

10/14/2005

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000023716

APPLICANT TIMOTHY MORGAN PHONE 758.7512

ADDRESS POB 1251 LAKE CITY FL 32056

OWNER TIMOTHY MORGAN PHONE 758.7512

ADDRESS 102 NW TURKEY TROT GLEN LAKE CITY FL 32055

CONTRACTOR TIMOTHY MORGAN PHONE 758.7512

LOCATION OF PROPERTY 41-N TO FIDDLERS LN, TL TO STAGE COACH WAY, TR TO TURKEY TROT GLRN, TR PROPERTY ON R @ CUL-DE-SAC

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 129050.00

HEATED FLOOR AREA 2581.00 TOTAL AREA 3436.00 HEIGHT 24.80 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirements: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO. _____

PARCEL ID 27-2S-16-01771-112 SUBDIVISION COUNTRY LANE ESTATES

LOT 12 BLOCK A PHASE _____ UNIT _____ TOTAL ACRES 5.01

00000843

Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number _____ Applicant/Owner/Contractor *Timothy Morgan*

WAIVER 05-0919-N BLK BLK JTH _____ N _____

Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: NOC ON FILE

1 FOOT ABOVE ROAD

Check # or Cash 100

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
 date/app. by _____ date/app. by _____ date/app. by _____

Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
 date/app. by _____ date/app. by _____ date/app. by _____

Framing _____ Rough-in plumbing above slab and below wood floor _____
 date/app. by _____ date/app. by _____

Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
 date/app. by _____ date/app. by _____ date/app. by _____

Permanent power _____ C.O. Final _____ Culvert _____
 date/app. by _____ date/app. by _____ date/app. by _____

M/H tie downs, blocking, electricity and plumbing _____ Pool _____
 date/app. by _____ date/app. by _____

Reconnection _____ Pump pole _____ Utility Pole _____
 date/app. by _____ date/app. by _____ date/app. by _____

M/H Pole _____ Travel Trailer _____ Re-roof _____
 date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 650.00 CERTIFICATION FEE \$ 17.18 SURCHARGE FEE \$ 17.18

MISC. FEES \$.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$.00 WASTE FEE \$ _____

FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ _____ **TOTAL FEE** 759.36

INSPECTORS OFFICE _____ CLERKS OFFICE CX

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Use Only Application # 0509-57 Date Received 9/19/05 By JW Permit # 843/23716
 Application Approved by - Zoning Official B2K Date 14.10.05 Plans Examiner OK JHT Date 9-27-05
 Flood Zone X P plot Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments Class package CW-099
- BP-098

Phone (386) 758-7512 / 867-0432

Applicants Name Timothy Morgan
 Address P.O. Box 1251 Lake City, FL 32056 Phone SAME
 Owners Name SAME
 911 Address 102 NW Turkey Trot Glen / Lake City, FL 32055 Phone SAME
 Contractors Name SAME
 Address SAME
 Fee Simple Owner Name & Address N/A Mercantile Bank
 Bonding Co. Name & Address N/A
 Architect/Engineer Name & Address Mark Discosway P.O. Box 868 Lake City, FL 32056
 Mortgage Lenders Name & Address Mercantile Bank

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Elec.
 Property ID Number 28-25-16-01771-112 Estimated Cost of Construction 120,000.
 Subdivision Name Country Lane Estates Lot 12 Block A Unit _____ Phase _____

Driving Directions From I-10 & Hwy 41 North travel north approx. (3) three miles to Fiddler Ln. turn left on Fiddler Ln. to NW Stage Coach Way, turn right on Stage Coach Way NW Turkey Trot Glen, turn right property on right to culdesac.

Type of Construction New Construction (Dwelling) Number of Existing Dwellings on Property None
 Total Acreage 5.01 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing
 Actual Distance of Structure from Property Lines - Front 85 ft ✓ Side 208 ft ✓ Side 455 ft ✓ Rear 121 ft ✓
 Total Building Height 24'-8" Number of Stories 1 Heated Floor Area 2581 sqft Roof Pitch 6/12
Porch 336 GARAGE 519 TOTAL 3436

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standard all laws regulating construction in this jurisdiction.

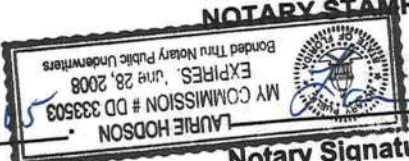
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAY TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

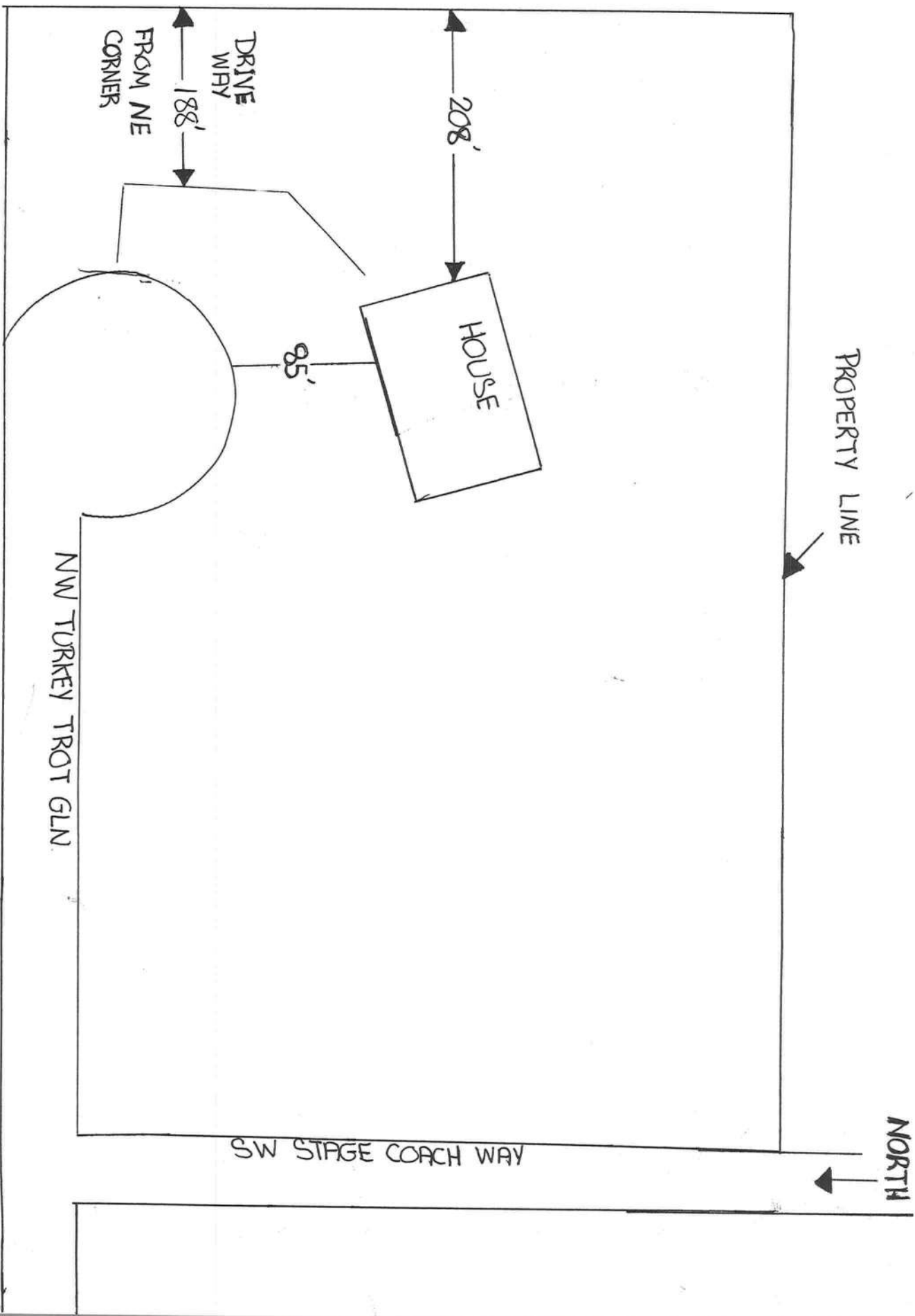
Timothy Morgan
 Owner Builder or Agent (Including Contractor)

Timothy Morgan
 Contractor Signature
 Contractors License Number _____
 Competency Card Number _____

STATE OF FLORIDA
 COUNTY OF COLUMBIA
 Sworn to (or affirmed) and subscribed before me
 this 19 day of 09 2005
 Personally known ✓ or Produced Identification _____



Notary Signature





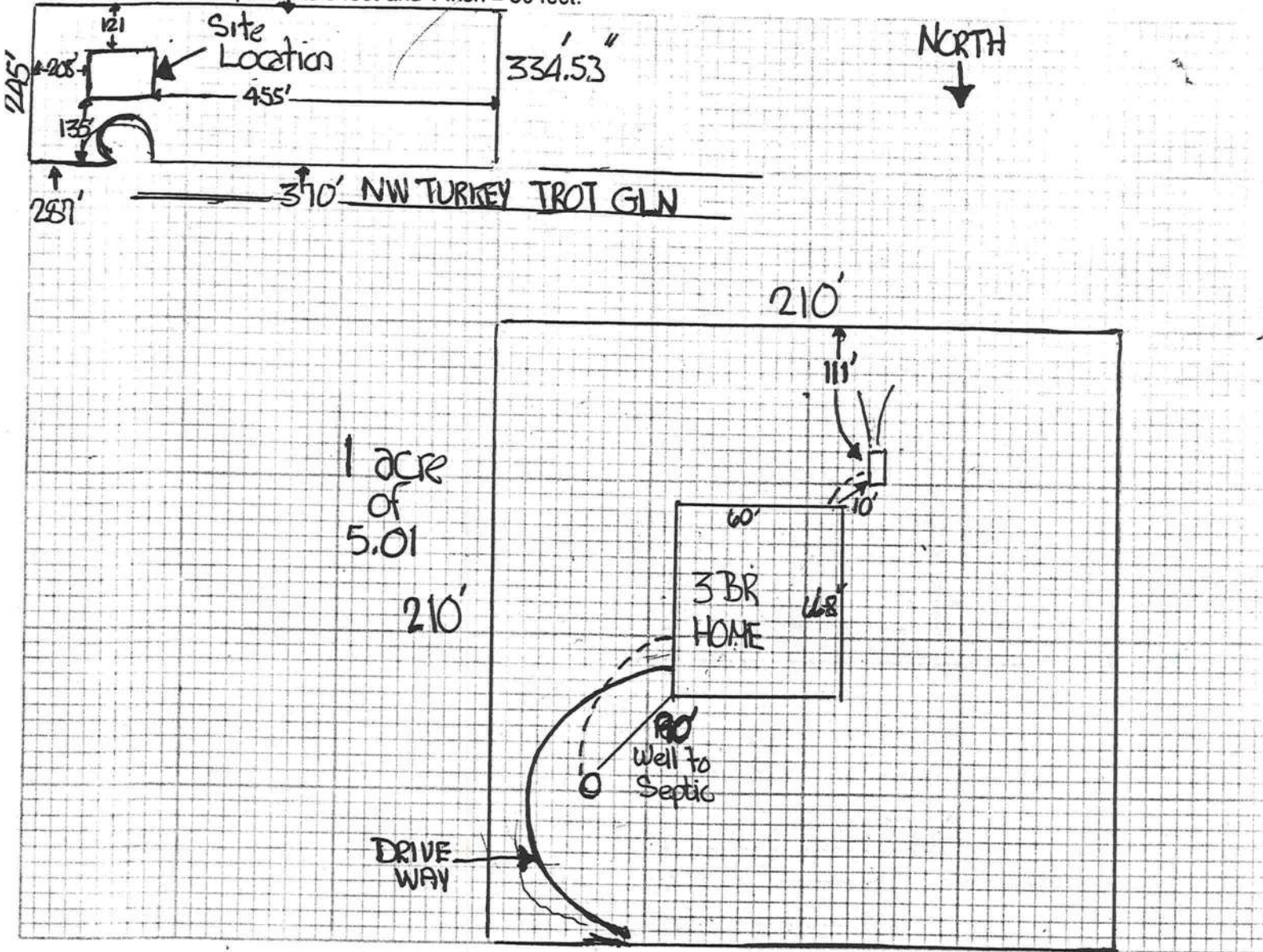
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-0919A

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: _____

Site Plan submitted by: Timothy Morgan Signature
 Plan Approved Not Approved _____
 By Morgan Columbia County Health Department
 Owner Title Owner
 Date 9/15/05 09/07/05

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4015 which may be used)
(Stock Number: 5744-002-4015-6)

DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

- Single Family Dwelling
- Farm Outbuilding
- New Construction

- Two-Family Residence
- Other _____

Addition, Alteration, Modification or other Improvement

NEW CONSTRUCTION OR IMPROVEMENT

I Jimmy Mayan, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

Jimmy Mayan
Signature

Date

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 7-19-05 Building Official/Representative

David Kinnun

Prepared by and return to:
GUY W. NORRIS, ATTORNEY AT LAW
NORRIS & JOHNSON, P.A.
P. O. DRAWER 2349
LAKE CITY, FL 32056-2349

Inst: 2005021451 Date: 09/01/2005 Time: 16:34
mk DC, P. DeWitt Cason, Columbia County B:1057 P:296

Permit No. _____

Tax Folio No. 28-2S-16-01771-112

NOTICE OF COMMENCEMENT

The undersigned hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description and address of Property: Lot 12, Block A, of COUNTRY LANE ESTATES SUBDIVISION, a subdivision as per the plat thereof filed at Plat Book 5, Pages 77, 77A and 77B, of the Public Records of Columbia County, Florida; Northwest Stagecoach Way, Lake City, FL 32055.
2. General description of Improvement: Residential Construction
3. Owner information:
 - a. Name and address: Timothy T. Morgan, P. O. Box 1251, Lake City, FL 32056-1251.
 - b. Interest in property: Fee Simple.
 - c. Name and address of fee simple title holder (if other than owner): _____
4. Contractor name and address: Timothy T. Morgan, P. O. Box 1251, Lake City, FL 32056-1251.
 - a. Phone number: (386) 867-0432.
 - b. Fax number: _____ (optional, if service by fax is acceptable).
5. Surety: _____
 - a. Name and address: _____
 - b. Phone number: _____
 - c. Fax number: _____ (optional, if service by fax is acceptable).
 - d. Amount of bond: \$ _____
6. Lender: Mercantile Bank, 425 22nd Avenue North, St. Petersburg, FL 33704; Attn: Brooke Wilson.
 - a. Phone Number: (727)298-2224.
 - b. Fax Number: _____ (optional, if service by fax is acceptable).
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: _____
 - a. Phone Number: _____
 - b. Fax Number: _____ (optional, if service by fax is acceptable).
8. In addition to himself, Owner designates Guy W. Norris of Norris & Johnson, P.A., P. O. Drawer 2349, Lake City, Florida 32056-2349, to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
 - a. Phone Number: (386)752-7240.
 - b. Fax Number: _____ (optional, if service by fax is acceptable).
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____

Timothy T. Morgan
TIMOTHY T. MORGAN

STATE OF FLORIDA
COUNTY OF COLUMBIA

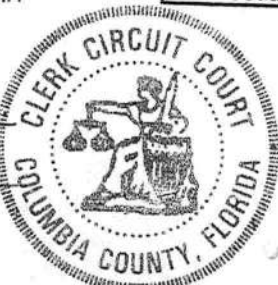
The foregoing instrument was acknowledged before me this 31st day of August, 2005, by TIMOTHY T. MORGAN, who is personally known to me or has produced FLA. DRIVER'S LICENSE as identification.

Notary Public: _____
Sign Diane A. Crews
State of Florida at Large (Seal)
My Commission Expires: _____

OFFICIAL NOTARY SEAL
DIANE A CREWS
NOTARY PUBLIC STATE OF FLORIDA
COMMISSION NO. DD086668
MY COMMISSION EXP. FEB. 9, 2006

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS

By Marcus Kern
Deputy Clerk
Date Sept 1, 2005



WARRANTY DEED
INDIVID TO INDIVID

DOCUMENTARY STAMP 91.00
INTANGIBLE TAX
P. DEWITT CASON, CLERK OF
COURTS, COLUMBIA COUNTY
BY *D. Christie*

This Warranty Deed

Made the 15th day of April A. D. 19 92 by
ROGERS & WOODLAND PARTNERSHIP, a partnership existing under the
laws of Florida whose post office address is P. O. Box 1776, Gainesville, FL 32602

hereinafter called the grantor, to TIMOTHY T. MORGAN and MELODY R. MORGAN,
taking title as husband and wife as tenants by the entireties
whose postoffice address is 510 Country Club Road, Lake city, FL 32055
hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and their heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

Lot(s) 12, Block A of COUNTRY LANE ESTATES SUBDIVISION, a subdivision as per the plat thereof filed at Plat Book 5, pages 77, 77A and 77B, of the Public Records of Columbia County, Florida.

SUBJECT TO any and all outstanding oil, gas and mineral rights of record.

Record and Return To:
P. O. Box 1776
Gainesville, FL 32602

PARCEL #1037400000

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY, FL

92-1308

1992 OCT 28 PM 1:06

RECORD VERIFIED

P. DeWitt Cason
CLERK OF COURTS
COLUMBIA COUNTY, FLORIDA

DEED: Consid. \$12,995.00
Record \$ 6.00
Doc. 78.00
MTG: Record 6.00
Doc. 40.00 43.75
Intg. 25.00

OTHER: 155.00 158.75
Date Pd. 2/28/92
LESS INTG.: 23.00
TOTAL: \$ 130.00 133.75

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that it is lawfully seized of said land in fee simple; that it has good right and lawful authority to sell and convey said land; that it hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever except for taxes accruing subsequent to December 31, 1991, and all restrictions, reservations, outstanding mineral rights, easements and limitations of record, common to the subdivision or as shown on the plat thereof.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Amy R. McKenzie
Amy R. McKenzie
Lisa Story Chapman
Lisa Story Chapman
STATE OF FLORIDA
COUNTY OF ALACHUA

ROGERS & WOODLAND PARTNERSHIP, a partnership existing under the laws of Florida

BY: *Dennis G. Lee*
DENNIS G. LEE, as President of FLORIDA WOODLAND, INC., a partner

I HEREBY CERTIFY that on this day, before me, an officer duly authorized in the State aforesaid and in the County aforesaid to take acknowledgments, personally appeared DENNIS G. LEE, as President of FLORIDA WOODLAND, INC., a partner, personally known to me and who did not take an oath to me known to be the person described in and who executed the foregoing instrument and acknowledged before me that executed the same.

WITNESS my hand and official seal in the County and State last aforesaid this 15th day of April A.D. 19 92.

Amy R. McKenzie
Notary Public, State of Florida
Amy R. McKenzie
My Commission Expires:
Commission #CC 047160

BK 0766 PG 1691

AS TO FORM ONLY
This Instrument Prepared By
H. EDWARD GARVIN
P.O. Box 23831
Gainesville, Florida 32602

OFFICIAL RECORDS

"OFFICIAL NOTARY SEAL"
AMY R. MCKENZIE
NOTARY PUBLIC STATE OF FLORIDA
My Commission Exp. OCT. 22, 1994

Name:
Address:
Instrument Recorded:
Name:
Address:
Property Appraisers Parcel Identification
File Number(s):
Ante(s) S.S. # (s)

BR 0854 PG0786

OFFICIAL RECORDS

98-03187

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY, FL.

1998 MAR -3 AM 11:37

RECORD VERIFIED
P. DeWitt Cason
CLERK OF COURTS
COLUMBIA COUNTY, FLORIDA
BY MCK D.C.

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

This Quit Claim Deed, Executed the 3rd day of March, 1998, by Melody R. Morgan, Single and Timothy T. Morgan, first party, to Timothy T. Morgan whose post office address is P.O. Box 1257 Lake City, FL 32056, second party.

(Wherever used herein the terms "first party" and "second party" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth, That the first party, for and in consideration of the sum of \$ 10. in hand paid by the said second party, the receipt whereof is hereby acknowledged, does hereby remise, release, and quit claim unto the second party forever, all the right, title, interest, claim and demand which the said first party has in and to the following described lot, piece or parcel of land, situate, lying and being in the County of Columbia, State of Florida, to-wit:

Lot 12, Block A, Country Lane Estates

Documentary Stamp .70
Intangible Tax 0
P. DeWitt Cason
Clerk of Court
BY MCK D.C.

To Have and to Hold The same together with all and singular the appurtenances thereunto belonging or in anywise appertaining, and all the estate, right, title, interest, lien, equity and claim whatsoever of the said first party, either in law or equity to the only proper use, benefit and behoof of the said second party forever.

In Witness Whereof, the said first party has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in the presence of:

Sonia A. Markham
Witness Signature (as to first Grantor)
Sonia A. Markham
Printed Name

Patricia A. Perry
Witness Signature (as to first Grantor)
Patricia A. Perry
Printed Name

Sonia A. Markham
Witness Signature (as to Co-Grantor, if any)
Sonia A. Markham
Printed Name

Sonia A. Markham
Witness Signature (as to Co-Grantor, if any)
Sonia A. Markham
Printed Name

STATE OF Florida

COUNTY OF Columbia

known to me to be the person 5 described in and who executed the foregoing instrument, who acknowledged before me that they executed the same, and an oath was not taken. (Check one:) Said person(s) is/are personally known to me. Said person(s) provided the following type of identification: Florida Drivers License

Melody R. Morgan
Grantor Signature
Melody R. Morgan
Printed Name

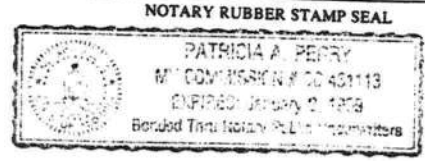
Post Office Address

Timothy T. Morgan
Co-Grantor Signature, (if any)
Timothy T. Morgan
Printed Name

Post Office Address

I hereby Certify that on this day, before me, an officer duly authorized to administer oaths and take acknowledgments, personally appeared Melody and Timothy Morgan

known to me to be the person 5 described in and who executed the foregoing instrument, who acknowledged before me that they executed the same, and an oath was not taken. (Check one:) Said person(s) is/are personally known to me. Said person(s) provided the following type of identification: Florida Drivers License



Witness my hand and official seal in the County and State last aforesaid this 3rd day of March, A.D. 1998
Patricia A. Perry
Notary Signature
Patricia A. Perry
Printed Name

THIS INSTRUMENT WAS PREPARED BY:
H. Edward Garvin, Esquire
P.O. Box 358041
Gainesville, FL 32635
Florida Bar No. 749753
(as to form only)

RECORD & RETURN TO:
Rogers & Woodland Partnership
P.O. Box 357845
Gainesville, FL 32635

Recorder's Use Only

Inst:2004009074 Date:04/22/2004 Time:10:30
 DC, P. Dewitt Cason, Columbia County B:1013 P:436

SATISFACTION OF MORTGAGE

KNOW ALL MEN BY THESE PRESENTS:

That **ROGERS & WOODLAND PARTNERSHIP, LLP**, a partnership existing under the laws of Florida, whose address is P.O. Box 357845, Gainesville, Florida 32635, the owner and holder of a certain mortgage given by **TIMOTHY T. MORGAN** and **MELODY R. MORGAN**, his wife, whose address is Post Office Box 1251, Lake City, Florida, 32056-1251, to **ROGERS & WOODLAND PARTNERSHIP**, a Florida partnership, bearing date the 17th day of February, 1992, recorded in O.R. Book 766, page 1692, of the Public Records of Columbia County, State of Florida; given to secure the sum of **Twelve Thousand Five Hundred Dollars (\$**12,500.00**)**, evidenced by that certain note, upon the following described property, situate, lying and being in Columbia County, State of Florida, to wit:

Lot 12, Block A of **COUNTRY LANE ESTATES SUBDIVISION**, a subdivision as per the plat thereof filed at Plat Book 5, pages 77, 77A and 77B, of the Public Records of Columbia County, Florida

Said mortgage was assumed by **TIMOTHY T. MORGAN** by virtue of that certain Quit Claim Deed dated 03-03-98 and filed 03-03-98 in O.R. Book 854, page 786 of the Public Records of Columbia County, Florida

has received full payment of said indebtedness, and does hereby acknowledge satisfaction of said mortgage, and hereby directs the Clerk of said Circuit Court to cancel the same of record.

WITNESS my hand and seal this 14th day of April, 2004.

Signed, sealed and delivered in our presence as witnesses:

Barbara Rea
Barbara Rea

Lisa Davies
Lisa Davies

ROGERS & WOODLAND PARTNERSHIP, LLP
a partnership existing under the laws of Florida

BY: Dennis G. Lee
DENNIS G. LEE, President of **FLORIDA WOODLAND, INC.**, a partner

STATE OF FLORIDA
COUNTY OF ALACHUA

The foregoing instrument was acknowledged before me this 14th day of April, 2004, by **DENNIS G. LEE**, as President of **FLORIDA WOODLAND, INC.**, a Florida corporation, and partner of **ROGERS & WOODLAND PARTNERSHIP, LLP**. He is personally known to me.

Lisa Davies
Lisa Davies
Notary Public, State of Florida.
Commission #DD182383
My Commission Expires: 02/20/07

LISA DAVIES
Notary Public, State of Florida
My comm. exp. Feb. 20, 2007
.Comm. No. DD 182383

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: September 8, 2005

ENHANCED 9-1-1 ADDRESS:

102 NW TURKEY TROT GLN (LAKE CITY, FL 32055)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS: _____

PROPERTY APPRAISER PARCEL NUMBER: 28-2S-16-01771-112

Other Contact Phone Number (If any): _____

Building Permit Number (If known): _____

Remarks: LOT 12 BLOCK A COUNTRY LANE ESTATES S/D

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

Application # 0509-57

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL
OWNERS

PHONE (386) 752-1854
FAX (386) 755-7022
904 NW MAIN BLVD.
LAKE CITY, FLORIDA 32055

From: Hall's Pump & Well Service, Inc.
Date: August 2, 2005
Re: New well

4" well with a 1hp submersible
pump & 81 gal diaphragm type
tank to 100 ft., including permit
Over 100 ft. of drilling is
\$10.00 per foot **\$2,590.00**

Thank you for considering our company to do this service for you. If there are any questions please feel free to call us at the office.

Sincerely,

Donald D. Hall
VP

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL, PERMIT #:

BASE				AS-BUILT			
CEILING TYPES Area X BSPM = Points				Type	R-Value	Area X SPM X SCM =	Points
Under Attic	2581.0	1.73	4465.1	Under Attic	30.0	2881.0 1.73 X 1.00	4984.1
Base Total:	2581.0		4465.1	As-Built Total:		2881.0	4984.1
FLOOR TYPES Area X BSPM = Points				Type	R-Value	Area X SPM =	Points
Slab	236.0(p)	-37.0	-8732.0	Slab-On-Grade Edge Insulation	0.0	236.0(p) -41.20	-9723.2
Raised	0.0	0.00	0.0				
Base Total:			-8732.0	As-Built Total:		236.0	-9723.2
INFILTRATION Area X BSPM = Points				Area X SPM = Points			
	2581.0	10.21	26352.0		2581.0	10.21	26352.0
Summer Base Points: 35342.1				Summer As-Built Points: 37476.9			
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier X Credit Multiplier = Cooling Points
35342.1	0.4266		15077.0	^{37476.9} 37476.9	1.00	(1.090 x 1.147 x 1.00)	^{0.263} 0.263 ^{1.000} 1.000 ^{12301.2} 12301.2

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL, PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X SPM X SOF = Points			
.18	2581.0	20.04	9310.2	Double, Clear	SE	1.5	7.0	18.0	42.75	0.92	706.9
				Double, Clear	SE	1.5	4.0	6.0	42.75	0.76	196.2
				Double, Clear	SW	99.0	8.0	10.0	40.16	0.37	147.8
				Double, Clear	SE	4.0	7.0	24.0	42.75	0.63	644.5
				Double, Clear	S	8.0	7.0	18.0	35.87	0.50	322.9
				Double, Clear	SW	99.0	7.0	18.0	40.16	0.37	266.0
				Double, Clear	SE	11.0	8.0	40.0	42.75	0.44	758.2
				Double, Clear	E	3.0	8.0	20.0	42.06	0.82	686.1
				Double, Clear	SE	1.5	7.0	24.0	42.75	0.92	942.5
				Double, Clear	SW	1.5	7.0	36.0	40.16	0.92	1329.6
				Double, Clear	SW	1.5	5.0	8.0	40.16	0.84	268.9
				Double, Clear	SW	1.5	5.0	16.0	40.16	0.84	537.8
				Double, Clear	NW	1.5	5.0	16.0	25.97	0.89	371.8
				Double, Clear	W	1.5	9.0	18.0	38.52	0.97	672.9
				Double, Clear	NW	1.5	9.0	24.0	25.97	0.97	607.2
				Double, Clear	N	1.5	9.0	18.0	19.20	0.98	337.2
				Double, Clear	W	1.5	2.5	6.0	38.52	0.67	155.1
				Double, Clear	NW	1.5	2.5	8.0	25.97	0.74	154.0
				Double, Clear	N	1.5	2.5	6.0	19.20	0.80	91.8
				Double, Clear	NW	8.0	10.0	20.0	25.97	0.68	352.1
				Double, Clear	NW	8.0	3.0	10.0	25.97	0.52	134.7
				Double, Clear	NW	1.5	9.0	36.0	25.97	0.97	910.9
				Double, Clear	NW	1.5	2.5	12.0	25.97	0.74	231.0
				Double, Clear	NE	1.5	7.0	54.0	29.56	0.94	1506.6
				Double, Clear	NE	1.5	3.0	8.0	29.56	0.76	179.4
				As-Built Total:				474.0	12512.1		
WALL TYPES Area X BSPM = Points				Type		R-Value		Area X SPM = Points			
Adjacent	278.0	0.70	194.6	Frame, Wood, Exterior		13.0		1856.0		1.50 2784.0	
Exterior	1856.0	1.70	3155.2	Frame, Wood, Adjacent		13.0		278.0		0.60 166.8	
Base Total:	2134.0		3349.8	As-Built Total:				2134.0		2950.8	
DOOR TYPES Area X BSPM = Points				Type		Area X SPM = Points					
Adjacent	20.0	2.40	48.0	Exterior Insulated				90.0		4.10 369.0	
Exterior	90.0	6.10	549.0	Adjacent Insulated				20.0		1.60 32.0	
Base Total:	110.0		597.0	As-Built Total:				110.0		401.0	

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: 506014MorganTimothyResidence Address: Lot: 12, Sub: Country Lane Es, Plat: City, State: Lake City, FL Owner: Morgan Timothy Climate Zone: North	Builder: Permitting Office: Columbia Permit Number: 23716 Jurisdiction Number: 221000
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<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 30%;">1. New construction or existing</td> <td style="width: 10%;"></td> <td style="width: 10%;">New</td> <td style="width: 5%; text-align: center;">—</td> </tr> <tr> <td>2. Single family or multi-family</td> <td></td> <td>Single family</td> <td style="text-align: center;">—</td> </tr> <tr> <td>3. Number of units, if multi-family</td> <td></td> <td>1</td> <td style="text-align: center;">—</td> </tr> <tr> <td>4. Number of Bedrooms</td> <td></td> <td>3</td> <td style="text-align: center;">—</td> </tr> <tr> <td>5. Is this a worst case?</td> <td></td> <td>Yes</td> <td style="text-align: center;">—</td> </tr> <tr> <td>6. Conditioned floor area (ft²)</td> <td></td> <td>2581 ft²</td> <td style="text-align: center;">—</td> </tr> <tr> <td>7. Glass area & type</td> <td>Single Pane</td> <td>Double Pane</td> <td></td> </tr> <tr> <td>a. 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Frame, Wood, Adjacent</td> <td></td> <td>R=13.0, 278.0 ft²</td> <td style="text-align: center;">—</td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> <td style="text-align: center;">—</td> </tr> <tr> <td>d. N/A</td> <td></td> <td></td> <td style="text-align: center;">—</td> </tr> <tr> <td>e. N/A</td> <td></td> <td></td> <td style="text-align: center;">—</td> </tr> <tr> <td>10. Ceiling types</td> <td></td> <td></td> <td></td> </tr> <tr> <td>a. Under Attic</td> <td></td> <td>R=30.0, 2881.0 ft²</td> <td style="text-align: center;">—</td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> <td style="text-align: center;">—</td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> <td style="text-align: center;">—</td> </tr> <tr> <td>11. Ducts</td> <td></td> <td></td> <td></td> </tr> <tr> <td>a. Sup: Unc. Ret: Unc. AH: Garage</td> <td></td> <td>Sup. R=6.0, 220.0 ft</td> <td style="text-align: center;">—</td> </tr> <tr> <td>b. 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Conservation credits			—			(HR-Heat recovery, Solar	—			DHP-Dedicated heat pump)	—	15. HVAC credits			—			(CF-Ceiling fan, CV-Cross ventilation,	—			HF-Whole house fan,	—			PT-Programmable Thermostat,	—			MZ-C-Multizone cooling,	—			MZ-H-Multizone heating)	—
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Glass/Floor Area: 0.18	Total as-built points: 35738	PASS
	Total base points: 36486	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Ben Sparks


DATE: 6/22/09 Ben Sparks

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL, PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X WPM X WOF = Points			
.18	2581.0	12.74	5918.7	Double, Clear	SE	1.5	7.0	18.0	14.71	1.07	283.0
				Double, Clear	SE	1.5	4.0	6.0	14.71	1.22	107.6
				Double, Clear	SW	99.0	8.0	10.0	16.74	2.03	339.9
				Double, Clear	SE	4.0	7.0	24.0	14.71	1.46	515.5
				Double, Clear	S	8.0	7.0	18.0	13.30	2.96	709.5
				Double, Clear	SW	99.0	7.0	18.0	16.74	2.03	611.9
				Double, Clear	SE	11.0	8.0	40.0	14.71	2.22	1303.0
				Double, Clear	E	3.0	8.0	20.0	18.79	1.07	403.8
				Double, Clear	SE	1.5	7.0	24.0	14.71	1.07	377.4
				Double, Clear	SW	1.5	7.0	36.0	16.74	1.04	628.0
				Double, Clear	SW	1.5	5.0	8.0	16.74	1.09	146.0
				Double, Clear	SW	1.5	5.0	16.0	16.74	1.09	292.1
				Double, Clear	NW	1.5	5.0	16.0	24.30	1.01	390.8
				Double, Clear	W	1.5	9.0	18.0	20.73	1.01	376.1
				Double, Clear	NW	1.5	9.0	24.0	24.30	1.00	583.2
				Double, Clear	N	1.5	9.0	18.0	24.58	1.00	442.6
				Double, Clear	W	1.5	2.5	6.0	20.73	1.11	137.6
				Double, Clear	NW	1.5	2.5	8.0	24.30	1.02	197.6
				Double, Clear	N	1.5	2.5	6.0	24.58	1.01	149.2
				Double, Clear	NW	8.0	10.0	20.0	24.30	1.02	496.3
				Double, Clear	NW	8.0	3.0	10.0	24.30	1.04	251.7
				Double, Clear	NW	1.5	9.0	36.0	24.30	1.00	874.7
				Double, Clear	NW	1.5	2.5	12.0	24.30	1.02	296.3
				Double, Clear	NE	1.5	7.0	54.0	23.57	1.00	1277.5
				Double, Clear	NE	1.5	3.0	8.0	23.57	1.02	193.1
				As-Built Total:				474.0			11384.6
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	278.0	3.60	1000.8	Frame, Wood, Exterior	13.0		1856.0	3.40		6310.4	
Exterior	1856.0	3.70	6867.2	Frame, Wood, Adjacent	13.0		278.0	3.30		917.4	
Base Total:	2134.0		7868.0	As-Built Total:			2134.0			7227.8	
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points						
Adjacent	20.0	11.50	230.0	Exterior Insulated	90.0 8.40 756.0						
Exterior	90.0	12.30	1107.0	Adjacent Insulated	20.0 8.00 160.0						
Base Total:	110.0		1337.0	As-Built Total:			110.0			916.0	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL, PERMIT #:

BASE				AS-BUILT						
CEILING TYPES Area X BWPM = Points				Type	R-Value	Area X WPM X WCM =	Points			
Under Attic	2581.0	2.05	5291.0	Under Attic	30.0	2881.0 2.05 X 1.00	5906.0			
Base Total:	2581.0		5291.0	As-Built Total:		2881.0	5906.0			
FLOOR TYPES Area X BWPM = Points				Type	R-Value	Area X WPM =	Points			
Slab	236.0(p)	8.9	2100.4	Slab-On-Grade Edge Insulation	0.0	236.0(p) 18.80	4436.8			
Raised	0.0	0.00	0.0							
Base Total:			2100.4	As-Built Total:		236.0	4436.8			
INFILTRATION Area X BWPM = Points				Area X WPM = Points						
	2581.0	-0.59	-1522.8		2581.0	-0.59	-1522.8			
Winter Base Points:			20992.4	Winter As-Built Points:			28348.5			
Total Winter Points	X System Multiplier	=	Heating Points	Total Component	X Cap Ratio	X Duct Multiplier	X System Multiplier	X Credit Multiplier	=	Heating Points
					(DM x DSM x AHU)					
20992.4	0.6274		13170.6	28348.5	1.000	1.250	0.432	1.000		15291.5

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL, PERMIT #:

BASE				AS-BUILT								
WATER HEATING												
Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	= Total Multiplier	
3		2746.00		8238.0	40.0	0.89	3		1.00	2715.15	1.00	8145.4
As-Built Total:											8145.4	

CODE COMPLIANCE STATUS													
BASE					AS-BUILT								
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
15077		13171		8238		36486	12301		15291		8145		35738

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.1

The higher the score, the more efficient the home.

Morgan Timothy, Lot: 12, Sub: Country Lane Es, Plat: , Lake City, FL,

<p>1. New construction or existing New <input type="checkbox"/></p> <p>2. Single family or multi-family Single family <input type="checkbox"/></p> <p>3. Number of units, if multi-family 1 <input type="checkbox"/></p> <p>4. Number of Bedrooms 3 <input type="checkbox"/></p> <p>5. Is this a worst case? Yes <input type="checkbox"/></p> <p>6. Conditioned floor area (ft²) 2581 ft² <input type="checkbox"/></p> <p>7. Glass area & type Single Pane Double Pane</p> <p style="padding-left: 20px;">a. Clear glass, default U-factor 0.0 ft² 474.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">b. Default tint, default U-factor 0.0 ft² 0.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">c. Labeled U-factor or SHGC 0.0 ft² 0.0 ft² <input type="checkbox"/></p> <p>8. Floor types</p> <p style="padding-left: 20px;">a. Slab-On-Grade Edge Insulation R=0.0, 236.0(p) ft <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>9. Wall types</p> <p style="padding-left: 20px;">a. Frame, Wood, Exterior R=13.0, 1856.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">b. Frame, Wood, Adjacent R=13.0, 278.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">d. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">e. N/A <input type="checkbox"/></p> <p>10. Ceiling types</p> <p style="padding-left: 20px;">a. Under Attic R=30.0, 2881.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>11. Ducts</p> <p style="padding-left: 20px;">a. Sup: Unc. Ret: Unc. AH: Garage Sup. R=6.0, 220.0 ft <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p>	<p>12. Cooling systems</p> <p style="padding-left: 20px;">a. Central Unit Cap: 58.0 kBtu/hr <input type="checkbox"/></p> <p style="padding-left: 40px;">SEER: 13.00 <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>13. Heating systems</p> <p style="padding-left: 20px;">a. Electric Heat Pump Cap: 58.0 kBtu/hr <input type="checkbox"/></p> <p style="padding-left: 40px;">HSPF: 7.90 <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>14. Hot water systems</p> <p style="padding-left: 20px;">a. Electric Resistance Cap: 40.0 gallons <input type="checkbox"/></p> <p style="padding-left: 40px;">EF: 0.89 <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. Conservation credits <input type="checkbox"/></p> <p style="padding-left: 40px;">(HR-Heat recovery, Solar</p> <p style="padding-left: 40px;">DHP-Dedicated heat pump)</p> <p>15. HVAC credits <input type="checkbox"/></p> <p style="padding-left: 20px;">(CF-Ceiling fan, CV-Cross ventilation,</p> <p style="padding-left: 20px;">HF-Whole house fan,</p> <p style="padding-left: 20px;">PT-Programmable Thermostat,</p> <p style="padding-left: 20px;">MZ-C-Multizone cooling,</p> <p style="padding-left: 20px;">MZ-H-Multizone heating)</p>
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I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____

Date: _____

Address of New Home: _____

City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs.*

Energy Gauge Version: FLR2PB v3.4)

COLUMBIA COUNTY BUILDING DEPARTMENT

**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR
FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS
ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant

Plans Examiner



All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.



Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.



Site Plan including:

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.



Wind-load Engineering Summary, calculations and any details required

- a) Plans or specifications must state compliance with FBC Section 1606
- b) The following information must be shown as per section 1606.1.7 FBC
 - a. Basic wind speed (MPH)
 - b. Wind importance factor (I) and building category
 - c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
 - d. The applicable internal pressure coefficient
 - e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional



Elevations including:

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories

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-
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N/A

N/A

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-

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-

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sec 2

-

sec 4

-

sec 3

-

-

Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and sealed by FI. Pro. Eng.
 - 2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
 - 1. All materials making up wall
 - 2. Block size and mortar type with size and spacing of reinforcement
 - 3. Lintel, tie-beam sizes and reinforcement
 - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
 - 6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - 7. Fire resistant construction (if required)
 - 8. Fireproofing requirements
 - 9. Shoe type of termite treatment (termicide or alternative method)
 - 10. Slab on grade
 - a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
 - a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

*****Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

Residential System Sizing Calculation

Summary

Morgan Timothy
Lake City, FL

Project Title:
506014MorganTimothyResidence

Class 3 Rating
Registration No. 0
Climate: North

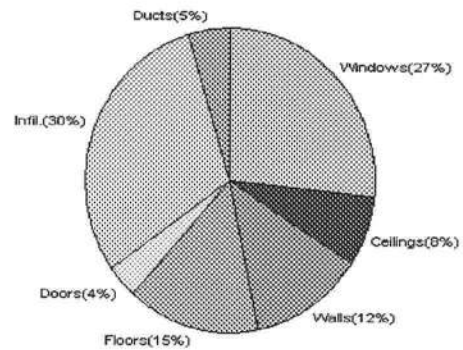
6/22/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)			
Winter design temperature	31 F	Summer design temperature	93 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	39 F	Summer temperature difference	18 F
Total heating load calculation	49818 Btuh	Total cooling load calculation	51418 Btuh
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	116.4 58000	Sensible (SHR = 0.5)	73.3 29000
Heat Pump + Auxiliary(0.0kW)	116.4 58000	Latent	244.9 29000
		Total (Electric Heat Pump)	112.8 58000

WINTER CALCULATIONS

Winter Heating Load (for 2581 sqft)

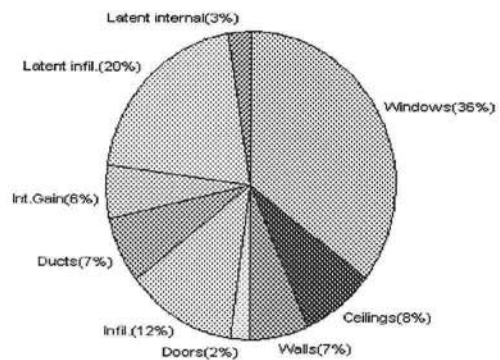
Load component		Load	
Window total	474 sqft	13414	Btuh
Wall total	2134 sqft	6198	Btuh
Door total	110 sqft	1838	Btuh
Ceiling total	2881 sqft	3745	Btuh
Floor total	236 ft	7458	Btuh
Infiltration	345 cfm	14793	Btuh
Subtotal		47446	Btuh
Duct loss		2372	Btuh
TOTAL HEAT LOSS		49818	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2581 sqft)

Load component		Load	
Window total	474 sqft	18277	Btuh
Wall total	2134 sqft	3519	Btuh
Door total	110 sqft	1115	Btuh
Ceiling total	2881 sqft	4091	Btuh
Floor total		0	Btuh
Infiltration	302 cfm	5974	Btuh
Internal gain		3000	Btuh
Subtotal(sensible)		35976	Btuh
Duct gain		3598	Btuh
Total sensible gain		39574	Btuh
Latent gain(infiltration)		10464	Btuh
Latent gain(internal)		1380	Btuh
Total latent gain		11844	Btuh
TOTAL HEAT GAIN		51418	Btuh



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: *Y. Ben Spawth*

DATE: *6/22/05*

System Sizing Calculations - Winter

Residential Load - Component Details

Morgan Timothy

Project Title:

Class 3 Rating

Lake City, FL

506014MorganTimothyResidence

Registration No. 0

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

6/22/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	18.0	28.3	509 Btuh
2	2, Clear, Metal, DEF	N	6.0	28.3	170 Btuh
3	2, Clear, Metal, DEF	E	10.0	28.3	283 Btuh
4	2, Clear, Metal, DEF	N	24.0	28.3	679 Btuh
5	2, Clear, Metal, DEF	NE	18.0	28.3	509 Btuh
6	2, Clear, Metal, DEF	E	18.0	28.3	509 Btuh
7	2, Clear, Metal, DEF	N	40.0	28.3	1132 Btuh
8	2, Clear, Metal, DEF	NW	20.0	28.3	566 Btuh
9	2, Clear, Metal, DEF	N	24.0	28.3	679 Btuh
10	2, Clear, Metal, DEF	E	36.0	28.3	1019 Btuh
11	2, Clear, Metal, DEF	E	8.0	28.3	226 Btuh
12	2, Clear, Metal, DEF	E	16.0	28.3	453 Btuh
13	2, Clear, Metal, DEF	S	16.0	28.3	453 Btuh
14	2, Clear, Metal, DEF	SE	18.0	28.3	509 Btuh
15	2, Clear, Metal, DEF	S	24.0	28.3	679 Btuh
16	2, Clear, Metal, DEF	SW	18.0	28.3	509 Btuh
17	2, Clear, Metal, DEF	SE	6.0	28.3	170 Btuh
18	2, Clear, Metal, DEF	S	8.0	28.3	226 Btuh
19	2, Clear, Metal, DEF	SW	6.0	28.3	170 Btuh
20	2, Clear, Metal, DEF	S	20.0	28.3	566 Btuh
21	2, Clear, Metal, DEF	S	10.0	28.3	283 Btuh
22	2, Clear, Metal, DEF	S	36.0	28.3	1019 Btuh
23	2, Clear, Metal, DEF	S	12.0	28.3	340 Btuh
24	2, Clear, Metal, DEF	W	54.0	28.3	1528 Btuh
25	2, Clear, Metal, DEF	W	8.0	28.3	226 Btuh
Window Total			474		13414 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1856	3.1	5754 Btuh
2	Frame - Adjacent	13.0	278	1.6	445 Btuh
Wall Total			2134		6198 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		90	18.3	1650 Btuh
2	Insulated - Adjac		20	9.4	188 Btuh
Door Total			110		1838Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	2881	1.3	3745 Btuh
Ceiling Total			2881		3745Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	236.0 ft(p)	31.6	7458 Btuh
Floor Total			236		7458 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Morgan Timothy
Lake City, FL

Project Title:
506014MorganTimothyResidence

Class 3 Rating
Registration No. 0
Climate: North

6/22/2005

Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.80	25810(sqft)	345	14793 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			345	14793 Btuh

Totals for Heating	Subtotal	47446 Btuh
	Duct Loss(using duct multiplier of 0.05)	2372 Btuh
	Total Btuh Loss	49818 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Morgan Timothy

Project Title:

Class 3 Rating

Lake City, FL

506014MorganTimothyResidence

Registration No. 0

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 18.0 F

6/22/2005

Window	Type	Ormt	Overhang		Window Area(sqft)			HTM		Load
	Panes/SHGC/U/InSh/ExSh		Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, DEF, N, N	N	1.5	7	18.0	0.0	18.0	22	22	396 Btuh
2	2, Clear, DEF, N, N	N	1.5	4	6.0	0.0	6.0	22	22	132 Btuh
3	2, Clear, DEF, N, N	E	99	8	10.0	10.0	0.0	22	72	220 Btuh
4	2, Clear, DEF, N, N	N	4	7	24.0	0.0	24.0	22	22	528 Btuh
5	2, Clear, DEF, N, N	NE	8	7	18.0	0.0	18.0	22	50	900 Btuh
6	2, Clear, DEF, N, N	E	99	7	18.0	18.0	0.0	22	72	396 Btuh
7	2, Clear, DEF, N, N	N	11	8	40.0	0.0	40.0	22	22	880 Btuh
8	2, Clear, DEF, N, N	NW	3	8	20.0	0.0	20.0	22	50	1000 Btuh
9	2, Clear, DEF, N, N	N	1.5	7	24.0	0.0	24.0	22	22	528 Btuh
10	2, Clear, DEF, N, N	E	1.5	7	36.0	1.5	34.5	22	72	2518 Btuh
11	2, Clear, DEF, N, N	E	1.5	5	8.0	0.0	8.0	22	72	576 Btuh
12	2, Clear, DEF, N, N	E	1.5	5	16.0	1.0	15.0	22	72	1103 Btuh
13	2, Clear, DEF, N, N	S	1.5	5	16.0	16.0	0.0	22	37	352 Btuh
14	2, Clear, DEF, N, N	SE	1.5	9	18.0	0.0	18.0	22	62	1116 Btuh
15	2, Clear, DEF, N, N	S	1.5	9	24.0	24.0	0.0	22	37	528 Btuh
16	2, Clear, DEF, N, N	SW	1.5	9	18.0	0.0	18.0	22	62	1116 Btuh
17	2, Clear, DEF, N, N	SE	1.5	2.5	6.0	6.0	0.0	22	62	132 Btuh
18	2, Clear, DEF, N, N	S	1.5	2.5	8.0	8.0	0.0	22	37	176 Btuh
19	2, Clear, DEF, N, N	SW	1.5	2.5	6.0	6.0	0.0	22	62	132 Btuh
20	2, Clear, DEF, N, N	S	8	10	20.0	20.0	0.0	22	37	440 Btuh
21	2, Clear, DEF, N, N	S	8	3	10.0	10.0	0.0	22	37	220 Btuh
22	2, Clear, DEF, N, N	S	1.5	9	36.0	36.0	0.0	22	37	792 Btuh
23	2, Clear, DEF, N, N	S	1.5	2.5	12.0	12.0	0.0	22	37	264 Btuh
24	2, Clear, DEF, N, N	W	1.5	7	54.0	9.6	44.4	22	72	3408 Btuh
25	2, Clear, DEF, N, N	W	1.5	3	8.0	3.0	5.0	22	72	424 Btuh
Window Total					474					18277 Btuh
Walls	Type	R-Value		Area			HTM		Load	
1	Frame - Exterior	13.0		1856.0			1.7		3229 Btuh	
2	Frame - Adjacent	13.0		278.0			1.0		289 Btuh	
Wall Total				2134.0					3519 Btuh	
Doors	Type	R-Value		Area			HTM		Load	
1	Insulated - Exter			90.0			10.1		913 Btuh	
2	Insulated - Adjac			20.0			10.1		203 Btuh	
Door Total				110.0					1115 Btuh	
Ceilings	Type/Color	R-Value		Area			HTM		Load	
1	Under Attic/Dark	30.0		2881.0			1.4		4091 Btuh	
Ceiling Total				2881.0					4091 Btuh	
Floors	Type	R-Value		Size			HTM		Load	
1	Slab-On-Grade Edge Insulation	0.0		236.0 ft(p)			0.0		0 Btuh	
Floor Total				236.0					0 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Morgan Timothy

Project Title:

Class 3 Rating

Lake City, FL

506014MorganTimothyResidence

Registration No. 0

Climate: North

6/22/2005

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.70	25810	301.7	5974 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			302	5974 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 300 +	1200	3000 Btuh

Totals for Cooling	Subtotal	35976 Btuh
	Duct gain(using duct multiplier of 0.10)	3598 Btuh
	Total sensible gain	39574 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	10464 Btuh
	Latent occupant gain (6 people @ 230 Btuh per person)	1380 Btuh
	Latent other gain	0 Btuh
	TOTAL GAIN	51418 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
 (U - Window U-Factor or 'DEF' for default)
 (InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
 (ExSh - Exterior shading device: none(N) or numerical value)
 (Ornt - compass orientation)

Project Information for: L133344
 Builder: Tim Morgan – Owner Bldr. Date: 9/26/2005
 Lot: Lot 12A Start Number: 206
 Subdivision: Country Lane Estates
 County or City: Columbia County
 Truss Page Count: 59

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2
 Gravity Wind Building Code: FBC2001
 Roof (psf): 42 Wind Standard: ASCE 7-98
 Floor (psf): 55 Wind Speed (mph): 110
 Note: See individual truss drawings for special loading conditions

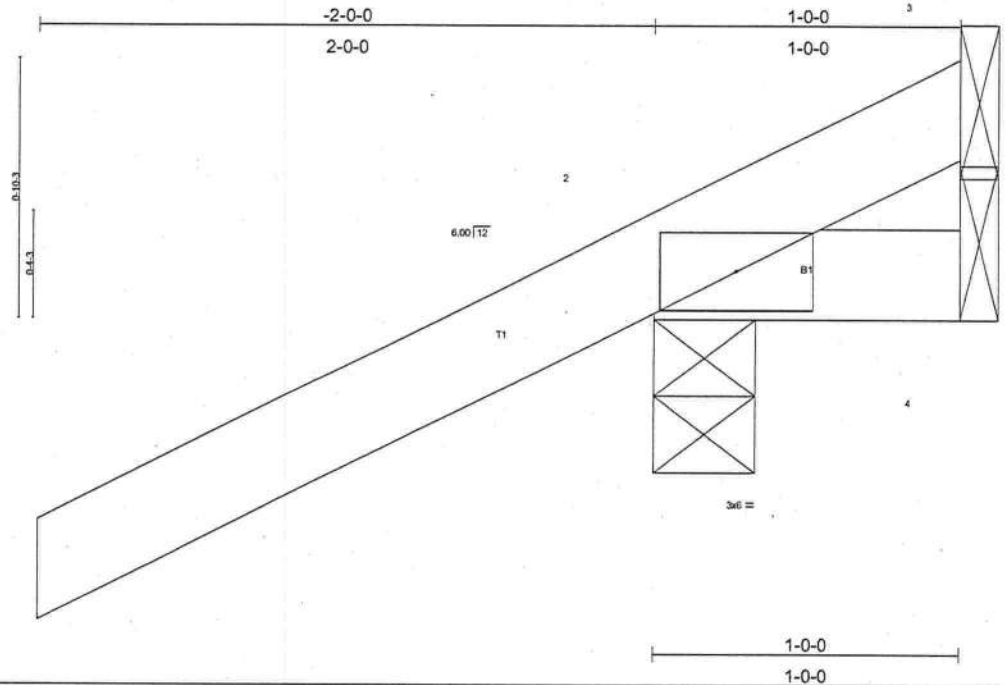
Building Designer, responsible for Structural Engineering: (See attached)
 Owner Builder
 Address: N/A
 SAINT AUGUSTINE, FLORIDA 32084 Designer: 80

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
 Company: Structural Engineering and Inspections, Inc. EB 9196
 Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
 1. Truss Design Engineer is responsible for the individual trusses as components only.
 2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
 3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	092605206	9/26/2005	41	T18	092605246	9/26/2005
2	CJ2	092605207	9/26/2005	42	T18	092605247	9/26/2005
3	CJ3	092605208	9/26/2005	43	T19	092605248	9/26/2005
4	CJ3B	092605209	9/26/2005	44	T20	092605249	9/26/2005
5	CJ3C	092605210	9/26/2005	45	T21	092605250	9/26/2005
6	CJ4	092605211	9/26/2005	46	T22	092605251	9/26/2005
7	CJ5	092605212	9/26/2005	47	T23	092605252	9/26/2005
8	CJ5B	092605213	9/26/2005	48	T24	092605253	9/26/2005
9	EJ6	092605214	9/26/2005	49	T25	092605254	9/26/2005
10	EJ7	092605215	9/26/2005	50	T26	092605255	9/26/2005
11	EJ7B	092605216	9/26/2005	51	T26G	092605256	9/26/2005
12	EJ7C	092605217	9/26/2005	52	V05	092605257	9/26/2005
13	HJ5A	092605218	9/26/2005	53	V05A	092605258	9/26/2005
14	HJ5B	092605219	9/26/2005	54	V05B	092605259	9/26/2005
15	HJ9	092605220	9/26/2005	55	V06	092605260	9/26/2005
16	HJ9A	092605221	9/26/2005	56	V08	092605261	9/26/2005
17	PB14A	092605222	9/26/2005	57	V10	092605262	9/26/2005
18	PB14B	092605223	9/26/2005	58	V10A	092605263	9/26/2005
19	PB14C	092605224	9/26/2005	59	V10B	092605264	9/26/2005
20	PB14D	092605225	9/26/2005				
21	PB14E	092605226	9/26/2005				
22	PB14F	092605227	9/26/2005				
23	T01	092605228	9/26/2005				
24	T01G	092605229	9/26/2005				
25	T02	092605230	9/26/2005				
26	T03	092605231	9/26/2005				
27	T04	092605232	9/26/2005				
28	T05	092605233	9/26/2005				
29	T06	092605234	9/26/2005				
30	T07	092605235	9/26/2005				
31	T08	092605236	9/26/2005				
32	T09	092605237	9/26/2005				
33	T10	092605238	9/26/2005				
34	T11	092605239	9/26/2005				
35	T12	092605240	9/26/2005				
36	T13	092605241	9/26/2005				
37	T14	092605242	9/26/2005				
38	T15	092605243	9/26/2005				
39	T16	092605244	9/26/2005				
40	T17	092605245	9/26/2005				

SEP 26 2005



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.28	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(LL) -0.00 2 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.00 2 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 3 n/a n/a		
	Code FBC2004/TPI2002				Weight: 7 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

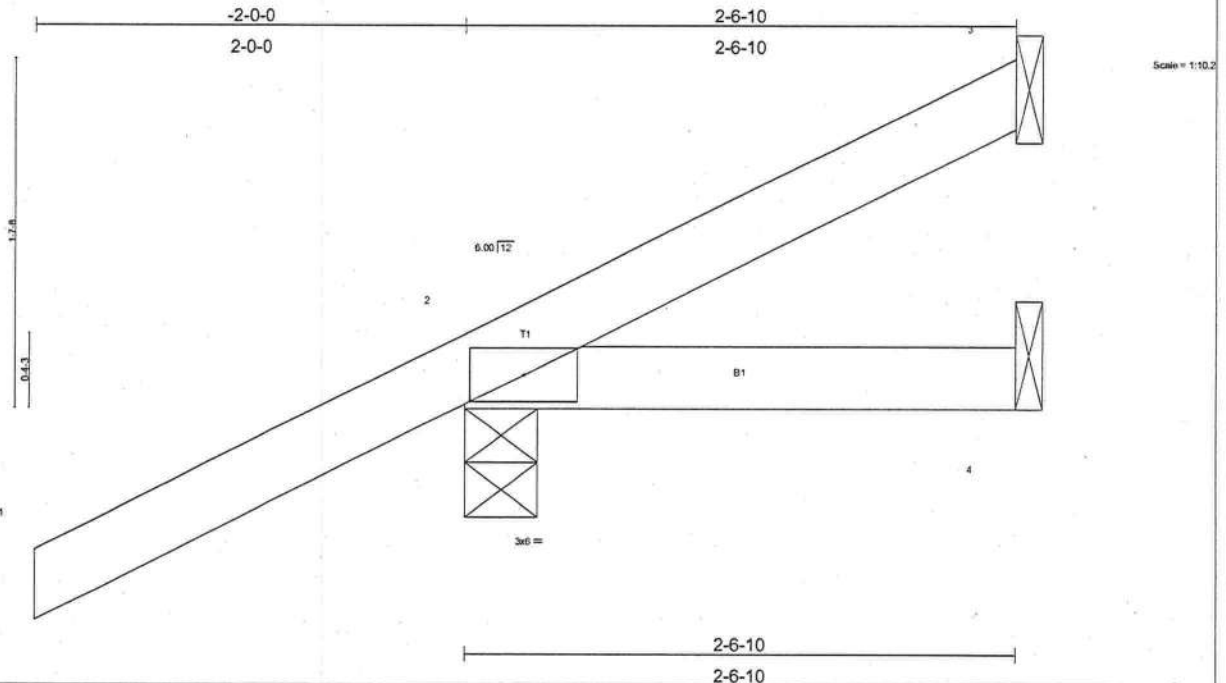
REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
 Max Horz 2=87(load case 5)
 Max Uplift 2=-275(load case 5), 3=-91(load case 1)
 Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-69/72
 BOT CHORD 2-4=0/0

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 91 lb uplift at joint 3.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.00 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.04	Vert(TL) -0.00 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 12 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 2-6-10 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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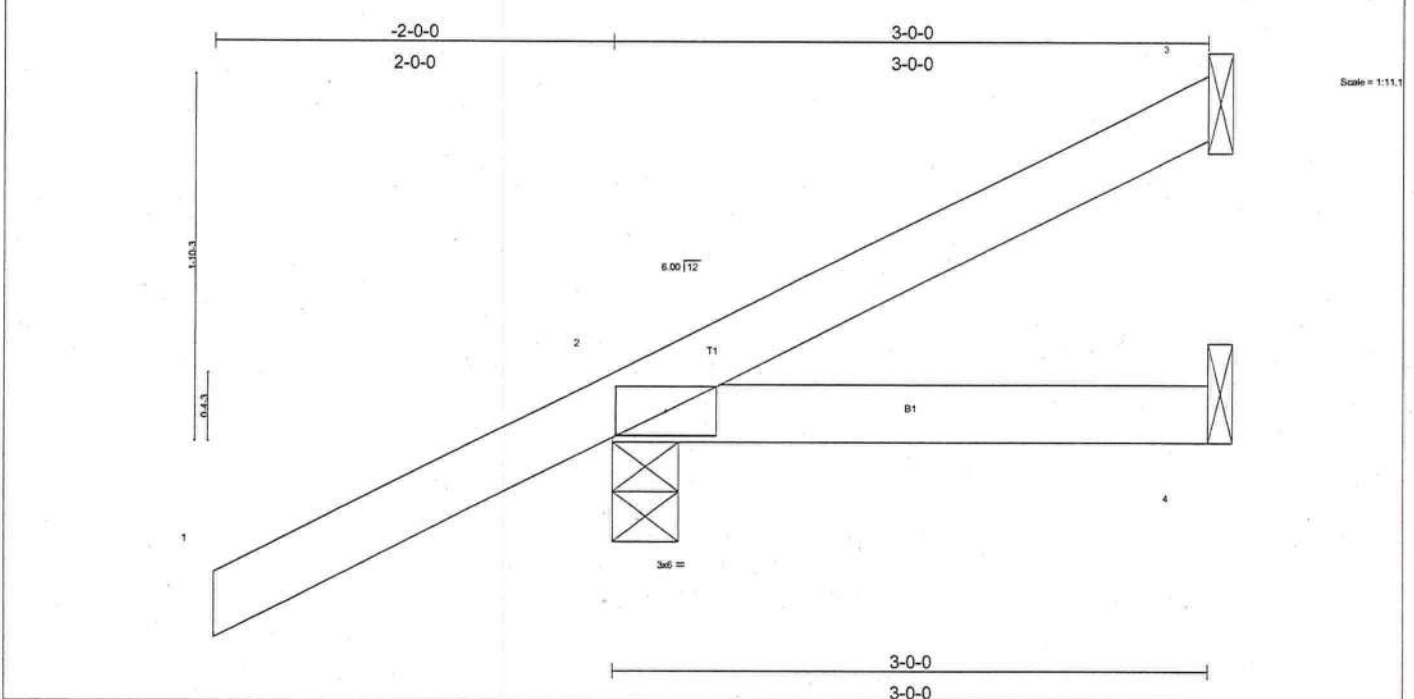
REACTIONS (lb/size) 3=8/Mechanical, 2=269/0-4-0, 4=35/Mechanical
 Max Horz 2=121(load case 5)
 Max Uplift 3=15(load case 4), 2=212(load case 5)
 Max Grav 3=19(load case 3), 2=269(load case 1), 4=35(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-57/8
 BOT CHORD 2-4=0/0

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 3 and 212 lb uplift at joint 2.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.00 2-4 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(TL) -0.01 2-4 >999 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 13 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift 3=-24(load case 4), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 3 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Builders FirstSource, Lake City, FL 32055

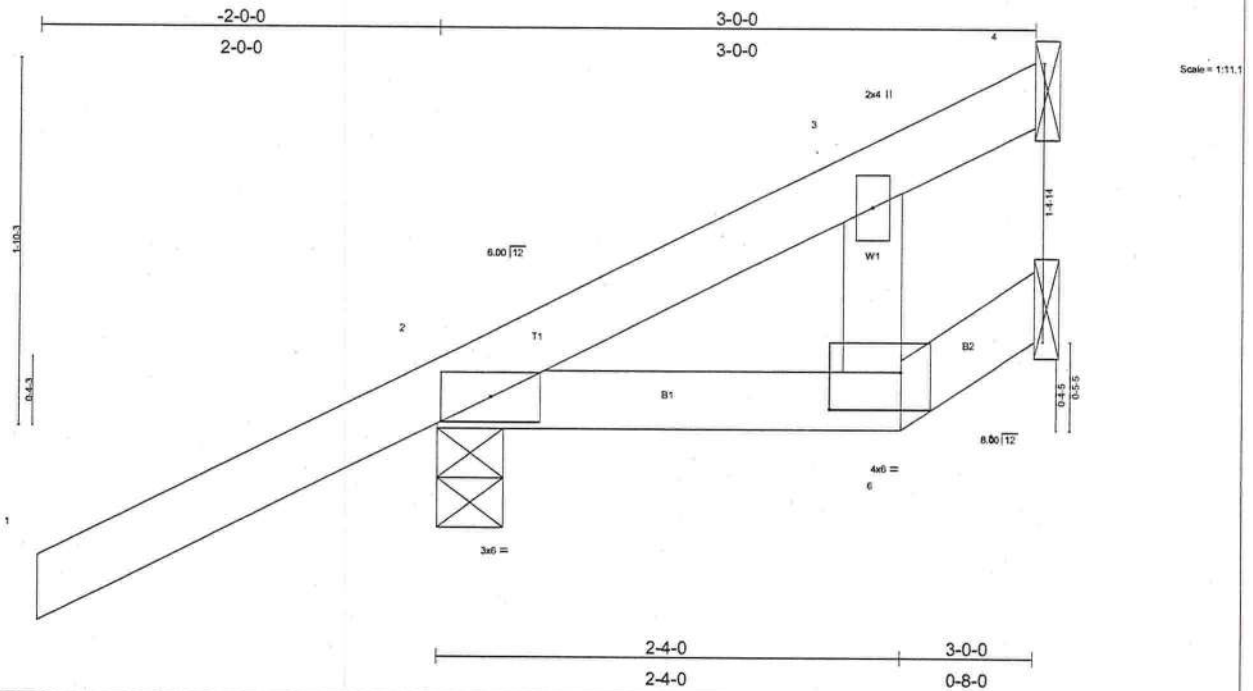


Plate Offsets (X,Y): [6:0-4:0-2-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.03	Vert(LL) -0.01 6 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.01	Vert(TL) -0.01 6 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 5 n/a n/a		
				Weight: 15 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
--	--

REACTIONS (lb/size) 4=62/Mechanical, 2=279/0-4-0, 5=9/Mechanical
Max Horz 2=132(load case 5)
Max Uplift 4=18(load case 4), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-59/3, 3-4=-28/26
BOT CHORD 2-6=-6/0, 5-6=-4/4
WEBS 3-6=0/42

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 4 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

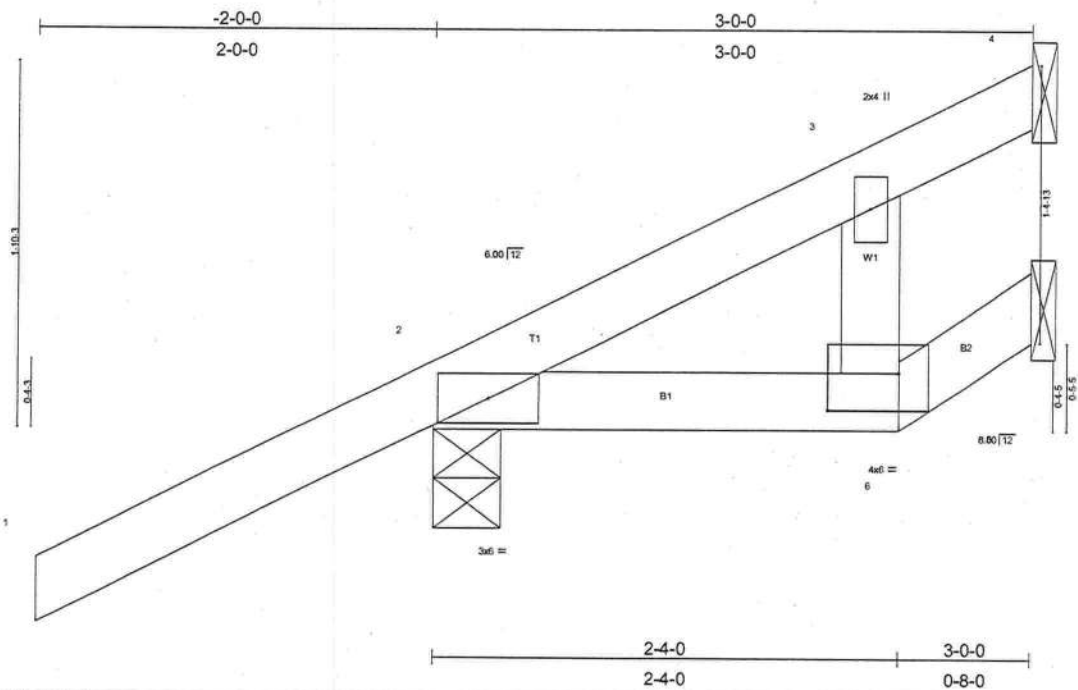


Plate Offsets (X,Y): [6:0-4-4,0-2-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL		PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.01	in (loc) 6	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.03	Vert(TL) -0.01	>999		
BCLL 10.0	Rep Stress Incr YES	WB 0.01	Horz(TL) -0.00	5		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		n/a		
				n/a		
					Weight: 15 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

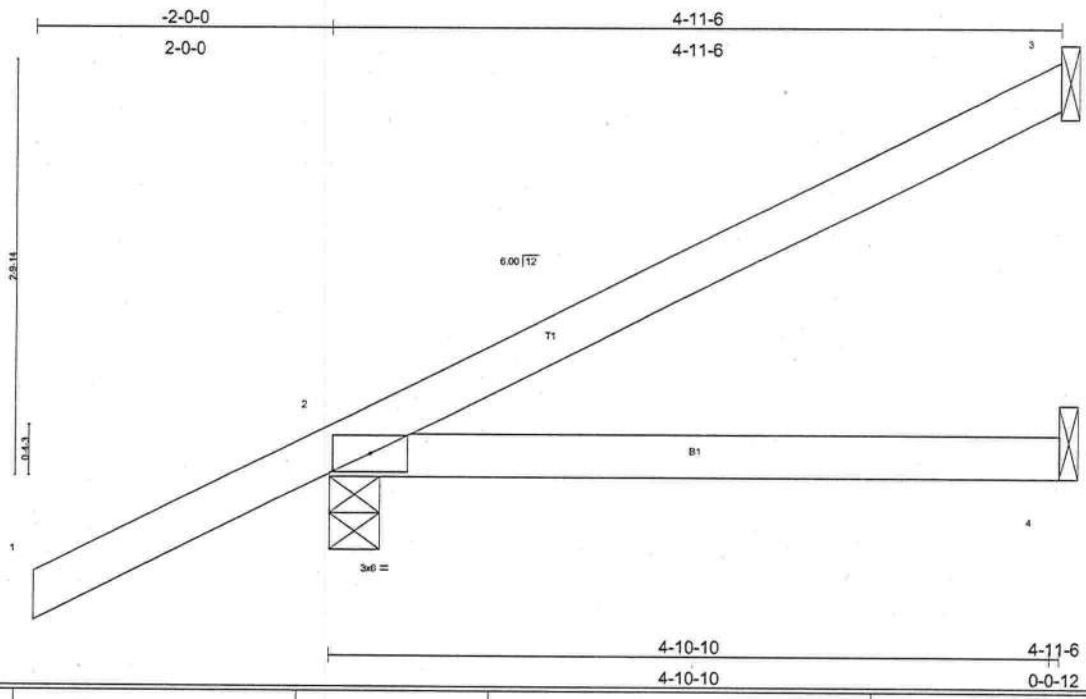
REACTIONS (lb/size) 4=62/Mechanical, 2=279/0-4-0, 5=9/Mechanical
 Max Horz 2=132(load case 5)
 Max Uplift 4=18(load case 4), 2=205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=59/3, 3-4=28/26
 BOT CHORD 2-6=6/0, 5-6=4/4
 WEBS 3-6=0/42

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 4 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.03 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.05 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 19 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

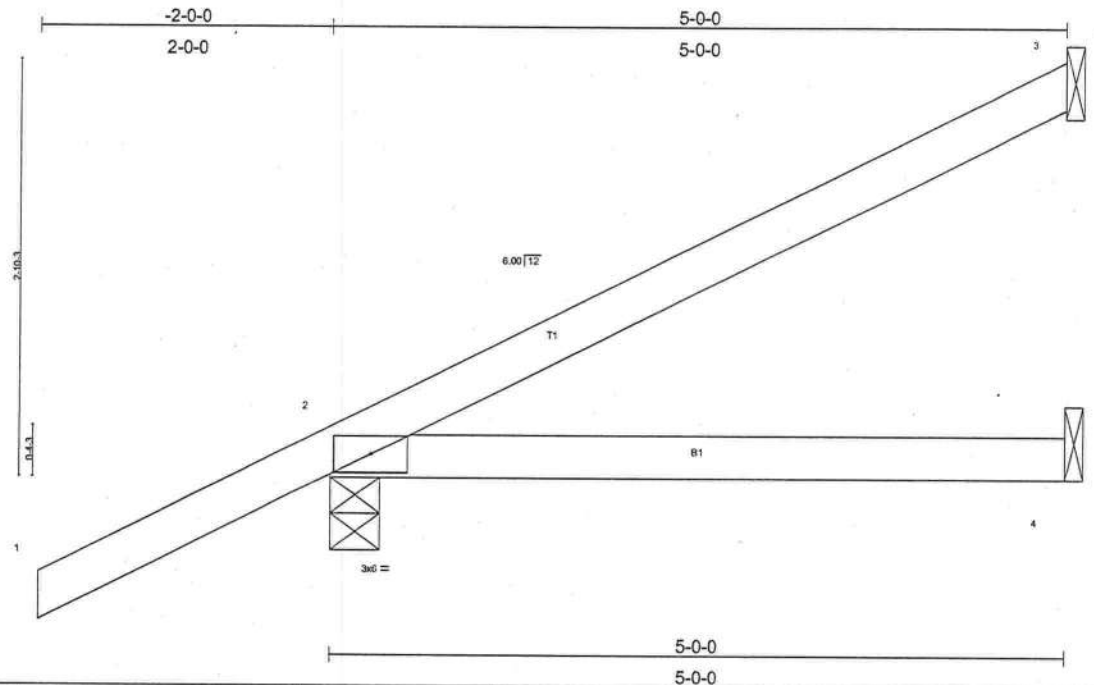
REACTIONS (lb/size) 3=101/Mechanical, 2=342/0-4-0, 4=71/Mechanical
Max Horz 2=177(load case 5)
Max Uplift 3=85(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-107/35
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 3 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.03 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.05 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			Weight: 19 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift 3=-86(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-108/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

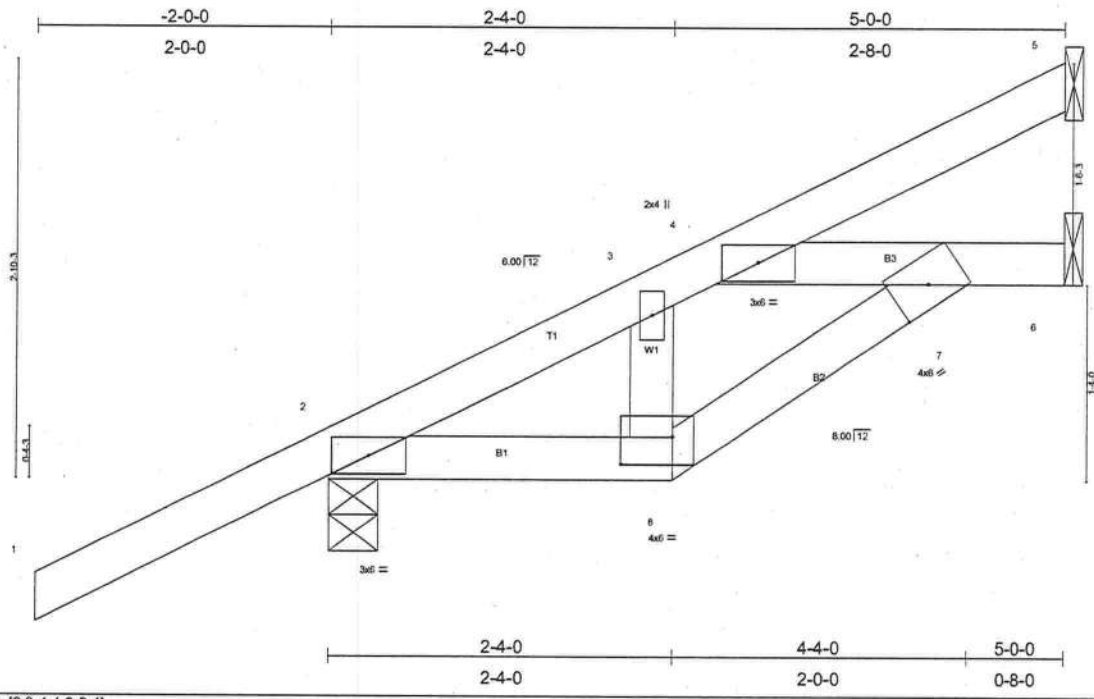


Plate Offsets (X,Y): [8:0-4:4,0-2-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) 0.01 4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) 0.01 4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.02	Horz(TL) 0.00 6 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 24 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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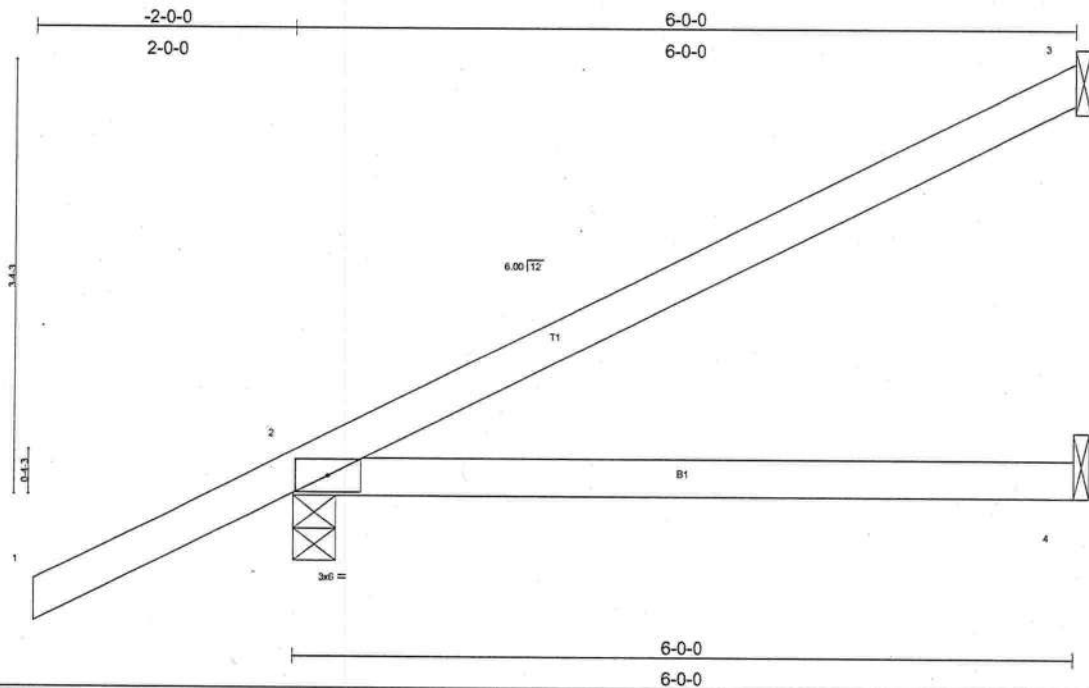
REACTIONS (lb/size) 5=74/Mechanical, 2=359/0-4-0, 6=134/Mechanical
 Max Horz 2=178(load case 5)
 Max Uplift 5=-65(load case 5), 2=-198(load case 5), 6=-5(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-250/24, 3-4=-208/57, 4-5=-62/28
 BOT CHORD 2-8=-141/168, 7-8=-158/208, 4-7=-162/128, 6-7=0/0
 WEBS 3-8=-46/99

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 5, 198 lb uplift at joint 2 and 5 lb uplift at joint 6.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/def L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	Vert(LL) -0.06 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.24	Vert(TL) -0.10 2-4 >680 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			Weight: 22 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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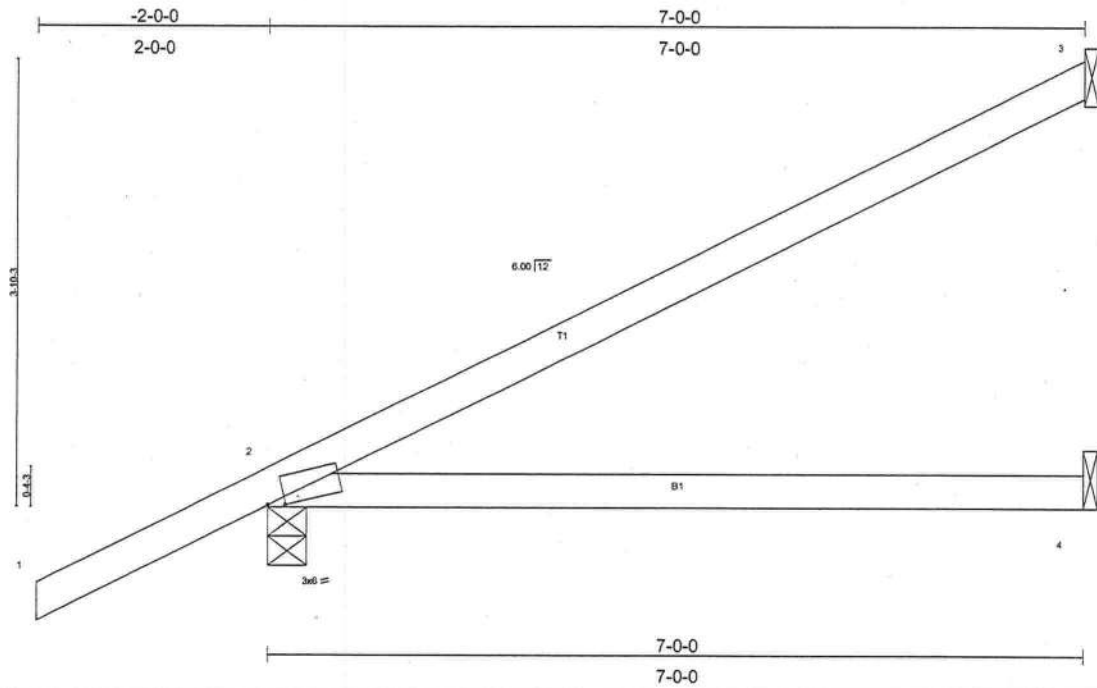
REACTIONS (lb/size) 3=134/Mechanical, 2=381/0-4-0, 4=87/Mechanical
 Max Horz 2=201(load case 5)
 Max Uplift 3=-117(load case 5), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-134/47
 BOT CHORD 2-4=0/0

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 3 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

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Scale = 1/16" = 1'-0"
Camber = 1/16" in

Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.56	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.34	Vert(LL) -0.12 2-4 >674 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.20 2-4 >403 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 26 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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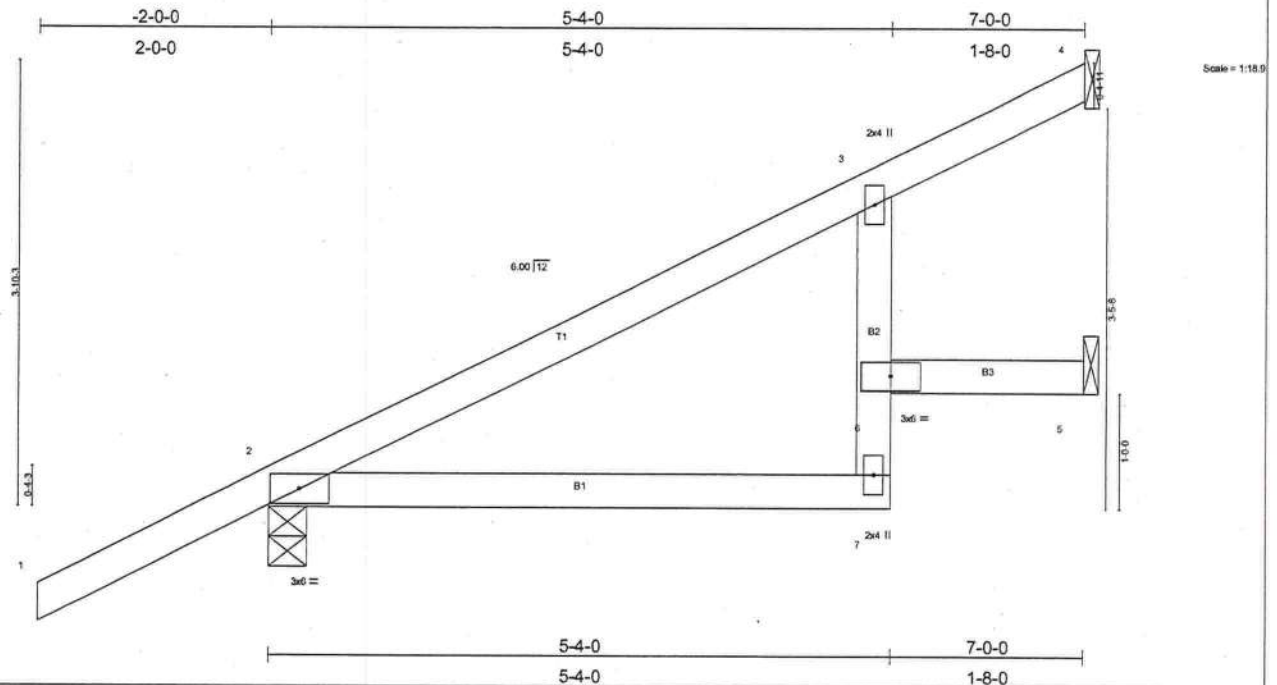
REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
 Max Horz 2=224(load case 5)
 Max Uplift 3=-133(load case 5), 2=-211(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-142/58
 BOT CHORD 2-4=0/0

- NOTES**
- 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) Refer to girder(s) for truss to truss connections.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 3 and 211 lb uplift at joint 2.

LOAD CASE(S) Standard

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.02 2-7 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.20	Vert(TL) -0.03 2-7 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 5 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 29 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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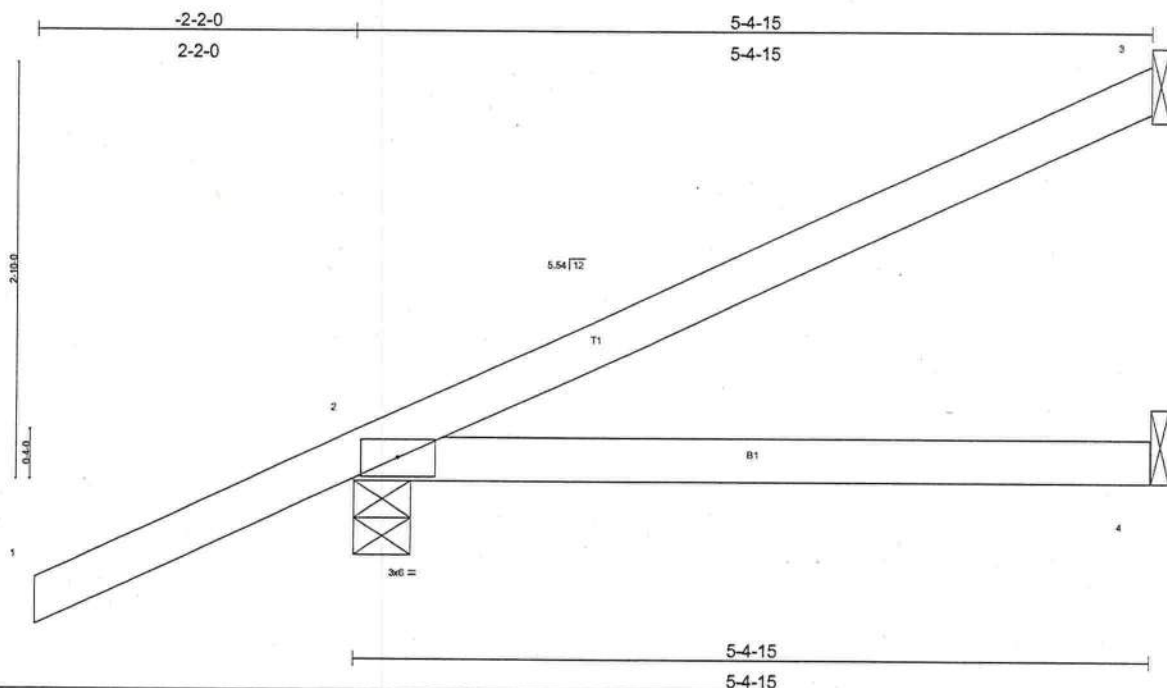
REACTIONS (lb/size) 4=89/Mechanical, 2=249/0-4-0, 5=76/Mechanical
Max Horz 2=102(load case 5)
Max Uplift 4=-42(load case 5), 2=-115(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/0, 3-4=-44/36
BOT CHORD 2-7=0/66, 6-7=0/63, 3-6=0/58, 5-6=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 4 and 115 lb uplift at joint 2.

LOAD CASE(S) Standard

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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.32 BC 0.14 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.03 2-4 >999 240 Vert(TL) -0.04 2-4 >999 180 Horz(TL) -0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 21 lb
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LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

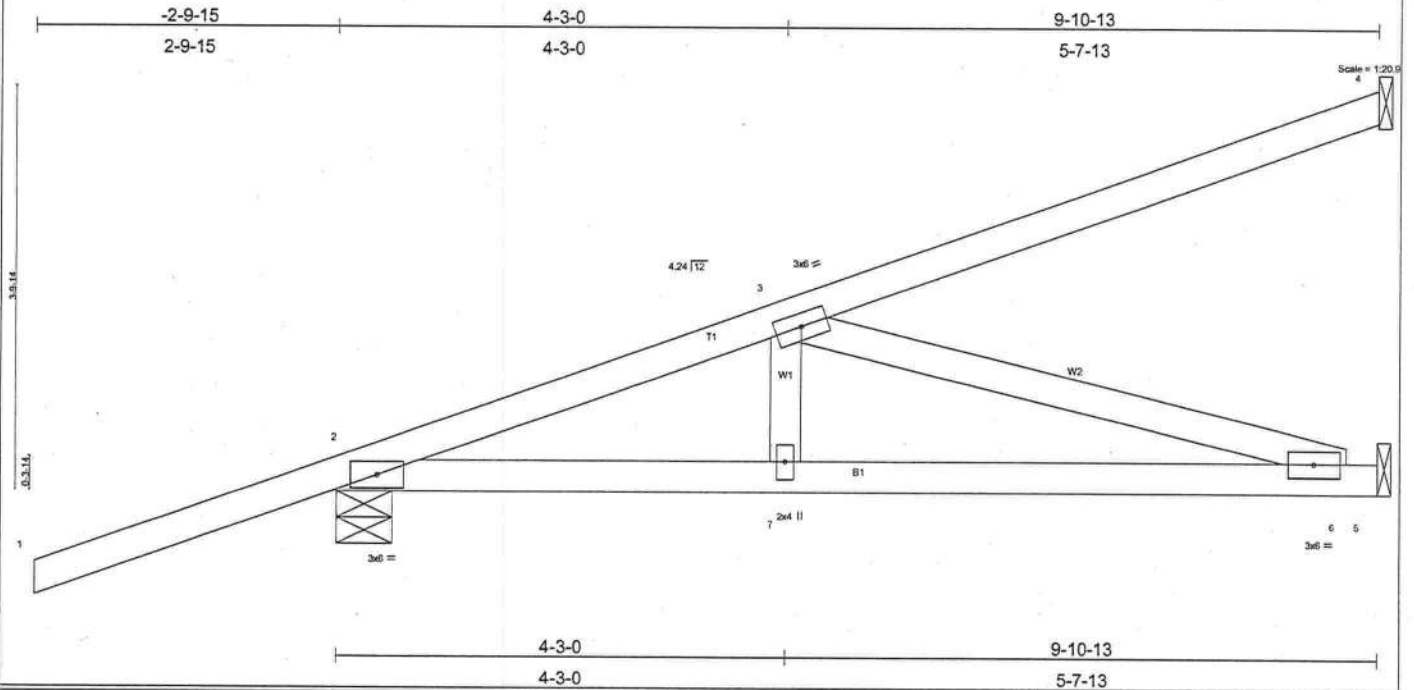
REACTIONS (lb/size) 3=99/Mechanical, 2=259/0-4-10, 4=70/Mechanical
Max Horz 2=139(load case 4)
Max Uplift 3=77(load case 4), 2=-176(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-47/27
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 3 and 176 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-73(F=-10, B=-10), 2=0(F=15, B=15)-to-4=-41(F=-5, B=-5)

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	0-0-1	TC 0.29	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.21	Vert(LL) -0.04 6-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.11	Vert(TL) -0.07 6-7 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 45 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 4=140/Mechanical, 2=57/0-6-7, 5=128/Mechanical
 Max Horz 2=46(load case 2)
 Max Uplift 4=128(load case 2), 2=57(load case 1), 5=16(load case 2)
 Max Grav 4=140(load case 1), 2=43(load case 5), 5=128(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/0, 2-3=-197/27, 3-4=-55/32
 BOT CHORD 2-7=-106/182, 6-7=-106/182, 5-6=0/0
 WEBS 3-7=0/33, 3-6=-189/110

- NOTES**
- 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 - 2) Refer to girder(s) for truss to truss connections.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 4, 57 lb uplift at joint 2 and 16 lb uplift at joint 5.
 - 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=0
 Trapezoidal Loads (plf)
 Vert: 2=50(F=25, B=25)-to-4=-80(F=-40, B=-40), 2=30(F=15, B=15)-to-5=-44(F=-22, B=-22)

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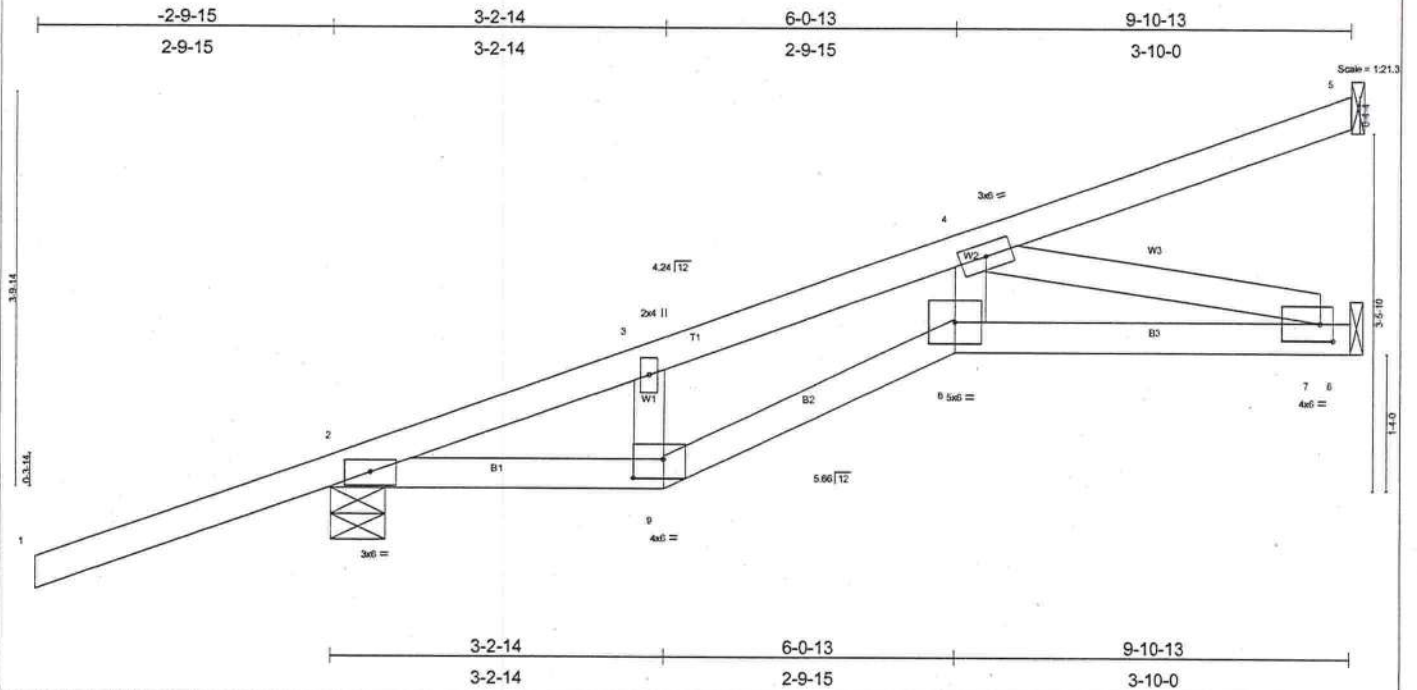


Plate Offsets (X,Y): [7:0-1-8,0-2-0], [9:0-3-8,0-2-4]

LOADING (psf)	SPACING	0-0-1	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.25	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.27	Vert(LL) 0.06 7-8 >999 240		
BCLL 10.0	Rep Stress Incr NO		WB 0.05	Vert(TL) -0.10 7-8 >999 180		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)	Horz(TL) 0.05 6 n/a n/a		
					Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

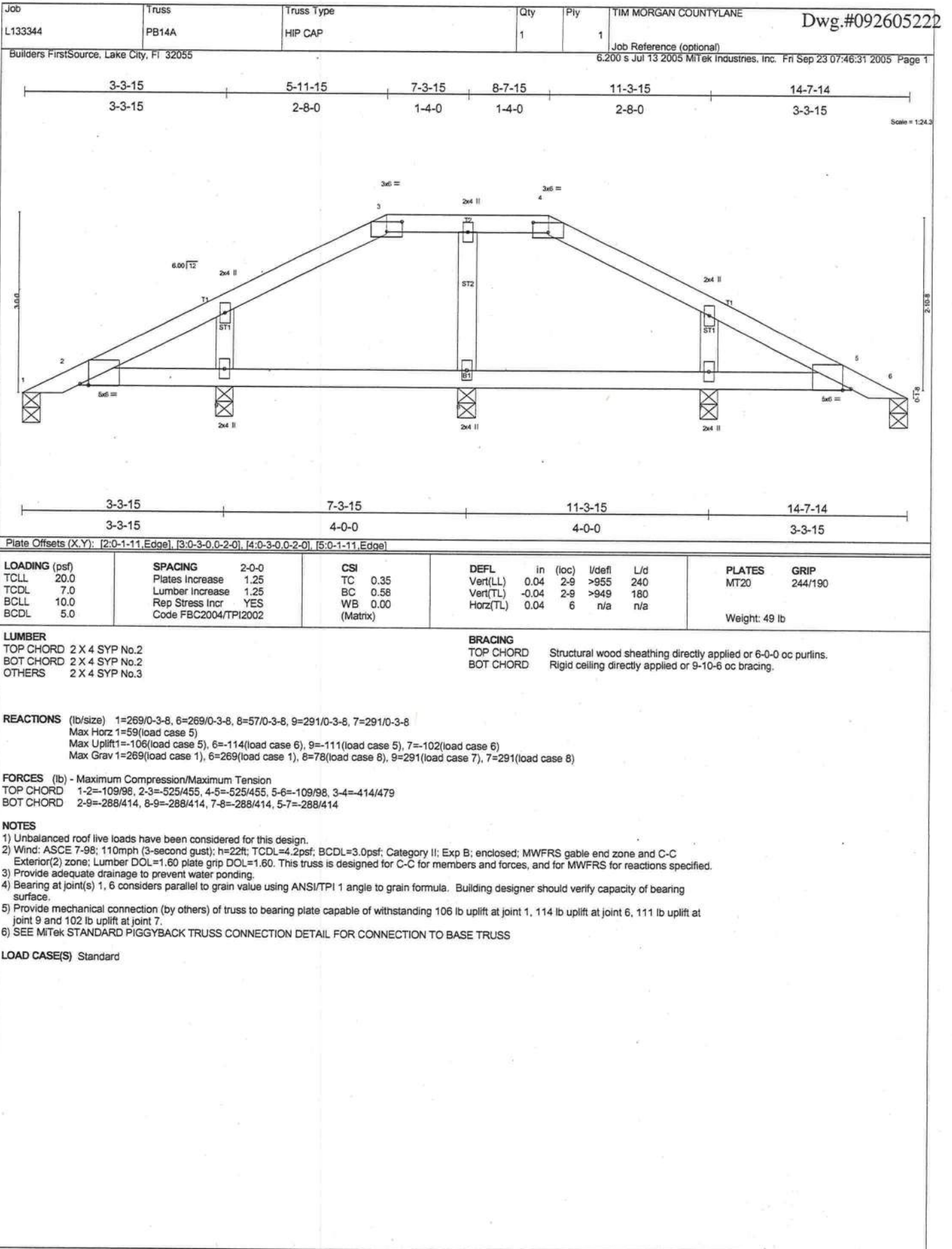
REACTIONS (lb/size) 5=131/Mechanical, 2=-57/0-6-7, 6=137/Mechanical
Max Horz 2=45(load case 2)
Max Uplift 5=-108(load case 2), 2=-57(load case 1), 6=-35(load case 2)
Max Grav 5=131(load case 1), 2=43(load case 5), 6=137(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/0, 2-3=-225/55, 3-4=-230/49, 4-5=-46/34
BOT CHORD 2-9=-117/206, 8-9=-112/202, 7-8=-121/192, 6-7=0/0
WEBS 3-9=-23/0, 4-8=0/68, 4-7=-199/125

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber
DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 5, 57 lb uplift at joint 2 and 35 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=0
Trapezoidal Loads (plf)
Vert: 2=50(F=25, B=25)-to-5=-80(F=-40, B=-40), 2=30(F=15, B=15)-to-9=7(F=3, B=3), 9=7(F=3, B=3)-to-8=-15(F=-7, B=-7), 8=-15(F=-7, B=-7)-to-6=-44(F=-22, B=-22)

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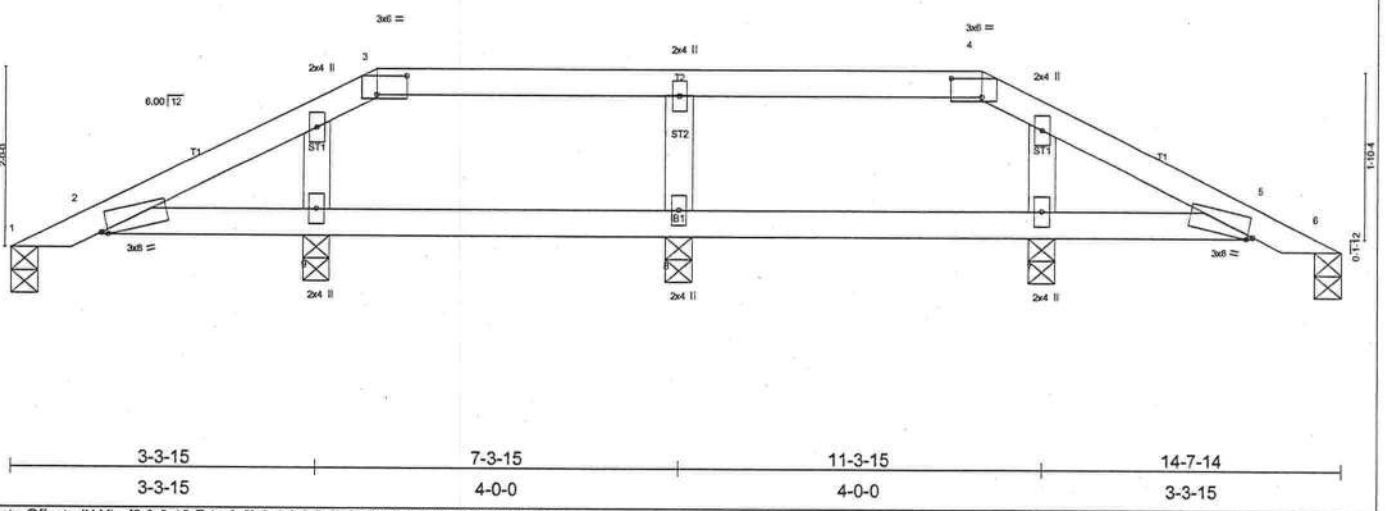


Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-4-0-0-2-8], [4:0-4-0-0-2-8], [5:0-0-13,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	in (loc) l/defi L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.49	Vert(LL) 0.03 2-9 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.03 2-9 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 6 n/a n/a		
	Code FBC2004/TPI2002				Weight: 47 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 1=309/0-3-8, 6=309/0-3-8, 8=74/0-3-8, 9=242/0-3-8, 7=242/0-3-8
 Max Horz 1=-39(load case 6)
 Max Uplift 1=-132(load case 4), 6=-136(load case 3), 9=63(load case 5), 7=-57(load case 6)
 Max Grav 1=309(load case 1), 6=309(load case 1), 8=85(load case 8), 9=248(load case 7), 7=248(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-126/112, 2-3=-721/615, 4-5=-721/615, 5-6=-126/112, 3-4=-640/614
 BOT CHORD 2-9=-487/640, 8-9=-487/640, 7-8=-487/640, 5-7=-487/640

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 1, 136 lb uplift at joint 6, 63 lb uplift at joint 9 and 57 lb uplift at joint 7.

LOAD CASE(S) Standard

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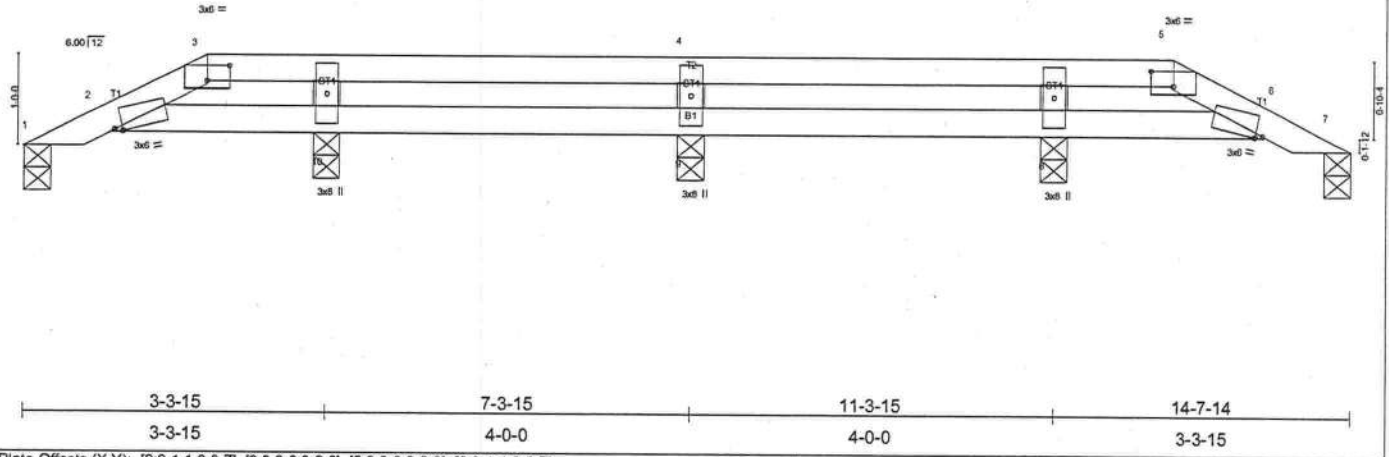


Plate Offsets (X,Y): [2:0-1-1,0-0-7], [3:0-3-0,0-2-0], [5:0-3-0,0-2-0], [6:0-1-1,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.24	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.30	Vert(LL) 0.02 2-10 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.07	Vert(TL) -0.02 2-10 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 43 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS (lb/size) 1=177/0-3-8, 7=177/0-3-8, 9=431/0-3-8, 10=195/0-3-8, 8=195/0-3-8
 Max Horz 1=-19(load case 6)
 Max Uplift 1=71(load case 4), 7=73(load case 3), 9=192(load case 3), 10=-28(load case 4), 8=-26(load case 3)
 Max Grav 1=177(load case 1), 7=177(load case 1), 9=433(load case 8), 10=197(load case 7), 8=197(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-71/62, 2-3=-388/325, 5-6=-388/325, 6-7=-71/62, 3-4=-348/315, 4-5=-348/315
 BOT CHORD 2-10=-254/348, 9-10=-254/348, 8-9=-254/348, 6-8=-254/348
 WEBS 4-9=-339/286

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 1, 73 lb uplift at joint 7, 192 lb uplift at joint 9, 28 lb uplift at joint 10 and 26 lb uplift at joint 8.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Builders FirstSource, Lake City, FL 32055

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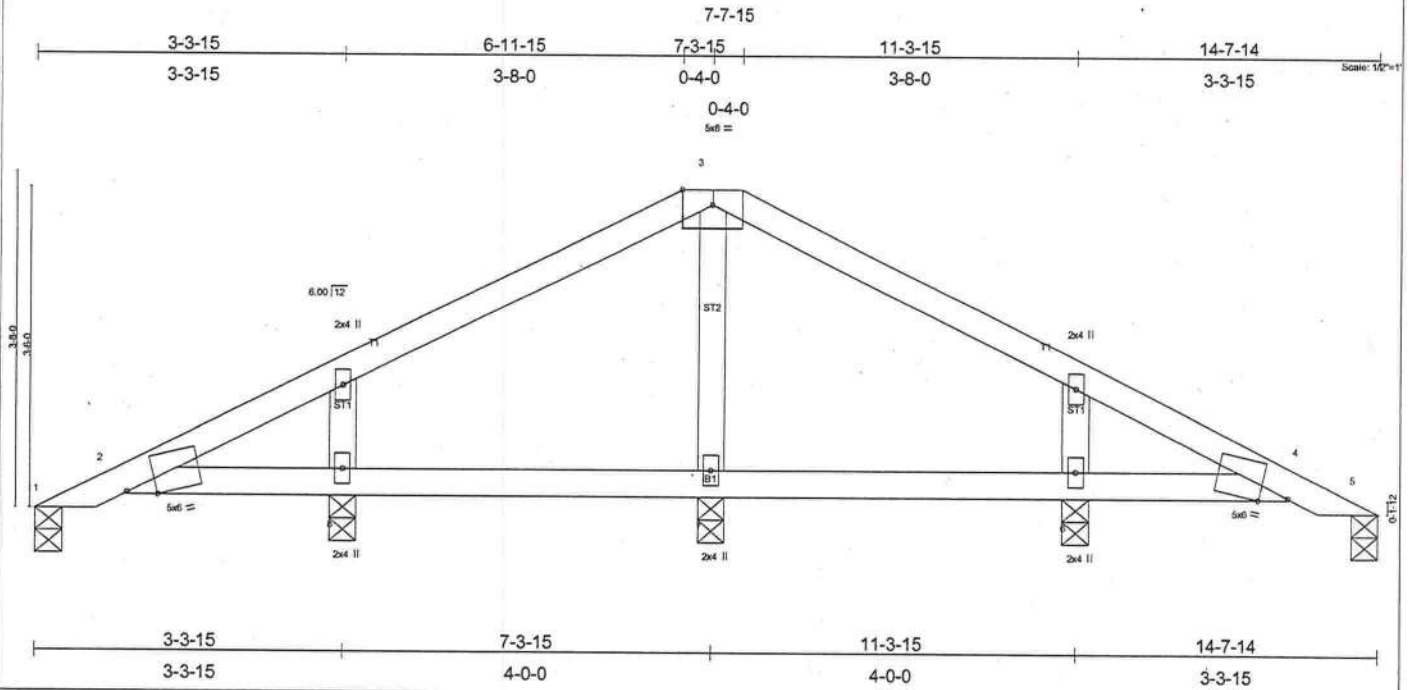


Plate Offsets (X,Y): [2:0-3-13,Edge], [4:0-3-13,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.34	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.63	Vert(LL) 0.05 4-6 >836 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.04 4-6 >882 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.04 5 n/a n/a		
				Weight: 50 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-8-5 oc bracing.

REACTIONS

(lb/size) 1=246/0-3-6, 7=48/0-3-8, 8=317/0-3-8, 6=317/0-3-8, 5=246/0-3-8
 Max Horz 1=-70(load case 6)
 Max Uplift 1=100(load case 5), 8=-134(load case 5), 6=-122(load case 6), 5=-109(load case 6)
 Max Grav 1=246(load case 1), 7=76(load case 7), 8=317(load case 1), 6=317(load case 1), 5=246(load case 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-100/109, 2-3=-489/450, 3-4=-489/450, 4-5=-100/93
 BOT CHORD 2-8=-272/377, 7-8=-272/377, 6-7=-272/377, 4-6=-272/377

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 1, 134 lb uplift at joint 8, 122 lb uplift at joint 6 and 109 lb uplift at joint 5.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

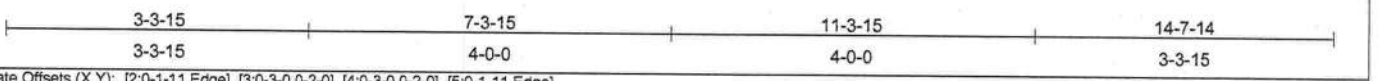
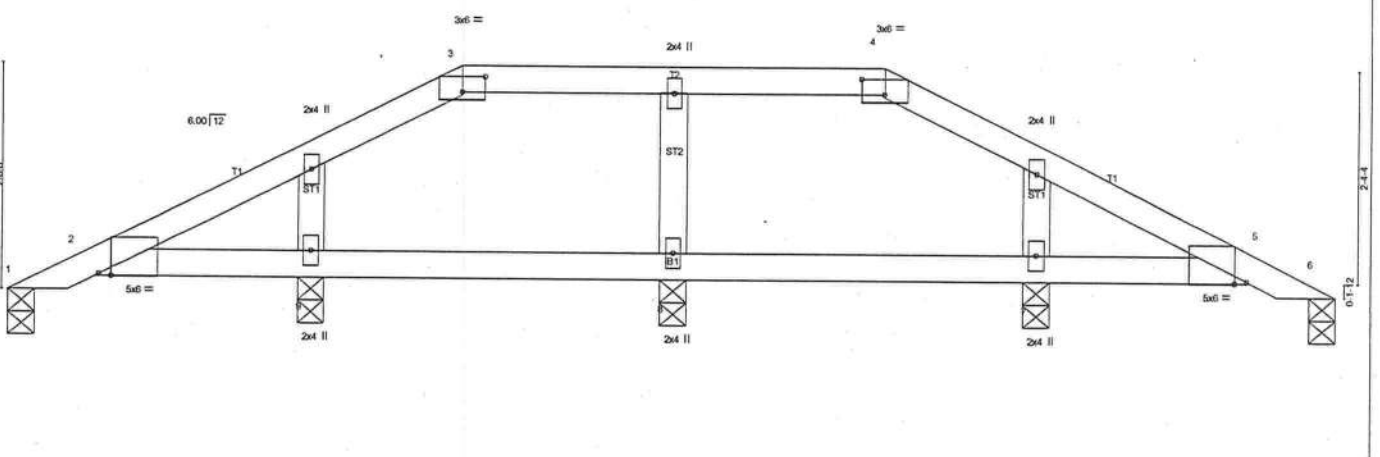
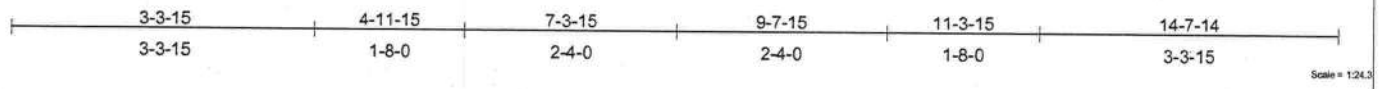


Plate Offsets (X,Y): [2:0-1-11.Edge], [3:0-3-0.0-2-0], [4:0-3-0.0-2-0], [5:0-1-11.Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.54	Vert(LL) 0.03 2-9 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.04 2-9 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 6 n/a n/a		
	Code FBC2004/TPI2002				Weight: 48 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 9-3-13 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 1=285/0-3-8, 6=285/0-3-8, 8=64/0-3-8, 9=271/0-3-8, 7=271/0-3-8
 Max Horz 1=-49(load case 6)
 Max Uplift 1=-112(load case 5), 6=-118(load case 6), 9=-90(load case 5), 7=-82(load case 6)
 Max Grav 1=285(load case 1), 6=285(load case 1), 8=80(load case 8), 9=275(load case 7), 7=275(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-116/104, 2-3=-586/509, 4-5=-586/509, 5-6=-116/104, 3-4=-494/517
 BOT CHORD 2-9=-358/494, 8-9=-358/494, 7-8=-358/494, 5-7=-358/494

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 1, 118 lb uplift at joint 6, 90 lb uplift at joint 9 and 82 lb uplift at joint 7.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

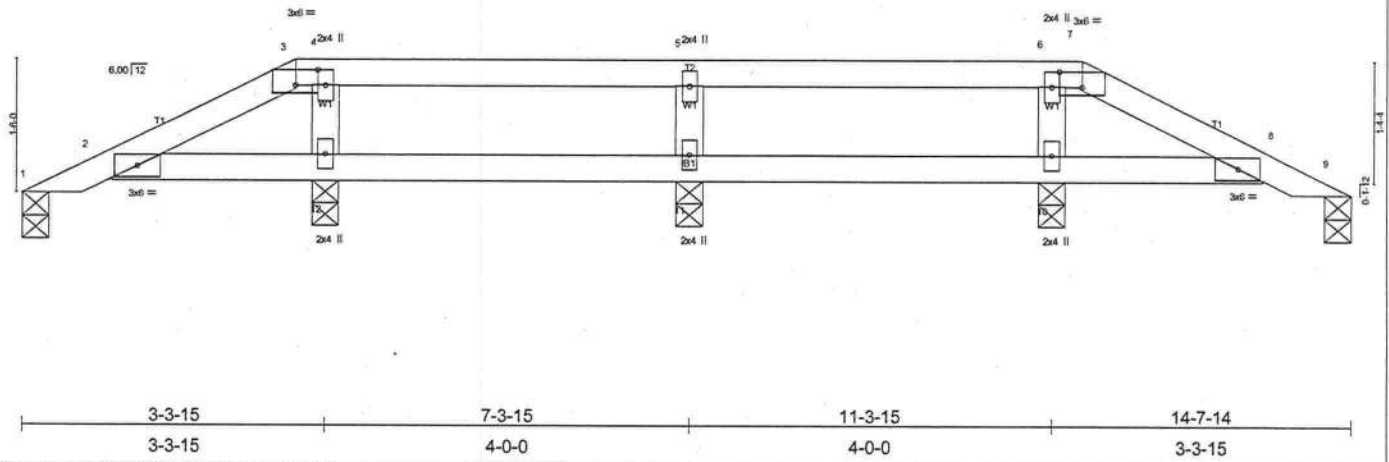
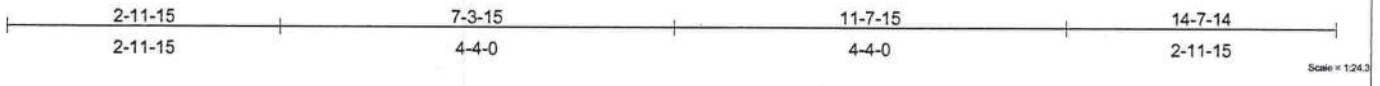


Plate Offsets (X,Y): [3:0-3-0,0-2-0], [7:0-3-0,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0 Plates Increase 1.25	TC 0.13	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(LL) -0.00 10-11 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.05	Vert(TL) -0.01 10-11 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.00 9 n/a n/a		
				Weight: 45 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
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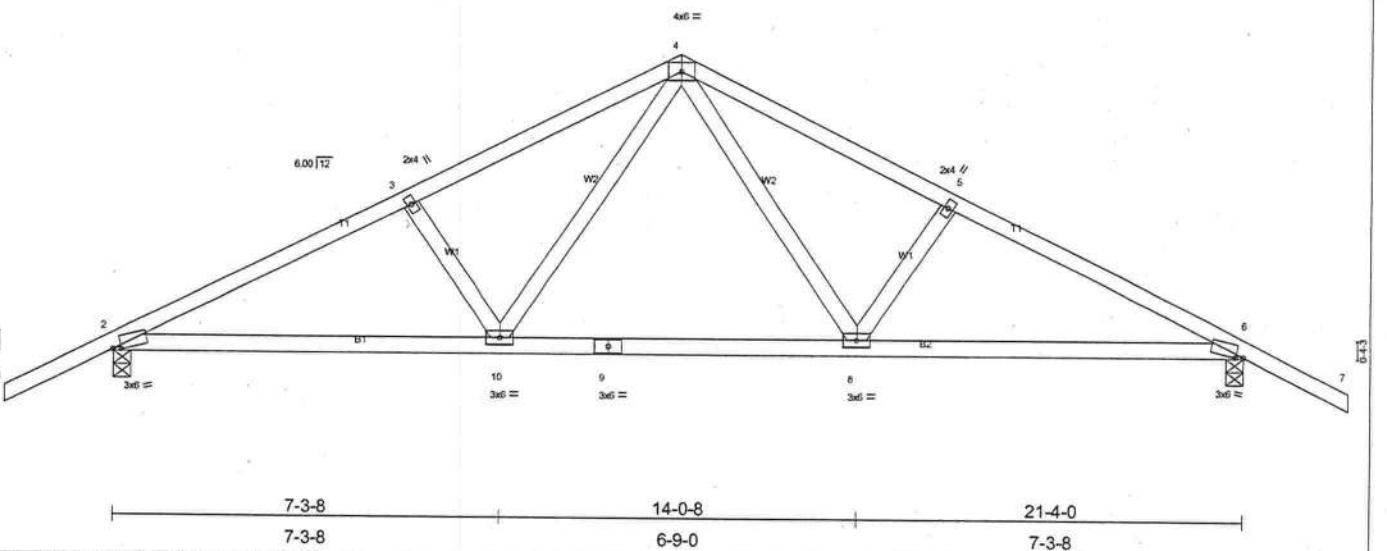
REACTIONS (lb/size) 1=61/0-3-8, 9=61/0-3-8, 11=340/0-3-8, 12=356/0-3-8, 10=356/0-3-8
 Max Horz 1=-29(load case 6)
 Max Uplift 1=-27(load case 5), 9=-37(load case 6), 11=-140(load case 3), 12=-102(load case 4), 10=-95(load case 3)
 Max Grav 1=69(load case 7), 9=69(load case 8), 11=350(load case 7), 12=356(load case 1), 10=356(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-26/38, 2-3=-54/118, 7-8=-54/118, 8-9=-26/25, 3-4=-6/75, 4-5=-6/75, 5-6=-6/75, 6-7=-6/75
 BOT CHORD 2-12=-75/100, 11-12=-75/100, 10-11=-75/100, 8-10=-75/100
 WEBS 5-11=-226/185, 4-12=-235/195, 6-10=-235/195

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1, 37 lb uplift at joint 9, 140 lb uplift at joint 11, 102 lb uplift at joint 12 and 95 lb uplift at joint 10.

LOAD CASE(S) Standard

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.79	Vert(LL) -0.19 8-10 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.23	Vert(TL) -0.31 8-10 >809 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.05 6 n/a n/a		
	Code FBC2004/TPI2002				Weight: 101 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-5-12 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-8-11 oc bracing.

REACTIONS (lb/size) 2=1168/0-4-0, 6=1168/0-4-0
 Max Horz 2=-150(load case 6)
 Max Uplift 2=458(load case 5), 6=-458(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-1859/933, 3-4=-1705/928, 4-5=-1705/928, 5-6=-1859/933, 6-7=0/47
 BOT CHORD 2-10=-650/1590, 9-10=-334/1098, 8-9=-334/1098, 6-8=-650/1590
 WEBS 3-10=-239/266, 4-10=-327/717, 4-8=-327/717, 5-8=-239/266

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 458 lb uplift at joint 2 and 458 lb uplift at joint 6.
 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

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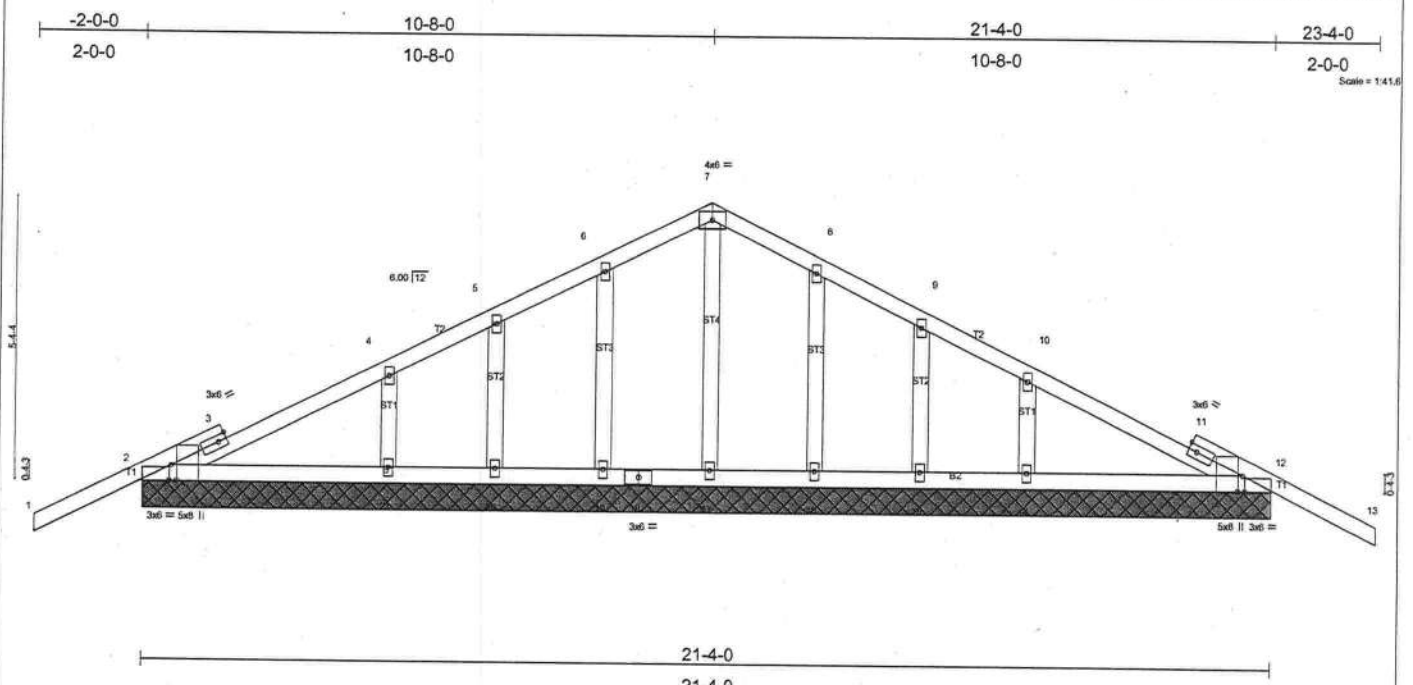


Plate Offsets (X,Y): [2-0-3-8,Edge], [2-0-0-8,Edge], [12-0-3-8,Edge], [12-0-0-8,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.49	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.13	Vert(LL) -0.02 13 n/r 120		
BCLL 10.0	Lumber Increase 1.25	WB 0.15	Vert(TL) -0.02 13 n/r 90		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 12 n/a n/a		
	Code FBC2004/TPI2002				Weight: 109 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=518/21-4-0, 12=518/21-4-0, 17=381/21-4-0, 19=325/21-4-0, 20=165/21-4-0, 21=565/21-4-0, 16=325/21-4-0, 15=165/21-4-0, 14=565/21-4-0
 Max Horz 2=-144(load case 6)
 Max Uplift 2=-263(load case 5), 12=-284(load case 6), 17=-35(load case 5), 19=-138(load case 5), 20=-105(load case 5), 21=-204(load case 5), 16=-137(load case 6), 15=-102(load case 6), 14=-211(load case 6)
 Max Grav 2=525(load case 7), 12=525(load case 8), 17=381(load case 1), 19=329(load case 7), 20=165(load case 1), 21=568(load case 7), 16=329(load case 8), 15=165(load case 1), 14=568(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-20/99, 2-3=-117/63, 3-4=-130/237, 4-5=-42/151, 5-6=-4/188, 6-7=0/176, 7-8=0/176, 8-9=0/188, 9-10=0/151, 10-11=-82/237, 11-12=-61/64, 12-13=-20/99
 BOT CHORD 2-21=-116/200, 20-21=-116/200, 19-20=-116/200, 18-19=-116/200, 17-18=-116/200, 16-17=-116/200, 15-16=-116/200, 14-15=-116/200, 12-14=-116/200
 WEBS 7-17=-327/47, 6-19=-257/196, 5-20=-146/147, 4-21=-422/312, 8-16=-257/196, 9-15=-146/147, 10-14=-422/312

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2, 284 lb uplift at joint 12, 35 lb uplift at joint 17, 138 lb uplift at joint 19, 105 lb uplift at joint 20, 204 lb uplift at joint 21, 137 lb uplift at joint 16, 102 lb uplift at joint 15 and 211 lb uplift at joint 14.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-7=-114(F=-60), 7-13=-114(F=-60), 2-12=-30

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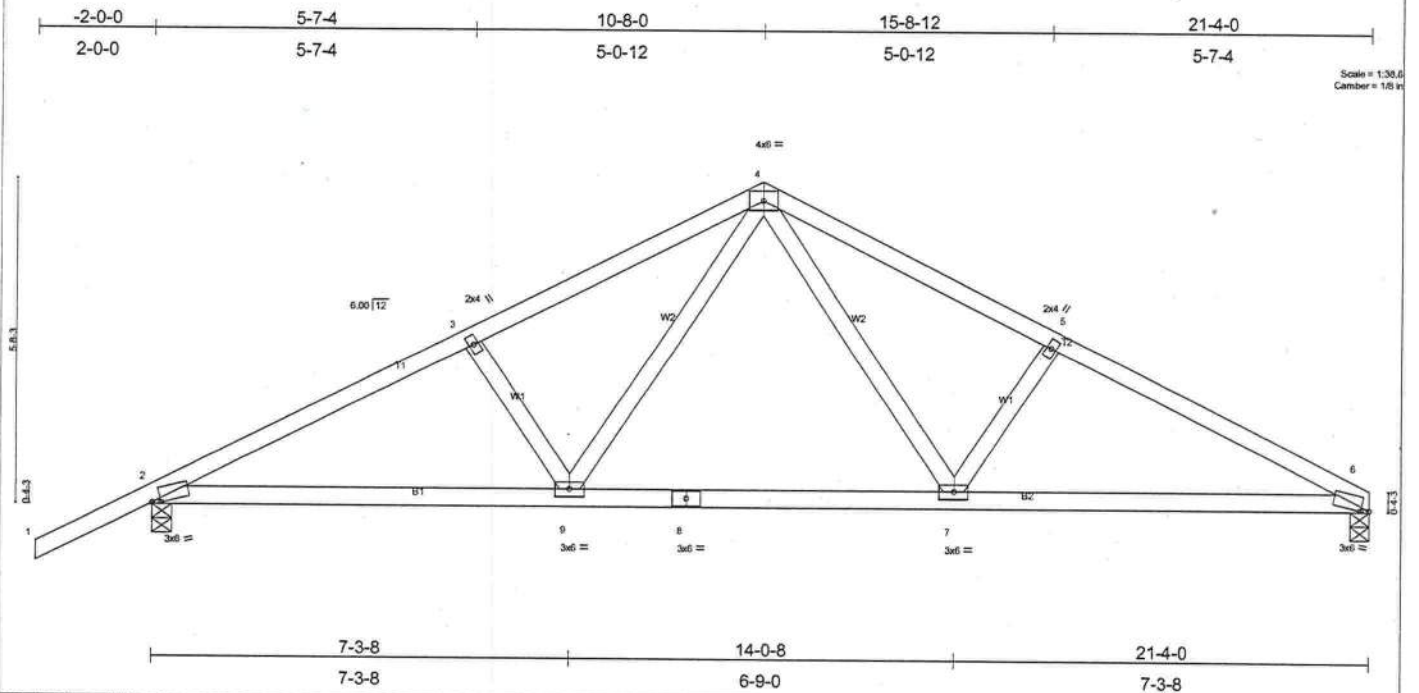


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.39	Vert(LL) -0.19 7-9 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.83	Vert(TL) -0.31 7-9 >823 180		
BCLL 10.0	Rep Stress Incr NO	WB 0.27	Horz(TL) 0.05 6 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 98 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-10-9 oc bracing.
--	---

REACTIONS (lb/size) 6=1045/0-4-0, 2=1174/0-4-0
 Max Horz 2=169(load case 5)
 Max Uplift 6=329(load case 6), 2=460(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-1872/953, 3-4=-1717/948, 4-5=-1741/986, 5-6=-1883/993
 BOT CHORD 2-9=-746/1601, 8-9=-432/1110, 7-8=-432/1110, 6-7=-792/1629
 WEBS 3-9=-240/268, 4-9=-322/716, 4-7=-382/750, 5-7=-257/295

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 6 and 460 lb uplift at joint 2.
 - 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

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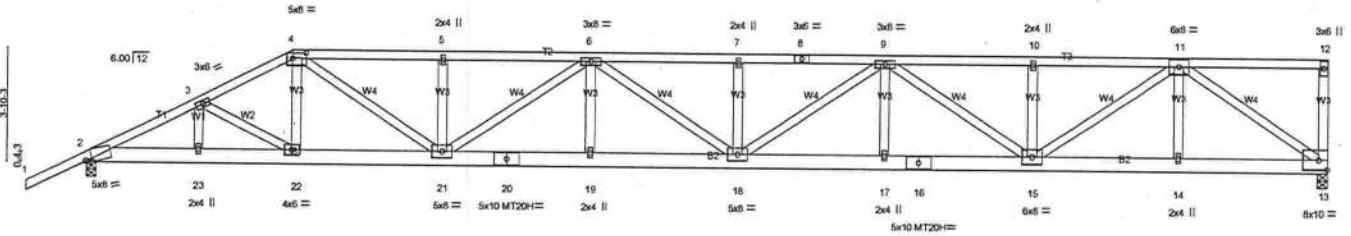
Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

6.200 s Jul 13 2005 MITek Industries, Inc. Fri Sep 23 07:57:55 2005 Page 1

-2-0-0	3-9-4	7-0-0	12-1-13	17-1-14	22-1-15	27-2-1	32-2-2	37-2-3	42-4-0
2-0-0	3-9-4	3-2-12	5-1-13	5-0-1	5-0-1	5-0-1	5-0-1	5-0-1	5-1-13

Scale = 1/75.0
Camber = 3/8 in



3-9-4	7-0-0	12-1-13	17-1-14	22-1-15	27-2-1	32-2-2	37-2-3	42-4-0
3-9-4	3-2-12	5-1-13	5-0-1	5-0-1	5-0-1	5-0-1	5-0-1	5-1-13

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-5-8,0-2-4]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.56	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.68	Vert(LL) -0.61 18-19 >832 240	MT20H	187/143
BCLL 10.0	Lumber Increase 1.25	WB 0.89	Vert(TL) -0.97 18-19 >520 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.16 13 n/a n/a		
	Code FBC2004/TP12002				Weight: 539 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-0-9 oc bracing.

REACTIONS (lb/size) 13=3855/0-4-0, 2=3774/0-4-0
Max Horz 2=228(load case 4)
Max Uplift 13=-1738(load case 3), 2=-1555(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-7383/3147, 3-4=-7525/3320, 4-5=-9720/4364, 5-6=-9720/4365, 6-7=-11726/5277, 7-8=-11726/5277, 8-9=-11726/5277, 9-10=-8520/3838, 10-11=-8520/3838, 11-12=-113/63, 12-13=-286/224
BOT CHORD 2-23=-2861/6521, 22-23=-2861/6521, 21-22=-3020/6762, 20-21=-5154/11455, 19-20=-5154/11455, 18-19=-5154/11455, 17-18=-4887/10847, 16-17=-4887/10847, 15-16=-4887/10847, 14-15=-2247/4989, 13-14=-2247/4989
WEBS 3-23=-218/158, 3-22=-253/232, 4-22=-288/896, 4-21=-1643/3564, 5-21=-570/486, 6-21=-2112/978, 6-19=0/413, 6-18=-170/330, 7-18=-562/466, 9-18=-474/1070, 9-17=0/397, 9-15=-2832/1277, 10-15=-565/463, 11-15=-1936/4296, 11-14=0/330, 11-13=-5935/2657

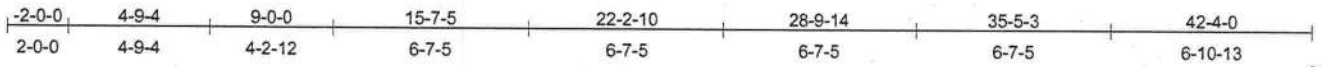
NOTES

- 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1738 lb uplift at joint 13 and 1555 lb uplift at joint 2.
- Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

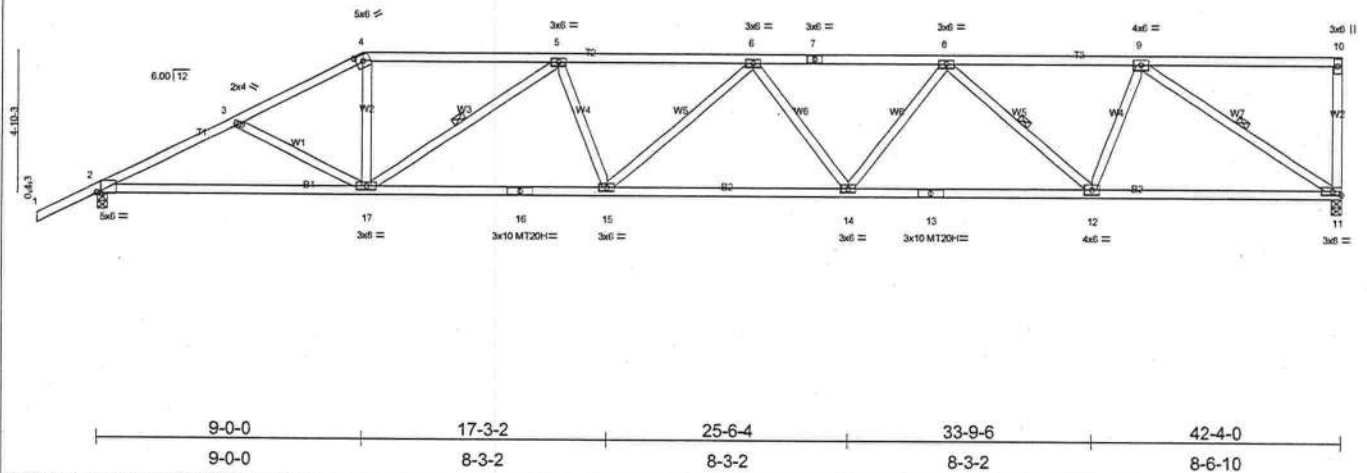
LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-12=-117(F=-63), 2-22=-30, 13-22=-65(F=-35)
Concentrated Loads (lb)
Vert: 22=-539(F)

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Scale = 1/75.0
Camber = 5/16 in



LOADING (psf)		SPACING		CSI		DEFL				PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.61	in	(loc)	l/defl	L/d	MT20	244/190		
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(LL)	-0.49 14-15	>999	240	MT20H	187/143		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Vert(TL)	-0.79 14-15	>637	180				
BCLD	5.0	Code	FBC2004/TPI2002	(Matrix)		Horz(TL)	0.23 11	n/a	n/a				
										Weight: 223 lb			

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-11-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-10-7 oc bracing.
 WEBS 1 Row at midpt 5-17, 8-12, 9-11

REACTIONS (lb/size) 11=1762/0-4-0, 2=1885/0-4-0
 Max Horz 2=272(load case 5)
 Max Uplift 11=656(load case 4), 2=547(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-3322/1402, 3-4=-3125/1290, 4-5=-2789/1228, 5-6=-3936/1630, 6-7=-3895/1569, 7-8=-3895/1569, 8-9=-2631/1041, 9-10=-69/19, 10-11=-177/121
 BOT CHORD 2-17=-1415/2904, 16-17=-1621/3806, 15-16=-1621/3806, 14-15=-1706/4066, 13-14=-1467/3543, 12-13=-1467/3543, 11-12=-920/2235
 WEBS 3-17=-167/222, 4-17=-320/1063, 5-17=-1244/560, 5-15=-70/381, 6-15=-177/167, 6-14=-292/233, 8-14=-173/599, 8-12=-1239/579, 9-12=-355/1161, 9-11=-2613/1067

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Provide adequate drainage to prevent water ponding.
 3) All plates are MT20 plates unless otherwise indicated.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 656 lb uplift at joint 11 and 547 lb uplift at joint 2.

LOAD CASE(S) Standard

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 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Scale = 1/75.0
Camber = 1/4 in

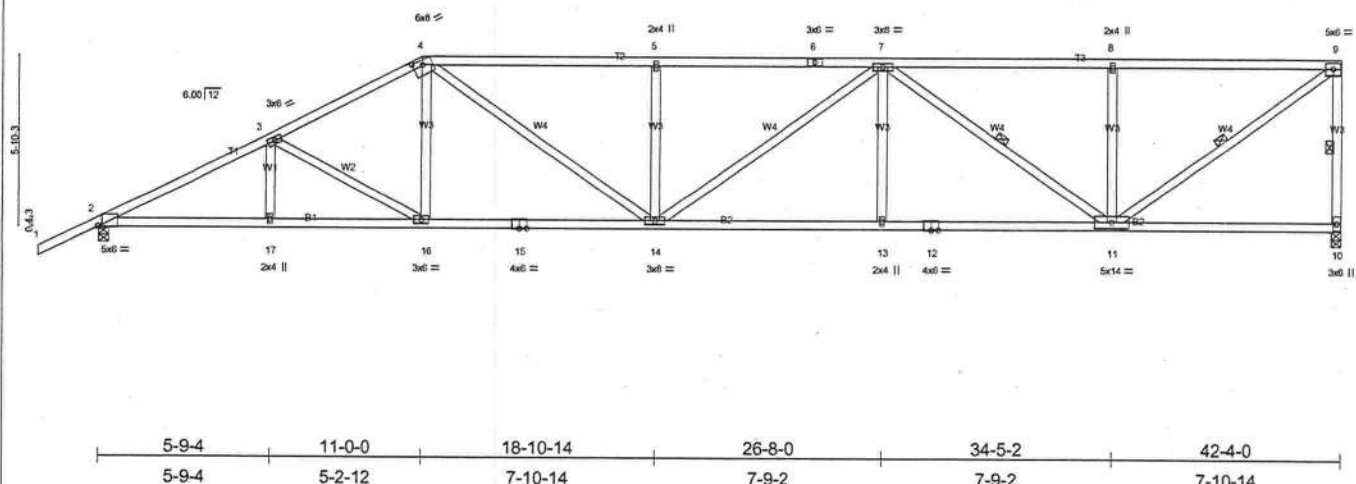


Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-4-0-0-1-15]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.77	Vert(LL) -0.37 13-14 >999 240	MT20	244/190
BCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(TL) -0.60 13-14 >837 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.80	Horz(TL) 0.17 10 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 237 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-15 oc bracing.
WEBS 1 Row at midpt 9-10, 7-11, 9-11

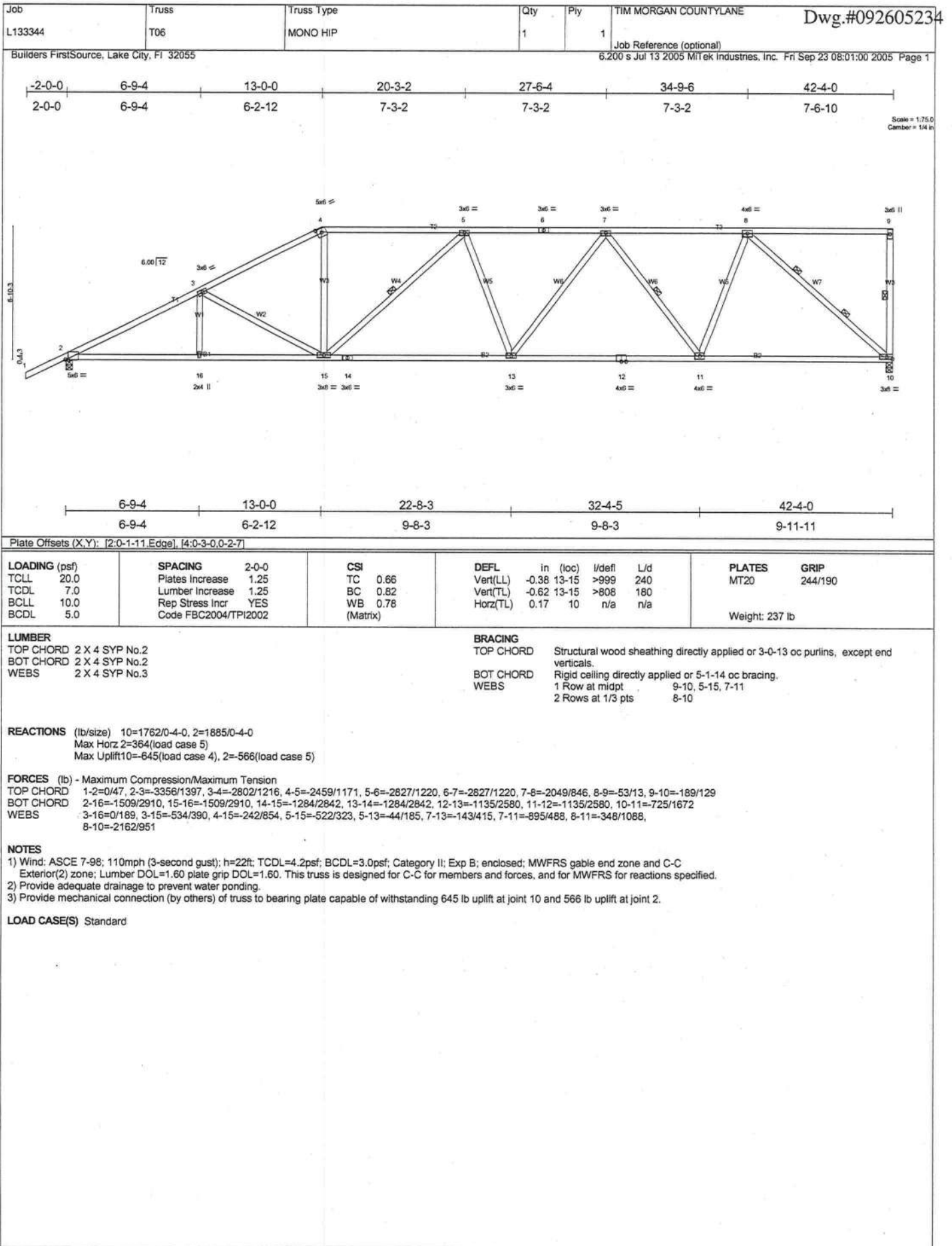
REACTIONS (lb/size) 10=1762/0-4-0, 2=1885/0-4-0
Max Horz 2=318(load case 5)
Max Uplift 10=651(load case 4), 2=544(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3383/1387, 3-4=-2955/1267, 4-5=-3363/1451, 5-6=-3362/1451, 6-7=-3362/1451, 7-8=-2083/860, 8-9=-2083/860, 9-10=-1646/736
BOT CHORD 2-17=-1453/2936, 16-17=-1453/2936, 15-16=-1208/2605, 14-15=-1208/2605, 13-14=-1332/3171, 12-13=-1332/3171, 11-12=-1332/3171, 10-11=-23/58
WEBS 3-17=0/151, 3-16=-388/282, 4-16=-101/395, 4-14=-429/924, 5-14=-431/309, 7-14=-147/235, 7-13=0/231, 7-11=-1338/581, 8-11=-436/318, 9-11=-1029/2490

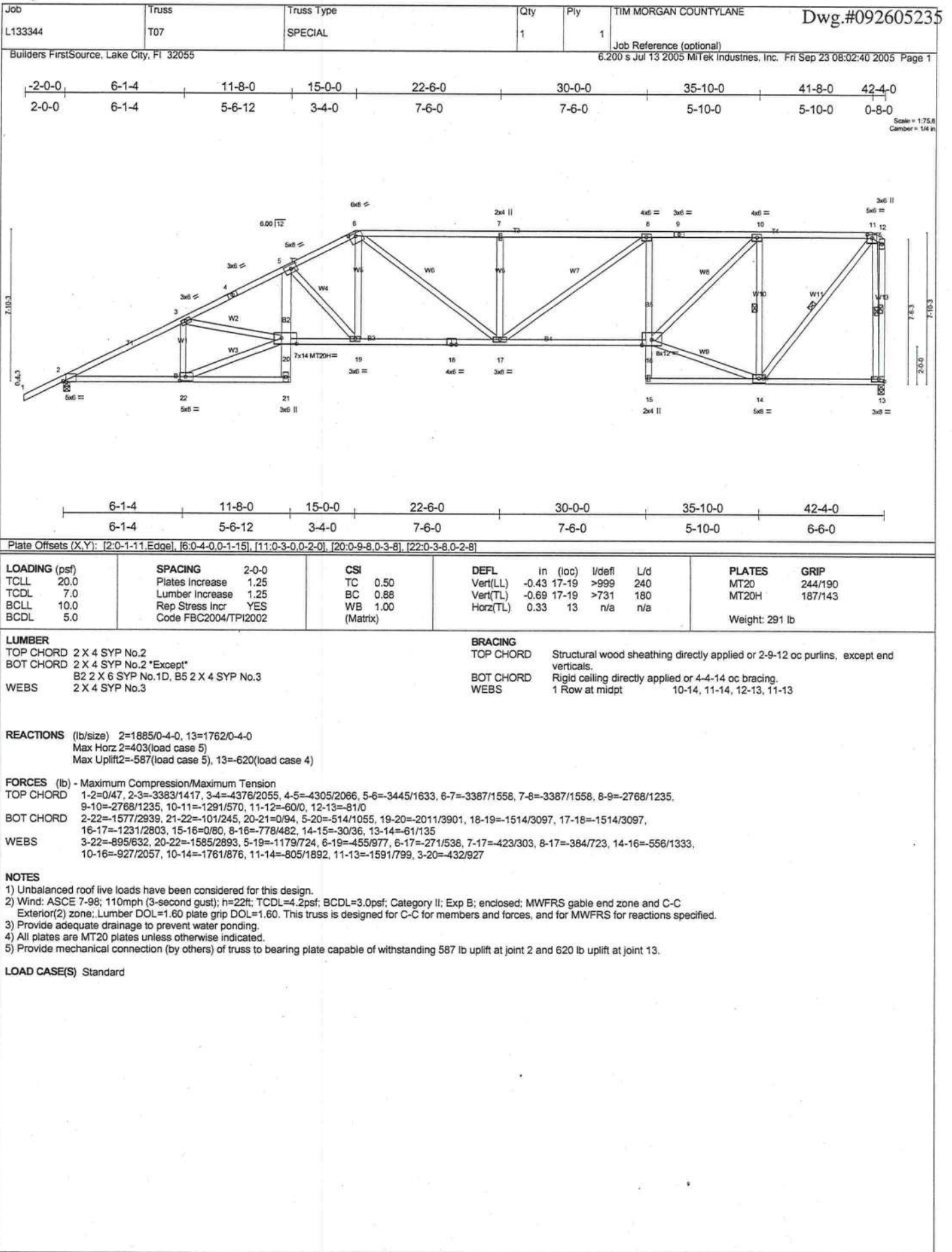
NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 10 and 544 lb uplift at joint 2.

LOAD CASE(S) Standard

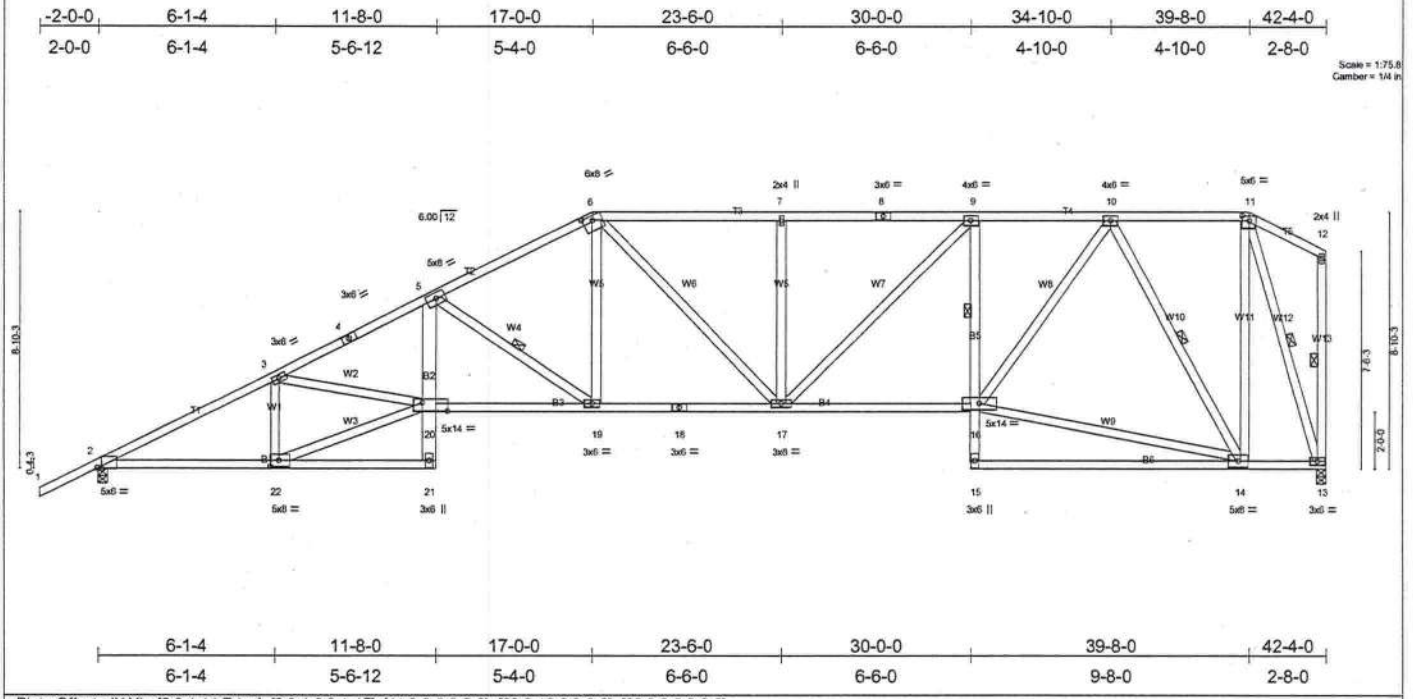
SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.86	Vert(LL) -0.39 19-20 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.97	Vert(TL) -0.63 19-20 >804 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.31 13 n/a n/a		
	Code FBC2004/TPI2002				Weight: 307 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-9-12 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-3-14 oc bracing. Except:
 WEBS 1 Row at midpt 9-16
 1 Row at midpt 5-19, 10-14, 12-13, 11-13

REACTIONS (lb/size) 2=1885/0-4-0, 13=1762/0-4-0
 Max Horiz 2=423(load case 5)
 Max Uplift 2=608(load case 5), 13=547(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-3383/1447, 3-4=-4387/2097, 4-5=-4317/2108, 5-6=-3118/1497, 6-7=-2831/1374, 7-8=-2831/1375, 8-9=-2831/1375,
 9-10=-2325/1114, 10-11=-569/308, 11-12=-28/44, 12-13=-50/65
 BOT CHORD 2-22=-1604/2938, 21-22=-119/230, 20-21=0/94, 5-20=-500/1100, 19-20=-2090/3961, 18-19=-1345/2753, 17-18=-1345/2753,
 16-17=-1047/2351, 15-16=0/145, 9-16=-783/474, 14-15=-11/127, 13-14=-213/512
 WEBS 3-22=-899/636, 20-22=-1595/2907, 5-19=-1470/903, 6-19=-442/952, 6-17=-200/313, 7-17=-370/263, 9-17=-356/680, 14-16=-633/1294,
 10-16=-704/1603, 10-14=-1765/847, 11-14=-548/1506, 11-13=-1787/733, 3-20=-444/939

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 608 lb uplift at joint 2 and 547 lb uplift at joint 13.

LOAD CASE(S) Standard

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Scale = 1/75.8
 Camber = 1/4 in

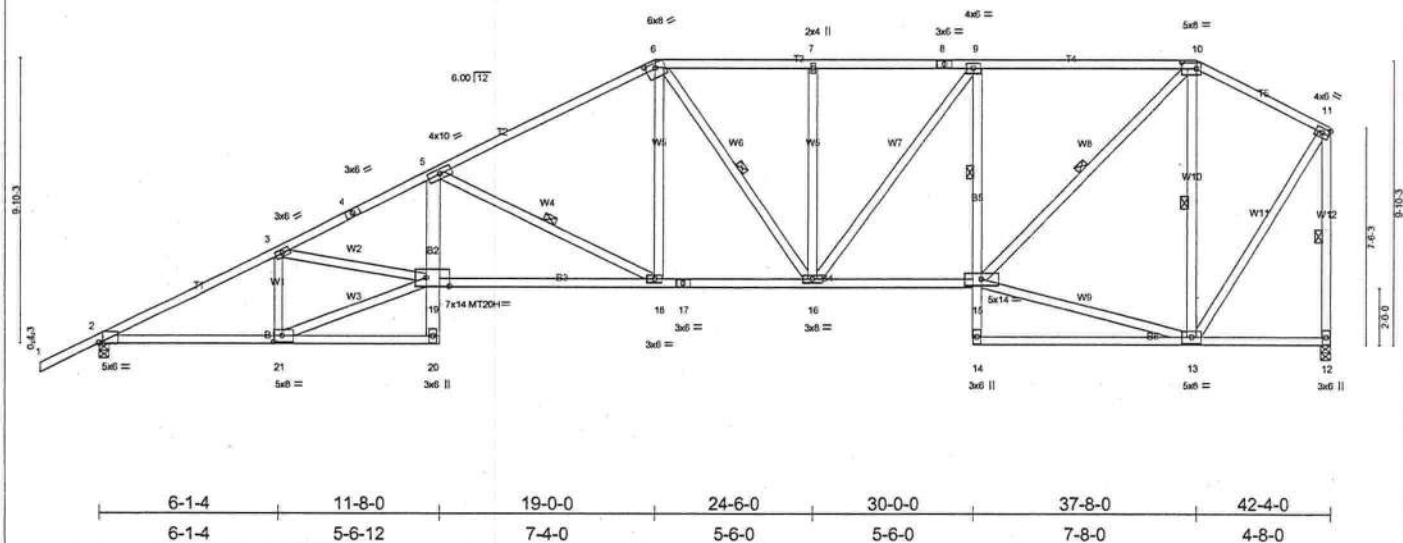


Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-4-0-0-1-15], [10:0-6-0-0-2-8], [19:0-9-12-0-3-8], [21:0-3-8-0-2-8]

LOADING (psf) TCCL 20.0 TCCL 7.0 BCLL 10.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.50 BC 0.85 WB 0.94 (Matrix)	DEFL in (loc) l/def L/d Vert(LL) -0.43 18-19 >999 240 Vert(TL) -0.70 18-19 >722 180 Horz(TL) 0.30 12 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 303 lb
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LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 B2 2 X 6 SYP No.1D, B5 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-9-14 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-2-9 oc bracing. Except:
 1 Row at midpt 9-15
 WEBS 1 Row at midpt 5-18, 6-16, 10-15, 10-13, 11-12

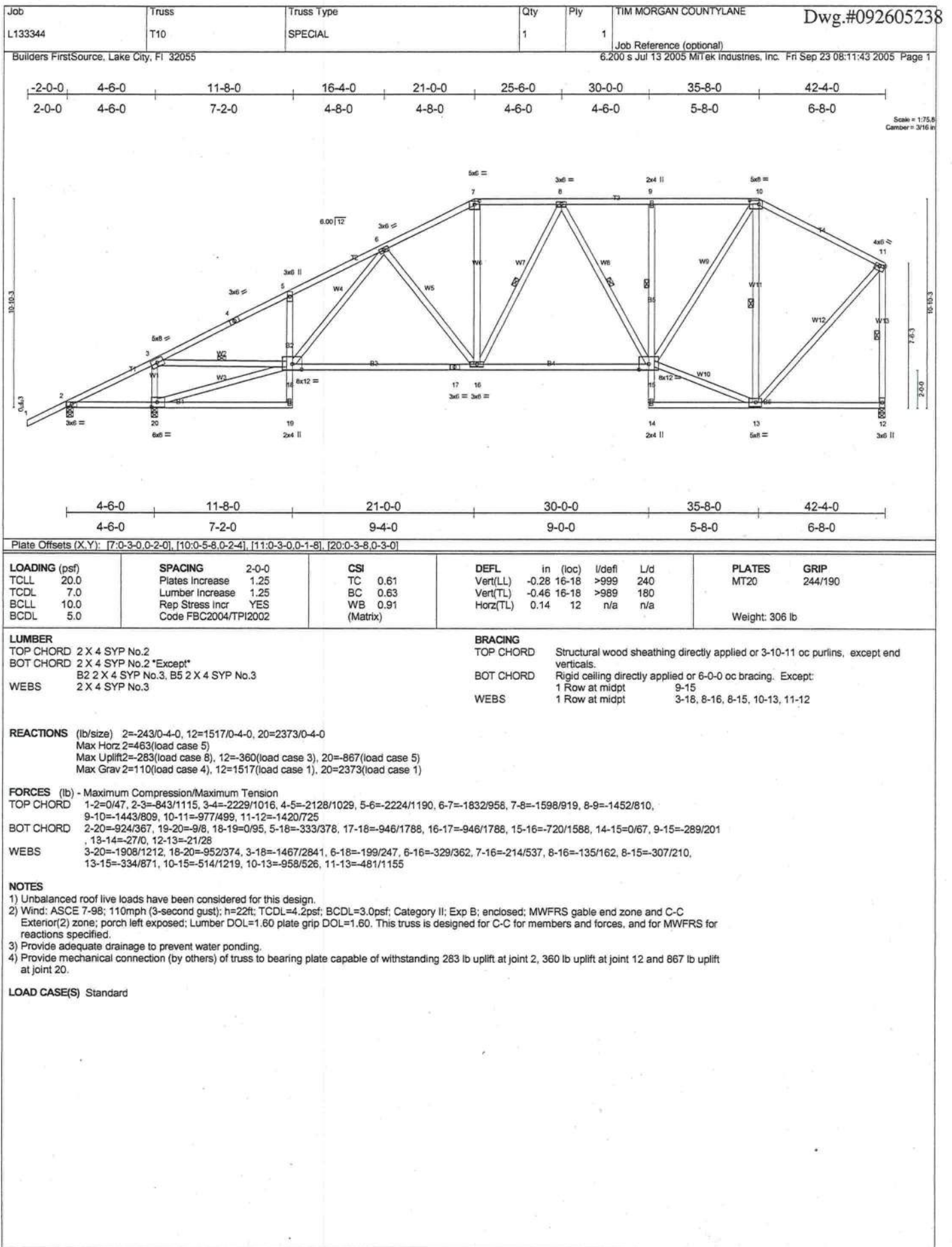
REACTIONS (lb/size) 2=1885/0-4-0, 12=1762/0-4-0
 Max Horz 2=443(load case 5)
 Max Uplift 2=626(load case 5), 12=476(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-3382/1477, 3-4=-4396/2135, 4-5=-4327/2146, 5-6=-2835/1384, 6-7=-2406/1244, 7-8=-2406/1244, 8-9=-2406/1244, 9-10=-2040/1055, 10-11=-927/457, 11-12=-1709/819
 BOT CHORD 2-21=-1630/2937, 20-21=-145/220, 19-20=0/94, 5-19=-488/1132, 18-19=-2176/4026, 17-18=-1206/2470, 16-17=-1206/2470, 15-16=-921/2044, 14-15=0/112, 9-15=-880/526, 13-14=-4/63, 12-13=-8/9
 WEBS 3-21=-903/636, 19-21=-1595/2917, 5-18=-1750/1088, 6-18=-422/938, 6-16=-146/190, 7-16=-285/204, 9-16=-318/617, 13-15=-345/740, 10-15=-829/1793, 10-13=-1227/669, 11-13=-627/1457, 3-19=-454/950

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) All plates are MT20 plates unless otherwise indicated.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 626 lb uplift at joint 2 and 476 lb uplift at joint 12.

LOAD CASE(S) Standard

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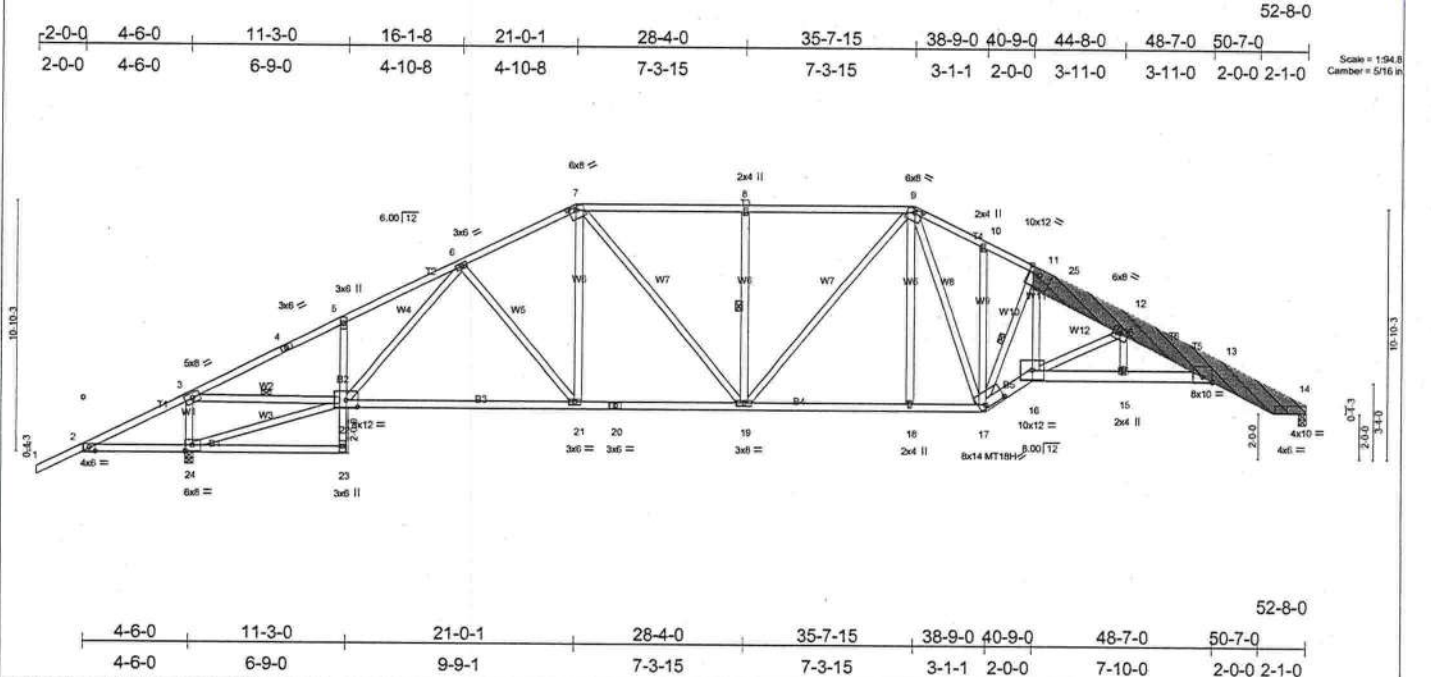


Plate Offsets (X,Y): [7:0-4-0,0-1-15], [9:0-4-0,0-1-15], [17:0-10-8,0-1-12], [22:11-3-0,0-0-9], [22:11-3-0,0-0-9], [24:0-3-8,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0 Lumber Increase 1.25	TC 0.86	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Rep Stress Incr YES	BC 0.82	Vert(LL) -0.51 18-19 >999 240	MT18H	244/190
BCLL 10.0	Code FBC2004/TPI2002	WB 1.00	Vert(TL) -0.84 21-22 >682 180		
BCDL 5.0		(Matrix)	Horz(TL) 0.53 14 n/a n/a		
				Weight: 446 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T6 2 X 12 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 6 SYP No.1D, B6 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2
LBR SCAB 11-14 2 X 12 SYP No.2 one side

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-1 oc bracing.
WEBS 1 Row at midpt 3-22, 8-19, 11-17

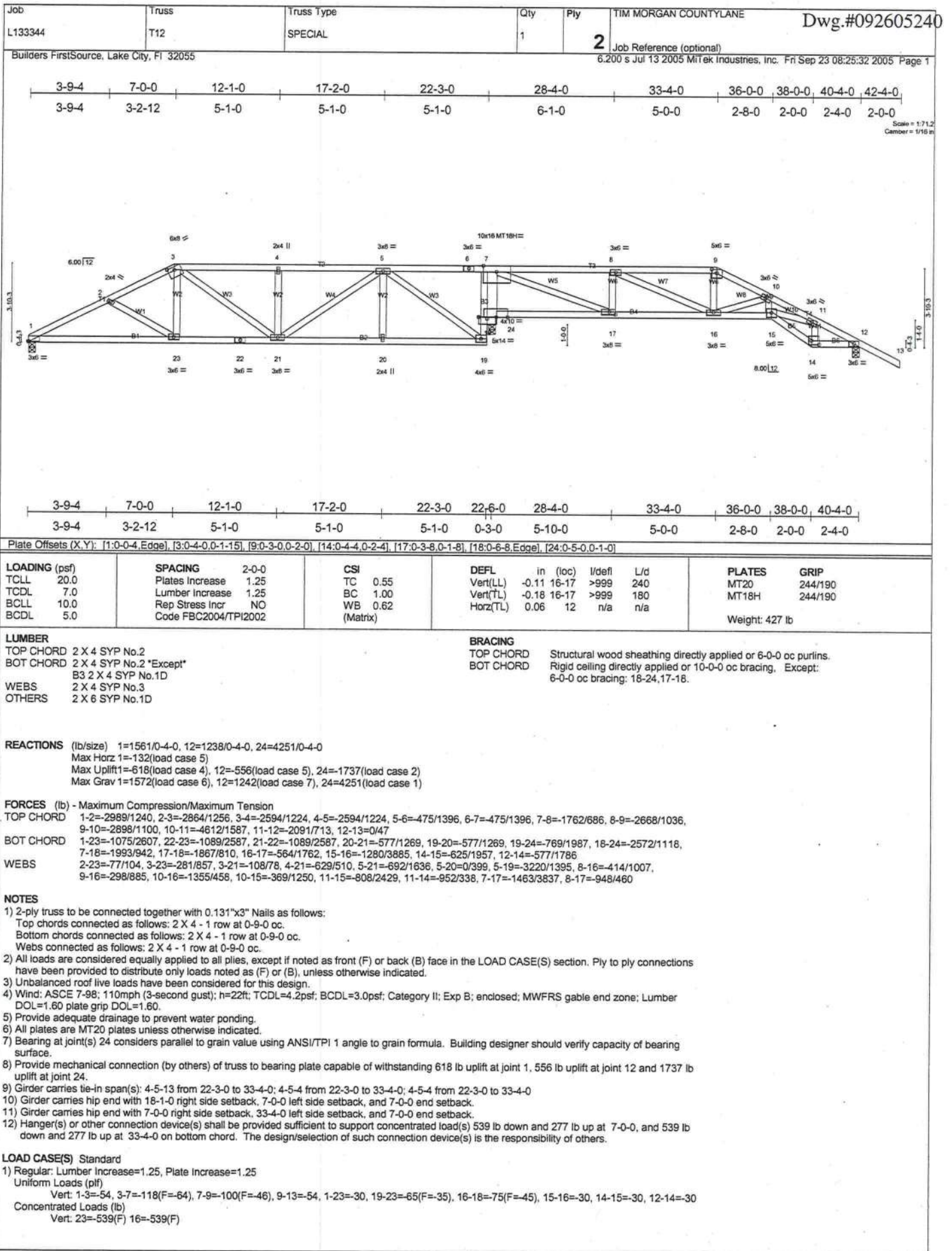
REACTIONS (lb/size) 24=2529/0-4-0, 14=1914/0-4-0
Max Horz 24=318(load case 5)
Max Uplift 24=-925(load case 5), 14=-531(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-734/616, 3-4=-3330/1335, 4-5=-3229/1347, 5-6=-3354/1510, 6-7=-2819/1394, 7-8=-2777/1457, 8-9=-2777/1457, 9-10=-3106/1673, 10-11=-3187/1638, 11-12=-4196/2012, 12-13=-4269/2000, 12-13=-5237/2418, 13-14=-738/389
BOT CHORD 2-24=-488/780, 23-24=-13/4, 22-23=0/90, 5-22=-335/371, 21-22=-997/2710, 20-21=-803/2486, 19-20=-803/2486, 18-19=-872/2541, 17-18=-873/2538, 16-17=-1905/4793, 15-16=-2190/5043, 13-15=-2188/5033
WEBS 3-24=-2185/1298, 22-24=-500/622, 3-22=-1668/3393, 6-22=-133/376, 6-21=-373/310, 7-21=-188/562, 7-19=-265/562, 8-19=-419/297, 9-19=-233/521, 9-18=0/152, 9-17=-503/690, 10-17=-68/145, 11-17=-3363/1516, 11-16=-1269/3104, 12-16=-1333/734, 12-15=-92/386

- NOTES**
- Attached 13-5-12 scab 11 to 14, front face(s) 2 X 12 SYP No.2 with 2 row(s) of 0.131"x3" Nails spaced 9" o.c. except : starting at 0-2-15 from end at joint 11, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 7-10-14 from end at joint 11, nail 3 row(s) at 4 o.c. for 2-0-0.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 925 lb uplift at joint 24 and 531 lb uplift at joint 14.

LOAD CASE(S) Standard

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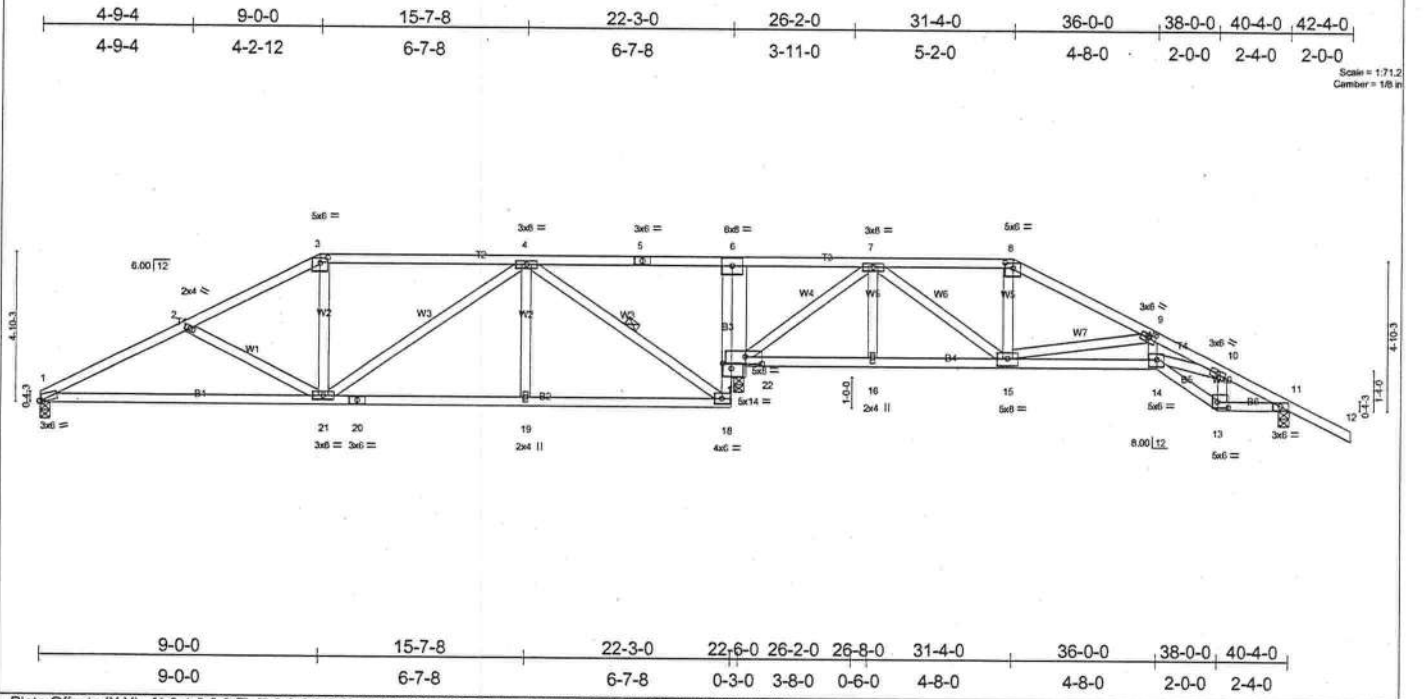


Plate Offsets (X,Y): [1:0-1-0-0-7], [3:0-3-0-0-2-0], [8:0-3-0-0-2-0], [13:0-4-4-0-2-4], [17:0-6-7-0-2-7], [22:0-3-7-0-2-1]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.42	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.86	Vert(LL) -0.18 1-21 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.58	Vert(TL) -0.30 1-21 >875 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 11 n/a n/a		
	Code FBC2004/TPI2002				Weight: 220 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-3-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
B3 2 X 4 SYP No.1D	8-11-3 oc bracing: 1-21
WEBS 2 X 4 SYP No.3	6-0-0 oc bracing: 17-22
OTHERS 2 X 6 SYP No.1D	8-1-12 oc bracing: 14-15.
	WEBS 1 Row at midpt 4-18

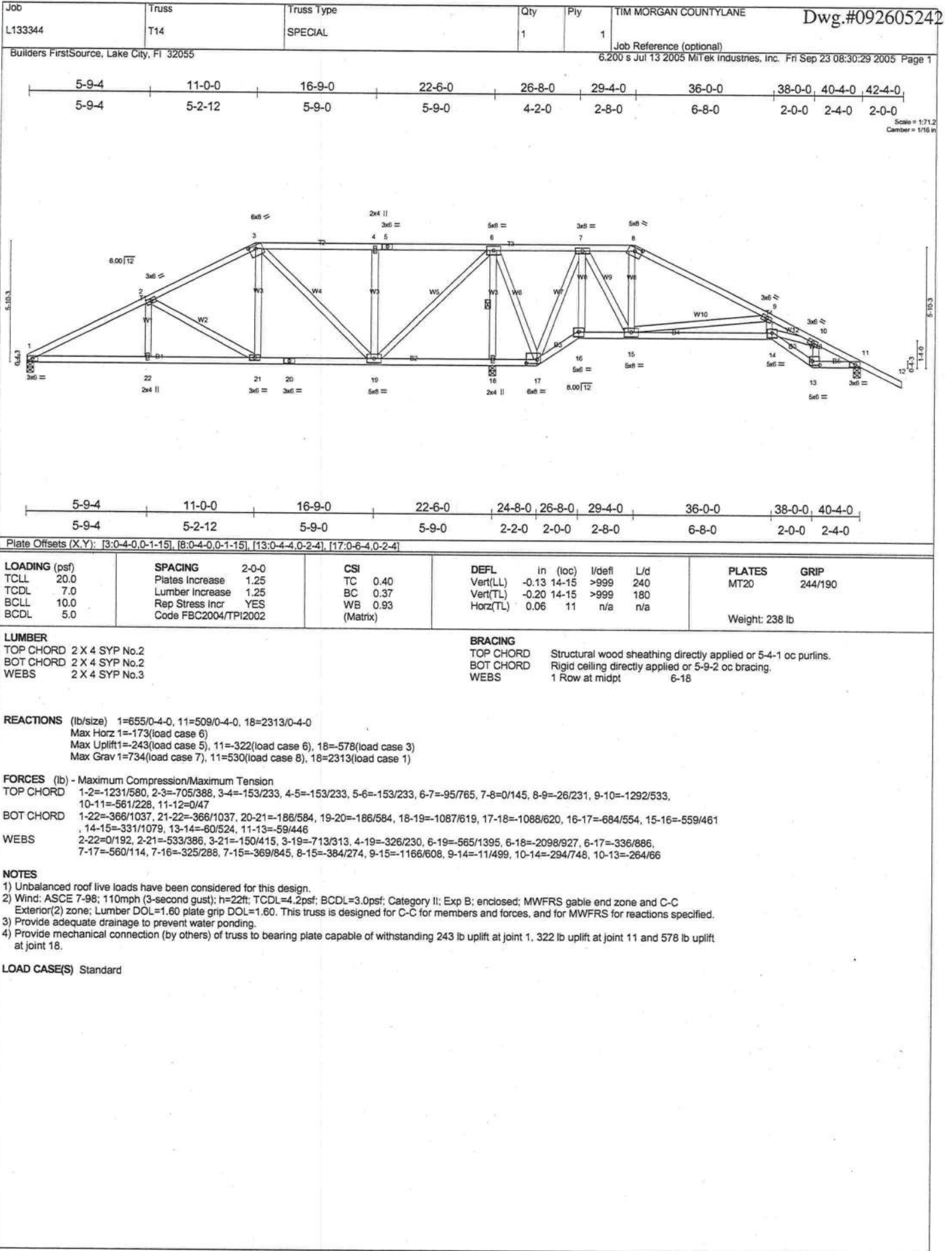
REACTIONS (lb/size) 1=787/0-4-0, 11=719/0-4-0, 22=1967/0-4-0
 Max Horz 1=-153(load case 6)
 Max Uplift 1=-245(load case 5), 11=-361(load case 6), 22=-588(load case 3)
 Max Grav 1=813(load case 7), 11=728(load case 8), 22=1967(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-1338/717, 2-3=-1080/560, 3-4=-927/560, 4-5=-47/601, 5-6=-47/601, 6-7=-135/802, 7-8=-670/410, 8-9=-778/392, 9-10=-2143/876, 10-11=-997/412, 11-12=0/47
 BOT CHORD 1-21=-483/1163, 20-21=-211/621, 19-20=-211/621, 18-19=-211/621, 18-22=-288/880, 17-22=-1214/558, 6-17=-311/221, 16-17=-28/196, 15-16=-28/196, 14-15=-595/1790, 13-14=-229/927, 11-13=-217/830
 WEBS 2-21=-274/300, 3-21=-7/164, 4-21=-150/418, 4-19=0/196, 4-18=-1385/549, 7-15=-228/601, 8-15=0/68, 9-15=-1146/537, 9-14=-135/692, 10-14=-424/1131, 10-13=-451/179, 7-17=-1180/444, 7-16=0/106

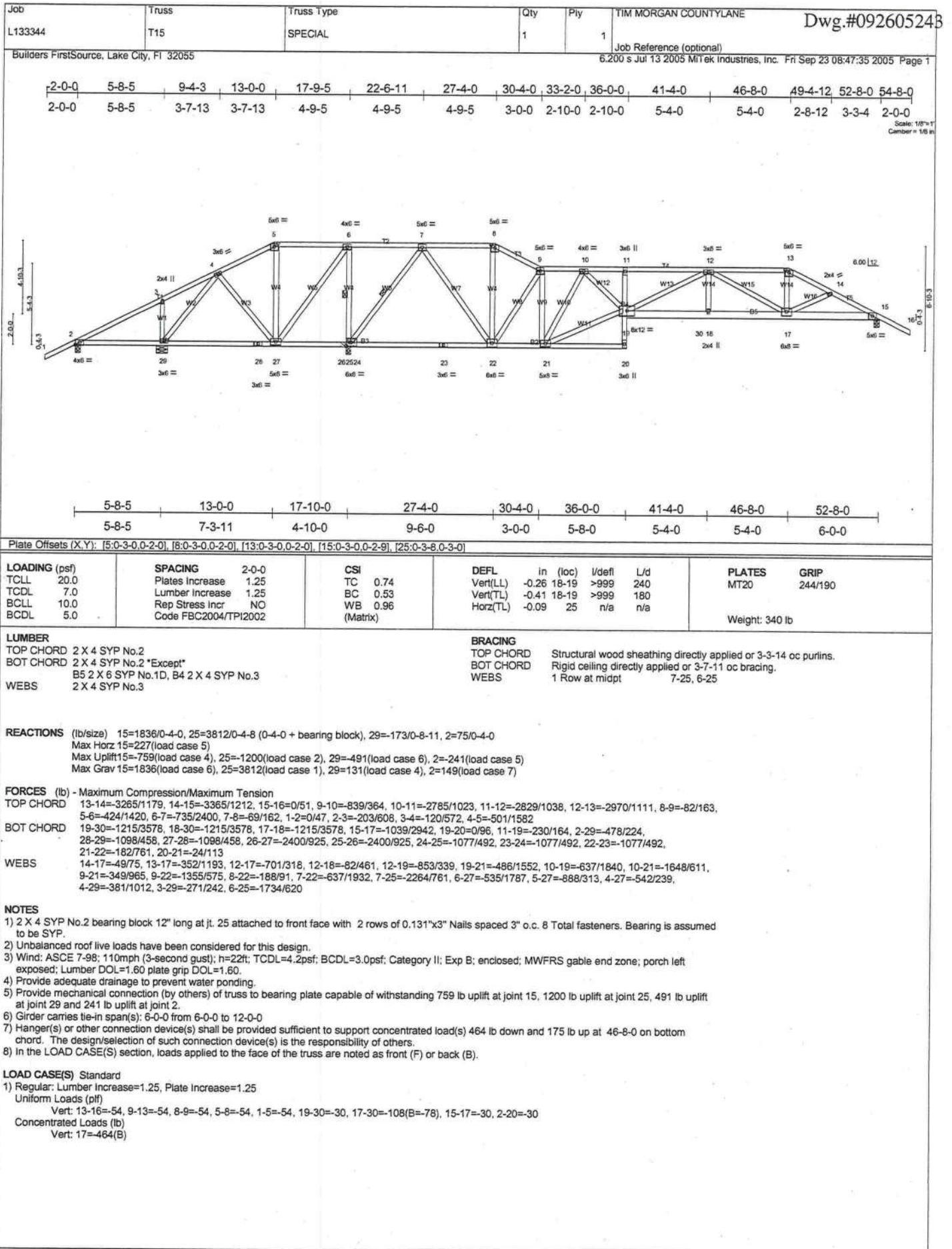
NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98: 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 22 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 1, 361 lb uplift at joint 11 and 588 lb uplift at joint 22.

LOAD CASE(S) Standard

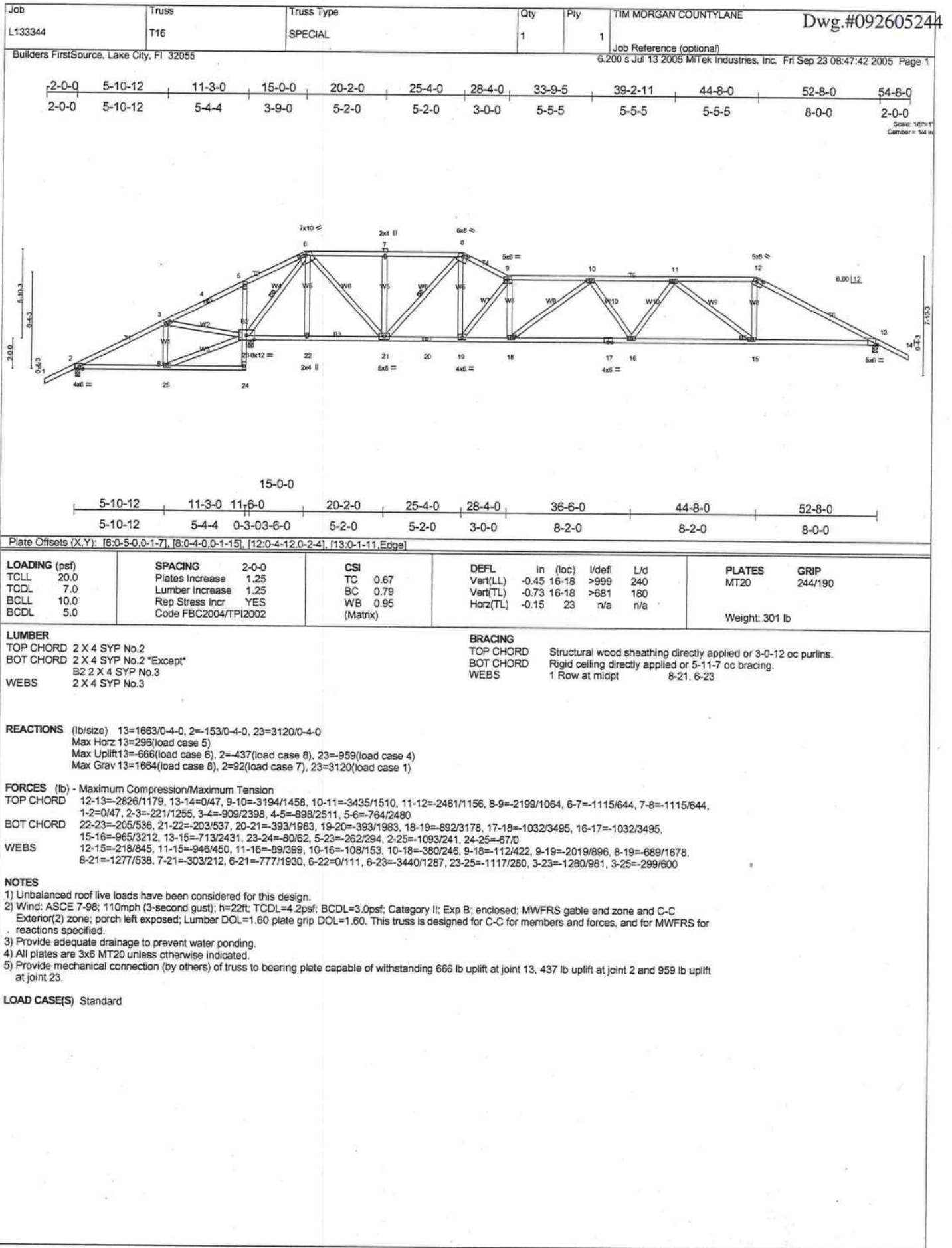
SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



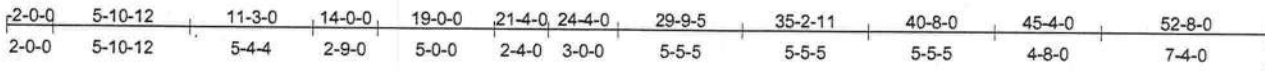
SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
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SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Scale: 1/8"=1'
Camber = 1/4"

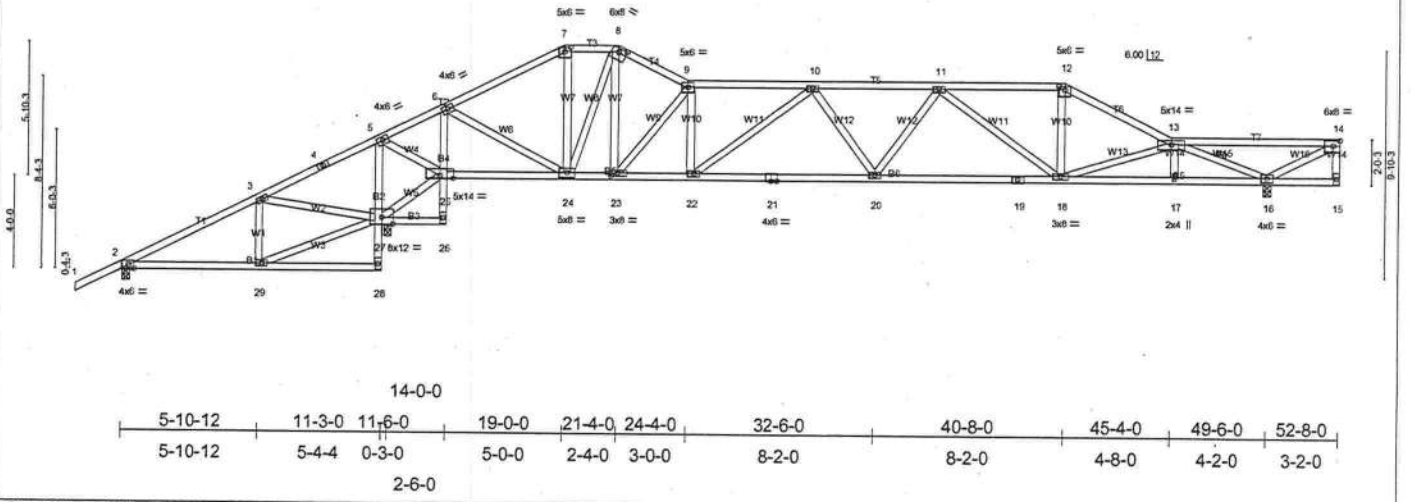


Plate Offsets (X,Y): [7:0-3-0-0-2-0], [8:0-4-0-0-1-15], [12:0-3-0-0-2-0], [23:0-3-8-0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.81	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(LL) -0.36 20-22 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.86	Vert(TL) -0.58 20-22 >797 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.15 27 n/a n/a		
				Weight: 314 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-6-12 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
B4 2 X 4 SYP No.3, B2 2 X 4 SYP No.3	WEBS 1 Row at midpt 13-16
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=84/0-4-0, 16=1685/0-4-0, 27=2913/0-4-0
 Max Horz 16=402(load case 5)
 Max Uplift2=395(load case 5), 16=694(load case 6), 27=874(load case 6)
 Max Grav 2=153(load case 7), 16=1685(load case 1), 27=2913(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 14-15=-53/26, 13-14=-230/251, 12-13=-2592/1106, 9-10=-2298/1106, 10-11=-2933/1304, 11-12=-2298/1056, 8-9=-1330/728, 7-8=-811/546, 1-2=0/47, 2-3=-157/1036, 3-4=-713/2168, 4-5=-703/2282, 5-6=-352/1034, 6-7=-967/538
BOT CHORD 24-25=-890/688, 23-24=-186/1186, 22-23=-590/2275, 21-22=-857/2851, 20-21=-857/2851, 19-20=-879/2846, 18-19=-879/2846, 17-18=-915/2872, 16-17=-911/2877, 15-16=-115/158, 25-26=-9/20, 6-25=-1993/906, 26-27=-59/12, 27-28=-79/63, 5-27=-1372/517, 2-29=-899/184, 28-29=-42/1
WEBS 14-16=-471/392, 13-16=-3382/1556, 13-17=0/88, 13-18=-643/321, 12-18=-271/864, 11-18=-691/339, 11-20=0/192, 10-20=0/164, 10-22=-722/328, 9-22=-161/649, 9-23=-1837/825, 8-23=-648/1439, 8-24=-1073/418, 7-24=-60/261, 6-24=-687/1911, 25-27=-2425/1205, 5-25=-299/1250, 27-29=-924/196, 3-27=-1211/900, 3-29=-242/528

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TC DL=4.2psf; BC DL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
 - C-C wind load user defined.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 3x6 MT20 unless otherwise indicated.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 2, 694 lb uplift at joint 16 and 874 lb uplift at joint 27.
 - Load case(s) 2, 3, 4, 5, 6 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

- LOAD CASE(S)** Standard Except:
- C-C Wind: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 13-14=39, 12-13=50, 9-12=39, 8-9=50, 7-8=39, 1-2=73, 2-7=50, 16-25=-6, 15-16=35, 26-27=-6, 2-28=35
 Horz: 12-13=58, 8-9=58, 1-2=-81, 2-7=-58
 Drag: 13-14=0, 9-10=0, 11-12=0
 - MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 13-14=18, 12-13=20, 9-12=18, 8-9=20, 7-8=38, 1-2=24, 2-7=5, 15-25=-6, 26-27=-6, 2-28=19
 Horz: 12-13=28, 8-9=28, 1-2=-32, 2-7=-14
 Drag: 13-14=0, 9-10=0, 11-12=0
 - MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60

Continued on page 2

Job L133344	Truss T18	Truss Type SPECIAL	Qty 1	Ply 1	TIM MORGAN COUNTYLANE Job Reference (optional)	Dwg.#092605247
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Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MITek Industries, Inc. Fri Sep 23 09:01:11 2005 Page 2

LOAD CASE(S) Standard Except:

Uniform Loads (plf)

Vert: 13-14=38, 12-13=5, 9-12=38, 8-9=5, 7-8=38, 1-2=13, 2-7=20, 16-25=-6, 15-16=19, 26-27=-6, 2-28=19
 Horz: 12-13=14, 8-9=14, 1-2=-22, 2-7=-28
 Drag: 13-14=0, 9-10=0, 11-12=0

5) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 13-14=18, 12-13=18, 9-12=18, 8-9=18, 7-8=18, 1-2=56, 2-7=38, 15-25=-6, 26-27=-6, 2-28=19
 Horz: 12-13=26, 8-9=26, 1-2=-65, 2-7=-46
 Drag: 13-14=0, 9-10=0, 11-12=0

6) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 13-14=38, 12-13=38, 9-12=38, 8-9=38, 7-8=18, 1-2=11, 2-7=18, 16-25=-6, 15-16=19, 26-27=-6, 2-28=19
 Horz: 12-13=46, 8-9=46, 1-2=-20, 2-7=-26
 Drag: 13-14=0, 9-10=0, 11-12=0

2-0-0	5-10-12	11-3-0	14-0-0	20-2-0	22-4-0	27-9-5	33-2-11	38-8-0	45-3-15	52-8-0
2-0-0	5-10-12	5-4-4	2-9-0	6-2-0	2-2-0	5-5-5	5-5-5	5-5-5	6-7-15	7-4-1

Scale: 1/8"=1'
Camber = 3/16"

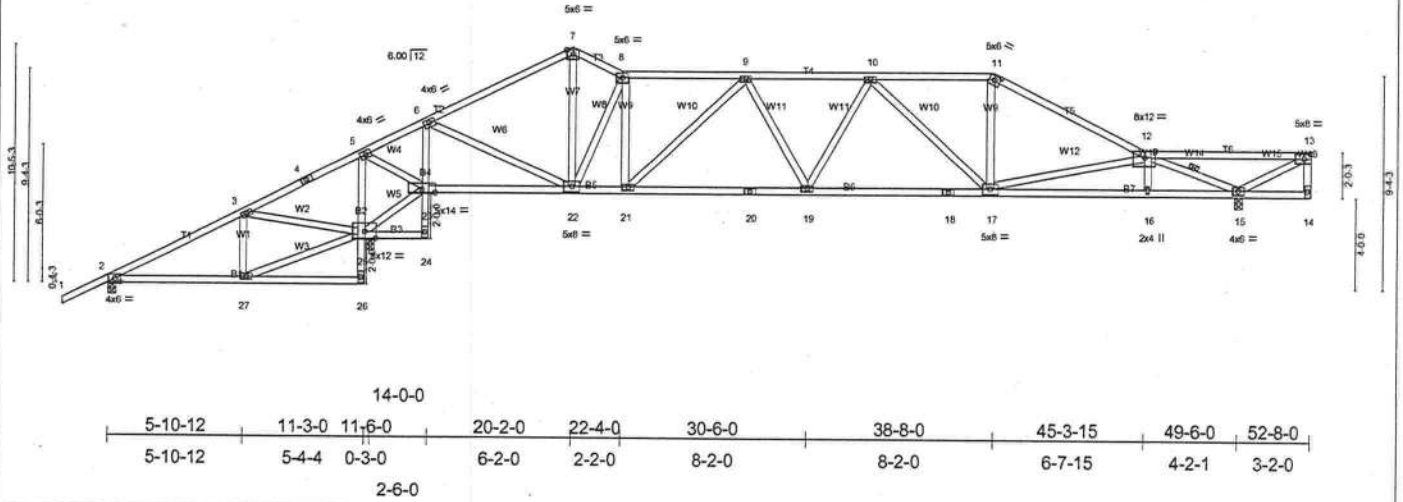


Plate Offsets (X,Y): [11:0-3-0,0-2-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.75	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.72	Vert(LL) -0.29 19-21 >999 240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.83	Vert(TL) -0.48 19-21 >966 180		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)	Horz(TL) 0.13 15 n/a n/a		
Weight: 312 lb						

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-9-1 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 5-6-9 oc bracing.
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3	WEBS 1 Row at midpt 12-15
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=17/0-4-0, 25=2783/0-4-0, 15=1714/0-4-0
 Max Horz 2=464(load case 5)
 Max Uplift 2=228(load case 6), 25=949(load case 5), 15=704(load case 6)
 Max Grav 2=219(load case 7), 25=2783(load case 1), 15=1714(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-281/808, 3-4=-987/1830, 4-5=-977/1943, 5-6=-313/639, 6-7=-1265/600, 7-8=-1193/629, 8-9=-1694/819, 9-10=-2430/1102, 10-11=-2161/1039, 11-12=-2472/1061, 12-13=-220/253, 13-14=-49/31
 BOT CHORD 2-27=-696/79, 26-27=-700, 25-26=-81/64, 5-25=-1422/761, 24-25=-60/5, 23-24=-3/21, 6-23=-1769/934, 22-23=-536/307, 21-22=-618/1672, 20-21=-895/2292, 19-20=-895/2292, 18-19=-949/2434, 17-18=-949/2434, 16-17=-1277/2999, 15-16=-1272/3009, 14-15=-116/148
 WEBS 3-27=-179/451, 25-27=-717/111, 3-25=-1099/819, 23-25=-2051/1019, 5-23=-585/1319, 6-22=-687/1700, 7-22=-283/844, 8-22=-1562/655, 8-21=-227/763, 9-21=-856/374, 9-19=-44/318, 10-19=-36/102, 10-17=-378/246, 11-17=-173/725, 12-17=-886/451, 12-16=0/145, 12-15=-3527/1612, 13-15=-461/365

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TC DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are 3x6 MT20 unless otherwise indicated.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2, 949 lb uplift at joint 25 and 704 lb uplift at joint 15.

LOAD CASE(S) Standard

Job L133344	Truss T20	Truss Type SPECIAL	Qty 1	Ply 1	TIM MORGAN COUNTY LANE	Dwg.#092605249
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.200 s Jul 13 2005 Mitek Industries, Inc. Fri Sep 23 09:03:00 2005 Page 1	

2-0-0	5-10-12	11-3-0	14-0-0	20-0-0	25-6-11	31-1-5	36-8-0	41-0-0	45-3-15	52-8-0
2-0-0	5-10-12	5-4-4	2-9-0	6-0-0	5-6-11	5-6-11	5-6-11	4-4-0	4-4-0	7-4-1

Scale: 1/8"=1'
Camber = 1/4"

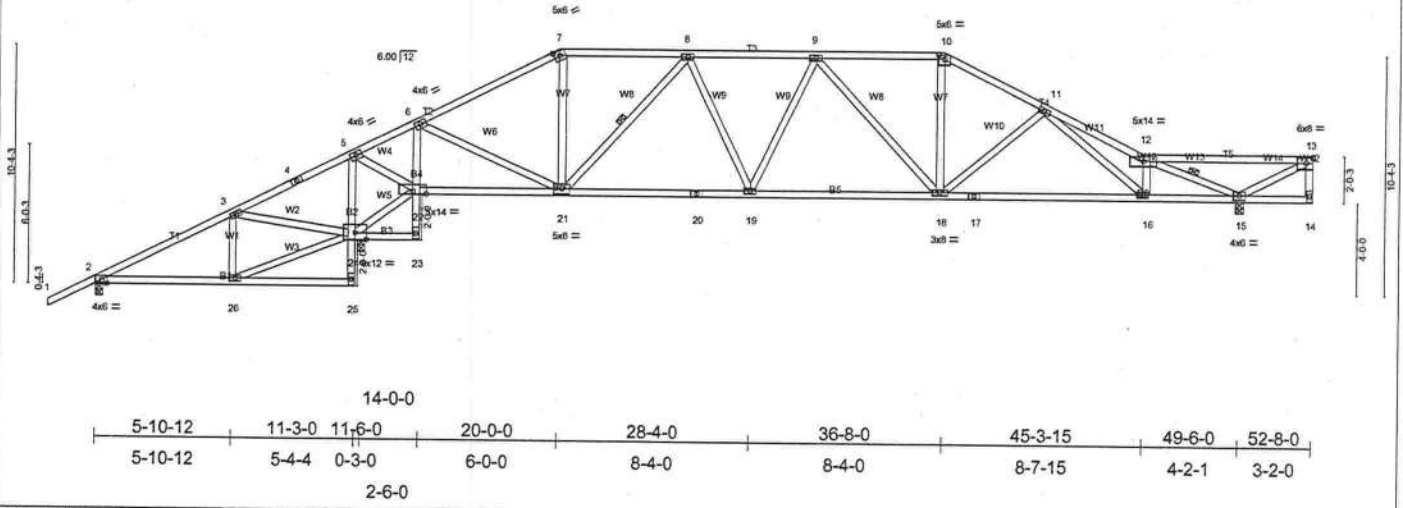


Plate Offsets (X,Y): [7:0-3-0-0-2-7], [10:0-3-0-0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.85	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.68	Vert(LL) -0.34 16-18 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.65	Vert(TL) -0.56 16-18 >818 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.12 15 n/a n/a		
	Code FBC2004/TPI2002				Weight: 310 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 5-6-12 oc bracing.
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3	WEBS 1 Row at midpt 8-21, 12-15
WEBS 2 X 4 SYP No.3	

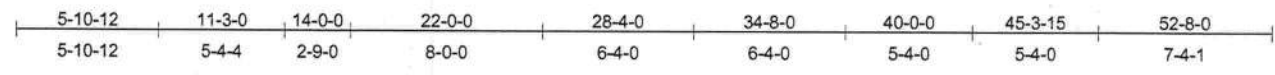
REACTIONS (lb/size) 2=97/0-4-0, 24=2681/0-4-0, 15=1737/0-4-0
 Max Horz 2=462(load case 5)
 Max Uplift 2=222(load case 5), 24=929(load case 5), 15=580(load case 6)
 Max Grav 2=149(load case 7), 24=2681(load case 1), 15=1737(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-208/642, 3-4=-874/1562, 4-5=-863/1675, 5-6=-195/344, 6-7=-1404/629, 7-8=-1195/640, 8-9=-2025/969, 9-10=-2001/996,
 10-11=-2265/1043, 11-12=-3540/1519, 12-13=-228/274, 13-14=-50/40
 BOT CHORD 2-26=-549/44, 25-26=-73/0, 24-25=-81/63, 5-24=-1453/767, 23-24=-42/6, 22-23=0/19, 6-22=-1629/854, 21-22=-285/183, 20-21=-665/1831,
 19-20=-665/1831, 18-19=-792/2108, 17-18=-1012/2348, 16-17=-1012/2348, 15-16=-1261/3076, 14-15=-114/170
 WEBS 3-26=-154/395, 24-26=-567/82, 3-24=-1009/781, 22-24=-1775/892, 5-22=-588/1350, 6-21=-604/1558, 7-21=-49/340, 8-21=-1008/415,
 8-19=-125/484, 9-19=-219/179, 9-18=-302/197, 10-18=-244/714, 11-18=-482/363, 11-16=-364/1016, 12-16=-482/310, 12-15=-3621/1610,
 13-15=-510/394

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed: MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) All plates are 3x6 MT20 unless otherwise indicated.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 2, 929 lb uplift at joint 24 and 580 lb uplift at joint 15.

LOAD CASE(S) Standard

Builders FirstSource, Lake City, FL 32055 6.200 s Jul 13 2005 MITek Industries, Inc. Fri Sep 23 09:05:01 2005 Page 1



Scale = 1/32.3
Camber = 1/8 in

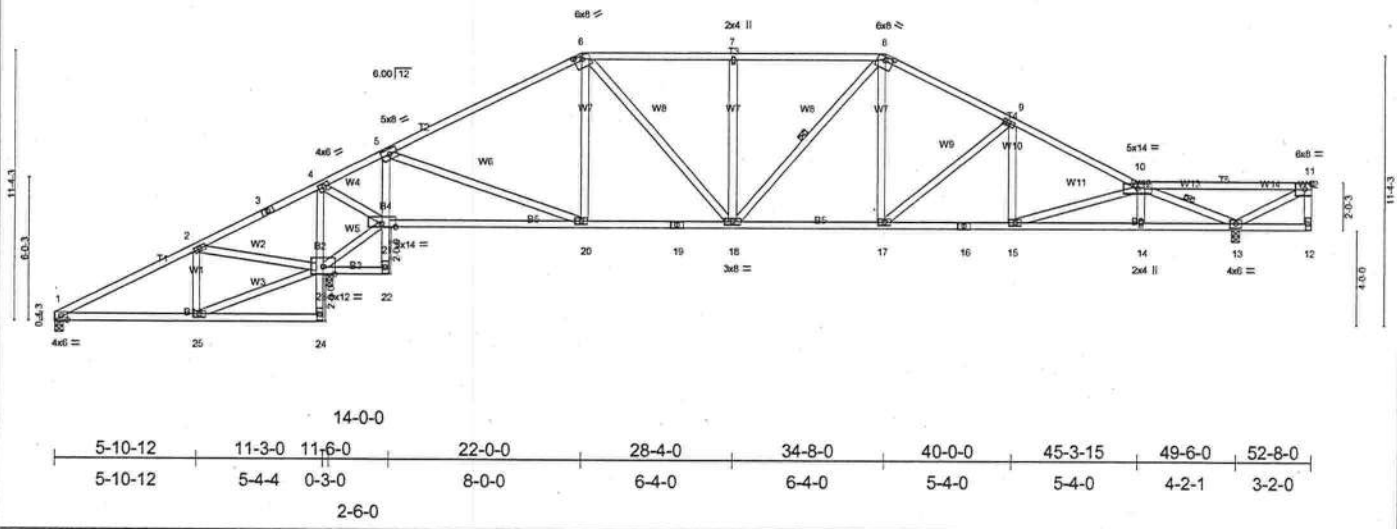


Plate Offsets (X,Y): [6:0-4-0,0-1-15], [8:0-4-0,0-1-15]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.80	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.74	Vert(LL) -0.22 14-15 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.72	Vert(TL) -0.35 14-15 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.11 13 n/a n/a		
				Weight: 315 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
 B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-14 oc bracing.
WEBS 1 Row at midpt 8-18, 10-13

REACTIONS (lb/size) 1=28/0-4-0, 23=2617/0-4-0, 13=1753/0-4-0
Max Horz 1=420(load case 5)
Max Uplift 1=111(load case 6), 23=981(load case 5), 13=605(load case 6)
Max Grav 1=81(load case 7), 23=2617(load case 1), 13=1753(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-125/475, 2-3=-822/1363, 3-4=-811/1476, 4-5=-108/99, 5-6=-1613/754, 6-7=-1811/959, 7-8=-1811/958, 8-9=-2090/1029, 9-10=-2688/1226, 10-11=-226/252, 11-12=-50/26
BOT CHORD 1-25=400/0, 24-25=68/0, 23-24=-74/62, 4-23=-1484/773, 22-23=-65/0, 21-22=0/14, 5-21=-1477/877, 20-21=-88/148, 19-20=-460/1347, 18-19=-460/1347, 17-18=-684/1828, 16-17=-1011/2345, 15-16=-1011/2345, 14-15=-1347/3061, 13-14=-1342/3069, 12-13=-112/155
WEBS 2-25=-152/343, 23-25=-412/0, 2-23=-978/823, 21-23=-1538/872, 4-21=-653/1424, 5-20=-528/1366, 6-20=-243/228, 6-18=-280/782, 7-18=-354/254, 8-18=-119/156, 8-17=-228/597, 9-17=-677/425, 9-15=-86/415, 10-15=-753/353, 10-14=0/111, 10-13=-3590/1695, 11-13=-468/384

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 1, 981 lb uplift at joint 23 and 605 lb uplift at joint 13.

LOAD CASE(S) Standard

2-0-0	5-10-12	11-3-0	14-0-0	21-0-1	28-4-0	35-7-15	38-4-0	45-2-12	52-8-0
2-0-0	5-10-12	5-4-4	2-9-0	7-0-1	7-3-15	7-3-15	2-8-1	6-10-12	7-5-4

Scale = 1/4" = 1'-0"
Camber = 3/16"

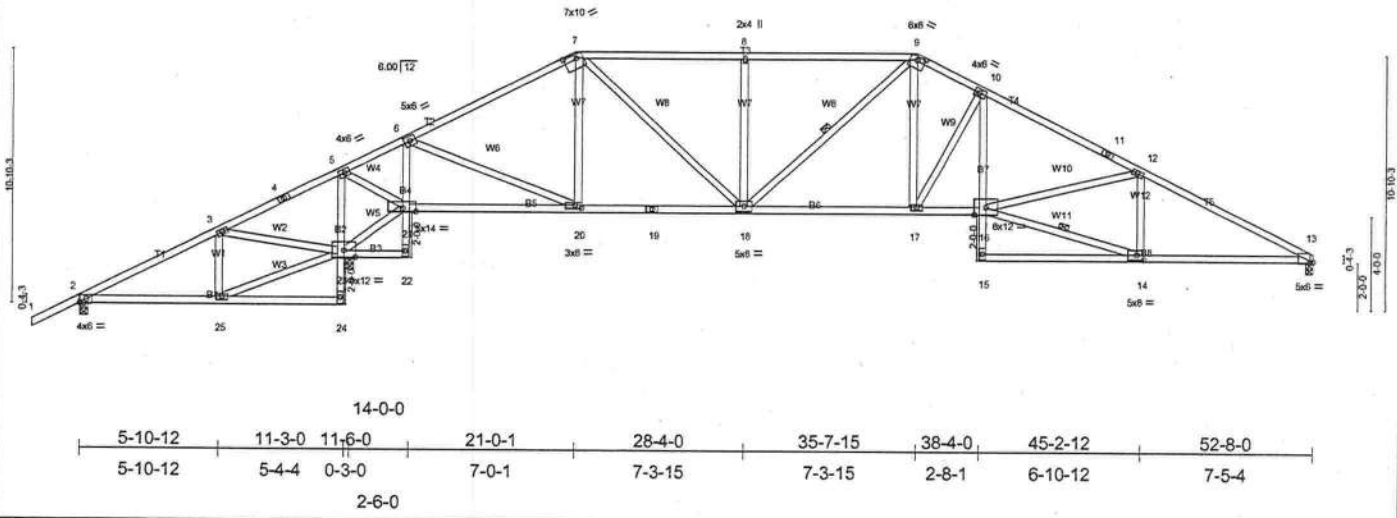


Plate Offsets (X,Y): [7:0-6-4,0-2-0], [9:0-4-0,0-1-15], [13:0-1-10,Edge], [14:0-3-8,0-2-8], [20:0-3-8,0-1-8]									
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.51	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.76	Vert(LL) -0.28 17-18 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.84	Vert(TL) -0.45 17-18 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.17 13 n/a n/a		
	Code FBC2004/TPI2002				Weight: 321 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins, except
BOT CHORD 2 X 4 SYP No.2 "Except"	2-0-0 oc purlins (3-10-11 max.): 7-9.
WEBS 2 X 4 SYP No.3, B4 2 X 4 SYP No.3, B7 2 X 4 SYP No.3	BOT CHORD Rigid ceiling directly applied or 5-6-7 oc bracing.
	WEBS 1 Row at midpt 9-18, 14-16

REACTIONS (lb/size) 2=85/0-4-0, 13=1606/0-4-0, 23=2823/0-4-0
 Max Horz 2=375(load case 5)
 Max Uplift 2=255(load case 5), 13=477(load case 6), 23=927(load case 5)
 Max Grav 2=153(load case 7), 13=1606(load case 1), 23=2823(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-111/651, 3-4=-728/1603, 4-5=-718/1716, 5-6=-104/253, 6-7=-1680/805, 7-8=-2233/1153, 8-9=-2233/1153, 9-10=-2626/1331, 10-11=-3123/1529, 11-12=-3215/1506, 12-13=-3032/1443
 BOT CHORD 2-25=-557/38, 24-25=-72/0, 23-24=-81/63, 5-23=-1577/707, 22-23=-58/2, 21-22=0/18, 6-21=-1697/910, 20-21=-195/299, 19-20=-316/1417, 18-19=-316/1417, 17-18=-744/2357, 16-17=-1011/2778, 15-16=0/98, 10-16=-354/805, 14-15=-10/118, 13-14=-1168/2625
 WEBS 3-25=-177/398, 23-25=-574/78, 3-23=-1023/810, 21-23=-1806/1013, 5-21=-543/1495, 6-20=-634/1659, 7-20=-410/285, 7-18=-467/1159, 8-18=-422/298, 9-18=-201/150, 9-17=-453/969, 10-17=-887/554, 14-16=-1210/2619, 12-16=-112/281, 12-14=-513/369

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 3x6 MT20 unless otherwise indicated.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 lb uplift at joint 2, 477 lb uplift at joint 13 and 927 lb uplift at joint 23.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job L133344	Truss T23	Truss Type SPECIAL	Qty 1	Ply 1	TIM MORGAN COUNTYLANE	Dwg.#092605252
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Sep 23 09:15:44 2005 Page 1			

5-10-12	11-3-0	11-6-0	21-0-1	28-4-0	35-7-15	38-9-0	40-9-0	48-7-0	50-7-0
5-10-12	5-4-4	0-3-0	9-6-1	7-3-15	7-3-15	3-1-1	2-0-0	7-10-0	2-0-0 2-1-0

Plate Offsets (X,Y): [7:0-4-0,0-1-15], [9:0-4-0,0-1-15], [13:2-8-7,1-0-0], [13:3-2-9,1-0-0], [13:0-8-8,Edge], [17:0-3-0,0-2-3]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.69	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.83	Vert(LL) -0.36 15-16 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.98	Vert(TL) -0.58 15-16 >862 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.29 14 n/a n/a		
	Code FBC2004/TPI2002				Weight: 399 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except* T6 2 X 12 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-4-7 oc purlins, except 2-0-0 oc purlins (4-5-12 max.); 7-9.
BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3	BOT CHORD Rigid ceiling directly applied or 4-8-12 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 6-22, 7-21, 8-19, 9-19
LBR SCAB 12-14 2 X 12 SYP No.2 one side	

REACTIONS (lb/size) 2=127/0-4-0, 22=2766/0-4-0, 14=1545/0-4-0
Max Horz 2=369(load case 5)
Max Uplift 2=274(load case 5), 22=904(load case 5), 14=465(load case 6)
Max Grav 2=223(load case 7), 22=2766(load case 1), 14=1545(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-54/624, 3-4=-775/1424, 4-5=-758/1537, 5-6=-611/1476, 6-7=-1316/726, 7-8=-1736/994, 8-9=-1736/994, 9-10=-2018/1118, 10-11=-3201/1659, 11-12=-3277/1576, 12-25=-4029/1916, 13-25=-4112/1902, 13-14=-568/311
BOT CHORD 2-24=-534/72, 23-24=-100/0, 22-23=-83/60, 5-22=-319/333, 21-22=-157/553, 20-21=-260/1125, 19-20=-260/1125, 18-19=-549/1814, 17-18=-710/2026, 16-17=-812/2346, 15-16=-1770/4101, 13-15=-1771/4092
WEBS 3-24=-200/369, 22-24=-480/133, 3-22=-937/826, 6-22=-2818/1353, 6-21=-286/997, 7-21=-508/277, 7-19=-435/994, 8-19=-420/297, 9-19=-165/144, 9-18=-330/665, 10-18=-556/416, 10-17=-1111/401, 10-16=-1019/2232, 11-16=-66/160, 12-16=-1472/801, 12-15=0/150

NOTES
1) Attached 9-1-3 scab 12 to 14, front face(s) 2 X 12 SYP No.2 with 2 row(s) of 0.131"x3" Nails spaced 9" o.c.except : starting at 0-2-15 from end at joint 12, nail 3 row(s) at 4 o.c. for 2-0-0; starting at 3-6-13 from end at joint 12, nail 2 row(s) at 4 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) Bearing at joint(s) 14 considers parallel to grain value using ANS/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2, 904 lb uplift at joint 22 and 465 lb uplift at joint 14.
7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

SEPTEMBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

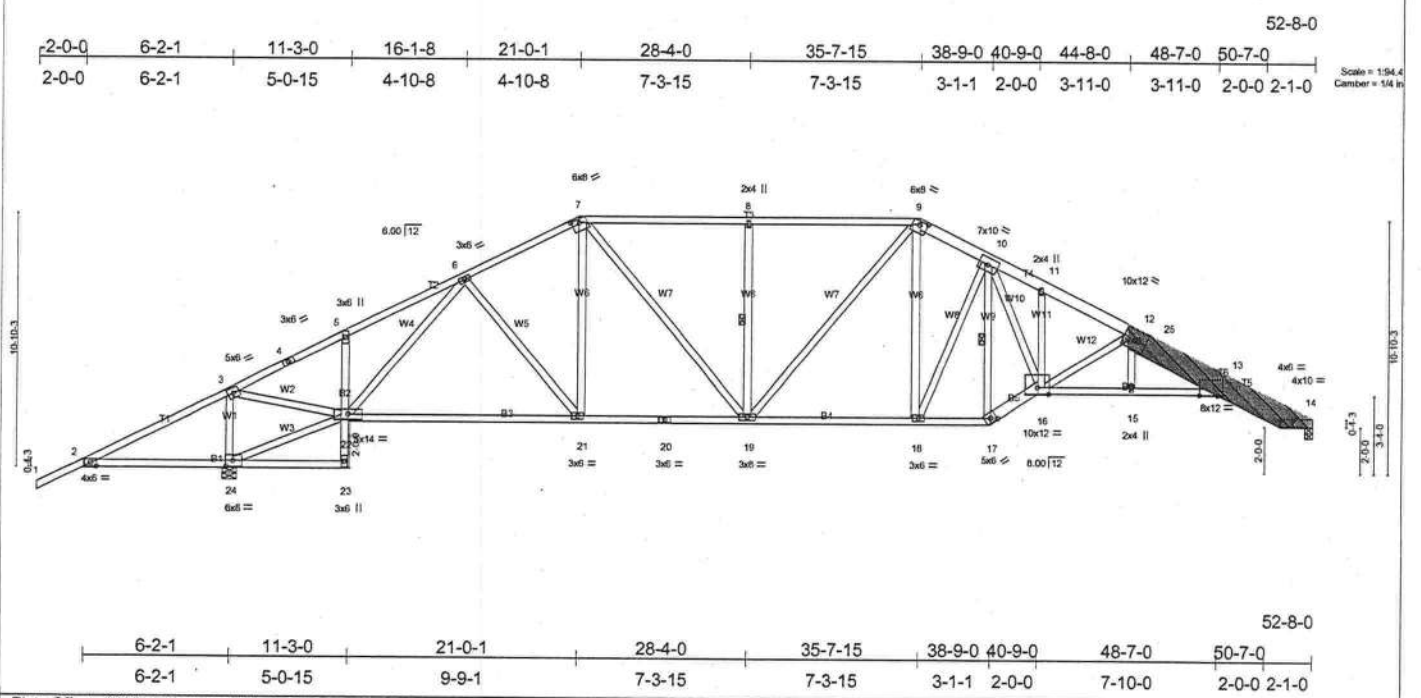


Plate Offsets (X,Y): [7:0-4-0,0-1-15], [9:0-4-0,0-1-15], [13:2-8-4,0-11-15], [13:3-2-14,0-11-14], [13:0-9-8,Edge], [17:0-3-0,0-2-3], [24:0-3-8,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.83	Vert(LL) -0.44 18-19 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.77	Vert(TL) -0.71 18-19 >786 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.84	Horz(TL) 0.45 14 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 409 lb

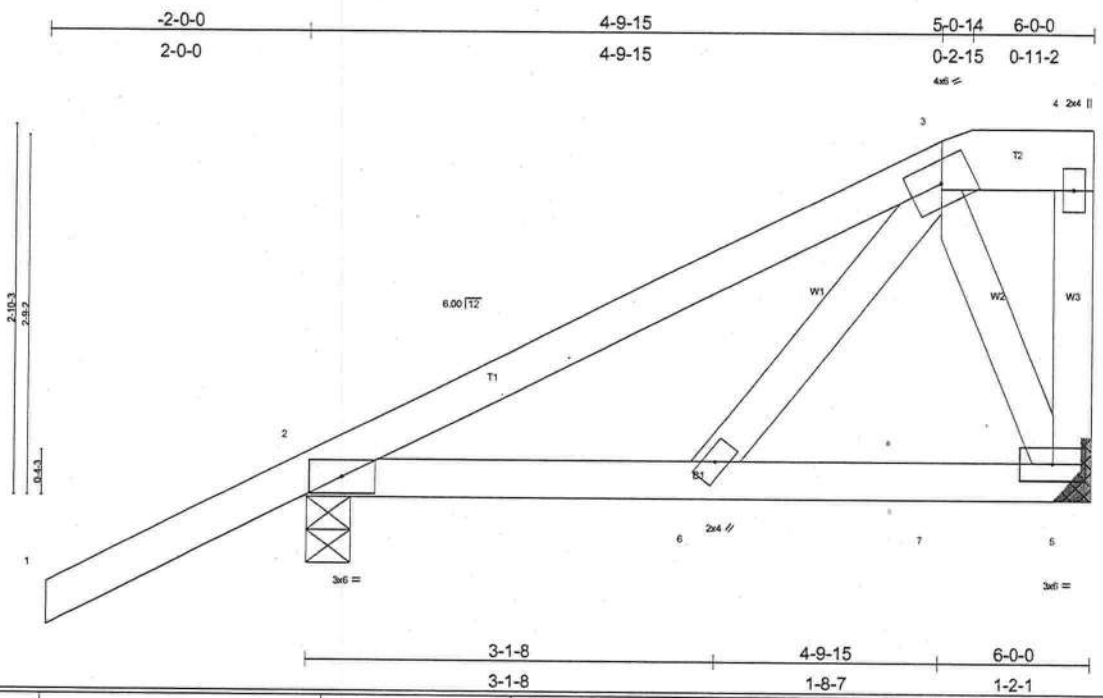
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except* T4 2 X 6 SYP No.1D, T6 2 X 12 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-1-0 oc purlins, except 2-0-0 oc purlins (3-7-13 max.); 7-9.
BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3, B5 2 X 6 SYP No.1D, B6 2 X 4 SYP No.1D	BOT CHORD Rigid ceiling directly applied or 4-6-13 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 8-19, 10-17
LBR SCAB 12-14 2 X 12 SYP No.2 one side	

REACTIONS (lb/size) 24=2620/0-7-3, 14=1823/0-4-0
Max Horz 24=318(load case 5)
Max Uplift 24=1001(load case 5), 14=515(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-871/815, 3-4=-2123/602, 4-5=-2051/618, 5-6=-2175/772, 6-7=-2447/1164, 7-8=-2518/1297, 8-9=-2519/1296, 9-10=-2667/1366, 10-11=-4017/1975, 11-12=-4097/1896, 12-25=-4976/2289, 13-25=-5058/2276, 13-14=-672/351
BOT CHORD 2-24=-655/932, 23-24=-58/10, 22-23=0/49, 5-22=-274/299, 21-22=-663/2168, 20-21=-595/2149, 19-20=-595/2149, 18-19=-771/2379, 17-18=-948/2630, 16-17=-1073/3007, 15-16=-2138/5030, 13-15=-2138/5020
WEBS 3-24=-2158/1223, 22-24=-646/800, 6-21=-115/156, 7-21=-67/315, 7-19=-291/662, 8-19=-394/286, 9-19=-196/378, 9-18=-343/695, 10-18=-647/452, 10-17=-1349/495, 10-16=-1162/2609, 11-16=-44/147, 12-16=-1670/880, 12-15=0/162, 6-22=-441/436, 3-22=-1165/2552

- NOTES**
- Attached 9-1-3 scab 12 to 14, front face(s) 2 X 12 SYP No.2 with 2 row(s) of 0.131"x3" Nails spaced 9" o.c. except : starting at 0-3-3 from end at joint 12, nail 4 row(s) at 4 o.c. for 2-0-0; starting at 3-6-12 from end at joint 12, nail 2 row(s) at 4 o.c. for 2-0-0.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1001 lb uplift at joint 24 and 515 lb uplift at joint 14.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) -0.02 5-6 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.36	Vert(TL) -0.03 5-6 >999 180		
BCLL 10.0	Rep Stress Incr NO	WB 0.09	Horz(TL) 0.00 5 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 34 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SYP No.2 *Except* T2 2 X 6 SYP No.1D</p> <p>BOT CHORD 2 X 4 SYP No.2</p> <p>WEBS 2 X 4 SYP No.3</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p>
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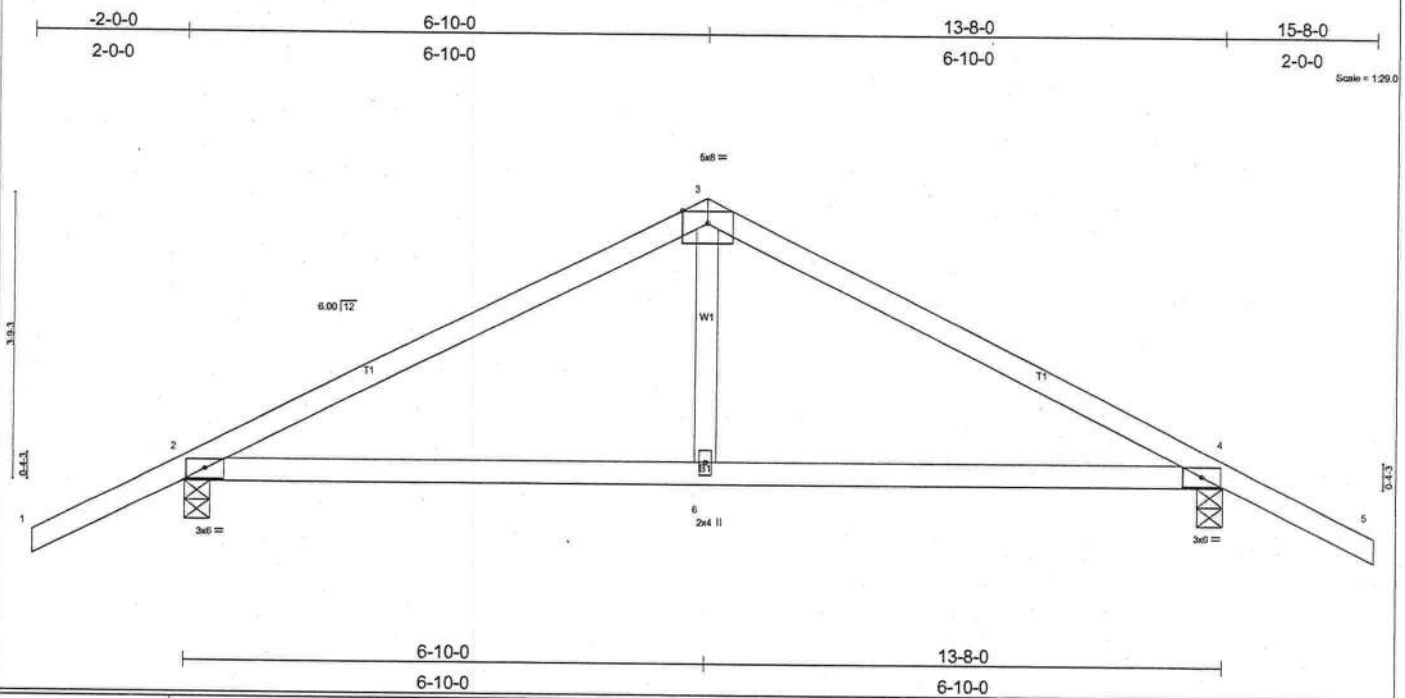
REACTIONS (lb/size) 5=464/Mechanical, 2=426/0-4-0
 Max Horz 2=176(load case 4)
 Max Uplift 5=-195(load case 4), 2=-238(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-371/53, 3-4=0/0, 4-5=-46/36
 BOT CHORD 2-6=-74/264, 6-7=-46/97, 5-7=-46/97
 WEBS 3-6=-49/282, 3-5=-252/119

- NOTES**
- 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 5 and 238 lb uplift at joint 2.
 - 5) Girder carries hip end with 0-0-0 right side setback, 4-9-15 left side setback, and 5-0-0 end setback.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 237 lb down and 122 lb up at 4-9-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-4=-91(F=-37), 2-7=-30, 5-7=-50(F=-20)
 Concentrated Loads (lb)
 Vert: 7=-237(F)



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2'-0'-0	TC 0.45	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.35	Vert(LL) 0.13 4-6 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.09	Vert(TL) 0.11 4-6 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.01 4 n/a n/a		
	Code FBC2004/TPI2002			Weight: 55 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6'-0'-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 7'-7'-14 oc bracing.
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REACTIONS (lb/size) 2=677/0-4-0, 4=677/0-4-0
 Max Horz 2=112(load case 5)
 Max Uplift 2=-468(load case 5), 4=-468(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-767/910, 3-4=-767/910, 4-5=0/47
 BOT CHORD 2-6=-620/616, 4-6=-620/616
 WEBS 3-6=-416/240

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 468 lb uplift at joint 2 and 468 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L133344	Truss T26G	Truss Type COMMON	Qty 1	Ply 1	TIM MORGAN COUNTYLANE	Dwg.#092605256
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)	
					6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Sep 22 17:09:49 2005 Page 1	

-2-0-0	3-8-4	6-10-0	9-11-12	13-8-0	15-8-0
2-0-0	3-8-4	3-1-12	3-1-12	3-8-4	2-0-0

Scale = 1/20

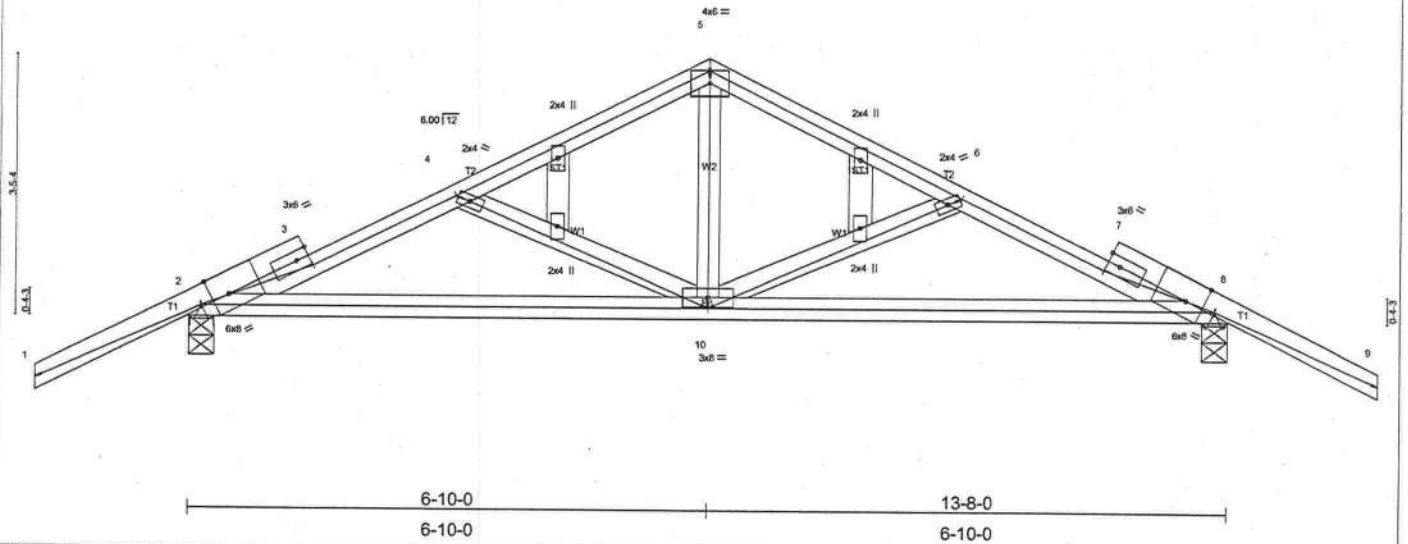


Plate Offsets (X,Y): [2:0-2-13,Edge], [8:0-2-13,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.64	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.44	Vert(LL) 0.16 8-10 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.20	Vert(TL) 0.13 8-10 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.04 8 n/a n/a		
	Code FBC2004/TP12002				Weight: 70 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-7-9 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=1207/0-4-0, 8=1207/0-4-0
 Max Horz 2=106(load case 5)
 Max Uplift 2=828(load case 5), 8=828(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-61/100, 2-3=-1674/2067, 3-4=-1624/2042, 4-5=-1209/1568, 5-6=-1209/1568, 6-7=-1624/2042, 7-8=-1674/2067, 8-9=-61/100
 BOT CHORD 2-10=-1731/1466, 8-10=-1731/1466
 WEBS 4-10=-504/653, 5-10=-903/618, 6-10=-504/653

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TC DL=4.2psf; BC DL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - Gable studs spaced at 2-0-0 oc.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 828 lb uplift at joint 2 and 828 lb uplift at joint 8.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-5=-114(F=60), 5-9=-114(F=60), 2-8=-30

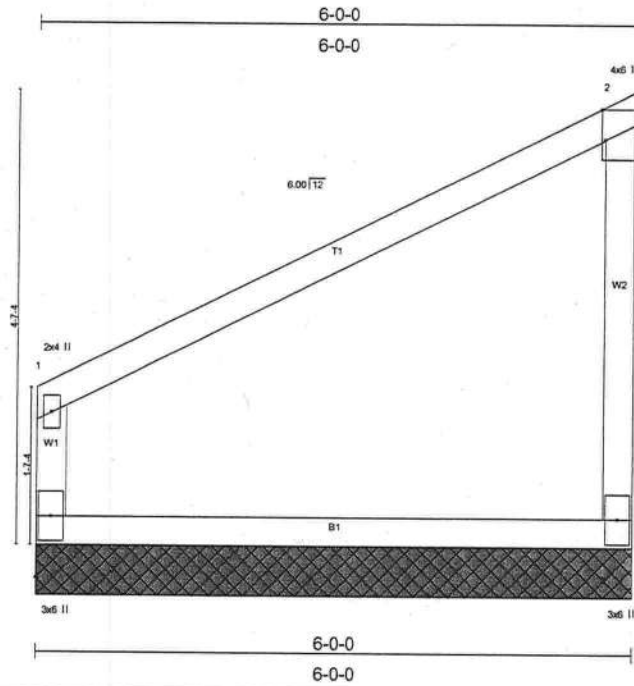


Plate Offsets (X,Y): [2:0-3-11,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.84	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.33	Vert(LL) n/a - n/a 999		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) n/a - n/a 999		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 27 lb	

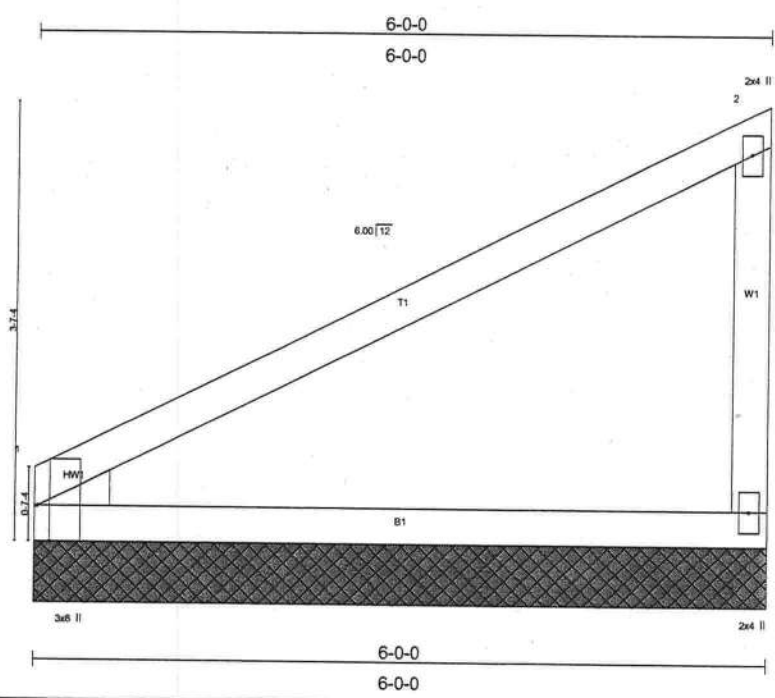
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 4=240/6-0-0, 3=240/6-0-0
 Max Horz 4=132(load case 5)
 Max Uplift 4=-26(load case 5), 3=-155(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-4=-172/160, 1-2=-129/9, 2-3=-137/260
 BOT CHORD 3-4=-98/58

- NOTES**
- 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) Gable requires continuous bottom chord bearing.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 4 and 155 lb uplift at joint 3.

LOAD CASE(S) Standard



Scale = 1:16.0

Plate Offsets (X, Y): [1:0-3-6, Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.60	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.24	Vert(LL) n/a - n/a 999		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) n/a - n/a 999		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 n/a n/a		
	Code FBC2004/TPI2002				Weight: 24 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SYP No.2</p> <p>BOT CHORD 2 X 4 SYP No.2</p> <p>WEBS 2 X 4 SYP No.3</p> <p>WEDGE</p> <p>Left: 2 X 4 SYP No.3</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p>
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REACTIONS (lb/size) 3=246/6-0-0, 1=246/6-0-0
 Max Horz 1=149(load case 5)
 Max Uplift 3=-134(load case 5), 1=-52(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-160/61, 2-3=-158/288

BOT CHORD 1-3=0/0

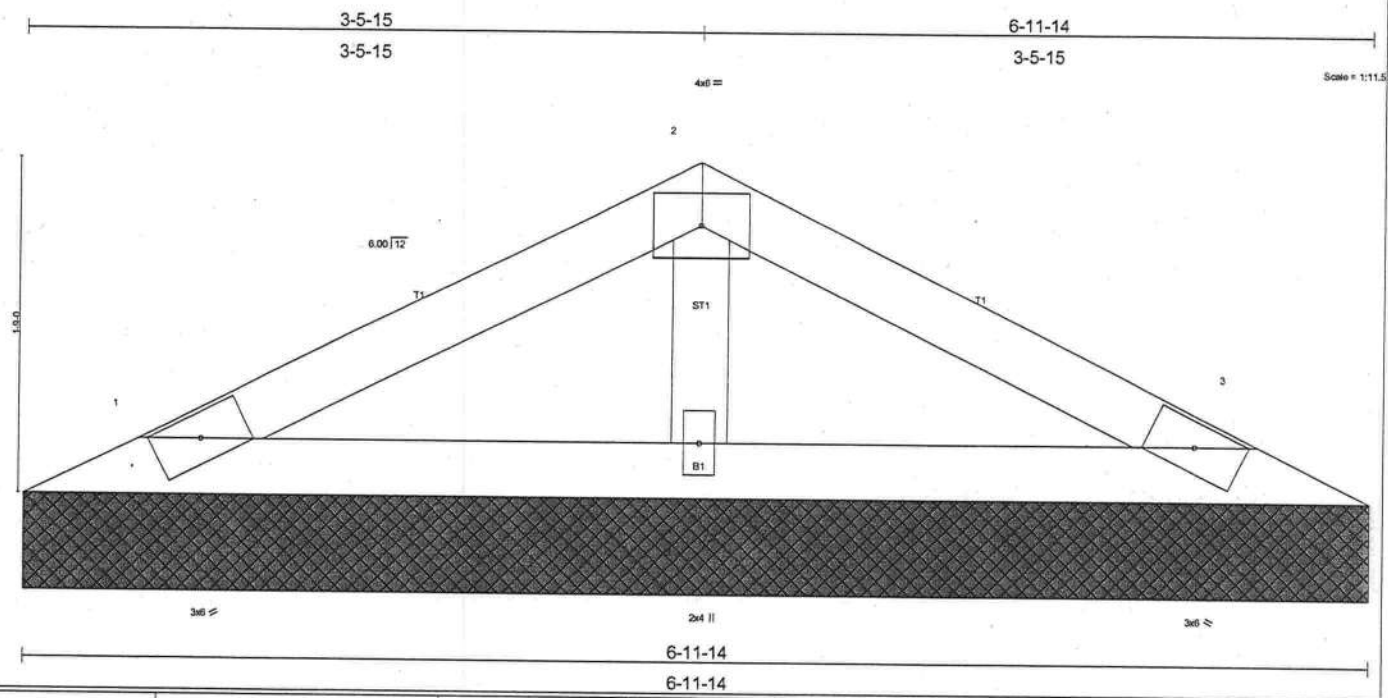
NOTES

1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Gable requires continuous bottom chord bearing.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 52 lb uplift at joint 1.

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.08	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(LL) n/a - n/a 999		
BCLL 10.0	Rep Stress Incr YES	WB 0.05	Vert(TL) n/a - n/a 999		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.00 3 n/a n/a		
				Weight: 22 lb	

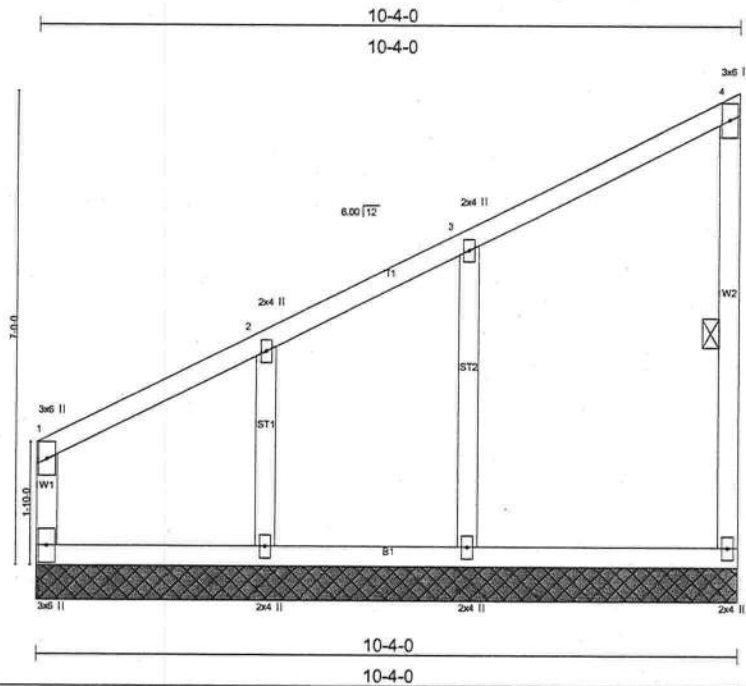
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS (lb/size) 1=102/6-11-14, 3=102/6-11-14, 4=279/6-11-14
 Max Horz 1=29(load case 5)
 Max Uplift 1=39(load case 5), 3=44(load case 6), 4=70(load case 5)
 Max Grav 1=105(load case 7), 3=105(load case 8), 4=279(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-54/53, 2-3=-54/53
 BOT CHORD 1-4=-3/22, 3-4=-3/22
 WEBS 2-4=-161/181

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Gable requires continuous bottom chord bearing.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1, 44 lb uplift at joint 3 and 70 lb uplift at joint 4.

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.78	Vert(LL) n/a - n/a 999	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.44	Vert(TL) n/a - n/a 999		
BCLL 10.0	Rep Stress Incr YES	WB 0.10	Horz(TL) -0.00 5 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			
				Weight: 55 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 4-5
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 5=149/10-4-0, 8=120/10-4-0, 7=271/10-4-0, 6=303/10-4-0
 Max Horz 8=232(load case 5)
 Max Uplift 5=96(load case 5), 7=328(load case 5), 6=87(load case 5)
 Max Grav 5=149(load case 1), 8=193(load case 5), 7=271(load case 1), 6=303(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-364/32, 2-3=-182/28, 3-4=-61/33, 4-5=-92/145, 1-8=-155/0
 BOT CHORD 7-8=-21/9, 6-7=-21/9, 5-6=-21/9
 WEBS 2-7=-171/383, 3-6=-197/244

- NOTES**
- 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) Gable requires continuous bottom chord bearing.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 5, 328 lb uplift at joint 7 and 87 lb uplift at joint 6.

LOAD CASE(S) Standard

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MITek Industries, Inc. Fri Sep 23 09:28:59 2005 Page 1

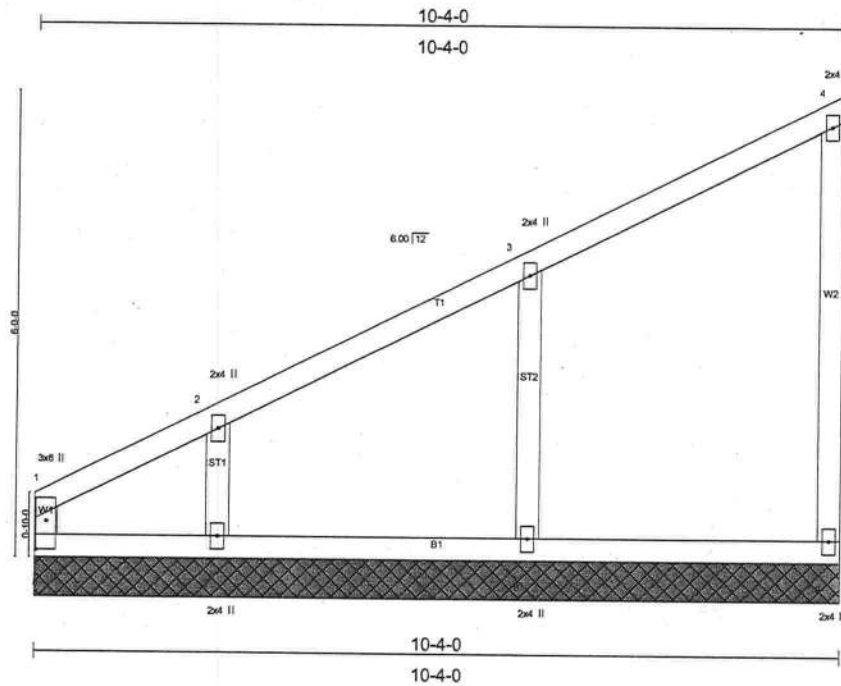


Plate Offsets (X,Y): [1:0-4-8,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr YES	WB 0.08	Horz(TL)	-0.00	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 49 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 OTHERS 2 X 4 SYP No.3

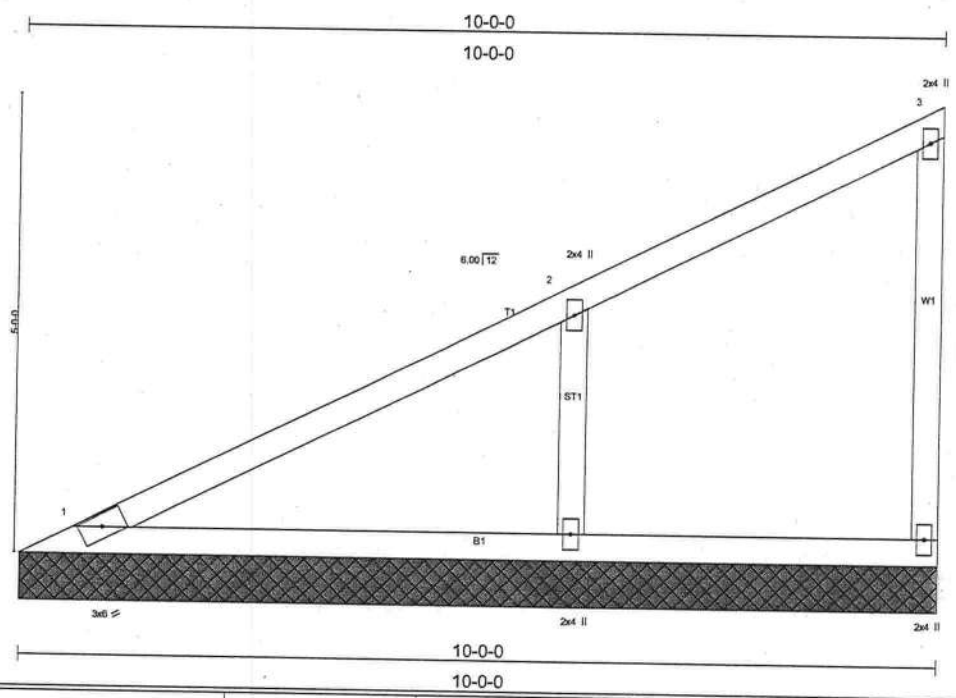
BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=138/10-4-0, 8=61/10-4-0, 6=360/10-4-0, 7=285/10-4-0
 Max Horz 8=232(load case 5)
 Max Uplift 5=75(load case 5), 6=171(load case 5), 7=231(load case 5)
 Max Grav 5=138(load case 1), 8=159(load case 5), 6=360(load case 1), 7=285(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-390/26, 2-3=-226/41, 3-4=-63/30, 4-5=-86/128, 1-8=-170/0
 BOT CHORD 7-8=-11/9, 6-7=-11/9, 5-6=-11/9
 WEBS 3-6=-233/333, 2-7=-183/326

NOTES
 1) Wind: ASCE 7-98; 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Gable requires continuous bottom chord bearing.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 5, 171 lb uplift at joint 6 and 231 lb uplift at joint 7.

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) n/a - n/a 999	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(TL) n/a - n/a 999		
BCLL 10.0	Rep. Stress Incr YES	WB 0.11	Horz(TL) -0.00 4 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 41 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 1=185/10-0-0, 4=104/10-0-0, 5=487/10-0-0
 Max Horz 1=213(load case 5)
 Max Uplift 4=54(load case 5), 5=-250(load case 5)

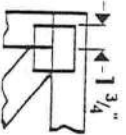
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-268/68, 2-3=-67/26, 3-4=-69/109
 BOT CHORD 1-5=-6/5, 4-5=-6/5
 WEBS 2-5=-301/425

- NOTES**
- 1) Wind: ASCE 7-98: 110mph (3-second gust); h=22ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) Gable requires continuous bottom chord bearing.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 4 and 250 lb uplift at joint 5.

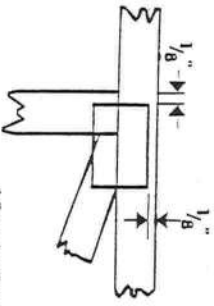
LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

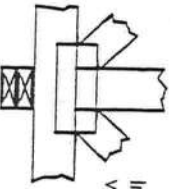
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



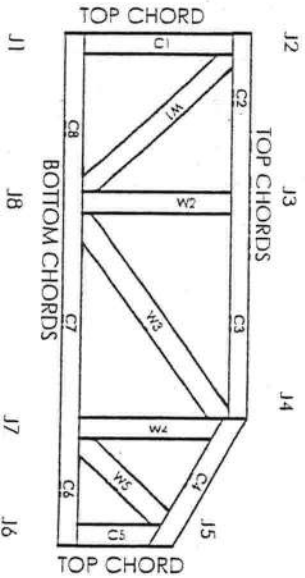
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System

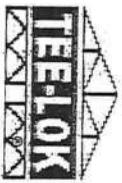


JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561



MITTEK Engineering Reference Sheet: MIT-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

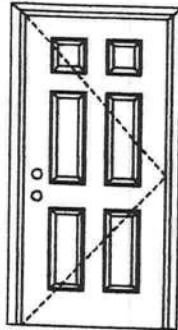
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X
Opaque Outswing Unit

COP-WL-JH4121-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3029447A and COP/Test Report Validation Matrix #3029447A-001 provides additional information - available from the ITS/WH website (www.etsmko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0011-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:



Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

Johnson
EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation

X

Opaque Outswing Unit

COP-WL-JH4121-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).



State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson
EntrySystems

June 17, 2002

Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from
Masonite

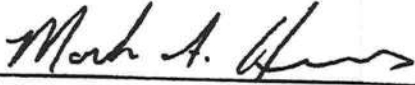
Masonite International Corporation

Test Results: (Continued)


<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	<i>*Exceeds L/175 for deflection, but meets all other test requirements.</i>		
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC.


Mark A. Hess
Technician

MAH:nlb
01-41641.01


Allen N. Reeves, P.E.
Director - Engineering Services
7 JUNE 2002



Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max. 0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs Right sash, meeting rail Right sash, bottom rail Middle sash, meeting rail Middle sash, bottom rail Left sash, meeting rail Left sash, bottom rail In remaining direction at 50 lbs Right sash, right stile Right sash, left stile Middle sash, right stile Middle sash, left stile Left sash, right stile Left sash, left stile	0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97) Type: A Grade: 10 Lock Manipulation Test Test A1 through A5 Test A7 Lock Manipulation Test	No entry No entry No entry No entry	No entry No entry No entry No entry

Allen N. Reeves
7 JUNE 2002



Test Specimen Description: (Continued)

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft ²	0.3 cfm/ft ² max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)
(with and without screen)
WTP = 2.86 psf

No leakage

No leakage

Allen N. Reeves
7 JUNE 2002



Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.





Architectural Testing

AAMA/NWWDA 101/LS.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/LS.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com



Allen N. Reeves
7 JUNE 2002

**AAMA/NWWDA 101/L.S.2-97
TEST REPORT**

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650

TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft ²
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

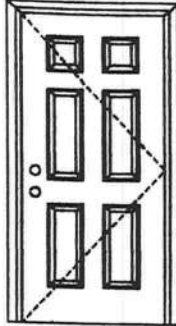
Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002



WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
 Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
 Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
 limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

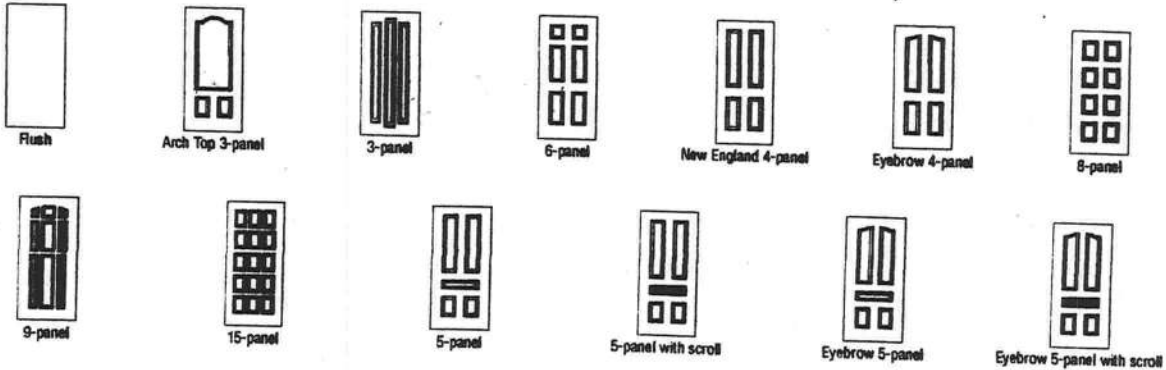
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:



WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).



State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A
and COP/Test Report Validation Matrix
#3026447A-001 provides additional
information - available from the ITS/WH
website (www.itswh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Johnson™
EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection
Premium Quality Doors



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Masonite®
Masonite International Corporation



NATIONAL CERTIFIED TESTING LABORATORIES

1484 GEMINI BOULEVARD • ORLANDO, FLORIDA 32837
PHONE (407) 240-1388 • FAX (407) 246-3882
www.nctl.com

STRUCTURAL PERFORMANCE TEST REPORT

Report No: NCTL-210-2065-1
Test Date: 08-21-00
Report Date: 09-25-00
Expiration Date: 09-25-04
Revision Date: 01/31/02

Client: ML Home Products, Inc.
650 West Market Street
Gratz, PA 17050-0379

Test Specimen: ML Home Products Product's Series 480 Type OXE Aluminum Sliding Glass Door.
(SQD-C95)(Single Glazed)(Steel Reinforced)(with and without sill rest).

Test Method: AAMA/NWWDA 101/LS-3-97, "Voluntary Specifications for Aluminum, Vinyl (PVC), and Wood Windows and Glass Doors."

Revision Note: Sill leg extension was revised from 1-1/8" to 1-1/4"

TEST SPECIMEN DESCRIPTION

General: The sample tested was a three (3) panel type OXE aluminum sliding glass door measuring 16-1/4" wide x 8'0-1/8" high overall. The active panel measured 5'0-1/2" wide by 7'11-1/8" high; the fixed panel measured 5'0-7/8" wide by 7'11-1/8" high. Frame and panel members were not thermally broken. A plastic spacer/guide was used at each panel head/stile corner. The fixed panel was secured to the jamb with two (2) 3" long aluminum angle retainers each fastened to the jamb stile with two (2) (#8 x 3/4") pan head screws. One (1) claw type door lock assembly was located at 40" from the bottom of each active panel lock stile each with two (2) screws. One (1) adjustable metal roller assembly was used at each end of the active bottom rails. The frame was of double screw coped corner construction. Panel corners were of single screw at bottom rail and double screw at the top rail. The interior vertical sill leg employed an extruded minimum 1-1/4" high extension; an overall height of 2.021. One (1) aluminum panel retainer was used at 2" from each of the active panel bottom rail. One (1) extruded aluminum female panel was fastened to the fixed panel but stile with five (5) (#8 x 1/2") screws. One extruded panel adapter was fastened to the batt stile using five (5) (#8 x 1/2") screws.

The frame was fastened to the wood test buck using forty-eight (48) (# 8 x 1-1/2") screws (see diagram).

Each panel was reinforced with galvanized steel reinforcing channels measuring 1-3/4" x 3/4" x 1/8" (the panel adapter stile. One (1) L-shaped galvanized steel channel measuring 3/4" x 7/8" x 1/16" thick fitted the length of each interlock stile.

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Glazing: All panels were channel glazed using 3/16" thick clear tempered glass with a flexible vinyl glazing bead.

Weatherseal: Double strips of centerfin weatherstrip (0.270" high) were located at each jamb, stile and lock stile. A double strip of centerfin weatherstrip (0.150" high) was located at each interlock stile. A double strip of centerfin weatherstrip (0.250" high) was located at each panel top rail. A double strip of side fin weatherstrip (0.430" high) was located at each panel bottom rail. An adhesive back polyurethane dust plug measuring 1.3/16" x 18.16" x 0.430" was located on the head and sill at each end of the vertical stile exterior track.

Weeps: One (1) weep notch measuring 1-1/2" x leg height was located at each end of the interior sill roller leg, exterior sill roller leg and screen sill roller leg.

Interior & Exterior Surface Finish: Non-painted aluminum.

Sealant: Frame and panel bottom rail corners were sealed with a small joint sealant.

Insect Screens: Two (2) insect screens; one (1) center insect screen measuring 59.174" wide by 71.1" high. Both were of spaced corner construction. The screen employed four glass mesh clips with a hollow vinyl spline. One (1) roller assembly was located at each end of the bottom rails. One (1) clevis type lock assembly.

TEST RESULTS

<u>Par. No.</u>	<u>Title of Test & Method</u>	<u>Measured</u>	<u>Allowed</u>	
2.2.1.6.1	<u>Operating Force</u> <u>Center Active Panel</u>	To open	20 lbf	
		In Motion	5 lbf	
	<u>Right Active Panel</u>	To open	20 lbf	
		In Motion	5 lbf	
	2.2.1.6.2	<u>Deglazing - ASTM E957</u> <u>Center Active Panel</u>	<u>Top Rail (30 lbf)</u>	10.2% (0.051)
			<u>Bottom Rail (50 lbf)</u>	7.3% (0.037)
<u>Left Stile (70 lbf)</u>			6.0% (0.030)	
<u>Right Stile (70 lbf)</u>			5.4% (0.027)	
<u>Right Active Panel</u>		<u>Meeting Rail (50 lbf)</u>	2.2% (0.011)	
		<u>Bottom Rail (50 lbf)</u>	2.2% (0.011)	
		<u>Left Stile (70 lbf)</u>	2.0% (0.010)	
		<u>Right Stile (70 lbf)</u>	6.2% (0.031)	

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TEST RESULTS (Continued)

<u>Par. No.</u>	<u>Title of Test & Method</u>	<u>Measured</u>	<u>Allowed</u>
2.1.2	Air Infiltration 1.57 psf (25mph)	Passed	0.30 cfm / ft ²
2.1.3	Water Resistance - ASTM E847 5.0 gph / ft ² WTP=4.50 psf	No entry	No entry
2.1.4.B	Uniform Load Structural - ASTM E999 45.0 psf Exterior 45.0 psf Interior	0.345" 0.355"	0.381" 0.381"

OPTIONAL PERFORMANCE

<u>Par. No.</u>	<u>Title of Test & Method</u>	<u>Measured</u>	<u>Allowed</u>
4.3	Water Resistance - ASTM E547 & E831 5.0 gph / ft ² WTP=6.25 psf	No Entry	No Entry

Note: At this point in testing, an additional sill test was attached to the existing main sill & interior vertical leg with the following results being obtained:

<u>Par. No.</u>	<u>Title of Test & Method</u>	<u>Measured</u>	<u>Allowed</u>
4.3	Water Resistance - ASTM E547 & E831 5.0 gph / ft ² WTP=6.00 psf	No Entry	No Entry
4.4.2	Uniform Load Structural - ASTM E999 52.5 psf Exterior 52.5 psf Interior	0.375" 0.380"	0.381" 0.381"

* Test performed with and without screens

TEST COMPLETED 06/21/00

Note: In addition, MI Home Products Series 430 and 440 also received an SGB-C35 rating being identical in panel construction and interior sill leg height.




MI Home Products

NCTL-210-2065-1

The tested specimen meets (or exceeds) the performance levels specified in Table 2.1 of AAMA/NWDA 101/19.2-97 for air infiltration. The listed results were secured by using the designated test methods and indicate compliance with the performance requirements of the referenced specification paragraphs for the SGD-C35 product designation.

Detailed drawings were available for laboratory records and compared to the test specimen at the time of this report. A copy of this report along with representative sections of the test specimen will be retained by NCTL for a period of four (4) years. The results obtained apply only to the specimen tested. No conclusions of any kind regarding the adequacy or inadequacy of the glass in the test specimen may be drawn from this test. This report does not constitute certification of the product which may only be granted by a certification program validator.

Michael E. Lane 
MICHAEL E. LANE
Division Manager





**AAMA/NWDA 101/I.S.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650

TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf
Air Infiltration	-47.2 psf
Water Resistance	0.04 cfm/ft ²
Structural Test Pressure	8.25 psf
Forced Entry Resistance	+67.5 psf
	-70.8 psf
	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.


Mark A. Hess, Technician

MAH:nlb

Allen N. Reeves
1 APRIL 2002





Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Picture Window

Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

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1 APR 12 2002





Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.04 cfm/ft ²	0.3 cfm/ft ² max.
	Water Resistance (ASTM E 547-00) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.01" 0.01"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.0" 0.01"	0.29" max. 0.29" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.



Allen H. Reeves
1 APRIL 2002



Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
	Forced Entry Resistance (ASTM F 588-97)		
	Type: D Grade: 10		
	Hand and Tool Manipulation Test	No entry	No entry
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) WTP = 8.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.02"	0.41" max.
	@ 47.2 psf (negative)	0.02"	0.41" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.01"	0.29" max.
	@ 70.8 psf (negative)	0.02"	0.29" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess
Technician

MAH:nlb
01-41135.01

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002





**AAMA/NWWDA 101/L.S.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650

TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf -47.2 psf
Air Infiltration	0.04 cfm/ft ²
Water Resistance	8.25 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Allen N. Reeves
1 APRIL 2002





Architectural Testing

AAMA/NWWDA 101/L.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/L.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Picture Window

Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

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Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.04 cfm/ft ²	0.3 cfm/ft ² max.
	Water Resistance (ASTM E 547-00) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.01" 0.01"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.0" 0.01"	0.29" max. 0.29" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.



Allen H. Reeves
1 APRIL 2002



Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
	Forced Entry Resistance (ASTM F 588-97)		
	Type: D Grade: 10		
	Hand and Tool Manipulation Test	No entry	No entry
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) WTP = 8.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.02"	0.41" max.
	@ 47.2 psf (negative)	0.02"	0.41" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.01"	0.29" max.
	@ 70.8 psf (negative)	0.02"	0.29" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:nlb
01-41135.01

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002



**AAMA/NWDA 101/LS.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

**SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window**

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft ²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.


Mark A. Hess, Technician

MAH:nlb

Allen N. Reeves
1 APRIL 2002





Architectural Testing

AAMA/NWWDA 101/L.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/L.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

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Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen H. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft ²	0.3 cfm/ft ² max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.42"* 0.43"*	0.26" max. 0.26" max.
	<i>*Exceeds L/175 for deflection, but passes all other test requirements.</i>		
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.02" 0.02"	0.18" max. 0.18" max.

Allen N. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.18" max.

Allen N. Reeves
1 APRIL 2002



Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:



Mark A. Hess
Technician

MAH:nlb
01-41134.01



Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002





SOS Item No.: 89087
Lead Time: 24 Days



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LOWE'S WINDCODE® RESIDENTIAL RETAIL PRICING



W3 WINDCODE® Price List
Design Load PSF +19 to +22
Test Load PSF +29 to +33

FEATURES:

- Approximately 110 MPH Test Wind Speed
- Approved up to 16' Wide and 8' High
- Heavier Grade Track
- Upgraded 14 Gauge Hinges (As Needed)
- Required Steel Reinforcement Struts
- Additional Stiles (As Needed)
- Upgraded Springing
- Commercial Duty 10 Ball Steel Rollers
- Approved Drawings Available At Your Local Distribution Center

W3 SOLID	6'6" TO 7'0" HIGH DOORS									7'6" TO 8'0" HIGH DOORS								
	WIDTH									WIDTH								
	8'	9'	10'	12'	14'	15'	16'	17'	18'	8'	9'	10'	12'	14'	15'	16'	17'	18'
42 W3	—	—	468	538	570	593	644	—	—	—	—	512	648	698	738	769	—	—
48 W3	—	—	565	653	754	755	782	—	—	—	—	673	824	932	956	974	—	—
52 W3	—	—	524	657	751	778	790	—	—	—	—	667	804	924	931	941	—	—
62, 64B, 65 W3	438	474	643	781	861	896	910	—	—	520	568	790	929	1035	1096	1135	—	—

Models in shaded area not available.
Window options: available with single strength clear glass — refer to Window Chart, page 7.
Each door includes two point locking; door cannot be purchased without two point locking.
All doors over 9' wide include EZ-Set Torsion Spring System™.
*Door does not include the EZ-Set Torsion Spring System™ due to weight restriction — door does include standard torsion springs.

Consult your local building codes for specifications.
For door specifications and other options refer to standard price list.

WC6 FOR ASSISTANCE CALL 1-877-266-8490 FAX 1-800-526-1618

Effective November 1, 2002
Subject to change without notice



Changes to the Owens Corning OAKRIDGE®
Architectural shingle line, **effective November 1, 2001**

<u>OLD Product Name</u>	<u>New Product Features</u>	<u>Shadowline Change</u>
Oakridge® 25 • 25-year Warranty • 60 mph Wind Resistance Coverage	<i>Oakridge Pro 30™ (name change)</i> • 30-year Warranty • 70 mph Wind Resistance Coverage	No change; soft shadowline with muted color blends
Oakridge® 30 • 30-year Warranty • 80 mph Wind Resistance Coverage	<i>Oakridge Pro 40™ (name change)</i> • 40-year Warranty • 80 mph Wind Resistance Coverage	No change; darker shadowline with more distinct color blends
Oakridge® 40 • 40-year Warranty • 80 mph Wind Resistance Coverage	<i>Oakridge Pro 50™ (name change)</i> • 50-year Warranty • 90 mph Wind Resistance Coverage	No change; patented double shadowline with distinct color blends

- these warranty changes are effective with roofs *installed* beginning November 1, 2001 regardless of package or literature notation (example, if an Oakridge 25 was sold and delivered to the jobsite and the roof was installed November 1, 2001 or after, the product is warranted for 30 years regardless if the package says Oakridge 25)
- updated package and literature materials to follow in the next several weeks.
- please note that there are NO shadowline or color blend changes; our Good-Better-Best product line aesthetics are still the same. This is NOT a DO-NOT-MIX situation. Only the wrappers and warranties are changing.
- use this document to help contractors and homeowners understand our changes or have them call 1-800-GET-PINK for more information.



SOS Item No.: 89087
Lead Time: 24 Days



Improving Home Improvement

Holmes Garage Door Company

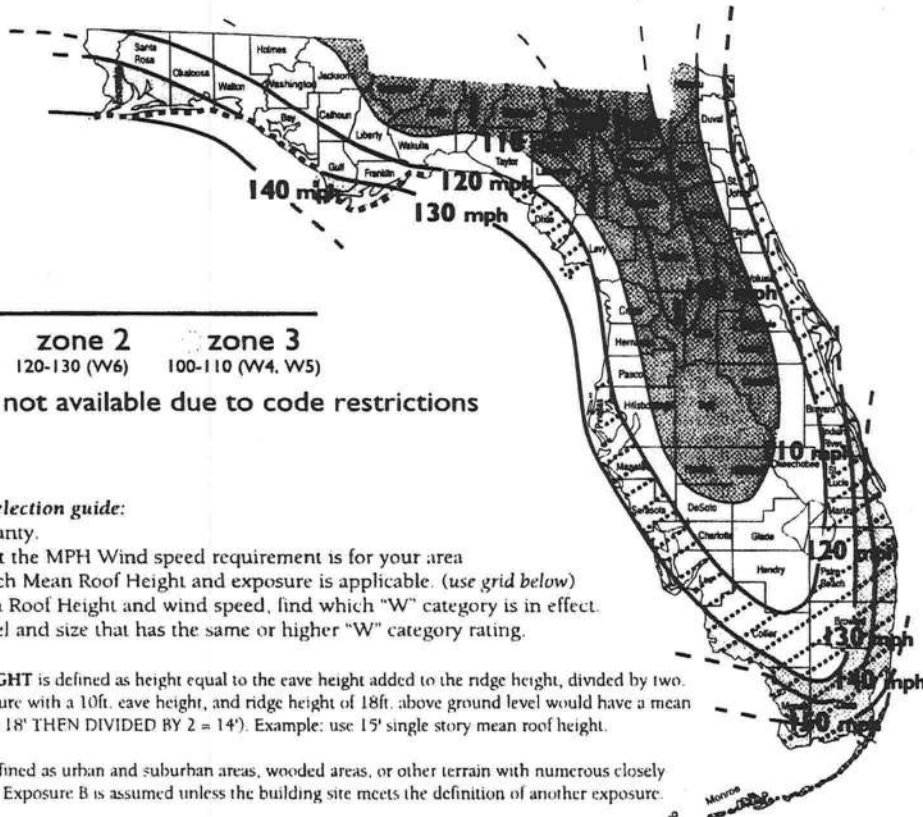


Phone: 1-877-266-8490
Fax: 1-800-526-1618

LOWE'S WINDCODE® RESIDENTIAL RETAIL PRICING



**WINDCODE®
Product Map/
Selection Guide**



zone 1 140-146 (W7, W8) zone 2 120-130 (W6) zone 3 100-110 (W4, W5)

☉ windows not available due to code restrictions

How to use this selection guide:

1. Locate your county.
2. Determine what the MPH Wind speed requirement is for your area
3. Determine which Mean Roof Height and exposure is applicable. (use grid below)
4. Using the Mean Roof Height and wind speed, find which "W" category is in effect.
5. Select the model and size that has the same or higher "W" category rating.

+MEAN ROOF HEIGHT is defined as height equal to the eave height added to the ridge height, divided by two. For example, a structure with a 10ft. eave height, and ridge height of 18ft. above ground level would have a mean height of 14ft. (10' + 18' THEN DIVIDED BY 2 = 14'). Example: use 15' single story mean roof height.

*EXPOSURE B is defined as urban and suburban areas, wooded areas, or other terrain with numerous closely spaced obstructions. Exposure B is assumed unless the building site meets the definition of another exposure.

*EXPOSURE C is defined as open terrain with scattered obstructions including flat open ground, grasslands, and shorelines in hurricane prone regions, and all of Miami-Dade and Broward CO is EXPOSURE C

** - BROWARD CO. IS 140 MPH AND MIAMI-DADE CO IS 146 MPH.

FLORIDA BUILDING CODE (ASCE7-98)- EXPOSURE B*

MEAN ROOF HEIGHT +	100 MPH	110 MPH	120 MPH	130 MPH	140 MPH	150 MPH
Up to 15' ONE-STORY	W3	W3	W4	W5	W6	W7
Up to 25' TWO-STORY	W3	W3	W4	W5	W6	W7

FLORIDA BUILDING CODE (ASCE7-98) - EXPOSURE C*

MEAN ROOF HEIGHT +	100 MPH	110 MPH	120 MPH	130 MPH	140 MPH**	146 MPH**	150 MPH
Up to 15' ONE-STORY	W3/W4	W4	W5	W6	W7	W7	W8
Up to 25' TWO-STORY	W4	W5	W6	W7	W7	W8/W9	W8/W9

1999 STANDARD BUILDING CODE (Code for all states excluding Florida)

MEAN ROOF HEIGHT +	70 MPH	80 MPH	90 MPH	100 MPH	110 MPH
Up to 15' ONE-STORY	W3	W3	W3	W4	W5
Up to 25' TWO-STORY	W3	W3	W4	W5	W6

This map is to be used as a guideline only. Contact your local building official for code details and building permit information.

WC4

FOR ASSISTANCE CALL 1-877-266-8490 FAX 1-800-526-1618

Effective November 1, 2002
Subject to change without notice

JTN: Weegie:

**Columbia County Building Department
Culvert Waiver**

**Culvert Waiver No.
000000843**

DATE: 10/14/2005 BUILDING PERMIT NO. 23716

APPLICANT TIMOTHY MORGAN PHONE 386.758.7512

ADDRESS POB 1251 LAKE CITY FL 320556

OWNER TIMOTHY MORGAN PHONE 758.7512

ADDRESS 102 NW TURKEY TROT GLEN LAKE CITY FL 32055

CONTRACTOR TIMOTHY MORGAN PHONE 386.758.7512

LOCATION OF PROPERTY 41-N TO FIDDLERS LN, TL TO STAGE COACH WAY, TR TO TURKEY TROT GLEN, TR
PROPERTY ON R @ CUL-DE-SAC.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT COUNTRY LANE ESTATES 12 A

PARCEL ID # 27-2S-16-01771-112

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: *Timothy Morgan*

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

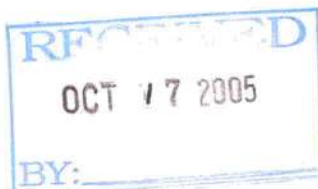
APPROVED NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: _____

SIGNED: *Peyghatto* DATE: 10/27/05

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160





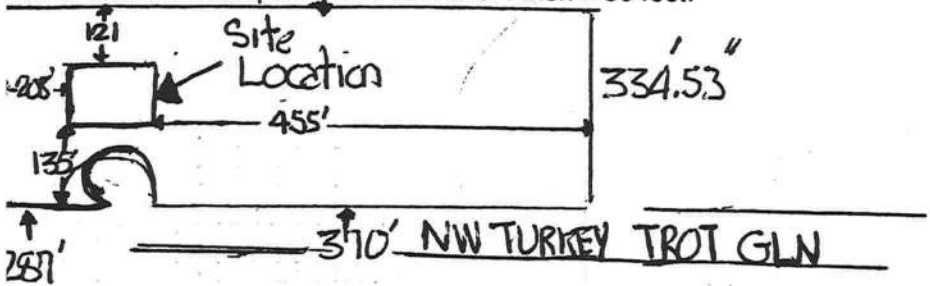
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-0919N

PART II - SITE PLAN

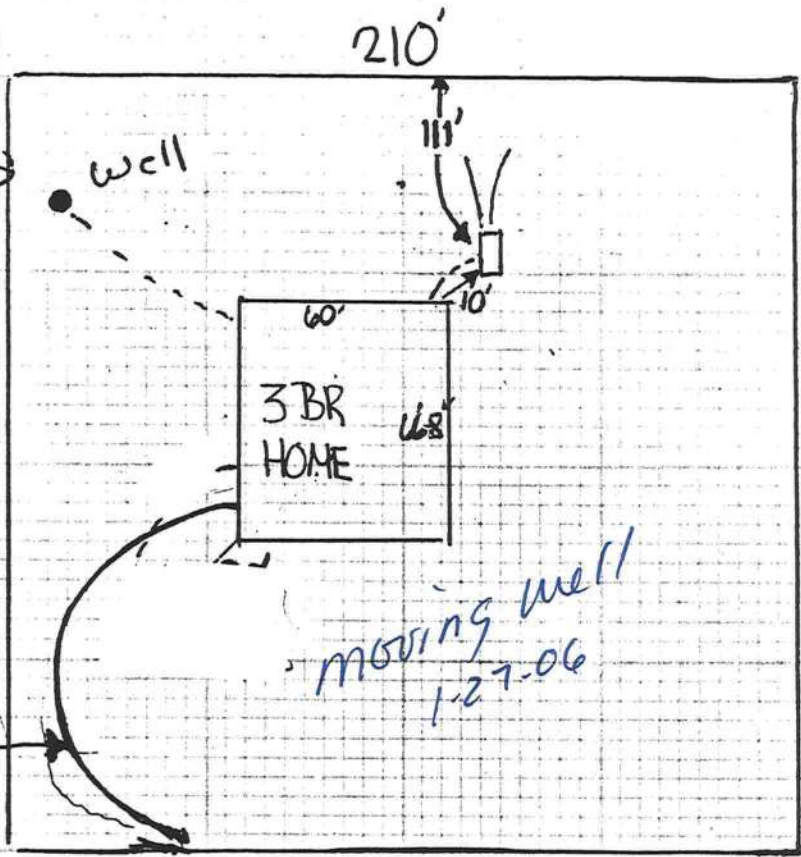
Scale: Each block represents 5 feet and 1 inch = 50 feet.



23716

NORTH
↓

Signed Timothy Mayan
Date 1/27/06
(for well replacement)
1 acre
of
5.01



Notes: _____

Site Plan submitted by: Timothy Mayan Signature

Owner Title

Plan Approved Not Approved _____

9/15/05 Date 09/07/05

By Timothy Mayan Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#23716

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any) _____

Section 2: Builder Information

Company Name: Tim Morgan Company Phone No. 758-7512
867-0432

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 102 NW Turkey Tail Glen
Lot 12A Country Lane Estates Lake City, FL
Type of Construction (More than one box may be checked) Slab Basement Crawl Other _____
Approximate Depth of Footing: Outside 1 Inside 2' Type of Fill sand

Section 4: Treatment Information

Date(s) of Treatment(s) 12/29/05
Brand Name of Product(s) Used Terminator
EPA Registration No. _____
Approximate Final Mix Solution % .06%
Approximate Size of Treatment Area: Sq. ft. 3436 Linear ft. 290 Linear ft. of Masonry Voids 274 1/2
Approximate Total Gallons of Solution Applied 675 gals.
Was treatment completed on exterior? Yes No
Service Agreement Available? Yes No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments _____

Name of Applicator(s) S. Gregory Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [Signature] Date 12/29/05

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)

CENTRAL AVENUE OF COLUMBIA COUNTY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 27-2S-16-01771-112

Building permit No. 000023716

Use Classification SFD/UTILITY

Fire: 22.32

Permit Holder TIMOTHY MORGAN

Waste: 67.00

Owner of Building TIMOTHY MORGAN

Total: 89.32

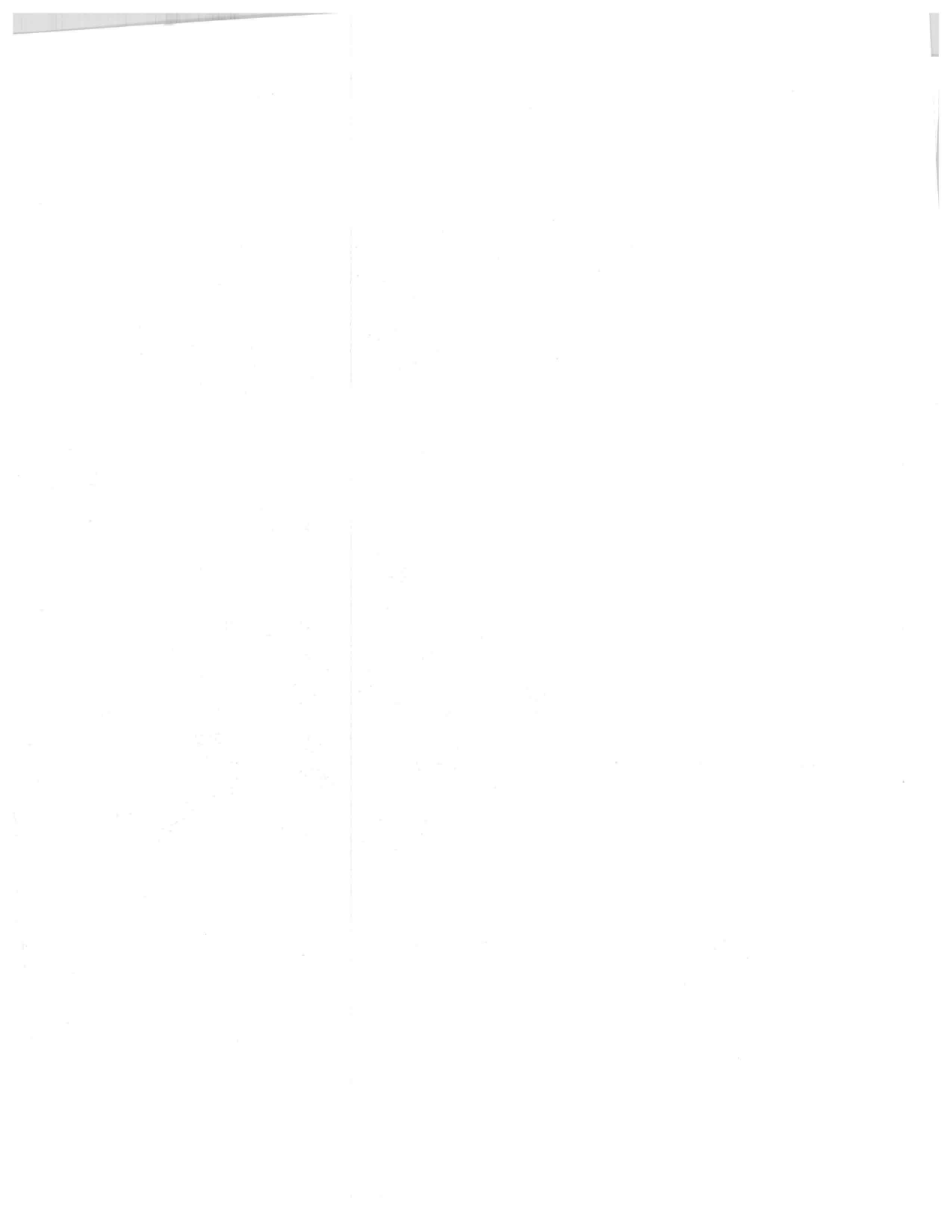
Location: 102 NW TURKEY TROT GLEN, LAKE CITY, FL

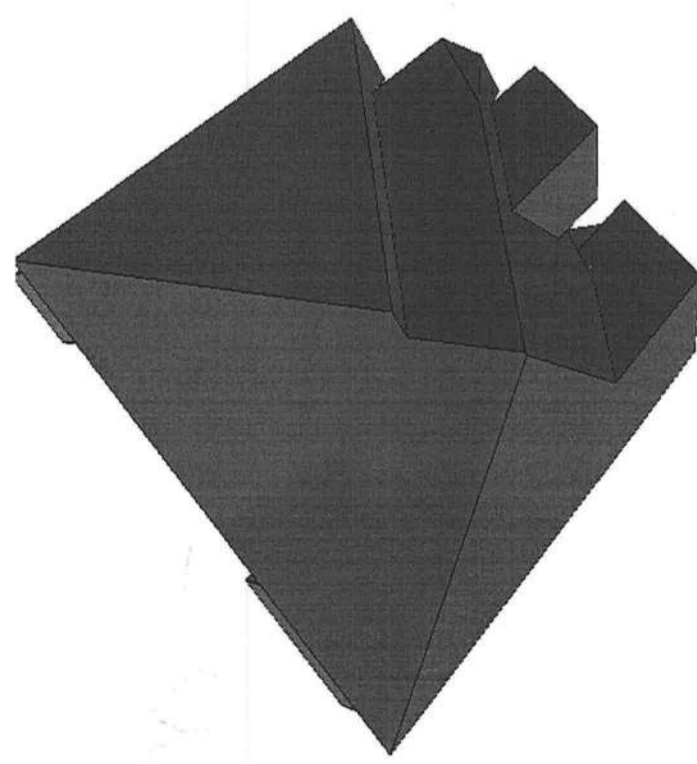
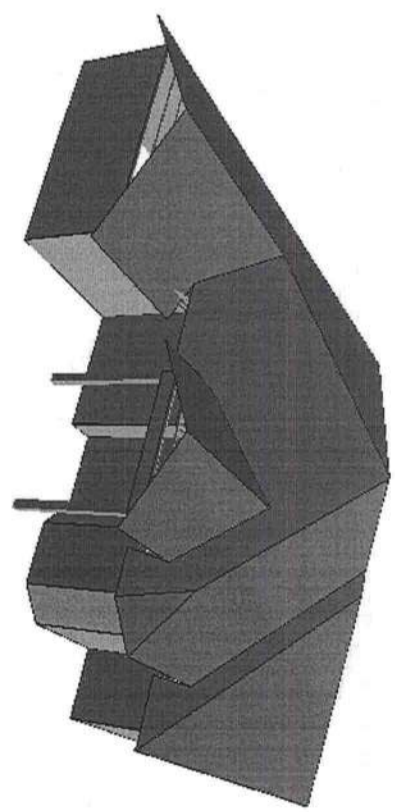
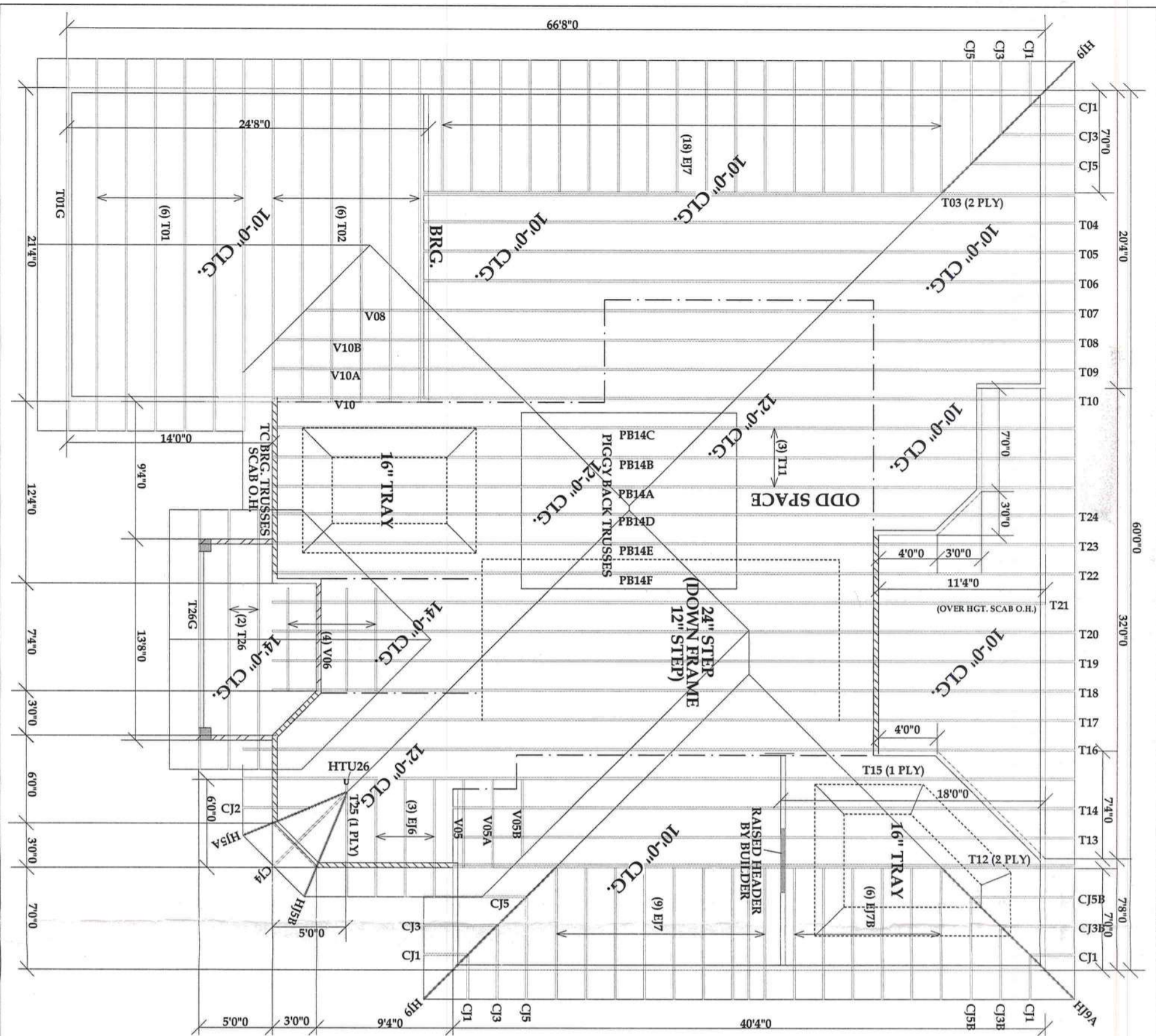
Date: 06/07/2007



[Signature]
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)





BEARING HEIGHT SCHEDULE

	10'-0"
	12'-0"
	14'-0"

EXTERIOR WALL SIZE OVERHANG ROOF PITCH(S)

NOTES:

- REFER TO HD 91 RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BE REFER TO ENGINEERED DRAWINGS FOR PERMAN BRACING REQUIRED.
- ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECAYED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS.
- ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- ALL TRUSSES ARE DESIGNED FOR 2' O.C. MAXIMUM SPACING UNLESS OTHERWISE NOTED.
- ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING UNLESS OTHERWISE NOTED.
- 5/42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- ALL 800# TRUSS HANGERS TO BE SUPPLIED BY BUILDER UNLESS OTHERWISE NOTED. ALL 1/2\"/>

SHOP DRAWING APPROVAL

THIS DRAWING IS THE SOLE SOURCE FOR FABRICATION TRUSSES AND V05S. ALL PREVIOUS ARCHITECTURAL OR TRUSS LAYOUTS, REVISED AND APPROVAL OF THIS LAYOUT BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VARIATIONS TO INSURE AGAINST CHANGES THAT WILL BE IN EXTRA CHARGES TO YOU.



Builder's FirstSource
 Durnell
 PHONE: 904-437-3549 FAX: 904-437-3549
 Jacksonville
 PHONE: 904-772-6100 FAX: 904-772-6100
 Lake City
 PHONE: 904-755-6894 FAX: 904-755-6894
 Sanford
 PHONE: 407-322-0059 FAX: 407-322-0059

OWNER/BUILDER

LEGAL ADDRESS:
MORGAN RESIDENCE I
 MODEL: CUSTOM
 REVISION: SCALE: 1/8\"/>

HANGER SCHEDULE
 (1) HTU26