

Julius Lee Engineering

RE: 317841 - GIEBEIG HOMES - CAMPBELL RES.

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: GIEBEIG HOMES Project Name: 317841 Model: CAMPBELL RES.
 Lot/Block: 5 Subdivision: REDFIELD
 Address:
 City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: BRIAN T. GIEBEIG License #: RR282811523
 Address: 462 SW FAIRLINGTON CT
 City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
 Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
 Roof Load: 40.0 psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.
 With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
 This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4137444	CJ1	10/22/09	18	I4137461	T09	10/22/09
2	I4137445	CJ3	10/22/09	19	I4137462	T10	10/22/09
3	I4137446	CJ5	10/22/09	20	I4137463	T11	10/22/09
4	I4137447	EJ4	10/22/09	21	I4137464	T12	10/22/09
5	I4137448	EJ7	10/22/09	22	I4137465	T13	10/22/09
6	I4137449	HJ6	10/22/09	23	I4137466	T14	10/22/09
7	I4137450	HJ9	10/22/09	24	I4137467	T15	10/22/09
8	I4137451	HJ9A	10/22/09	25	I4137468	T16	10/22/09
9	I4137452	PB01	10/22/09	26	I4137469	T17	10/22/09
10	I4137453	T01	10/22/09	27	I4137470	T18	10/22/09
11	I4137454	T02	10/22/09	28	I4137471	T19	10/22/09
12	I4137455	T03	10/22/09	29	I4137472	T20	10/22/09
13	I4137456	T04	10/22/09				
14	I4137457	T05	10/22/09				
15	I4137458	T06	10/22/09				
16	I4137459	T07	10/22/09				
17	I4137460	T08	10/22/09				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

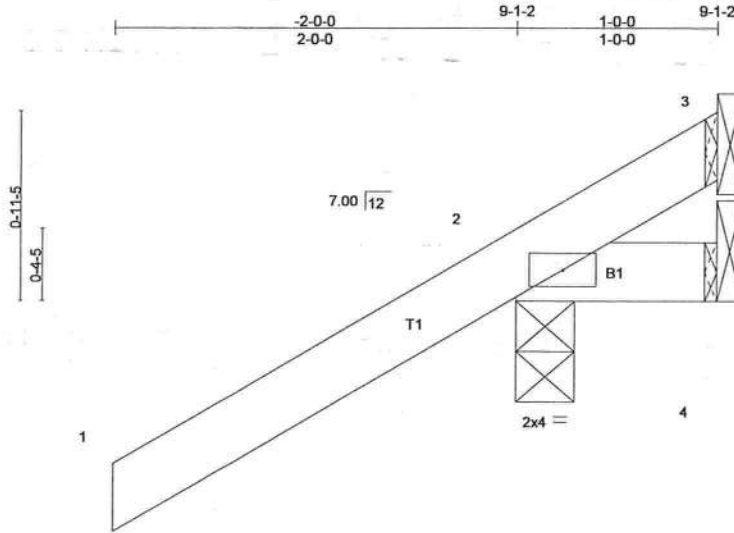
My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 317841	Truss CJ1	Truss Type JACK	Qty 14	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137444
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Builders FrstSource, Lake City, FL 32055 7,140 s Oct 1 2009 Mitek Industries, Inc. Thu Oct 22 15:31:54 2009 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) -0.00 2 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.01	Vert(TL) -0.00 2 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2 >999 240		Weight: 7 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

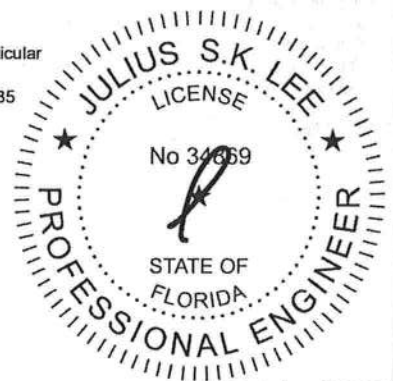
Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=289/0-3-8, 4=10/Mechanical, 3=100/Mechanical
Max Horz 2=101(LC 6)
Max Uplift 2=295(LC 6), 4=11(LC 4), 3=100(LC 1)
Max Grav 2=289(LC 1), 4=19(LC 2), 3=136(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES (8-9)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2, 11 lb uplift at joint 4 and 100 lb uplift at joint 3.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

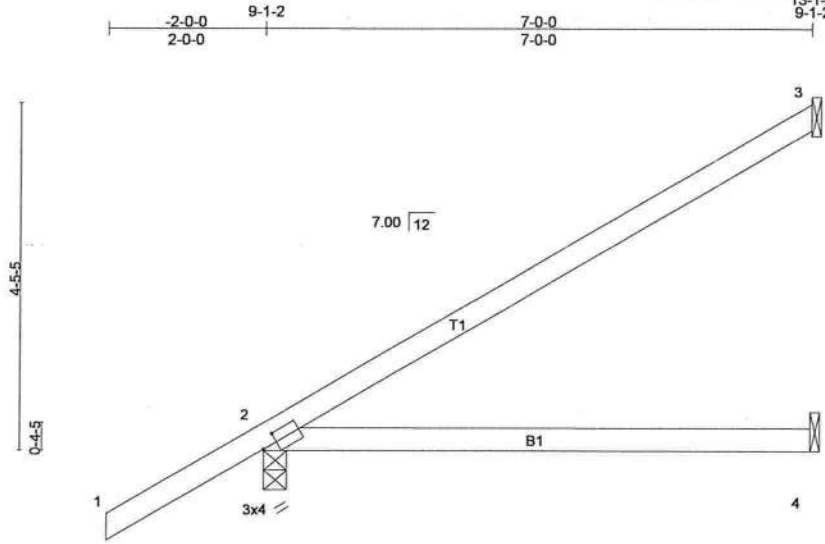
LOAD CASE(S) Standard



October 22, 2009

Job 317841	Truss EJ7	Truss Type MONO TRUSS	Qty 29	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137448
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Scale = 1:28.4

Plate Offsets (X,Y): [2-0-2-4,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.47	Vert(LL)	-0.09	2-4	>918	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.44	Vert(TL)	-0.25	2-4	>331	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.32	2-4	>258	240		Weight: 26 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

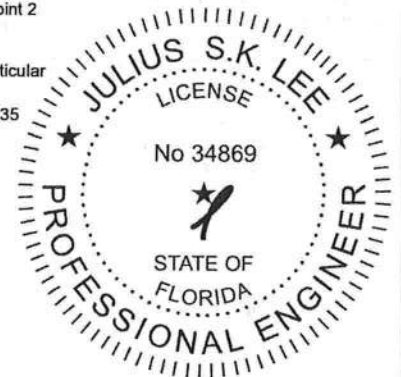
REACTIONS (lb/size) 3=177/Mechanical, 2=421/0-3-8, 4=74/Mechanical
Max Horz 2=188(LC 6)
Max Uplift 3=-102(LC 6), 2=-215(LC 6), 4=-67(LC 4)
Max Grav 3=177(LC 1), 2=421(LC 1), 4=128(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 3, 215 lb uplift at joint 2 and 67 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss HL9	Truss Type MONO TRUSS	Qty 4	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137450
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:31:57 2009 Page 1 13-1-11 9-1-2

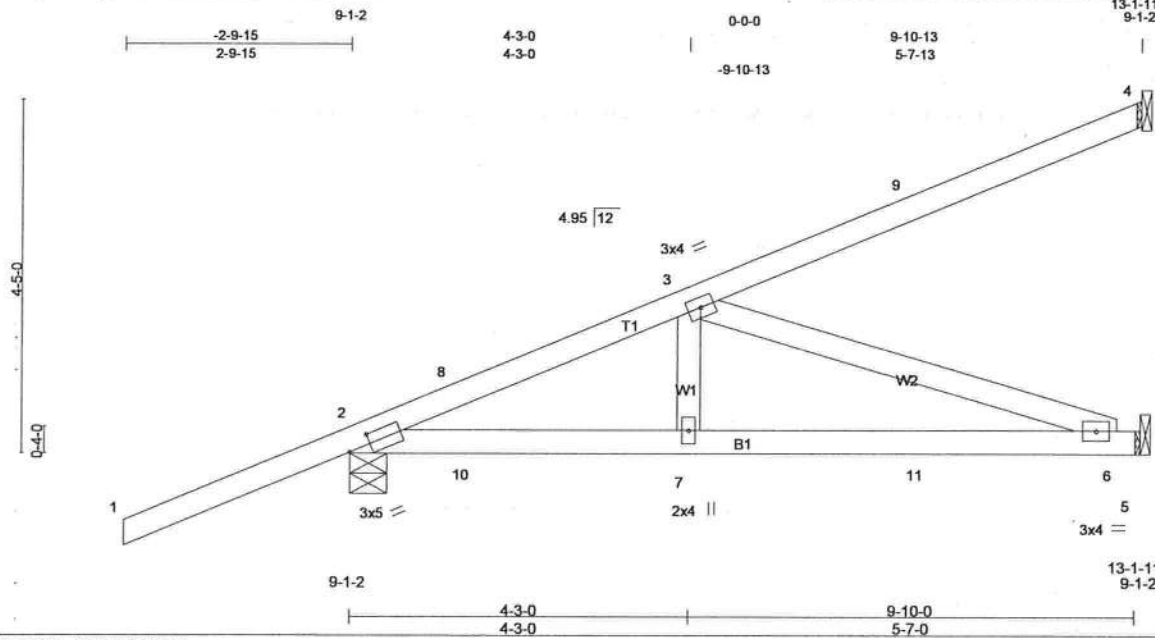


Plate Offsets (X,Y): [2:0-3-5,0-1-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.63	Vert(LL) -0.07 6-7 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.62	Vert(TL) -0.19 6-7 >606 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.34	Horz(TL) -0.01 5 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.13 6-7 >874 240		Weight: 46 lb

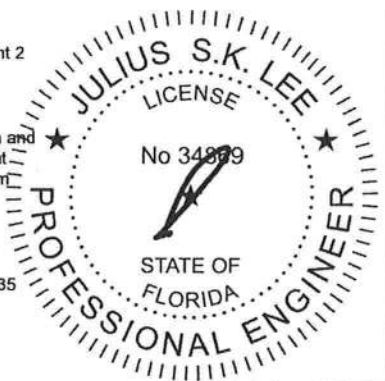
LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-4-12 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=193/Mechanical, 2=512/0-5-11, 5=250/Mechanical
Max Horz 2=262(LC 5)
Max Uplift 4=158(LC 5), 2=524(LC 5), 5=274(LC 6)
Max Grav 4=193(LC 1), 2=512(LC 1), 5=302(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=619/570, 3-8=632/564
BOT CHORD 2-10=620/550, 7-10=620/550, 7-11=620/550, 6-11=620/550
WEBS 3-7=196/330, 3-6=580/654

- NOTES (10-11)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 4, 524 lb uplift at joint 2 and 274 lb uplift at joint 5.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 53 lb up at 1-5-12, 53 lb up at 1-5-12, 7 lb down and 26 lb up at 4-3-11, 7 lb down and 26 lb up at 4-3-11, and 55 lb down and 84 lb up at 7-1-10, and 55 lb down and 84 lb up at 7-1-10 on top chord, and 8 lb down and 21 lb up at 1-5-12, 8 lb down and 21 lb up at 1-5-12, 16 lb down and 14 lb up at 4-3-11, 16 lb down and 14 lb up at 4-3-11, and 56 lb down and 37 lb up at 7-1-10, and 56 lb down and 37 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer. Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss HJ9A	Truss Type MONO TRUSS	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137451
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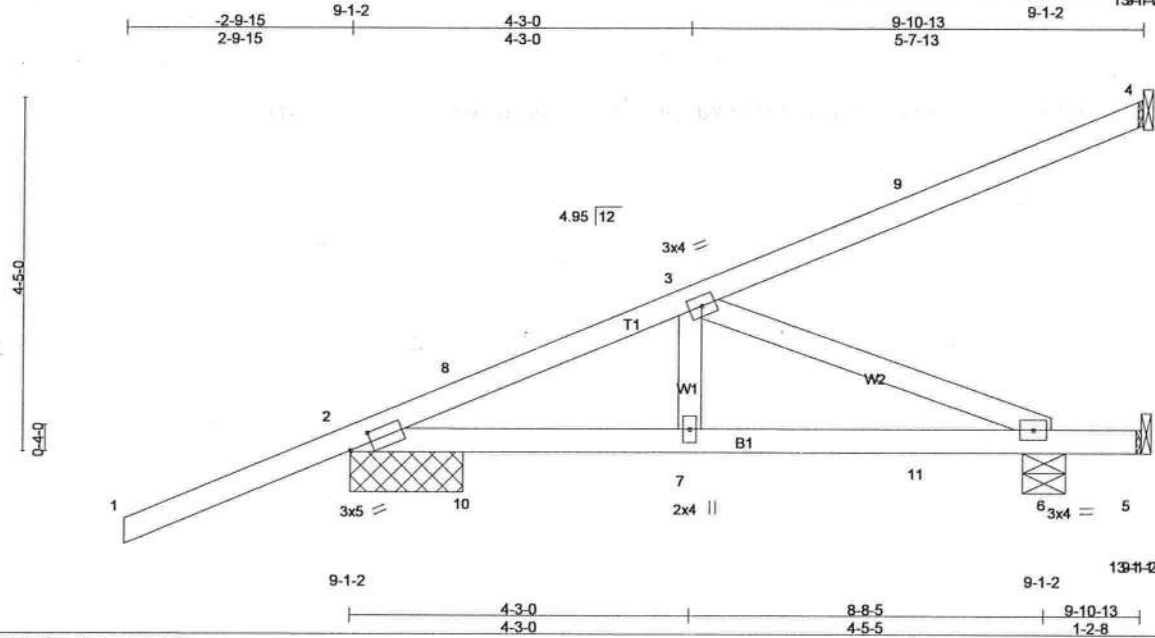


Plate Offsets (X,Y): [2:0-3-5,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase	1.25	TC 0.62	Vert(LL)	-0.02	6-7	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.30	Vert(TL)	-0.05	6-7	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.22	Horz(TL)	-0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.04	6-7	>999	240		Weight: 45 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-7-8 oc bracing.

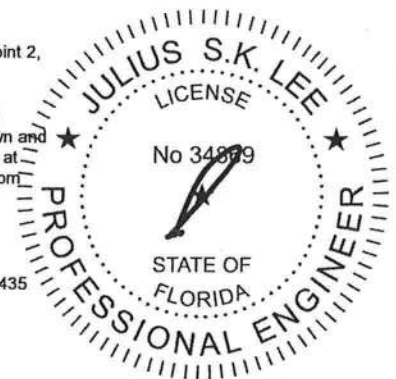
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings Mechanical except (jt=length) 2=1-5-0, 6=0-6-7.
 (lb) - Max Horz 2=262(LC 5)
 Max Uplift All uplift 100 lb or less at joint(s) except 4=155(LC 5), 2=467(LC 5), 5=115(LC 2), 6=399(LC 5)
 Max Grav All reactions 250 lb or less at joint(s) 4, 5 except 2=463(LC 1), 6=478(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-8=518/448, 3-8=471/440
 BOT CHORD 2-10=514/463, 7-10=514/463, 7-11=514/463, 6-11=514/463
 WEBS 3-6=498/552

- NOTES** (10-11)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; TCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 4, 467 lb uplift at joint 2, 115 lb uplift at joint 5 and 399 lb uplift at joint 6.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 53 lb up at 1-5-12, 53 lb up at 1-5-12, 7 lb down and 26 lb up at 4-3-11, 7 lb down and 26 lb up at 4-3-11, and 55 lb down and 84 lb up at 7-1-10, and 55 lb down and 84 lb up at 7-1-10 on top chord, and 8 lb down and 21 lb up at 1-5-12, 8 lb down and 21 lb up at 1-5-12, 16 lb down and 14 lb up at 4-3-11, 16 lb down and 14 lb up at 4-3-11, and 56 lb down and 37 lb up at 7-1-10, and 56 lb down and 37 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25



October 22, 2009

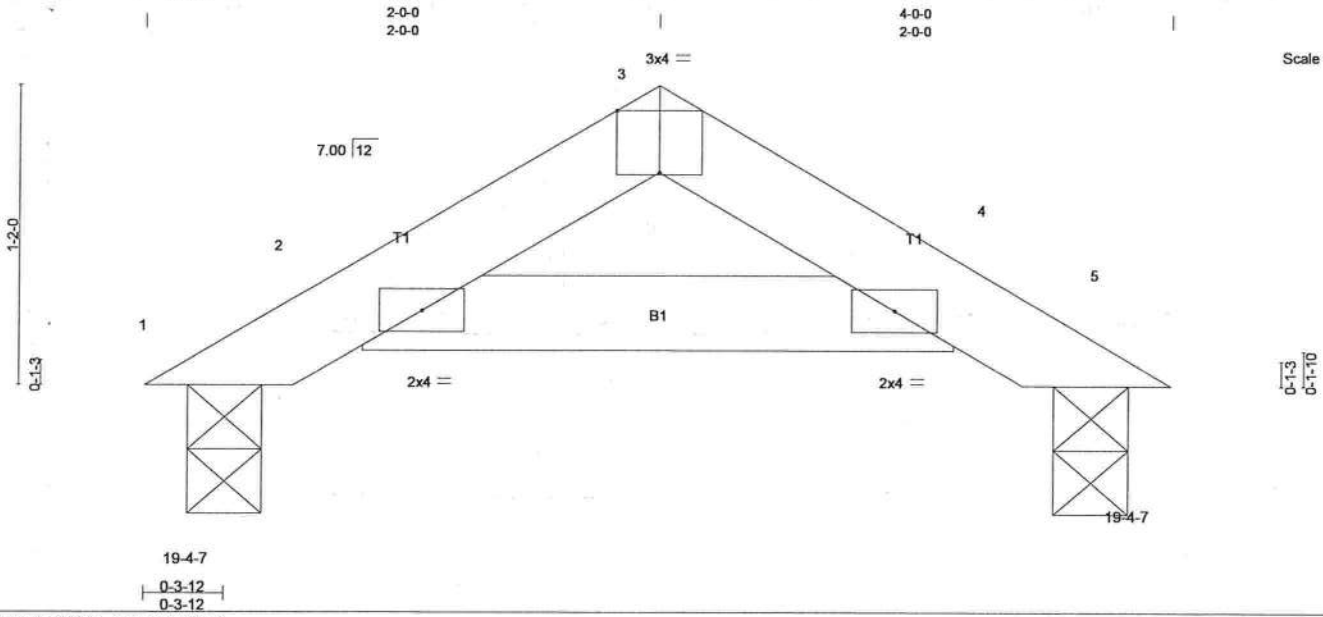
Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss PB01	Truss Type PIGGYBACK	Qty 13	Ply 1	GIEBEIG HOMES - CAMPBELL RES. 14137452
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Builders FrstSource, Lake City, FL 32055
19-4-7
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:31:58 2009 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.15	Vert(LL) -0.01 2-4 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.08	Vert(TL) -0.02 2-4 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.01 5 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.01 2-4 >999 240		Weight: 10 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=150/0-3-8, 5=150/0-3-8
Max Horz 1=30(LC 4)
Max Uplift 1=25(LC 6), 5=25(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (10-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1 and 25 lb uplift at joint 5.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

Job 317841	Truss T01	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137453
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Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:32:00 2009 Page 2

- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

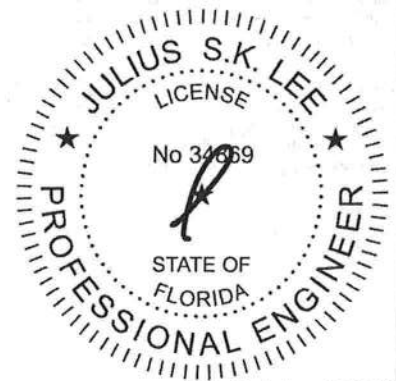
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=60, 3-6=70(F=10), 6-8=60, 2-7=20

Concentrated Loads (lb)

Vert: 3=56(F) 6=56(F) 12=70(F) 9=70(F) 23=37(F) 24=37(F) 25=37(F) 26=37(F) 27=37(F) 28=37(F) 29=23(F) 30=23(F) 31=23(F) 32=23(F) 33=23(F) 34=23(F)



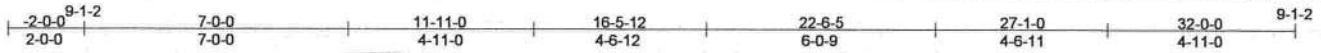
October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T03	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137455
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:02 2009 Page 1



Scale = 1:58.7

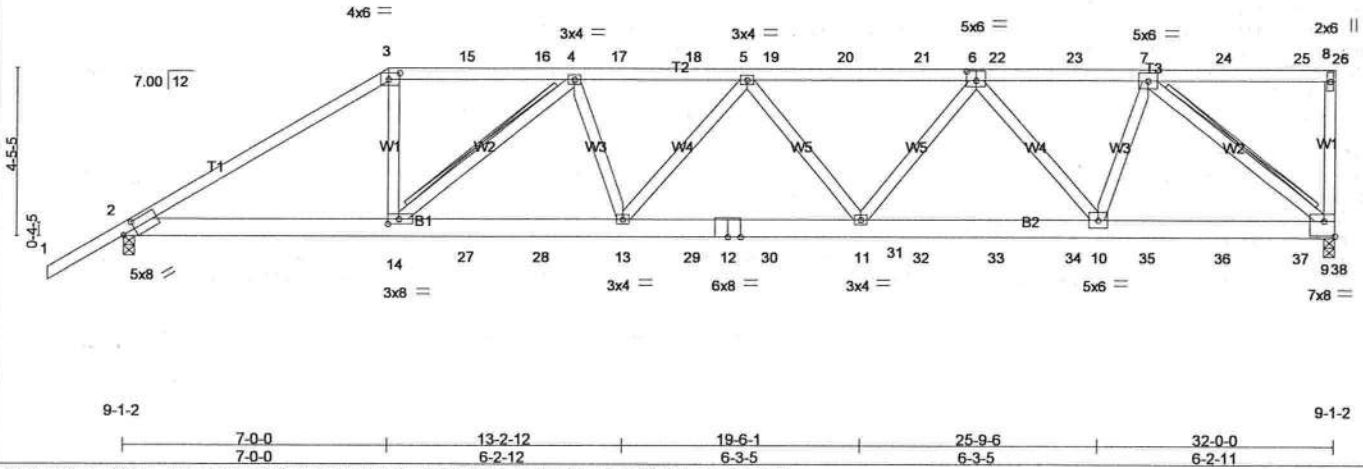


Plate Offsets (X,Y): [2:0-3-15,0-2-8], [3:0-3-8,0-2-0], [6:0-3-0,0-3-0], [9:Edge,0-4-12], [14:0-3-8,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.92	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.62	Vert(LL) -0.20 11-13 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.92	Vert(TL) -0.54 11-13 >706 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.13 9 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.33 11-13 >999 240		Weight: 197 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

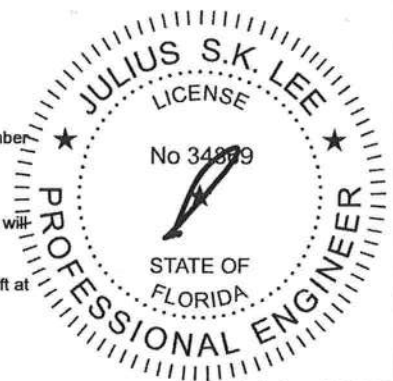
BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=2841/0-3-8, 2=2585/0-3-8
Max Horz 2=191(LC 5)
Max Uplift 9=1706(LC 3), 2=1444(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4405/2587, 3-15=-3750/2291, 15-16=-3749/2291, 4-16=-3749/2291, 4-17=-5113/3041, 17-18=-5113/3041, 5-18=-5113/3041, 5-19=-4925/2884, 19-20=-4925/2884, 20-21=-4925/2884, 6-21=-4925/2884, 6-22=-3255/1907, 22-23=-3255/1907, 7-23=-3255/1907, 8-9=-374/216
BOT CHORD 2-14=-2258/3695, 14-27=-2952/4955, 27-28=-2952/4955, 13-28=-2952/4955, 13-29=-3092/5266, 12-29=-3092/5266, 12-30=-3092/5266, 30-31=-3092/5266, 11-31=-3092/5266, 11-32=-2548/4367, 32-33=-2548/4367, 33-34=-2548/4367, 10-34=-2548/4367, 10-35=-1582/2709, 35-36=-1582/2709, 36-37=-1582/2709, 37-38=-1582/2709, 9-38=-1582/2709
WEBS 3-14=-977/1519, 4-14=-1585/972, 4-13=-326/586, 5-11=-571/349, 6-11=-579/936, 6-10=-1781/1030, 7-10=-1062/1784, 7-9=-3508/2049

- NOTES (11-12)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) All bearings are assumed to be SYP No.2.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1706 lb uplift at joint 9 and 1444 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



October 22, 2009

Continued on page 2

Job 317841	Truss T04	Truss Type MOND HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. I4137456
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Builders FratSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:02 2009 Page 1

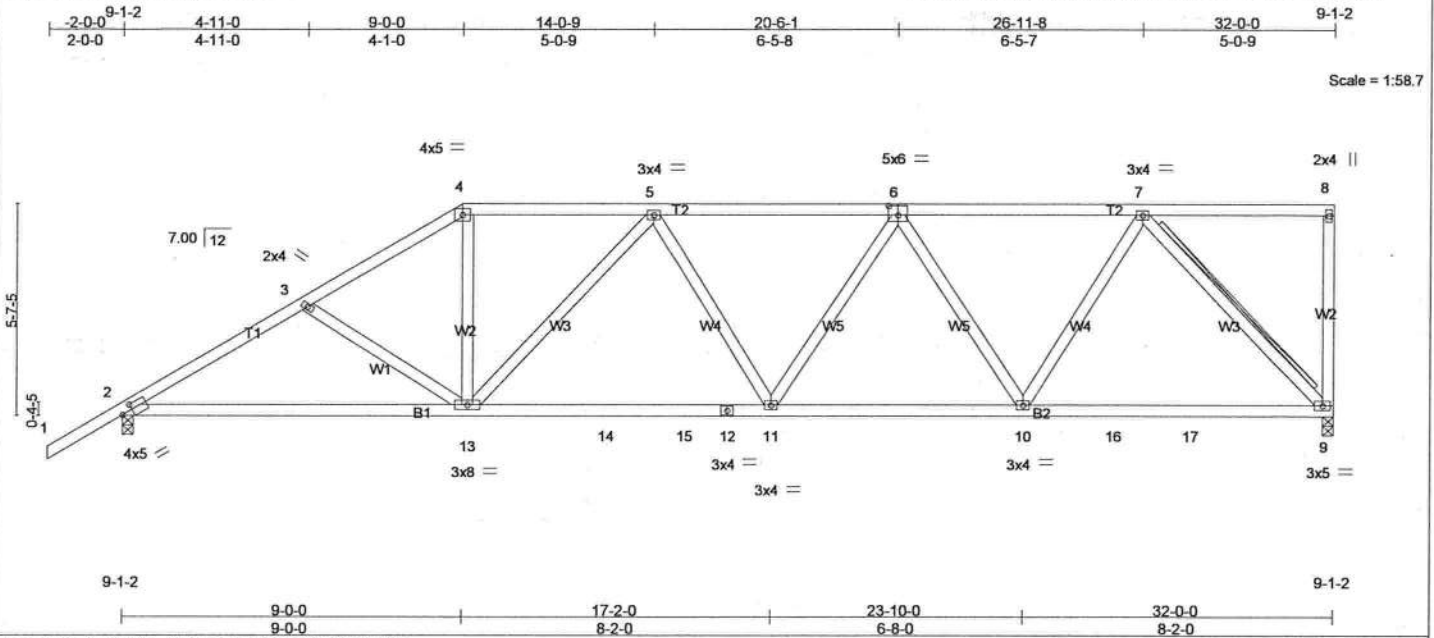


Plate Offsets (X,Y): [2:0-3-8,0-1-12], [6:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL) -0.19 11-13 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.63	Vert(TL) -0.42 2-13 >912 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.51	Horz(TL) 0.09 9 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.10 11-13 >999 240		Weight: 179 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-11-4 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.

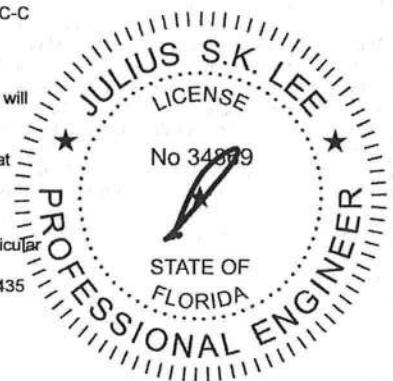
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=1368/0-3-8, 2=1459/0-3-8
 Max Horz 2=227(LC 6)
 Max Uplift 9=320(LC 4), 2=264(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=2213/777, 3-4=1971/715, 4-5=1662/667, 5-6=2001/779, 6-7=1535/580
 BOT CHORD 2-13=813/1826, 13-14=819/1998, 14-15=819/1998, 12-15=819/1998, 11-12=819/1998,
 10-11=760/1893, 10-16=440/1076, 16-17=440/1076, 9-17=440/1076
 WEBS 4-13=201/712, 5-13=496/249, 6-10=674/340, 7-10=276/907, 7-9=1558/647

- NOTES** (9-10)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 5) All bearings are assumed to be SYP No.2.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 320 lb uplift at joint 9 and 264 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T08	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137460
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:06 2009 Page 1

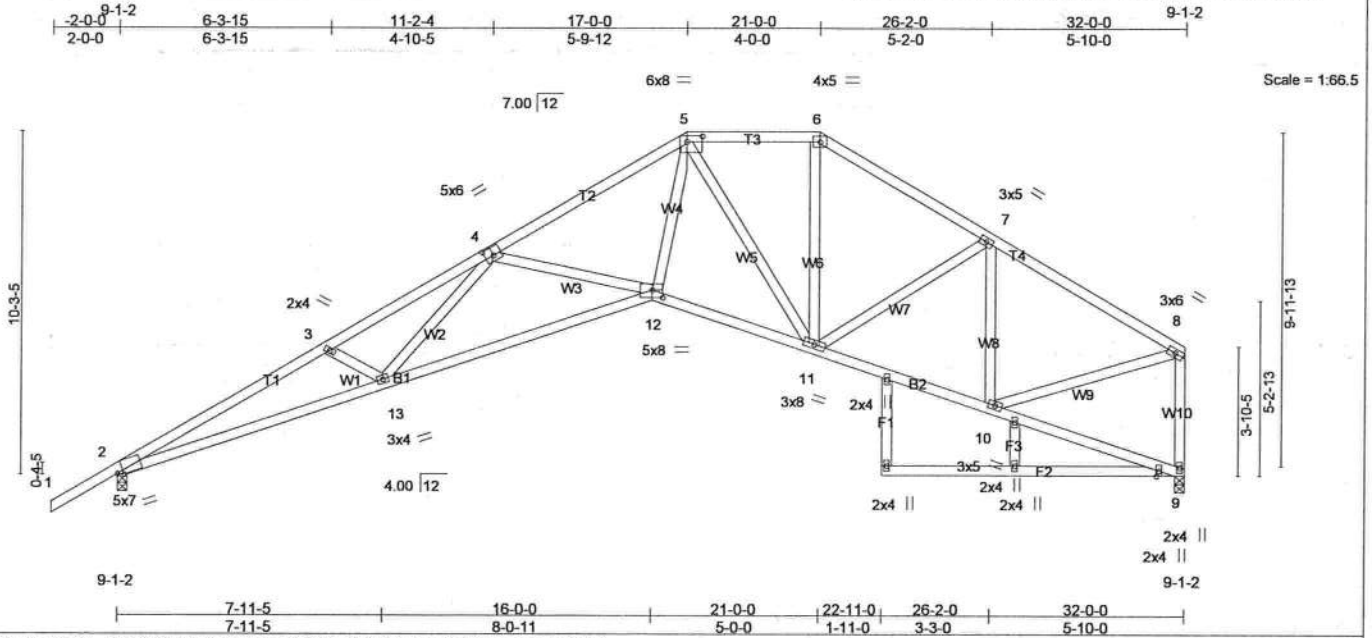


Plate Offsets (X,Y): [2:0-2-2,Edge], [4:0-3-0,0-3-0], [5:0-5-8,0-2-0], [12:0-3-12,0-2-12]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.54	Vert(LL) -0.29 12-13 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.81	Vert(TL) -0.85 12-13 >447 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 1.00	Horz(TL) 0.57 9 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.34 12-13 >999 240		Weight: 201 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-6-15 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-0-5 oc bracing. Except:
WEBS 2 X 4 SYP No.3 *Except* W10: 2 X 4 SYP No.2	9-6-0 oc bracing: 9-11

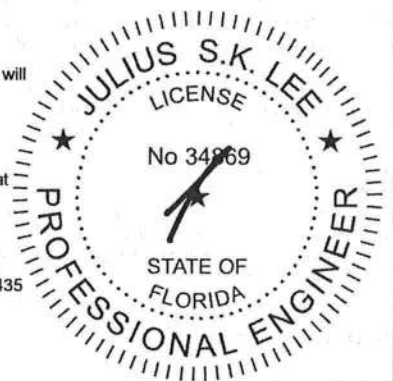
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1401/0-3-8, 9=1264/0-3-8
Max Horz 2=294(LC 5)
Max Uplift 2=-311(LC 6), 9=-187(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4374/1642, 3-4=-4093/1568, 4-5=-2882/1102, 5-6=-1442/686, 6-7=-1745/735,
7-8=-1469/568, 8-9=-1219/491
BOT CHORD 2-13=-1546/3872, 12-13=-1240/3297, 11-12=-626/2101, 10-11=-445/1285
WEBS 4-13=-187/670, 4-12=-731/451, 5-12=-688/2054, 5-11=-1036/347, 6-11=-219/589,
7-11=-137/330, 7-10=-646/301, 8-10=-423/1235

- NOTES** (10-11)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) All bearings are assumed to be SYP No.2.
 - 7) Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 187 lb uplift at joint 9.
 - 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

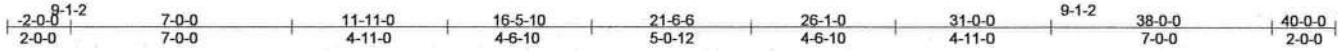
Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss T10	Truss Type HIP	Qty 1	Ply 1	GIEBIG HOMES - CAMPBELL RES.	14137462
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Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:32:09 2009 Page 1

Job Reference (optional)



Scale = 1:70.2

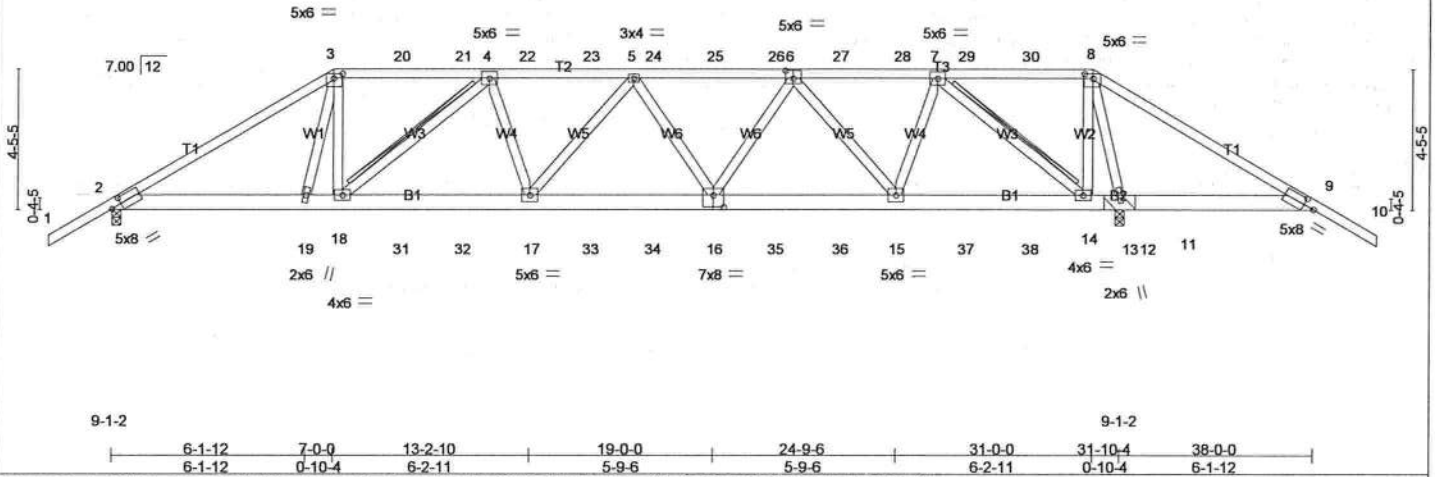


Plate Offsets (X,Y): [2:0-3-15,0-2-8], [3:0-3-4,0-1-12], [6:0-3-0,0-3-0], [8:0-3-4,0-1-12], [9:0-3-15,0-2-8], [16:0-4-0,0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.90	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.61	Vert(LL) -0.21 16-17 >999 360		
BCLL 0.0	Rep Stress Incr NO	WB 0.88	Vert(TL) -0.53 16-17 >714 240		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.13 12 n/a n/a		
			Wind(LL) 0.34 16-17 >999 240	Weight: 238 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied or 4-9-2 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 4-18, 7-14
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

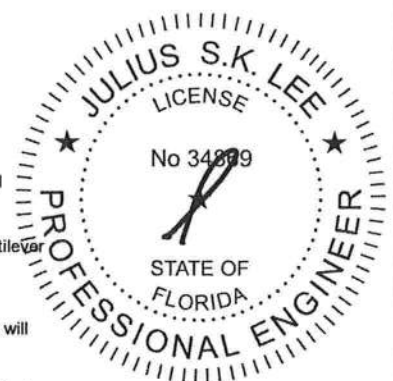
REACTIONS (lb/size) 2=2519/0-3-8, 12=3396/0-4-0 (0-3-8 + bearing block)
 Max Horz 2=111(LC 3)
 Max Uplift 2=1453(LC 5), 12=1997(LC 6)
 Max Grav 2=2561(LC 9), 12=3396(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=4369/2630, 3-20=3703/2354, 20-21=3703/2354, 4-21=3702/2353,
 4-22=5033/3105, 22-23=5033/3105, 5-23=5033/3105, 5-24=4896/2996,
 24-25=4896/2996, 25-26=4896/2996, 6-26=4896/2996, 6-27=3478/2140,
 27-28=3478/2140, 7-28=3478/2140, 7-29=440/404, 29-30=441/404, 8-30=442/404,
 8-9=374/708
BOT CHORD 2-19=2282/3663, 18-19=2297/3650, 18-31=3004/4890, 31-32=3004/4890,
 17-32=3004/4890, 17-33=3136/5156, 33-34=3136/5156, 16-34=3136/5156,
 16-35=2710/4474, 35-36=2710/4474, 15-36=2710/4474, 15-37=1814/2975,
 37-38=1814/2975, 14-38=1814/2975, 13-14=266/377, 12-13=266/377, 11-12=530/443,
 9-11=530/443
WEBS 3-18=1114/1471, 4-18=1560/958, 4-17=303/541, 5-16=542/304, 6-16=504/845,
 6-15=1653/947, 7-15=998/1700, 7-14=3378/1984, 8-14=1146/1788, 8-12=2754/1599

NOTES (13-14)

- 1) 2 X 6 SYP No.1D bearing block 12" long at jt. 12 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SYP No.2.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1453 lb uplift at joint 2 and 1997 lb uplift at joint 12



October 22, 2009

Continued on page 2

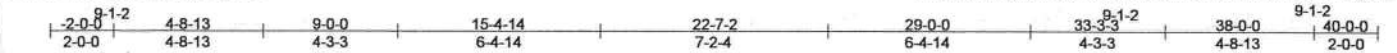
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T11	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137463
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:10 2009 Page 1



Scale = 1:70.2

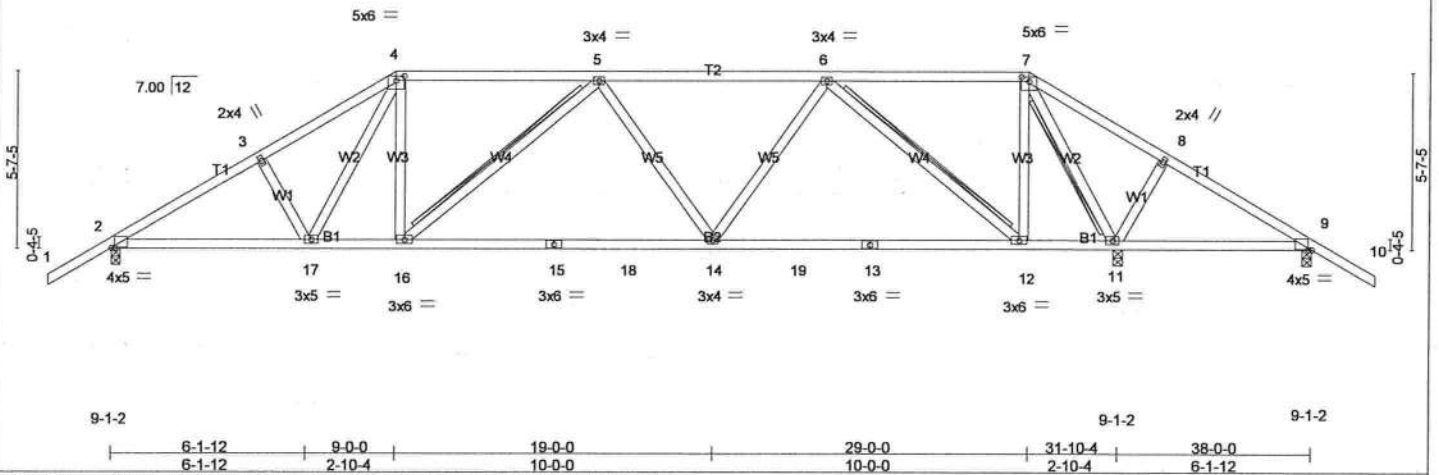


Plate Offsets (X,Y): [2:0-1-5,0-0-2], [4:0-3-0,0-1-12], [7:0-3-0,0-1-12], [9:0-1-5,0-0-2]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.35	Vert(LL) -0.18 14-16	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.74	Vert(TL) -0.49 14-16	>778	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.60	Horz(TL) 0.08 11	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.08 14	>999	240		Weight: 207 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

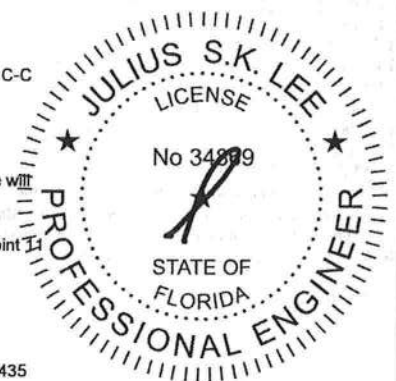
BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 5-16, 6-12, 7-11
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1382/0-3-8, 11=2154/0-3-8, 9=77/0-3-8
 Max Horz 2=145(LC 5)
 Max Uplift 2=268(LC 5), 11=505(LC 4), 9=284(LC 10)
 Max Grav 2=1382(LC 1), 11=2154(LC 1), 9=37(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=2080/735, 3-4=1943/757, 4-5=1535/647, 5-6=1713/689, 6-7=257/185,
 7-8=205/951, 8-9=228/831
 BOT CHORD 2-17=475/1712, 16-17=370/1522, 15-16=488/1833, 15-18=488/1833, 14-18=488/1833,
 14-19=345/1363, 13-19=345/1363, 12-13=345/1363, 9-11=686/322
 WEBS 4-16=114/516, 5-16=481/241, 6-14=113/627, 6-12=1479/578, 7-12=284/1141,
 7-11=2113/632, 8-11=256/201

- NOTES** (10-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2, 505 lb uplift at joint 11 and 284 lb uplift at joint 9.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T13	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137465
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Builders FirstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MITEK Industries, Inc. Thu Oct 22 15:32:12 2009 Page 1



Scale = 1:70.2

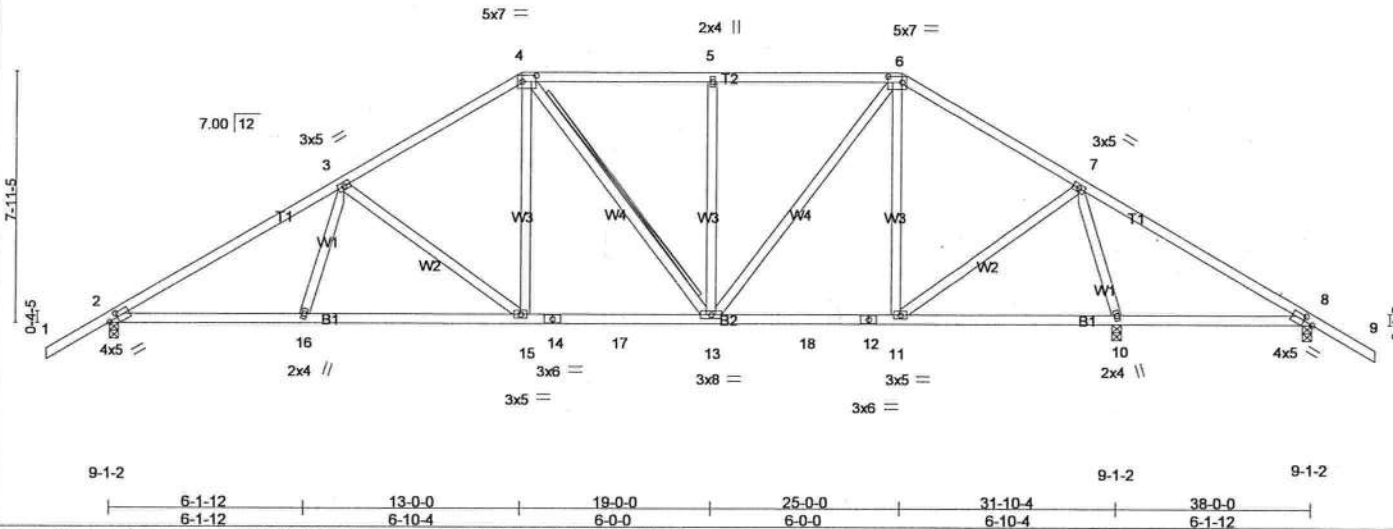


Plate Offsets (X,Y): [2:0-3-8,0-1-12], [4:0-5-4,0-2-4], [6:0-5-4,0-2-4], [8:0-3-8,0-1-12]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	BC 0.48	Vert(LL) -0.10 13-15 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.45	Vert(TL) -0.20 15-16 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.71	Horz(TL) 0.06 10 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.09 8-10 >806 240		Weight: 220 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-9-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 4-13
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MITEK recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size)

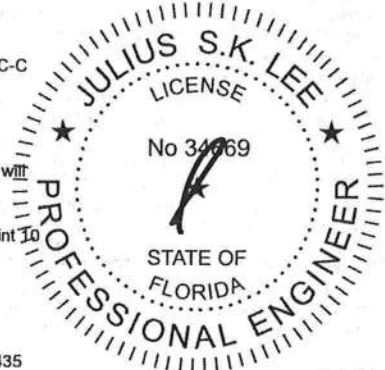
2=1444/0-3-8, 10=1951/0-3-8, 8=109/0-3-8
 Max Horz 2=210(LC 5)
 Max Uplift 2=292(LC 6), 10=300(LC 4), 8=234(LC 7)
 Max Grav 2=1444(LC 1), 10=1951(LC 1), 8=160(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2176/720, 3-4=-1660/665, 4-5=-1341/638, 5-6=-1341/638, 6-7=-1120/488, 7-8=-121/623
 BOT CHORD 2-16=-434/1773, 15-16=-445/1731, 14-15=-280/1356, 14-17=-280/1356, 13-17=-280/1356, 13-18=-117/871, 12-18=-117/871, 11-12=-117/871, 8-10=-433/256
 WEBS 3-16=0/299, 3-15=-482/286, 4-15=-131/511, 5-13=-410/259, 6-13=-277/801, 6-11=-378/169, 7-11=-197/981, 7-10=-1871/673

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2, 300 lb uplift at joint 10 and 234 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



October 22, 2009

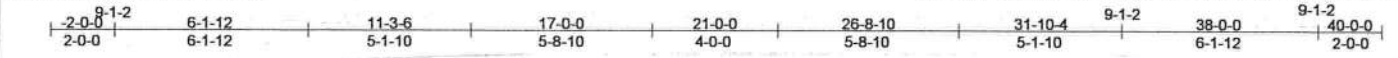
LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T15	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137467
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Builders FratSource, Lake City, FL 32055 7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:32:14 2009 Page 1



Scale = 1:70.2

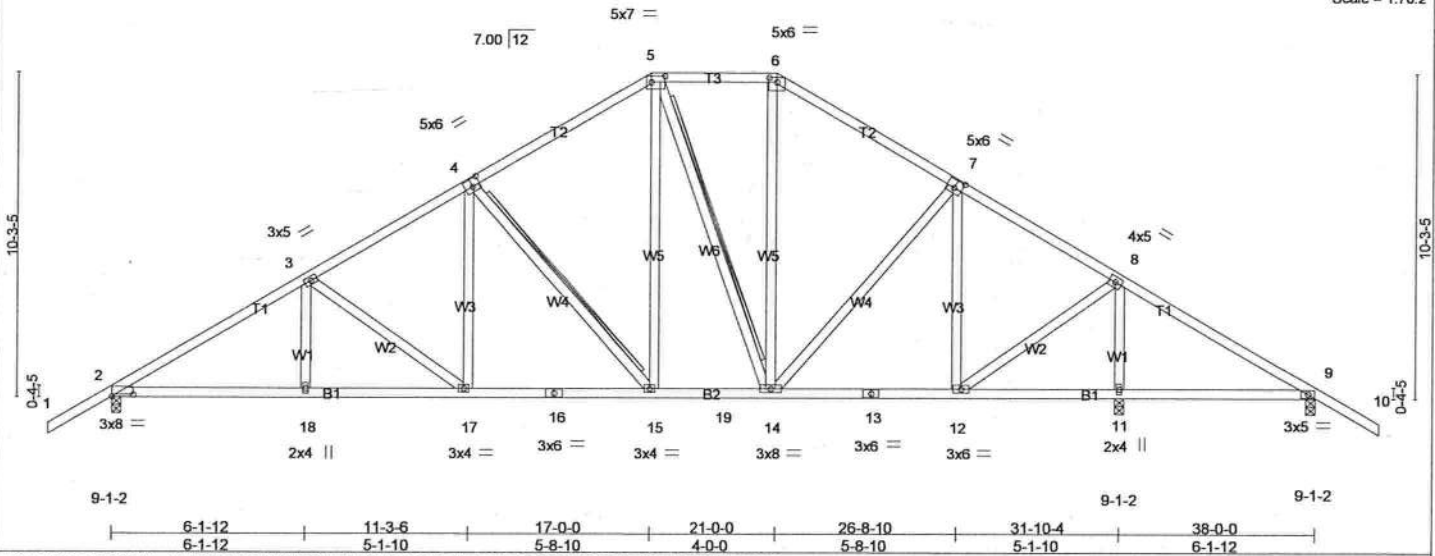


Plate Offsets (X,Y): [2:0-8-1,0-0-10], [4:0-3-0,0-3-0], [5:0-5-4,0-2-4], [6:0-3-0,0-1-12], [7:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL) -0.10 15-17 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.47	Vert(TL) -0.23 15-17 >999 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.48	Horz(TL) 0.06 11 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.07 9-11 >997 240		Weight: 244 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-6 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 4-15, 5-14
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

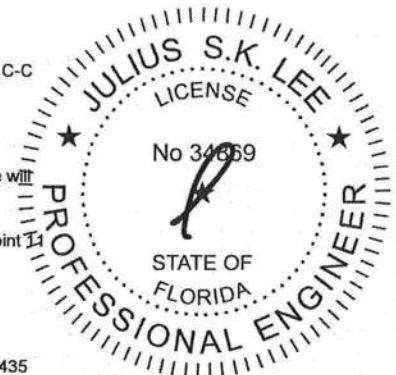
REACTIONS (lb/size) 2=1480/0-3-8, 11=1958/0-3-8, 9=141/0-3-8
 Max Horz 2=275(LC 5)
 Max Uplift 2=307(LC 6), 11=306(LC 7), 9=225(LC 7)
 Max Grav 2=1480(LC 1), 11=1958(LC 1), 9=209(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2253/710, 3-4=1856/669, 4-5=1355/587, 5-6=973/529, 6-7=1216/546,
 7-8=1009/405, 8-9=86/500
BOT CHORD 2-18=438/1849, 17-18=438/1849, 16-17=272/1535, 15-16=272/1535, 15-19=138/1098,
 14-19=138/1098, 13-14=43/806, 12-13=43/806, 11-12=346/215, 9-11=346/215
WEBS 3-17=384/202, 4-17=86/427, 4-15=677/332, 5-15=217/686, 5-14=433/156,
 6-14=102/312, 7-14=127/323, 7-12=620/244, 8-12=314/1406, 8-11=1833/615

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 2, 306 lb uplift at joint 11 and 225 lb uplift at joint 9.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T16	Truss Type HIP	Qty 2	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137468
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7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:15 2009 Page 2

LOAD CASE(S) Standard



October 22, 2009

Job 317841	Truss T18	Truss Type SPECIAL	Qty 7	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137470
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Builders FratSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:16 2009 Page 1

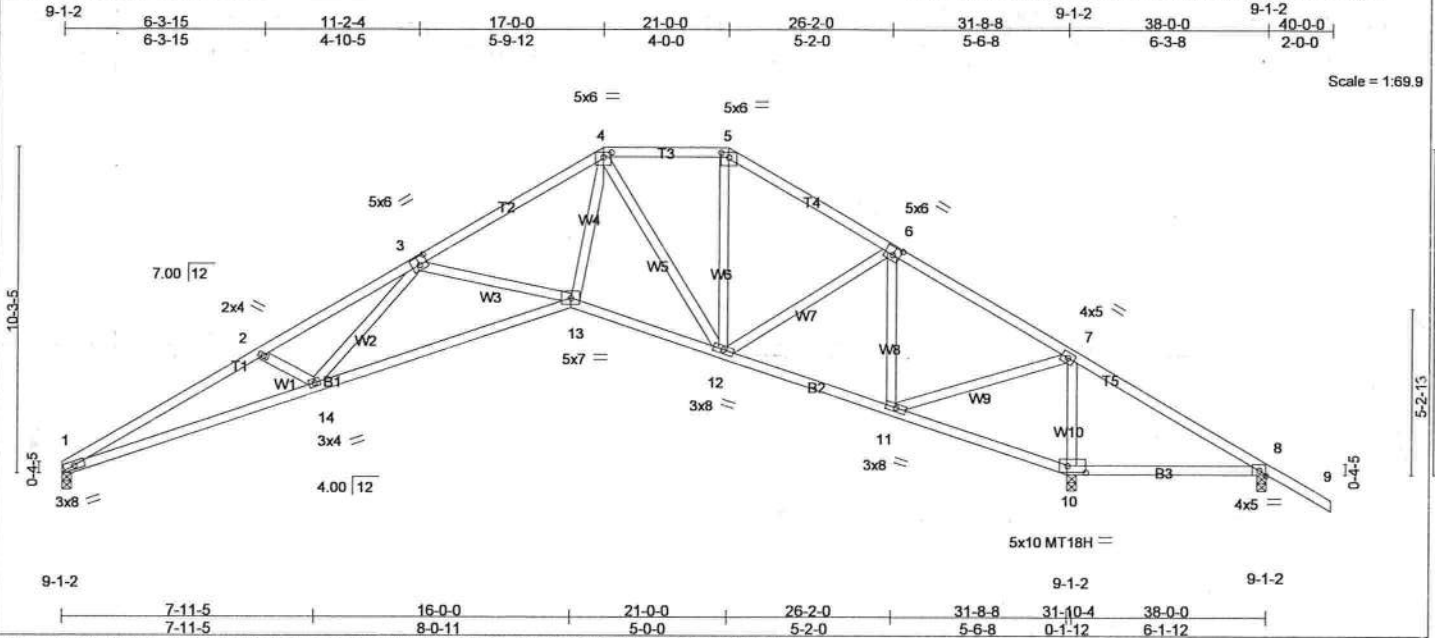


Plate Offsets (X,Y): [3:0-3-0-0-3-0], [4:0-3-0-0-1-12], [5:0-3-0-0-1-12], [6:0-2-12,0-3-0], [10:0-7-0,0-2-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.66	Vert(LL) -0.22 13-14 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.74	Vert(TL) -0.67 13-14 >566 240	MT18H	244/190
BCLL 0.0	Rep Stress Incr YES	WB 0.98	Horz(TL) 0.38 10 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.21 13-14 >999 240		Weight: 201 lb

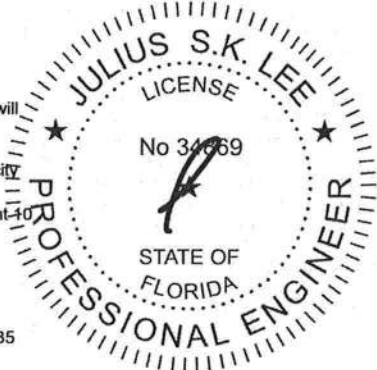
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-10-6 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-4-2 oc bracing.
WEBS 2 X 4 SYP No.3	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1031/0-3-8, 10=2903/0-3-8, 8=789/0-3-8
 Max Horz 1=298(LC 4)
 Max Uplift 1=183(LC 6), 10=487(LC 6), 8=889(LC 10)
 Max Grav 1=1031(LC 1), 10=2903(LC 1), 8=99(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=3462/1198, 2-3=3175/1101, 3-4=1805/526, 4-5=677/380, 5-6=869/381, 6-7=290/257, 7-8=575/2302
 BOT CHORD 1-14=917/3083, 13-14=500/2361, 12-13=192/1285, 10-11=2095/706, 8-10=1884/631
 WEBS 2-14=261/257, 3-14=282/751, 3-13=778/482, 4-13=239/1475, 4-12=1021/281, 6-12=212/820, 6-11=1154/354, 7-11=496/2056, 7-10=2121/721

- NOTES (12-13)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2
 - Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 1, 487 lb uplift at joint 10 and 889 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



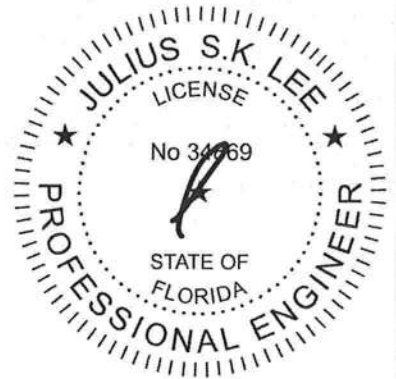
LOAD CASE(S) Standard October 22, 2009

Job 317841	Truss T19	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. I4137471
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LOAD CASE(S) Standard
 Concentrated Loads (lb)
 Vert: 3=-250(F) 4=-250(F) 8=-284(F) 7=-284(F)



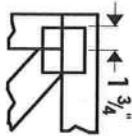
October 22, 2009

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 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

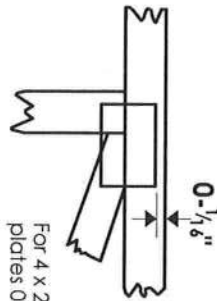
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 Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MITek 20/20 software or upon request.**

PLATE SIZE

4 X 4

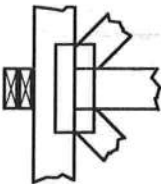
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

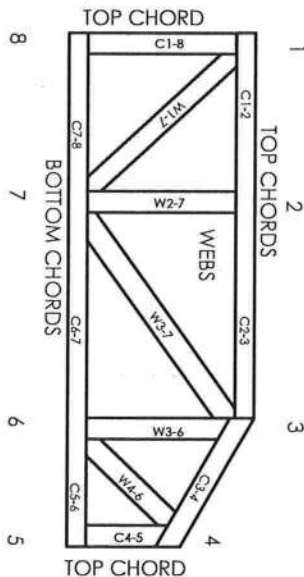
Industry Standards:

ANSI/TFP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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1109 Coastal Bay Blvd.
Boynton, FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wear at joint locations are regulated by ANSI/TFP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TFP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TFP 1 Quality Criteria.

STEPPED DOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
 BOT CHORD 2X4 SO. PINE #2 or Better
 WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

2' TYP. MAX

#1 HIP TRUSS

#2 HIP OR COMMON TRUSS

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC: *

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. FILE

UPLIFT: 400# or Less

BRG LOC: *

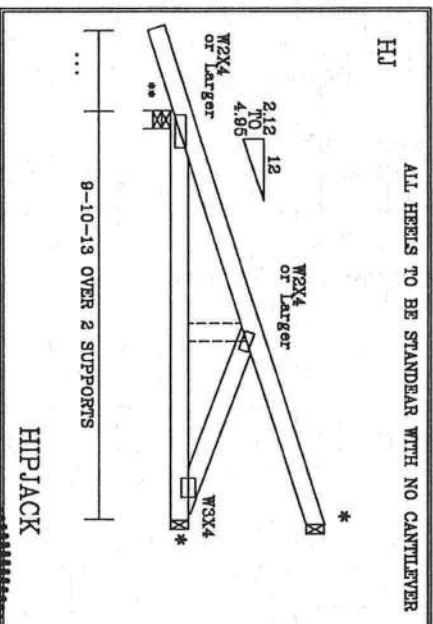
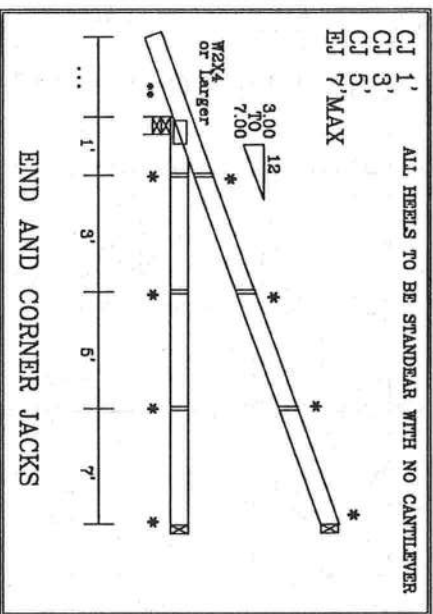
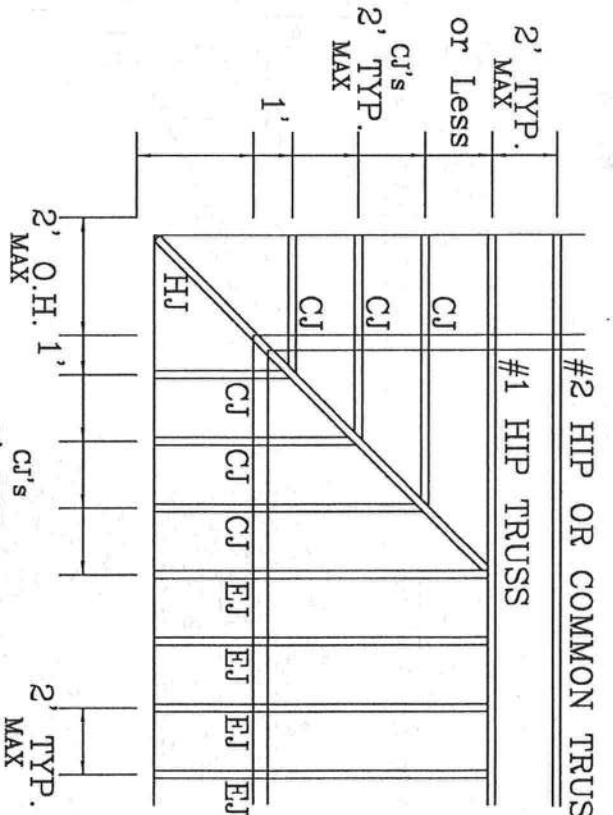
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC: *

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



C1 1'
 CJ 3'
 EJ 5'
 7' MAX

HJ
 ALL HEELS TO BE STANDEAR WITH NO CANTILEVER

* (3) 16d TOENAILS
 ** SEE EOR FOR TIE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
 BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
 SETBACK
 7'0" MAX

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC1-1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 BROMER RD, SUITE 200, MADISON, WI 53719 AND VTCB CODED TRUSS COUNCIL OF AMERICA, 6800 ENTERPRISE LANE, MADISON, WI 53719 FOR MORE INFORMATION. ALL TRUSS BRACING, STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING INSTRUCTIONS. ALL TRUSS BRACING, STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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MEMBER	TYPE	EXPIRES	STATUS
AC	DL	20	MAX PSF
BC	DL	10*	MAX PSF
BC	DL	5	MAX PSF
DL	DL	20	MAX PSF
DL	DL	10*	MAX PSF
DL	DL	5	MAX PSF
DUR. FAC.		1.25	
SPACING		2' MAX	

REF 7' MAX STBK CS
 DATE Jun./27/2008
 DRWG -ENG
 REVIEWED
 By: julius lee at 10:52 am, Jun 27, 2008

TOP CHORD 2x4 #2 OR BETTER
 BOT CHORD 2x4 #2 OR BETTER
 WEBS 2x4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
 SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG. LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

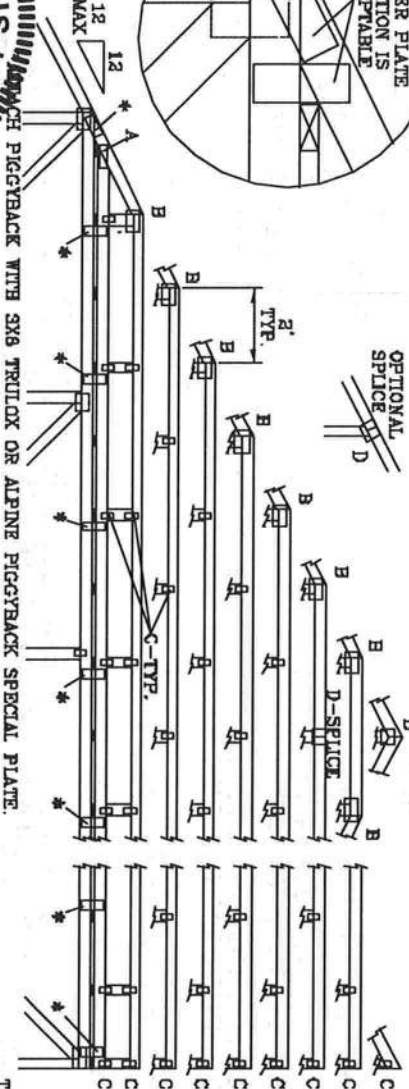
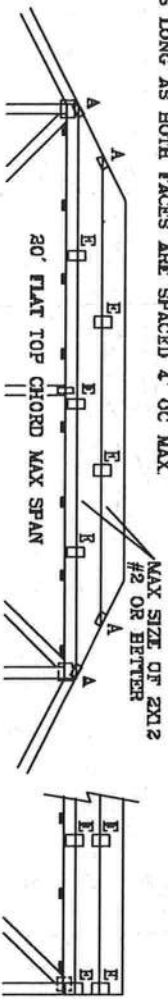
CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF

110 MPH WIND, 30' MEAN HGT, PEG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL-5 PSF, WIND BC DL-5 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX

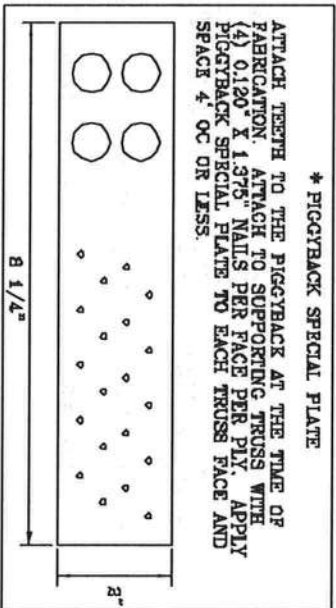
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



JOINT TYPE	SPANS UP TO		
	30'	34'	38'
A	2x4	2.6x4	3x5
B	4x6	6x6	6x8
C	1.5x3	1.6x4	1.6x4
D	5x4	6x5	6x5
E	4x6 OR 3x6 TRUSS AT 4' OC, HOTWATER VERTICALLY		

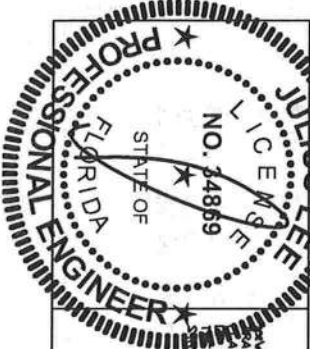
ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 T ¹ BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2x4 T ¹ BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



* PIGGYBACK SPECIAL PLATE
 ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATOR ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045



PROVISIONS: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, UNLOADING AND ERECTION. REFER TO BEST PRACTICE BUILDING COMPONENT SAFETY PROGRAMS, PUBLISHED BY THE TRUSS AND ROOFING ASSOCIATION OF AMERICA, FOR THE LATEST RECOMMENDATIONS FOR TRUSS DESIGN. TRUSS CHORDS SHALL BE PROPERLY ATTACHED TO THE STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID COLLING.

REVIEWED
 By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
 CONS. ENGINEERS P.A.

1446 SW 4th AVENUE
 DEALEY BEACH, FL 33441-2181

No. 34869
 STATE OF FLORIDA

MAX LOADING	REF	PIGGYBACK
55 PSF AT	DATE	09/12/07
1.33 DUR. FAC.	DRWG/MITEK STD	PIGGY
50 PSF AT	-ENG	JL
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING		24.0"

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AP&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

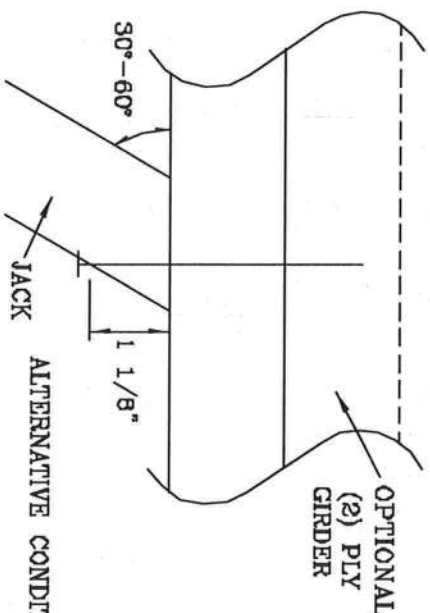
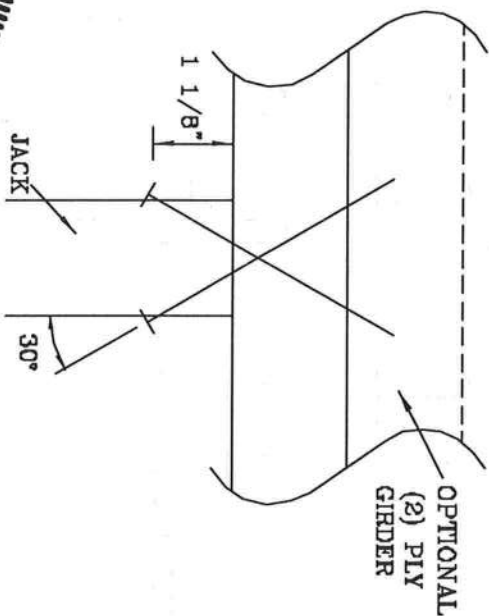
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

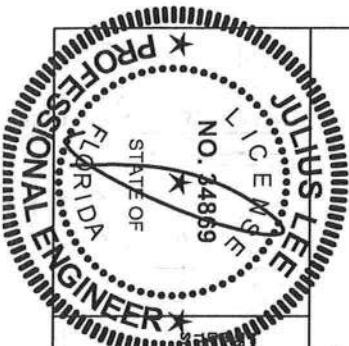
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	187#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS CONSTRUCTION. SAFETY INFORMATION: PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 288 PRINCEFORD DR., SUITE 200, NATION, VA 25715 AND VTGA (WOOD TRUSS COUNCIL), 100 EAST TOWN ST., UNLESS OTHERWISE INDICATED, TYP GRD SHALL HAVE PROPERLY ATTACHED CONNECTIONS. UNLESS OTHERWISE INDICATED, TYP GRD SHALL HAVE PROPERLY ATTACHED CONNECTIONS. UNLESS OTHERWISE INDICATED, TYP GRD SHALL HAVE PROPERLY ATTACHED CONNECTIONS.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 ST 4th AVENUE
DELRAY BEACH, FL 33444-3181

No: 34869
STATE OF FLORIDA

TC IL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC IL	PSF	ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

TRULOX CONNECTION DETAIL

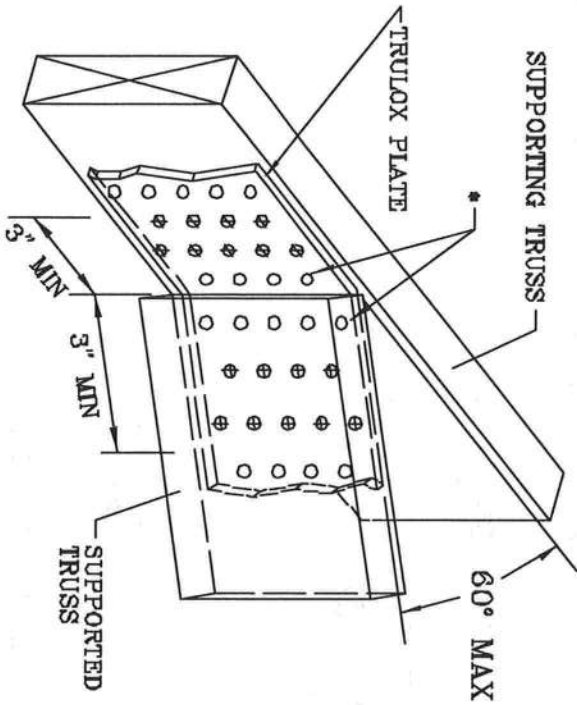
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

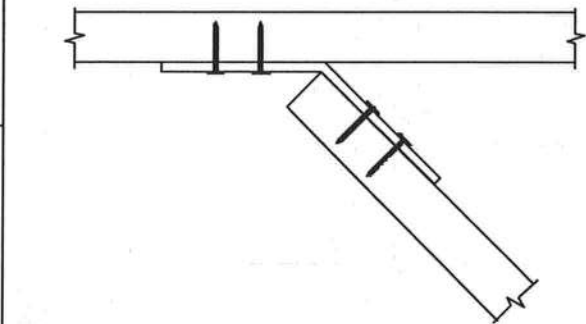
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

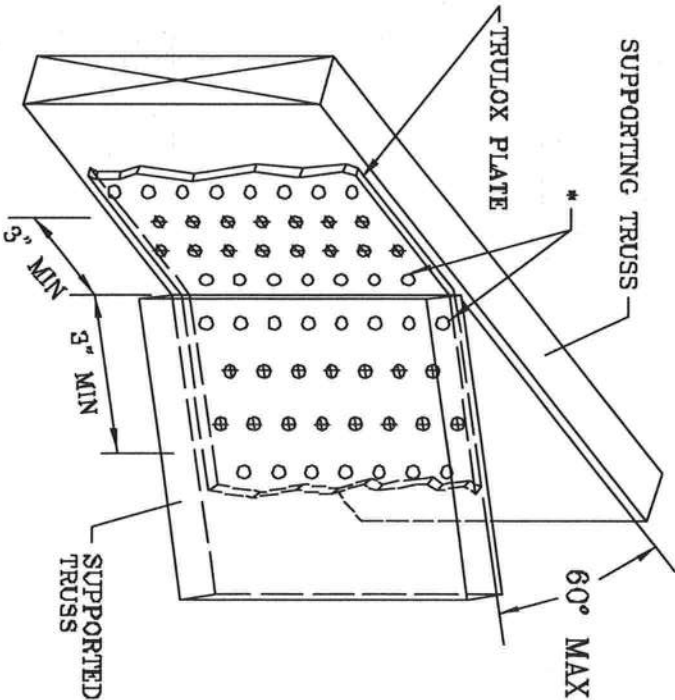
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE



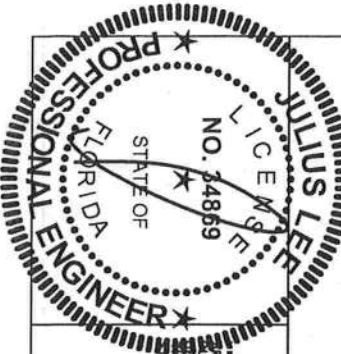
TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#



MINIMUM 5X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:58 am, Jun 14, 2008

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,988/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2001 I-BO BUILDING CONSTRUCTION SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 285 PENNINGTON DR., SUITE 800, MADISON, VT 05750 AND VITA CYCLO TRUSS COUNCIL, 1000 AMERICA, 6300 ENTERPRISE LN, MADISON, VT 05750 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

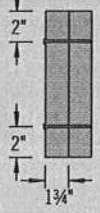
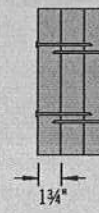
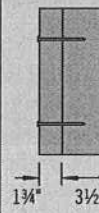
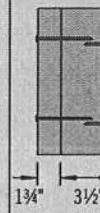
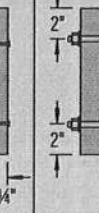
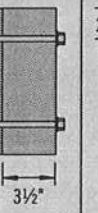
145 5TH AVE. AVENUE
DEARBORN BEACH, IL 60444-8181

No: 34889
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

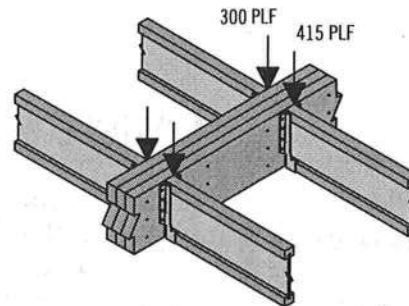
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply			
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 1/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- **Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.