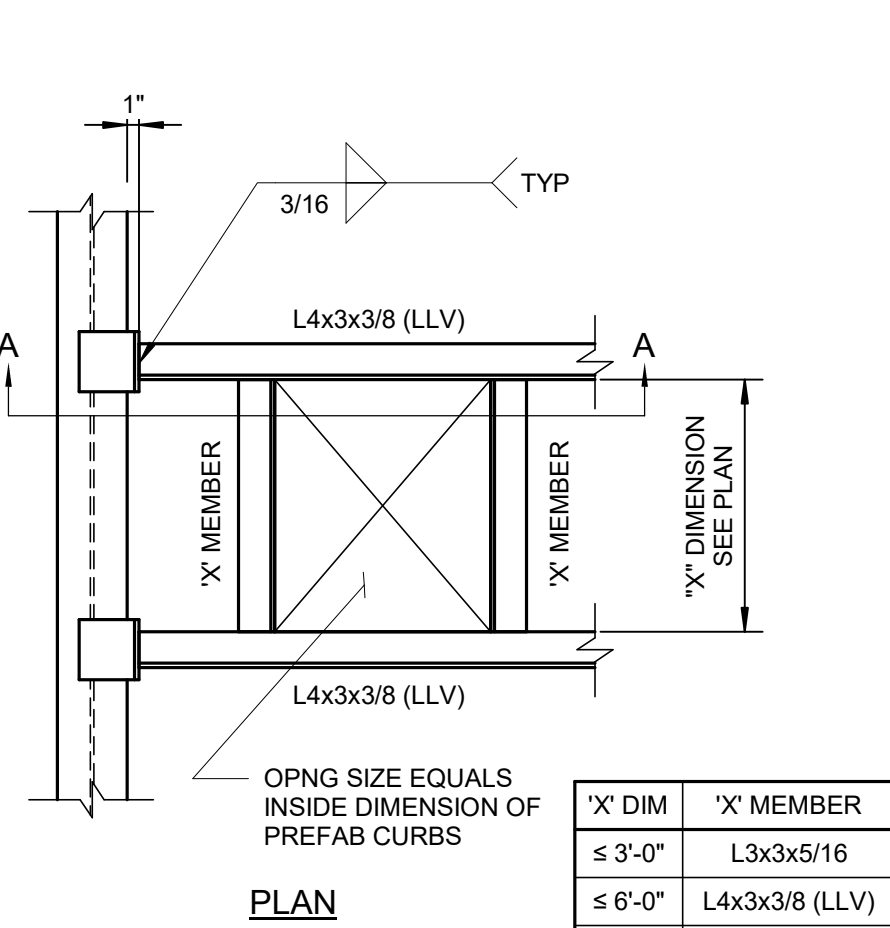
SCALE: NTS



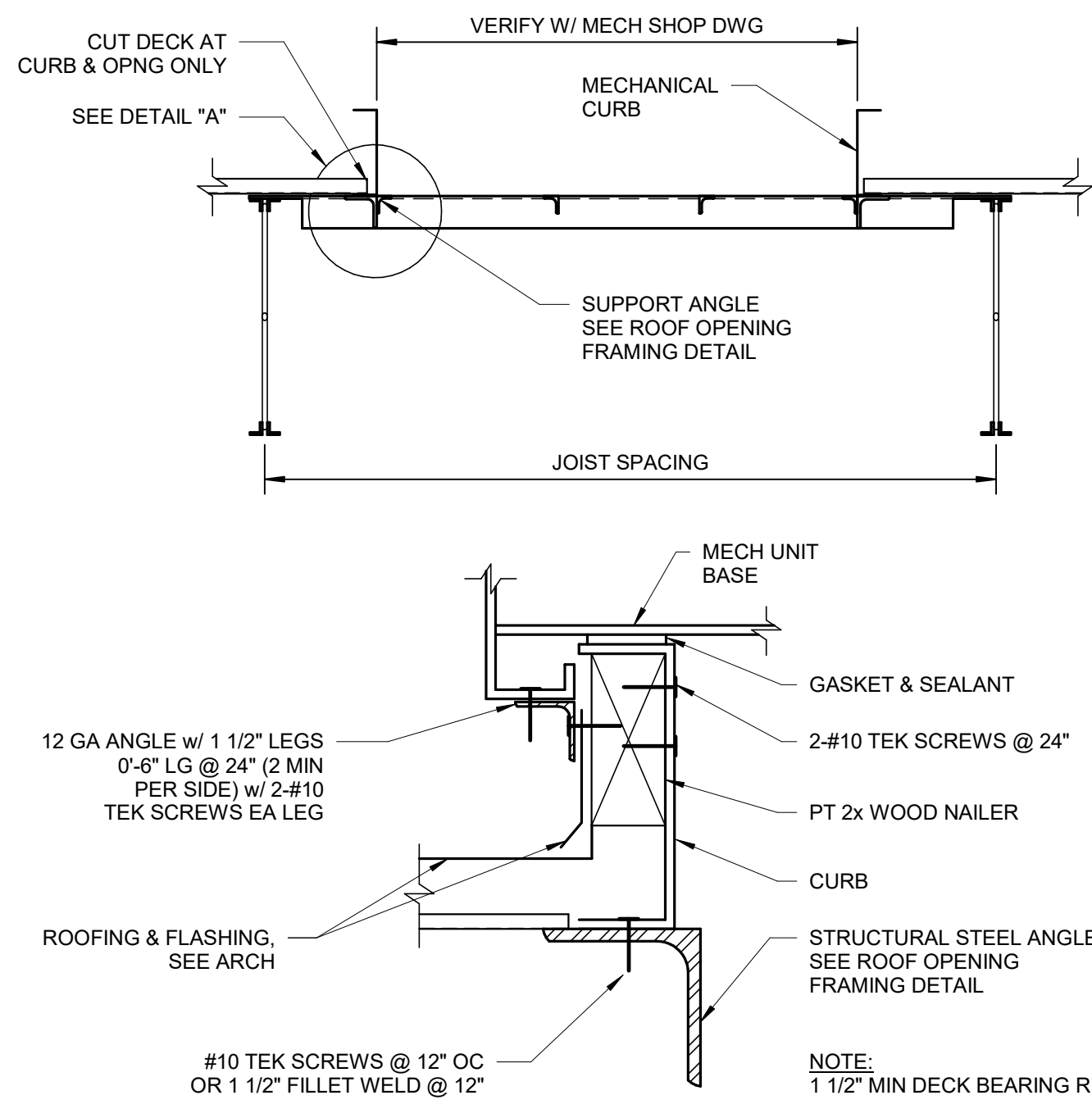
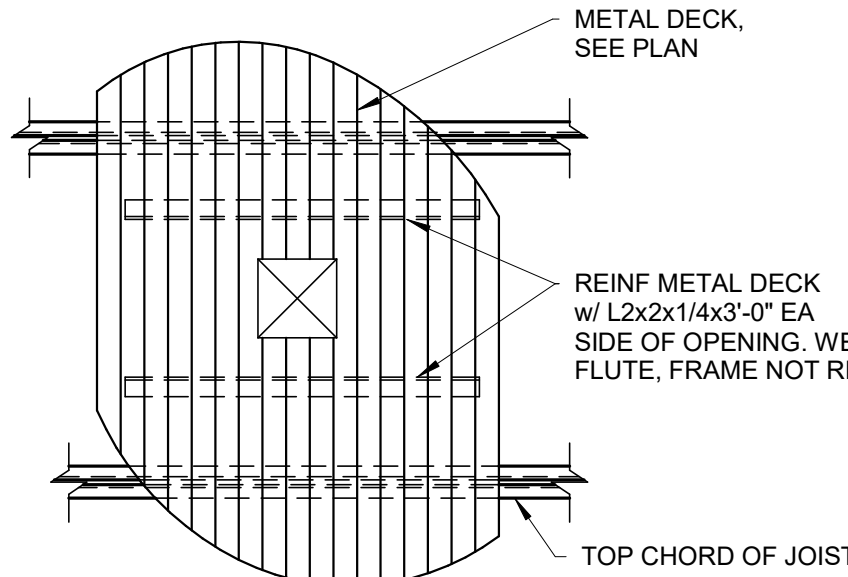
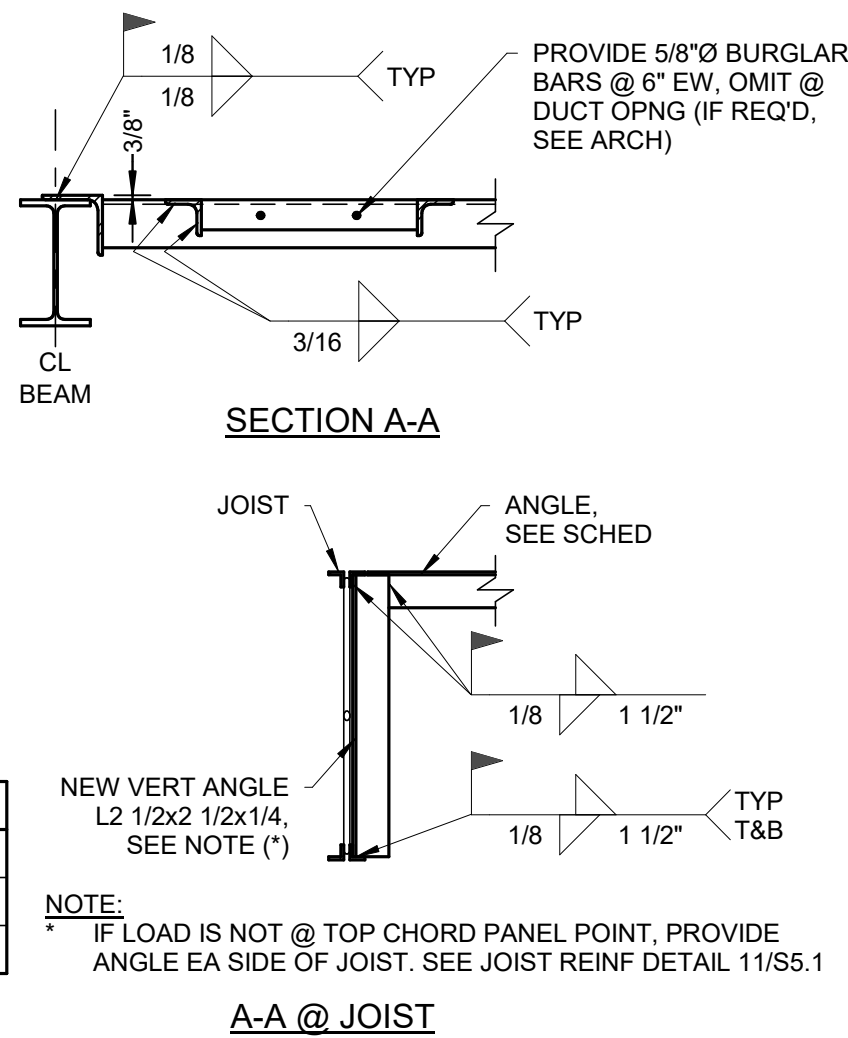
TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.



- NOTES:
- 1) ATTACHMENT AT SUPPORTS SHALL BE WELDS.
 - 2) ATTACHMENT AT SIDE LAPS SHALL BE #10 TEK SCREWS.
 - 3) ATTACHMENT AROUND OPENINGS SHALL BE WELDS @ 6" OC.
 - 4) ATTACHMENT AT SIDE SUPPORTS ALONG PERIMETER SHALL BE WELDS @ 6" OC.
 - 5) MINIMUM ENDLAP 3", TYP.
 - 6) SIDELAPS
ZONE A - 6 SIDELAPS PER SPAN
ZONE B - 8 SIDELAPS PER SPAN



- NOTES:
- 1) VERIFY ALL DIM w/ SUPPLIER BEFORE FABRICATION.
 - 2) INSTALL PREFAB CURBS DIRECTLY ON STEEL SUPPORT ANGLES.
 - 3) ROOF DECK IS NOT REQUIRED INSIDE MECH UNIT CURBS.
 - 4) AT OPENINGS 6" TO 12" PROVIDE L2x2x1/4x3'-0" LG PERPENDICULAR TO DECK SPAN. 5/8" WELD TO EA FLUTE.



TYP FRAMING AT ROOF OPNG

SCALE: NTS

SMALL OPENING DETAIL

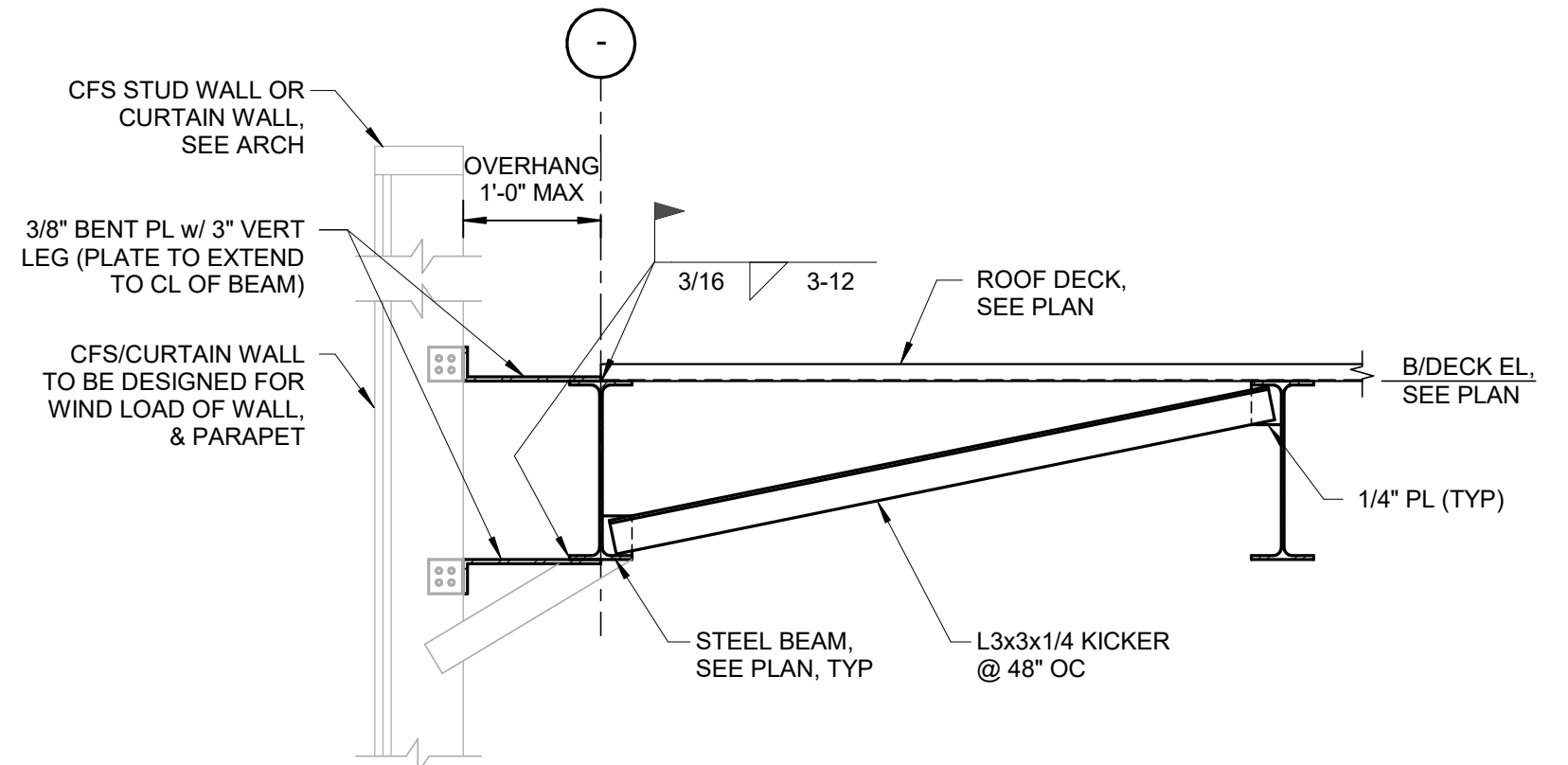
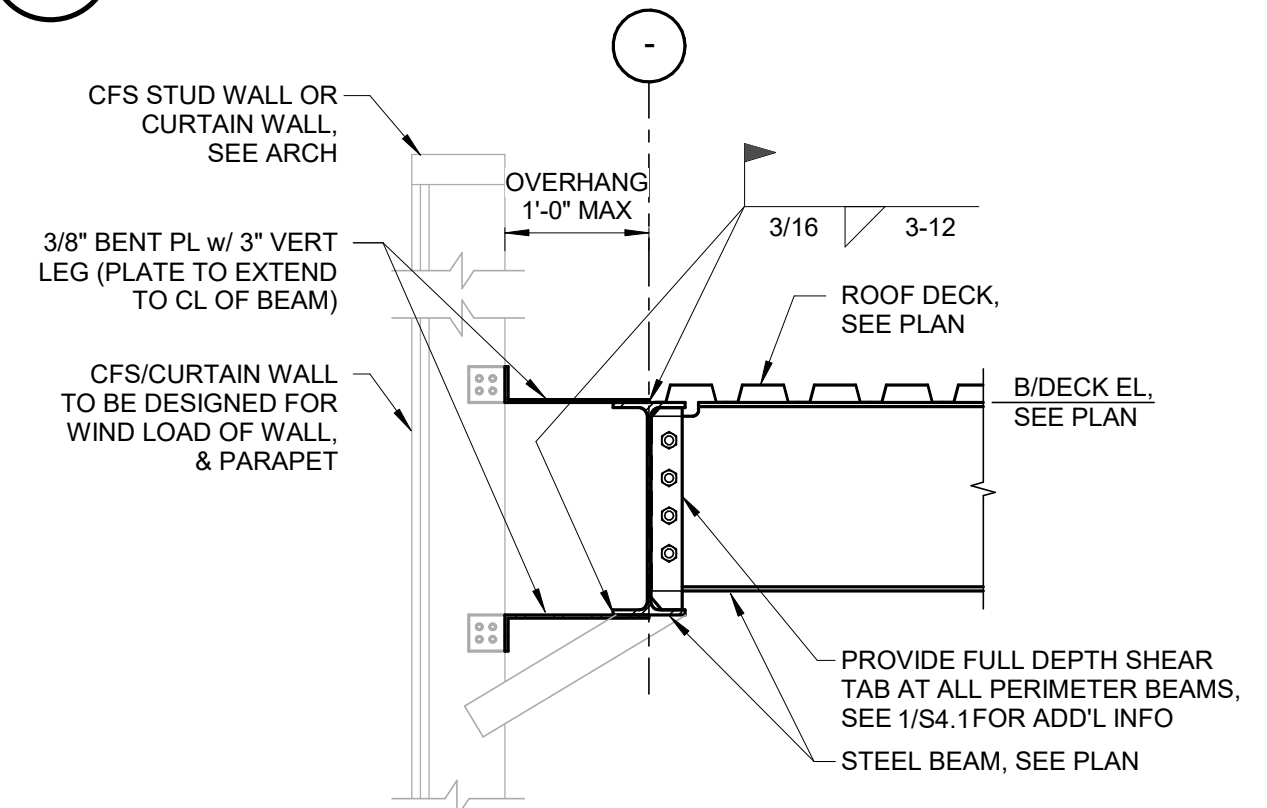
SCALE: NTS

ROOF TOP UNIT ATTACHMENT

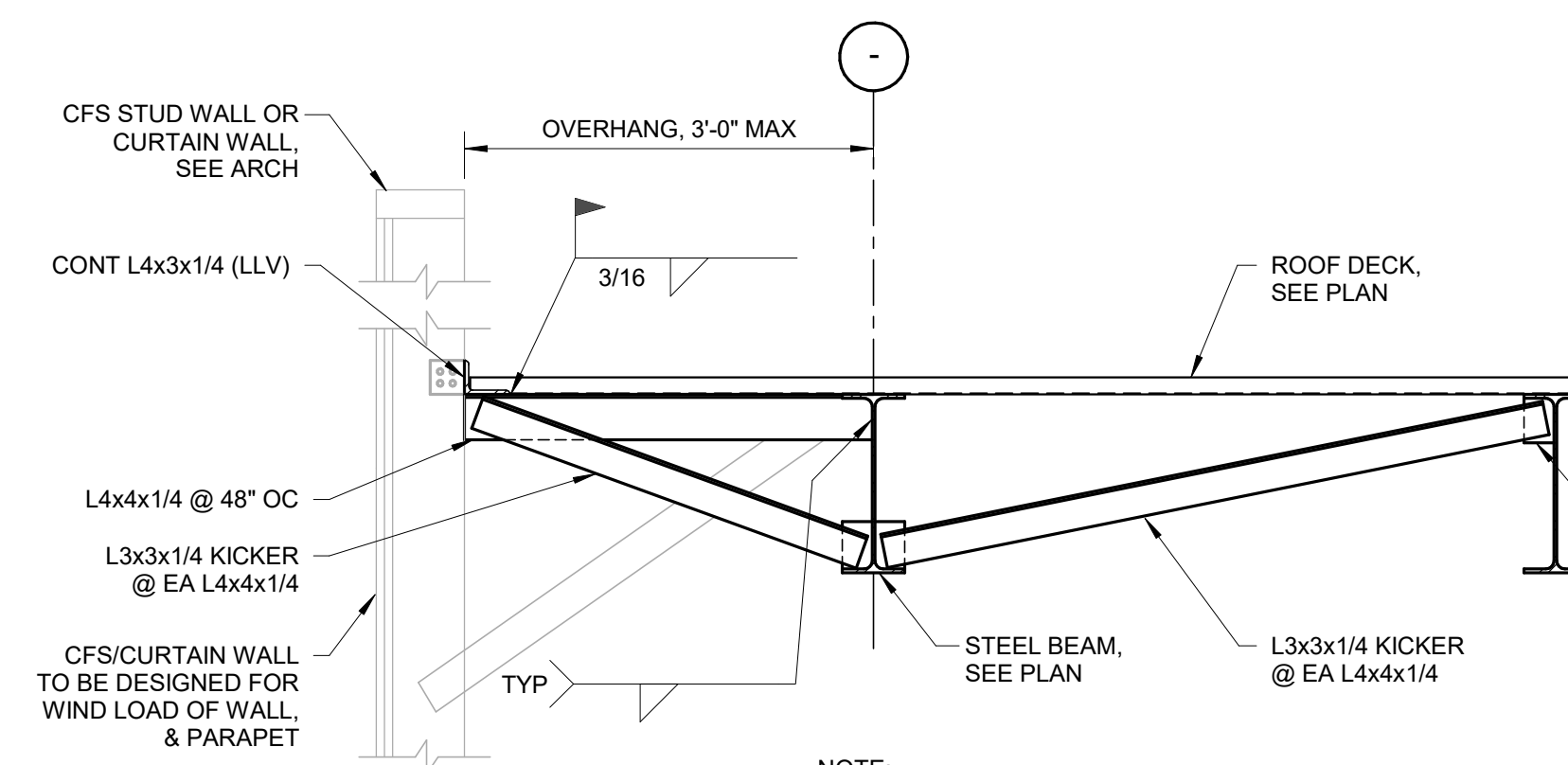
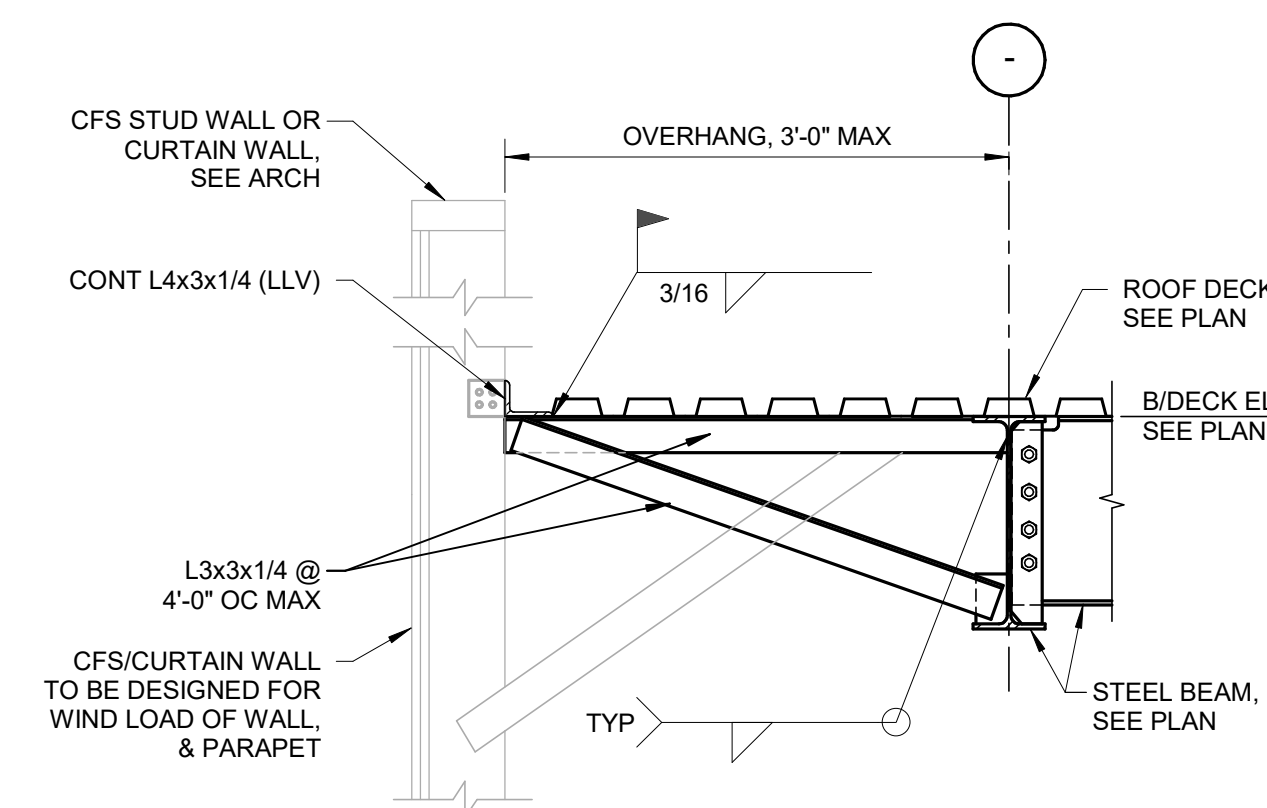
SCALE: NTS

TYP WELD PATTERN FOR ROOF DECK

SCALE: NTS



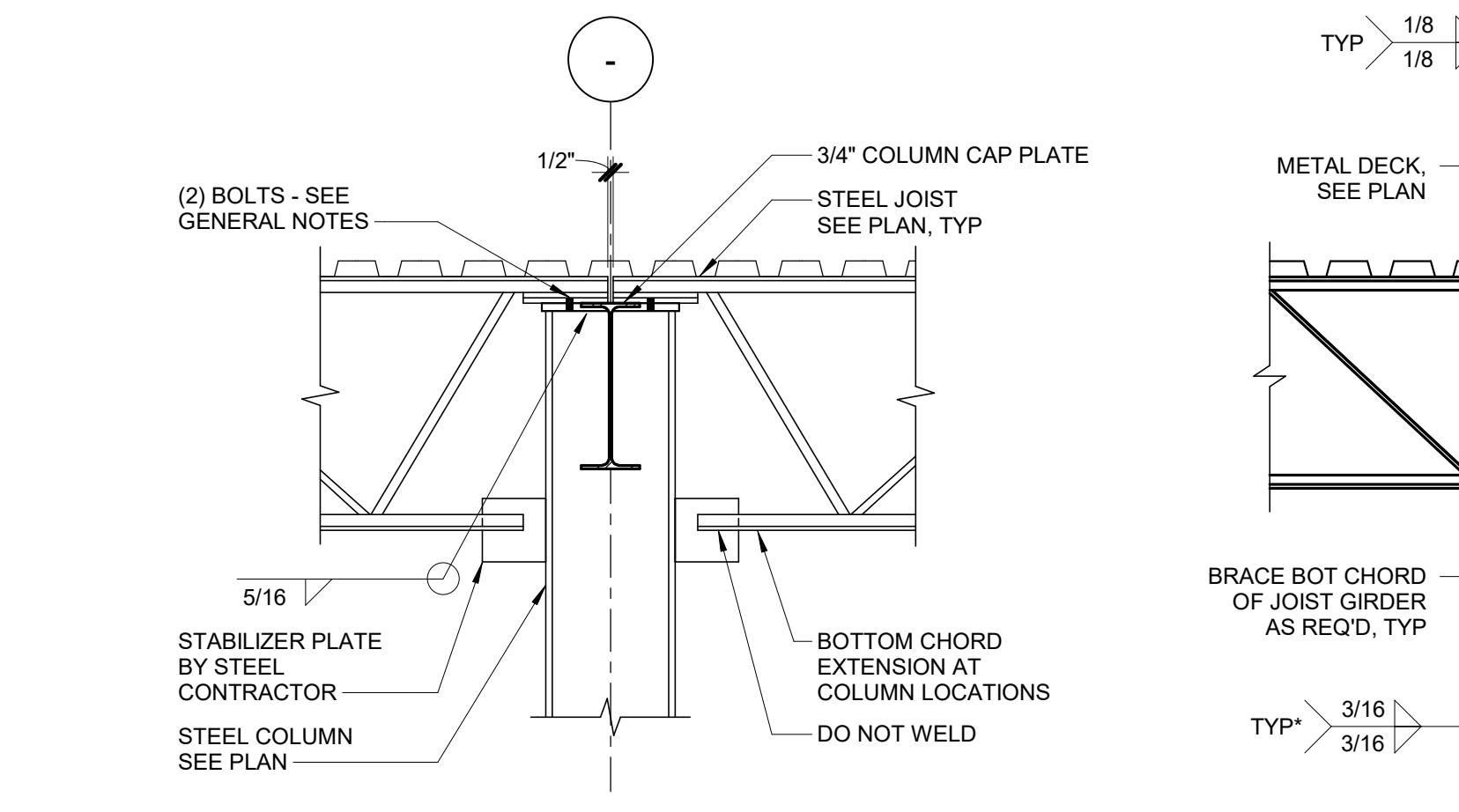
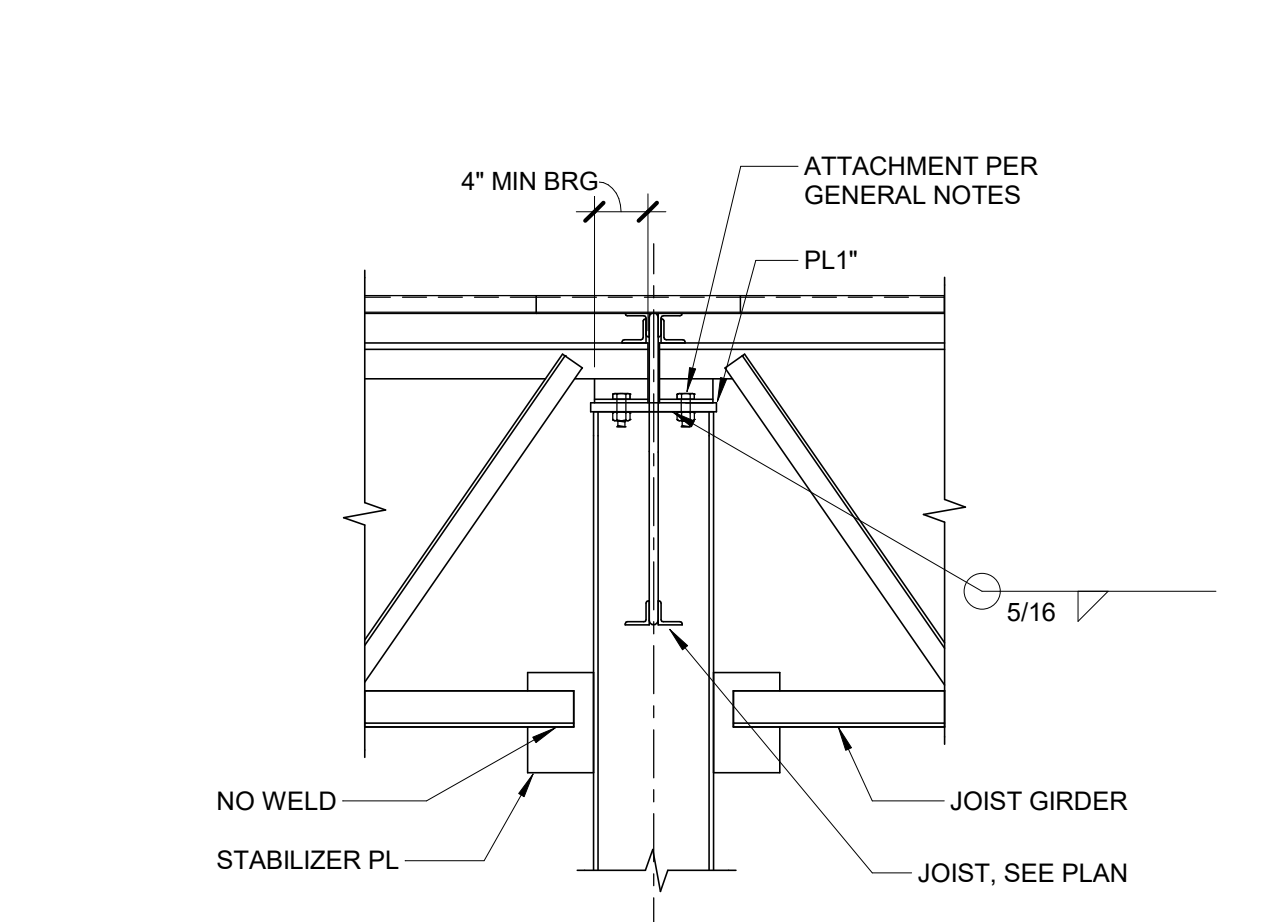
OVERHANG ≤ 1'-0" @ STEEL BEAMS



OVERHANG ≤ 3'-0" @ STEEL BEAMS

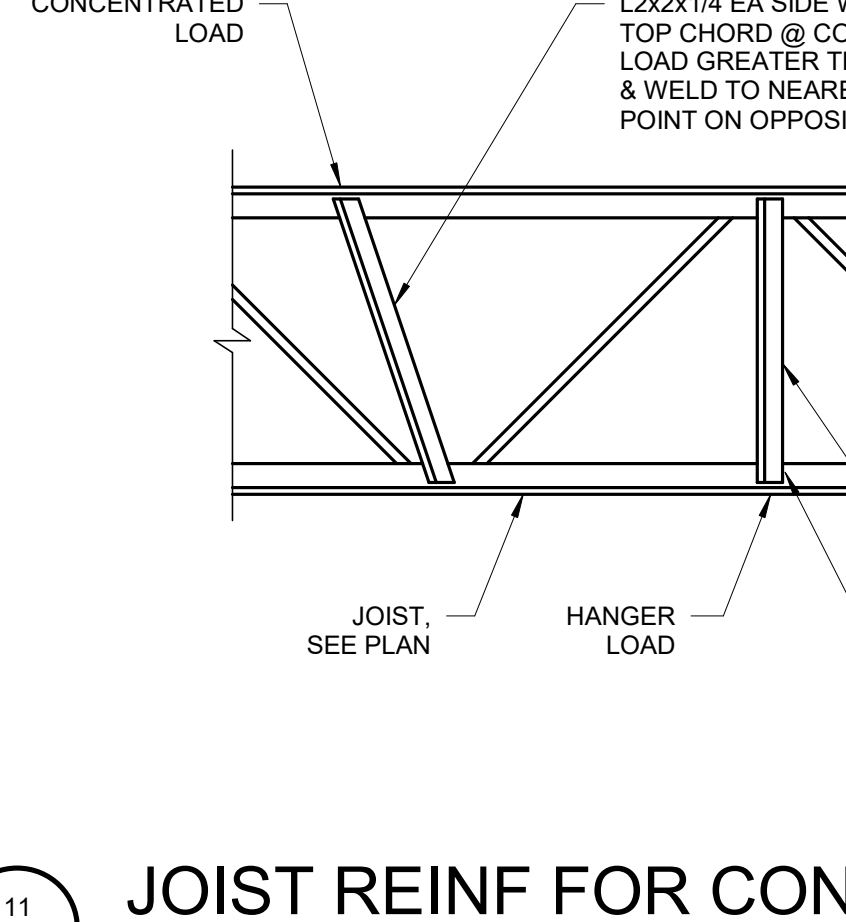
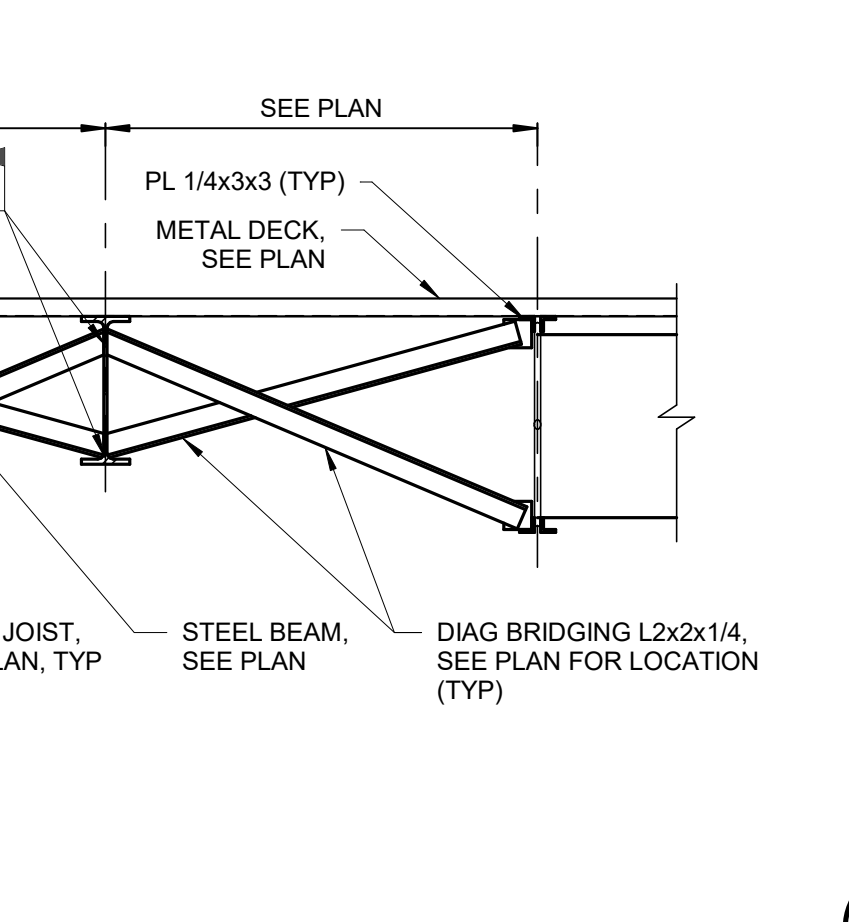
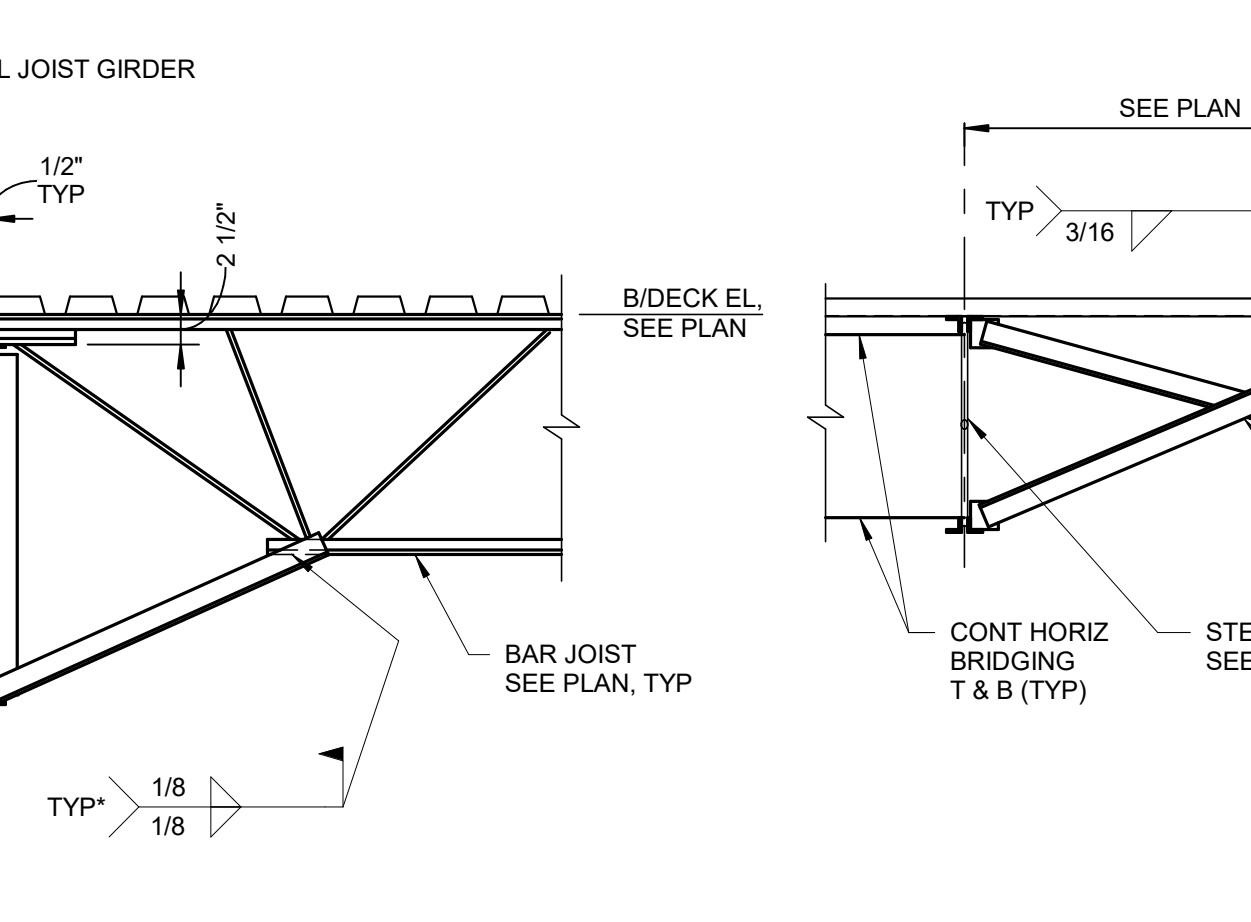
CANTILEVER ROOF DECK EDGE DETAIL

SCALE: NTS



TYP ROOF BEAM EDGE CONN

SCALE: NTS



JOIST REIN FOR CONCENTRATED LOADS

SCALE: NTS

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.

INT JOIST TO GIRDER W/ BRACE

SCALE: NTS



INT JOIST TO COLUMN W/ BRACE

SCALE: NTS



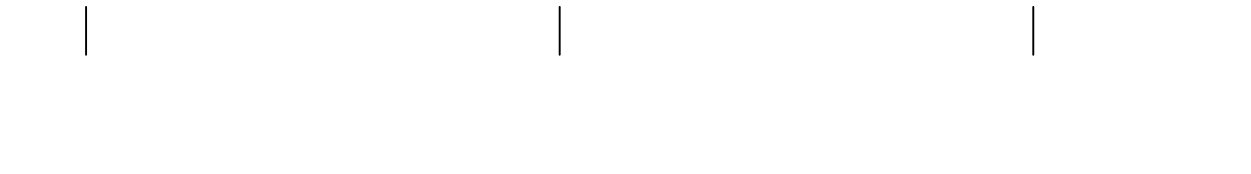
INT JOIST TO GRIDER W/ BRACE

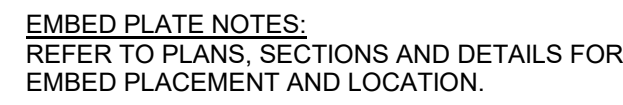
SCALE: NTS



JOIST BRIDGING TO BEAM

SCALE: NTS



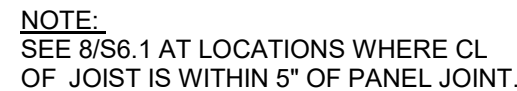


SCALE: NTS

SCALE: NTS

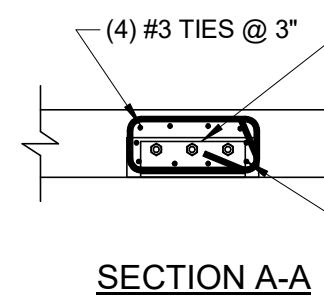
S6.1

SCALE: NTS



SCALE: NTS

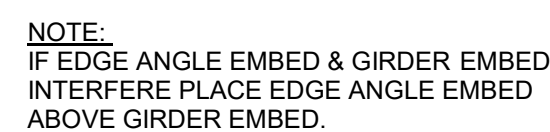
SCALE: NTS



NOTE:
MIN REINF REQUIREMENTS
SHOWN, SEE SHEET S7.1-S7.4
FOR ADDITIONAL INFO.

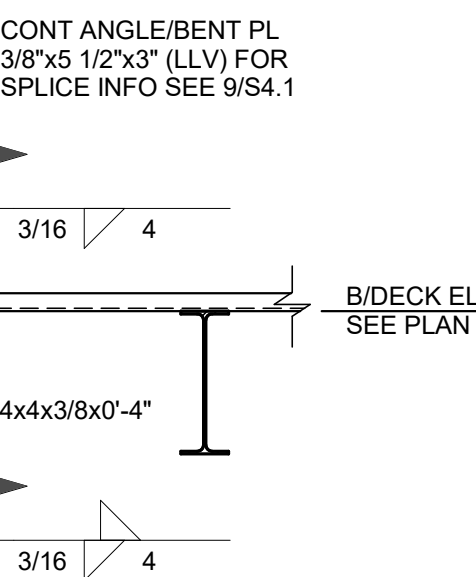
S6.1

SCALE: NTS



S6.1

SCALE: NTS



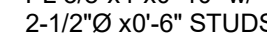
SCALE: NTS

SCALE: NTS



S6.

SCALE: NT



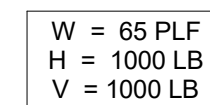
S6.1

SCALE: NTS



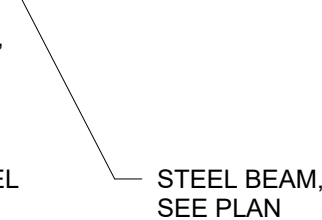
S6.1

SCALE: NTS

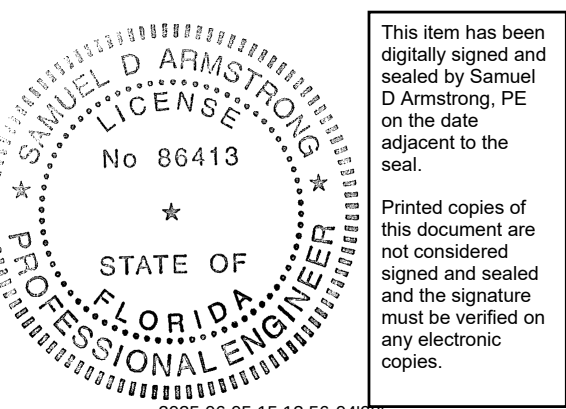


S6.1

SCALE: NTS



TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.



ERIK C. KASPER AR#0017162

BUSY BEE
ELLISVILLE

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

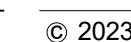
PRIOR ISSUES:

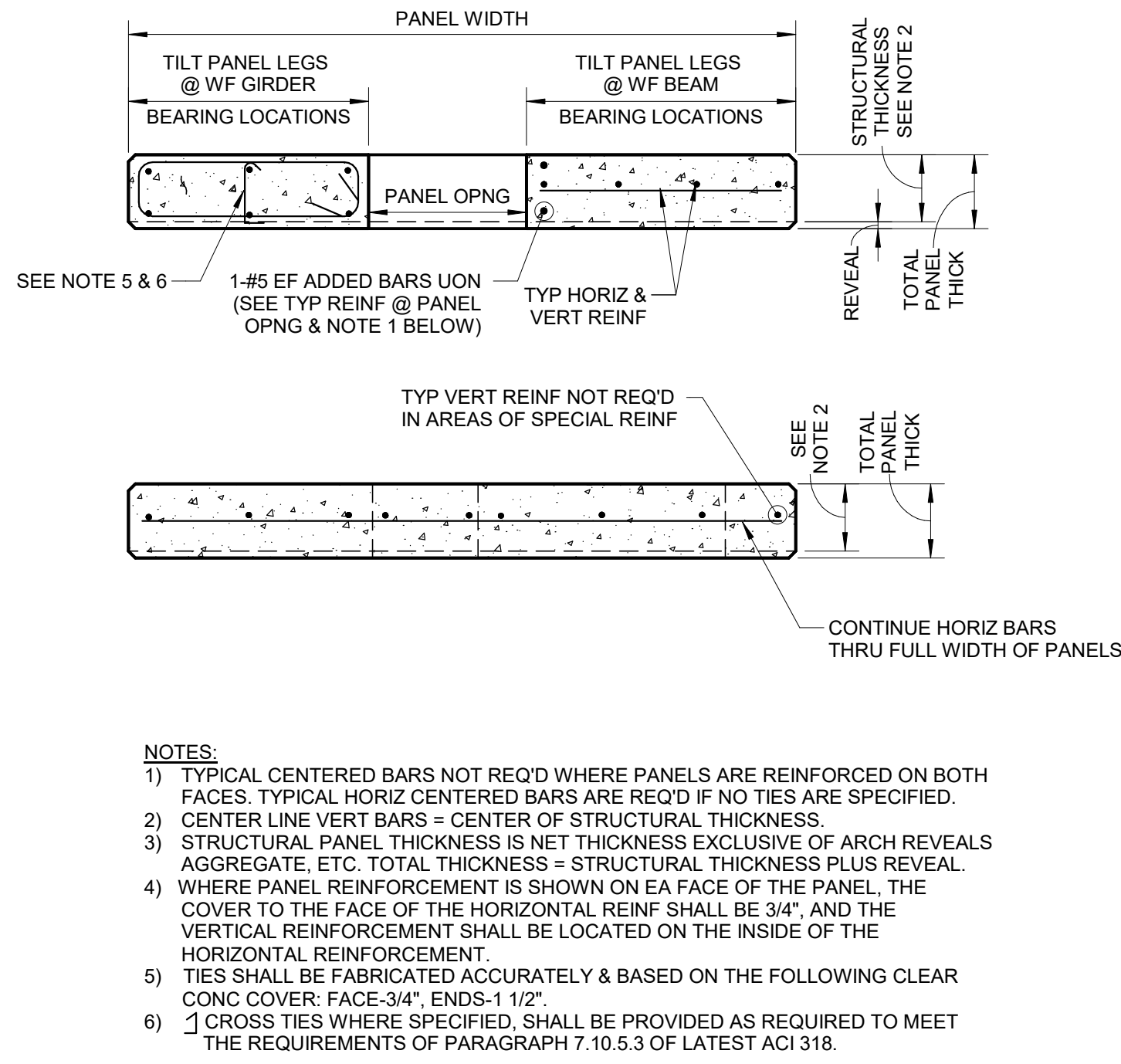
TILT PANEL SECTIONS & DETAILS

S6.1

DRAWN BY:	ES
CHECKED BY:	CF

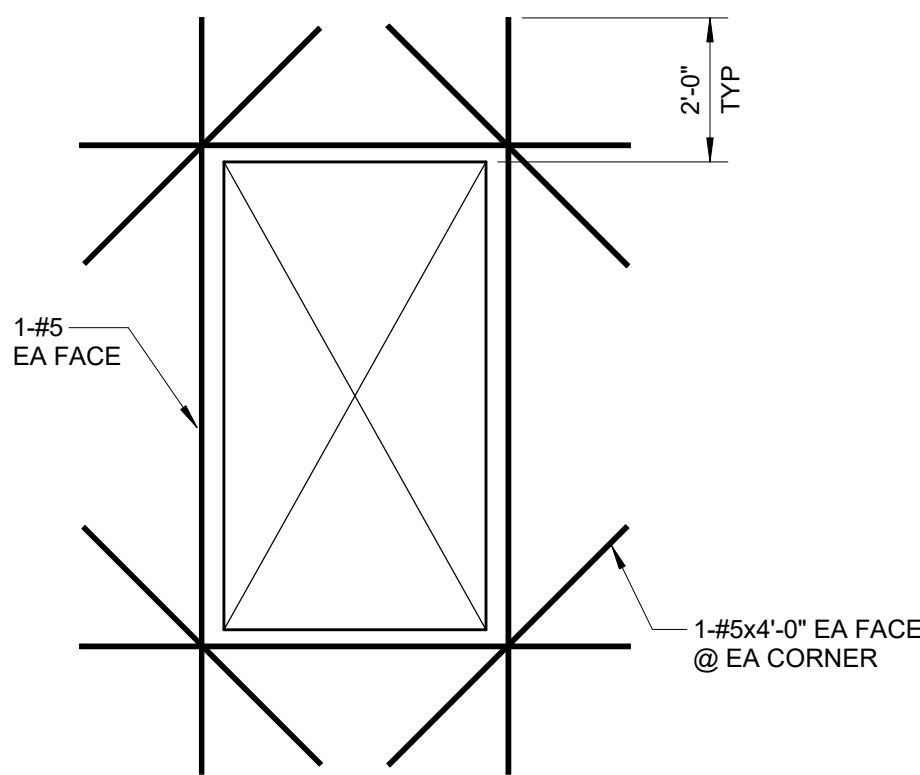
100%
CONSTRUCTION
DOCUMENTS





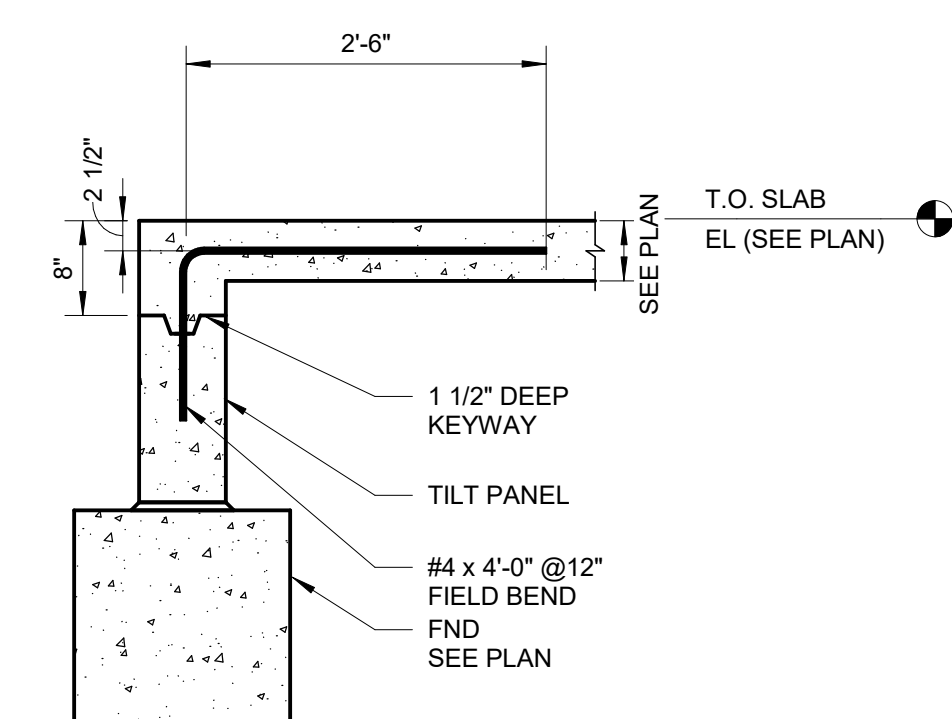
PANEL REINFORCING DETAIL

SCALE: NTS



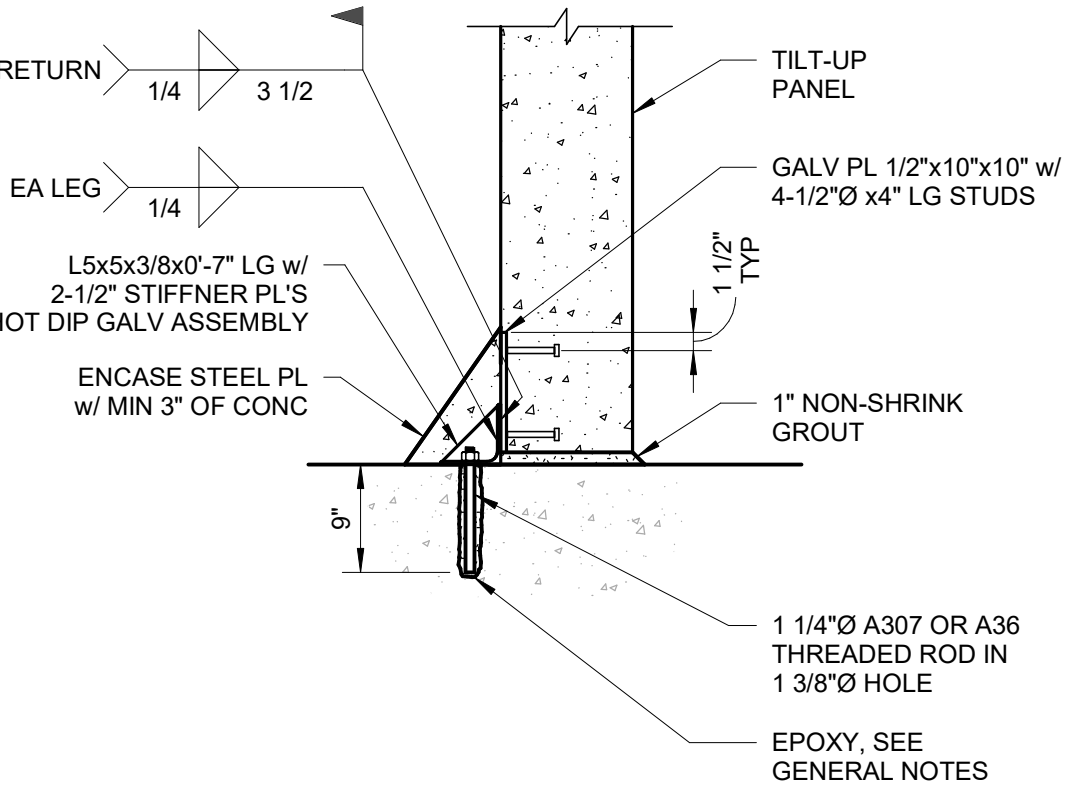
TYP REINF @ PANEL OPNG

SCALE: NTS



OPENING AT SLAB DETAIL

SCALE: NTS

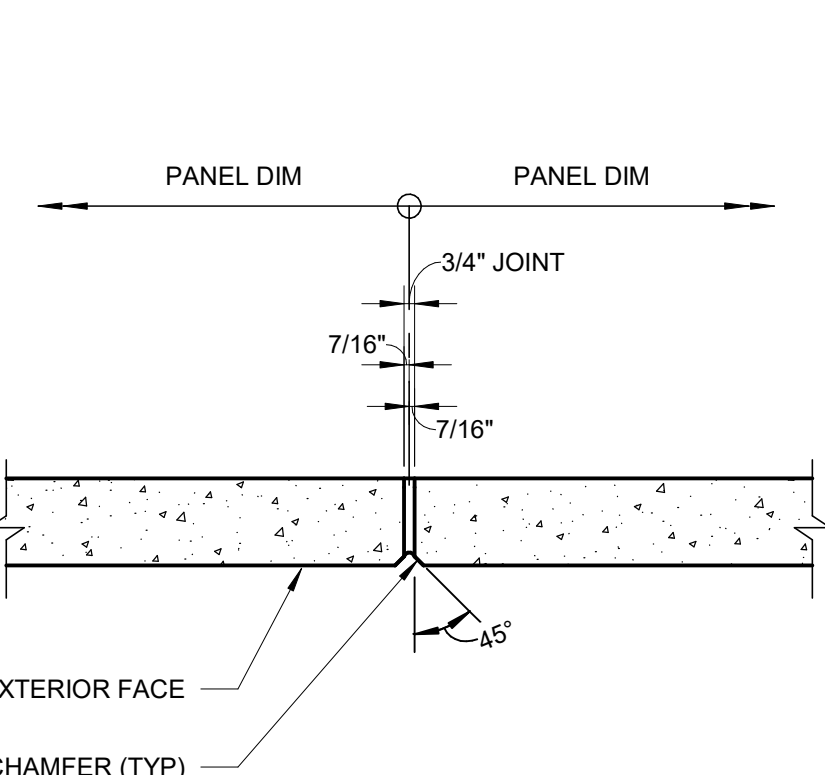


SHEARWALL CONN TO FTG

SCALE: NTS

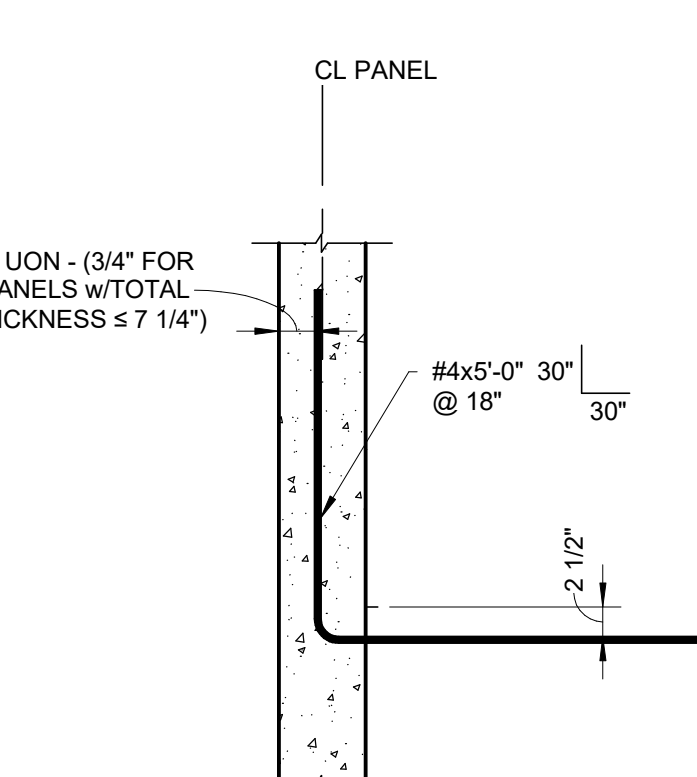
PANEL ARCH REVEAL

SCALE: NTS



PANEL JOINT

SCALE: NTS



WALL TO SLAB DOWEL DETAIL

SCALE: NTS

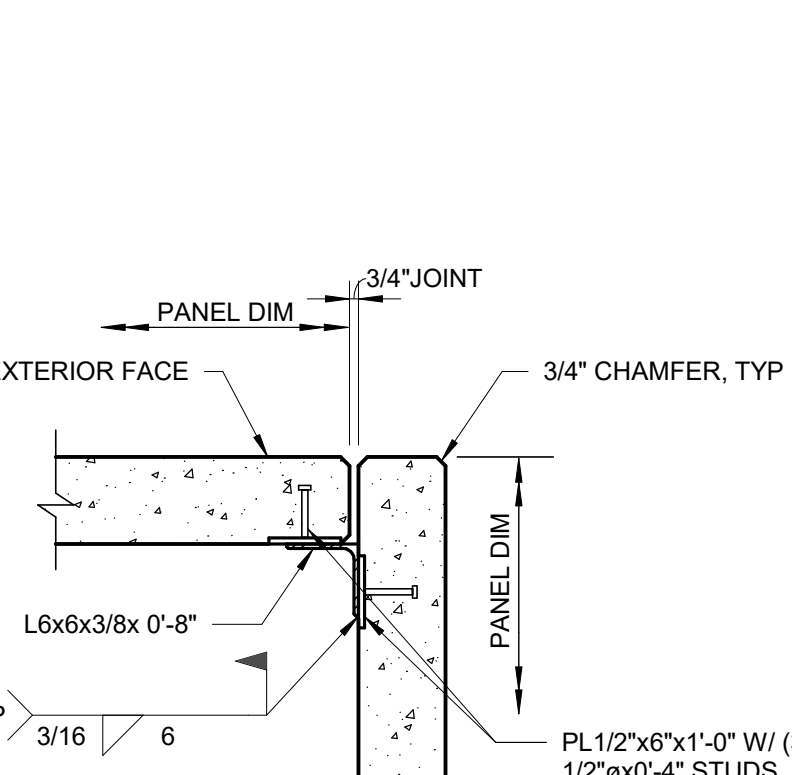


WALL TO SLAB DOWEL DETAIL

SCALE: NTS

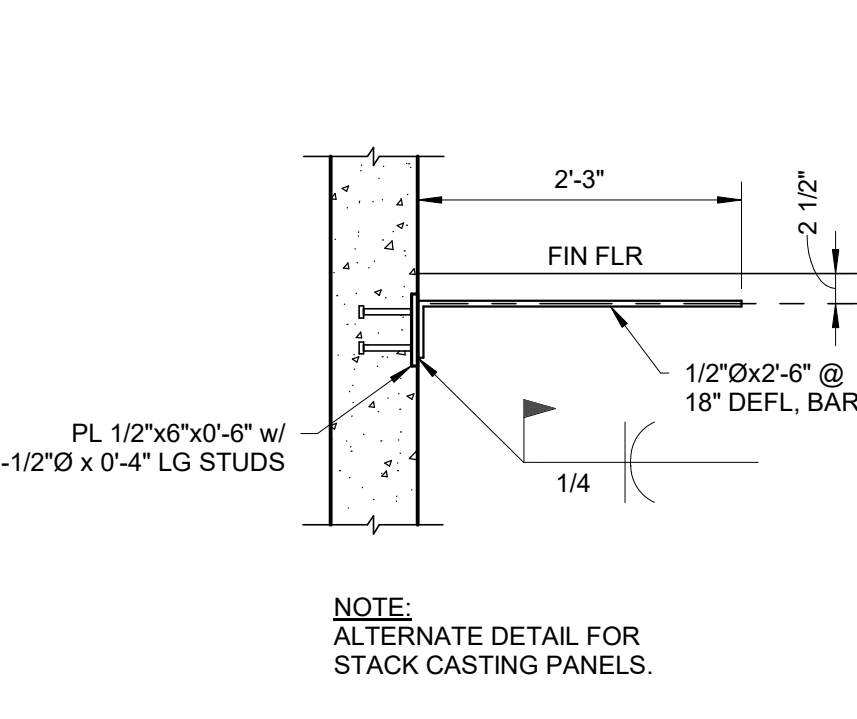
PANEL DOOR OPNG

SCALE: NTS



PANEL BUTT JOINT

SCALE: NTS



WALL TO SLAB DOWEL DETAIL

SCALE: NTS

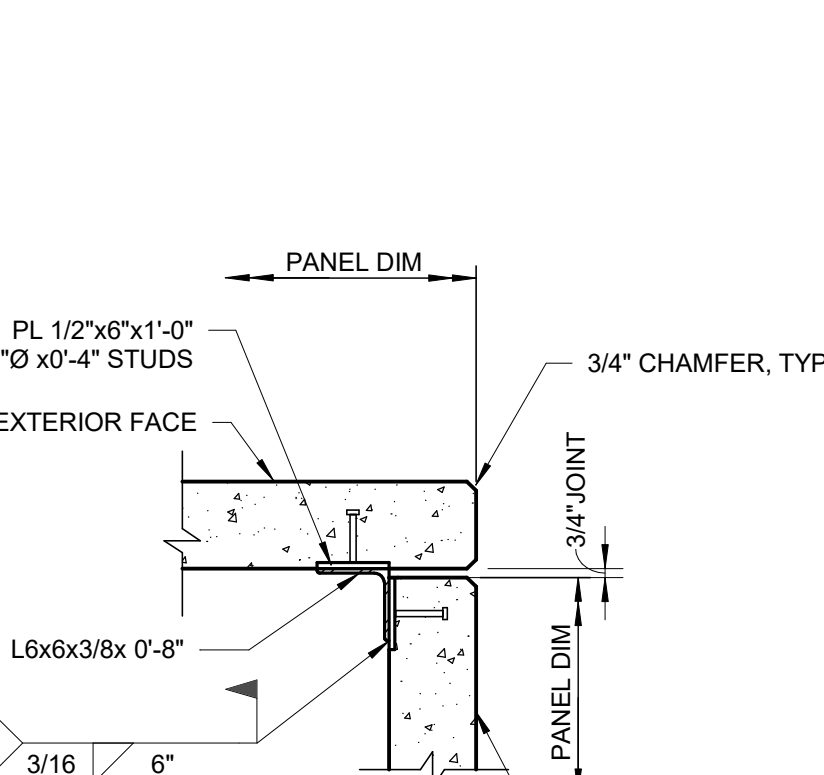


WALL TO SLAB DOWEL DETAIL

SCALE: NTS

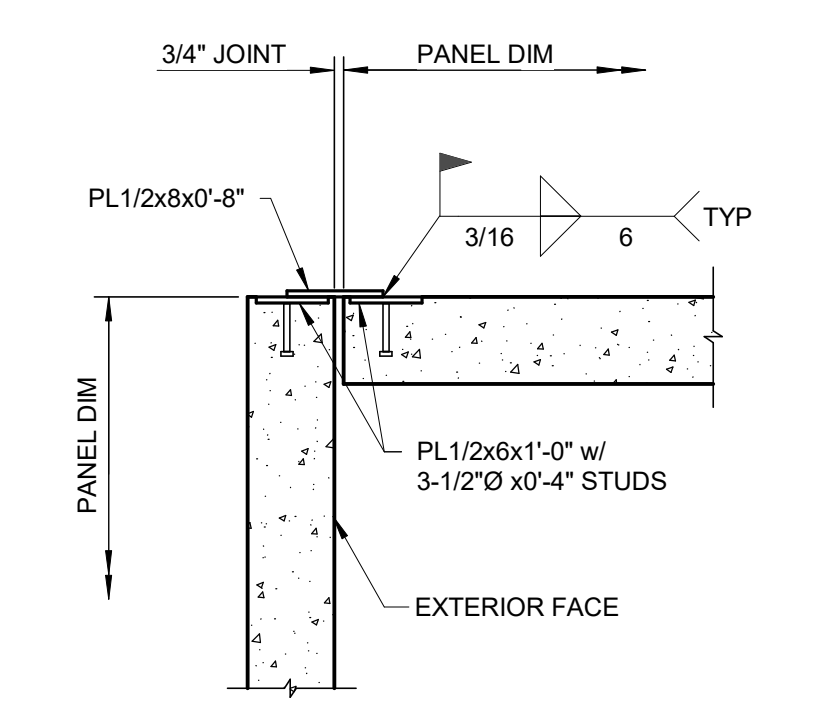
DOOR FRAME OPTION B

SCALE: NTS



PANEL BUTT JOINT

SCALE: NTS



CORNER PANEL TO PANEL JOINT

SCALE: NTS

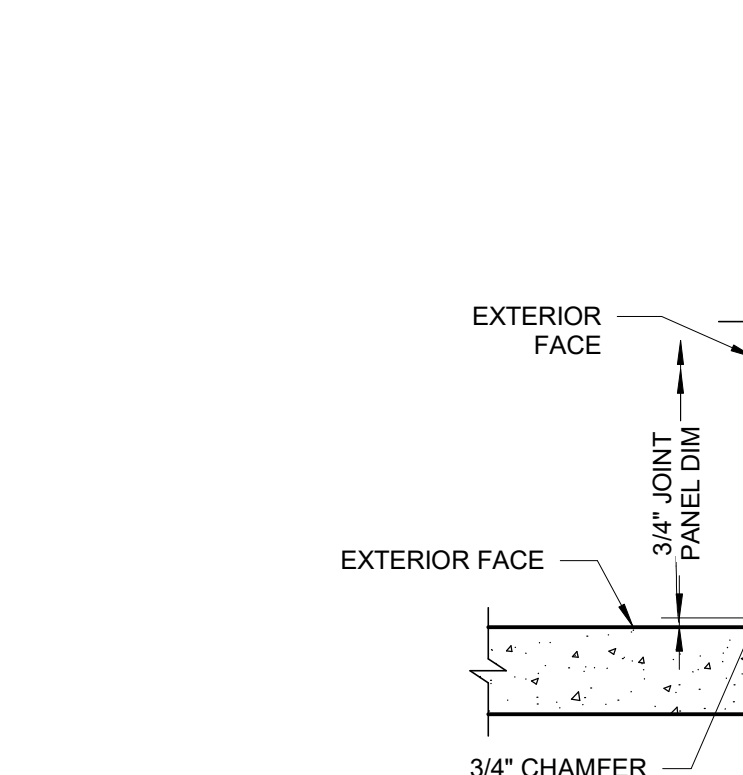


CORNER PANEL TO PANEL JOINT

SCALE: NTS

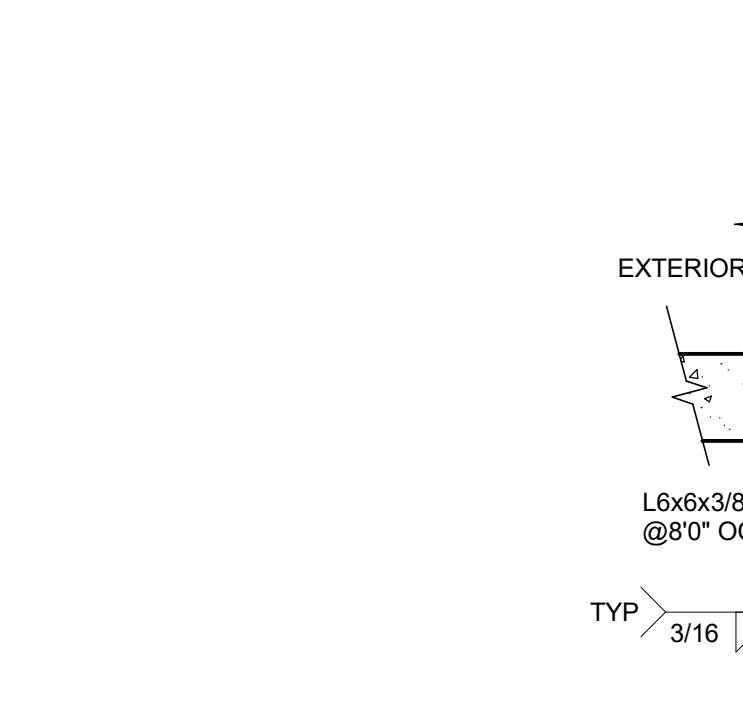
PANEL OPENING

SCALE: NTS



PANEL INTERSECTION JOINT

SCALE: NTS



SECTION

SCALE: NTS

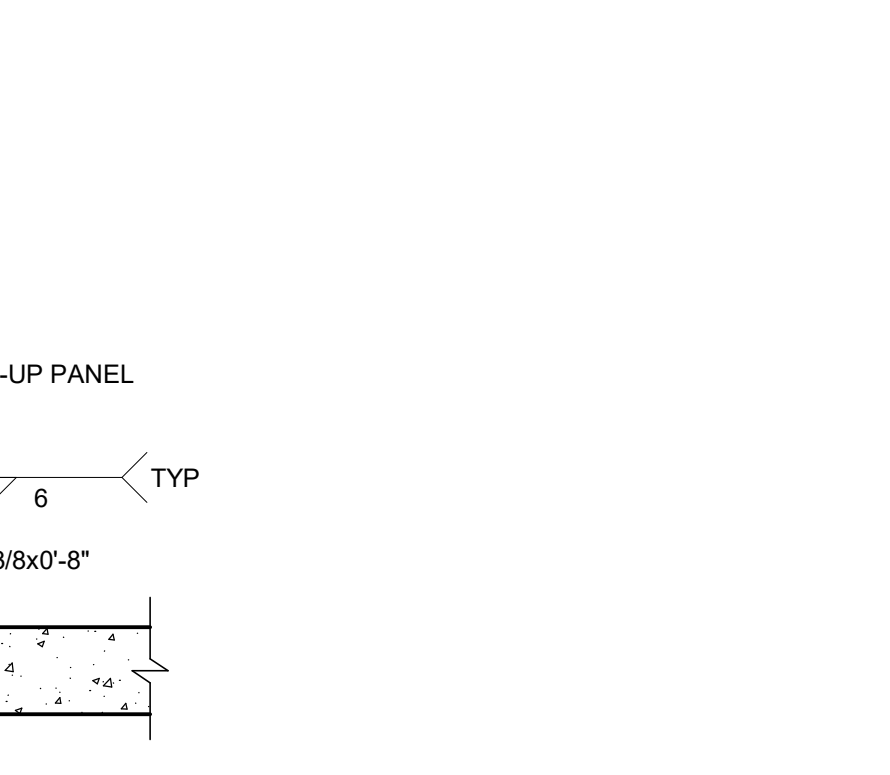


SECTION

SCALE: NTS

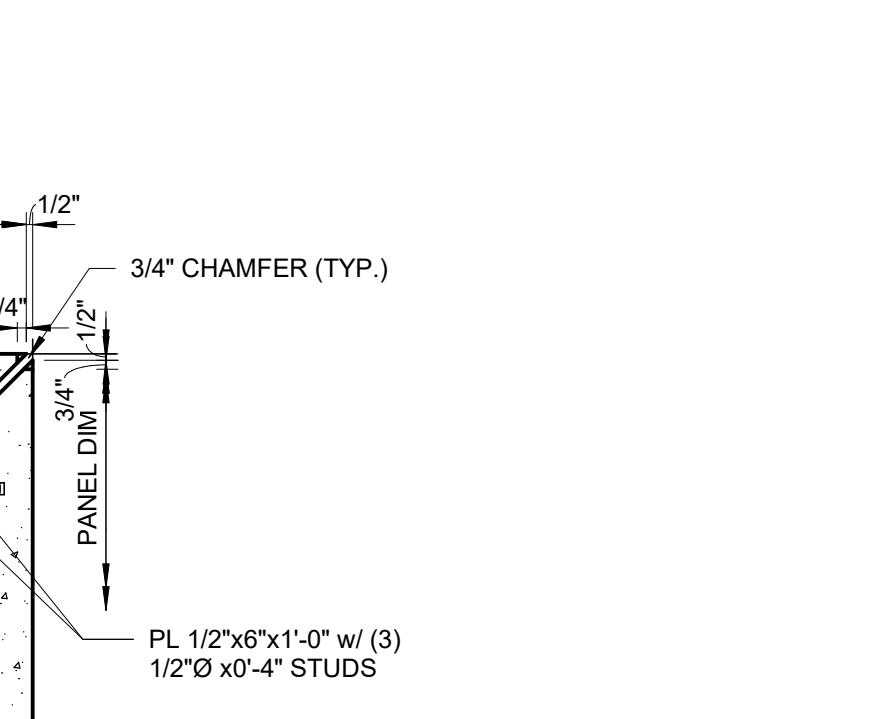
LIFTING INSERT DETAIL

SCALE: NTS



PANEL INTERSECTION JOINT

SCALE: NTS



PANEL INTERSECTION JOINT

SCALE: NTS



PANEL INTERSECTION JOINT

SCALE: NTS

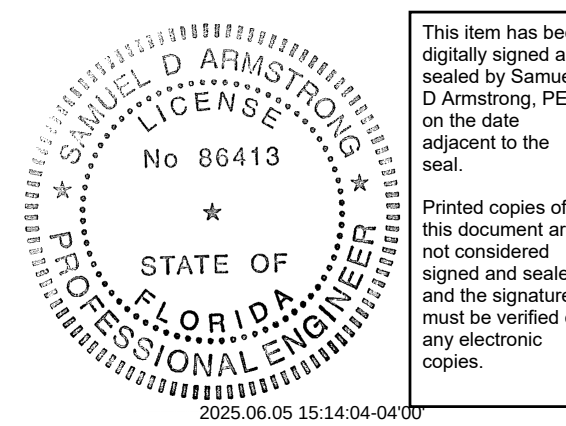


10175 Fortune Parkway, Suite 701
Jacksonville, FL 32256
p 904.683.9201

www.kasperarch.com
FL License No.: AA26000802



9133 R.G. SKINNER PARKWAY
SUITE 1
JACKSONVILLE, FL 32256
P: 904.483.5200
PROJECT #23000902.00
IMEG RESERVES PROPRIETARY RIGHTS, INCLUDING COPYRIGHTS TO THIS DRAWING AND THE DATA SHOWN THEREON. SAID DRAWINGS AND/OR DATA ARE THE EXCLUSIVE PROPERTY OF IMEG AND SHALL NOT BE USED OR REPRODUCED FOR ANY OTHER PROJECT WITHOUT THE EXPRESS WRITTEN APPROVAL AND PARTICIPATION OF IMEG. ©2023 IMEG CONSULTANTS CORP.



ERIK C. KASPER AR#0017162

BUSY BEE
ELLISVILLE

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

PRIOR ISSUES:

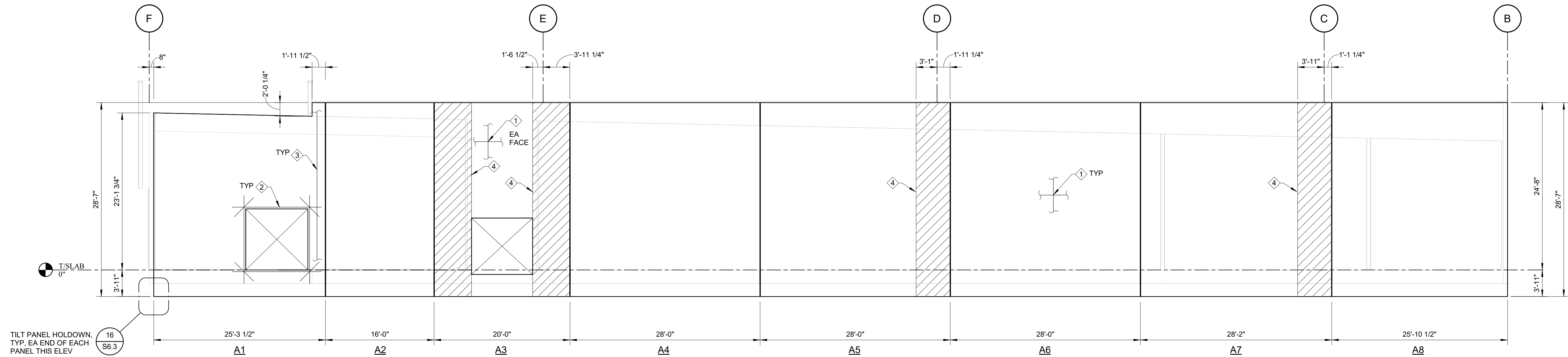
TILT PANEL
SECTIONS &
DETAILS

S6.3

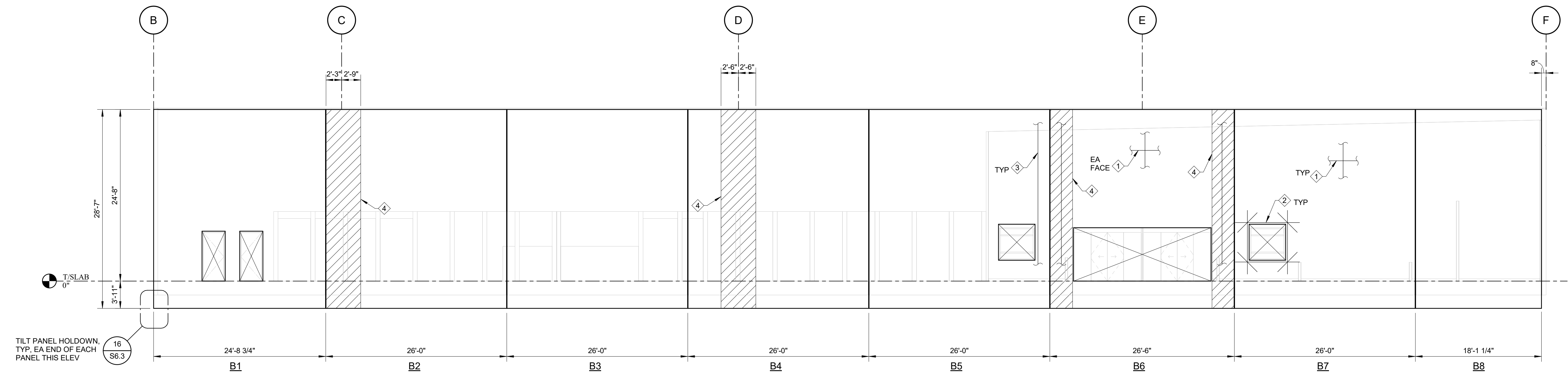
DRAWN BY: ES
CHECKED BY: CF

100%
CONSTRUCTION
DOCUMENTS

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.



ELEVATION A
SCALE: 1/8" = 1'-0"



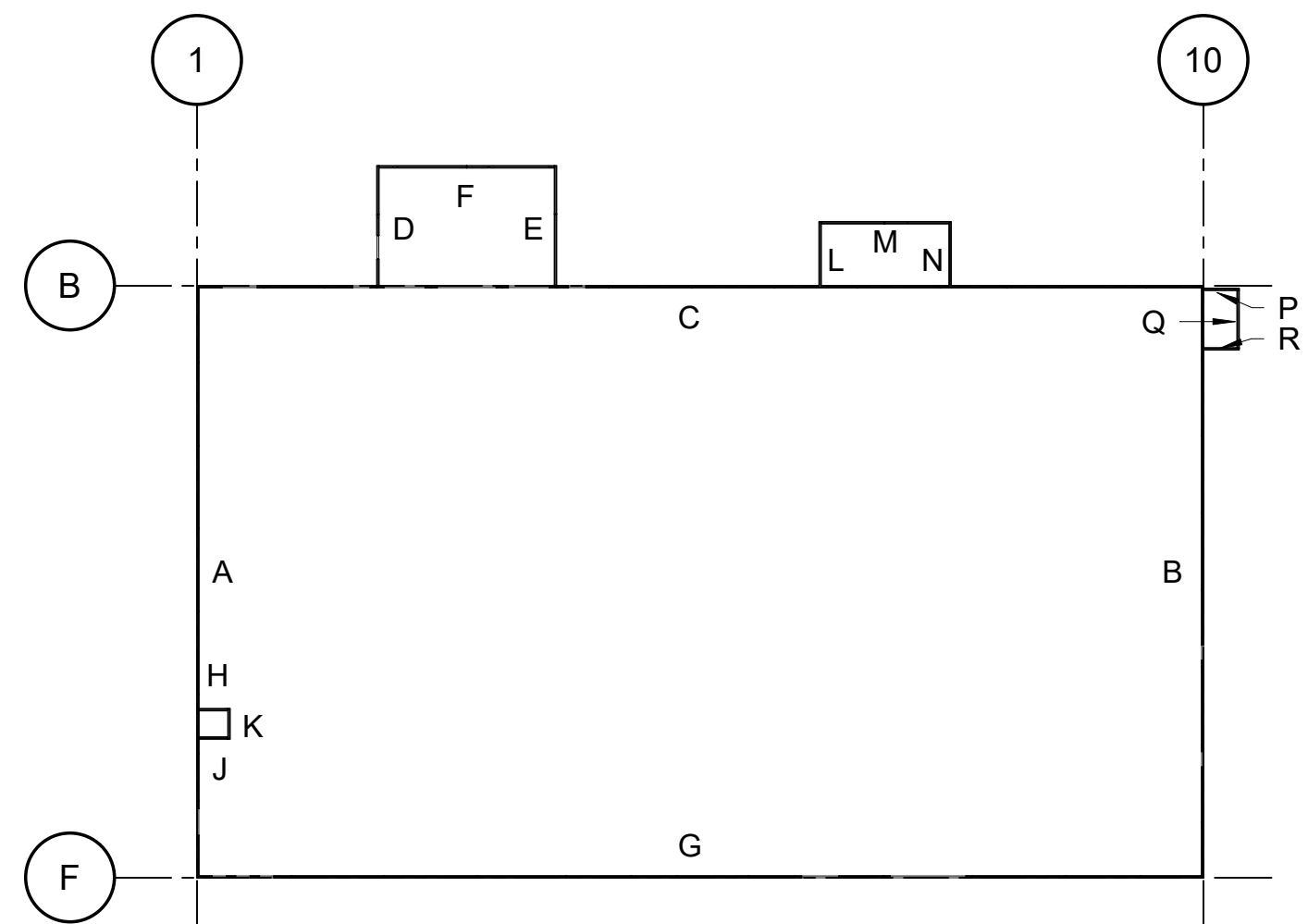
ELEVATION B
SCALE: 1/8" = 1'-0"

TILT-UP ELEVATION NOTES:

- 1) PANEL SHALL BE CONSTRUCTED WITH FAR SIDE DOWN UON.
- 2) SEE DETAIL 1/S6.3 FOR REINFORCING PLACING DETAILS.
- 3) FOR REINFORCING AROUND PANEL OPENINGS SEE DETAIL 6/S6.3 (VERTICAL REINFORCING SHOWN IN DETAIL NOT REQUIRED WHERE COLUMN CAGE REINFORCING IS SPECIFIED ON PANEL ELEVATIONS-COLUMN CAGE TO EXTEND FULL HEIGHT OF PANEL).
- 4) SEE SHEET S6.3 FOR STANDARD PANEL DIMENSIONING METHODOLOGY, SECTIONS & DETAILS.
- 5) PANELS COMPOSED OF NORMAL WEIGHT CONCRETE. SEE GENERAL NOTES FOR STRENGTH REQUIREMENTS.
- 6) SEE ARCH ELEVATIONS FOR PANEL REVEALS AND DEPRESSIONS. SEE SECTION 2/S6.3 FOR REVEAL DETAIL.
- 7) SEE DETAIL 12/S6.3 FOR SLAB DOWELS TO BE CAST INTO PANEL.
- 8) SEE PANEL LIFTING INSERT SHOP DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS TO ACCOMMODATE LIFTING STRESSES, WHICH MAY EXCEED THE REINFORCING SPECIFIED ON ELEVATIONS ABOVE.
- 9) WHERE PANEL REINFORCING IS SPECIFIED EA FACE (EF), EXCEPT @ REINFORCING CAGES w/ TIES, THE VERTICAL REINFORCING OF EA LAYER SHALL PLACED CLOSEST TO THE FACE OF THE PANEL.
- 10) SEE 7/S6.3 FOR INFO AT PANEL JOINTS.
- 11) SEE 5/S6.3 FOR FOR ADDITIONAL REINFORCEMENT @TILT PANEL LIFTING INSERT.
- 12) CONTRACTOR TO COORDINATE CONDUIT LOCATIONS w/ELECTRICAL SUBCONTRACTOR. CONDUITS SHALL BE PLACED PRIOR TO TILT WALL PLACEMENT. NO CUTTING OF REINFORCEMENT.
- 13) CONTRACTOR TO COORDINATE FINAL SCUPPER LOCATIONS WITH ARCH.

TILT-UP KEYNOTE LEGEND

- ① PANEL FIELD REINF. #5 @ 8" CTR'D VERT AND #5 @ 12" CTR'D HORIZ. TYP.
- ② PANEL OPENING REINF. SEE DETAIL 6/S6.3.
- ③ PANEL LEG REINF (FULL HT WALL SEGMENT ≤ 5FT WIDE); #5 VERT @ 4" OC W/ #5 HORIZ @ 12" OC. TYP.
- ④ PANEL LEG REINF (FULL HT WALL SEGMENT AS SHOWN ON ELEV); #5 VERT @ 4" OC EA FACE W/ #4 TIES @ 12" OC.



KEY PLAN

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.

ERIK C. KASPER AR#0017162

**BUSY BEE
ELLISVILLE**

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

PRIOR ISSUES:

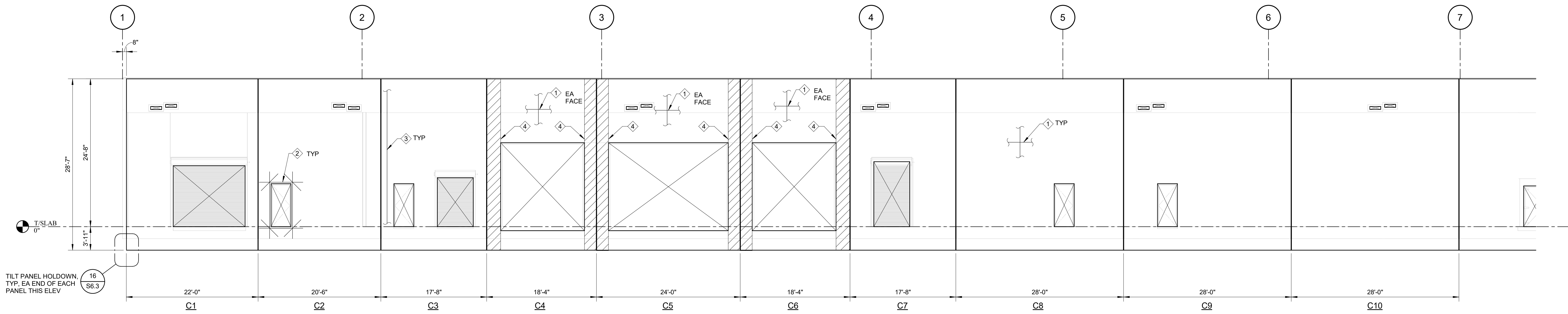
**TILT-UP
ELEVATIONS**

S7.1

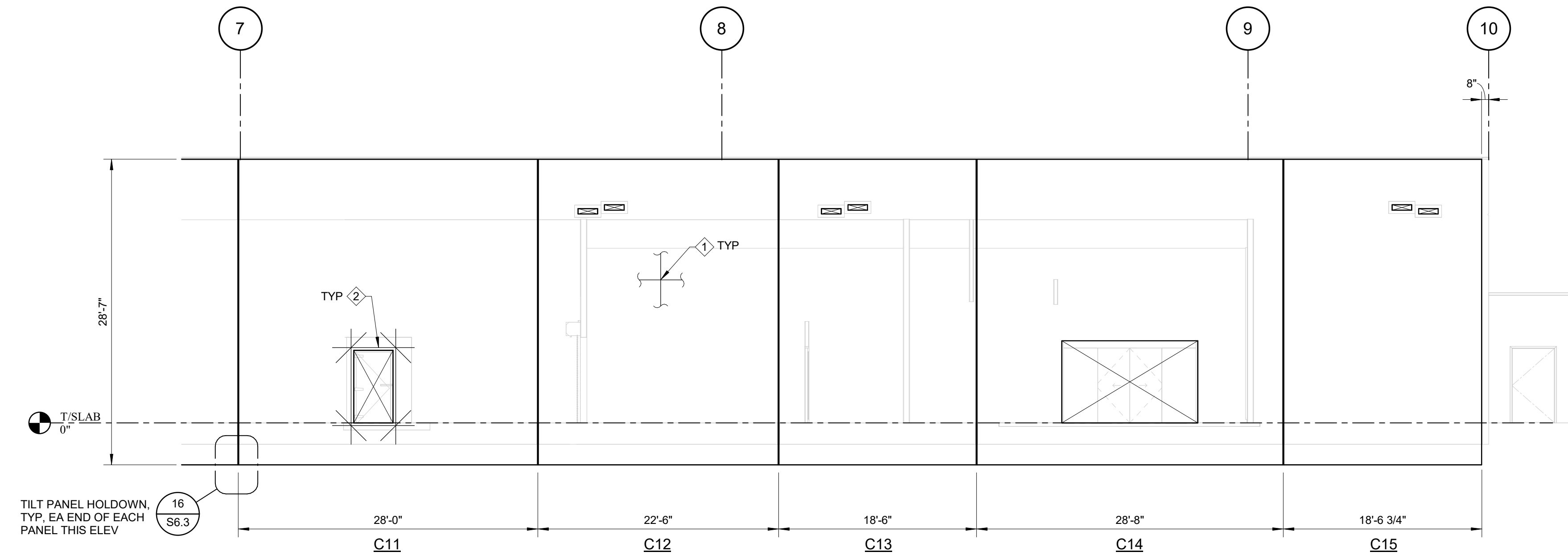
DRAWN BY: ES

CHECKED BY: CF

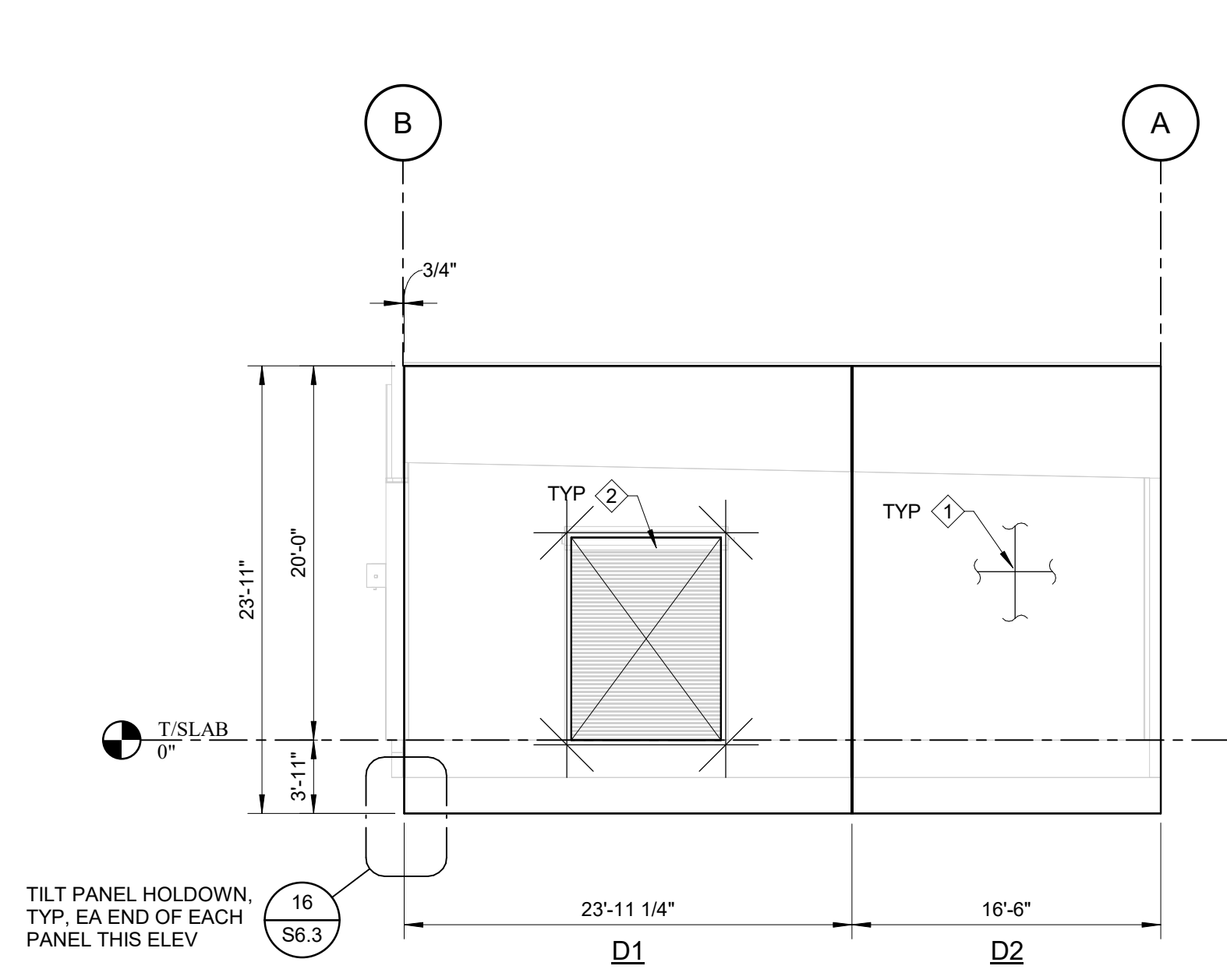
**100%
CONSTRUCTION
DOCUMENTS**



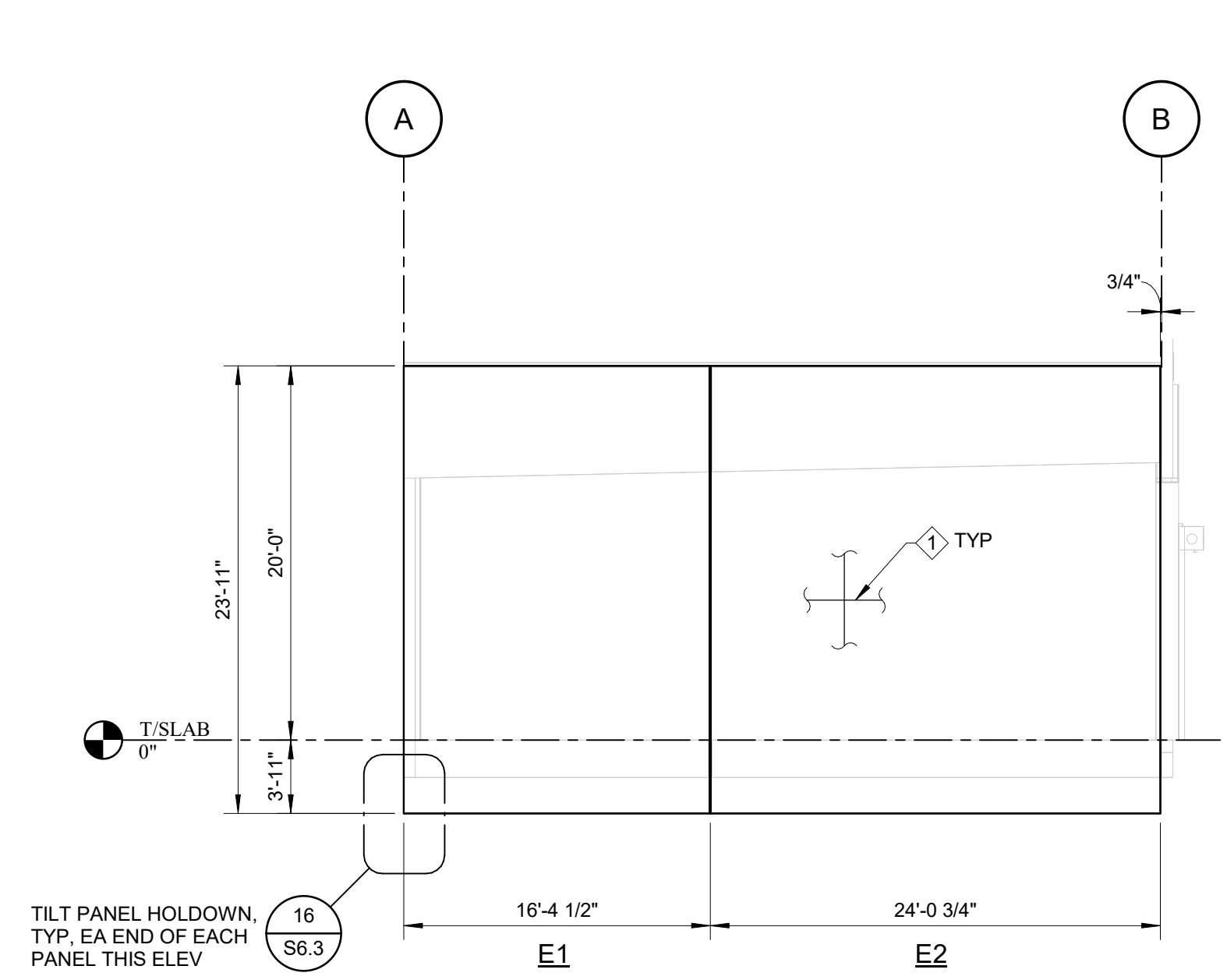
ELEVATION C
SCALE: 1/8" = 1'-0"



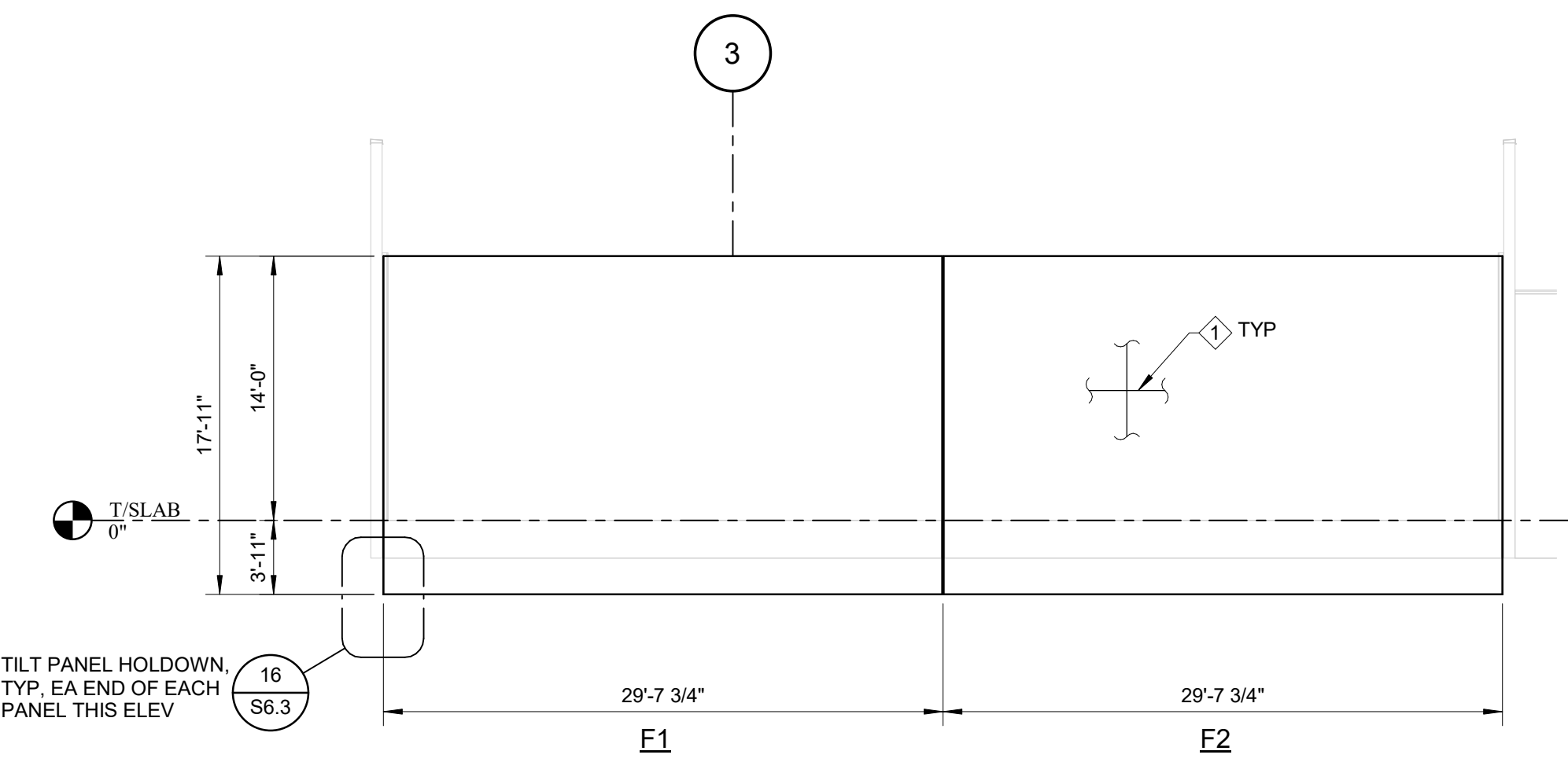
ELEVATION C
SCALE: 1/8" = 1'-0"



ELEVATION D
SCALE: 1/8" = 1'-0"



ELEVATION E
SCALE: 1/8" = 1'-0"



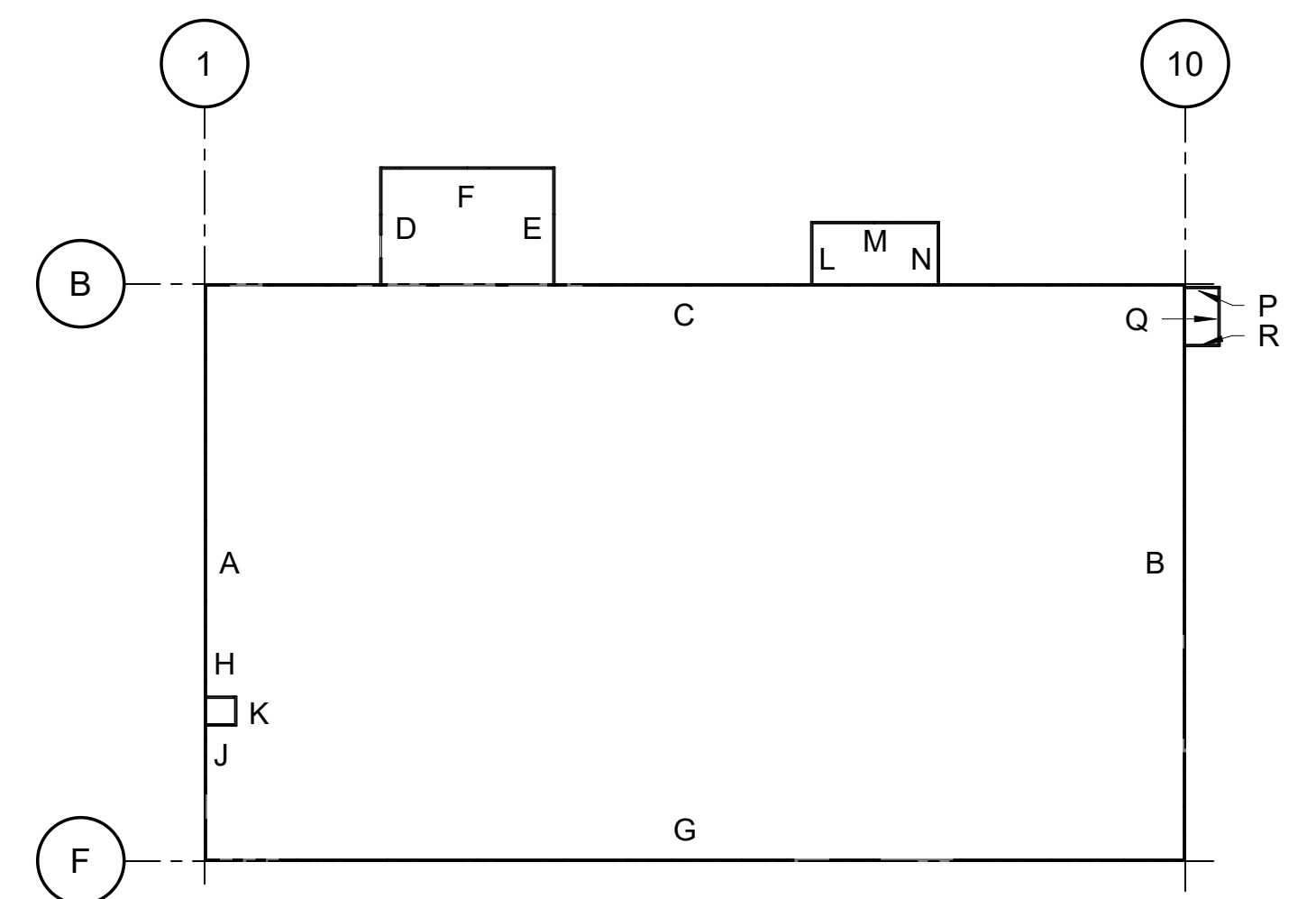
ELEVATION F
SCALE: 1/8" = 1'-0"

TILT-UP ELEVATION NOTES:

- 1) PANEL SHALL BE CONSTRUCTED WITH FAR SIDE DOWN UON.
- 2) SEE DETAIL 1/S6.3 FOR REINFORCING PLACING DETAILS.
- 3) FOR REINFORCING AROUND PANEL OPENINGS SEE DETAIL 6/S6.3 (VERTICAL REINFORCING SHOWN IN DETAIL NOT REQUIRED WHERE COLUMN CAGE REINFORCING IS SPECIFIED ON PANEL ELEVATIONS-COLUMN CAGE TO EXTEND FULL HEIGHT OF PANEL).
- 4) SEE SHEET S6.3 FOR STANDARD PANEL DIMENSIONING METHODOLOGY, SECTIONS & DETAILS.
- 5) PANELS COMPOSED OF NORMAL WEIGHT CONCRETE. SEE GENERAL NOTES FOR STRENGTH REQUIREMENTS.
- 6) SEE ARCH ELEVATIONS FOR PANEL REVEALS AND DEPRESSIONS. SEE SECTION 2/S6.3 FOR REVEAL DETAIL.
- 7) SEE DETAIL 12/S6.3 FOR SLAB DOWELS TO BE CAST INTO PANEL.
- 8) SEE PANEL LIFTING INSERT SHOP DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS TO ACCOMMODATE LIFTING STRESSES, WHICH MAY EXCEED THE REINFORCING SPECIFIED ON ELEVATIONS ABOVE.
- 9) WHERE PANEL REINFORCING IS SPECIFIED EA FACE (EF), EXCEPT @ REINFORCING CAGES w/ TIES, THE VERTICAL REINFORCING OF EA LAYER SHALL PLACED CLOSEST TO THE FACE OF THE PANEL.
- 10) SEE 7/S6.3 FOR INFO AT PANEL JOINTS.
- 11) SEE 5/S6.3 FOR FOR ADDITIONAL REINFORCEMENT @TILT PANEL LIFTING INSERT.
- 12) CONTRACTOR TO COORDINATE CONDUIT LOCATIONS w/ELECTRICAL SUBCONTRACTOR. CONDUITS SHALL BE PLACED PRIOR TO TILT WALL PLACEMENT. NO CUTTING OF REINFORCEMENT.
- 13) CONTRACTOR TO COORDINATE FINAL SCUPPER LOCATIONS WITH ARCH.

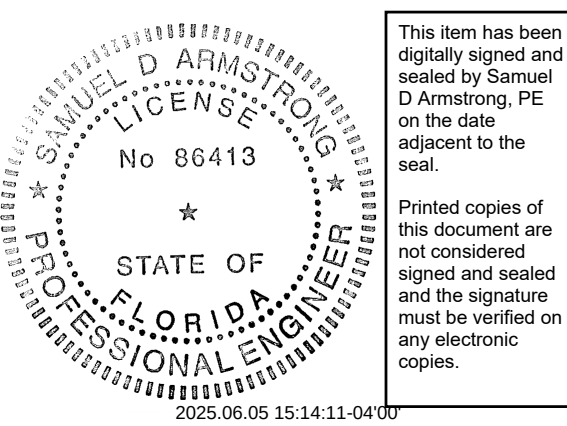
TILT-UP KEYNOTE LEGEND

- ① PANEL FIELD REINF: #5 @ 8" CTR'D VERT AND #5 @ 12" CTR'D HORIZ. TYP.
- ② PANEL OPENING REINF, SEE DETAIL 6/S6.3.
- ③ PANEL LEG REINF (FULL HT WALL SEGMENT ≤ 5FT WIDE); #5 VERT @ 4" OC W/ #5 HORIZ @ 12" OC, TYP.
- ④ PANEL LEG REINF (FULL HT WALL SEGMENT AS SHOWN ON ELEV); #5 VERT @ 4" OC EA FACE W/ #4 TIES @ 12" OC.



KEY PLAN

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.



ERIK C. KASPER AR#0017162

**BUSY BEE
ELLISVILLE**

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

PRIOR ISSUES:

**TILT-UP
ELEVATIONS**

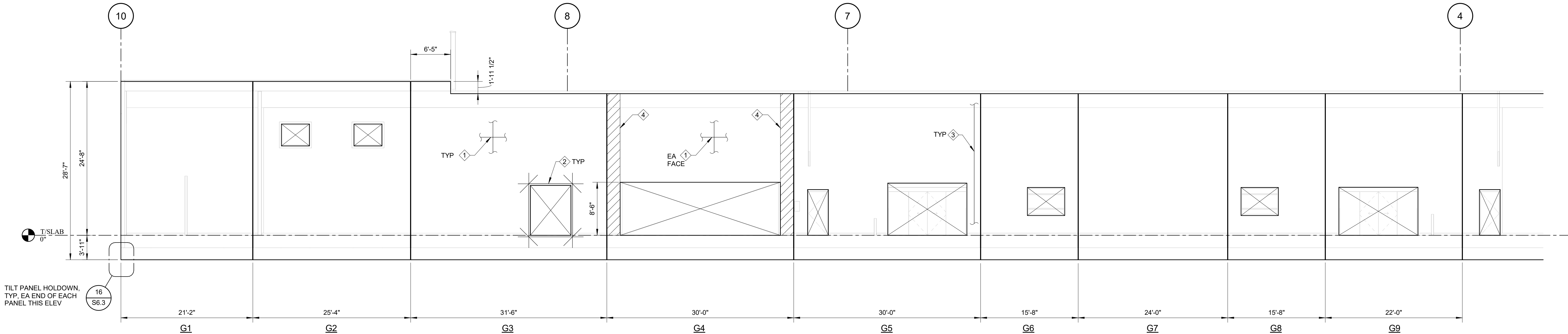
S7.2

DRAWN BY: ES

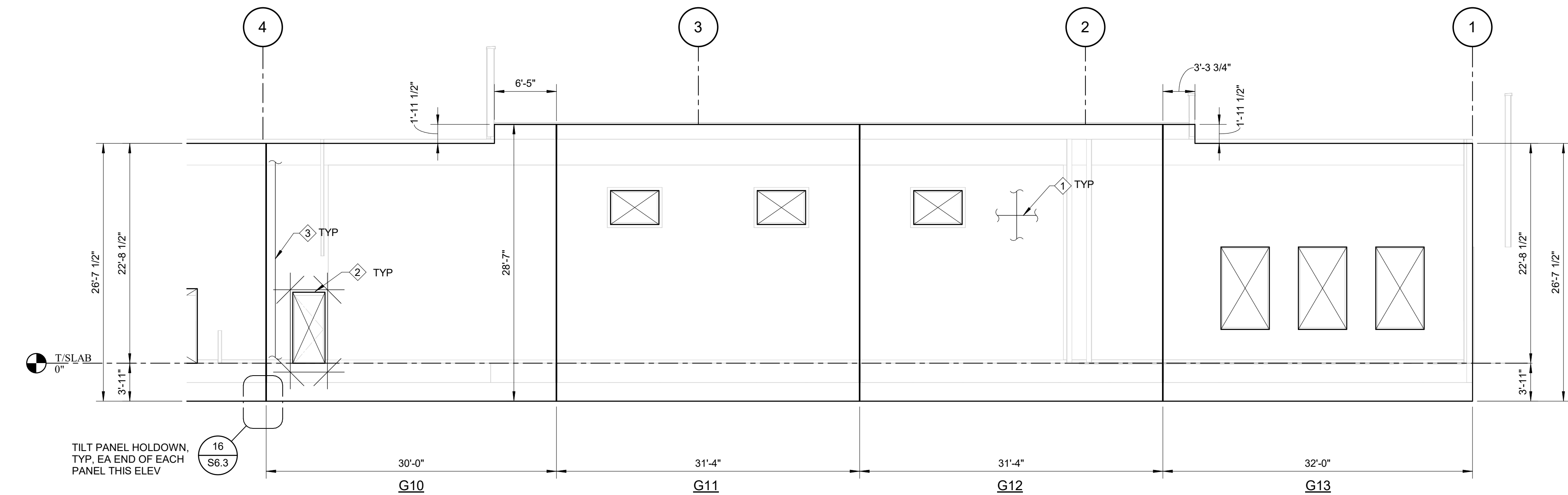
CHECKED BY: CF

**100%
CONSTRUCTION
DOCUMENTS**

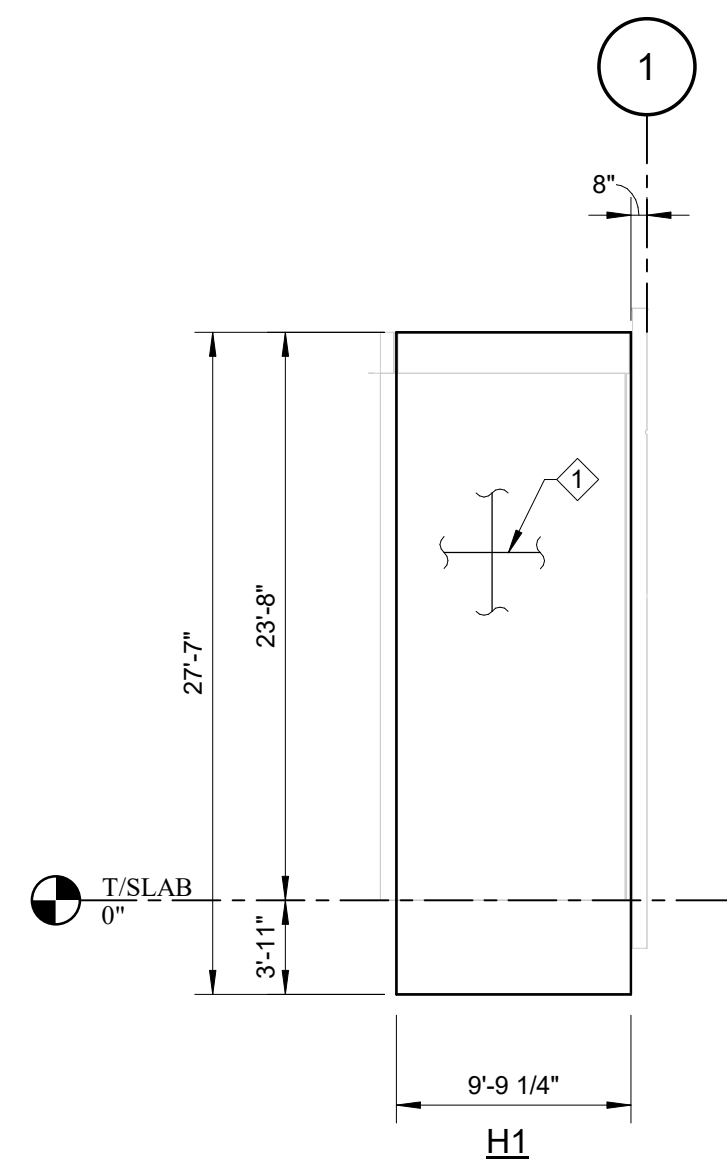
© 2023



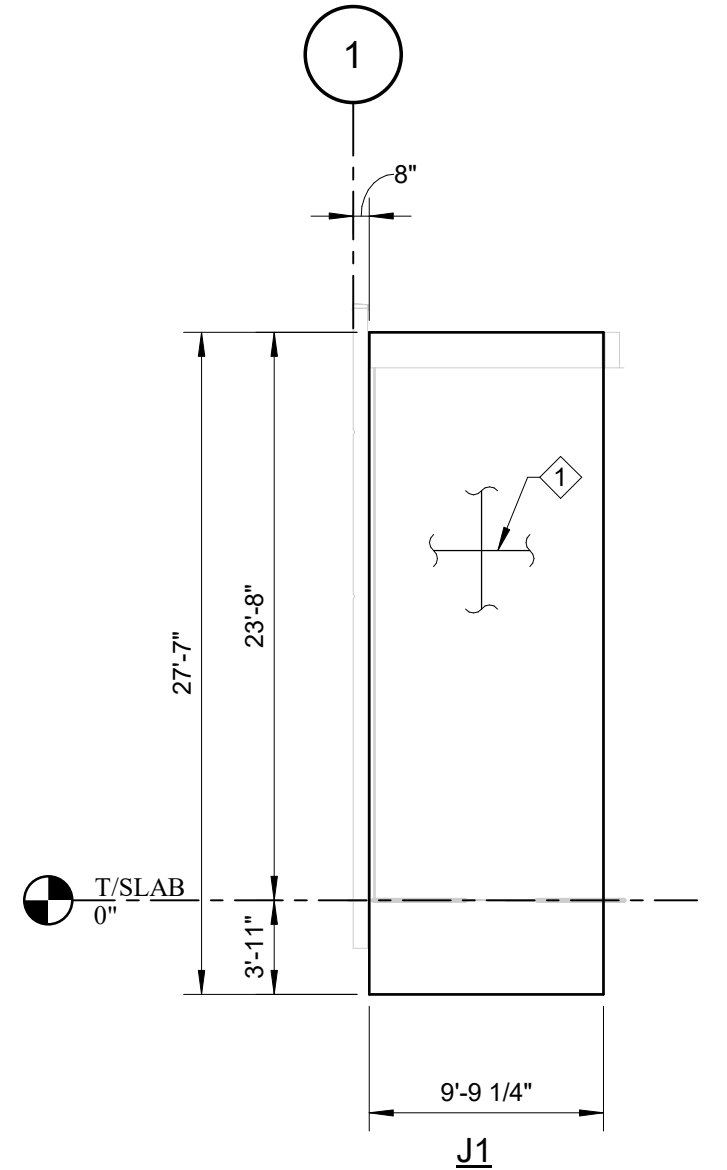
1
S7.3
ELEVATION G
SCALE: 1/8" = 1'-0"



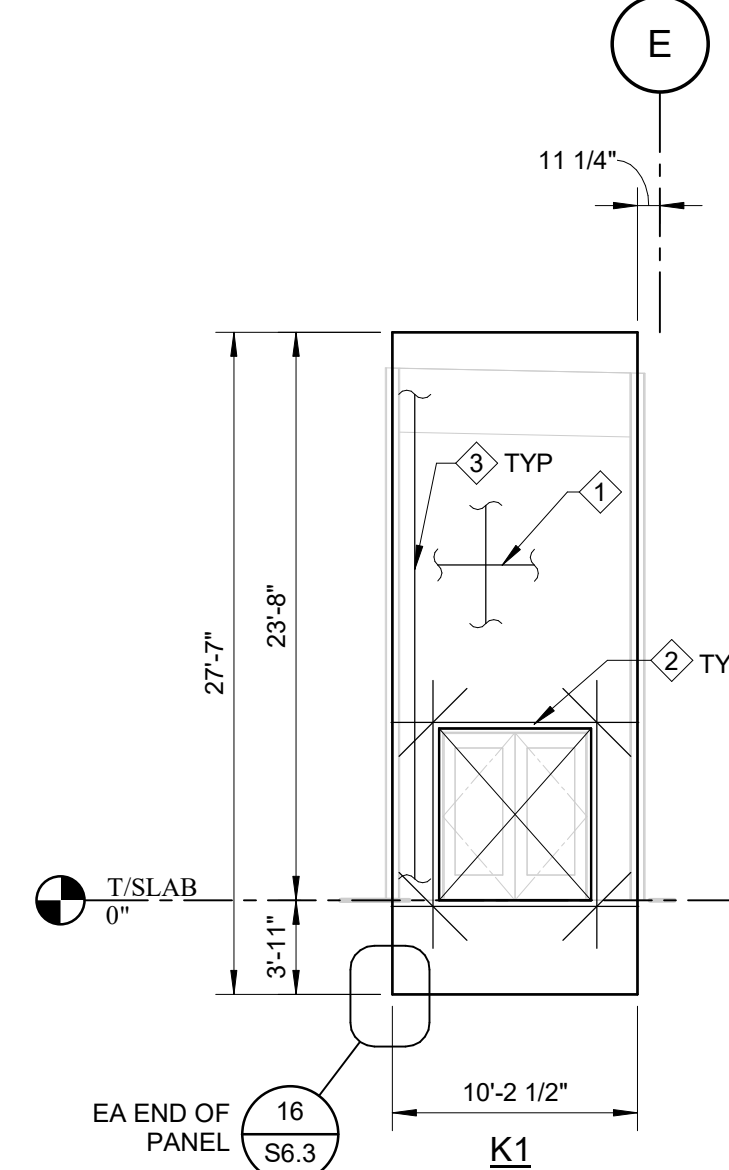
2
S7.3
ELEVATION G
SCALE: 1/8" = 1'-0"



3
S7.3
ELEVATION H
SCALE: 1/8" = 1'-0"



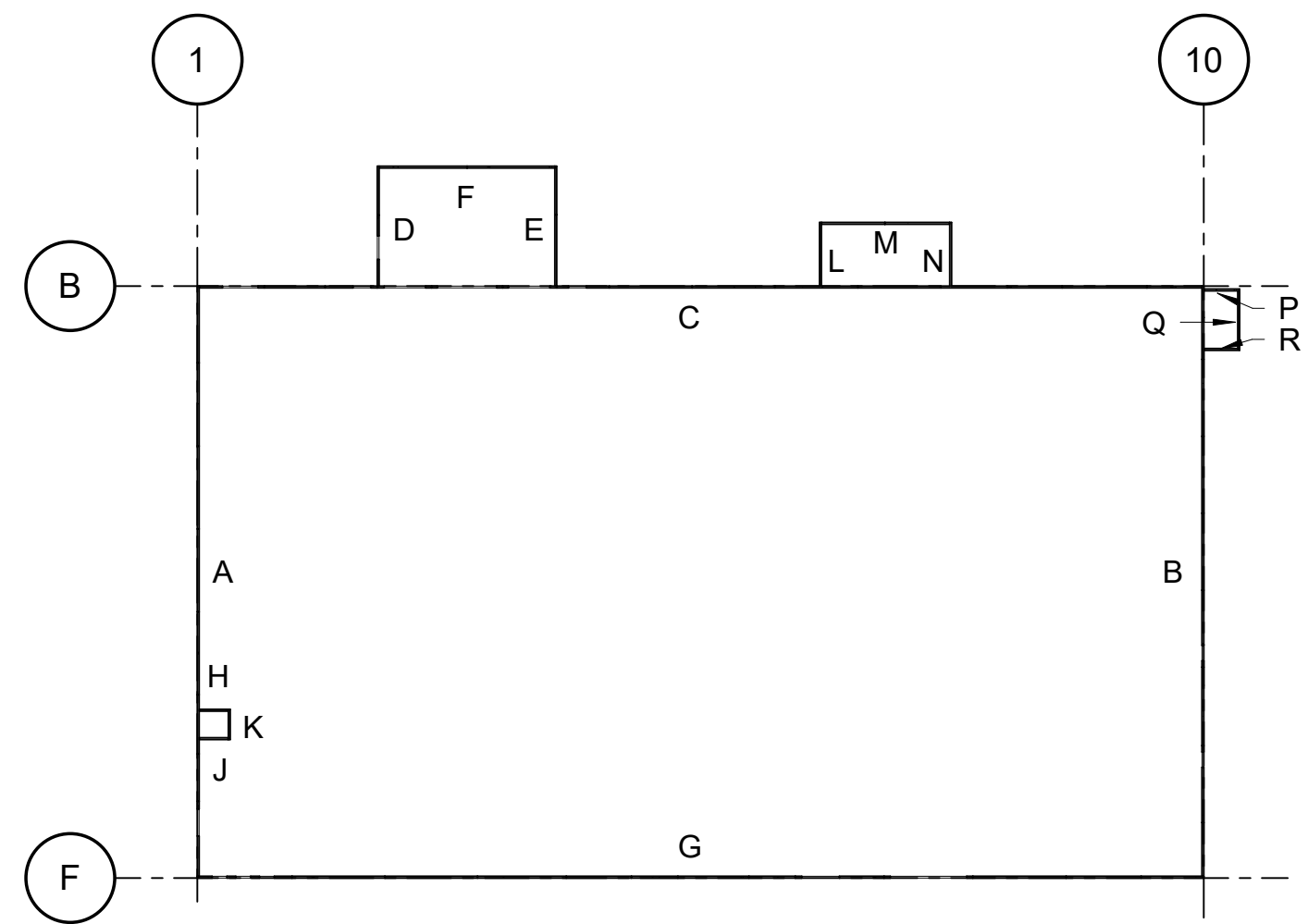
4
S7.3
ELEVATION J
SCALE: 1/8" = 1'-0"



5
S7.3
ELEVATION K
SCALE: 1/8" = 1'-0"

- TILT-UP ELEVATION NOTES:**
- 1) PANEL SHALL BE CONSTRUCTED WITH FAR SIDE DOWN UON.
 - 2) SEE DETAIL 1/S6.3 FOR REINFORCING PLACING DETAILS.
 - 3) FOR REINFORCING AROUND PANEL OPENINGS SEE DETAIL 6/S6.3 (VERTICAL REINFORCING SHOWN IN DETAIL NOT REQUIRED WHERE COLUMN CAGE REINFORCING IS SPECIFIED ON PANEL ELEVATIONS-COLUMN CAGE TO EXTEND FULL HEIGHT OF PANEL).
 - 4) SEE SHEET S6.3 FOR STANDARD PANEL DIMENSIONING METHODOLOGY, SECTIONS & DETAILS.
 - 5) PANELS COMPOSED OF NORMAL WEIGHT CONCRETE. SEE GENERAL NOTES FOR STRENGTH REQUIREMENTS.
 - 6) SEE ARCH ELEVATIONS FOR PANEL REVEALS AND DEPRESSIONS. SEE SECTION 2/S6.3 FOR REVEAL DETAIL.
 - 7) SEE DETAIL 12/S6.3 FOR SLAB DOWELS TO BE CAST INTO PANEL.
 - 8) SEE PANEL LIFTING INSERT SHOP DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS TO ACCOMMODATE LIFTING STRESSES, WHICH MAY EXCEED THE REINFORCING SPECIFIED ON ELEVATIONS ABOVE.
 - 9) WHERE PANEL REINFORCING IS SPECIFIED EA FACE (EF), EXCEPT @ REINFORCING CAGES w/ TIES, THE VERTICAL REINFORCING OF EA LAYER SHALL PLACED CLOSEST TO THE FACE OF THE PANEL.
 - 10) SEE 7/S6.3 FOR INFO AT PANEL JOINTS.
 - 11) SEE 5/S6.3 FOR ADDITIONAL REINFORCEMENT @TILT PANEL LIFTING INSERT.
 - 12) CONTRACTOR TO COORDINATE CONDUIT LOCATIONS w/ELECTRICAL SUBCONTRACTOR. CONDUITS SHALL BE PLACED PRIOR TO TILT WALL PLACEMENT. NO CUTTING OF REINFORCEMENT.
 - 13) CONTRACTOR TO COORDINATE FINAL SCUPPER LOCATIONS WITH ARCH.

- TILT-UP KEYNOTE LEGEND**
- ① PANEL FIELD REINF. #5 @ 8" CTR'D VERT AND #5 @ 12" CTR'D HORIZ. TYP.
 - ② PANEL OPENING REINF. SEE DETAIL 6/S6.3.
 - ③ PANEL LEG REINF (FULL HT WALL SEGMENT ≤ 5FT WIDE); #5 VERT @ 4" OC W/ #5 HORIZ @ 12" OC. TYP.
 - ④ PANEL LEG REINF (FULL HT WALL SEGMENT AS SHOWN ON ELEV); #5 VERT @ 4" OC EA FACE W/ #4 TIES @ 12" OC.



KEY PLAN

TO THE BEST OF THE ENGINEERS
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE MINIMUM BUILDING
CODES FOR THIS PART OF THE WORK
IN ACCORDANCE WITH THE
APPLICABLE FLORIDA STATUTES.

ERIK C. KASPER AR#0017162

**BUSY BEE
ELLISVILLE**

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

PRIOR ISSUES:

**TILT-UP
ELEVATIONS**

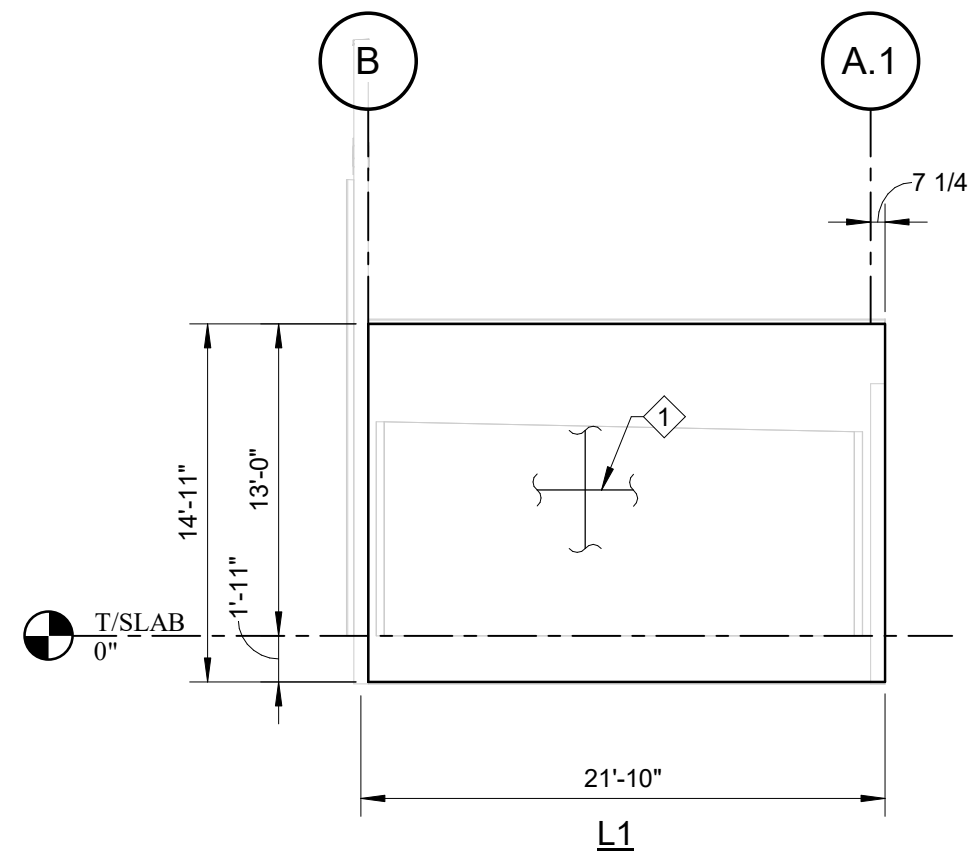
S7.3

DRAWN BY: ES

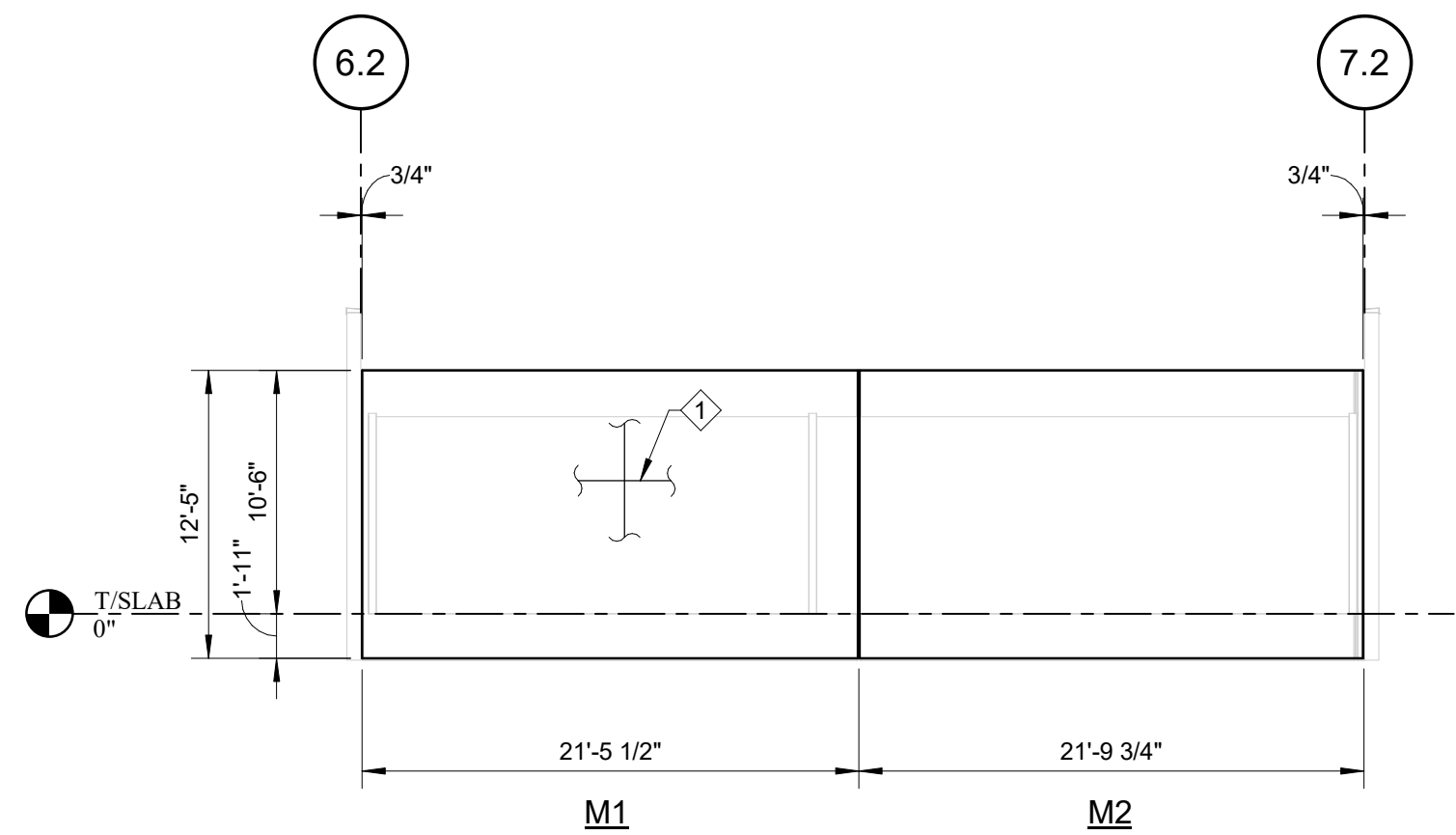
CHECKED BY: CF

**100%
CONSTRUCTION
DOCUMENTS**

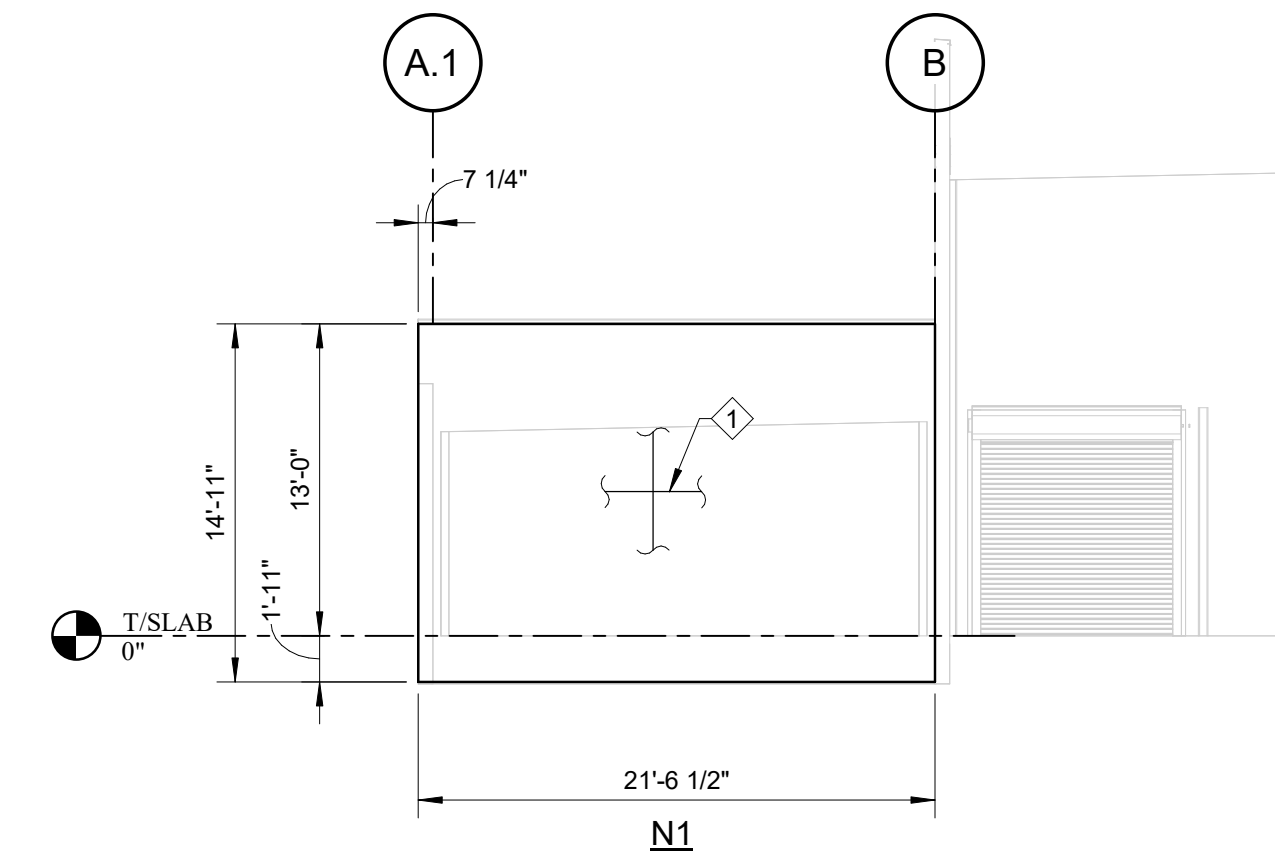
ALL REVISIONS, SPECIFICATIONS, COMPUTER FILES, FIELD DATA AND OTHER DOCUMENTATION PREPARED BY KASPER ARCHITECTS & ASSOCIATES, INC. (KASPER) IS IN FULL COMPLIANCE WITH THE PROFESSIONAL ENGINEERING AND ARCHITECTURAL BOARD OF FLORIDA. NO PART OF THIS DOCUMENT SHALL BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, WITHOUT PERMISSION OF KASPER ARCHITECTS & ASSOCIATES, INC. (KASPER).



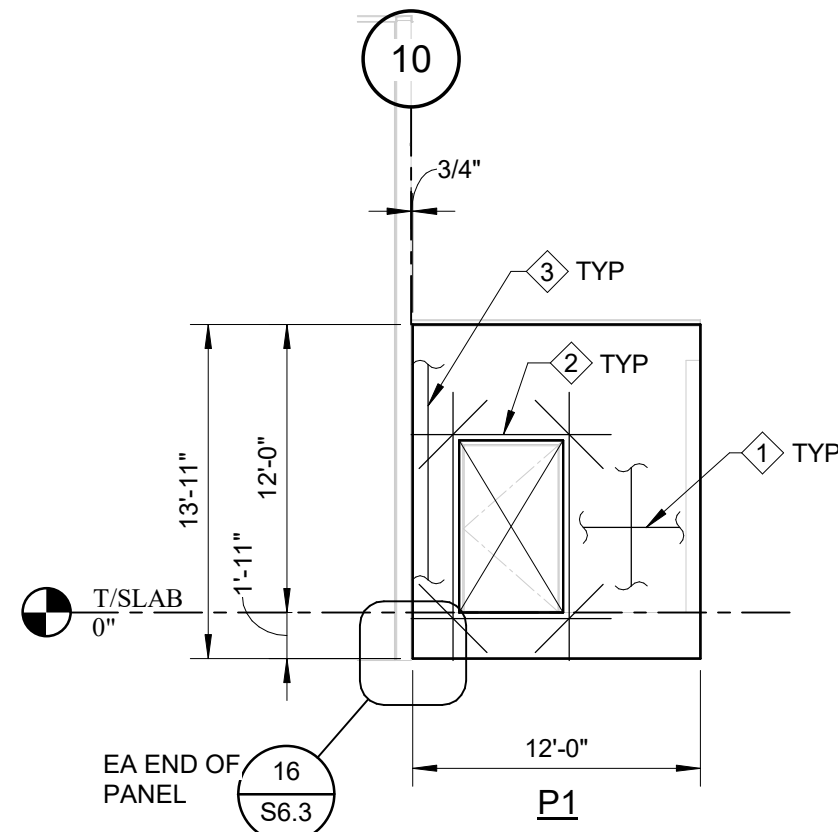
1
S7.4
ELEVATION L
SCALE: 1/8" = 1'-0"



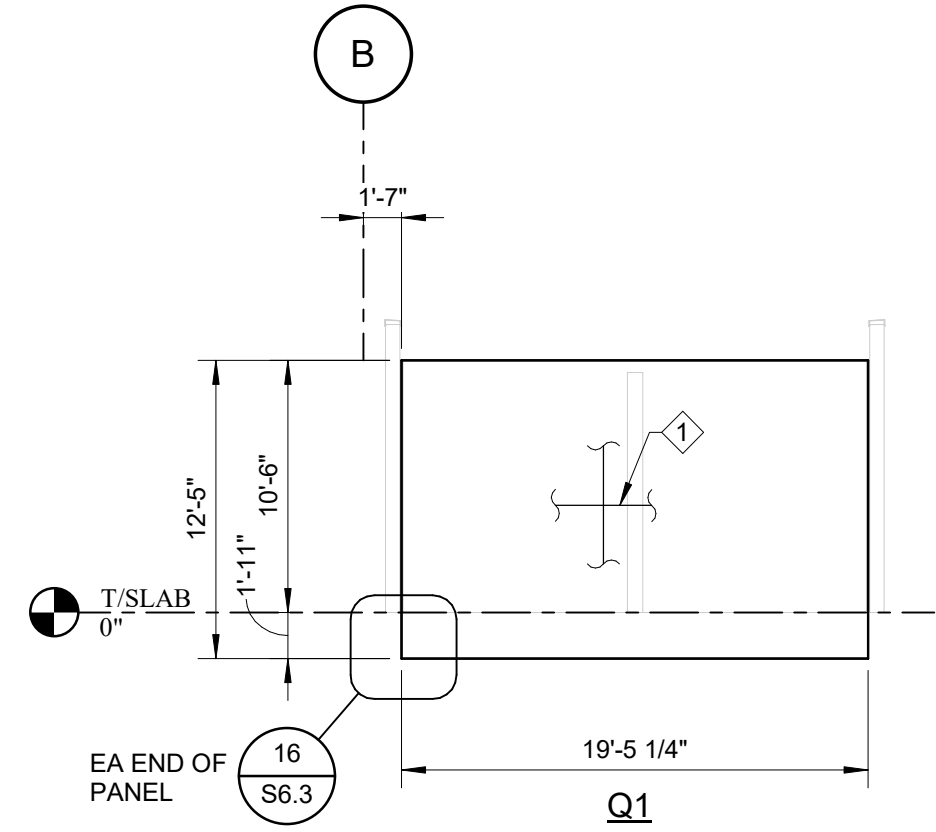
2
S7.4
ELEVATION M
SCALE: 1/8" = 1'-0"



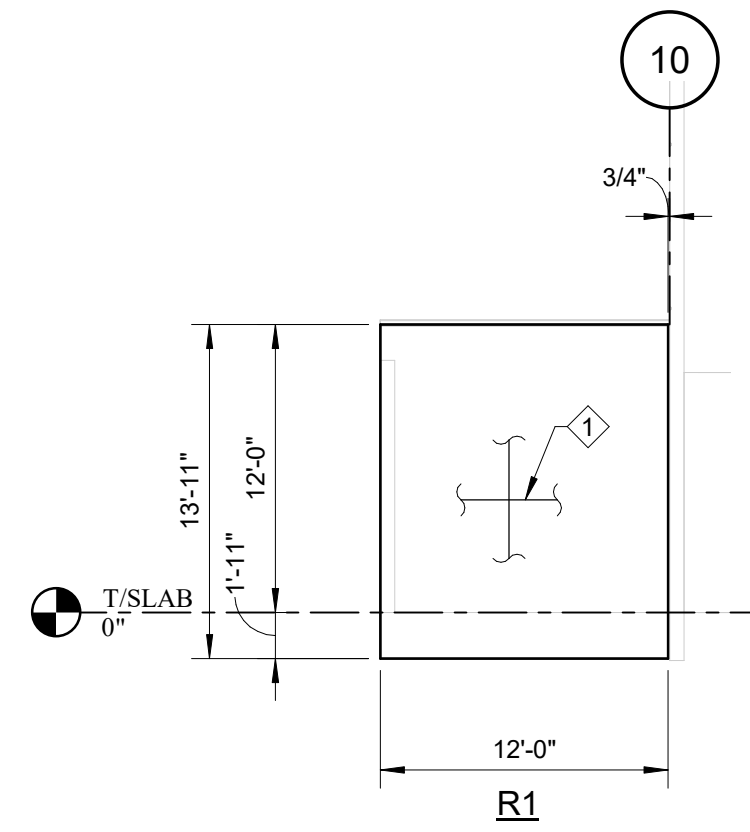
3
S7.4
ELEVATION N
SCALE: 1/8" = 1'-0"



4
S7.4
ELEVATION P
SCALE: 1/8" = 1'-0"



5
S7.4
ELEVATION Q
SCALE: 1/8" = 1'-0"



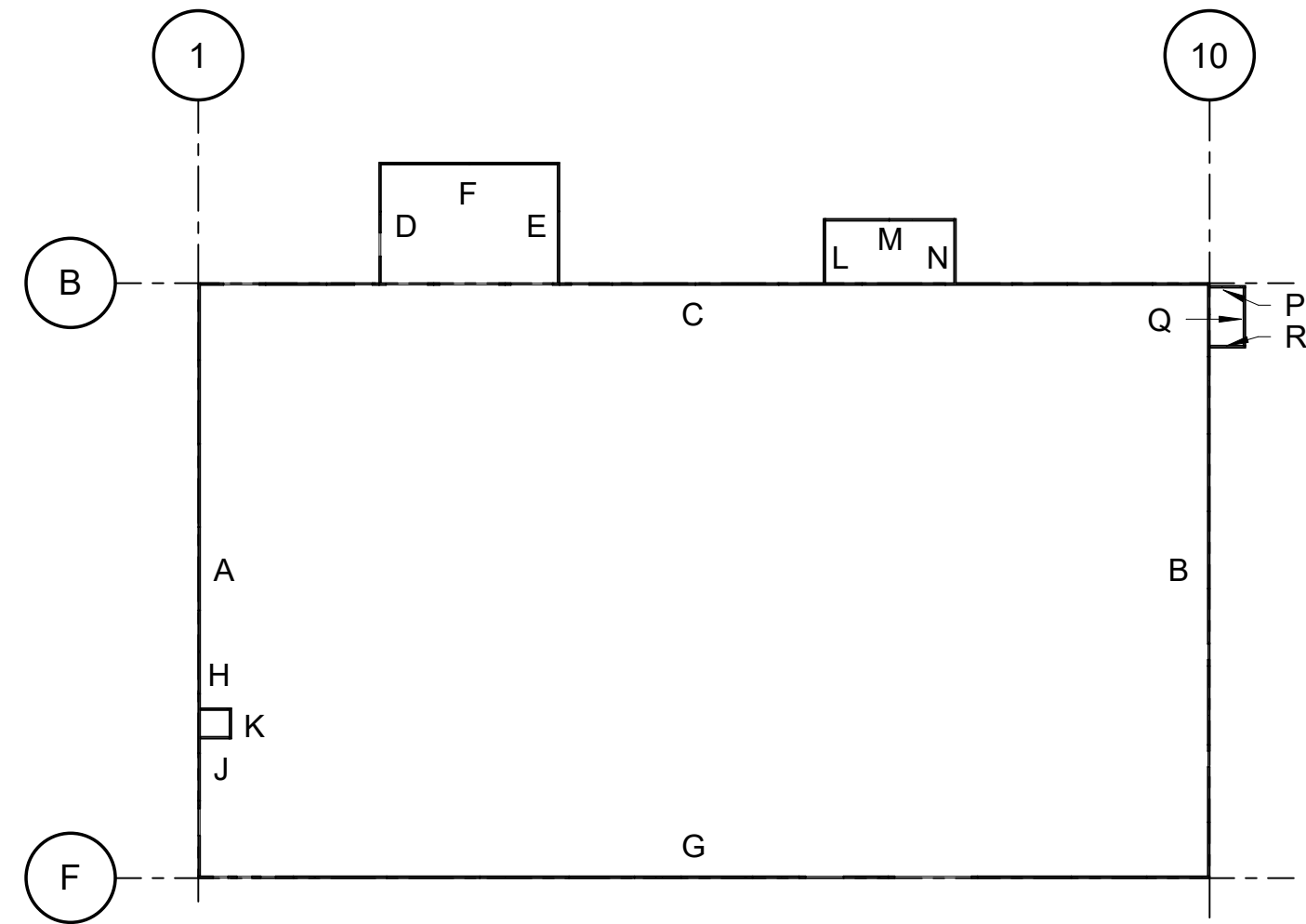
6
S7.4
ELEVATION R
SCALE: 1/8" = 1'-0"

TILT-UP ELEVATION NOTES:

- 1) PANEL SHALL BE CONSTRUCTED WITH FAR SIDE DOWN UON.
- 2) SEE DETAIL 1/S6.3 FOR REINFORCING PLACING DETAILS.
- 3) FOR REINFORCING AROUND PANEL OPENINGS SEE DETAIL 6/S6.3 (VERTICAL REINFORCING SHOWN IN DETAIL NOT REQUIRED WHERE COLUMN CAGE REINFORCING IS SPECIFIED ON PANEL ELEVATIONS-COLUMN CAGE TO EXTEND FULL HEIGHT OF PANEL).
- 4) SEE SHEET S6.3 FOR STANDARD PANEL DIMENSIONING METHODOLOGY, SECTIONS & DETAILS.
- 5) PANELS COMPOSED OF NORMAL WEIGHT CONCRETE. SEE GENERAL NOTES FOR STRENGTH REQUIREMENTS.
- 6) SEE ARCH ELEVATIONS FOR PANEL REVEALS AND DEPRESSIONS. SEE SECTION 2/S6.3 FOR REVEAL DETAIL.
- 7) SEE DETAIL 12/S6.3 FOR SLAB DOWELS TO BE CAST INTO PANEL.
- 8) SEE PANEL LIFTING INSERT SHOP DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS TO ACCOMMODATE LIFTING STRESSES, WHICH MAY EXCEED THE REINFORCING SPECIFIED ON ELEVATIONS ABOVE.
- 9) WHERE PANEL REINFORCING IS SPECIFIED EA FACE (EF), EXCEPT @ REINFORCING CAGES w/ TIES, THE VERTICAL REINFORCING OF EA LAYER SHALL PLACED CLOSEST TO THE FACE OF THE PANEL.
- 10) SEE 7/S6.3 FOR INFO AT PANEL JOINTS.
- 11) SEE 5/S6.3 FOR FOR ADDITIONAL REINFORCEMENT @TILT PANEL LIFTING INSERT.
- 12) CONTRACTOR TO COORDINATE CONDUIT LOCATIONS w/ELECTRICAL SUBCONTRACTOR. CONDUITS SHALL BE PLACED PRIOR TO TILT WALL PLACEMENT. NO CUTTING OF REINFORCEMENT.
- 13) CONTRACTOR TO COORDINATE FINAL SCUPPER LOCATIONS WITH ARCH.

TILT-UP KEYNOTE LEGEND

- ① PANEL FIELD REINF. #5 @ 8" CTR'D VERT AND #5 @ 12" CTR'D HORIZ. TYP.
- ② PANEL OPENING REINF. SEE DETAIL 6/S6.3.
- ③ PANEL LEG REINF. (FULL HT WALL SEGMENT ≤ 5FT WIDE); #5 VERT @ 4" OC W/ #5 HORIZ @ 12" OC. TYP.
- ④ PANEL LEG REINF. (FULL HT WALL SEGMENT AS SHOWN ON ELEV); #5 VERT @ 4" OC EA FACE W/ #4 TIES @ 12" OC.



KEY PLAN

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.

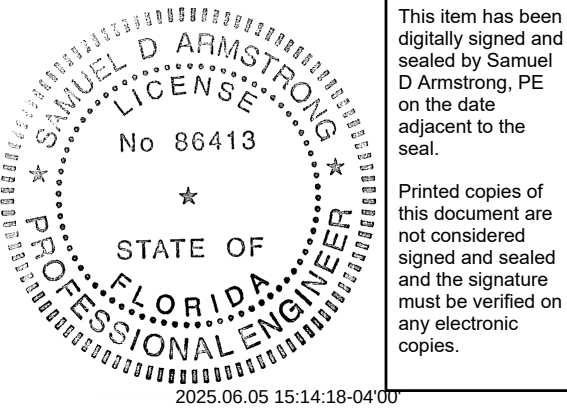


10175 Fortune Parkway, Suite 701
Jacksonville, FL 32256
p 904.683.9201

www.kasperarch.com
FL License No.: AA2600802



www.imegcorp.com
9133 R.G. SKINNER PARKWAY
SUITE 1
JACKSONVILLE, FL 32256
P: 904.483.5200
PROJECT #23000902.00
IMEG RESERVES PROPRIETARY RIGHTS, INCLUDING COPYRIGHTS TO THIS DRAWING AND THE DATA SHOWN THEREON. SAID DRAWINGS AND/OR DATA ARE THE EXCLUSIVE PROPERTY OF IMEG AND SHALL NOT BE USED OR REPRODUCED FOR ANY OTHER PROJECT WITHOUT THE EXPRESS WRITTEN APPROVAL AND PARTICIPATION OF IMEG. ©2025 IMEG CONSULTANTS CORP.



ERIK C. KASPER AR#0017162

**BUSY BEE
ELLISVILLE**

136 SW WEBBS GLN
LAKE CITY, FL 32024

PROJECT NO.: 22024

DATE: 06/02/2025

PRIOR ISSUES:

**TILT-UP
ELEVATIONS**

S7.4

DRAWN BY: ES
CHECKED BY: CF

**100%
CONSTRUCTION
DOCUMENTS**