

Daniel Shaheen
Daniel Shaheen

October 16, 2007

D.D.S. Studios

ARCHITECTURAL DESIGN

P.O. Box 273
LAKE CITY FL 32056
(386) 754-0181

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MARK DISOSWAY, P.E.
POB 888 LAKE CITY, FL 32056
PH: (804) 754-5419
FLORIDA P.E. 53915

ENGINEERED BY:

A CUSTOM HOME BY EWPL INC.

WRUBEL RESIDENCE

LOT 19, ROLLING MEADOWS

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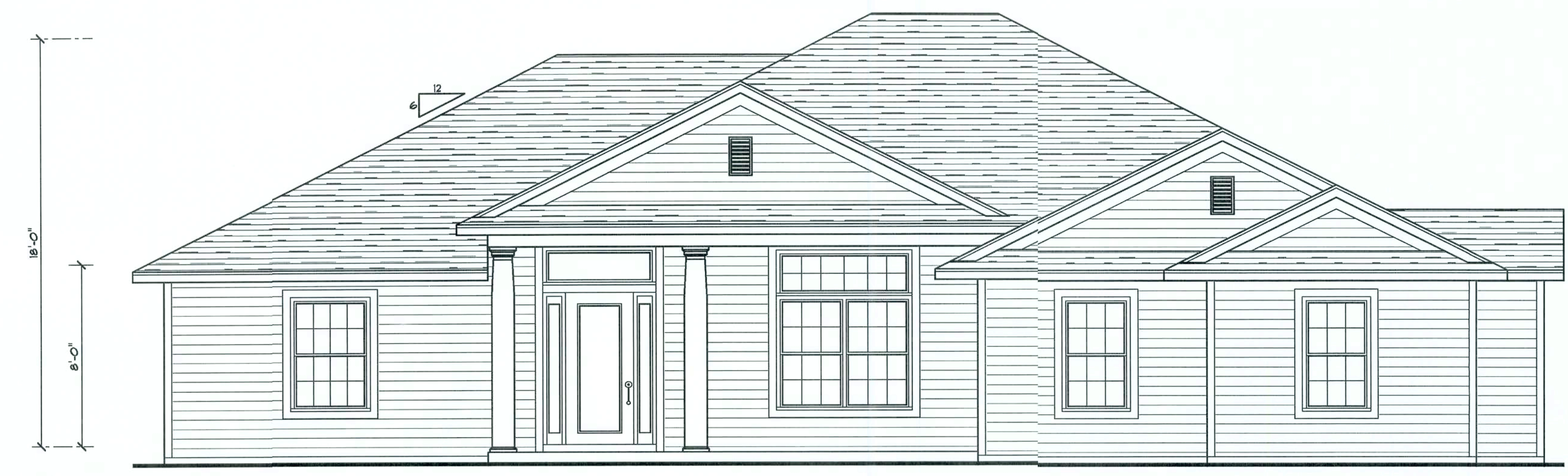
PROJECT INFO:

EXTERIOR ELEVATIONS

SHEET NUMBER
1 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

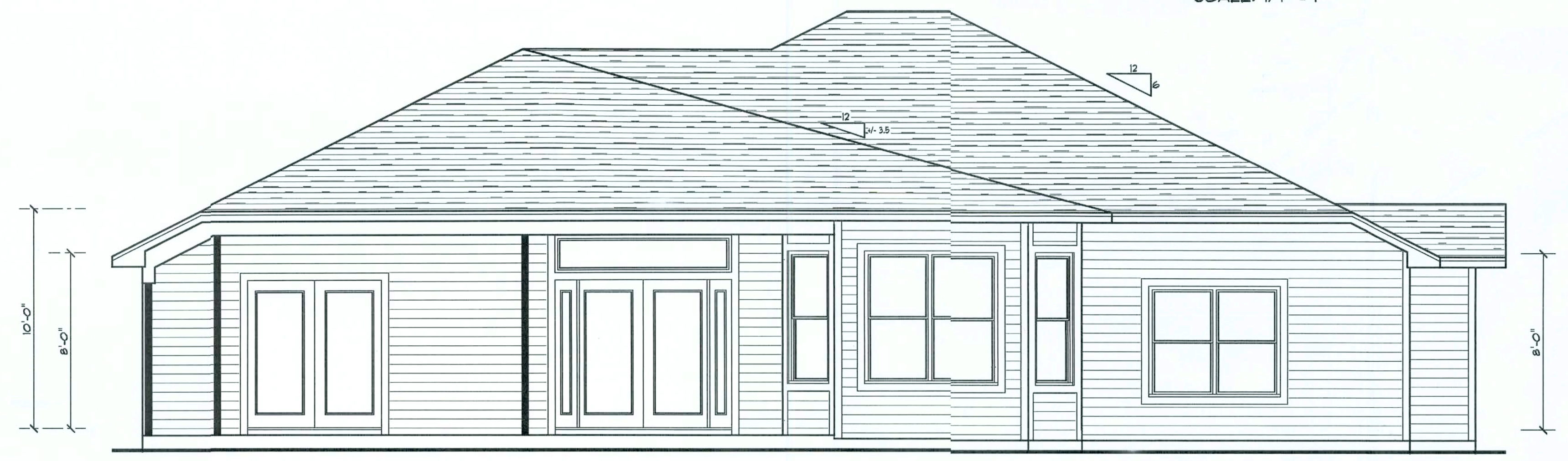
ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



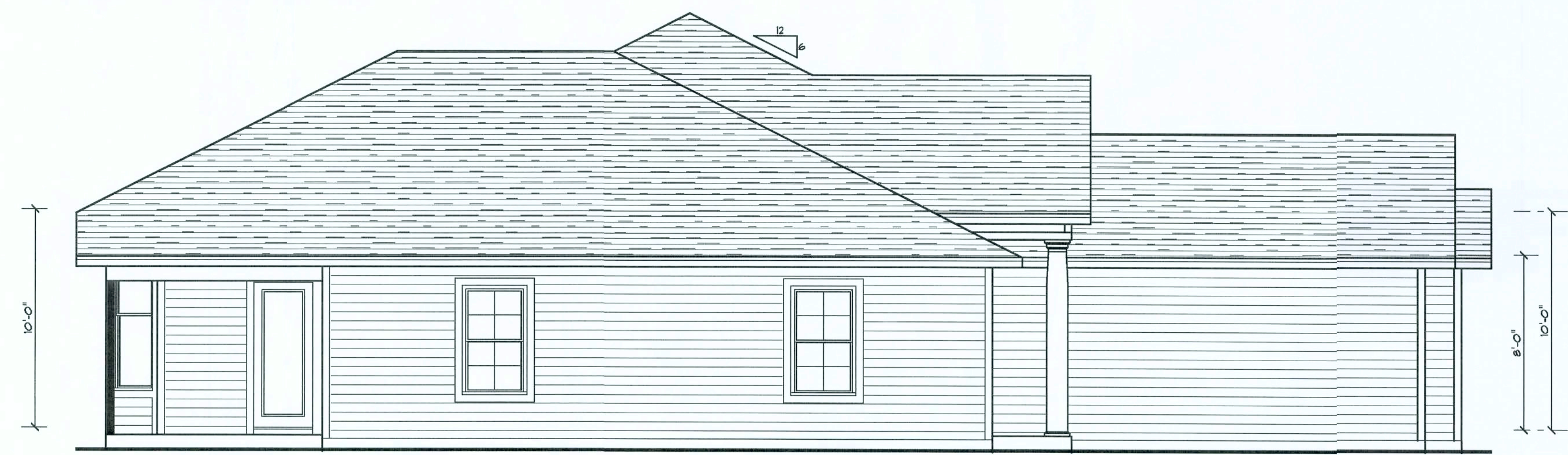
FRONT ELEVATION
SCALE: 1/4" = 1'



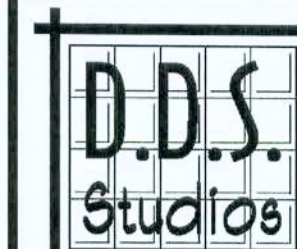
RIGHT ELEVATION
SCALE: 1/4" = 1'



REAR ELEVATION
SCALE: 1/4" = 1'



LEFT ELEVATION
SCALE: 1/4" = 1'



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LOT 19, ROLLING MEADOWS
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PROJECT INFO:

FLOOR PLAN
TYPICAL WALL SECTION

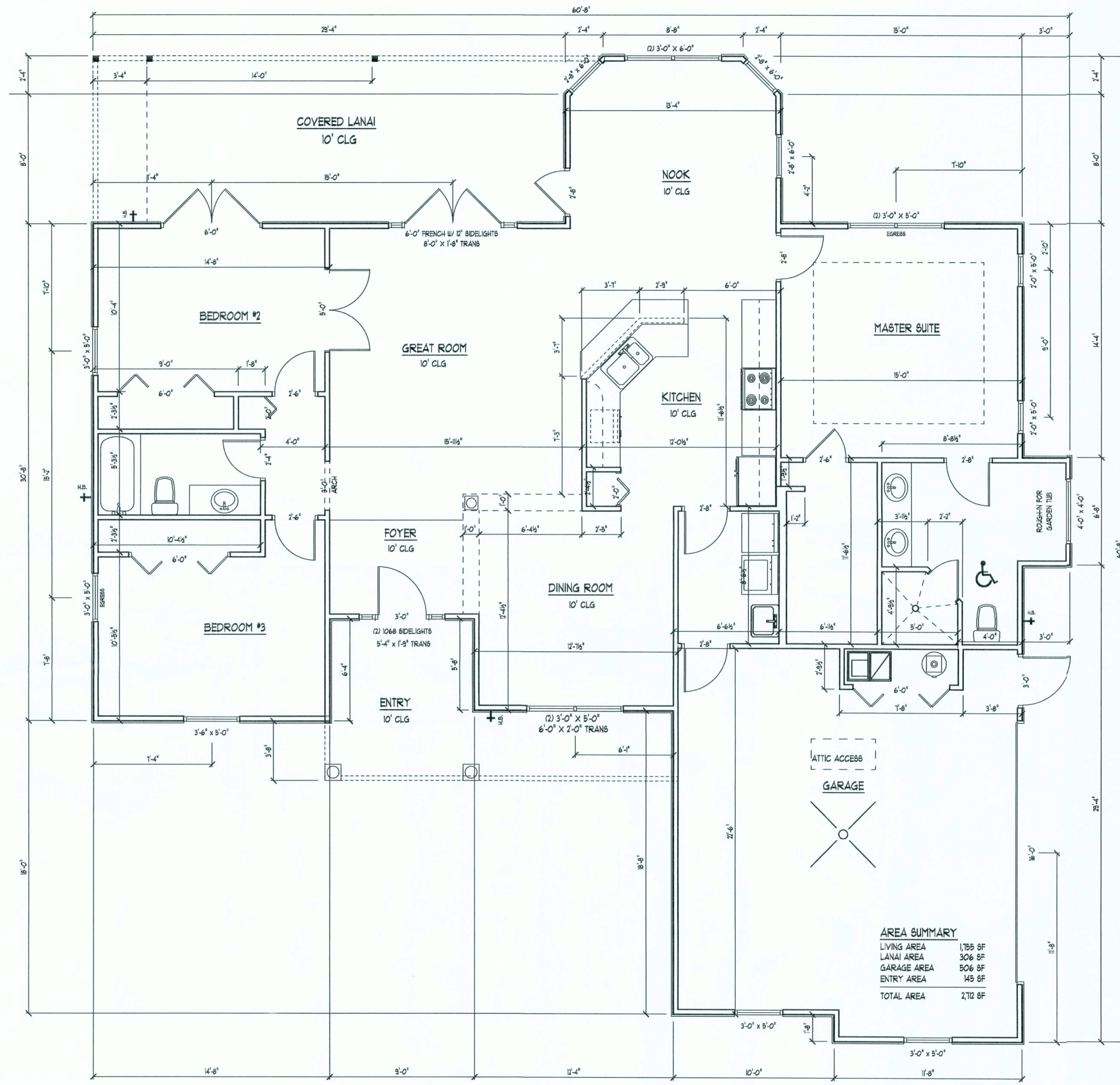
SHEET NUMBER
2 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

ALL DRAWINGS NOT TO BE SCALED. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

- GAF-TIMBERLINE SHINGLES W/ 4-NAILS IN EACH SHINGLE STRIP ON 30-LB FELT PAPER OVER 1/8" ORIENTED STRAND BOARD ROOF SHEATHING FASTENED AS PER WINDLOAD ANALYSIS
- FLASHING: 26 ga. GALVANIZED STEEL
- PRE-ENGINEERED WOOD ROOF TRUSSES AT 24" O.C. (SELECT TRUSS CONNECTORS PER WINDLOAD ANALYSIS)
- BLOW-IN INSULATION EQUAL TO R-30
- (2) 2X4 GYP DOUBLE TOP PLATE
NOTE: SEAL ALL PENETRATIONS IN TOP PLATE AND FIRE STOP BLOCKING WITH CODE APPROVED SEALANT
- 2x8 P.T. FASCIA W/ 1X4 DRIP NAILER
- ALUMINUM DRIP EDGE HOLDING, AND VENTED SOFFIT
- INTERIOR FINISH - 1/2" GYP/UM WALLBOARD
- 2X4 #2 SPF PRECUT STUDS AT 16" O.C. WITH FULL-THICK FIBERGLASS INSULATION EQUAL TO R-11
- EXTERIOR FINISH TO BE HARD-PLANK LAP SIDING
- 1/8" O.S.B. WALL SHEATHING (BLOCK ALL EDGES) FASTENED AS PER WINDLOAD ANALYSIS
- FLOORING AND INTERIOR TRIM PER SPECIFICATIONS
- 4" CONCRETE FLOOR SLAB REINFORCED WITH WELDED WIRE MESH EMBEDDED 2" IN SLAB ON 6 MIL POLY VAPOR BARRIER (6" LAPS SEALED WITH POLY TAPE) OVER COMPACTED FILL TREATED WITH TERMITICIDE
- 2 x 4 P.T. PINE SOLE PLATE ANCHORED WITH WITH ANCHOR BOLTS AS PER WINDLOAD ANALYSIS
- 1.45, CONTINUOUS, IN CONCRETE BOND BEAM AT SLAB EDGE INTERSECTION WITH STEMWALL
- APPROXIMATE FINISH GRADE

TYPICAL WALL SECTION
SCALE: 1" = 1'-0"



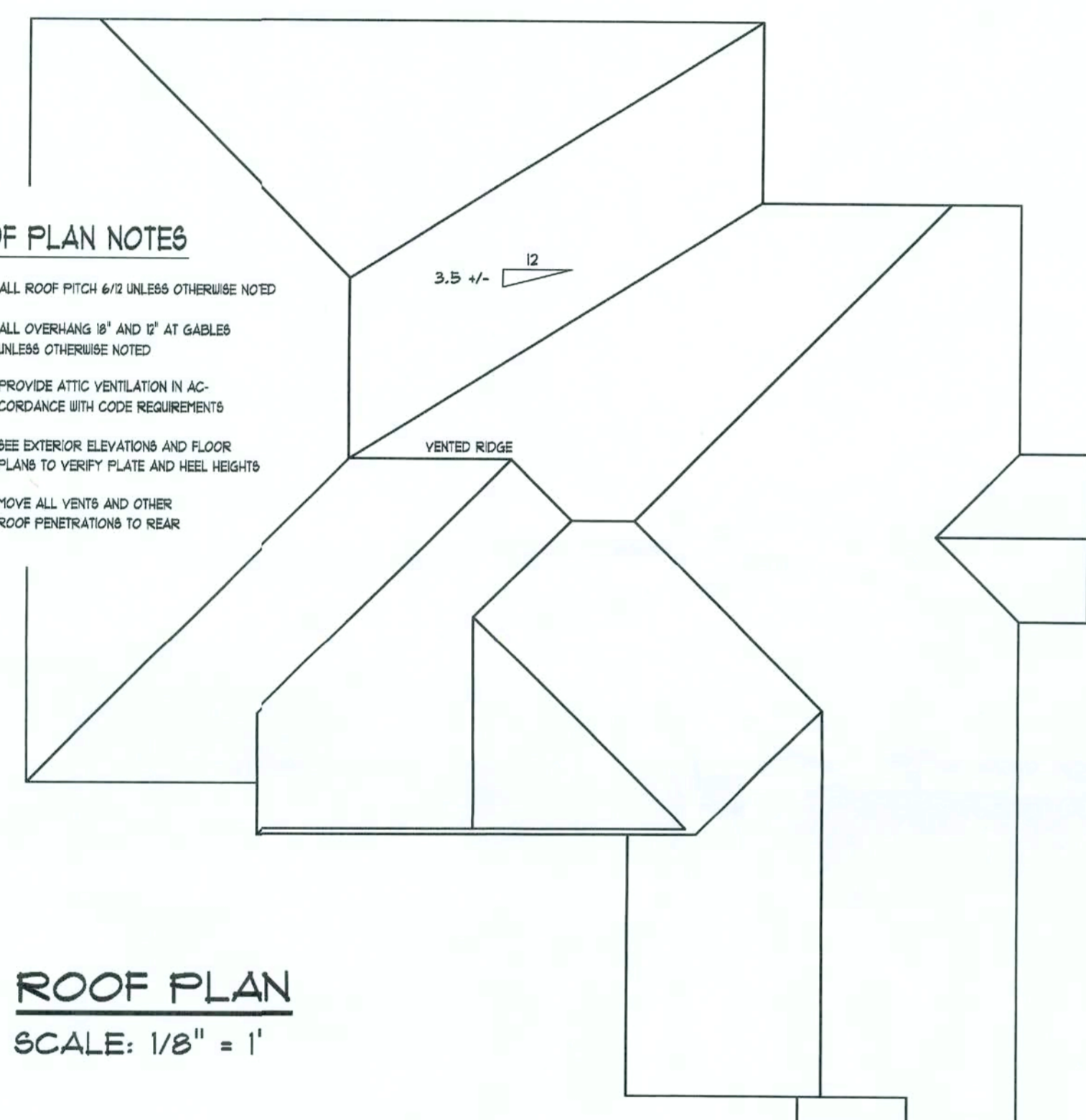
FLOOR PLAN
SCALE: 1/4" = 1'

AREA SUMMARY

LIVING AREA	1,755 SF
LANAI AREA	306 SF
GARAGE AREA	506 SF
ENTRY AREA	145 SF
TOTAL AREA	2,712 SF

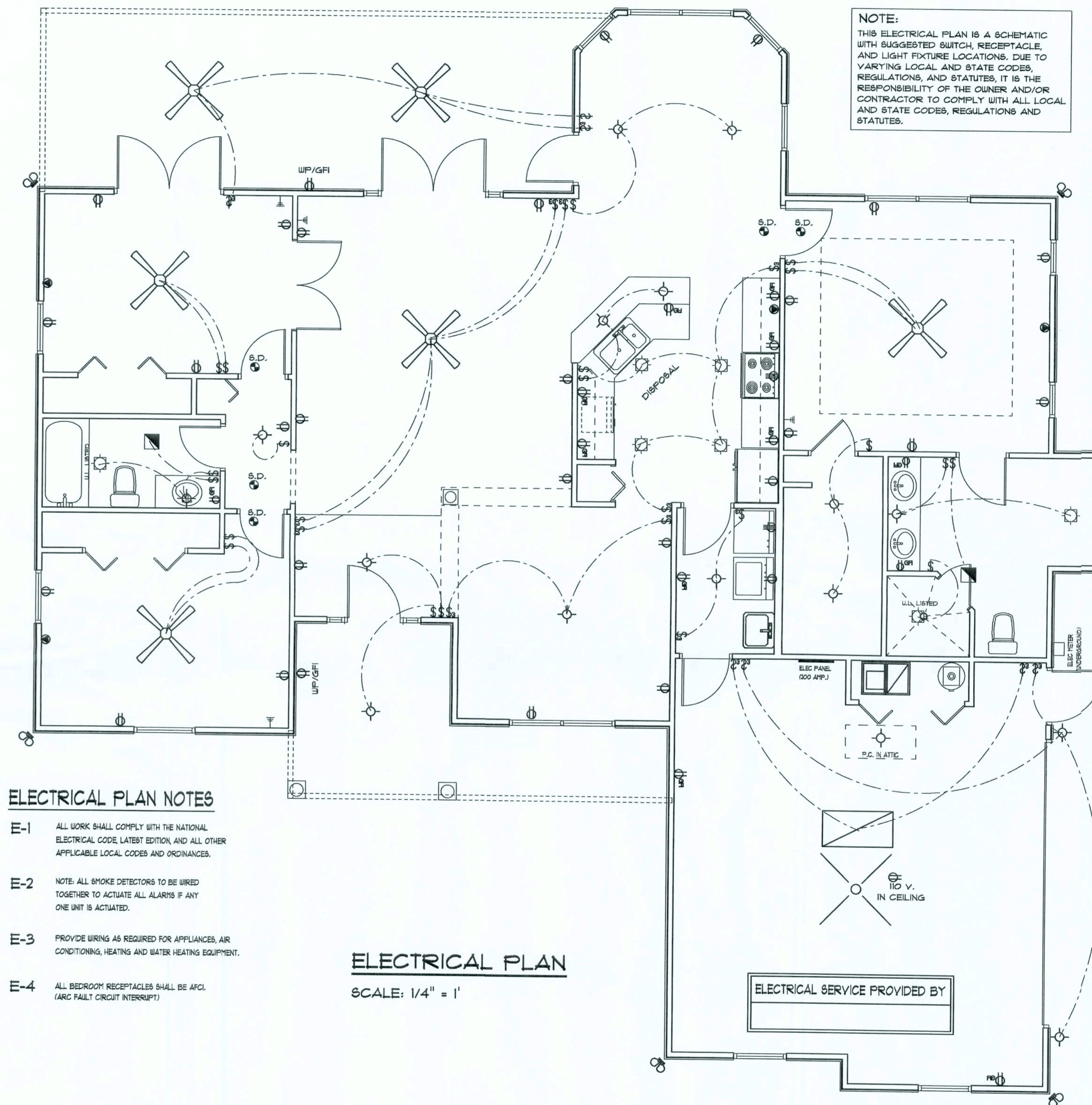
ROOF PLAN NOTES

- R-1 ALL ROOF PITCH 6/12 UNLESS OTHERWISE NOTED
- R-2 ALL OVERHANG 18" AND 1' AT GABLES UNLESS OTHERWISE NOTED
- R-3 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIREMENTS
- R-4 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HEEL HEIGHTS
- R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR



ROOF PLAN
SCALE: 1/8" = 1'

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



NOTE:
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS. DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.

ELECTRICAL PLAN NOTES

- E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.
- E-2 NOTE: ALL SMOKE DETECTORS TO BE WIRED TOGETHER TO ACTIVATE ALL ALARMS IF ANY ONE UNIT IS ACTIVATED.
- E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.
- E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

ELECTRICAL PLAN
SCALE: 1/4" = 1'

ELECTRICAL SERVICE PROVIDED BY

NO.	REVISIONS



WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056; 386-754-5419

DESIGNER: Stuffed dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
25 OCT 07
SEAL

Ewpl, Inc.
Wrubel Residence

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Fax: (386) 269 - 4871

PRINTED DATE:
October 24, 2007

STRUCTURAL BY:

FINALS DATE:
24 / Oct / 07

JOB NUMBER:
710164

DRAWING NUMBER
S-1
OF 3 SHEETS

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTERS/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-6d	4-6d	
< 360	< 235	H4	4-6d	4-6d	
< 455	< 320	H3	4-6d	4-6d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 965	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 1050	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 625	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	10-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	10-10d, 1 1/2"	
< 1000	< 960	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
< 3965	< 3330	HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 10980	< 6485	HGT-2	16-10d	12-5/8" THREADED ROD 1/2" EMBEDMENT	
< 10530	< 9035	HGT-3	16-10d	2-5/8" THREADED ROD 1/2" EMBEDMENT	
< 9250	< 9250	HGT-4	16-10d	2-5/8" THREADED ROD 1/2" EMBEDMENT	
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		TO STUDS
< 455	< 420	SSP SINGLE BILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE BILL PLATE	2-10d		8-10d
< 885	< 760	SP4		6-10d, 1 1/2"	
< 1240	< 1065	SPH4		10-10d, 1 1/2"	
< 885	< 760	SP6		6-10d, 1 1/2"	
< 1240	< 1065	SPH6		10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
< 1350	< 1305	LTT19	8-16d		TO FOUNDATION
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2670	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, Fy = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM C 62, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 308. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WITH OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi, UNO, SUPPLIER MAY SUBMIT AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. ALL ROOFERS ARE HORIZONTAL DIAPHRAGMS; 7/16" O.S.B SHEATHING, UNBLOCKED APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH #4 COMMON NAILS (1 1/2" OOC PANEL EDGES, 12" OOC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OOC, UNO).

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICE LISTED IN THE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/64"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

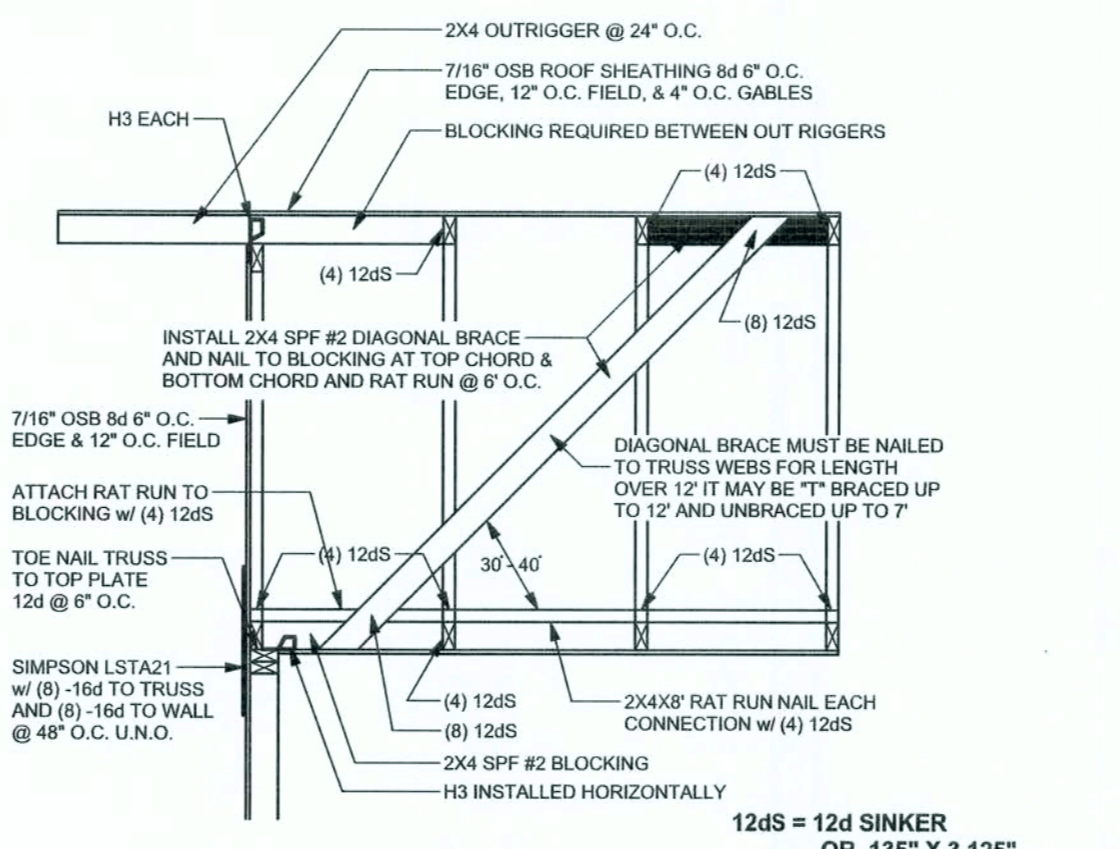
ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia., (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



SPACE RAT RUN & DIAGONAL BRACE 6" x 6" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

GABLE BRACING DETAIL SCALE: 1/2" = 1'-0"

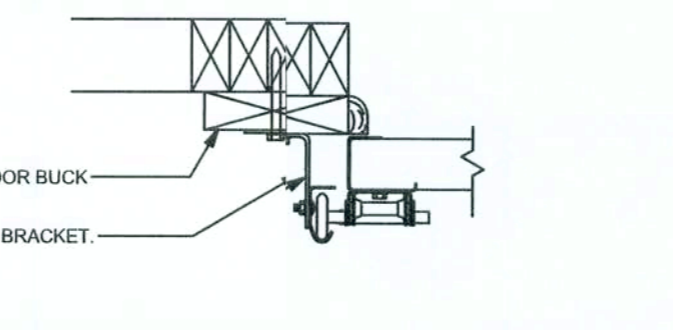
GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8 SYP #2	1200	1.6
2x10 SYP #2	1050	1.6
2x12 SYP #2	975	1.6
GLB 24F-V3 SP	2400	1.8
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	1600	1.9
PSL PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSINK, HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW.

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	18" O.C.	3" O.C.	3" O.C.

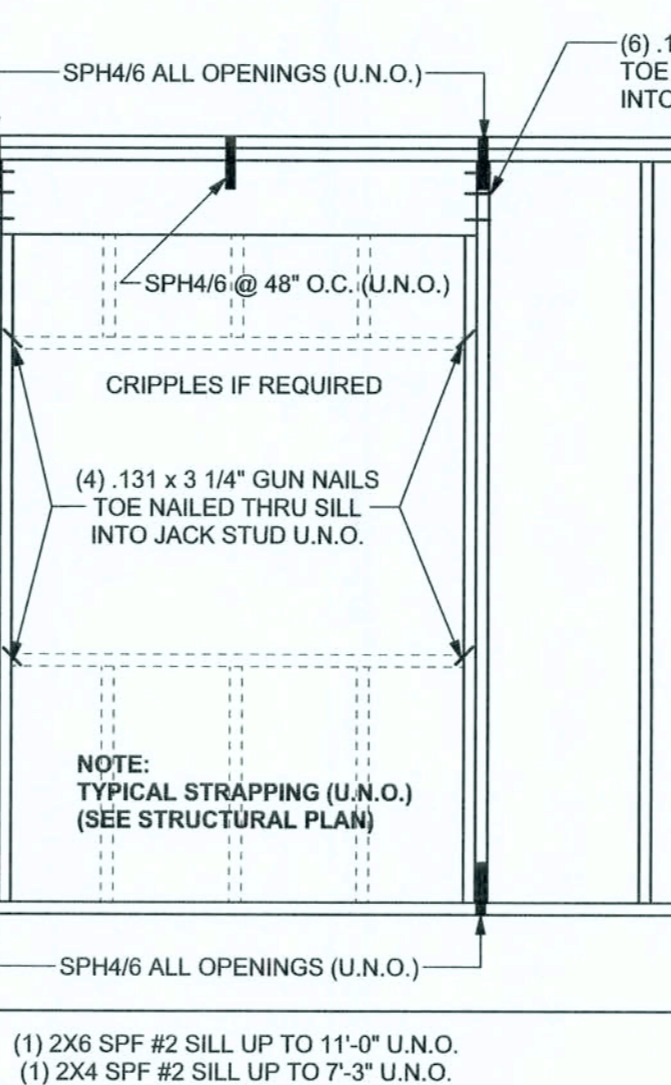


GARAGE DOOR BUCK INSTALLATION DETAIL SCALE: N.T.S.

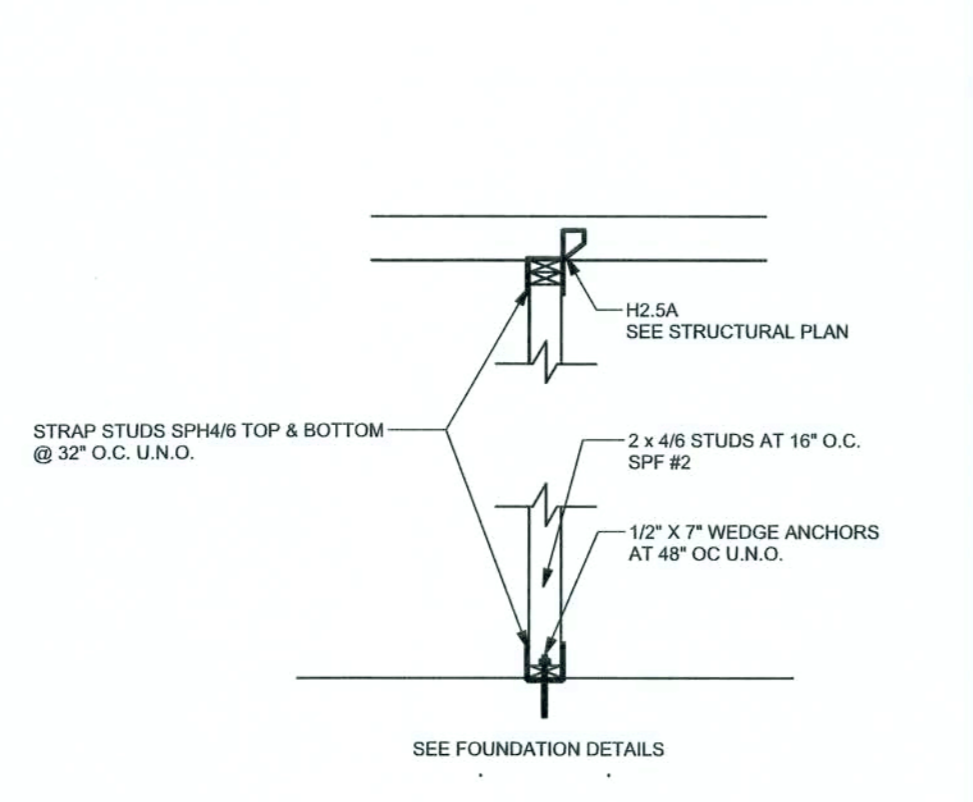
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

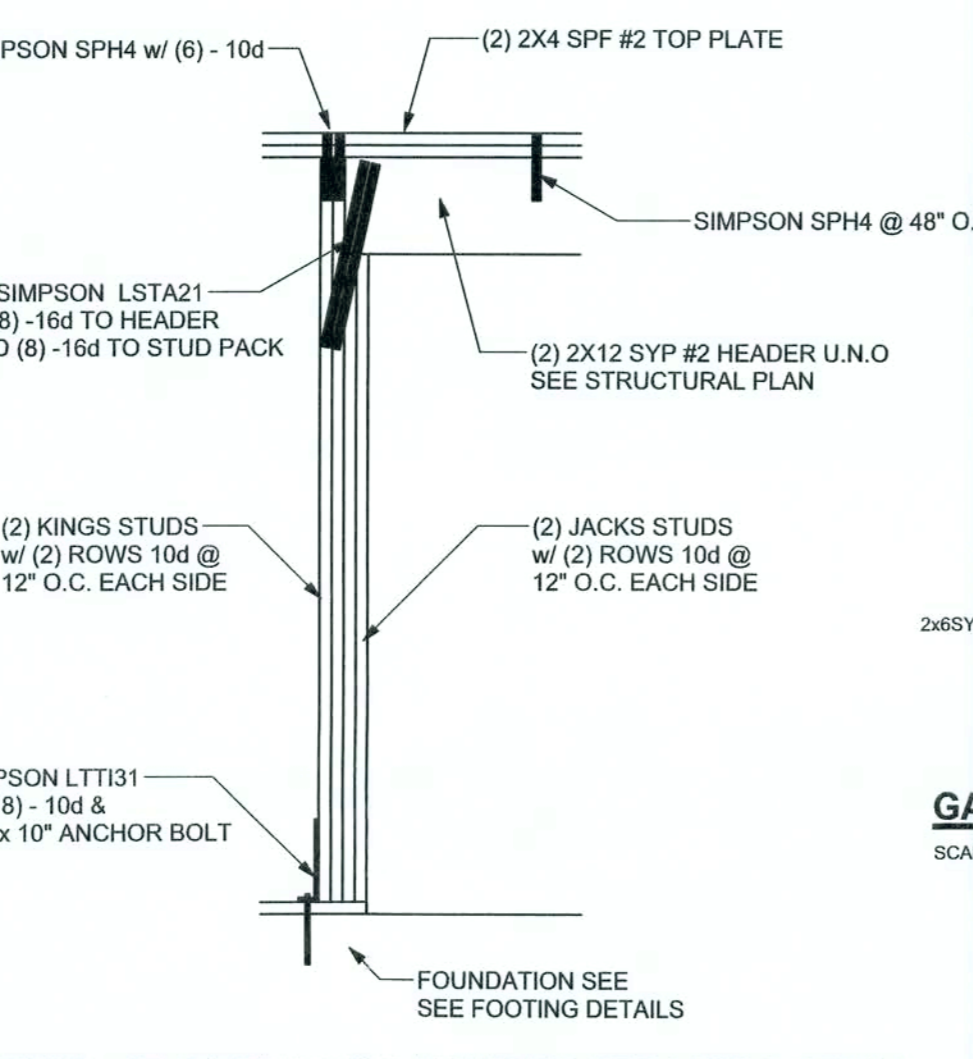
NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED



TYPICAL HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"



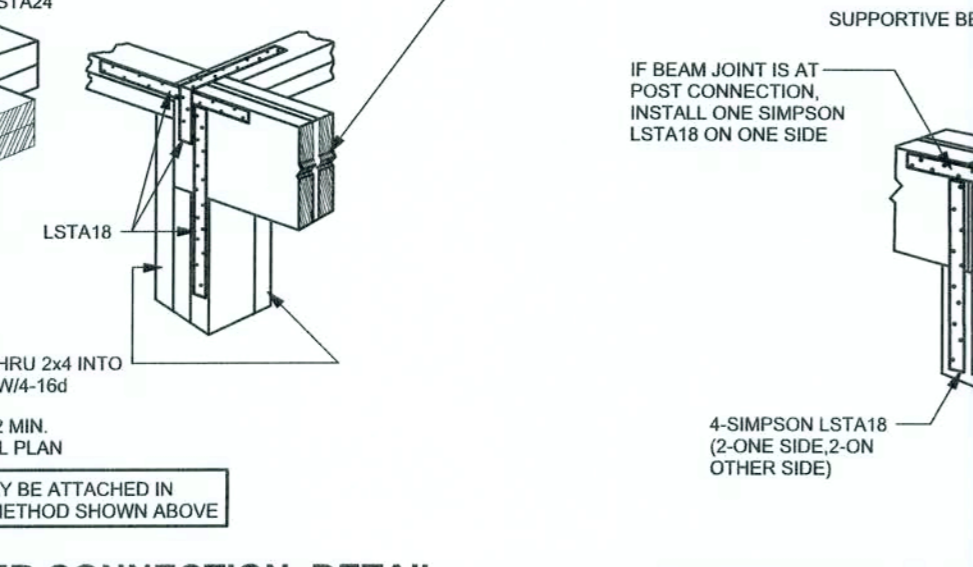
INTERIOR BEARING WALL SCALE: 1/2" = 1'-0"



TYPICAL GARAGE DOOR HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"

BEAM MID-WALL CONNECTION DETAIL

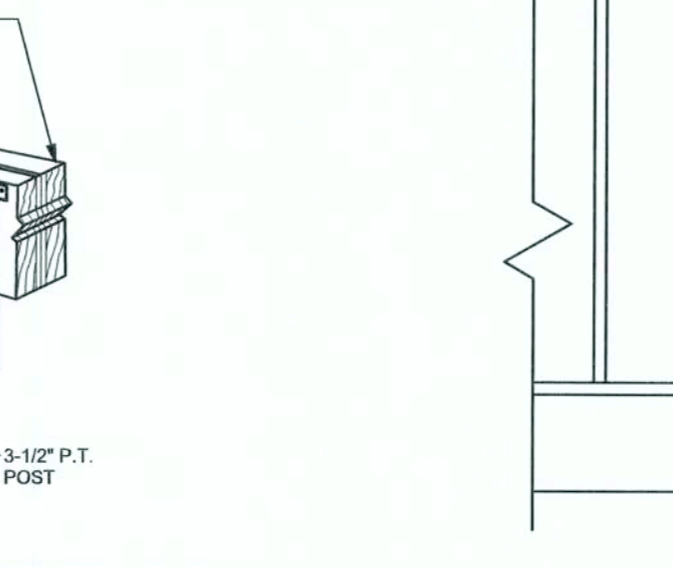
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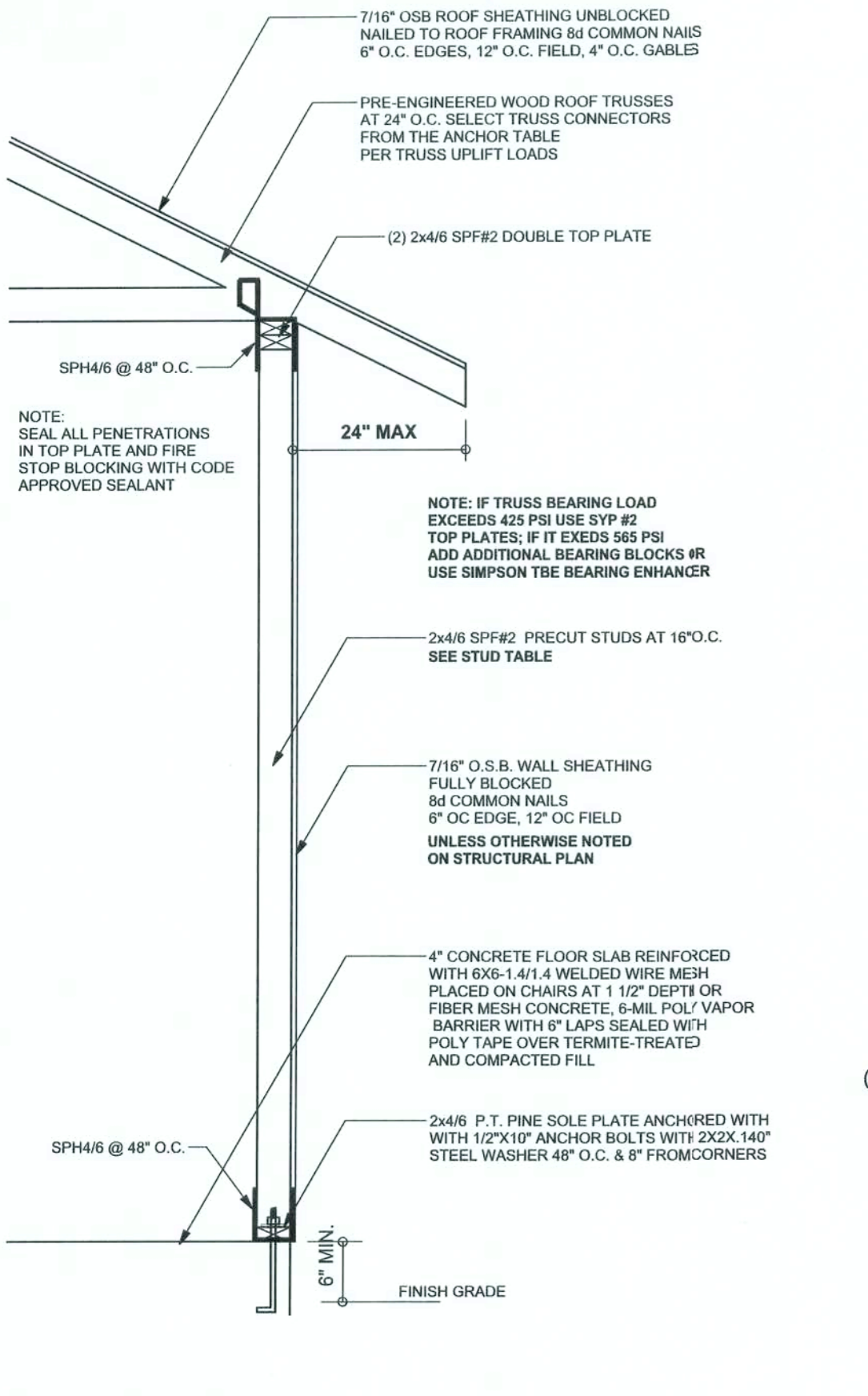
BEAM CORNER CONNECTION DETAIL SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

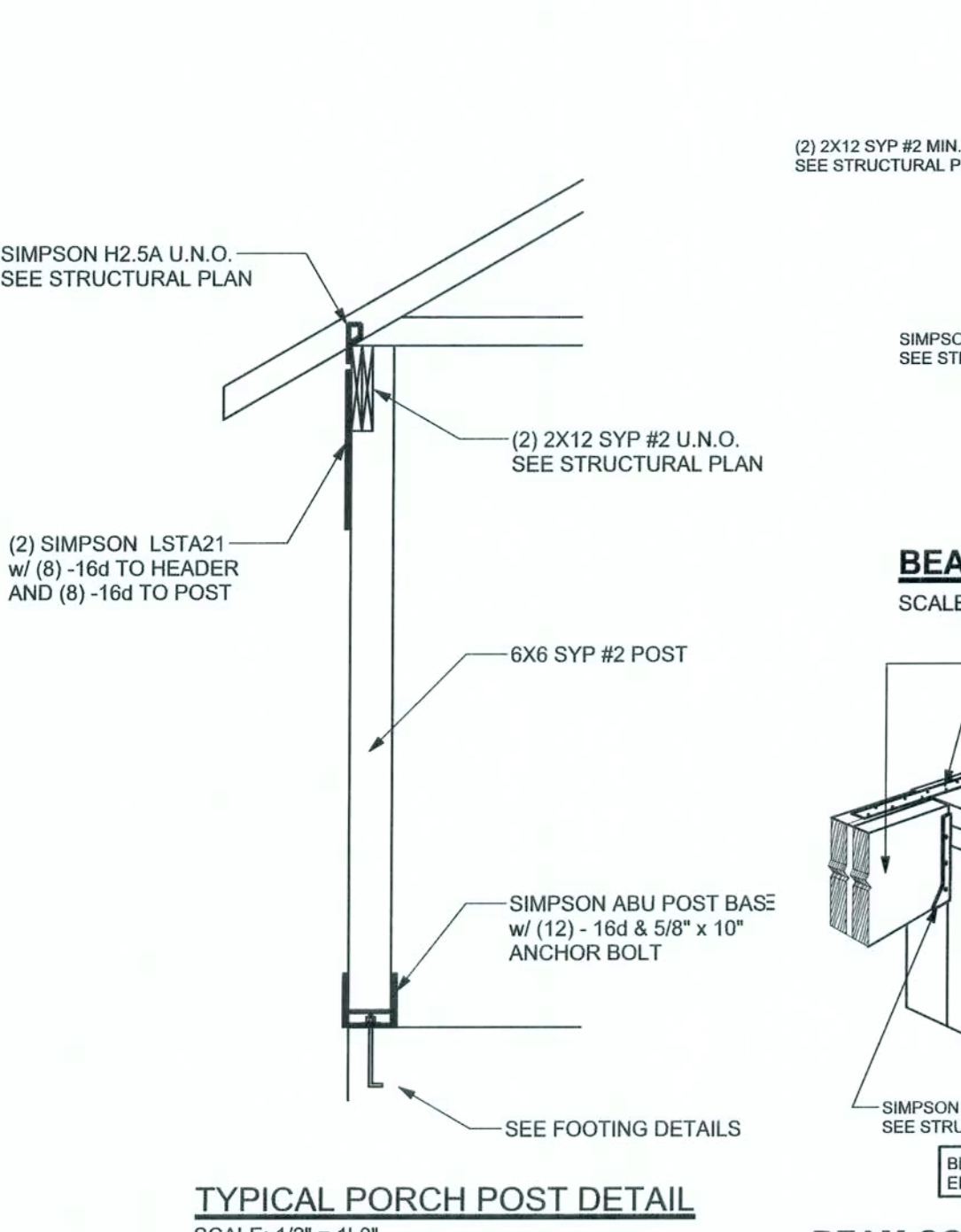


ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS: 110 MPH EXPOSURE. STUD SPACINGS SHALL BE MULTIPLIED BY 0.82 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

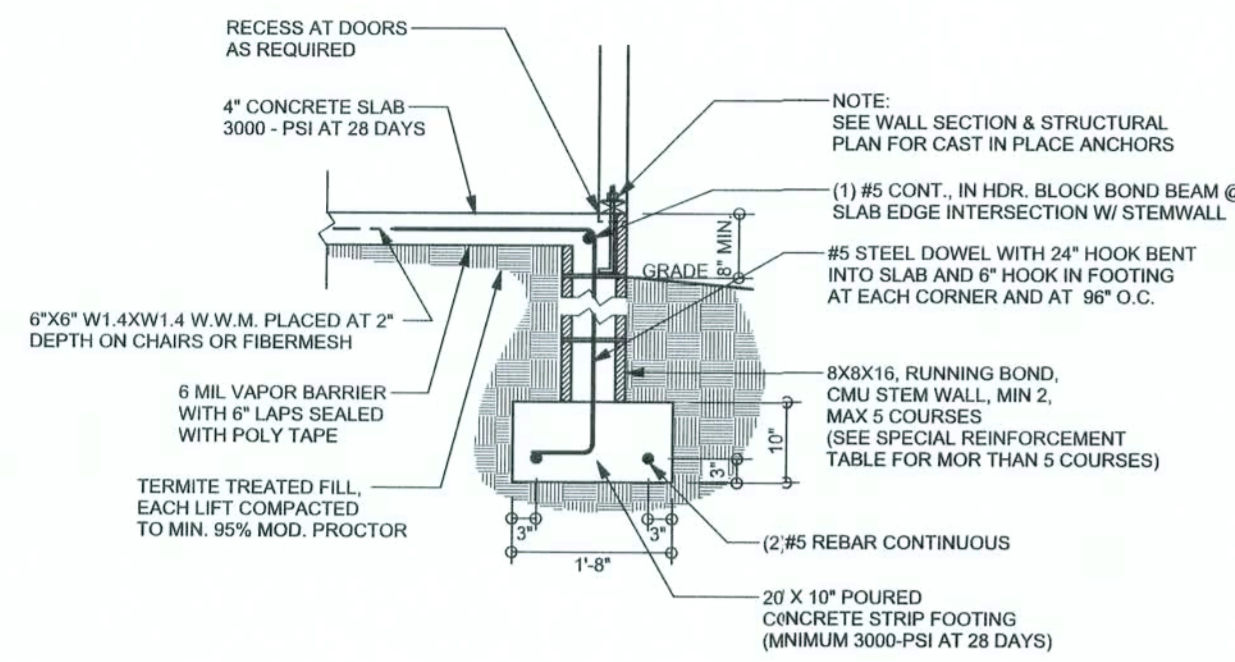


TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0"

BEAM CORNER CONNECTION DETAIL SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

REVISIONS	

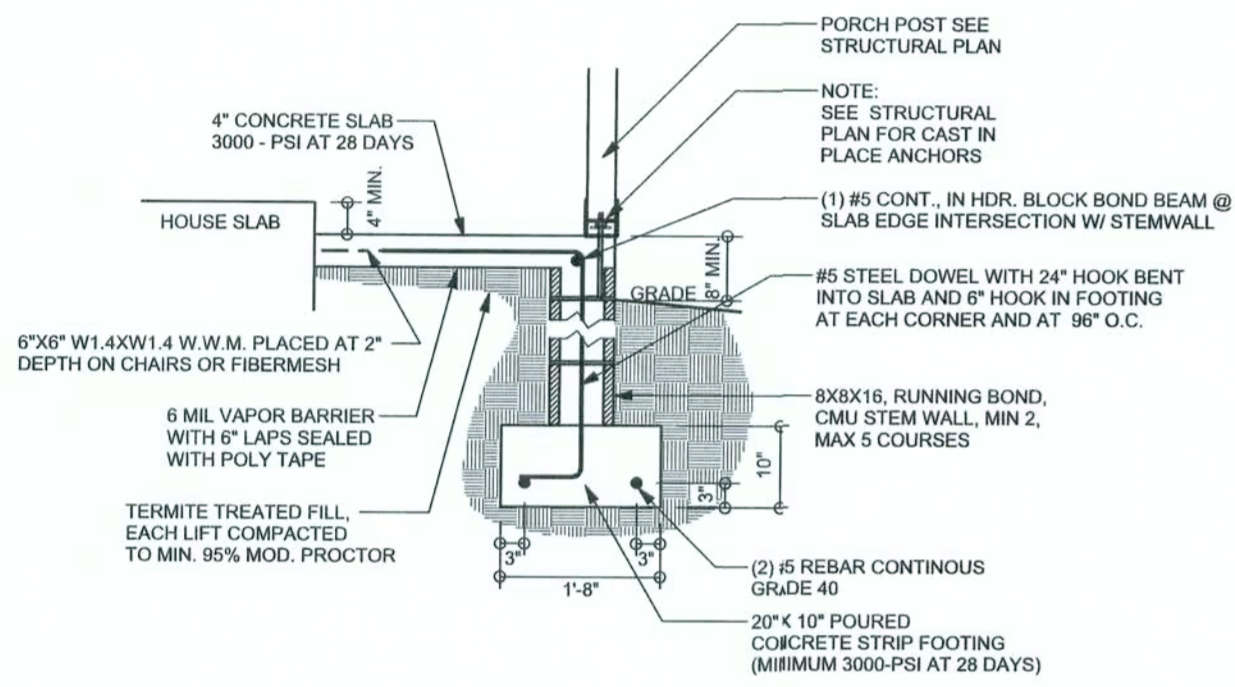


F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

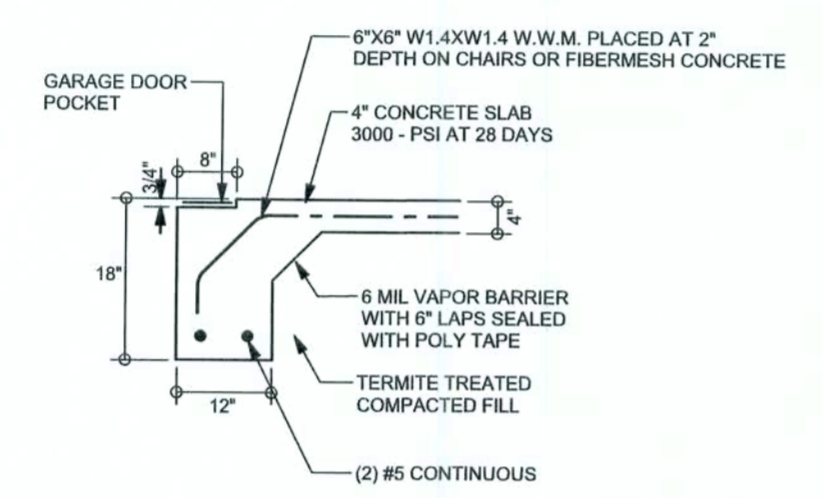
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 18C continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

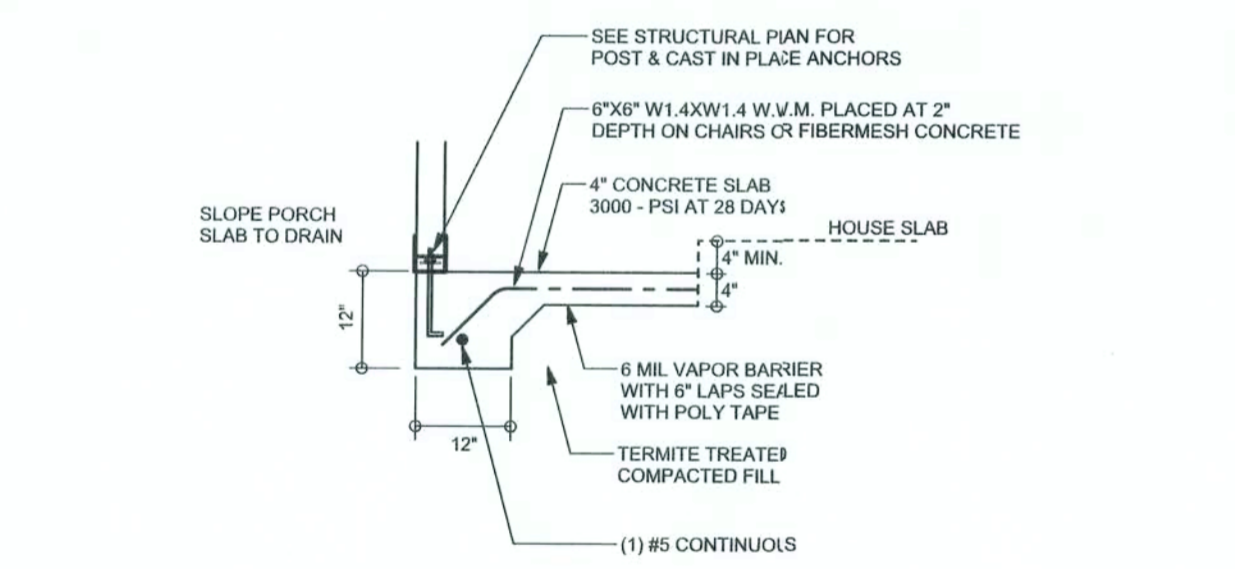
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



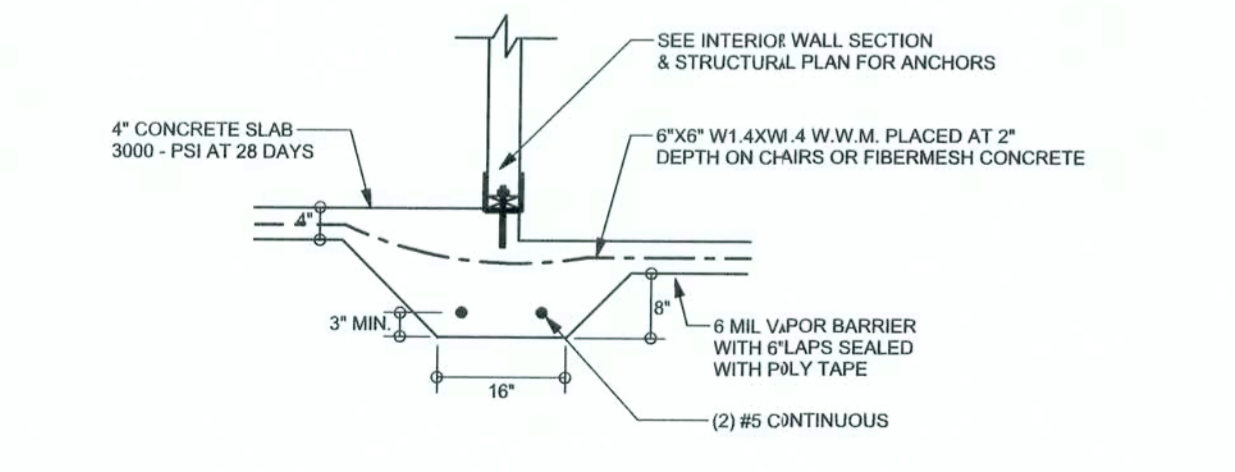
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



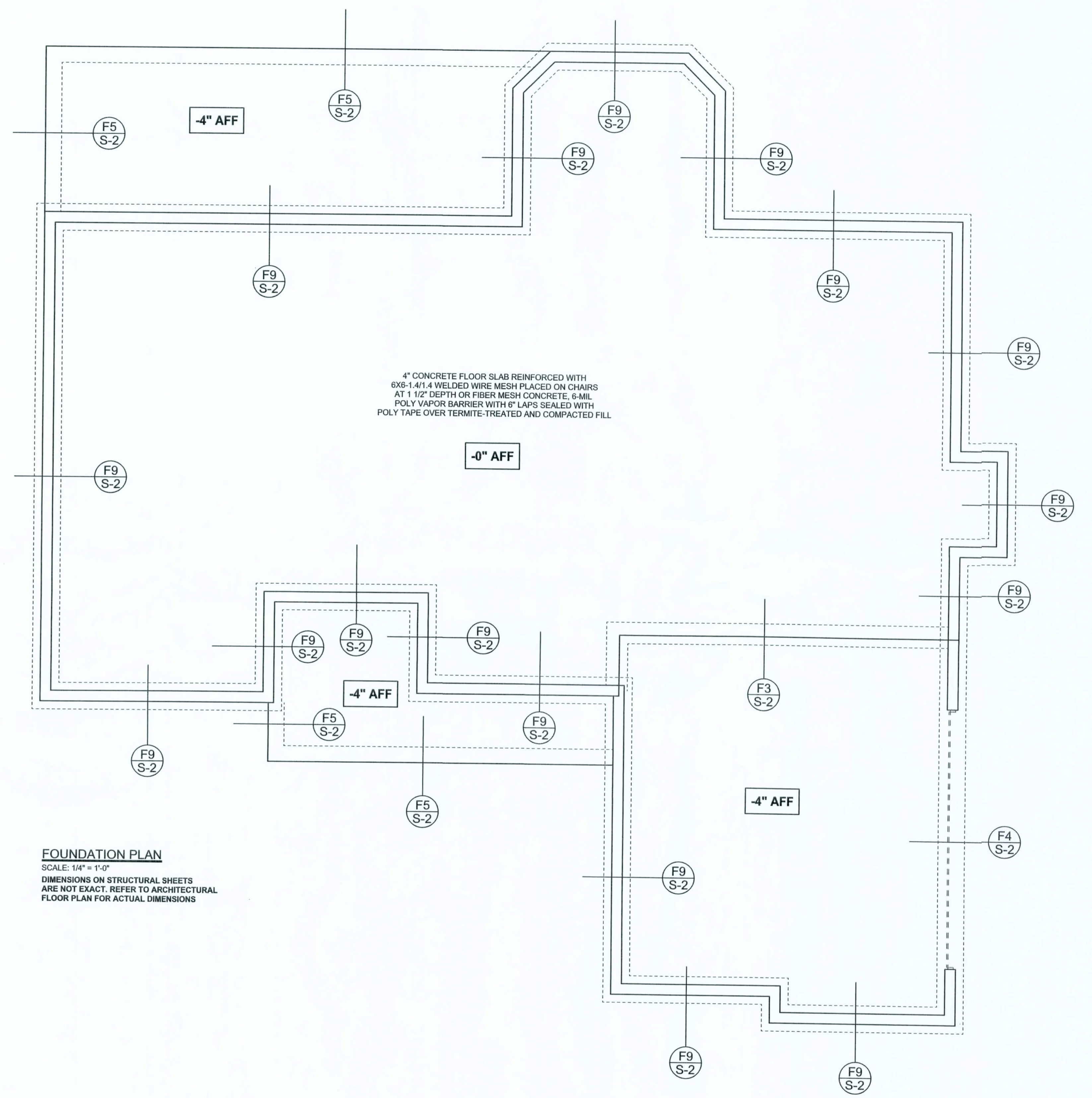
F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, PE No. 53915, PCB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F501.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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P.E. 320507
SEAL

Ewpl, Inc.

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Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
October 24, 2007

STRUCTURAL BY:

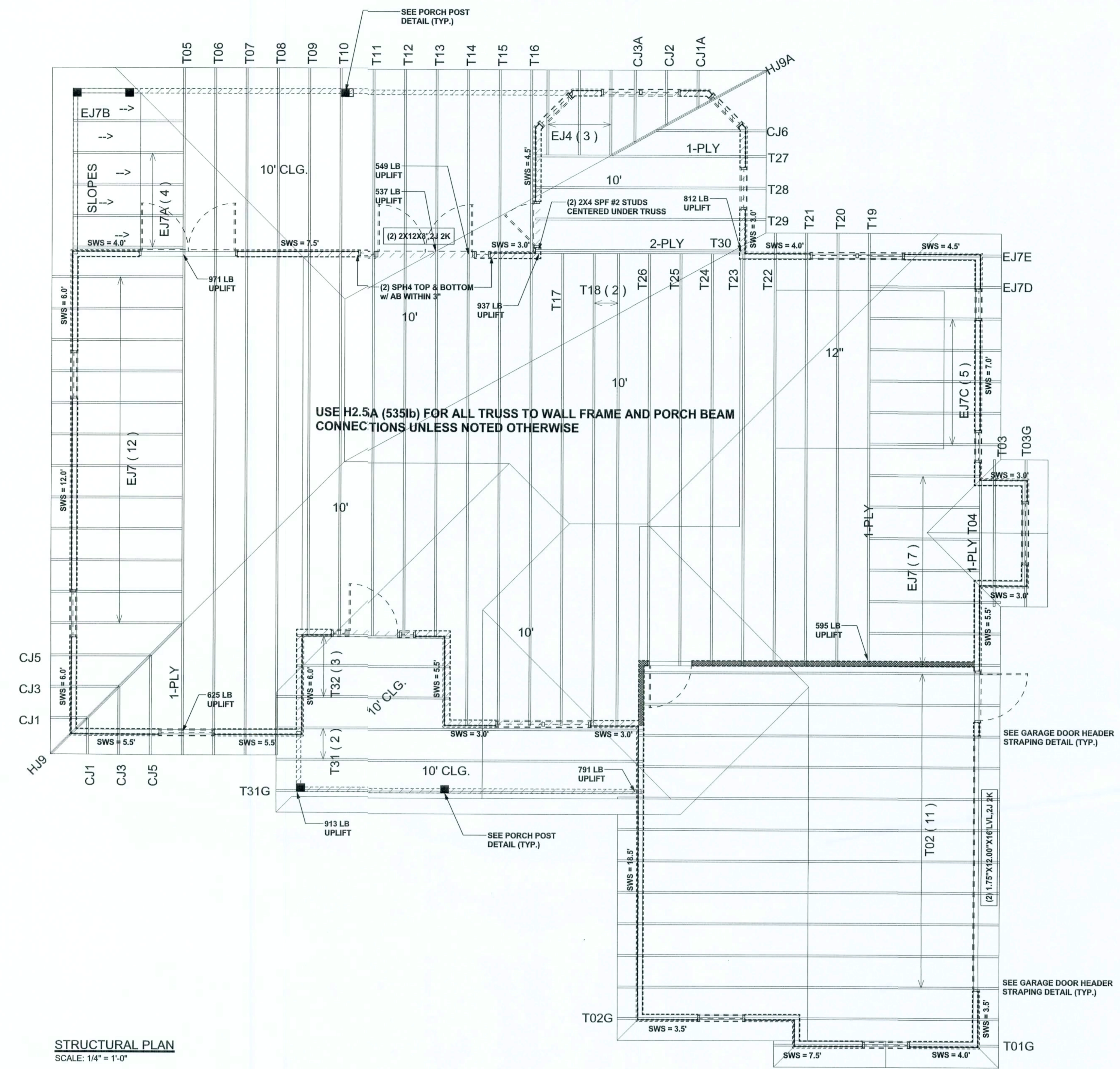
FINALS DATE:
24 / Oct / 07

JOB NUMBER:
710164

DRAWING NUMBER
S-2

OF 3 SHEETS

REVISIONS	



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

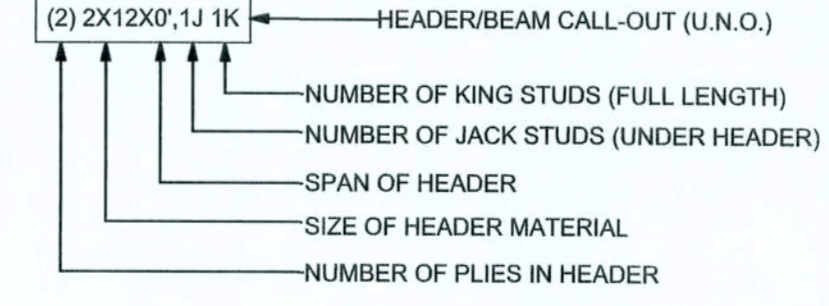
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	1ST FLOOR EXTERIOR WALL
	2ND FLOOR EXTERIOR WALL
	1ST FLOOR INTERIOR BEARING WALL
	2ND FLOOR INTERIOR BEARING WALL

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	39.3'	77.5'
LONGITUDINAL	33.6'	61.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L259252

WINDLOAD ENGINEER: Mark Disosway, PE No 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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LIMITATION: This design is valid for one building, at specified location.

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25 OCT 07
SEAL

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DRAWING NUMBER
S-3

OF 3 SHEETS