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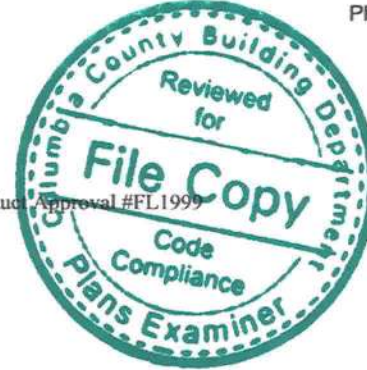
Alpine, an ITW Company  
 155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025  
 Phone: (800)755-6001  
 www.alpineitw.com



*[Handwritten signature]*

03/23/2023

COA#0-278  
 Florida Certificate of Product Approval #FL1999



<b>Site Information:</b>	<b>Page 1:</b>
Customer: W. B. Howland Company, Inc.	Job Number: 22-8029
Job Description: Mouras House	
Address:	

<b>Job Engineering Criteria:</b>	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 through 22.02.00 JRef #: 1XO72150002
Wind Standard: ASCE 7-16      Wind Speed (mph): 130	Design Loading (psf): 40.00, 45.00
Building Type: Closed	

This package contains general notes pages, 109 truss drawing(s) and 9 detail(s).

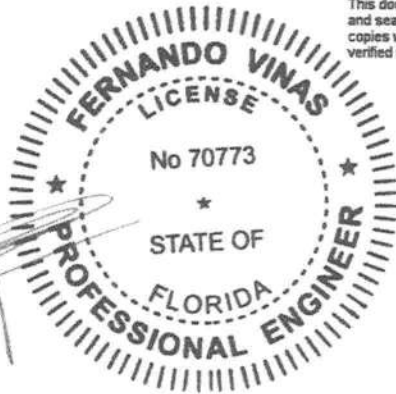
Item	Drawing Number	Truss
1	081.23.1511.48255	A01
3	081.23.1511.48731	A03
5	081.23.1511.49333	A05
7	081.23.1511.48013	A07
9	081.23.1511.46555	A09
11	081.23.1511.47697	A11
13	081.23.1511.49445	A13
15	081.23.1511.49910	A15
17	081.23.1511.52201	A17
19	081.23.1511.48631	A19
21	081.23.1511.46518	B02
23	081.23.1511.49651	B04
25	081.23.1511.46554	B06
27	081.23.1511.50730	B08
29	081.23.1511.50204	B10
31	081.23.1511.50059	B12
33	081.23.1511.51000	B14
35	081.23.1511.50977	B16
37	081.23.1511.47109	B18
39	081.23.1511.48959	B20
41	081.23.1511.48199	B22
43	081.23.1511.48527	C02
45	081.23.1545.04777	D01
47	081.23.1511.51294	D03
49	081.23.1511.51729	D05

Item	Drawing Number	Truss
2	081.23.1511.49062	A02
4	081.23.1511.48129	A04
6	081.23.1511.46963	A06
8	081.23.1511.46988	A08
10	081.23.1511.47963	A10
12	081.23.1511.49188	A12
14	081.23.1511.48336	A14
16	081.23.1511.47604	A16
18	081.23.1511.49561	A18
20	081.23.1511.49146	B01
22	081.23.1511.48978	B03
24	081.23.1511.49299	B05
26	081.23.1511.50355	B07
28	081.23.1511.50501	B09
30	081.23.1511.50856	B11
32	081.23.1511.50979	B13
34	081.23.1511.50168	B15
36	081.23.1511.49282	B17
38	081.23.1511.48462	B19
40	081.23.1511.49506	B21
42	081.23.1511.47894	C01
44	081.23.1511.51917	C03
46	081.23.1511.51390	D02
48	081.23.1511.51244	D04
50	081.23.1511.46565	D06

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<b>Site Information:</b>	<b>Page 2:</b>
Customer: W. B. Howland Company, Inc.	Job Number: 22-8029
Job Description: Mouras House	
Address:	

Item	Drawing Number	Truss
51	081.23.1511.47558	D07
53	081.23.1511.50354	D09
55	081.23.1511.52049	D11
57	081.23.1511.51004	D13
59	081.23.1511.48937	D15
61	081.23.1511.49235	E01
63	081.23.1511.48502	E03
65	081.23.1511.49020	E05
67	081.23.1511.52188	G01
69	081.23.1511.46524	J01
71	081.23.1511.48616	J02
73	081.23.1511.46506	J03
75	081.23.1511.47308	J05
77	081.23.1511.48931	J07
79	081.23.1511.48276	J09
81	081.23.1511.47639	PB01
83	081.23.1511.47286	PB03
85	081.23.1511.51280	PB05
87	081.23.1511.50551	PB07
89	081.23.1511.49457	PB09
91	081.23.1511.47487	PB11
93	081.23.1511.48449	PB13
95	081.23.1511.51620	PB15
97	081.23.1511.47805	PB17
99	081.23.1511.49854	PB19
101	081.23.1511.48574	PB21
103	081.23.1511.50680	V02
105	081.23.1511.50764	V04
107	081.23.1511.48045	V06
109	081.23.1511.47006	V08
111	A14030ENC160118	
113	CNNAILSP1014	
115	GBLLETIN0118	

Item	Drawing Number	Truss
52	081.23.1511.47843	D08
54	081.23.1511.51853	D10
56	081.23.1511.52092	D12
58	081.23.1511.51892	D14
60	081.23.1511.50122	D16
62	081.23.1511.48829	E02
64	081.23.1511.47538	E04
66	081.23.1511.47246	E06
68	081.23.1511.47088	H01
70	081.23.1511.47488	J01HJ
72	081.23.1511.47188	J02HJ
74	081.23.1511.47369	J04
76	081.23.1511.46493	J06
78	081.23.1511.48724	J08
80	081.23.1511.50429	K01
82	081.23.1511.47368	PB02
84	081.23.1511.47459	PB04
86	081.23.1511.49116	PB06
88	081.23.1511.51609	PB08
90	081.23.1511.47221	PB10
92	081.23.1511.48411	PB12
94	081.23.1511.51587	PB14
96	081.23.1511.51715	PB16
98	081.23.1511.47063	PB18
100	081.23.1511.47753	PB20
102	081.23.1511.51198	V01
104	081.23.1511.50607	V03
106	081.23.1511.46517	V05
108	081.23.1511.48763	V07
110	A14015ENC160118	
112	BRCLBSUB0119	
114	DEFLCAMB1014	
116	PB160160118	

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<b>Site Information:</b>	<b>Page 3:</b>
<i>Customer:</i> W. B. Howland Company, Inc.	<i>Job Number:</i> 22-8029
<i>Job Description:</i> Mouras House	
<i>Address:</i>	

Item	Drawing Number	Truss
117	VAL180160118	

Item	Drawing Number	Truss
118	VALTN160118	

## **General Notes**

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

## **General Notes** (continued)

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

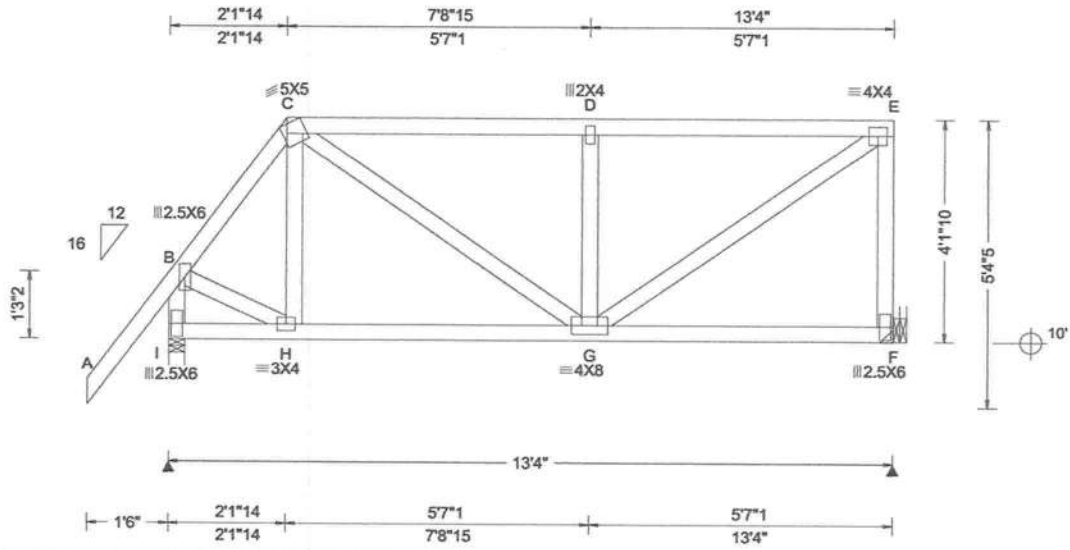
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

**References:**

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcacomponents.com](http://www.sbcacomponents.com).



<b>Loading Criteria (psf)</b> TCELL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.024 D 999 240 VERT(CL): 0.049 D 999 180 HORZ(LL): 0.007 C - - HORZ(TL): 0.015 C - - Creep Factor: 2.0 Max TC CSI: 0.803 Max BC CSI: 0.510 Max Web CSI: 0.470  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 1174 /- /- /- /272 /- F 1051 /- /- /- /275 /- Wind reactions based on MWFRS I Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing I is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 234 -1009 D - E 270 -1035 C - D 272 -1038					
				<b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. H - G 568 -137  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. B - I 275 -1189 D - G 346 -684 B - H 625 -152 G - E 1235 -322 C - G 564 -161 E - F 298 -927					

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

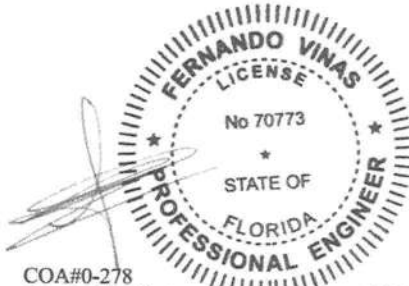
**Special Loads**  
 ——(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 73 plf at -1.50 to 73 plf at 2.16	TC: From 37 plf at 2.16 to 37 plf at 13.33
BC: From 7 plf at -1.50 to 7 plf at 0.00	BC: From 20 plf at 0.00 to 20 plf at 2.21
BC: From 10 plf at 2.21 to 10 plf at 13.33	
TC: 207 lb Conc. Load at 2.21	
TC: 133 lb Conc. Load at 4.26, 6.26, 8.26, 10.26, 12.26	
BC: 51 lb Conc. Load at 2.21	
BC: 91 lb Conc. Load at 4.26, 6.26, 8.26, 10.26, 12.26	

**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.



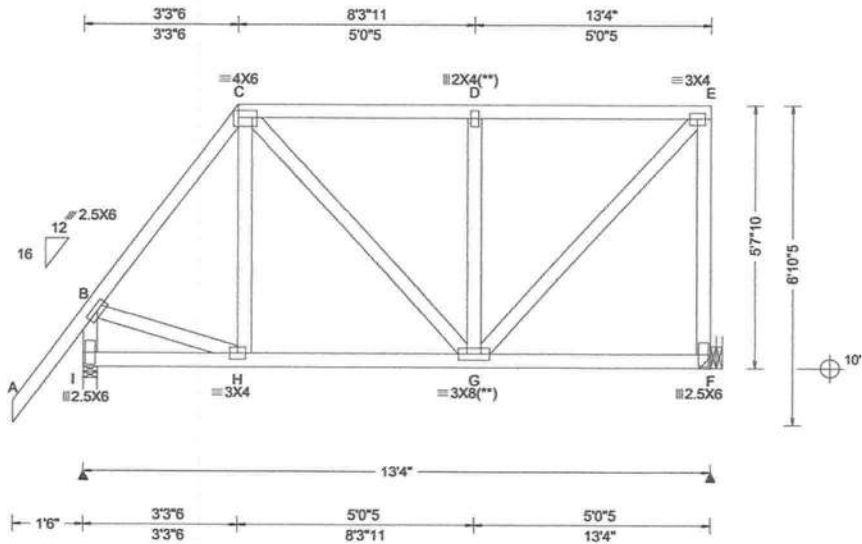
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**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: [alpineitw.com](http://alpineitw.com); TPI: [tpinst.org](http://tpinst.org); SBCA: [sbcacomponents.com](http://sbcacomponents.com); ICC: [iccsafe.org](http://iccsafe.org); AWC: [awc.org](http://awc.org)





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																																	
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.010 D 999 240 VERT(CL): 0.022 D 999 180 HORZ(LL): 0.003 C - - HORZ(TL): 0.007 C - -  Creep Factor: 2.0 Max TC CSI: 0.493 Max BC CSI: 0.255 Max Web CSI: 0.514  VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>I</td> <td>749</td> <td>-</td> <td>-</td> <td>1468</td> <td>163</td> <td>1208</td> </tr> <tr> <td>F</td> <td>615</td> <td>-</td> <td>-</td> <td>1305</td> <td>1135</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS I Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing I is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>349</td> <td>-599</td> <td>D - E</td> <td>362</td> <td>-405</td> </tr> <tr> <td>C - D</td> <td>364</td> <td>-407</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	I	749	-	-	1468	163	1208	F	615	-	-	1305	1135	-	Chords	Tens.Comp.		Chords	Tens. Comp.						B - C	349	-599	D - E	362	-405	C - D	364	-407			
Loc	Gravity			Non-Gravity																																																	
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F	615	-	-	1305	1135	-																																															
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B - C	349	-599	D - E	362	-405																																																
C - D	364	-407																																																			

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Hangers / Ties**

(J) Hanger Support Required, by others

**Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



COA#0-278  
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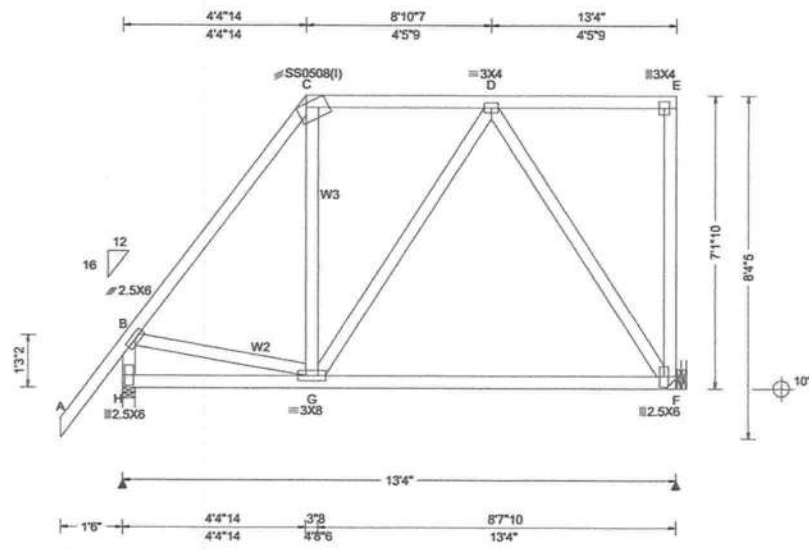
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	A Maximum Reactions (lbs)																											
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.004 G 999 240 VERT(CL): 0.009 G 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.136 Max BC CSI: 0.288 Max Web CSI: 0.622 VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>749</td> <td>-</td> <td>-</td> <td>480</td> <td>34</td> <td>261</td> </tr> <tr> <td>F</td> <td>615</td> <td>-</td> <td>-</td> <td>332</td> <td>157</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 264 -625	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	749	-	-	480	34	261	F	615	-	-	332	157	-
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**Lumber**  
 Top chord: 2x4 SP M-31;  
 Bot chord: 2x4 SP M-31;  
 Webs: 2x4 SP #3; W2, W3 2x4 SP M-31;

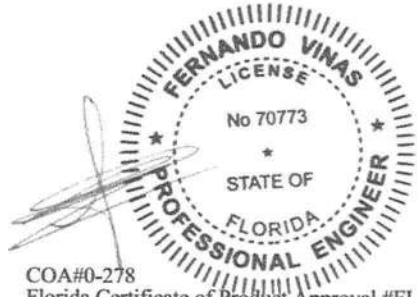
**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Purlins**  
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**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**  
 Chords Tens.Comp.  
 H - G 219 -414

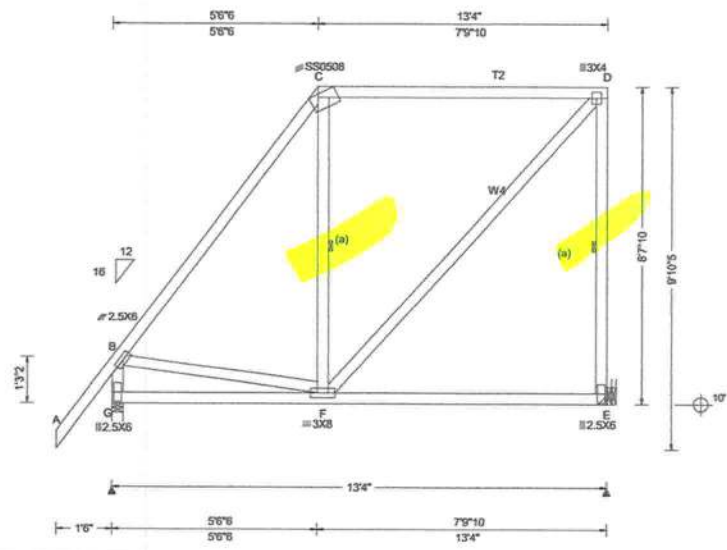
**Maximum Web Forces Per Ply (lbs)**  
 Webs Tens.Comp. Webs Tens. Comp.  
 B - H 292 -734 D - F 528 -494



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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 *	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.007 C 999 240 VERT(CL): 0.015 C 999 180 HORZ(LL): -0.007 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.685 Max BC CSI: 0.569 Max Web CSI: 0.579  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL G 749 /- /- /486 /- /314 E 615 /- /- /366 /184 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 194 -612
				<b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. G - F 285 -494  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. B - G 225 -711 F - D 420 -460 C - F 396 -184 D - E 614 -554

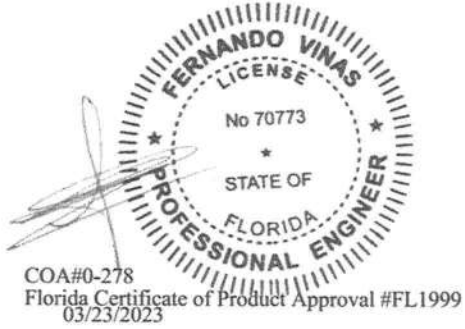
**Lumber**  
 Top chord: 2x4 SP #2; T2 2x4 SP M-31;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3; W4 2x4 SP M-31;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
 (J) Hanger Support Required, by others

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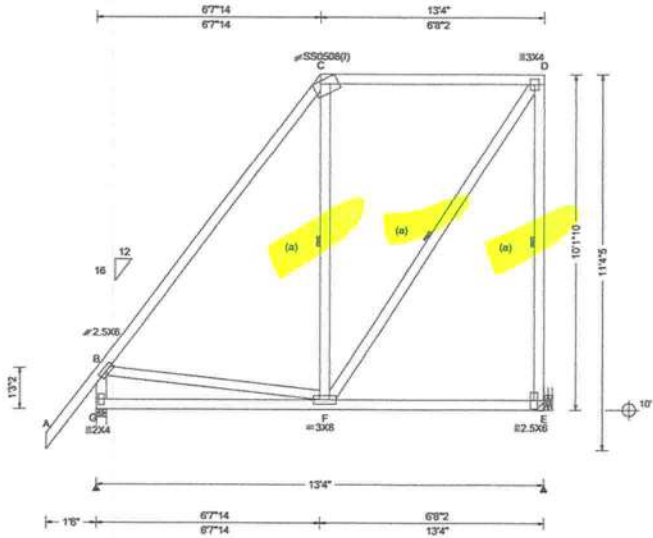


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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.008 C 999 240 VERT(CL): 0.018 C 999 180 HORZ(LL): -0.012 C - - HORZ(TL): 0.015 C - - Creep Factor: 2.0 Max TC CSI: 0.368 Max BC CSI: 0.522 Max Web CSI: 0.622  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 749 /- /- /484 /- /367 E 615 /- /- /408 /217 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 104 -590

**Lumber**  
Top chord: 2x4 SP M-31;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

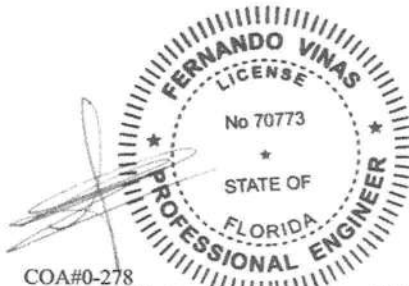
**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
(l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
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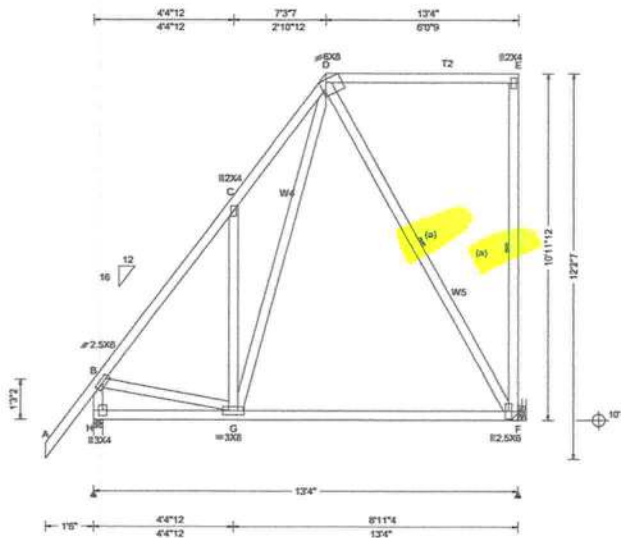


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155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																
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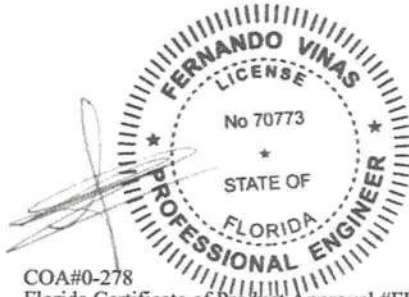
**Lumber**  
Top chord: 2x4 SP M-31; T2 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3; W4,W5 2x4 SP M-31;

**Bracing**  
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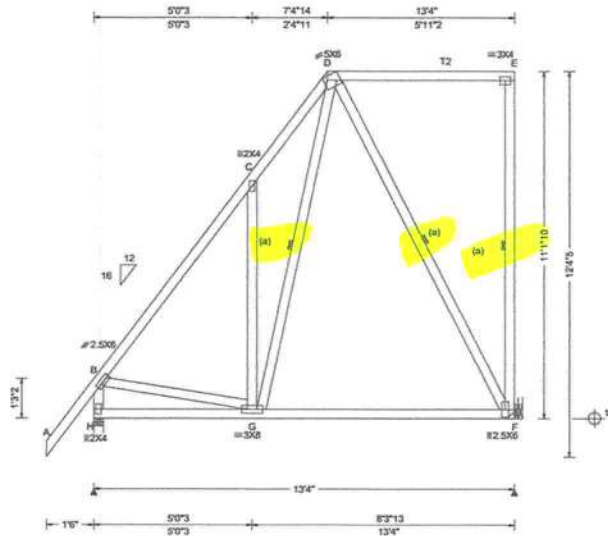
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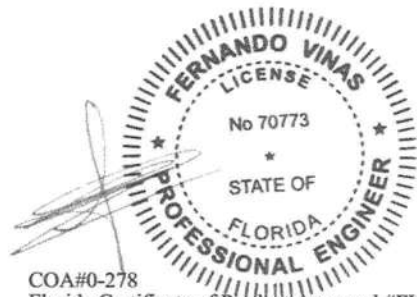
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Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

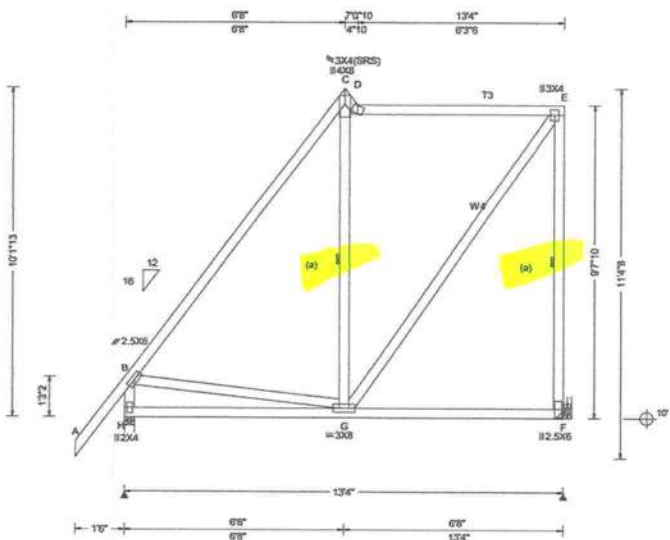
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



COA#0-278  
Florida Certificate of Product Approval #FL1999  
03/23/2023

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.028 D 999 240 VERT(CL): 0.057 D 999 180 HORZ(LL): -0.013 D - - HORZ(TL): 0.025 C - - Creep Factor: 2.0 Max TC CSI: 0.801 Max BC CSI: 0.518 Max Web CSI: 0.597  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
				H 772 /- /- /482 /- /288 F 738 /- /- /411 /159 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp.

**Lumber**  
 Top chord: 2x4 SP #2; T3 2x4 SP M-31;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3; W4 2x4 SP M-31;

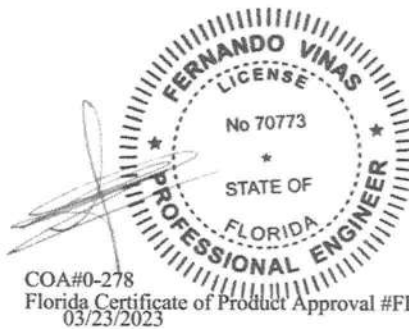
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Loading**  
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

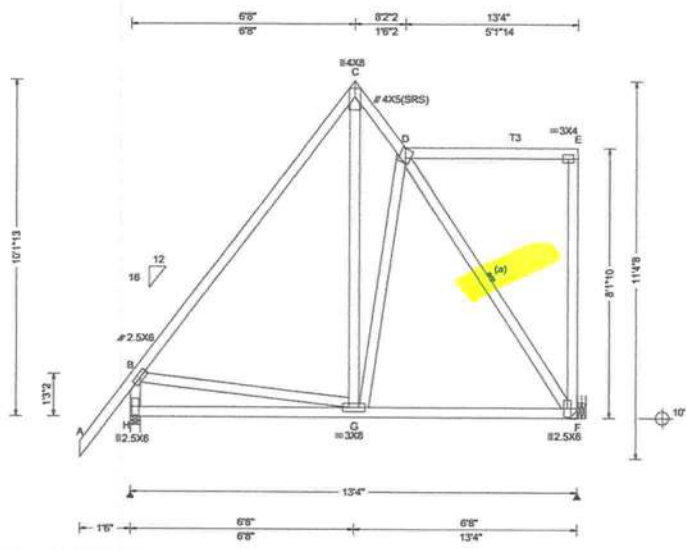
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.



COA#0-278  
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 D 999 240 VERT(CL): 0.017 D 999 180 HORZ(LL): -0.007 E - - HORZ(TL): 0.009 E - - Creep Factor: 2.0 Max TC CSI: 0.462 Max BC CSI: 0.568 Max Web CSI: 0.258  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 787 /- /- /469 /- /307 F 697 /- /- /424 /164 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 142 -643 C - D 261 -431

**Lumber**  
Top chord: 2x4 SP M-31; T3 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
(J) Hanger Support Required, by others

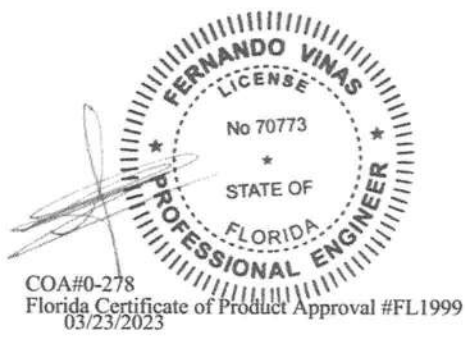
**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
H - G 375 -522

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
B - H 188 -745 D - F 393 -534

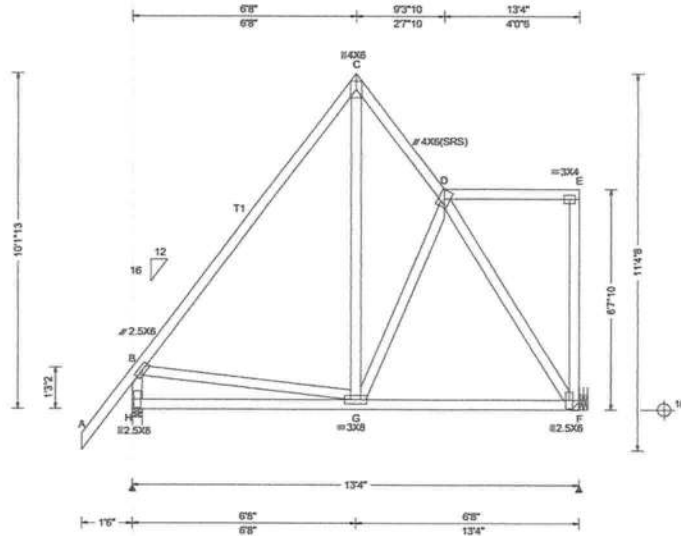


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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 G 999 240 VERT(CL): 0.016 G 999 180 HORZ(LL): 0.005 C - - HORZ(TL): 0.007 E - - Creep Factor: 2.0 Max TC CSI: 0.694 Max BC CSI: 0.568 Max Web CSI: 0.580  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 774 /- /- /459 /22 /325 F 680 /- /- /434 /131 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 173 -625 C - D 272 -459

**Lumber**  
Top chord: 2x4 SP #2; T1 2x4 SP M-31;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

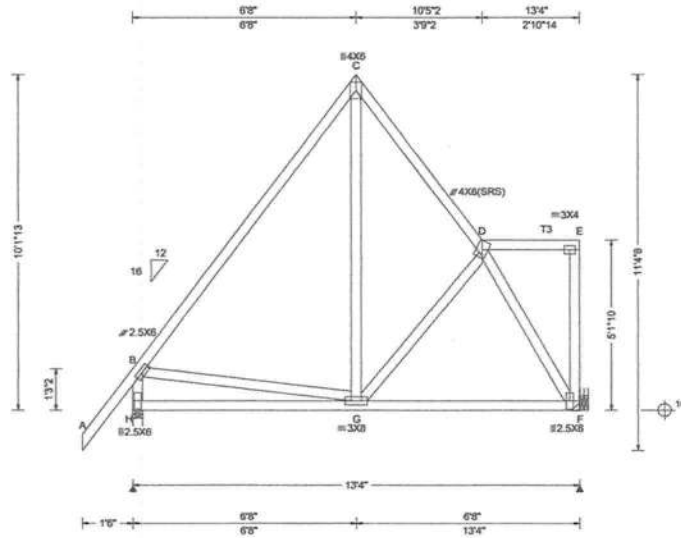
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.006 G 999 240 VERT(CL): 0.013 G 999 180 HORZ(LL): 0.005 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.295 Max BC CSI: 0.569 Max Web CSI: 0.329  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ /R- /Rh Non-Gravity /Rw /U /RL H 749 /- /- /451 /43 /344 F 615 /- /- /442 /102 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 192 -588 C - D 265 -468
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**Lumber**  
Top chord: 2x4 SP M-31; T3 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
H - G 417 -406

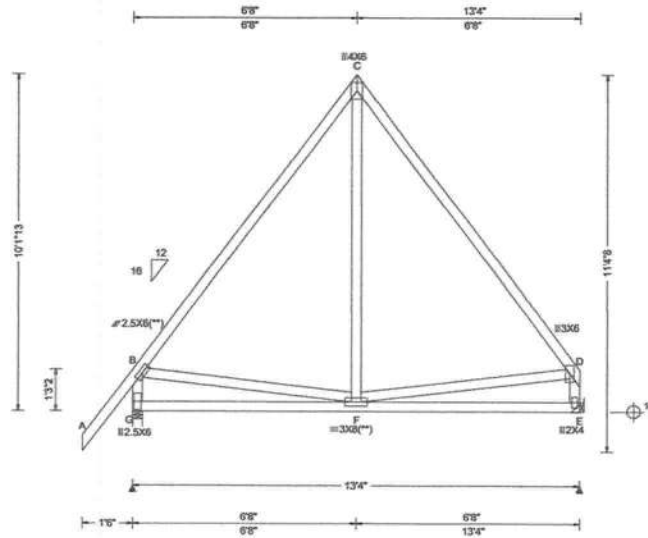
**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
B - H 232 -696 D - F 293 -546



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.004 F 999 240 VERT(CL): 0.011 F 999 180 HORZ(LL): 0.005 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.339 Max BC CSI: 0.157 Max Web CSI: 0.295  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL G 749 /- /- /481 /73 /392 E 615 /- /- /452 /54 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 224 -591 C - D 212 -583

**Lumber**  
Top chord: 2x4 SP M-31;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3;

**Plating Notes**  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Hangers / Ties**  
(J) Hanger Support Required, by others

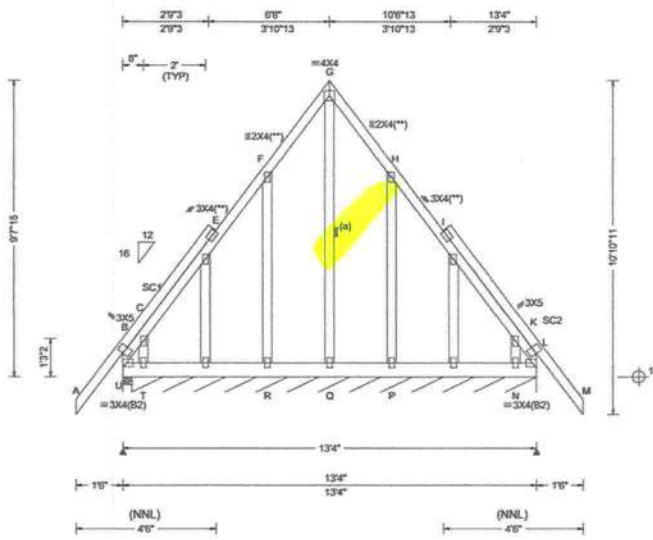
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Loading Criteria (psf)	
TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

Wind Criteria	
Wind Std:	ASCE 7-16
Speed:	130 mph
Enclosure:	Closed
Risk Category:	II
EXP:	C Kzt: NA
Mean Height:	15.00 ft
TCDL:	5.0 psf
BCDL:	5.0 psf
MWFRS Parallel Dist:	0 to h/2
C&C Dist a:	3.00 ft
Loc. from endwall:	Any
GCpi:	0.18
Wind Duration:	1.60

Snow Criteria (Pg,Pf in PSF)	
Pg:	NA
Ct:	NA
CAT:	NA
Pf:	NA
Ce:	NA
Lu:	NA
Cs:	NA
Snow Duration:	NA
Building Code:	
FBC	7th Ed. 2020 Res.
TPI Std:	2014
Rep Fac:	Yes
FT/RT:	20(0)/10(0)
Plate Type(s):	WAVE

Defl/CSI Criteria	
PP Deflection in loc L/defl L/#	
VERT(LL):	-0.003 I 999 240
VERT(CL):	0.004 E 999 180
HORZ(LL):	-0.011 L - -
HORZ(TL):	0.015 L - -
Creep Factor:	2.0
Max TC CSI:	0.438
Max BC CSI:	0.020
Max Web CSI:	0.156
VIEW Ver:	21.02.00.1005.17

▲ Maximum Reactions (lbs), or * = PLF							
Gravity			Non-Gravity				
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL		
U 314	-	-	/296	/89	/413		
L* 90	-	-	/66	/22	-		
Wind reactions based on MWFRS							
U Brg Wid = 3.5	Min Req = 1.5 (Truss)						
L Brg Wid = 156	Min Req = -						
Bearings U & U are a rigid surface.							
Members not listed have forces less than 375#							
Maximum Top Chord Forces Per Ply (lbs)							
Chords		Tens. Comp.		Chords		Tens. Comp.	
B - C	454	-714	I - K	381	-648		
B - E	493	-264	I - L	454	-264		
C - E	421	-665	K - L	414	-695		
F - G	383	-136					

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x6 SP 2400f-2.0E;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**

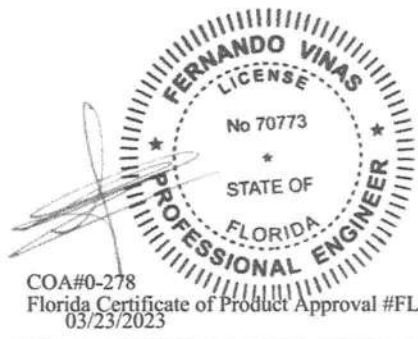
All plates are 2X4 except as noted.  
(\*\*) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



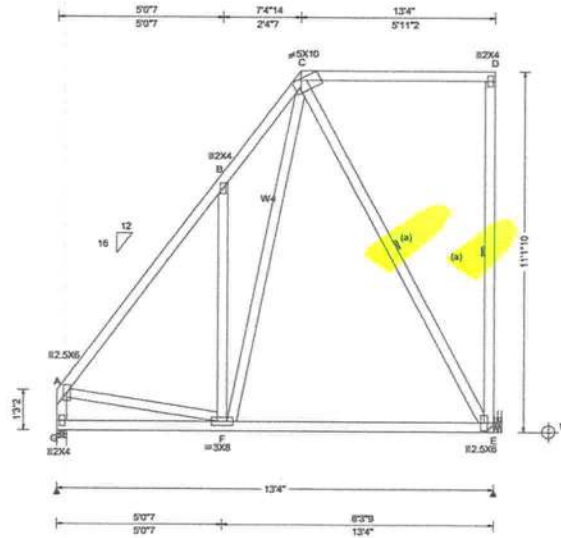
Maximum Bot Chord Forces Per Ply (lbs)							
Chords		Tens. Comp.		Chords		Tens. Comp.	
T - R	384	-231	Q - P	390	-229		
R - Q	390	-229	P - N	383	-226		

Maximum Gable Forces Per Ply (lbs)							
Gables		Tens. Comp.		Gables		Tens. Comp.	
F - R	408	-241	P - H	412	-240		
G - Q	63	-502					

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.20 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.010 B 999 240 VERT(CL): 0.023 B 999 180 HORZ(LL): 0.011 B - - HORZ(TL): 0.020 B - - Creep Factor: 2.0 Max TC CSI: 0.797 Max BC CSI: 0.662 Max Web CSI: 0.636  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity G 622 /- /- /361 /- /256 E 622 /- /- /447 /191 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 23 -619 B - C 505 -607

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3; W4 2x4 SP M-31;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



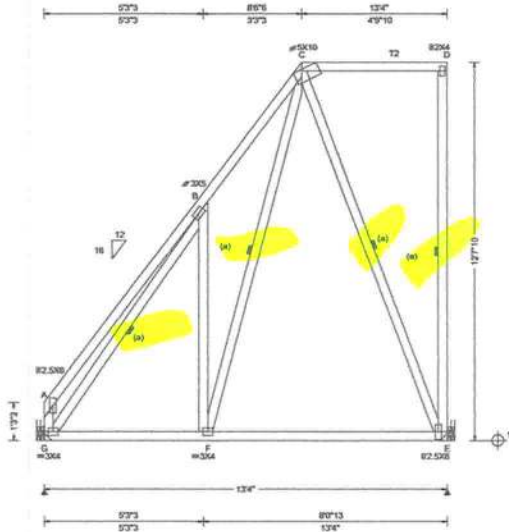
COA#0-278  
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03/23/2023

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
G - F 276 -541

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
A - G 14 -596 F - C 647 -540  
B - F 583 -466 C - E 471 -385

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.95 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.011 B 999 240 VERT(CL): 0.026 B 999 180 HORZ(LL): 0.016 B - - HORZ(TL): 0.027 B - - Creep Factor: 2.0 Max TC CSI: 0.505 Max BC CSI: 0.608 Max Web CSI: 0.725  VIEW Ver: 21.02.00B.1108.20	<b>▲ Maximum Reactions (lbs)</b> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>G 622</td> <td>/-</td> <td>/-</td> <td>/347</td> <td>/-</td> <td>/298</td> <td></td> </tr> <tr> <td>E 622</td> <td>/-</td> <td>/-</td> <td>/502</td> <td>/220</td> <td>/-</td> <td></td> </tr> </tbody> </table> Wind reactions based on MWFRS G Brg Wid = - Min Req = - E Brg Wid = - Min Req = - Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 400 -597	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	G 622	/-	/-	/347	/-	/298		E 622	/-	/-	/502	/220	/-	
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
G 622	/-	/-	/347	/-	/298																										
E 622	/-	/-	/502	/220	/-																										

**Lumber**  
Top chord: 2x4 SP M-31; T2 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

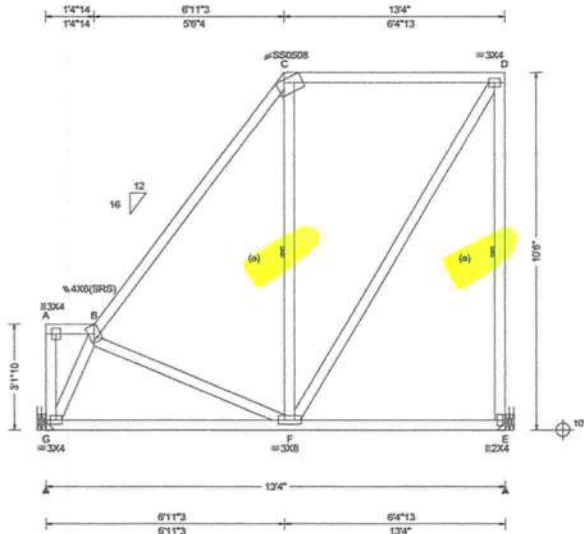


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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.64 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.008 C 999 240 VERT(CL): 0.020 C 999 180 HORZ(LL): 0.006 C - - HORZ(TL): 0.014 C - - Creep Factor: 2.0 Max TC CSI: 0.684 Max BC CSI: 0.523 Max Web CSI: 0.719  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity G 622 /- /- /336 /- /196 E 622 /- /- /437 /- /- Wind reactions based on MWFRS G Brg Wid = - Min Req = - E Brg Wid = - Min Req = - Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 0 -536  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. G - F 378 -454  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. G - B 158 -658 D - E 165 -576 F - D 448 -266
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**

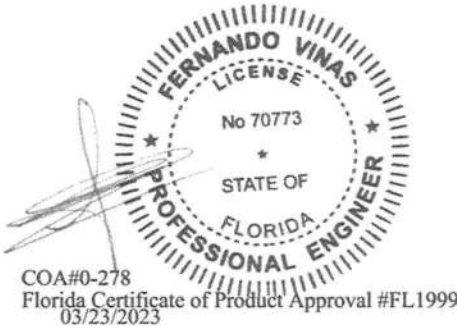
(J) Hanger Support Required, by others

**Purlins**

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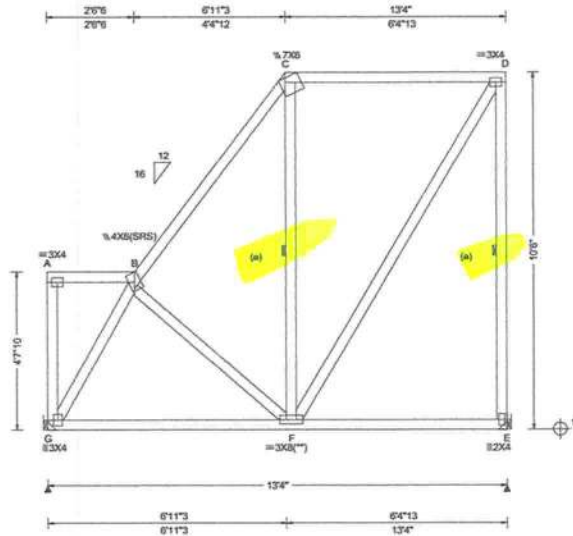
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>																														
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.14 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 C 999 240 VERT(CL): 0.020 C 999 180 HORZ(LL): 0.006 C - - HORZ(TL): 0.013 C - - Creep Factor: 2.0 Max TC CSI: 0.717 Max BC CSI: 0.588 Max Web CSI: 0.656  VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>Rh</th> <th>Rw</th> <th>U</th> <th>RL</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>622</td> <td>-</td> <td>-</td> <td>329</td> <td>-</td> <td>158</td> </tr> <tr> <td>E</td> <td>622</td> <td>-</td> <td>-</td> <td>434</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS G Brg Wid = - Min Req = - E Brg Wid = - Min Req = - Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp.</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td>B - C</td> <td>0</td> <td>-488</td> </tr> </table>	Loc	Gravity			Non-Gravity			R+	R-	Rh	Rw	U	RL	G	622	-	-	329	-	158	E	622	-	-	434	-	-	B - C	0	-488
Loc	Gravity			Non-Gravity																														
	R+	R-	Rh	Rw	U	RL																												
G	622	-	-	329	-	158																												
E	622	-	-	434	-	-																												
B - C	0	-488																																

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

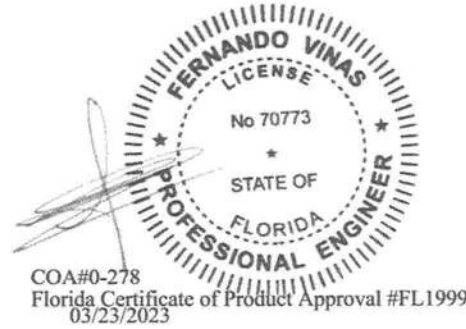
**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
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**Hangers / Ties**  
(J) Hanger Support Required, by others

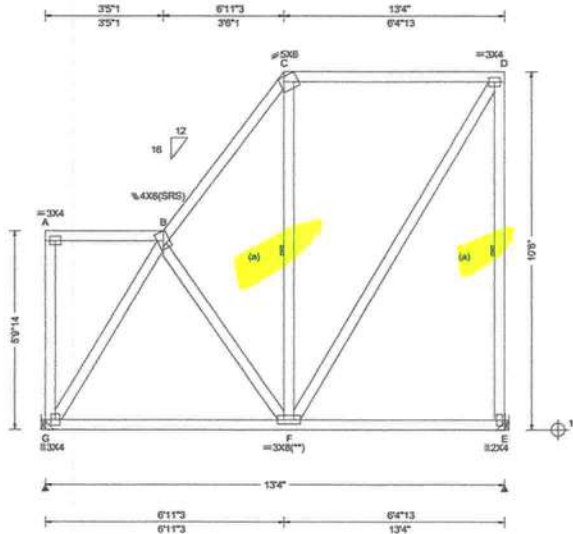
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.32 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.008 C 999 240 VERT(CL): 0.019 C 999 180 HORZ(LL): 0.005 C - - HORZ(TL): 0.012 C - - Creep Factor: 2.0 Max TC CSI: 0.734 Max BC CSI: 0.587 Max Web CSI: 0.605  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity G 622 /- /- /326 /- /127 E 622 /- /- /426 /- /- Wind reactions based on MWFRS G Brg Wid = - Min Req = - E Brg Wid = - Min Req = - Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 11 -451

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Hangers / Ties**  
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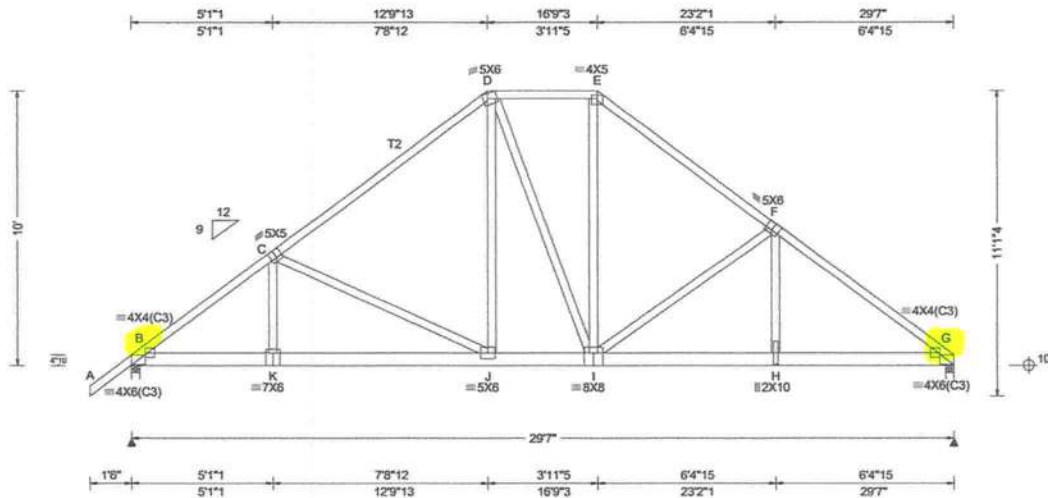


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**2 Complete Trusses Required**



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.37 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.120 J 999 240 VERT(CL): 0.242 J 999 180 HORZ(LL): 0.039 C - - HORZ(TL): 0.080 C - - Creep Factor: 2.0 Max TC CSI: 0.762 Max BC CSI: 0.553 Max Web CSI: 0.723  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw /U /RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>5505</td> <td>-</td> <td>-</td> <td>- /1324 -</td> </tr> <tr> <td>G</td> <td>6434</td> <td>-</td> <td>-</td> <td>- /1239 -</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 2.3 (Truss) G Brg Wid = 3.5 Min Req = 2.7 (Truss) Bearings B &amp; G are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b>  <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1055</td> <td>-4370</td> <td></td> </tr> <tr> <td>C - D</td> <td>794</td> <td>-3190</td> <td></td> </tr> <tr> <td>D - E</td> <td>590</td> <td>-2478</td> <td></td> </tr> <tr> <td>E - F</td> <td>755</td> <td>-3134</td> <td></td> </tr> <tr> <td>F - G</td> <td>927</td> <td>-4278</td> <td></td> </tr> </tbody> </table>  <b>Maximum Bot Chord Forces Per Ply (lbs)</b>  <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - K</td> <td>3459</td> <td>-832</td> <td></td> </tr> <tr> <td>K - J</td> <td>3444</td> <td>-829</td> <td></td> </tr> <tr> <td>J - I</td> <td>2491</td> <td>-613</td> <td></td> </tr> <tr> <td>I - H</td> <td>3367</td> <td>-724</td> <td></td> </tr> <tr> <td>H - G</td> <td>3381</td> <td>-725</td> <td></td> </tr> </tbody> </table>  <b>Maximum Web Forces Per Ply (lbs)</b>  <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Webs</th> <th colspan="2">Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>K - C</td> <td>1001</td> <td>-197</td> <td></td> </tr> <tr> <td>C - J</td> <td>242</td> <td>-1066</td> <td></td> </tr> <tr> <td>D - J</td> <td>1706</td> <td>-447</td> <td></td> </tr> <tr> <td>I - E</td> <td>1692</td> <td>-385</td> <td></td> </tr> <tr> <td>I - F</td> <td>174</td> <td>-1139</td> <td></td> </tr> <tr> <td>F - H</td> <td>1154</td> <td>-133</td> <td></td> </tr> </tbody> </table> </p>	Loc	Gravity		Non-Gravity		R+	/R-	/Rh	/Rw /U /RL	B	5505	-	-	- /1324 -	G	6434	-	-	- /1239 -	Chords		Tens. Comp.		B - C	1055	-4370		C - D	794	-3190		D - E	590	-2478		E - F	755	-3134		F - G	927	-4278		Chords		Tens. Comp.		B - K	3459	-832		K - J	3444	-829		J - I	2491	-613		I - H	3367	-724		H - G	3381	-725		Webs		Tens. Comp.		K - C	1001	-197		C - J	242	-1066		D - J	1706	-447		I - E	1692	-385		I - F	174	-1139		F - H	1154	-133	
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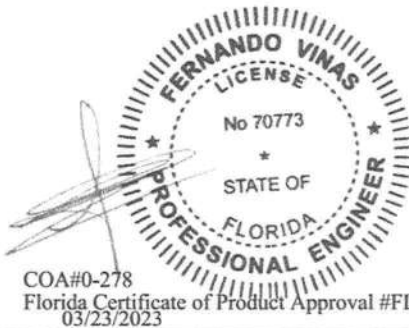
**Lumber**  
Top chord: 2x4 SP #2; T2 2x4 SP M-31;  
Bot chord: 2x6 SP 2400F-2.0E;  
Webs: 2x4 SP #3;

**Nailnote**  
Nail Schedule: 0.131"x3", min. nails  
Top Chord: 1 Row @12.00" o.c.  
Bot Chord: 1 Row @ 5.00" o.c.  
Webs : 1 Row @ 4" o.c.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC: From 65 plf at -1.50 to 65 plf at 29.58  
BC: From 5 plf at -1.50 to 5 plf at 0.00  
BC: From 20 plf at 0.00 to 20 plf at 29.58  
BC: 1051 lb Conc. Load at 5.06  
BC: 615 lb Conc. Load at 7.06, 9.06, 11.06, 13.06  
14.19, 15.19, 23.19, 25.19, 27.19, 29.19  
BC: 738 lb Conc. Load at 17.19  
BC: 697 lb Conc. Load at 19.19  
BC: 680 lb Conc. Load at 21.19

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads and reactions based on MWFRS.  
Wind loading based on both gable and hip roof types.



COA#0-278  
Florida Certificate of Product Approval #FL1999  
03/23/2023

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

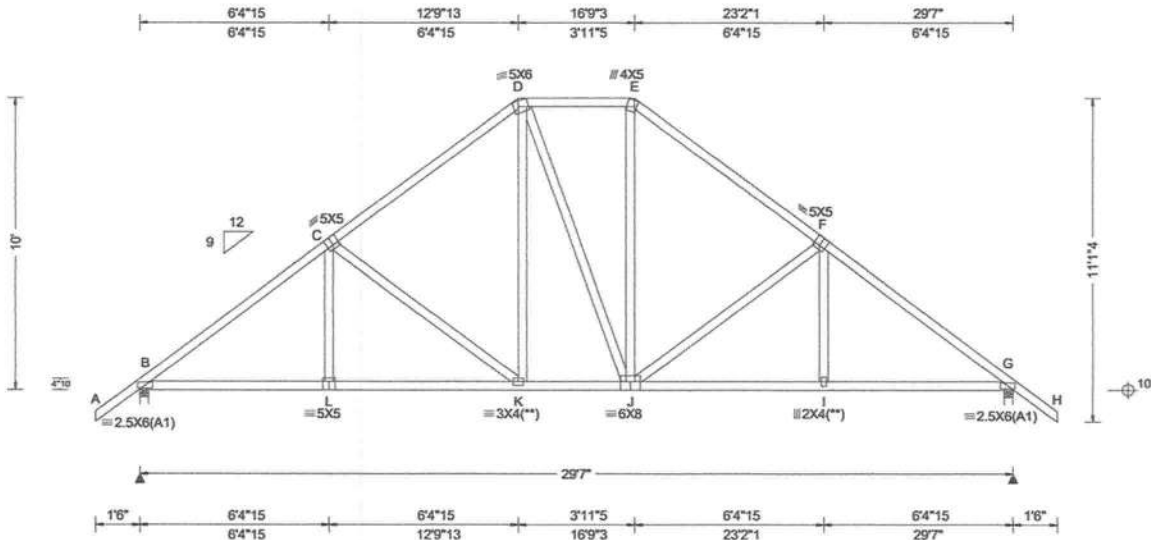
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155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



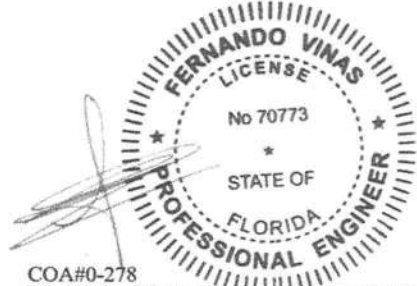
<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Def/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.37 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.054 K 999 240 VERT(CL): 0.114 K 999 180 HORZ(LL): 0.028 G - - HORZ(TL): 0.059 G - - Creep Factor: 2.0 Max TC CSI: 0.457 Max BC CSI: 0.491 Max Web CSI: 0.578  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1362 /- /- /848 /- /342 G 1361 /- /- /848 /- /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) G Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings B & G are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 194 -1735 E - F 220 -1292 C - D 222 -1298 F - G 193 -1735 D - E 238 -933

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

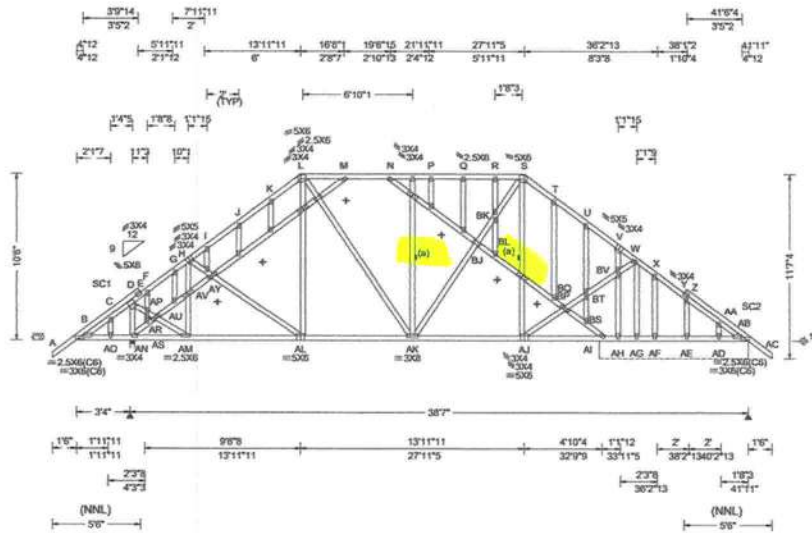
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.68 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.19 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.058 K 999 240 VERT(CL): 0.122 K 999 180 HORZ(LL): 0.037 K - - HORZ(TL): 0.081 K - - Creep Factor: 2.0 Max TC CSI: 0.305 Max BC CSI: 0.467 Max Web CSI: 0.532  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL AN 1714 /- /- /1087 /- /152 AB*222 /- /- /129 /- /- Wind reactions based on MWFRS AN Brg Wid = 3.5 Min Req = 1.6 (Truss) AB Brg Wid = 111 Min Req = - Bearings AN & AI are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - E 401 -212 M - N 14 -911 D - E 0 -836 N - P 39 -704 E - F 0 -579 P - Q 40 -690 F - G 0 -648 Q - R 39 -693 G - H 0 -595 R - S 39 -693 H - I 0 -831 S - T 100 -570 I - J 67 -963 T - U 41 -596 J - K 99 -908 U - V 0 -592 K - L 131 -849 V - W 0 -532 L - M 96 -751
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**

All plates are 2X4 except as noted.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**

In lieu of structural panels use purlins to brace TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

+ MEMBER TO BE Laterally Braced For HORIZONTAL WIND LOADS.

**Additional Notes**

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

**Maximum Bot Chord Forces Per Ply (lbs)**

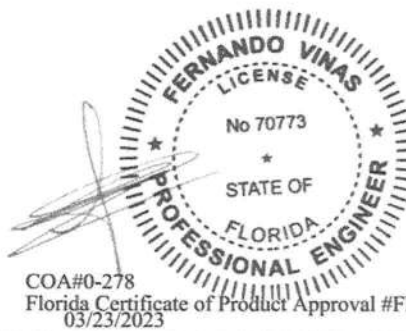
Chords	Tens.Comp.	Chords	Tens. Comp.
B -AO	354 -449	AL-AK	858 0
AO-AN	354 -449	AK-AJ	742 0
AM-AL	836 0		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
D -AP	1114 -67	BL-BP	0 -407
AN-AR	118 -595	AJ-BQ	638 0
AP-AS	1072 -41	BP-BQ	0 -419
AR-AS	95 -540	BQ-BS	17 -477
AS-AU	49 -461	BQ-BT	712 0
AS-AM	1044 -4	BS-AI	50 -519
AU-AY	47 -445	BT-BV	750 0
BJ-BK	420 0	BV -W	722 0
BK -S	477 0		

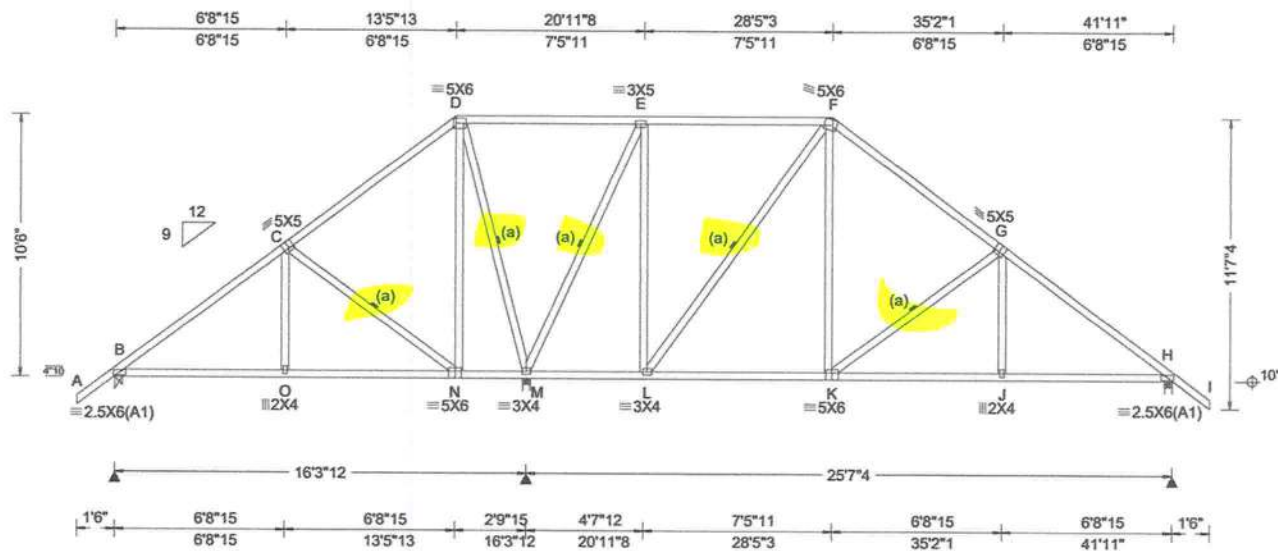
**Maximum Gable Forces Per Ply (lbs)**

Gables	Tens.Comp.	Gables	Tens. Comp.
-AN	105 -1225	AV -H	48 -463
AM-AV	70 -473	W -AG	0 -788



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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.68 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.19 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.034 K 999 240 VERT(CL): 0.071 K 999 180 HORZ(LL): 0.015 H - - HORZ(TL): 0.032 H - - Creep Factor: 2.0 Max TC CSI: 0.854 Max BC CSI: 0.479 Max Web CSI: 0.819 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL					
				B 671 /- /- /418 /27 /378 M 2090 /- /- /1245 /- /- H 1096 /- /- /785 /6 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 2.1 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, M, & H are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.					

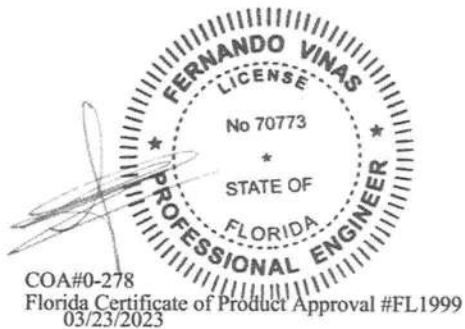
**Lumber**  
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 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

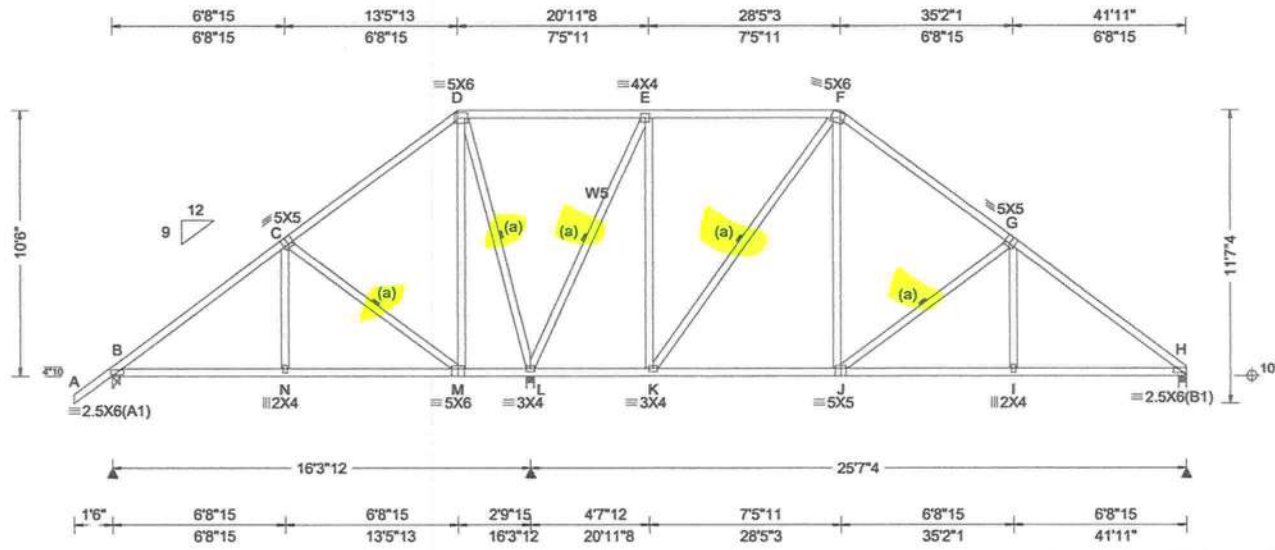
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

<b>Maximum Bot Chord Forces Per Ply (lbs)</b>			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	472 -255	K - J	938 0
O - N	471 -256	J - H	940 0
L - K	561 0		
<b>Maximum Web Forces Per Ply (lbs)</b>			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - N	250 -548	E - L	624 -1
D - N	442 -97	L - F	83 -588
D - M	48 -883	F - K	512 -75
M - E	0 -1270	K - G	244 -479



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.19 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.039 J 999 240 HORZ(LL): 0.018 H - - HORZ(TL): 0.035 H - - Creep Factor: 2.0 Max TC CSI: 0.866 Max BC CSI: 0.567 Max Web CSI: 0.521 VIEW Ver: 21.02.00.1005.17	<b>Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>649</td> <td>-</td> <td>-</td> <td>1417</td> <td>18</td> <td>1351</td> </tr> <tr> <td>L</td> <td>2341</td> <td>-</td> <td>-</td> <td>11200</td> <td>197</td> <td>-</td> </tr> <tr> <td>H</td> <td>1052</td> <td>-</td> <td>-</td> <td>1670</td> <td>140</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) L Brg Wid = 3.5 Min Req = 2.4 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, L, & H are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>99</td> <td>F - G</td> <td>381</td> </tr> <tr> <td>D - E</td> <td>382</td> <td>G - H</td> <td>955</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	649	-	-	1417	18	1351	L	2341	-	-	11200	197	-	H	1052	-	-	1670	140	-	Chords	Tens.Comp.	Chords	Tens.Comp.	B - C	99	F - G	381	D - E	382	G - H	955
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**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3; W5 2x4 SP M-31;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Loading**  
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

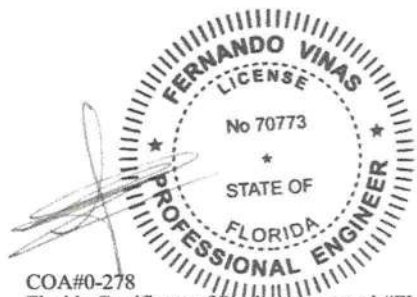
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens.Comp.
B - N	447	J - I	1036
N - M	445	I - H	1037
K - J	657		0

**Maximum Web Forces Per Ply (lbs)**

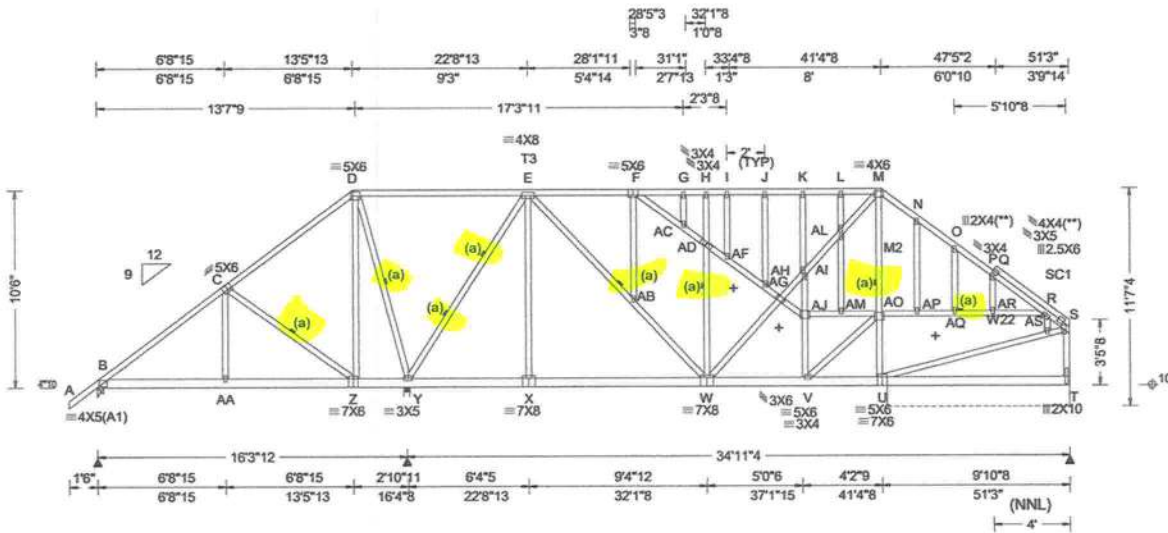
Webs	Tens.Comp.	Webs	Tens. Comp.
C - M	252	E - K	808
D - M	443	K - F	146
D - L	255	F - J	562
L - E	483	J - G	234



COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.20 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.239 O 484 240 VERT(CL): 0.578 O 199 180 HORZ(LL): -0.163 O - - HORZ(TL): 0.396 O - - Creep Factor: 2.0 Max TC CSI: 0.752 Max BC CSI: 0.159 Max Web CSI: 0.874 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b>																																																																			
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>814</td> <td>-</td> <td>-</td> <td>/579</td> <td>/9</td> <td>/334</td> </tr> <tr> <td>Y</td> <td>2369</td> <td>-</td> <td>-</td> <td>/1528</td> <td>-</td> <td>-</td> </tr> <tr> <td>T*</td> <td>203</td> <td>-</td> <td>/0</td> <td>/133</td> <td>-</td> <td>-</td> </tr> </tbody> </table>		Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	814	-	-	/579	/9	/334	Y	2369	-	-	/1528	-	-	T*	203	-	/0	/133	-	-	Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) Y Brg Wid = 3.5 Min Req = 1.6 (Truss) T Brg Wid = 115 Min Req = - Bearings B, Y, & U are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>128 -792</td> <td>K - L</td> <td>208 -674</td> </tr> <tr> <td>E - F</td> <td>163 -1026</td> <td>L - M</td> <td>211 -684</td> </tr> <tr> <td>F - G</td> <td>209 -675</td> <td>O - P</td> <td>189 -392</td> </tr> <tr> <td>G - H</td> <td>208 -676</td> <td>P - Q</td> <td>84 -485</td> </tr> <tr> <td>H - I</td> <td>207 -673</td> <td>Q - R</td> <td>71 -452</td> </tr> <tr> <td>I - J</td> <td>207 -672</td> <td>R - S</td> <td>387 -761</td> </tr> <tr> <td>J - K</td> <td>207 -675</td> <td></td> <td></td> </tr> </tbody> </table>				Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	128 -792	K - L	208 -674	E - F	163 -1026	L - M	211 -684	F - G	209 -675	O - P	189 -392	G - H	208 -676	P - Q	84 -485	H - I	207 -673	Q - R	71 -452	I - J	207 -672	R - S	387 -761
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**Lumber**  
 Top chord: 2x4 SP #2; T3 2x4 SP M-31;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3; W22 2x4 SP M-31; M2 2x4 SP #2;  
 Stack Chord: SC1 2x4 SP #2;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

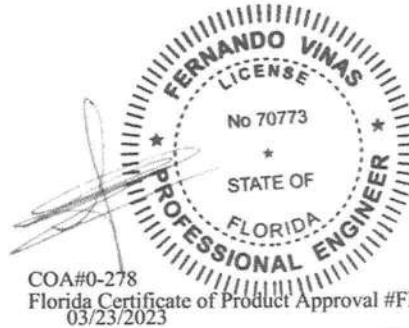
**Plating Notes**  
 All plates are 2X4 except as noted.  
 (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.  
 + MEMBER TO BE Laterally Braced For HORIZONTAL WIND LOADS.

**Additional Notes**  
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B-AA	556 -196	X-W	630 0
AA-Z	554 -197	W-V	916 -21
Y-X	630 0	V-U	694 -377

**Maximum Web Forces Per Ply (lbs)**

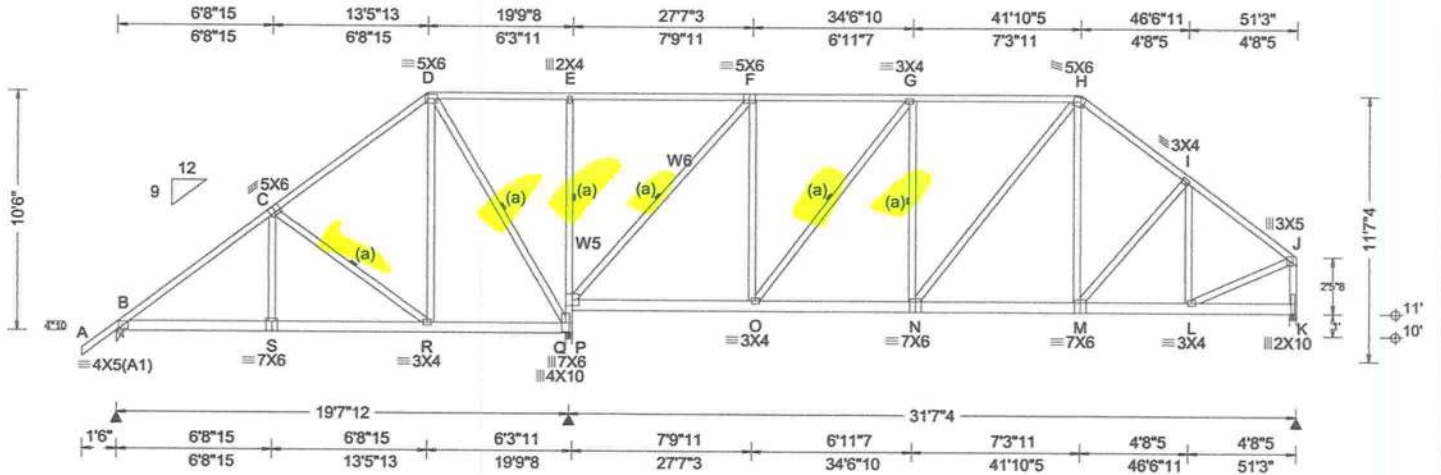
Webs	Tens.Comp.	Webs	Tens. Comp.
C-Z	260 -637	AI-AL	600 0
D-Y	69 -920	AJ-AM	51 -648
Y-E	0 -1547	V-AO	447 -92
E-AB	599 -102	AL-M	574 0
AB-W	599 -103	AM-AO	51 -650
F-AC	0 -431	AO-AP	395 -410
AC-AD	0 -460	U-AS	693 -393
AD-AF	0 -432	AP-AQ	393 -407
AF-AG	0 -409	AQ-AR	390 -403
AG-AH	0 -531	AR-R	388 -400
AH-AI	413 0	AS-S	716 -404
AH-AJ	15 -672		

**Maximum Gable Forces Per Ply (lbs)**

Gables	Tens.Comp.	Gables	Tens. Comp.
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.036 G 999 240 VERT(CL): 0.076 G 999 180 HORZ(LL): -0.012 I - - HORZ(TL): 0.026 I - - Creep Factor: 2.0 Max TC CSI: 0.678 Max BC CSI: 0.121 Max Web CSI: 0.521  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL					
				B 894 /- /- /579 /- /320 Q 2290 /- /- /1293 /211 /- K 1309 /- /- /820 /63 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) Q Brg Wid = 3.5 Min Req = 1.9 (Truss) K Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, Q, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.					

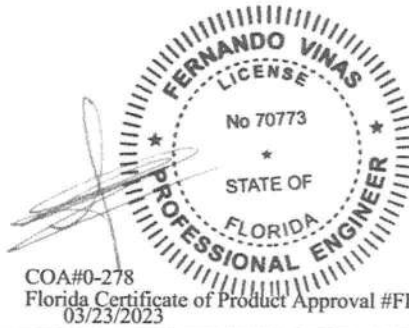
**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3; W5,W6 2x4 SP M-31;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - S	702 -212	O - N	1133 -374
S - R	701 -213	N - M	967 -274
P - O	821 -242	M - L	958 -301

**Maximum Web Forces Per Ply (lbs)**

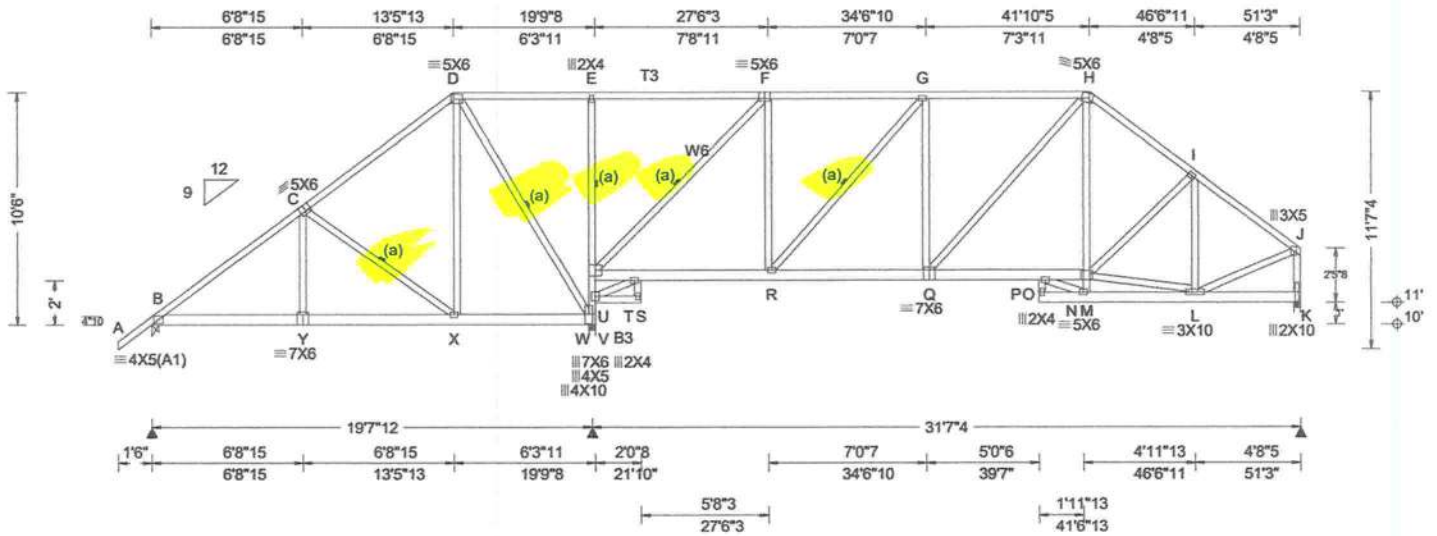
Webs	Tens.Comp.	Webs	Tens. Comp.
C - R	260 -536	P - F	455 -1381
D - R	518 -87	F - O	617 -80
D - Q	142 -697	O - G	222 -516
Q - P	661 -1643	L - J	1023 -314
E - P	396 -517	J - K	444 -1263

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist > 2h C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.046 P 999 240 VERT(CL): 0.097 P 999 180 HORZ(LL): -0.013 I - - HORZ(TL): 0.027 I - - Creep Factor: 2.0 Max TC CSI: 0.659 Max BC CSI: 0.121 Max Web CSI: 0.662  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 886 /- /- /570 /- /320 W 2296 /- /- /1307 /198 /- K 1307 /- /- /815 /68 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) W Brg Wid = 3.5 Min Req = 1.9 (Truss) K Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, W, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 276 -973 G - H 703 -1271 C - D 296 -487 H - I 647 -1414 F - G 558 -912 I - J 471 -1272
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**Lumber**  
Top chord: 2x4 SP #2; T3 2x4 SP M-31;  
Bot chord: 2x6 SP 2400F-2.0E; B3 2x4 SP #2;  
Webs: 2x4 SP #3; W6 2x4 SP M-31;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

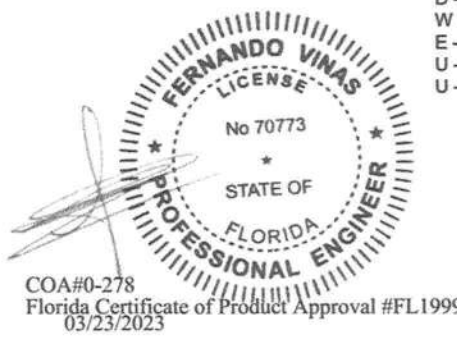
**Plating Notes**  
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**Purlins**  
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**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp. Chords Tens. Comp.  
B - Y 692 -225 R - Q 1272 -436  
Y - X 690 -226 Q - O 1071 -305  
U - T 1045 -324 O - M 865 -194  
T - R 894 -283

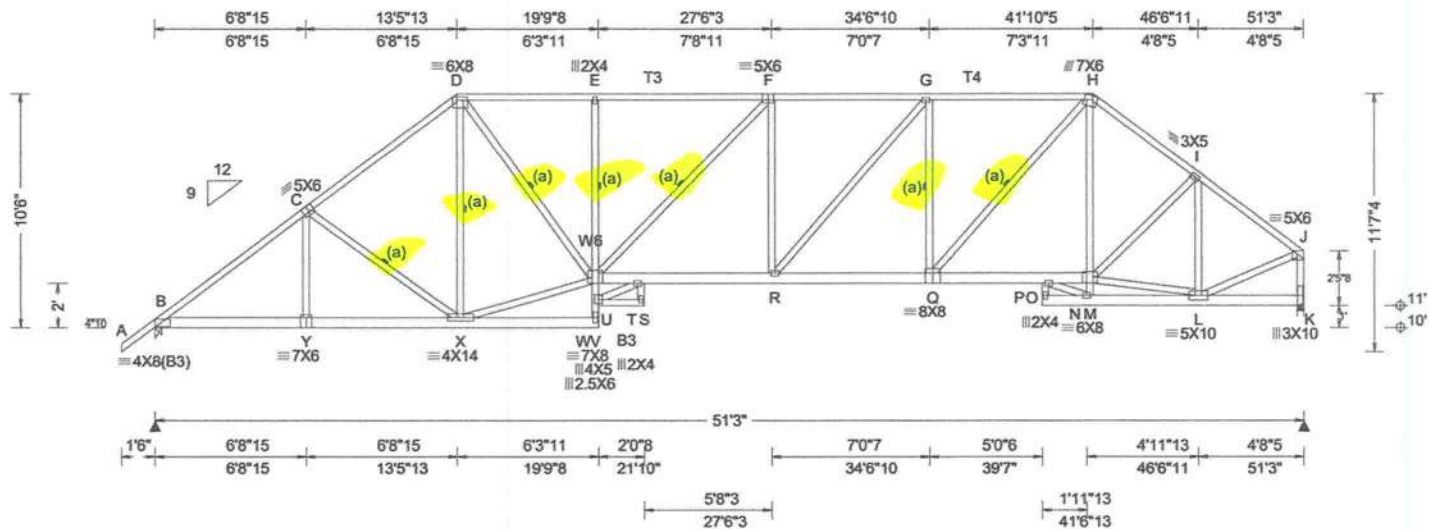
**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
C - X 260 -536 F - R 598 -67  
D - X 519 -85 R - G 237 -577  
D - W 129 -702 M - L 757 -191  
W - V 654 -1645 I - L 234 -495  
E - U 394 -515 L - J 1025 -321  
U - V 644 -1540 J - K 450 -1259  
U - F 458 -1419



**Additional Notes**  
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
Laterally brace chord member above/below filler @ 24" O.C. or as specified, including a brace at chord ends.

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>																																																
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.245 S 999 240 VERT(CL): 0.470 S 999 180 HORIZ(LL): 0.089 K - - HORIZ(TL): 0.171 K - - Creep Factor: 2.0 Max TC CSI: 0.713 Max BC CSI: 0.263 Max Web CSI: 0.859  VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rw</th> <th>/U /RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>2438</td> <td>-</td> <td>-</td> <td>/1382 /118 /320</td> </tr> <tr> <td>K</td> <td>2398</td> <td>-</td> <td>-</td> <td>/1199 /78 -</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 2.0 (Truss) K Brg Wid = 3.5 Min Req = 2.0 (Truss) Bearings B &amp; K are a rigid surface. Members not listed have forces less than 375#</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th>Tens.</th> <th>Comp.</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1009</td> <td>-3485</td> <td>F - G</td> <td>1312 -3891</td> </tr> <tr> <td>C - D</td> <td>1047</td> <td>-3050</td> <td>G - H</td> <td>1217 -3442</td> </tr> <tr> <td>D - E</td> <td>1298</td> <td>-3612</td> <td>H - I</td> <td>1013 -2988</td> </tr> <tr> <td>E - F</td> <td>1297</td> <td>-3613</td> <td>I - J</td> <td>735 -2413</td> </tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	/R-	/Rw	/U /RL	B	2438	-	-	/1382 /118 /320	K	2398	-	-	/1199 /78 -	Chords	Tens.Comp.		Tens. Comp.		Tens.	Comp.	Tens.	Comp.	B - C	1009	-3485	F - G	1312 -3891	C - D	1047	-3050	G - H	1217 -3442	D - E	1298	-3612	H - I	1013 -2988	E - F	1297	-3613	I - J	735 -2413
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**Lumber**  
Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;  
Bot chord: 2x6 SP 2400f-2.0E; B3 2x4 SP #2;  
Webs: 2x4 SP #3; W6 2x4 SP M-31;

Laterally brace chord member above/below filler @ 24" O.C. or as specified, including a brace at chord ends.

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 3X4 except as noted.

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
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**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Y	2694 -807	R - Q	3477 -964
Y - X	2693 -808	Q - O	2327 -595
U - T	3545 -856	O - M	1930 -440
T - R	3898 -1048	N - L	424 -166

**Maximum Web Forces Per Ply (lbs)**

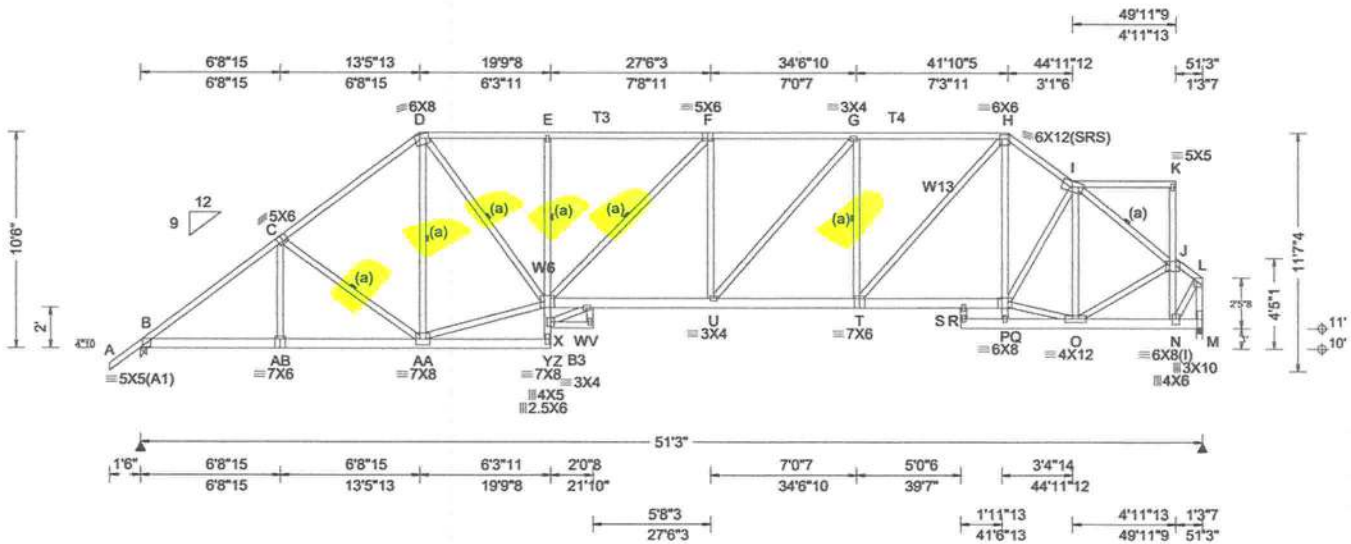
Webs	Tens.Comp.	Webs	Tens. Comp.
C - X	223 -452	Q - H	1684 -548
D - U	2102 -642	O - N	480 -174
X - U	2254 -565	M - I	607 -119
E - U	359 -448	M - L	1485 -360
U - F	150 -410	I - L	368 -1073
V - T	411 -224	L - J	2017 -551
R - G	649 -228	J - K	701 -2341
G - Q	465 -980		



COA#0-278  
Florida Certificate of Product Approval #FL1999  
03/23/2023

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**Loading Criteria (psf)**

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

**Wind Criteria**

Wind Std: ASCE 7-16  
 Speed: 130 mph  
 Enclosure: Closed  
 Risk Category: II  
 EXP: C Kzt: NA  
 Mean Height: 15.00 ft  
 TC DL: 5.0 psf  
 BC DL: 5.0 psf  
 MWFRS Parallel Dist: > 2h  
 C&C Dist a: 5.12 ft  
 Loc. from endwall: not in 13.00 ft  
 GCpl: 0.18  
 Wind Duration: 1.60

**Snow Criteria (Pg,Pf in PSF)**

Pg: NA Ct: NA CAT: NA  
 Pf: NA Ce: NA  
 Lu: NA Cs: NA  
 Snow Duration: NA

**Building Code:**  
 FBC 7th Ed. 2020 Res.  
 TPI Std: 2014  
 Rep Fac: Yes  
 FT/RT: 20(0)/10(0)  
 Plate Type(s):  
 WAVE

**Defl/CSI Criteria**

PP Deflection in loc L/defl L/#  
 VERT(LL): 0.195 V 999 240  
 VERT(CL): 0.412 V 999 180  
 HORZ(LL): 0.075 M - -  
 HORZ(TL): 0.159 M - -  
 Creep Factor: 2.0  
 Max TC CSI: 0.650  
 Max BC CSI: 0.204  
 Max Web CSI: 0.817

VIEW Ver: 21.02.00.1005.17

**Maximum Reactions (lbs)**

Loc	Gravity		Non-Gravity		
	R+	R-	/Rh	/Rw	/U / RL
B	2291	-	-	/1386	/119 /371
M	2170	-	-	/1085	/218 -

Wind reactions based on MWFRS  
 B Brg Wid = 3.5 Min Req = 1.9 (Truss)  
 M Brg Wid = 3.5 Min Req = 1.8 (Truss)  
 Bearings B & M are a rigid surface.  
 Members not listed have forces less than 375#

**Maximum Top Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	888 -3247	F - G	1193 -3426
C - D	932 -2807	G - H	1108 -3022
D - E	1188 -3253	H - I	944 -2654
E - F	1187 -3253	J - L	398 -1136

**Lumber**  
 Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;  
 Bot chord: 2x6 SP 2400F-2.0E; B3 2x4 SP #2;  
 Webs: 2x4 SP #3; W6,W13 2x4 SP M-31;

Laterally brace chord member above/below filler @ 24" O.C. or as specified, including a brace at chord ends.

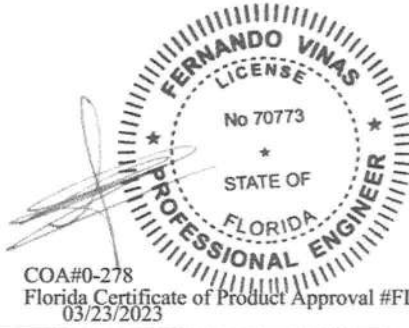
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 2X4 except as noted.  
 (I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B-AB	2504 -852	U - T	3055 -1009
AB-AA	2503 -853	T - R	2094 -686
X - W	3029 -879	R - P	2084 -679
W - U	3436 -1086	O - N	950 -318

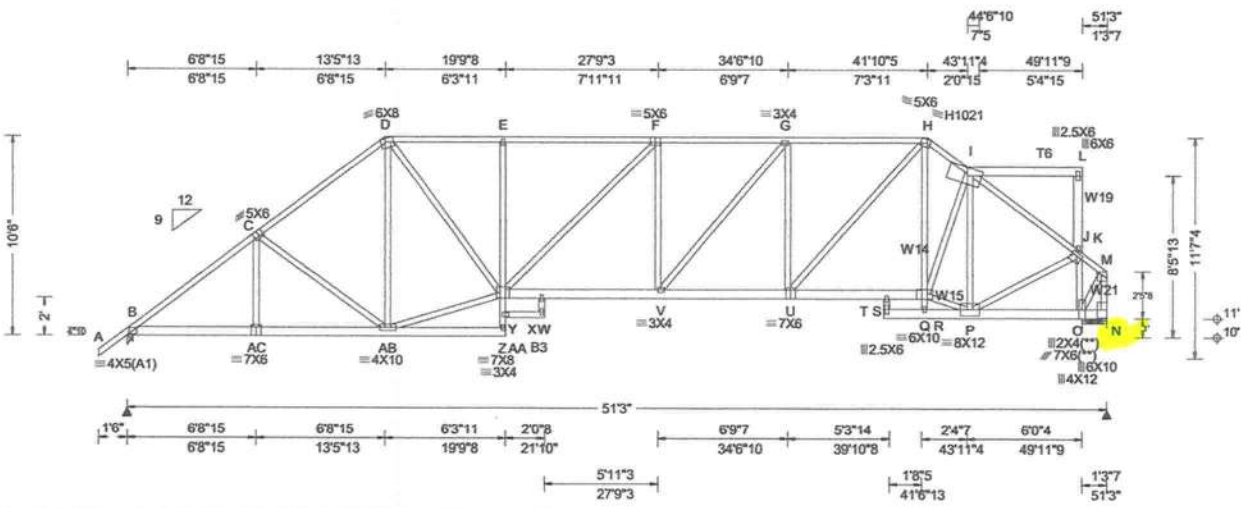
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
C-AA	201 -457	P - I	600 -172
D - X	1829 -615	P - O	1849 -615
AA - X	2024 -622	I - O	373 -935
E - X	333 -450	I - J	785 -2246
Y - W	471 -240	O - J	949 -316
U - G	580 -220	J - N	572 -1490
G - T	428 -932	N - L	1779 -597
T - H	1401 -465	L - M	719 -2126

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2 Complete Trusses Required



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.211 V 999 240 VERT(CL): 0.430 V 999 180 HORZ(LL): 0.086 N - - HORZ(TL): 0.176 N - - Creep Factor: 2.0 Max TC CSI: 0.826 Max BC CSI: 0.271 Max Web CSI: 0.886 VIEW Ver: 21.02.00.1005.17	<b>Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>3192</td> <td>-</td> <td>-</td> <td>-</td> <td>/496</td> <td>-</td> </tr> <tr> <td>N</td> <td>9291</td> <td>-</td> <td>-</td> <td>-</td> <td>/1423</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) N Brg Wid = 15.5 Min Req = 3.8 (Truss) Bearings B & O are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr><td>B - C</td><td>361 -2354</td><td>G - H</td><td>505 -3425</td></tr> <tr><td>C - D</td><td>333 -2149</td><td>H - I</td><td>627 -4248</td></tr> <tr><td>D - E</td><td>411 -2748</td><td>I - J</td><td>683 -4629</td></tr> <tr><td>E - F</td><td>412 -2751</td><td>J - K</td><td>786 -5125</td></tr> <tr><td>F - G</td><td>477 -3234</td><td>K - M</td><td>375 -2489</td></tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	R-	/Rh	/Rw	/U	/RL	B	3192	-	-	-	/496	-	N	9291	-	-	-	/1423	-	Chords	Tens.Comp.	Chords	Tens.Comp.	B - C	361 -2354	G - H	505 -3425	C - D	333 -2149	H - I	627 -4248	D - E	411 -2748	I - J	683 -4629	E - F	412 -2751	J - K	786 -5125	F - G	477 -3234	K - M	375 -2489
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**Lumber**  
 Top chord: 2x4 SP #2; T6 2x6 SP 2400f-2.0E;  
 Bot chord: 2x6 SP 2400f-2.0E; B3 2x4 SP #2;  
 Webs: 2x4 SP #3; W14 2x4 SP M-31; W15, W21 2x4 SP #2; W19 2x6 SP 2400f-2.0E;

**Nailnote**  
 Nail Schedule: 0.131"x3", min. nails  
 Top Chord: 1 Row @ 4.75" o.c.  
 Bot Chord: 1 Row @ 12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 65 plf at -1.50 to 65 plf at 43.94	TC: From 665 plf at 43.94 to 665 plf at 49.96
TC: From 65 plf at 49.96 to 65 plf at 51.25	BC: From 5 plf at -1.50 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 51.25	BC: 4408 lb Conc. Load at 44.33

**Plating Notes**  
 All plates are 2X4 except as noted.  
 (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

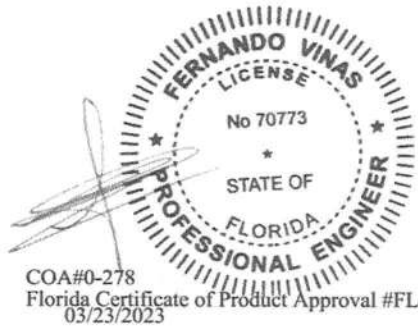
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 WIND LOAD CASE MODIFIED!  
 Laterally brace chord member above/below filler @ 24"O.C. or as specified, including a brace at chord ends.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens.Comp.
B - AC	1835 -273	V - U	3424 -507
AC - AB	1835 -273	U - S	3369 -493
Y - X	3195 -469	S - Q	3353 -489
X - V	3226 -478	P - O	2213 -337

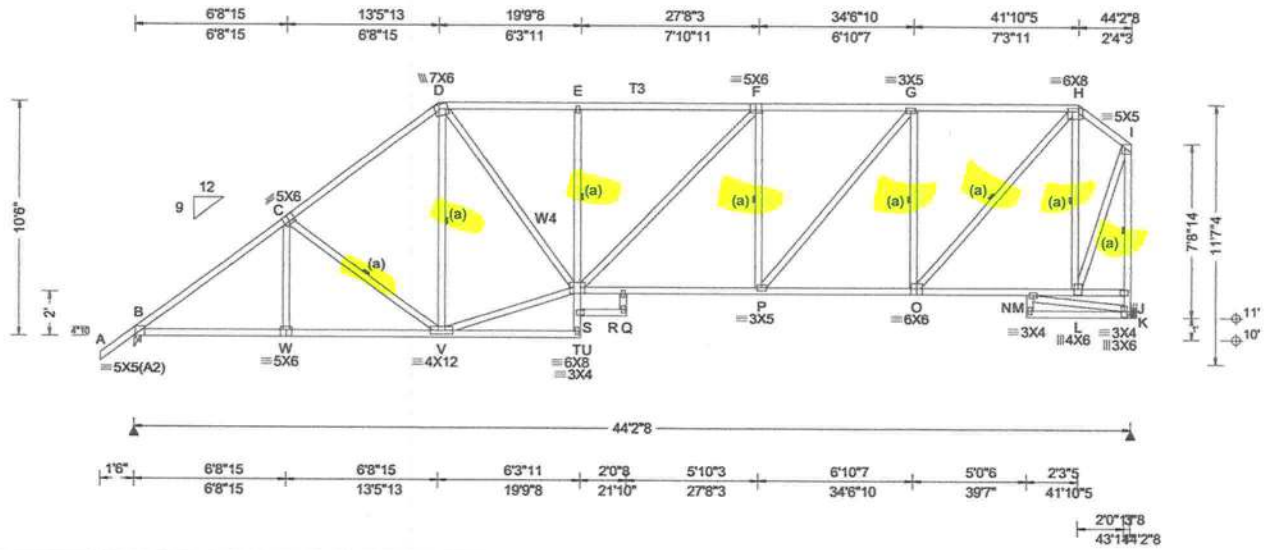
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens.Comp.
D - Y	1779 -271	P - I	86 -1018
AB - Y	1720 -252	P - K	1612 -221
Y - F	93 -672	J - L	191 -921
U - H	504 -57	K - O	508 -3085
Q - H	2328 -303	O - M	3766 -568
Q - P	3924 -571	M - N	700 -4650
Q - I	116 -826		



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 4.42 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.200 E 999 240 VERT(CL): 0.380 E 999 180 HORZ(LL): 0.113 N - - HORZ(TL): 0.215 N - - Creep Factor: 2.0 Max TC CSI: 0.756 Max BC CSI: 0.894 Max Web CSI: 0.828  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ / R- / Rh / Rw / U / RL B 2113 /- /- /1236 /116 /290 J 2199 /- /- /955 /227 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 2.5 (Truss) J Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.
				B - C 716 -2932 F - G 850 -2750 C - D 762 -2492 G - H 649 -2039 D - E 971 -2844 H - I 242 -801 E - F 971 -2846

**Lumber**  
 Top chord: 2x4 SP #2; T3 2x4 SP M-31;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3; W4 2x4 SP M-31;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

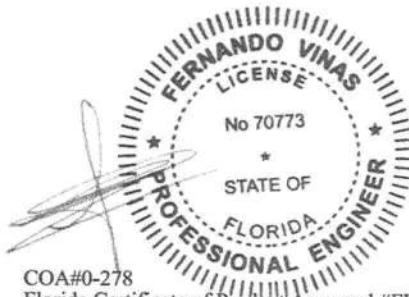
**Plating Notes**  
 All plates are 2X4 except as noted.

**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Loading**  
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.  
 Laterally brace chord member above/below filler @ 24" O.C. or as specified, including a brace at chord ends.



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**Maximum Bot Chord Forces Per Ply (lbs)**

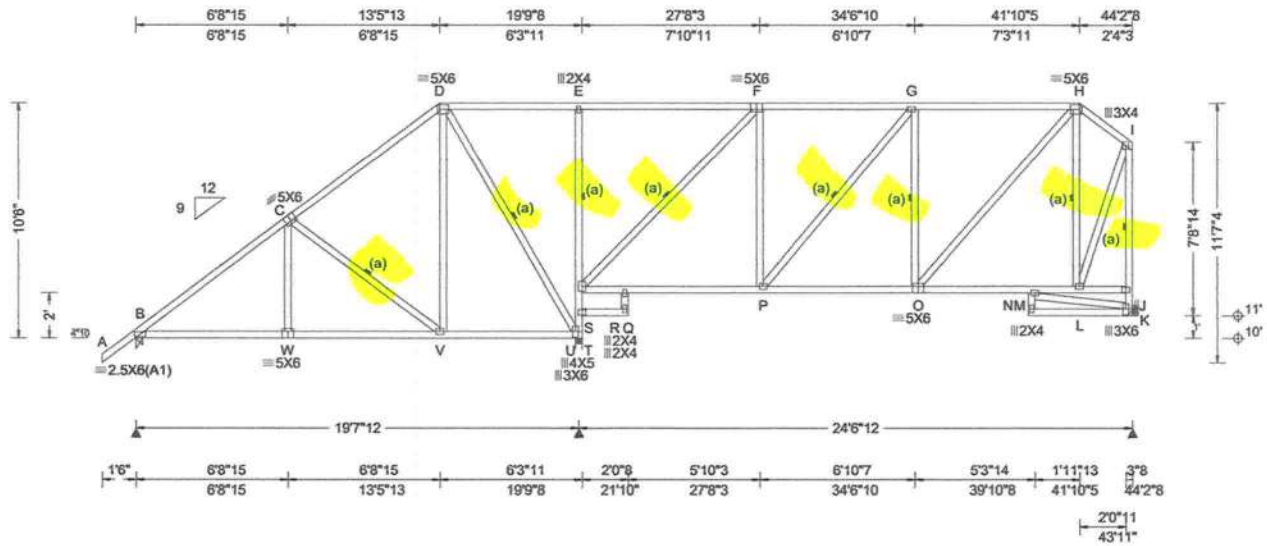
Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	2239 -765	P - O	2085 -605
W - V	2238 -766	O - M	640 -186
S - R	2757 -757	M - L	690 -187
R - P	2770 -801		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
C - V	200 -442	G - O	525 -1309
D - S	1580 -487	O - H	2125 -609
V - S	1969 -616	H - L	563 -1502
E - S	340 -463	L - I	1834 -540
F - P	363 -554	K - I	634 -2205
P - G	1064 -340	K - J	653 -2143

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 4.42 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.050 R 999 240 VERT(CL): 0.164 R 999 180 HORZ(LL): -0.009 R - - HORZ(TL): 0.029 N - - Creep Factor: 2.0 Max TC CSI: 0.680 Max BC CSI: 0.625 Max Web CSI: 0.682  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 912 /- /- /616 /- /290 U 1948 /- /- /1106 /248 /- J 1021 /- /- /538 /118 /- <b>Wind reactions based on MWFRS</b> B Brg Wid = 3.5 Min Req = 1.5 (Truss) U Brg Wid = 3.5 Min Req = 2.3 (Truss) J Brg Wid = - Min Req = - Bearings B & U are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens.Comp. B - C 104 -1008 F - G 288 -687 C - D 134 -529 G - H 341 -752
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

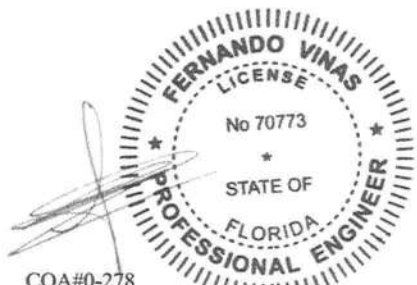
**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 3X4 except as noted.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
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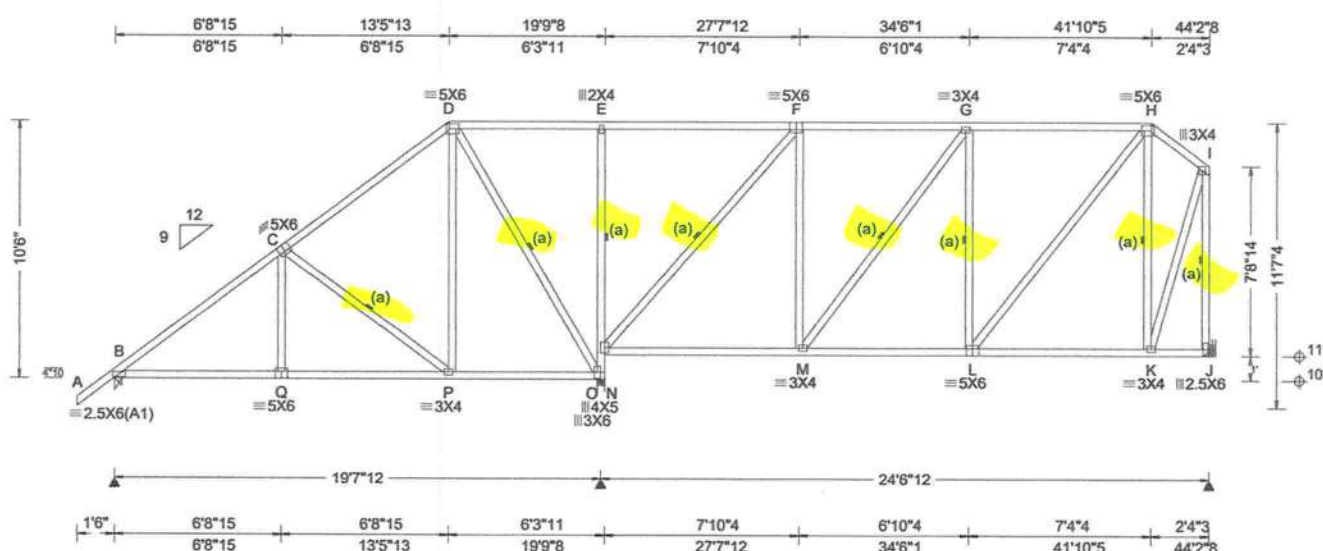
Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	715 -282	R - P	678 -226
W - V	714 -283	P - O	765 -287
S - R	712 -209		

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - V	237 -527	F - P	384 0
D - V	518 -74	G - O	261 -379
D - U	208 -668	O - H	699 -256
U - T	530 -1332	H - L	354 -691
E - S	370 -521	L - I	835 -320
S - T	549 -1293	K - I	370 -1001
S - F	322 -1025	K - J	388 -972

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 4.42 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.030 G 999 240 VERT(CL): 0.064 G 999 180 HORZ(LL): -0.009 O - - HORZ(TL): 0.020 O - - Creep Factor: 2.0 Max TC CSI: 0.682 Max BC CSI: 0.625 Max Web CSI: 0.741  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b>																																																																																							
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**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**

(J) Hanger Support Required, by others

**Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

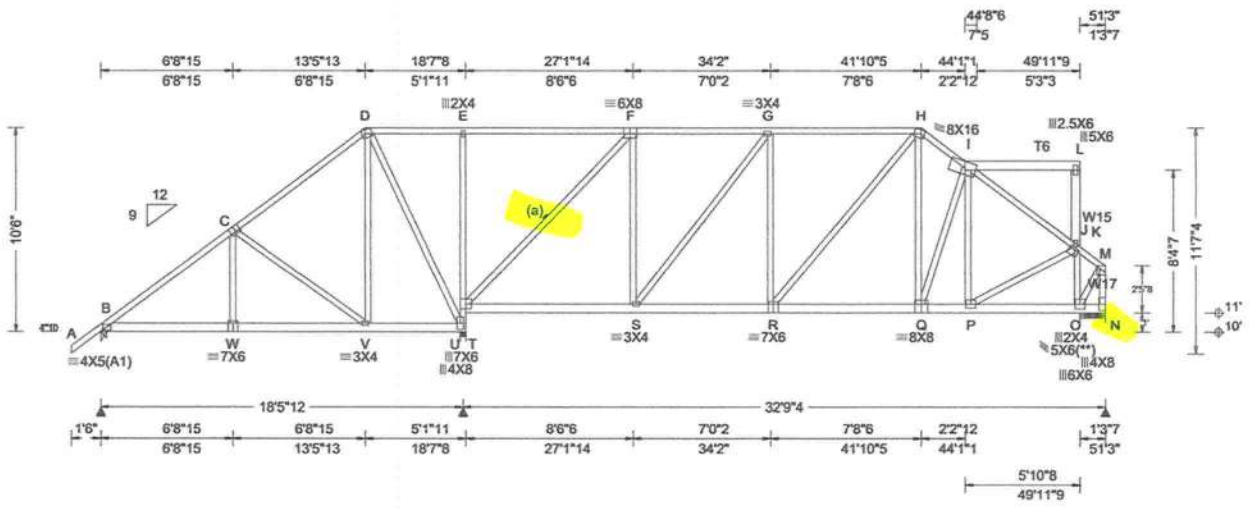


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2 Complete Trusses Required



**Loading Criteria (psf)**

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

**Wind Criteria**

Wind Std: ASCE 7-16  
 Speed: 130 mph  
 Enclosure: Closed  
 Risk Category: II  
 EXP: C Kzt: NA  
 Mean Height: 15.00 ft  
 TC DL: 5.0 psf  
 BCDL: 5.0 psf  
 MWFRS Parallel Dist: 0 to h/2  
 C&C Dist a: 5.12 ft  
 Loc. from endwall: not in 13.00 ft  
 GCpi: 0.18  
 Wind Duration: 1.60

**Snow Criteria (Pg,Pf in PSF)**

Pg: NA Ct: NA CAT: NA  
 Pf: NA Ce: NA  
 Lu: NA Cs: NA  
 Snow Duration: NA

**Building Code:**  
 FBC 7th Ed. 2020 Res.  
 TPI Std: 2014  
 Rep Fac: No  
 FT/RT: 20(0)/10(0)  
 Plate Type(s):  
 WAVE

**Defl/CSI Criteria**

PP Deflection in loc L/defl L/#  
 VERT(LL): 0.074 P 999 240  
 VERT(CL): 0.151 P 999 180  
 HORZ(LL): -0.032 L - -  
 HORZ(TL): 0.065 L - -  
 Creep Factor: 2.0  
 Max TC CSI: 0.584  
 Max BC CSI: 0.222  
 Max Web CSI: 0.988

VIEW Ver: 21.02.00.1005.17

**Maximum Reactions (lbs)**

Loc	Gravity			Non-Gravity		
	R+	R-	/Rh	/Rw	/U	/RL
B	827	-	-	-	143	-
U	3374	-	-	-	517	-
N	6587	-	-	-	1083	-

Wind reactions based on MWFRS  
 B Brg Wid = 3.5 Min Req = 1.5 (Truss)  
 U Brg Wid = 3.5 Min Req = 1.5 (Truss)  
 N Brg Wid = 15.5 Min Req = 2.7 (Truss)  
 Bearings B, U, & O are a rigid surface.  
 Members not listed have forces less than 375#

**Maximum Top Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	75 -439	I - J	481 -3026
F - G	145 -944	J - K	581 -3509
G - H	233 -1492	K - M	284 -1752
H - I	374 -2367		

**Lumber**  
 Top chord: 2x4 SP #2; T6 2x6 SP 2400F-2.0E;  
 Bot chord: 2x6 SP 2400F-2.0E;  
 Webs: 2x4 SP #3; W15 2x6 SP 2400F-2.0E;  
 W17 2x4 SP M-31;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Nailnote**  
 Nail Schedule: 0.131"x3", min. nails  
 Top Chord: 1 Row @ 4.75" o.c.  
 Bot Chord: 1 Row @ 12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

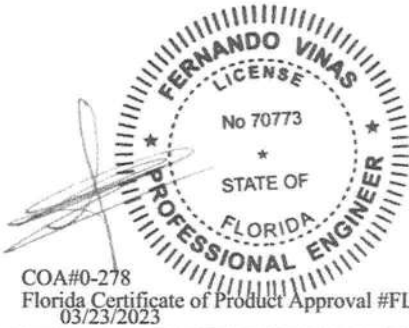
**Additional Notes**  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
 WIND LOAD CASE MODIFIED!

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
T - S	921 -144	Q - P	2326 -366
S - R	1476 -232	P - O	1569 -257
R - Q	1835 -288		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
U - T	232 -1360	Q - I	250 -1561
T - F	218 -1439	I - P	823 -150
F - S	780 -67	P - K	893 -129
S - G	146 -898	J - L	187 -899
G - R	481 -31	K - O	395 -2211
R - H	86 -538	O - M	2668 -432
H - Q	1652 -224	M - N	531 -3277



**Special Loads**  
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
 TC: From 65 plf at -1.50 to 65 plf at 44.09  
 TC: From 665 plf at 44.09 to 665 plf at 49.96  
 TC: From 65 plf at 49.96 to 65 plf at 51.25  
 BC: From 5 plf at -1.50 to 5 plf at 0.00  
 BC: From 20 plf at 0.00 to 20 plf at 51.25  
 BC: 2758 lb Conc. Load at 44.33

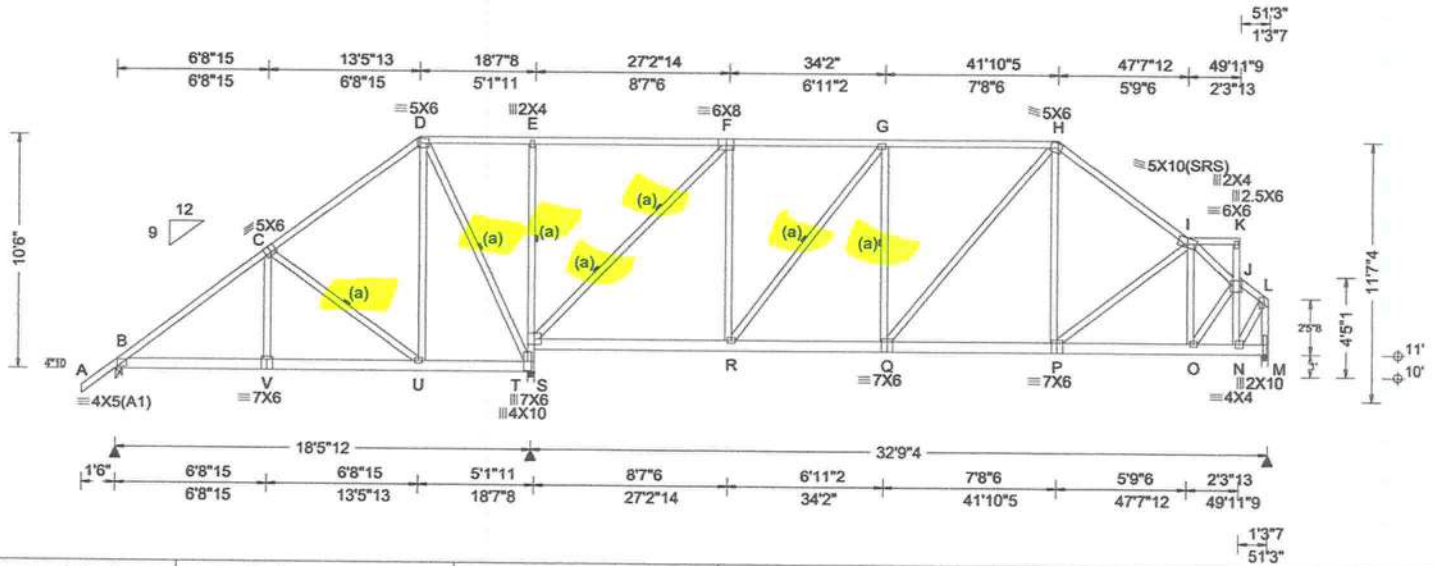
**Plating Notes**  
 All plates are 5X6 except as noted.  
 (\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L# VERT(LL): 0.043 G 999 240 VERT(CL): 0.092 G 999 180 HORZ(LL): -0.013 H - - HORZ(TL): 0.027 H - - Creep Factor: 2.0 Max TC CSI: 0.773 Max BC CSI: 0.142 Max Web CSI: 0.872  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 847 -/- /- /552 -/ /310 T 2285 -/- /- /1293 /220 -/ M 1361 -/- /- /790 /52 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) T Brg Wid = 3.5 Min Req = 1.9 (Truss) M Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, T, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3;

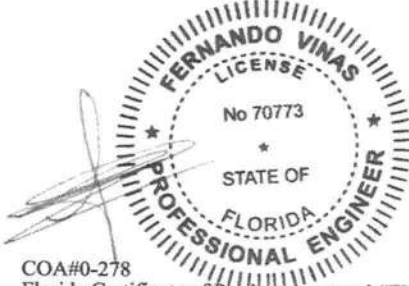
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 3X4 except as noted.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



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**Maximum Bot Chord Forces Per Ply (lbs)**

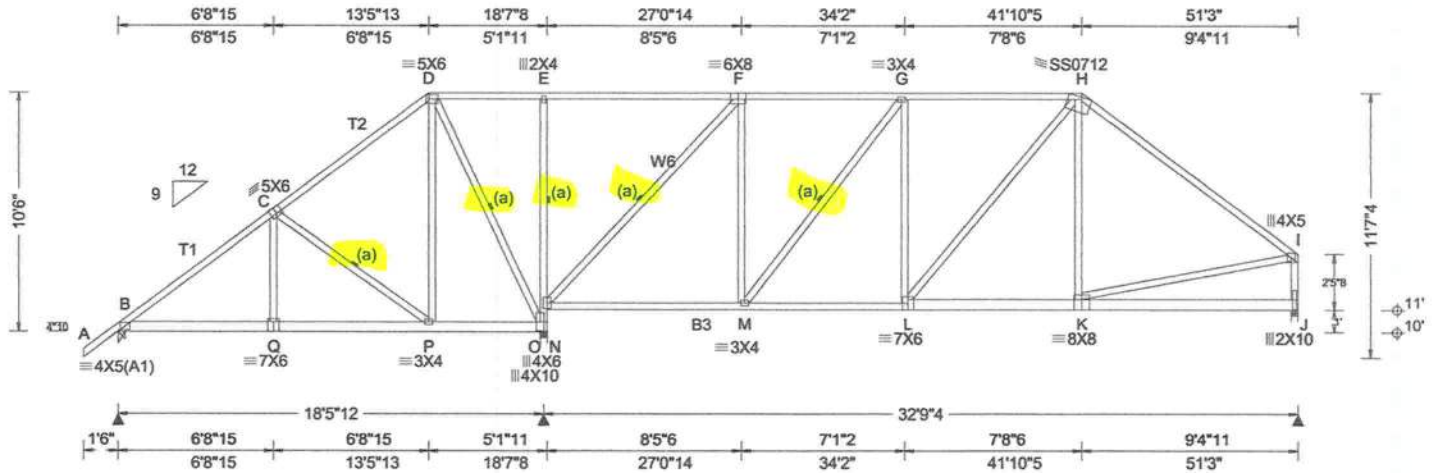
Chords	Tens.Comp.	Chords	Tens. Comp.
B - V	641 -200	Q - P	1030 -307
V - U	640 -201	P - O	930 -297
S - R	936 -280	O - N	588 -192
R - Q	1224 -399		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
C - U	236 -541	R - G	202 -476
D - U	492 -77	I - J	415 -1189
D - T	126 -651	O - J	546 -161
T - S	617 -1666	J - N	323 -946
E - S	369 -516	N - L	1099 -359
S - F	446 -1460	L - M	435 -1318
F - R	615 -60		

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.056 L 999 240 VERT(CL): 0.105 L 999 180 HORZ(LL): -0.016 O - - HORZ(TL): 0.029 O - - Creep Factor: 2.0 Max TC CSI: 0.689 Max BC CSI: 0.374 Max Web CSI: 0.762  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ /R- /Rh Non-Gravity /Rw /U /RL B 845 /- /- /537 /27 /320 O 2719 /- /- /1311 /204 /- J 1562 /- /- /847 /71 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) O Brg Wid = 3.5 Min Req = 2.3 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, O, & J are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.
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**Lumber**  
Top chord: 2x4 SP M-31; T1,T2 2x4 SP #2;  
Bot chord: 2x6 SP 2400f-2.0E; B3 2x4 SP M-31;  
Webs: 2x4 SP #3; W6 2x4 SP M-31;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

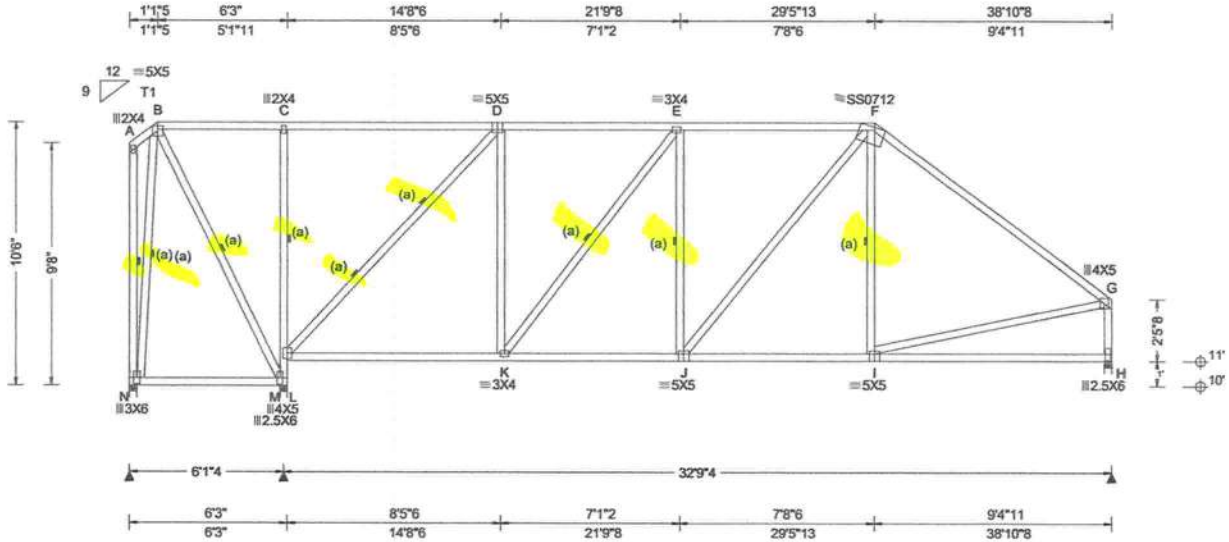
**Additional Notes**  
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.98 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.89 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.047 E 999 240 VERT(CL): 0.100 E 999 180 HORZ(LL): 0.016 M - - HORZ(TL): 0.022 G - - Creep Factor: 2.0 Max TC CSI: 0.614 Max BC CSI: 0.780 Max Web CSI: 0.870  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ /R- /Rh Non-Gravity /Rw /U /RL N 153 /-25 /- /226 /246 /188 M 1853 /- /- /1034 /36 /- H 1370 /- /- /843 /100 /- Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 2.2 (Truss) H Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings N, M, & H are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. D - E 670 -956 F - G 650 -1488 E - F 780 -1222
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**Lumber**  
Top chord: 2x4 SP M-31; T1 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



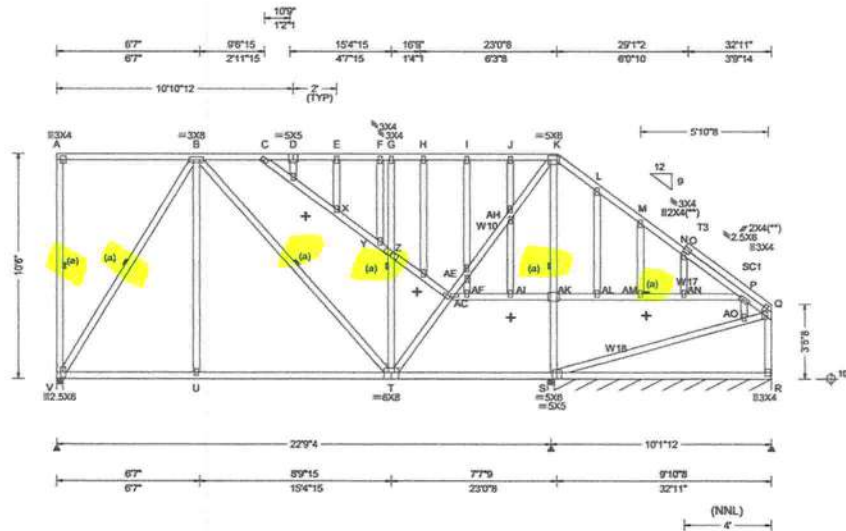
COA#0-278  
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03/23/2023

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp. Chords Tens. Comp.  
L - K 941 -399 J - I 1062 -387  
K - J 1224 -519

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
M - L 884 -1651 K - E 200 -456  
C - L 420 -504 I - G 1017 -370  
L - D 678 -1456 G - H 550 -1288  
D - K 618 -59

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or * = PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.80 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.29 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.199 M 594 240 VERT(CL): 0.466 M 254 180 HORZ(LL): -0.139 M - - HORZ(TL): 0.336 M - - Creep Factor: 2.0 Max TC CSI: 0.644 Max BC CSI: 0.823 Max Web CSI: 0.656  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL V 985 /- /- /492 /233 /189 S - /-576 /- /- /293 /- R* 208 /- /- /134 /2 /- Wind reactions based on MWFRS V Brg Wid = 3.5 Min Req = 1.5 (Truss) S Brg Wid = 3.5 Min Req = 1.5 R Brg Wid = 120 Min Req = - Bearings V, S, & S are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.

**Lumber**  
Top chord: 2x4 SP M-31; T3 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3; W10, W17, W18 2x4 SP M-31;  
Stack Chord: SC1 2x4 SP #2;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 2X4 except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.  
+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.

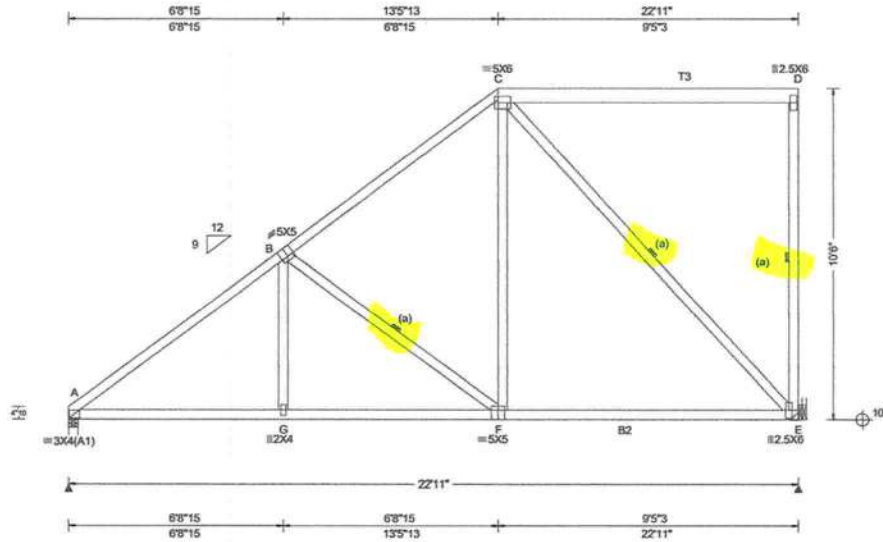
**Additional Notes**  
Negative reaction(s) of -576# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.  
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



<b>Maximum Bot Chord Forces Per Ply (lbs)</b>			
Chords	Tens.Comp.	Chords	Tens. Comp.
V - U	488 -30	T - S	801 -335
U - T	488 -30		
<b>Maximum Web Forces Per Ply (lbs)</b>			
Webs	Tens.Comp.	Webs	Tens. Comp.
V - B	529 -908	AF-AI	463 -606
X - Y	288 -378	AH - K	955 -462
Y - Z	287 -381	AI-AK	463 -606
G - Z	280 -408	AK-AL	462 -606
T - AC	547 -52	S -AO	398 -180
Z - T	315 -424	AL-AM	460 -604
AC-AE	842 -362	AM-AN	459 -602
AC-AF	463 -606	AN - P	459 -602
AE-AH	939 -445	AO - Q	409 -183
<b>Maximum Gable Forces Per Ply (lbs)</b>			
Gables	Tens.Comp.	Gables	Tens. Comp.
AK - K	560 -1144	AK - S	679 -1316

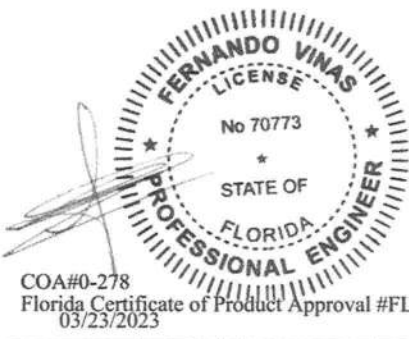
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.033 G 999 240 VERT(CL): 0.063 G 999 180 HORZ(LL): 0.016 B - - HORZ(TL): 0.030 B - - Creep Factor: 2.0 Max TC CSI: 0.646 Max BC CSI: 0.844 Max Web CSI: 0.861  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 1042 /- /- /634 /4 /261 E 1125 /- /- /586 /192 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 247 -1393 B - C 264 -954
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<b>Lumber</b> Top chord: 2x4 SP #2; T3 2x6 SP 2400f-2.0E; Bot chord: 2x4 SP #2; B2 2x4 SP M-31; Webs: 2x4 SP #3;	<b>Bracing</b> (a) Continuous lateral restraint equally spaced on member.	<b>Hangers / Ties</b> (J) Hanger Support Required, by others	<b>Loading</b> Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	<b>Purlins</b> In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	<b>Wind</b> Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	<b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - G 1027 -461 F - E 639 -268 G - F 1026 -462  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. B - F 245 -489 C - E 393 -938 C - F 685 -53
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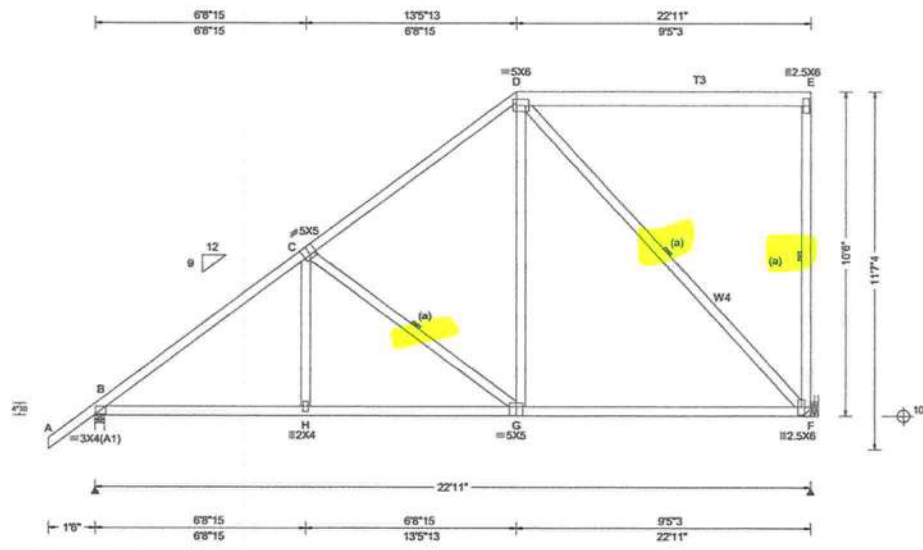


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 North Building, 4th Floor  
 Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.025 H 999 240 VERT(CL): 0.053 H 999 180 HORZ(LL): 0.010 F - - HORZ(TL): 0.020 F - - Creep Factor: 2.0 Max TC CSI: 0.634 Max BC CSI: 0.345 Max Web CSI: 0.379  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1089 /- /- /729 /12 /281 F 964 /- /- /582 /185 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 236 -1288 C - D 258 -829

**Lumber**  
 Top chord: 2x4 SP #2; T3 2x6 SP 2400F-2.0E;  
 Bot chord: 2x4 SP M-31;  
 Webs: 2x4 SP #3; W4 2x4 SP M-31;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

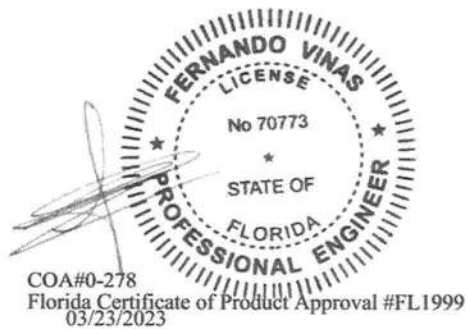
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - H	938 -450	G - F	539 -264
H - G	936 -451		

**Maximum Web Forces Per Ply (lbs)**

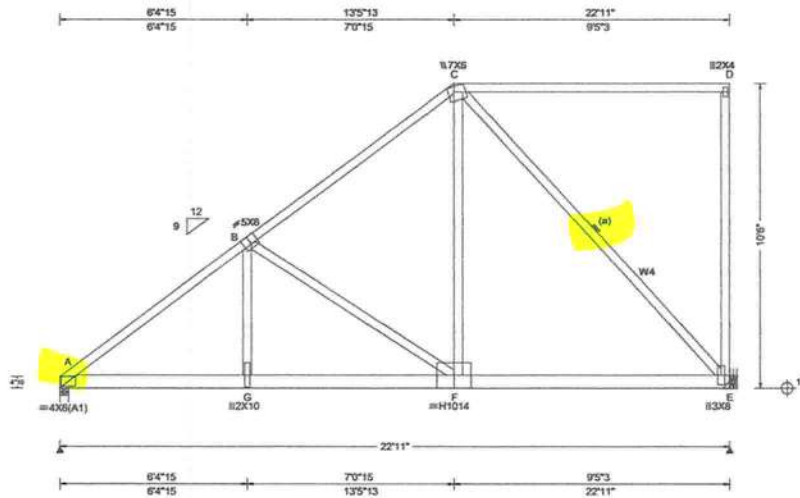
Webs	Tens.Comp.	Webs	Tens. Comp.
C - G	237 -502	D - F	387 -792
D - G	604 -47		



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2 Complete Trusses Required



<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.060 G 999 240 VERT(CL): 0.120 G 999 180 HORZ(LL): 0.021 B - - HORZ(TL): 0.043 B - - Creep Factor: 2.0 Max TC CSI: 0.549 Max BC CSI: 0.779 Max Web CSI: 0.856  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b>																																				
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>4518</td> <td>-</td> <td>-</td> <td>-</td> <td>740</td> <td>-</td> </tr> <tr> <td>E</td> <td>4288</td> <td>-</td> <td>-</td> <td>-</td> <td>1552</td> <td>-</td> </tr> </tbody> </table>		Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	4518	-	-	-	740	-	E	4288	-	-	-	1552	-	<b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>514 -2716</td> <td>B - C</td> <td>321 -1667</td> </tr> </tbody> </table>				Chords	Tens.Comp.	Chords	Tens. Comp.
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**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x6 SP 2400F-2.0E;  
 Webs: 2x4 SP #3; W4 2x4 SP M-31;

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
A - G	2135 -396	F - E	1299 -245
G - F	2122 -394		

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

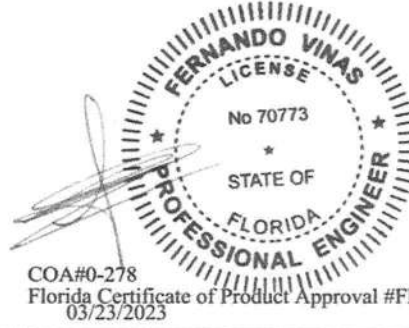
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
G - B	1039 -164	C - F	2248 -403
B - F	181 -999	C - E	359 -1904

**Nailnote**  
 Nail Schedule: 0.131"x3", min. nails  
 Top Chord: 1 Row @ 12.00" o.c.  
 Bot Chord: 1 Row @ 4.75" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
 TC: From 65 plf at 0.00 to 65 plf at 4.40  
 TC: From 33 plf at 4.40 to 33 plf at 22.92  
 BC: From 10 plf at 0.00 to 10 plf at 22.92  
 BC: 615 lb Conc. Load at 0.40, 2.40, 4.40, 6.40  
 BC: 680 lb Conc. Load at 8.40  
 BC: 697 lb Conc. Load at 10.40  
 BC: 738 lb Conc. Load at 12.40  
 BC: 622 lb Conc. Load at 14.40, 16.40, 18.40, 20.40  
 21.98

**Hangers / Ties**  
 (J) Hanger Support Required, by others

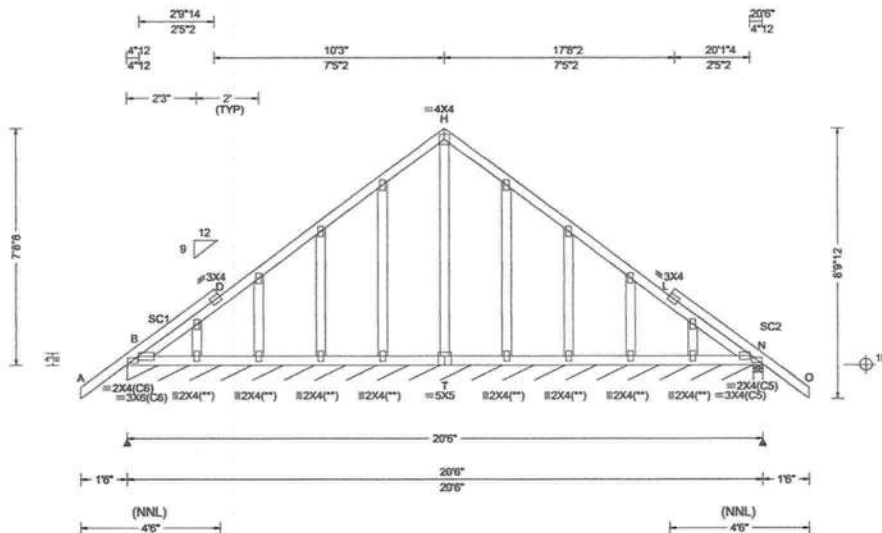


COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.004 D 999 180 HORZ(LL): 0.004 L - - HORZ(TL): 0.005 L - - Creep Factor: 2.0 Max TC CSI: 0.247 Max BC CSI: 0.071 Max Web CSI: 0.204  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B* 83 /- /- /48 /15 /14 N 277 /- /- /184 /27 /- Wind reactions based on MWFRS B Brg Wid = 242 Min Req = - N Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & N are a rigid surface. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Plating Notes**

All plates are 2X4 except as noted.  
(\*\*) 8 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**

In lieu of structural panels use purlins to brace TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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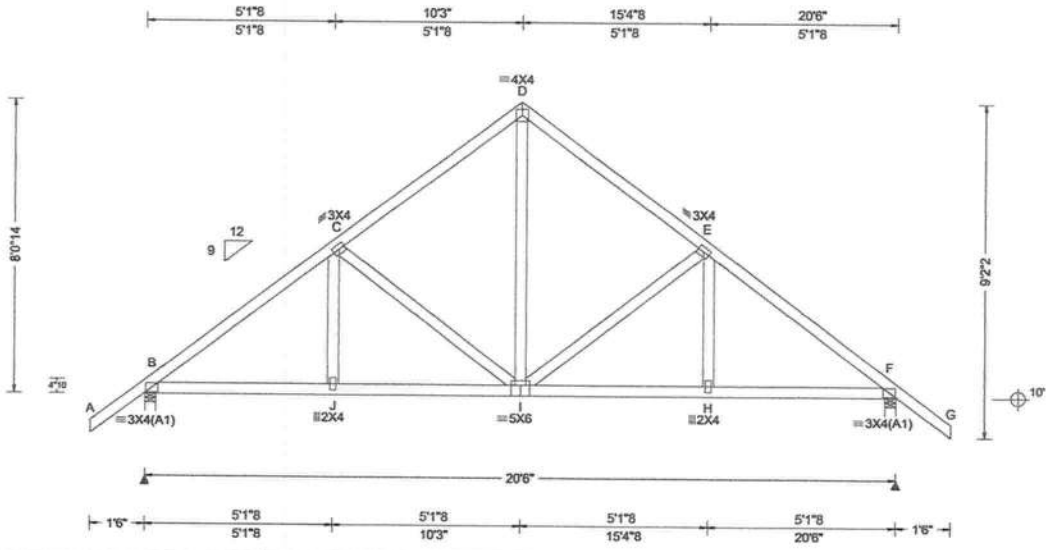
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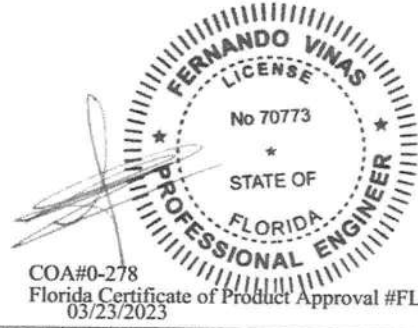
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.028 I 999 240 VERT(CL): 0.057 I 999 180 HORZ(LL): 0.014 F - - HORZ(TL): 0.029 F - -  Creep Factor: 2.0 Max TC CSI: 0.284 Max BC CSI: 0.307 Max Web CSI: 0.265  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>976</td> <td>-</td> <td>-</td> <td>/612</td> <td>/153</td> <td>/285</td> </tr> <tr> <td>F</td> <td>976</td> <td>-</td> <td>-</td> <td>/612</td> <td>/153</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	976	-	-	/612	/153	/285	F	976	-	-	/612	/153	-
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**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

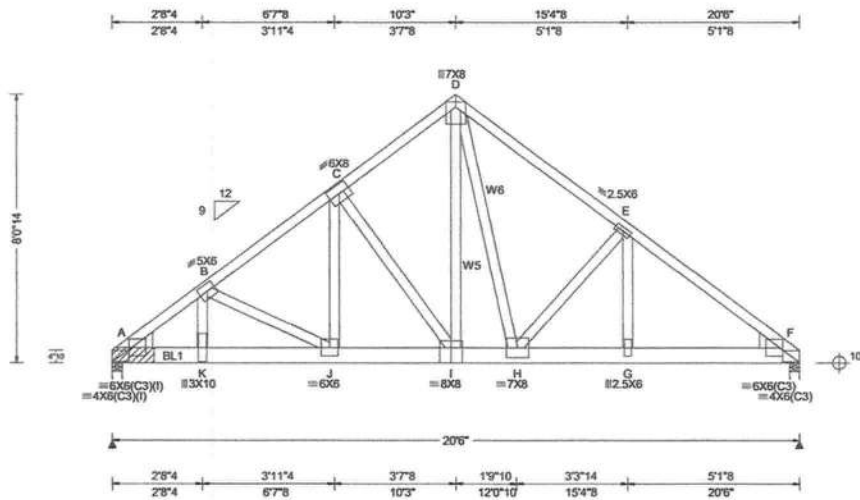
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.



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2 Complete Trusses Required



<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 *	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.130 J 999 240 VERT(CL): 0.260 J 934 180 HORZ(LL): 0.042 B - - HORZ(TL): 0.083 B - - Creep Factor: 2.0 Max TC CSI: 0.780 Max BC CSI: 0.711 Max Web CSI: 0.845  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity A 10934 /- /- /- /441 /- F 7375 /- /- /- /226 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = - F Brg Wid = 3.5 Min Req = 3.1 (Truss) Bearings A & F are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 326 -7163 D - E 298 -4890 B - C 294 -5531 E - F 250 -5447 C - D 232 -4146

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x6 SP 2400F-2.0E;  
Webs: 2x4 SP #3; W5 2x4 SP M-31; W6 2x4 SP #2;  
Lt Wedge: 2x6 SP 2400F-2.0E;  
Rt Wedge: 2x6 SP 2400F-2.0E;

**Nailnote**  
Nail Schedule: 0.131"x3", min. nails  
Top Chord: 1 Row @ 12.00" o.c.  
Bot Chord: 2 Rows @ 4.00" o.c. (Each Row)  
Webs : 1 Row @ 4" o.c.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
—(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC: From 65 plf at 0.00 to 65 plf at 4.69  
TC: From 33 plf at 4.69 to 33 plf at 16.77  
TC: From 65 plf at 16.77 to 65 plf at 20.50  
BC: From 10 plf at 0.00 to 10 plf at 20.50  
BC: 2273 lb Conc. Load at 0.69, 2.69, 4.69  
BC: 1159 lb Conc. Load at 6.69, 8.69  
BC: 1366 lb Conc. Load at 10.19  
BC: 4171 lb Conc. Load at 12.13  
BC: 622 lb Conc. Load at 13.19, 14.77, 16.77, 18.77

**Plating Notes**  
(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

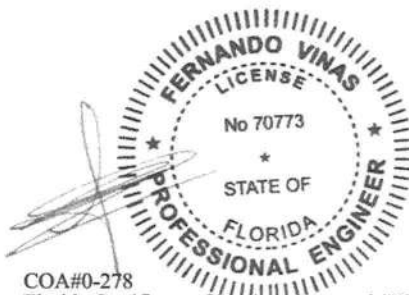
**Wind**  
Wind loads and reactions based on MWFRS.  
Wind loading based on both gable and hip roof types.  
**Bearing Block(s)**  
Brg blocks: 0.131"x3", min. nails  
brg x-loc #blocks length/blk #nails/blk wall plate  
1 0.000' 1 15" 19 Rigid Surface  
Brg block to be same size and species as chord.  
Refer to drawing CNNAILSP1014 for more information.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
A - K	5700 -255	I - H	3258 -180
K - J	5654 -255	H - G	4307 -188
J - I	4313 -223	G - F	4318 -186

**Maximum Web Forces Per Ply (lbs)**

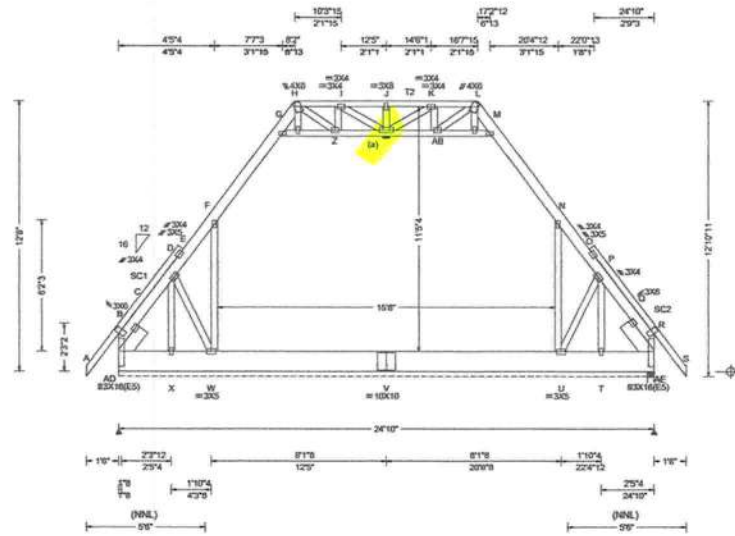
Web	Tens.Comp.	Webs	Tens. Comp.
K - B	1600 -4	I - D	2652 -70
B - J	33 -1423	D - H	2737 -222
J - C	2219 -71	H - E	0 -640
C - I	75 -1777	E - G	730 0



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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or * = PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.025 G 999 240 VERT(CL): 0.047 K 999 180 HORZ(LL): 0.036 M - - HORZ(TL): 0.051 M - - Creep Factor: 2.0 Max TC CSI: 0.321 Max BC CSI: 0.294 Max Web CSI: 0.356  VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL AD*158 /- /- /54 /11 /20 AE 1008 /- /- /580 /32 /- Wind reactions based on MWFRS AD Brg Wid = 294 Min Req = - AE Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AD & AE are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 344 -806 J - K 0 -1018 B - E 387 -258 K - L 0 -785 C - D 343 -793 L - M 0 -610 D - E 348 -882 M - N 37 -1001 E - F 159 -818 N - O 153 -818 F - G 37 -1001 O - P 342 -882 G - H 0 -610 O - R 387 -258 H - I 0 -785 P - Q 331 -800 I - J 0 -1018 Q - R 332 -814

**Lumber**  
Top chord: 2x6 SP #2; T2 2x4 SP #2;  
Bot chord: 2x12 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;  
Lt Slider: 2x8 SP #2; block length = 1.500'  
Rt Slider: 2x8 SP #2; block length = 1.847'

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 2X4 except as noted.

**Loading**  
Attic room loading from 4-7-0 to 20-3-0: Live Load: 40 PSF, Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

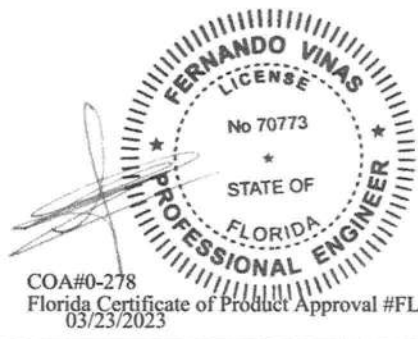
**Additional Notes**  
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
The overall height of this truss excluding overhang is 12-8-0.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - X	440 -261	V - U	521 -216
X - W	430 -255	U - T	432 -177
W - V	521 -216	T - R	440 -184

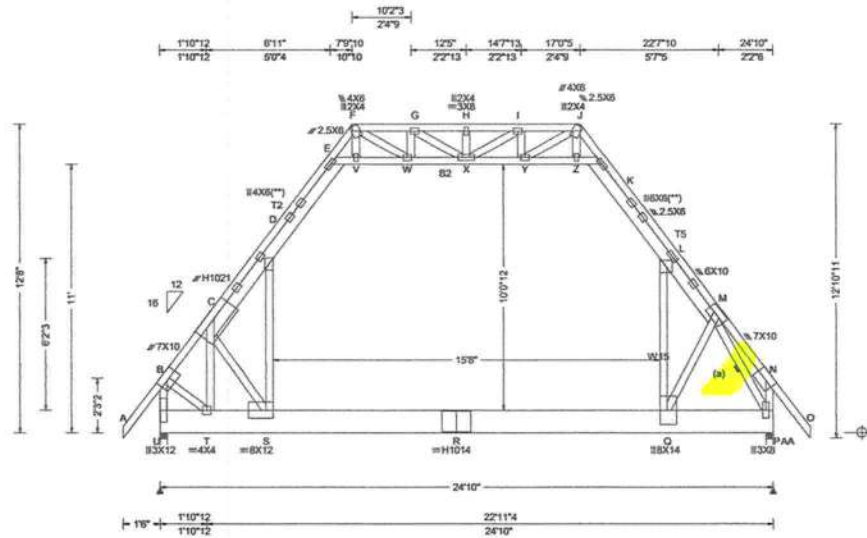
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
F - W	316 -443	AB - L	574 0
H - Z	574 0	U - N	316 -437



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.46 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.097 S 999 240 VERT(CL): 0.213 S 999 180 HORZ(LL): 0.123 D - - HORZ(TL): 0.279 D - -  Creep Factor: 2.0 Max TC CSI: 0.662 Max BC CSI: 0.375 Max Web CSI: 0.739  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ /R- /Rh /Rw /U /RL U 2306 /- /- /802 /4 /501 AA 2298 /- /- /799 /5 /- Non-Gravity Wind reactions based on MWFRS U Brg Wid = 3.5 Min Req = 1.9 (Truss) AA Brg Wid = 3.5 Min Req = 1.9 (Truss) Bearings U & AA are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. C - D 1689 -573 F - G 0 -640 D - E 1024 -1890 G - H 0 -825 K - L 2807 -717 H - I 0 -825 L - M 934 -3116 I - J 0 -654 B - C 99 -1683 J - K 0 -478 E - F 0 -465 K - Z 959 -1874

**Lumber**  
Top chord: 2x4 SP #2; T2, T5 2x6 SP 2400F-2.0E;  
Bot chord: 2x12 SP 2400F-2.0E; B2 2x4 SP #2;  
Webs: 2x4 SP #3; W15 2x4 SP #2;

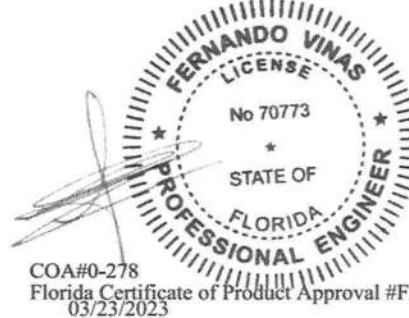
**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 3X4 except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Attic room loading from 4-7-0 to 20-3-0: Live Load: 40 PSF, Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

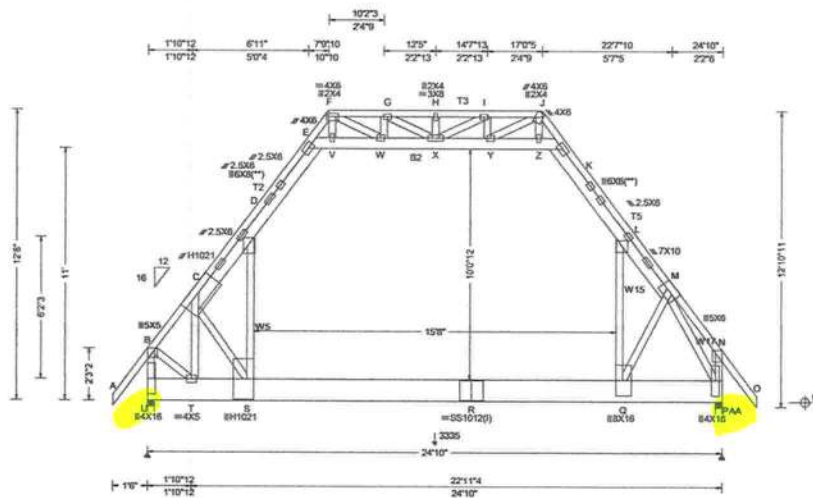


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**4 Complete Trusses Required**



<b>Loading Criteria (psf)</b> TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NC BCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS, 18SS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.162 Q 999 240 VERT(CL): 0.322 Q 925 180 HORZ(LL): -0.203 L - - HORZ(TL): 0.403 L - - Creep Factor: 2.0 Max TC CSI: 0.814 Max BC CSI: 0.624 Max Web CSI: 0.750  VIEW Ver: 22.02.00.0914.12	<b>▲ Maximum Reactions (lbs)</b>																																																																																								
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>U</td> <td>10001</td> <td>-</td> <td>-</td> <td>-</td> <td>/3549</td> <td>-</td> </tr> <tr> <td>AA</td> <td>12739</td> <td>-</td> <td>-</td> <td>-</td> <td>/3501</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS                  U Brg Wid = 3.5 Min Req = 2.1 (Truss)                  AA Brg Wid = 3.5 Min Req = 2.6 (Truss)                  Bearings U &amp; AA are a rigid surface.                  Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>3859 -1221</td> <td>L - M</td> <td>2782 -5284</td> </tr> <tr> <td>D - E</td> <td>2151 -2779</td> <td>B - C</td> <td>685 -2074</td> </tr> <tr> <td>K - L</td> <td>5477 -1577</td> <td>K - Z</td> <td>2215 -2914</td> </tr> </tbody> </table> <p><b>Maximum Bot Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>T - S</td> <td>1316 -428</td> <td>R - Q</td> <td>1392 -421</td> </tr> <tr> <td>S - R</td> <td>1392 -421</td> <td>Q - P</td> <td>1606 -478</td> </tr> </tbody> </table> <p><b>Maximum Web Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - U</td> <td>894 -2701</td> <td>X - Y</td> <td>505 -1649</td> </tr> <tr> <td>B - T</td> <td>1586 -523</td> <td>Y - Z</td> <td>561 -1771</td> </tr> <tr> <td>T - C</td> <td>539 -2040</td> <td>Z - K</td> <td>559 -1763</td> </tr> <tr> <td>D - S</td> <td>3321 -1024</td> <td>Q - L</td> <td>2883 -861</td> </tr> <tr> <td>E - V</td> <td>539 -1874</td> <td>Q - M</td> <td>144 -537</td> </tr> <tr> <td>V - W</td> <td>541 -1882</td> <td>M - P</td> <td>1074 -3604</td> </tr> <tr> <td>W - X</td> <td>497 -1693</td> <td></td> <td></td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	U	10001	-	-	-	/3549	-	AA	12739	-	-	-	/3501	-	Chords	Tens.Comp.	Chords	Tens.Comp.	C - D	3859 -1221	L - M	2782 -5284	D - E	2151 -2779	B - C	685 -2074	K - L	5477 -1577	K - Z	2215 -2914	Chords	Tens.Comp.	Chords	Tens. Comp.	T - S	1316 -428	R - Q	1392 -421	S - R	1392 -421	Q - P	1606 -478	Webs	Tens.Comp.	Webs	Tens. Comp.	B - U	894 -2701	X - Y	505 -1649	B - T	1586 -523	Y - Z	561 -1771	T - C	539 -2040	Z - K	559 -1763	D - S	3321 -1024	Q - L	2883 -861	E - V	539 -1874	Q - M	144 -537	V - W	541 -1882	M - P	1074 -3604
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**Lumber**  
 Top chord: 2x4 SP M-31; T2,T5 2x6 SP 2400F-2.0E;  
 T3 2x4 SP #2;  
 Bot chord: 2x12 SP 2400F-2.0E;  
 B2 2x6 SP 2400F-2.0E;  
 Webs: 2x4 SP #3; W5,W15,W17 2x4 SP #2;

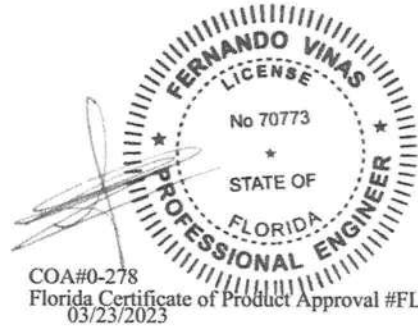
**Nailnote**  
 Nail Schedule: 0.131"x3", min. nails  
 Top Chord: 1 Row @ 3.25" o.c.  
 Bot Chord: 2 Rows @ 5.50" o.c. (Each Row)  
 Webs : 1 Row @ 4" o.c.  
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.  
 In addition, apply (1) 0.22"-0.25" min/max dia. X 6.0" length wood screw at each joint location.

**Special Loads**

—(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 73 plf at -1.50 to 73 plf at 26.33	TC: From 427 plf at 17.52 to 427 plf at 24.83
PLT: From 38 plf at 4.88 to 38 plf at 6.98	PLT: From 20 plf at 6.92 to 20 plf at 17.91
PLT: From 38 plf at 17.85 to 38 plf at 19.96	PLT: From 33 plf at 4.58 to 33 plf at 4.24
PLT: From 100 plf at 4.58 to 100 plf at 20.25	BC: From 7 plf at -1.50 to 7 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 24.83	BC: From 480 plf at 4.29 to 480 plf at 24.83
BC: From 7 plf at 24.83 to 7 plf at 26.33	BC: 1815 lb Conc. Load at 4.52
BC: 124 lb Conc. Load at 4.58,20.25	BC: 3335 lb Conc. Load at 12.42

**Plating Notes**  
 All plates are 3X4 except as noted.  
 (l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.  
 (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.  
  
**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.



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SEQN: 457058 / FROM: Page 2 of 2	COMN Ply: 4 Qty: 2	Job Number: 22-8029 Mouras House Truss Label: D03	Cust: R 215 JRef: 1XO72150002 T1 DrwNo: 081.23.1511.51294 JB / DF 03/22/2023
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**Wind**

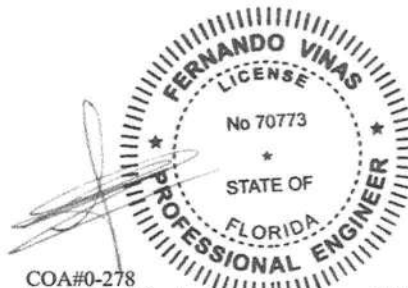
Wind loads and reactions based on MWFRS.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Blocking**

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:  
Bearing 2 located at 24.5' (blocking >= 3.50" if used)

**Additional Notes**

The overall height of this truss excluding overhang is 12-8-0.  
WIND LOAD CASE MODIFIED!  
APPLY (4) ADDITIONAL (0.128"x3.0" min nail) ON THE BOTTOM CHORD OF 3RD PLY ON OPPOSITE FACE OF HANGER AT 12.42 WITHOUT SPLITTING LUMBER



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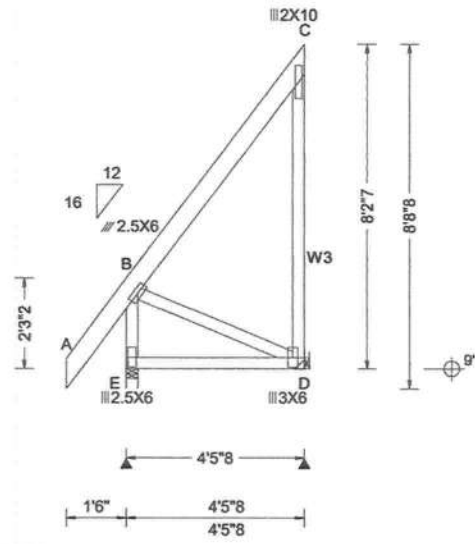
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155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



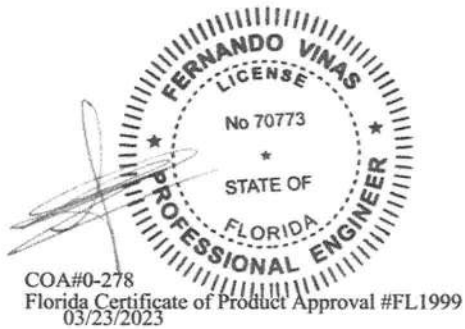
<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): 0.004 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.130 Max BC CSI: 0.219 Max Web CSI: 0.183  VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 348 /- /- /191 /- /214 D 188 /- /- /313 /167 /- Wind reactions based on MWFRS E Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. E - D 220 -572

**Lumber**  
Top chord: 2x6 SP 2400f-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3; W3 2x4 SP M-31;

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 8-2-7.

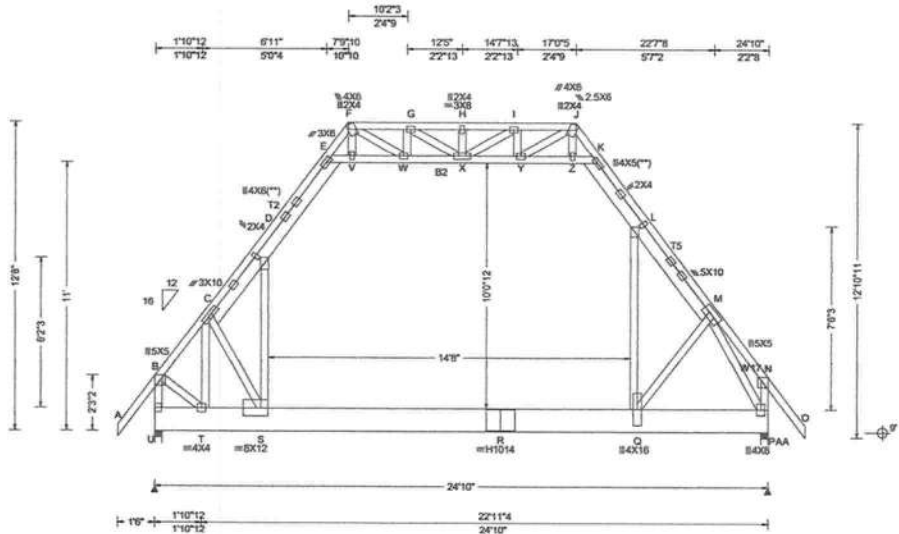


**Maximum Web Forces Per Ply (lbs)**

Loc	Tens.Comp.	Webs	Tens. Comp.
B - D	614	-236	C - D 379 -185

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.46 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.092 Q 999 240 VERT(CL): 0.219 Q 999 180 HORZ(LL): -0.110 L - - HORZ(TL): 0.268 L - - Creep Factor: 2.0 Max TC CSI: 0.769 Max BC CSI: 0.327 Max Web CSI: 0.829  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL					
				U 2296 /- /- /809 /- /502 AA 2234 /- /- /809 /- /- Wind reactions based on MWFRS U Brg Wid = 3.5 Min Req = 1.9 (Truss) AA Brg Wid = 3.5 Min Req = 1.8 (Truss) Bearings U & AA are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.					

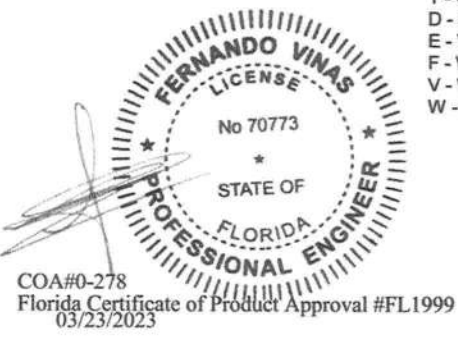
**Lumber**  
 Top chord: 2x4 SP #2; T2,T5 2x6 SP 2400F-2.0E;  
 Bot chord: 2x12 SP 2400F-2.0E; B2 2x4 SP #2;  
 Webs: 2x4 SP #3; W17 2x4 SP #2;

**Plating Notes**  
 All plates are 3X4 except as noted.  
 (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
 Attic room loading from 4-7-0 to 19-3-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.



**Maximum Bot Chord Forces Per Ply (lbs)**

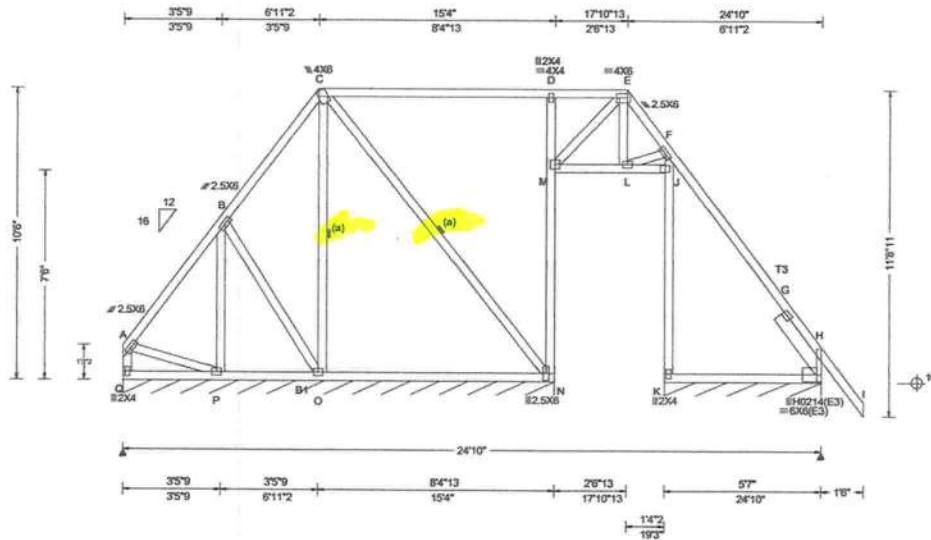
Chords	Tens.Comp.	Chords	Tens. Comp.
U - T	495 -501	R - Q	1105 -135
T - S	1026 -247	Q - P	1043 -78
S - R	1105 -135		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
B - U	4 -2292	X - Y	402 -517
B - T	1239 -50	Y - Z	306 -926
T - C	191 -1303	Y - J	554 0
D - S	1695 -183	Z - K	305 -928
E - V	314 -1049	Q - L	1295 -132
F - W	634 0	M - P	156 -2354
V - W	315 -1047	N - P	335 -378
W - X	404 -561		

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.049 G 999 240 VERT(CL): 0.085 G 768 180 HORZ(LL): -0.069 G - - HORZ(TL): 0.113 G - - Creep Factor: 2.0 Max TC CSI: 0.434 Max BC CSI: 0.333 Max Web CSI: 0.610  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL Q* 110 /- /- /68 /12 /26 K* 135 /- /- /144 /25 /- Wind reactions based on MWFRS Q Brg Wid = 183 Min Req = - K Brg Wid = 67.0 Min Req = - Bearings Q & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 452 -329 F - G 434 -308 C - D 446 -293 G - H 402 -345 D - E 450 -292
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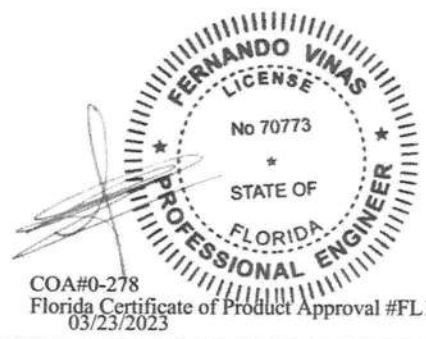
**Lumber**  
Top chord: 2x4 SP M-31; T3 2x4 SP #2;  
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;  
Webs: 2x4 SP #3;  
Rt Slider: 2x6 SP 2400F-2.0E; block length = 2.591'

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 3X4 except as noted.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

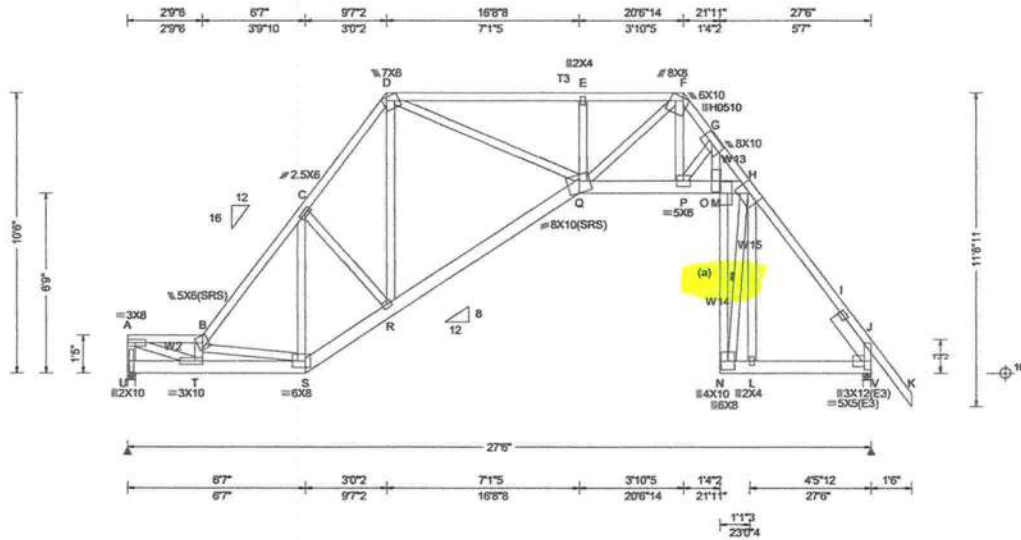
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



COA#0-278  
Florida Certificate of Product Approval #FL1999  
03/23/2023

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.289 N 999 240 VERT(CL): 0.681 N 484 180 HORZ(LL): 0.440 L - - HORZ(TL): 1.035 L - - Creep Factor: 2.0 Max TC CSI: 0.564 Max BC CSI: 0.675 Max Web CSI: 0.767  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL U 1304 /- /- /707 /8 /403 V 1424 /- /- /844 /- /- Wind reactions based on MWFRS U Brg Wid = 3.5 Min Req = 1.5 (Truss) V Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings U & V are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 878 -2516 F - G 784 -3396 B - C 703 -1948 G - H 996 -5282 C - D 742 -1829 H - I 520 -1261 D - E 785 -2612 I - J 576 -1417 E - F 802 -2629

**Lumber**  
Top chord: 2x4 SP #2; T3 2x4 SP M-31;  
Bot chord: 2x6 SP 2400F-2.0E;  
Webs: 2x4 SP #3; W2,W13 2x4 SP #2; W14, W15 2x4 SP M-31;  
Rt Slider: 2x6 SP 2400F-2.0E; block length = 2.143'

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 3X4 except as noted.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Deflection**  
Max JT VERT DEFL: LL: 0.29" DL: 0.40". See detail DEFLCAMB1014 for camber recommendations.  
Provide for adequate drainage of roof.

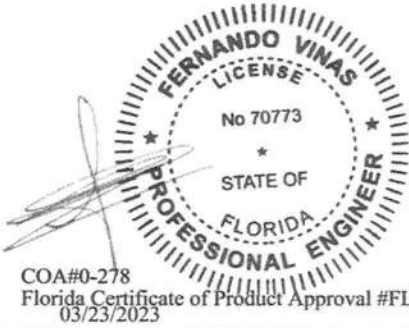
**Additional Notes**  
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
U - T	363 -398	P - O	3104 -228
T - S	2894 -920	O - M	3155 -235
S - R	1374 -266	N - L	652 -99
R - Q	1307 -258	M - H	3098 -229
Q - P	2024 -206	L - J	660 -100

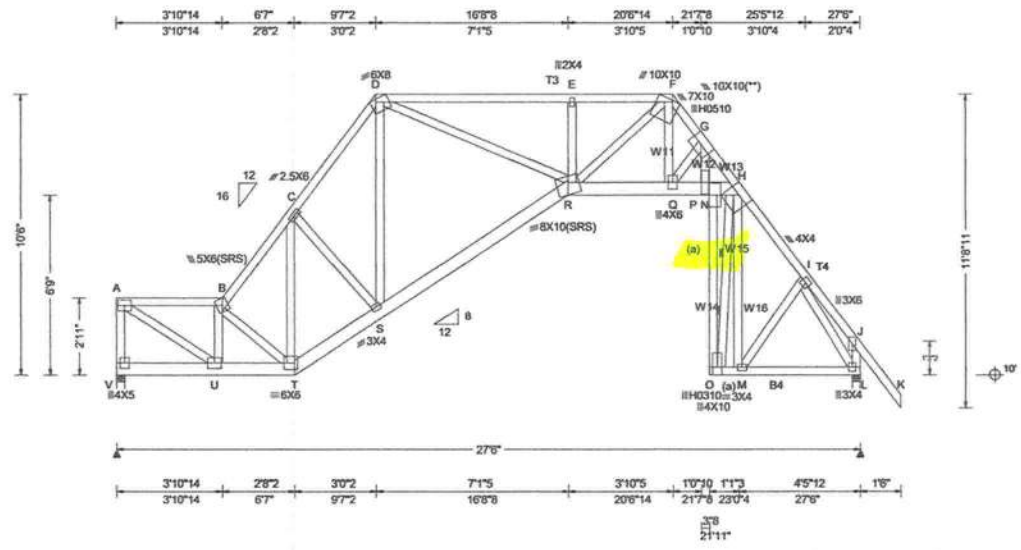
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
A - U	419 -1101	Q - F	795 -432
A - T	2705 -941	F - P	2012 -291
T - B	539 -1345	P - G	279 -1916
B - S	728 -1833	G - O	2990 -391
D - Q	1711 -300	N - M	4297 -634
Q - E	410 -420	N - H	620 -4110



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.279 O 999 240 VERT(CL): 0.772 O 427 180 HORZ(LL): 0.444 L - - HORZ(TL): 1.228 L - - Creep Factor: 2.0 Max TC CSI: 0.504 Max BC CSI: 0.734 Max Web CSI: 0.888 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b>																																																										
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>V</td> <td>1532</td> <td>-</td> <td>-</td> <td>/944</td> <td>-</td> <td>/398</td> </tr> <tr> <td>L</td> <td>1678</td> <td>-</td> <td>-</td> <td>/1175</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS                  V Brg Wid = 3.5 Min Req = 1.5 (Truss)                  L Brg Wid = 3.5 Min Req = 2.0 (Truss)                  Bearings V &amp; L are a rigid surface.                  Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>44</td> <td>-1875</td> <td>E - F</td> <td>0</td> <td>-3083</td> </tr> <tr> <td>B - C</td> <td>22</td> <td>-2232</td> <td>F - G</td> <td>0</td> <td>-4027</td> </tr> <tr> <td>C - D</td> <td>0</td> <td>-2137</td> <td>G - H</td> <td>0</td> <td>-6303</td> </tr> <tr> <td>D - E</td> <td>0</td> <td>-3062</td> <td>H - I</td> <td>19</td> <td>-1498</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	V	1532	-	-	/944	-	/398	L	1678	-	-	/1175	-	-	Chords	Tens.	Comp.	Chords	Tens.	Comp.	A - B	44	-1875	E - F	0	-3083	B - C	22	-2232	F - G	0	-4027	C - D	0	-2137	G - H	0	-6303	D - E	0
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C - D	0	-2137	G - H	0	-6303																																																									
D - E	0	-3062	H - I	19	-1498																																																									

**Lumber**  
 Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;  
 Bot chord: 2x6 SP 2400F-2.0E; B4 2x4 SP #2;  
 Webs: 2x4 SP #3; W11,W12,W13,W14,  
 W16 2x4 SP M-31; W15 2x4 SP #2;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 5X6 except as noted.  
 (\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Deflection**  
 Max JT VERT DEFL: LL: 0.28" DL: 0.50". See detail DEFLCAMB1014 for camber recommendations.  
 Provide for adequate drainage of roof.

**Additional Notes**  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

**Maximum Bot Chord Forces Per Ply (lbs)**

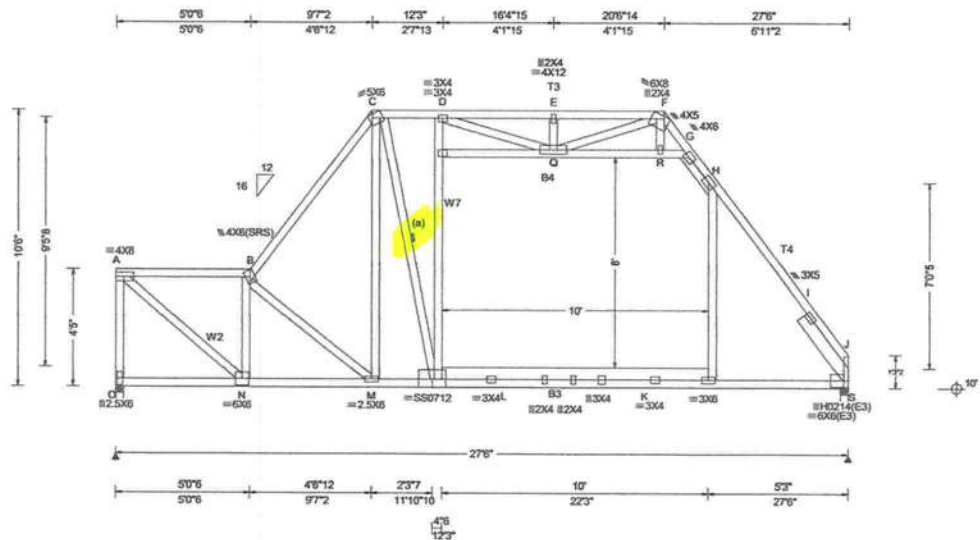
Chords	Tens.	Comp.	Chords	Tens.	Comp.
V - U	361	-396	Q - P	3774	0
U - T	2031	0	P - N	3836	0
T - S	1595	-113	O - M	770	0
S - R	1526	-133	N - H	3788	0
R - Q	2382	-29	M - L	770	0

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - V	140	-1451	F - Q	2412	0
A - U	2276	-46	Q - G	0	-2470
U - B	67	-1357	G - P	3669	0
B - T	140	-1012	O - N	4879	0
D - R	2002	0	O - H	0	-4878
R - E	0	-520	I - L	0	-1532
R - F	921	0	J - L	327	-420

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.88 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.358 P 922 240 S 2273 I- I- 1789 I- I- HORZ(LL): 0.332 Q - - HORZ(TL): 0.850 Q - - Creep Factor: 2.0 Max TC CSI: 0.656 Max BC CSI: 0.797 Max Web CSI: 0.671  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL					
				O 1860 I- I- 1720 I- /306 S 2273 I- I- 1789 I- I- Wind reactions based on MWFRS O Brg Wid = 3.5 Min Req = 1.7 (Truss) S Brg Wid = 3.5 Min Req = 2.1 (Truss) Bearings O & S are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.					

**Lumber**  
 Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;  
 Bot chord: 2x4 SP M-31; B3 2x6 SP 2400f-2.0E;  
 B4 2x4 SP #2;  
 Webs: 2x4 SP #3; W2 2x4 SP #2; W7 2x4 SP M-31;  
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 2.973'

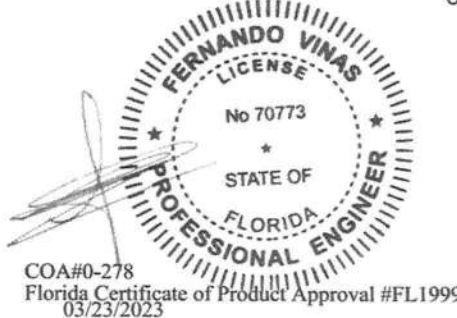
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Loading**  
 Attic room loading from 12-3-0 to 22-3-0: Live Load: 40 PSF, Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

**Purlins**  
 In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.  
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Left end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Deflection**  
 Max JT VERT DEFL: LL: 0.36" DL: 0.56". See detail DEFLCAMB1014 for camber recommendations.  
 Provide for adequate drainage of roof.



**Maximum Bot Chord Forces Per Ply (lbs)**

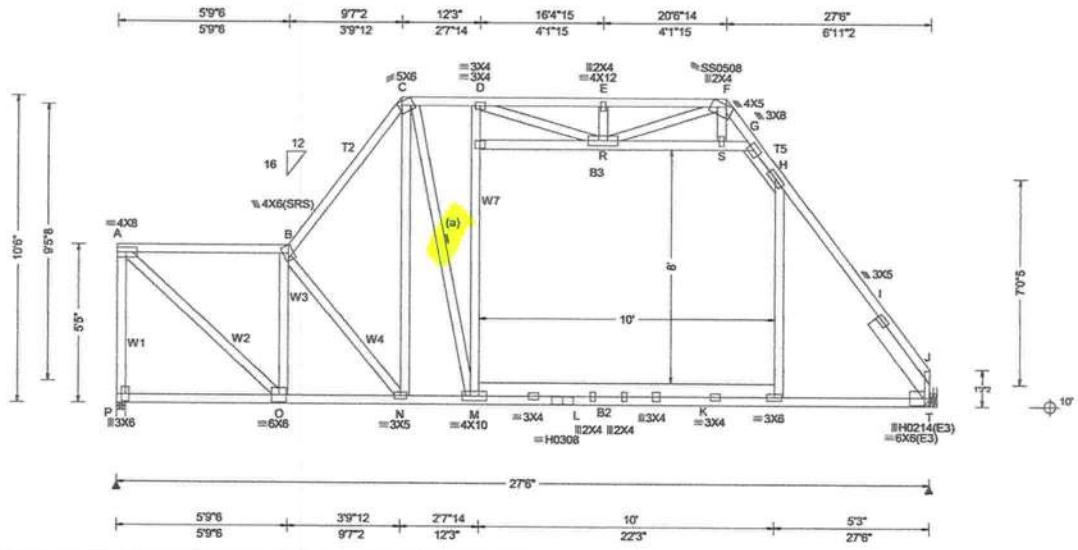
Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	2035 -1	K - J	1406 0
M - L	1352 -4		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
A - O	217 -1814	E - Q	0 -462
A - N	2547 -133	Q - R	304 -1371
N - B	131 -1586	Q - F	1688 0
B - M	264 -917	K - H	1019 0
C - M	851 -30	R - G	307 -1406
C - L	769 -389		

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.88 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS, 18SS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.347 M 950 240 VERT(CL): 0.890 M 370 180 HORZ(LL): 0.330 R - - HORZ(TL): 0.845 R - - Creep Factor: 2.0 Max TC CSI: 0.658 Max BC CSI: 0.796 Max Web CSI: 0.692  VIEW Ver: 21.02.00.1005.17	<b>Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>Rh</th> <th>Rw</th> <th>U</th> <th>RL</th> </tr> </thead> <tbody> <tr> <td>P</td> <td>1860</td> <td>-</td> <td>-</td> <td>/656</td> <td>/133</td> <td>/285</td> </tr> <tr> <td>T</td> <td>2273</td> <td>-</td> <td>-</td> <td>/716</td> <td>/12</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS P Brg Wid = 3.5 Min Req = 1.5 (Truss) T Brg Wid = - Min Req = - Bearing P is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>528 -1754</td> <td>F - G</td> <td>408 -816</td> </tr> <tr> <td>B - C</td> <td>757 -2363</td> <td>G - H</td> <td>612 -1638</td> </tr> <tr> <td>C - D</td> <td>561 -1481</td> <td>H - I</td> <td>575 -2591</td> </tr> <tr> <td>D - E</td> <td>810 -1643</td> <td>I - J</td> <td>568 -2774</td> </tr> <tr> <td>E - F</td> <td>812 -1644</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	R-	Rh	Rw	U	RL	P	1860	-	-	/656	/133	/285	T	2273	-	-	/716	/12	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	528 -1754	F - G	408 -816	B - C	757 -2363	G - H	612 -1638	C - D	561 -1481	H - I	575 -2591	D - E	810 -1643	I - J	568 -2774	E - F	812 -1644		
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**Lumber**  
 Top chord: 2x4 SP M-31; T2,T5 2x4 SP #2;  
 Bot chord: 2x4 SP M-31; B2 2x6 SP 2400F-2.0E;  
 B3 2x4 SP #2;  
 Webs: 2x4 SP #3; W1,W2,W3,W4,W7 2x4 SP M-31;  
 Rt Slider: 2x6 SP 2400F-2.0E; block length = 3.273'

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Loading**  
 Attic room loading from 12-3-0 to 22-3-0: Live Load: 40 PSF, Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
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 Wind loads based on MWFRS with additional C&C member design.  
 Left end vertical not exposed to wind pressure.  
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**Maximum Bot Chord Forces Per Ply (lbs)**

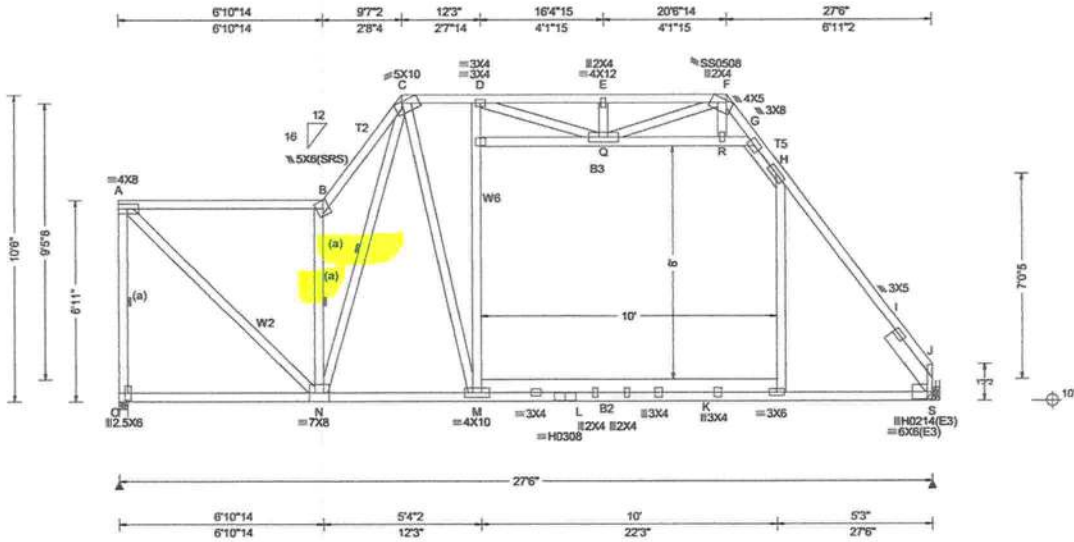
Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	1840 -415	M - L	3071 -1915
N - M	1347 -191	K - J	1406 -192

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
A - P	648 -1810	E - R	324 -458
A - O	2396 -719	R - S	433 -1373
O - B	555 -1537	R - F	1664 -733
B - N	380 -821	K - H	1022 -12
C - N	887 -343	S - G	442 -1409
C - M	787 -366		

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.88 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.381 M 866 240 VERT(CL): 0.976 M 338 180 HORZ(LL): 0.347 Q - - HORZ(TL): 0.889 Q - - Creep Factor: 2.0 Max TC CSI: 0.661 Max BC CSI: 0.812 Max Web CSI: 0.728  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 1860 /- /- /652 /172 /271 S 2273 /- /- /720 /25 /- Wind reactions based on MWFRS O Brg Wid = 3.5 Min Req = 1.5 (Truss) S Brg Wid = - Min Req = - Bearing O is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 519 -1618 F - G 409 -823 B - C 1075 -2879 G - H 611 -1645 C - D 560 -1487 H - I 573 -2621 D - E 814 -1668 I - J 661 -2843 E - F 816 -1670

**Lumber**  
Top chord: 2x4 SP M-31; T2, T5 2x4 SP #2;  
Bot chord: 2x4 SP M-31; B2 2x6 SP 2400F-2.0E;  
B3 2x4 SP #2;  
Webs: 2x4 SP #3; W2, W6 2x4 SP M-31;  
Rt Slider: 2x6 SP 2400F-2.0E; block length = 2.452'

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Loading**  
Attic room loading from 12-3-0 to 22-3-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

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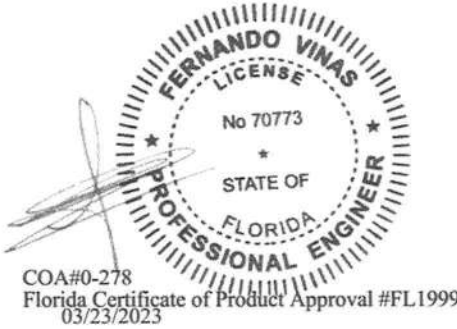
**Deflection**  
Max JT VERT DEFL: LL: 0.38" DL: 0.60". See detail DEFLCAMB1014 for camber recommendations.  
Provide for adequate drainage of roof.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	1331 -179	K - J	1409 -193
M - L	2755 -1584		

**Maximum Web Forces Per Ply (lbs)**

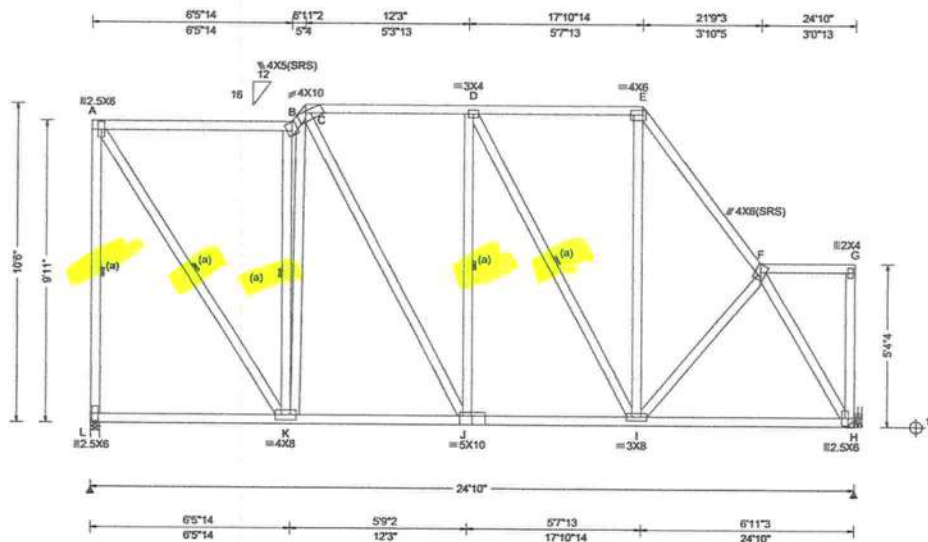
Webs	Tens.Comp.	Webs	Tens. Comp.
A - O	703 -1805	E - Q	325 -469
A - N	2262 -724	Q - R	430 -1375
N - B	1152 -2629	Q - F	1689 -737
N - C	1277 -620	K - H	1013 -4
C - M	786 -243	R - G	440 -1411



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.038 B 999 240	L	1159	-	-	1577	1240	139
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.088 B 999 180	H	1159	-	-	1580	176	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.017 A - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.039 A - -	L Brg Wid = 3.5 Min Req = 1.5 (Truss)						
NCBCLL: 10.00	Mean Height: 17.93 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	H Brg Wid = - Min Req = -						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.845	Bearing L is a rigid surface.						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.570	Members not listed have forces less than 375#						
Spacing: 24.0"	MWFRS Parallel Dist: > 2h	FT/RT: 20(0)/10(0)	Max Web CSI: 0.780	<b>Maximum Top Chord Forces Per Ply (lbs)</b>						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.00.1005.17	Chords	Tens.Comp.	Chords	Tens. Comp.			
	Loc. from endwall: not in 9.00 ft	WAVE		A - B	390	-595	D - E	448	-597	
	GCpi: 0.18			B - C	699	-980	E - F	555	-1081	
	Wind Duration: 1.60			C - D	514	-723				

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**

(J) Hanger Support Required, by others

**Purlins**

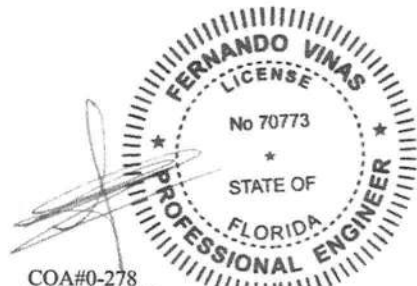
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**Wind**

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
K - J	593 -204	I - H	616 -303
J - I	726 -306		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
A - L	787 -1107	I - E	459 -178
A - K	1077 -705	F - H	581 -1179
K - B	826 -1037		

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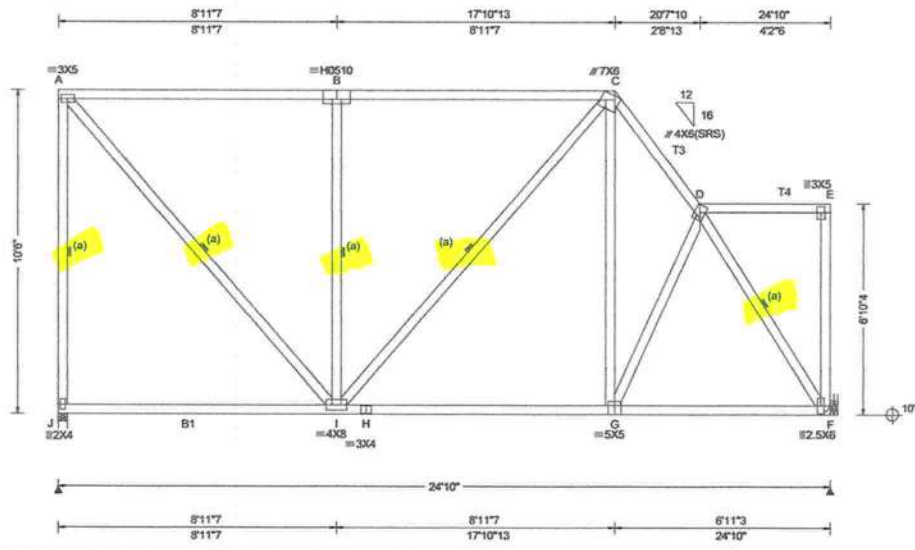
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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.34 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.033 B 999 240 VERT(CL): 0.073 B 999 180 HORZ(LL): 0.007 F - - HORZ(TL): 0.016 F - - Creep Factor: 2.0 Max TC CSI: 0.677 Max BC CSI: 0.664 Max Web CSI: 0.593 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 1159 /- /- /735 /11 /99 F 1159 /- /- /576 /66 /- Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing J is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 106 -730 C - D 249 -1029 B - C 108 -731					
				<b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. I - H 603 -114 G - F 618 -149 H - G 603 -114 <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. A - J 36 -1088 B - I 84 -772 A - I 1107 -161 D - F 275 -1142					

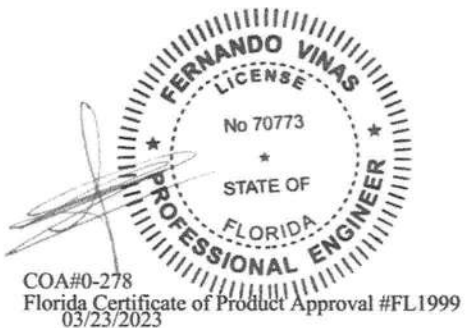
**Lumber**  
 Top chord: 2x4 SP M-31; T3,T4 2x4 SP #2;  
 Bot chord: 2x4 SP #2; B1 2x4 SP M-31;  
 Webs: 2x4 SP #3;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
 (J) Hanger Support Required, by others

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

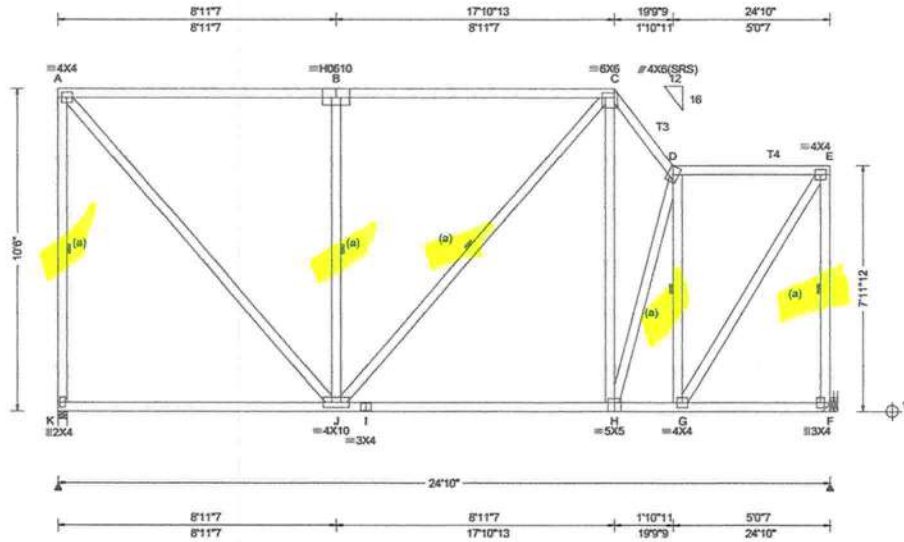


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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.90 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.036 B 999 240 VERT(CL): 0.099 B 999 180 HORZ(LL): 0.007 A - - HORZ(TL): 0.019 A - - Creep Factor: 2.0 Max TC CSI: 0.829 Max BC CSI: 0.865 Max Web CSI: 0.706  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ / R- / Rh / Rw / U / RL K 1366 /- /- /751 /- /68 F 1366 /- /- /765 /- /- Wind reactions based on MWFRS K Brg Wid = 3.5 Min Req = 1.6 (Truss) F Brg Wid = - Min Req = - Bearing K is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 0 -861 C - D 48 -1164 B - C 0 -863 D - E 0 -695
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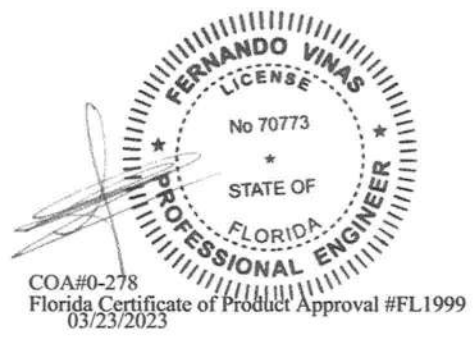
**Lumber**  
Top chord: 2x4 SP M-31; T3,T4 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

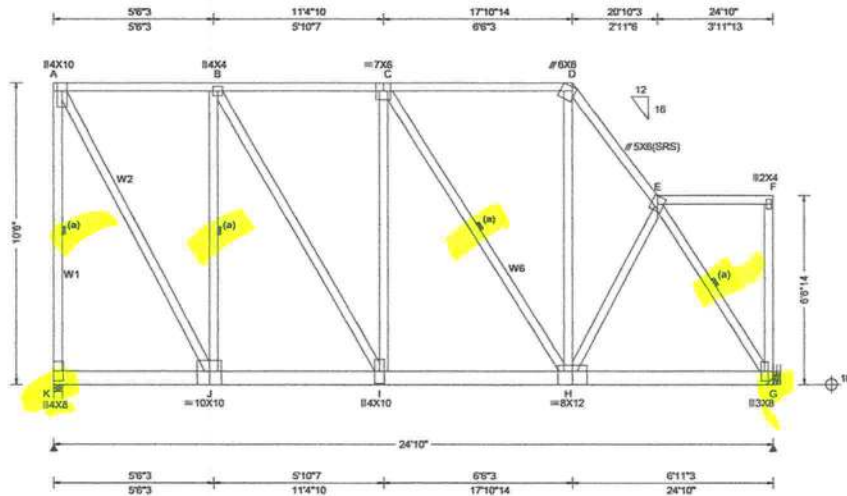
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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**2 Complete Trusses Required**



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg, Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.095 I 999 240 VERT(CL): 0.195 I 999 180 HORZ(LL): -0.021 F - - HORZ(TL): 0.043 F - - Creep Factor: 2.0 Max TC CSI: 0.351 Max BC CSI: 0.408 Max Web CSI: 0.897  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b>																
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw / U / RL</th> </tr> </thead> <tbody> <tr> <td>K</td> <td>7367</td> <td>-</td> <td>-</td> <td>- / 1152 / -</td> </tr> <tr> <td>G</td> <td>4171</td> <td>-</td> <td>-</td> <td>- / 610 / -</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS                  K Brg Wid = 3.5 Min Req = 3.1 (Truss)                  G Brg Wid = - Min Req = -                  Bearing K is a rigid surface.                  Members not listed have forces less than 375#</p>		Loc	Gravity		Non-Gravity		R+	/R-	/Rh	/Rw / U / RL	K	7367	-	-	- / 1152 / -	G
Loc	Gravity		Non-Gravity																	
	R+	/R-	/Rh	/Rw / U / RL																
K	7367	-	-	- / 1152 / -																
G	4171	-	-	- / 610 / -																

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3; W1 2x4 SP M-31; W2, W6 2x4 SP #2;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Nailnote**  
 Nail Schedule: 0.131"x3", min. nails  
 Top Chord: 1 Row @ 12.00" o.c.  
 Bot Chord: 1 Row @ 4.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
 TC: From 37 plf at 0.00 to 37 plf at 11.39  
 TC: From 73 plf at 11.39 to 73 plf at 24.83  
 BC: From 10 plf at 0.00 to 10 plf at 11.38  
 BC: From 20 plf at 11.38 to 20 plf at 24.83  
 BC: 1125 lb Conc. Load at 1.44, 3.44, 5.44, 7.44  
 BC: 964 lb Conc. Load at 9.44  
 BC: 4288 lb Conc. Load at 11.38

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
 The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 End verticals not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

Maximum Top Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	259 - 1675	C - D	197 - 1361
B - C	351 - 2386	D - E	330 - 2243

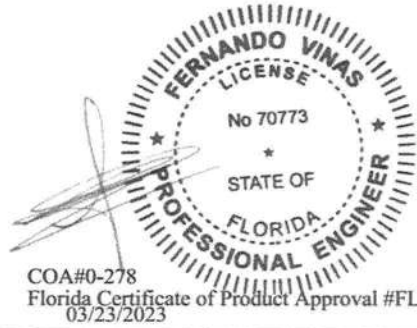
  

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
J - I	1713 - 264	H - G	1283 - 192
I - H	2345 - 346		

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A - K	502 - 3203	I - C	1436 - 186
A - J	3574 - 552	C - H	285 - 1881
J - B	192 - 1327	H - D	1563 - 194
B - I	1396 - 180	E - G	353 - 2359

**Hangers / Ties**  
 (J) Hanger Support Required, by others

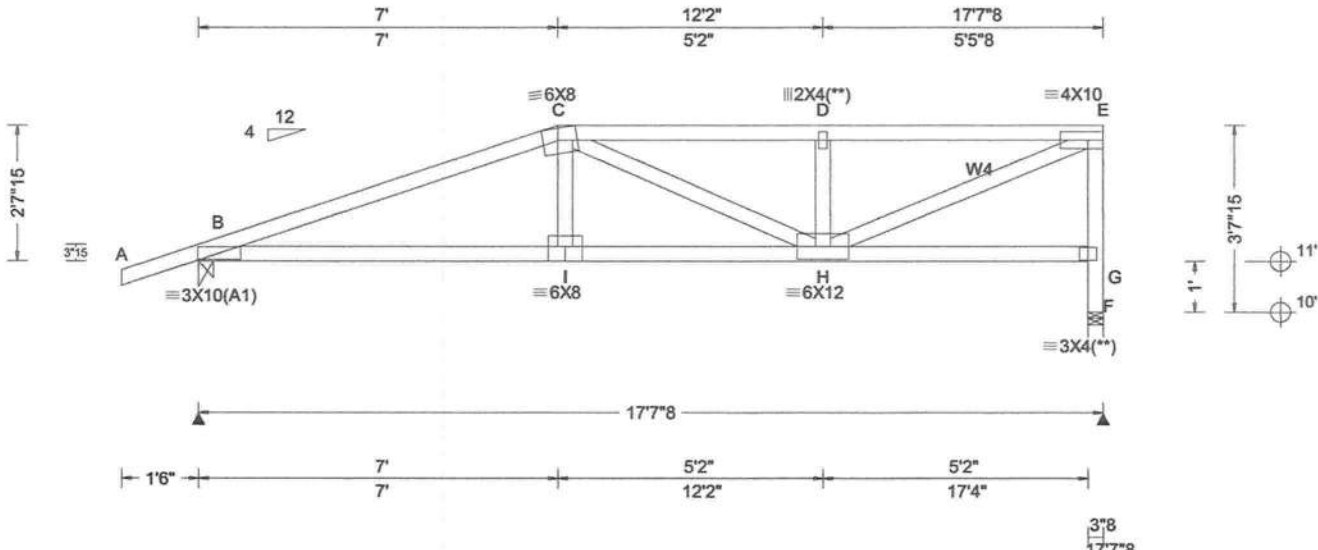


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.119 I 999 240 VERT(CL): 0.268 I 775 180 HORZ(LL): 0.026 H - - HORZ(TL): 0.059 H - - Creep Factor: 2.0 Max TC CSI: 0.572 Max BC CSI: 0.538 Max Web CSI: 0.885  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1846 /- /- /- /325 /- F 2147 /- /- /- /372 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings B & F are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 747 -4387 D - E 604 -3514 C - D 604 -3515

**Lumber**  
Top chord: 2x4 SP M-31;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3; W4 2x4 SP #2;

**Special Loads**  
——(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 72 plf at -1.50 to 72 plf at 7.00	TC: From 36 plf at 7.00 to 36 plf at 17.63
BC: From 4 plf at -1.50 to 4 plf at 0.00	BC: From 20 plf at 0.00 to 20 plf at 7.15
BC: From 10 plf at 7.15 to 10 plf at 17.33	TC: 334 lb Conc. Load at 7.03
TC: 212 lb Conc. Load at 9.06,11.06,13.06,15.06	17.06
BC: 705 lb Conc. Load at 7.03	BC: 130 lb Conc. Load at 9.06,11.06,13.06,15.06
17.06	

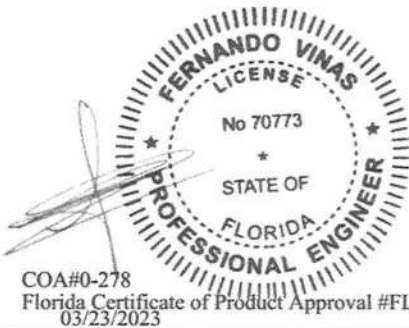
**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp. Chords Tens. Comp.  
B - I 4093 -687 I - H 4148 -687

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
C - I 881 -1 D - H 307 -849  
C - H 91 -697 G - F 371 -2145  
H - E 3918 -677 G - E 401 -1933

**Plating Notes**  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads and reactions based on MWFRS.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

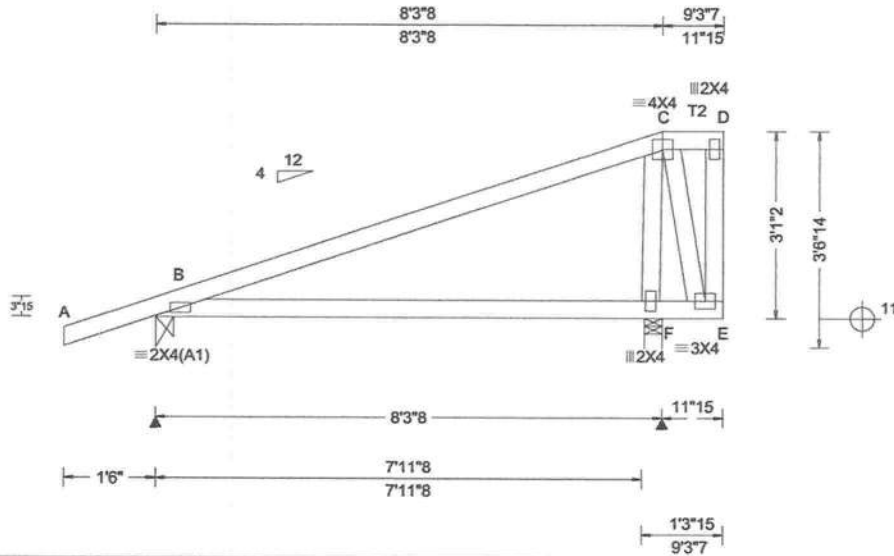


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**ALPINE**  
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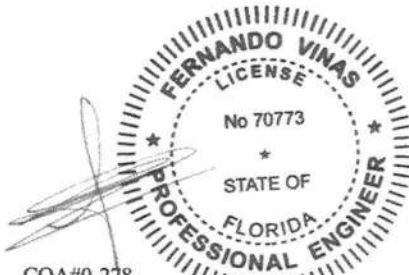


<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.044 B 999 240 VERT(CL): 0.098 B 972 180 HORZ(LL): 0.018 B - - HORZ(TL): 0.040 B - -  Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.665 Max Web CSI: 0.078  VIEW Ver: 21.02.00B.1108.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 500 /- /- /349 /72 /112 F 468 /- /- /302 /85 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP M-31; T2 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Right cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.



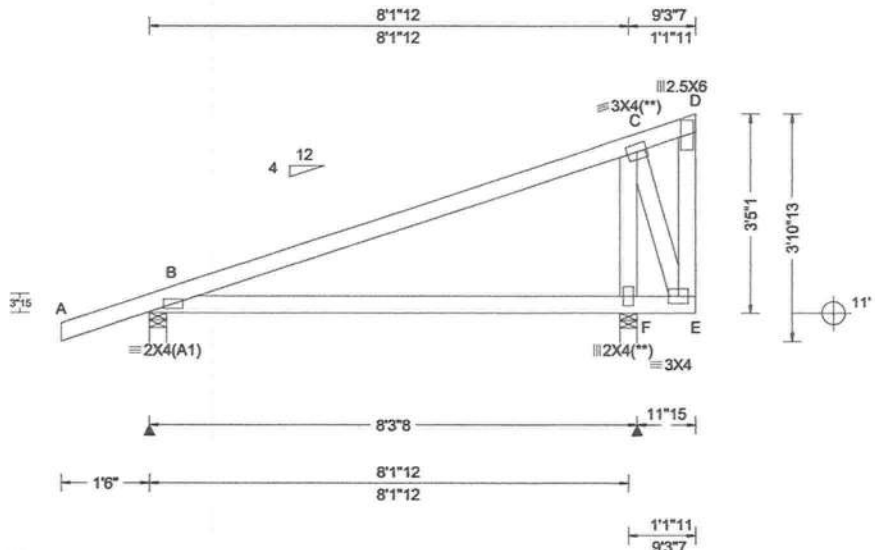
COA#0-278  
Florida Certificate of Product Approval #FL1999  
03/23/2023

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.034 B 999 240 VERT(CL): 0.076 B 999 180 HORZ(LL): 0.014 B - - HORZ(TL): 0.030 B - - Creep Factor: 2.0 Max TC CSI: 0.792 Max BC CSI: 0.645 Max Web CSI: 0.139  VIEW Ver: 21.02.00B.1108.20	<b>▲ Maximum Reactions (lbs)</b> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>502</td> <td>-</td> <td>-</td> <td>/349</td> <td>/68</td> <td>/123</td> </tr> <tr> <td>F</td> <td>466</td> <td>-</td> <td>-</td> <td>/314</td> <td>/89</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# <b>Maximum Web Forces Per Ply (lbs)</b> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>C - E</td> <td>330 -546</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	502	-	-	/349	/68	/123	F	466	-	-	/314	/89	-	Webs	Tens.Comp.	C - E	330 -546
Loc	Gravity			Non-Gravity																															
	R+	/R-	/Rh	/Rw	/U	/RL																													
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F	466	-	-	/314	/89	-																													
Webs	Tens.Comp.																																		
C - E	330 -546																																		

**Lumber**

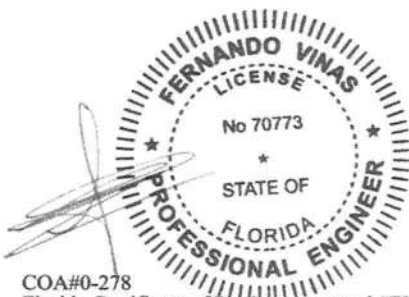
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Wind**

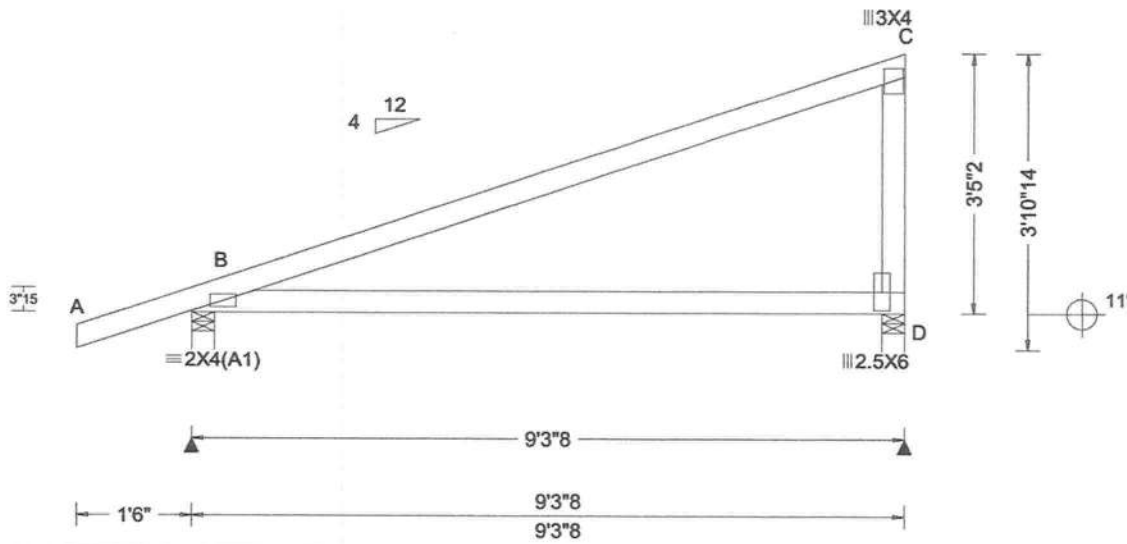
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Right cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.



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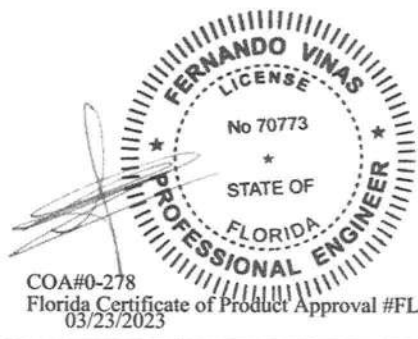
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.028 B - - HORZ(TL): 0.063 B - - Creep Factor: 2.0 Max TC CSI: 0.562 Max BC CSI: 0.802 Max Web CSI: 0.515  VIEW Ver: 21.02.00B.1108.20	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 557 /- /- /389 /47 /90 D 408 /- /- /281 /32 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP M-31;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Wind**

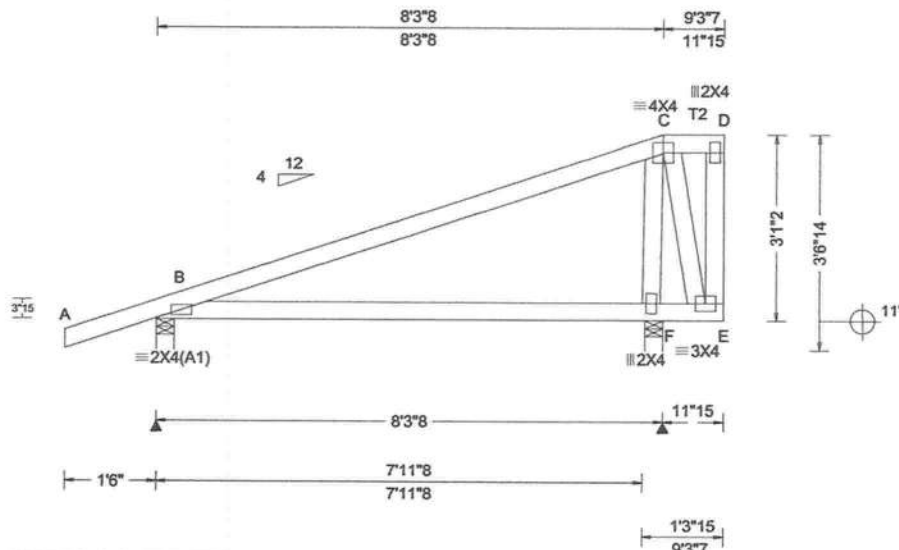
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.044 B 999 240 VERT(CL): 0.098 B 972 180 HORZ(LL): 0.018 B - - HORZ(TL): 0.040 B - - Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.665 Max Web CSI: 0.078  VIEW Ver: 21.02.00B.1108.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 500 /- /- /350 /72 /112 F 468 /- /- /302 /85 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP M-31; T2 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Right cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.



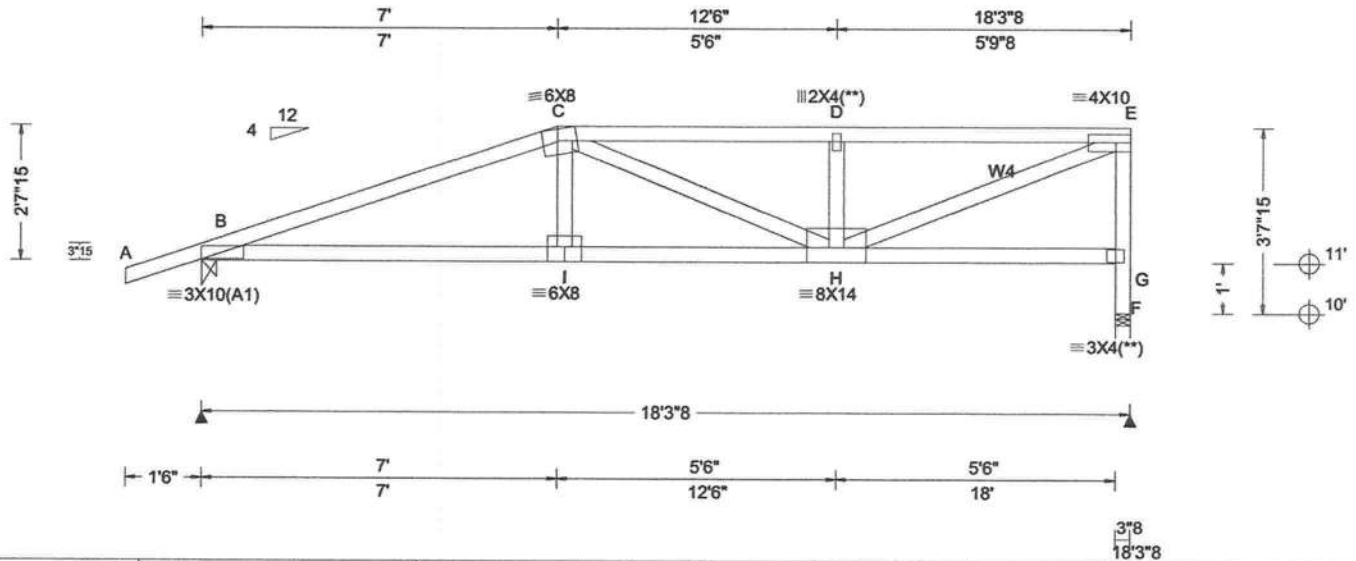
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 155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.133 I 999 240 VERT(CL): 0.299 I 722 180 HORZ(LL): 0.028 H - - HORZ(TL): 0.064 H - - Creep Factor: 2.0 Max TC CSI: 0.593 Max BC CSI: 0.581 Max Web CSI: 0.953 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1926 /- /- /- /338 /- F 2098 /- /- /- /363 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings B & F are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 790 -4636 D - E 660 -3837 C - D 660 -3837					
				<b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - I 4330 -728 I - H 4385 -728 <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. C - I 888 0 H - E 4223 -730 C - H 73 -597 G - F 362 -2096 D - H 328 -908 G - E 388 -1919					

**Lumber**  
 Top chord: 2x4 SP M-31;  
 Bot chord: 2x4 SP M-31;  
 Webs: 2x4 SP #3; W4 2x4 SP #2;

**Special Loads**  
 ——(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
 TC: From 72 plf at -1.50 to 72 plf at 7.00  
 TC: From 36 plf at 7.00 to 36 plf at 18.29  
 BC: From 4 plf at -1.50 to 4 plf at 0.00  
 BC: From 20 plf at 0.00 to 20 plf at 7.15  
 BC: From 10 plf at 7.15 to 10 plf at 18.00  
 TC: 334 lb Conc. Load at 7.03  
 TC: 212 lb Conc. Load at 9.06,11.06,13.06,15.06  
 17.06  
 BC: 705 lb Conc. Load at 7.03  
 BC: 130 lb Conc. Load at 9.06,11.06,13.06,15.06  
 17.06

**Plating Notes**  
 (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

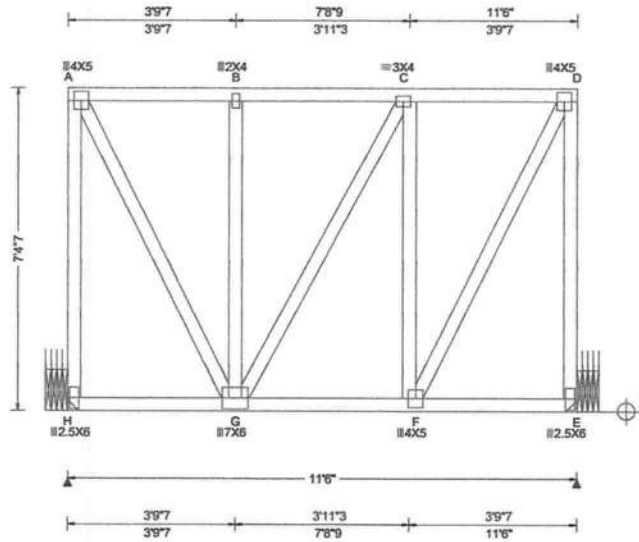
**Wind**  
 Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.37 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.013 B 999 240 VERT(CL): 0.022 B 999 180 HORZ(LL): -0.003 A - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.110 Max BC CSI: 0.139 Max Web CSI: 0.602  VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL H 684 /- /- /- /452 /- E 715 /- /- /- /479 /- <b>Wind reactions based on MWFRS</b> H Brg Wid = - Min Req = - E Brg Wid = - Min Req = - <b>Members not listed have forces less than 375#</b> <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. A - H 405 -607 F - D 641 -441 A - G 635 -434 D - E 414 -619

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3;

**Special Loads**

—(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC: From 30 plf at 0.00 to 30 plf at 11.50  
BC: From 10 plf at 0.00 to 10 plf at 11.50  
BC: 188 lb Conc. Load at 1.94, 3.94, 5.94, 7.94, 9.94

**Hangers / Ties**

(J) Hanger Support Required, by others

**Purlins**

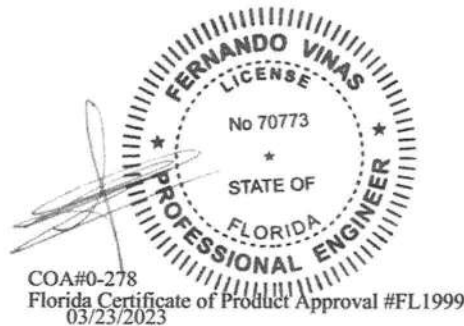
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

**Wind**

Wind loads and reactions based on MWFRS.  
End verticals not exposed to wind pressure.

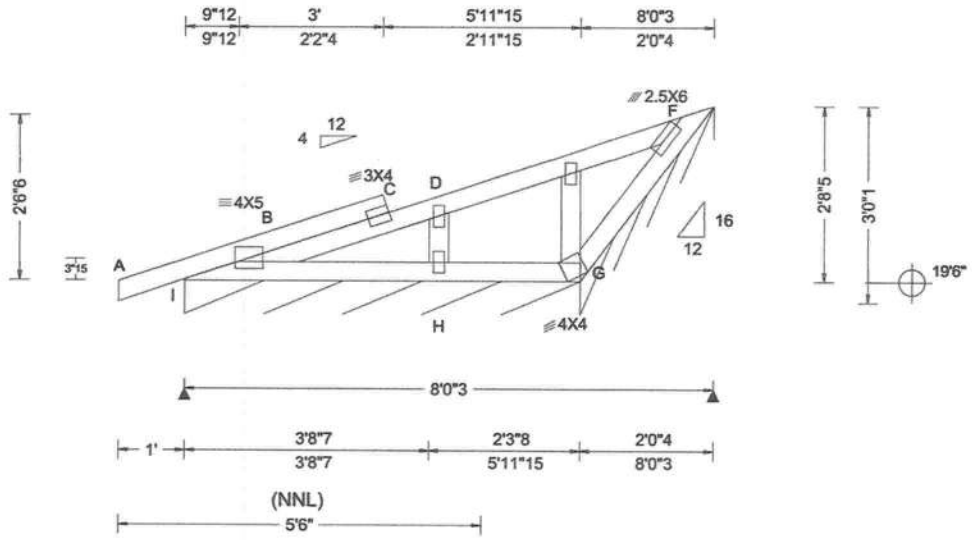
**Additional Notes**

Truss must be installed as shown with top chord up.  
The overall height of this truss excluding overhang is 7-4-7.



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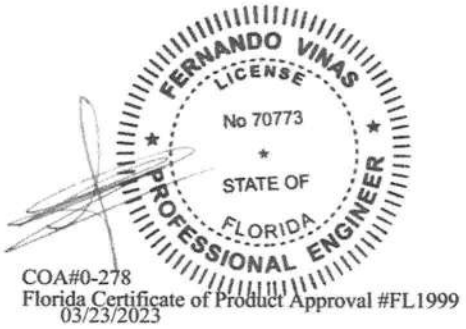
<b>Loading Criteria (psf)</b> TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.68 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 F 999 240 VERT(CL): 0.002 F 999 180 HORZ(LL): 0.001 F - - HORZ(TL): 0.001 F - - Creep Factor: 2.0 Max TC CSI: 0.417 Max BC CSI: 0.044 Max Web CSI: 0.063 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>I*</td> <td>114</td> <td>-</td> <td>-</td> <td>/63</td> <td>-</td> <td>/8</td> </tr> <tr> <td>G*</td> <td>31</td> <td>-</td> <td>-</td> <td>/14</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS I Brg Wid = 72.0 Min Req = - G Brg Wid = 24.2 Min Req = - Bearings I & G are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	I*	114	-	-	/63	-	/8	G*	31	-	-	/14	-	-
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
I*	114	-	-	/63	-	/8																									
G*	31	-	-	/14	-	-																									

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Plating Notes**  
 All plates are 2X4 except as noted.

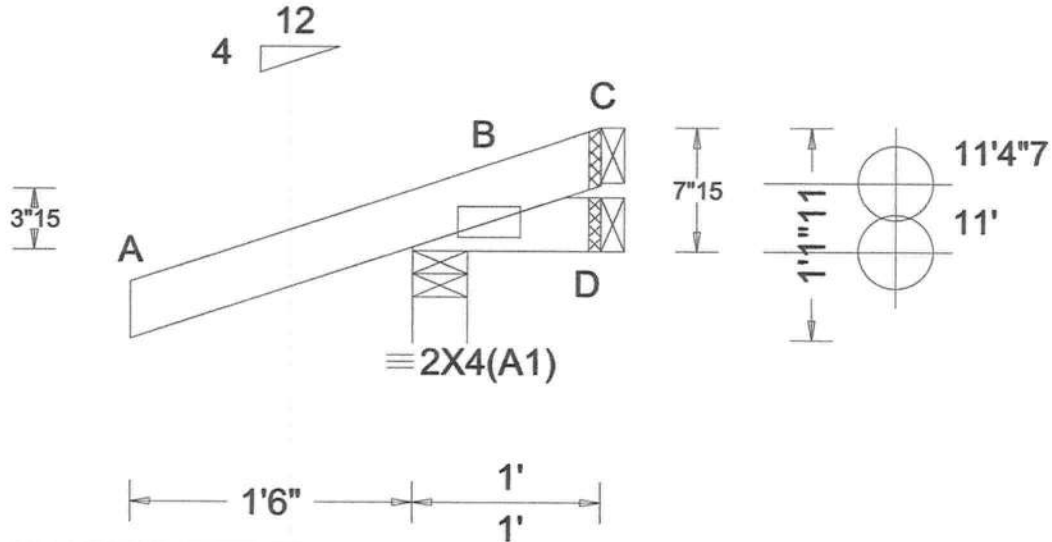
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
 Shim all supports to solid bearing.



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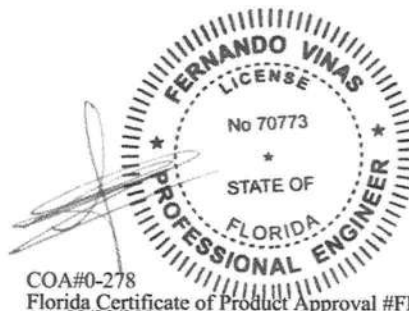
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
Loc	R+	/R-	/Rh	/Rw	/U	/RL				
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	B	287	-	-	/224	/95	/28
TCDL: 15.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	D	-	/-24	/-	/20	/20	/-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	C	-	/-58	/-	/35	/51	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 D - -	Wind reactions based on MWFRS						
Des Ld: 45.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.000 D - -	B Brg Wid = 3.5 Min Req = 1.5 (Truss)						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	D Brg Wid = 1.5 Min Req = -						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.261	C Brg Wid = 1.5 Min Req = -						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.033	Bearing B is a rigid surface.						
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT: 20(0)/10(0)	Max Web CSI: 0.000	Members not listed have forces less than 375#						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.00.1005.17							
	Loc. from endwall: Any	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

#### Wind

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.



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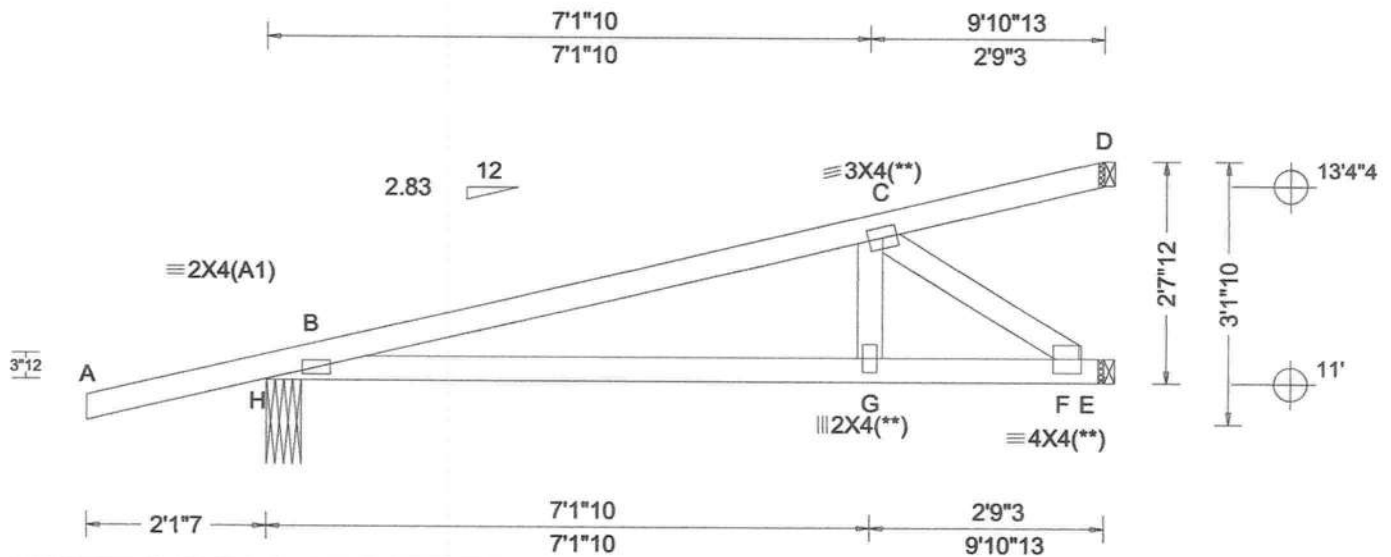
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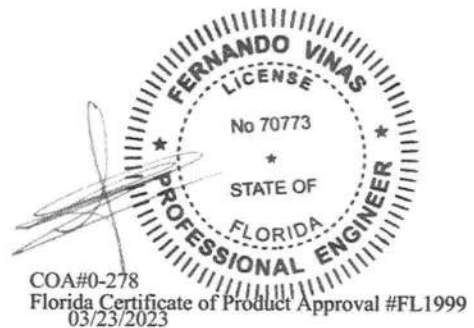
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																														
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.019 B 999 240 VERT(CL): 0.043 B 999 180 HORZ(LL): 0.004 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.545 Max BC CSI: 0.212 Max Web CSI: 0.102  VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>482</td> <td>-</td> <td>-</td> <td>-</td> <td>/91</td> <td>-</td> </tr> <tr> <td>E</td> <td>575</td> <td>-</td> <td>-</td> <td>-</td> <td>/68</td> <td>-</td> </tr> <tr> <td>D</td> <td>122</td> <td>-</td> <td>-</td> <td>-</td> <td>/43</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS  H Brg Wid = 4.9 Min Req = 1.5 (Truss)  E Brg Wid = 1.5 Min Req = -  D Brg Wid = 1.5 Min Req = -  Bearing H is a rigid surface.  Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b>  Chords Tens.Comp.</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td>B - C</td> <td>149</td> <td>-922</td> </tr> </table> <p><b>Maximum Bot Chord Forces Per Ply (lbs)</b>  Chords Tens.Comp. Chords Tens. Comp.</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td>B - G</td> <td>865</td> <td>-133</td> <td>G - F</td> <td>849</td> <td>-137</td> </tr> </table> <p><b>Maximum Web Forces Per Ply (lbs)</b>  Webs Tens.Comp.</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td>C - F</td> <td>169</td> <td>-1046</td> </tr> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	482	-	-	-	/91	-	E	575	-	-	-	/68	-	D	122	-	-	-	/43	-	B - C	149	-922	B - G	865	-133	G - F	849	-137	C - F	169	-1046
Loc	Gravity			Non-Gravity																																														
	R+	/R-	/Rh	/Rw	/U	/RL																																												
H	482	-	-	-	/91	-																																												
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D	122	-	-	-	/43	-																																												
B - C	149	-922																																																
B - G	865	-133	G - F	849	-137																																													
C - F	169	-1046																																																

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3;

**Plating Notes**  
(\*\*) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Hipjack supports 7-0-0 setback jacks with no webs.

**Wind**  
Wind loads and reactions based on MWFRS.  
Wind loading based on both gable and hip roof types.

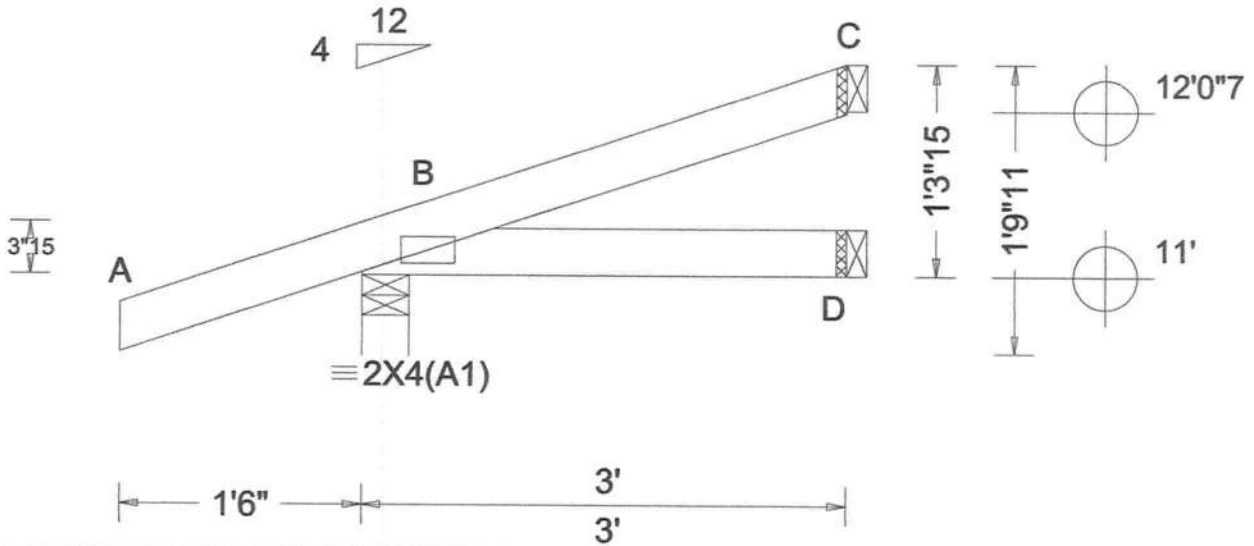


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.175 Max BC CSI: 0.059 Max Web CSI: 0.000  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity B 294 /- /- /217 /59 /49 D 48 /- /- /26 /- /- C 70 /- /- /40 /27 /-  Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

#### Wind

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.



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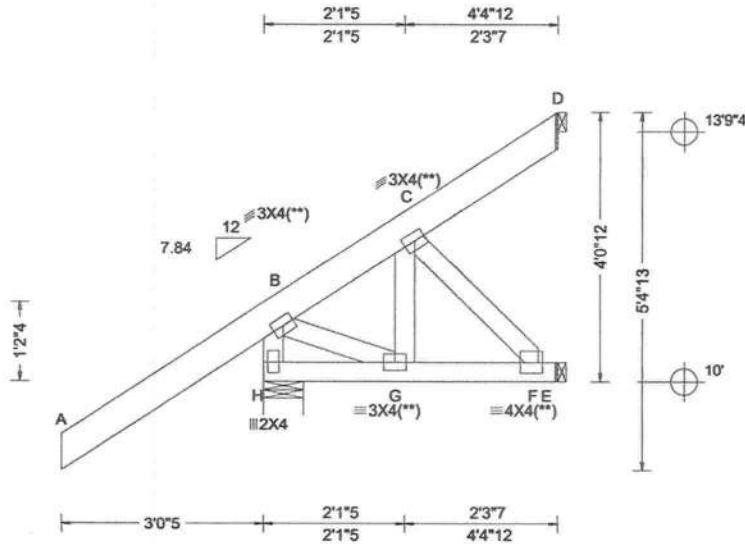
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>																																		
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 F 999 240 VERT(CL): -0.001 F 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.208 Max BC CSI: 0.027 Max Web CSI: 0.040  VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>377</td> <td>-</td> <td>-</td> <td>-</td> <td>/79</td> <td>-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-40</td> <td>-</td> <td>/25</td> <td>-</td> <td>-</td> </tr> <tr> <td>D</td> <td>74</td> <td>-</td> <td>-</td> <td>-</td> <td>/21</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS  H Brg Wid = 7.1 Min Req = 1.5 (Truss)  E Brg Wid = 1.5 Min Req = -  D Brg Wid = 1.5 Min Req = -  Bearing H is a rigid surface.  Members not listed have forces less than 375#  <b>Maximum Web Forces Per Ply (lbs)</b>  Webs Tens.Comp.</p> <p>H - B 80 -380</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	377	-	-	-	/79	-	E	-	/-40	-	/25	-	-	D	74	-	-	-	/21	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
H	377	-	-	-	/79	-																																
E	-	/-40	-	/25	-	-																																
D	74	-	-	-	/21	-																																

**Lumber**  
Top chord: 2x6 SP 2400f-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
(\*\*) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Hipjack supports 3-1-5 setback jacks with no webs.

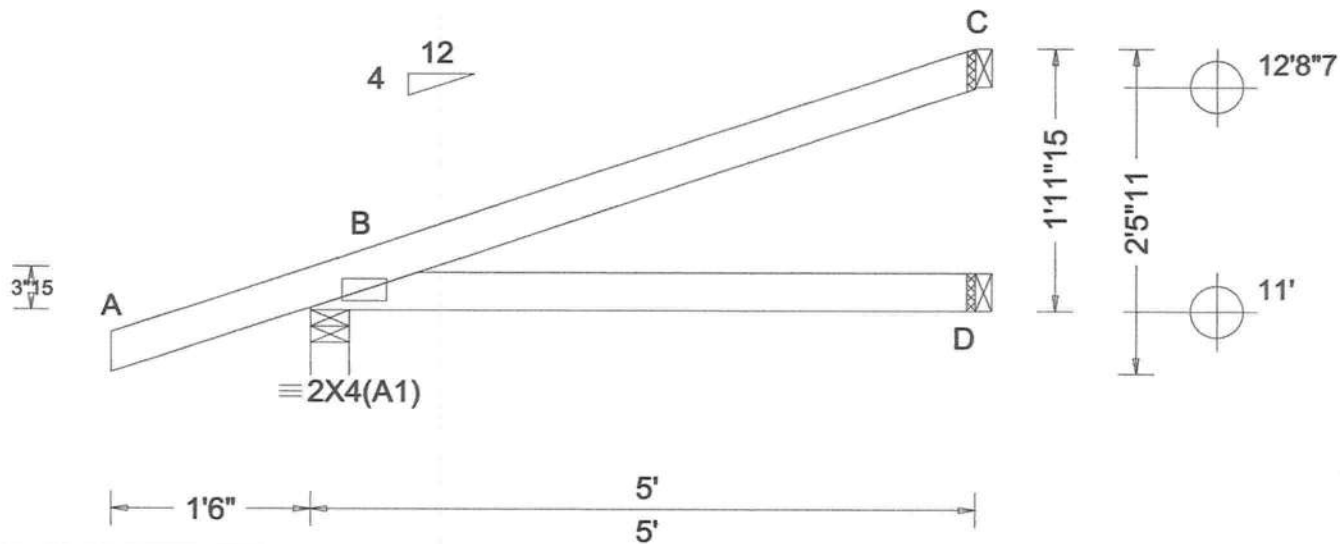
**Wind**  
Wind loads and reactions based on MWFRS.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.336 Max BC CSI: 0.234 Max Web CSI: 0.000  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 371 /- /- /266 /59 /73 D 90 /- /- /50 /- /- C 144 /- /- /88 /54 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

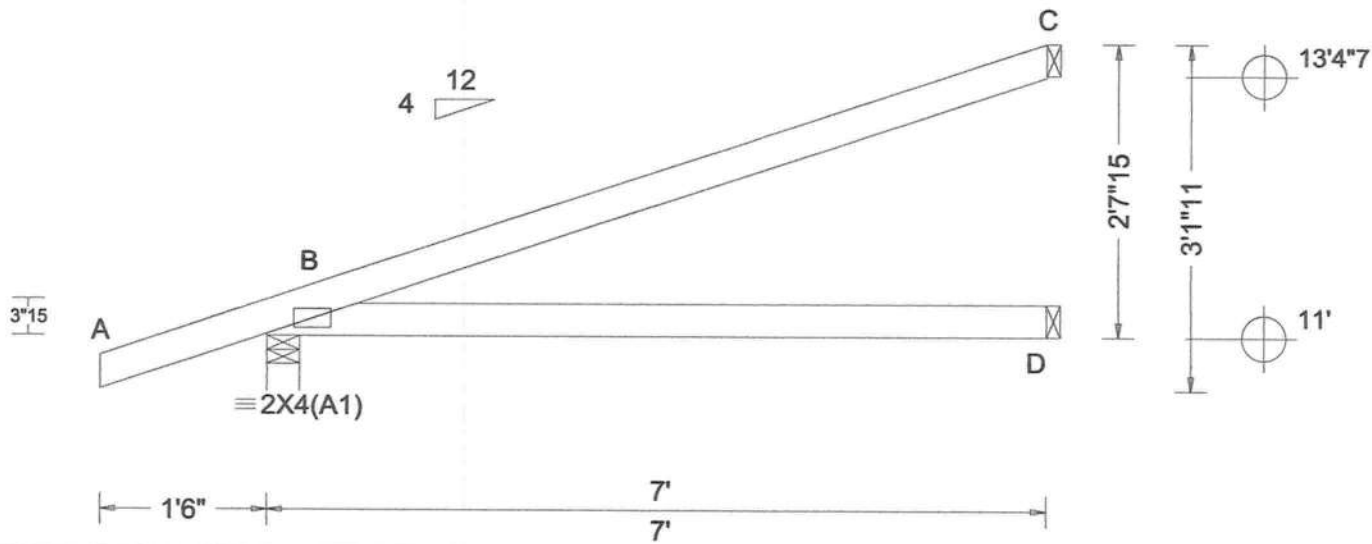
COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



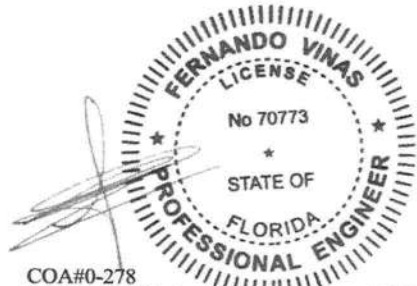
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																		
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.015 B - - HORZ(TL): 0.033 B - - Creep Factor: 2.0 Max TC CSI: 0.783 Max BC CSI: 0.525 Max Web CSI: 0.000  VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>456</td> <td>-</td> <td>-</td> <td>/322</td> <td>/68</td> <td>/96</td> </tr> <tr> <td>D</td> <td>130</td> <td>-</td> <td>-</td> <td>/75</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>212</td> <td>-</td> <td>-</td> <td>/131</td> <td>/78</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS            B Brg Wid = 3.5 Min Req = 1.5 (Truss)            D Brg Wid = 1.5 Min Req = -            C Brg Wid = 1.5 Min Req = -            Bearing B is a rigid surface.            Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	456	-	-	/322	/68	/96	D	130	-	-	/75	-	-	C	212	-	-	/131	/78	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
B	456	-	-	/322	/68	/96																																
D	130	-	-	/75	-	-																																
C	212	-	-	/131	/78	-																																

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

**Wind**

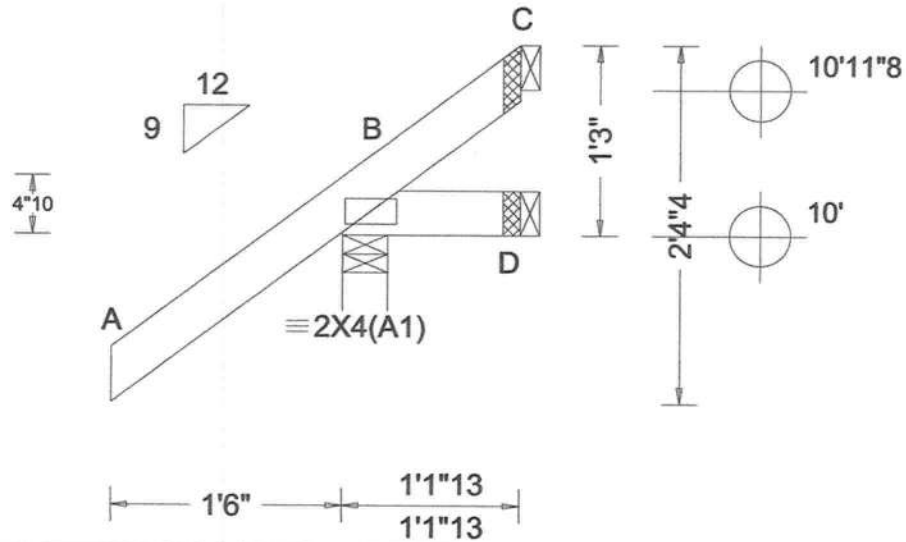
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.051 Max Web CSI: 0.000  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>255</td> <td>/-</td> <td>/-</td> <td>/220</td> <td>/52</td> <td>/60</td> </tr> <tr> <td>D</td> <td>9</td> <td>/-12</td> <td>/-</td> <td>/15</td> <td>/14</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-40</td> <td>/-</td> <td>/35</td> <td>/58</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	255	/-	/-	/220	/52	/60	D	9	/-12	/-	/15	/14	/-	C	-	/-40	/-	/35	/58	/-
				Loc		Gravity			Non-Gravity																													
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**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.



COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

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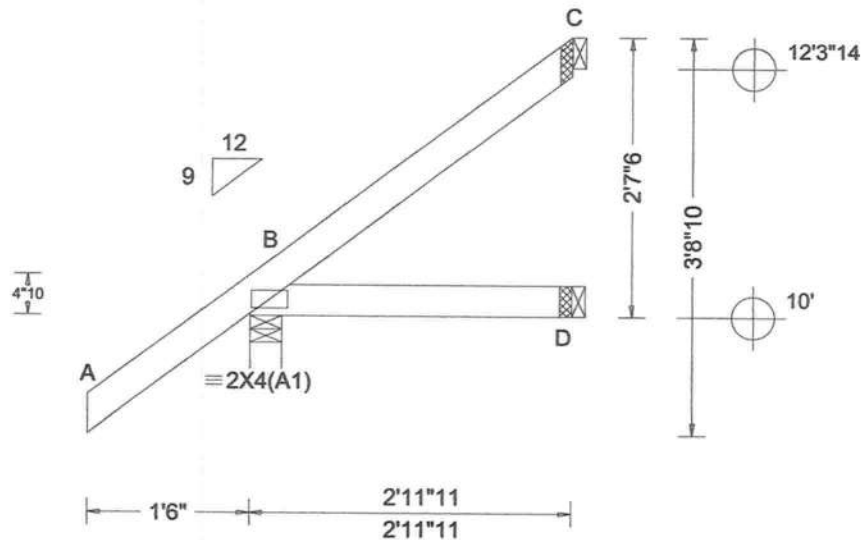
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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.067 Max Web CSI: 0.000  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 271 /- /- /205 /22 /109 D 50 /- /- /32 /- /- C 64 /- /- /47 /47 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.



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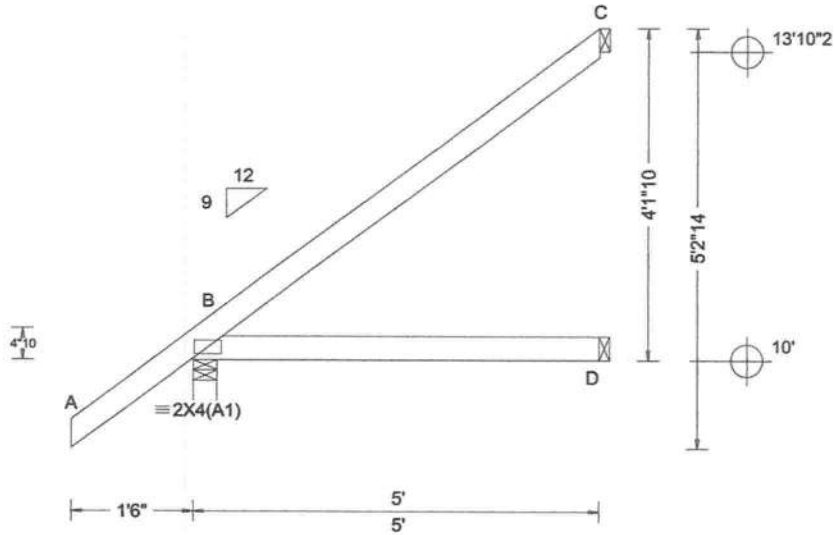
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL:	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	B	343	-	-	/244	/10	/163
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D	91	-	-	/53	-	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B - -	C	133	-	-	/105	/88	-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.007 B - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.487	D Brg Wid = 1.5 Min Req = -						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.246	C Brg Wid = 1.5 Min Req = -						
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT: 20(0)/10(0)	Max Web CSI: 0.000	Bearing B is a rigid surface.						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.00.1005.17	Members not listed have forces less than 375#						
	Loc. from endwall: Any	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

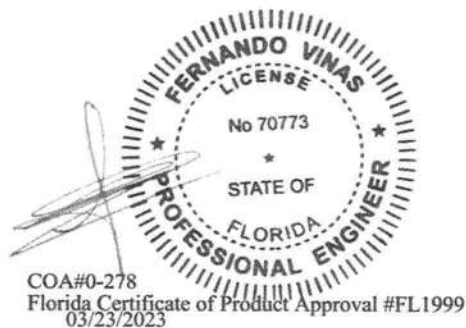
#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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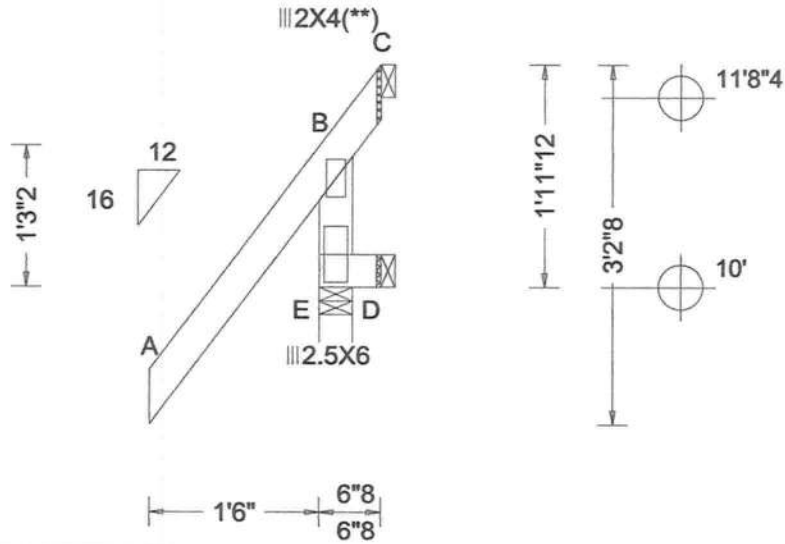
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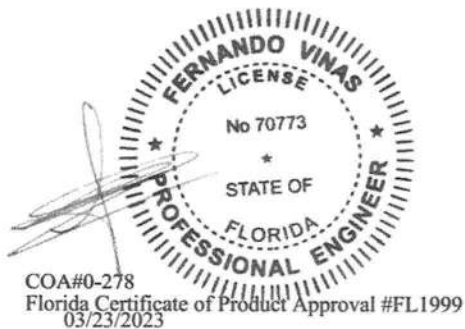


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.501 Max BC CSI: 0.003 Max Web CSI: 0.320  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E 312 /- /- /521 /209 /- D 11 /- /- /5 /- /- C - /-147 /- /187 /376 /96 Wind reactions based on MWFRS E Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 888 -373  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. B - E 1075 -516

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

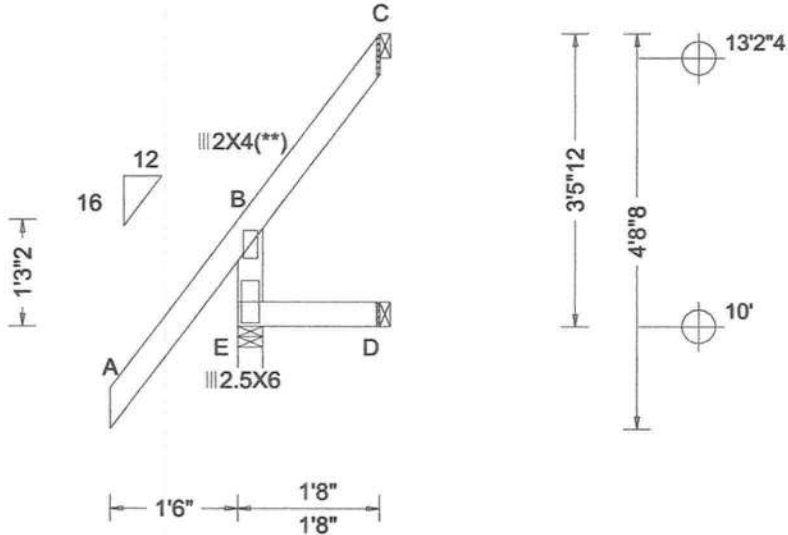
**Plating Notes**  
(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.002 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.510 Max BC CSI: 0.027 Max Web CSI: 0.253  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E 252 /- /- /402 /177 /- D 33 /- /- /17 /- /- C 7 /- /- /140 /185 /131 Wind reactions based on MWFRS E Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 722 -269  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. B - E 850 -386

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

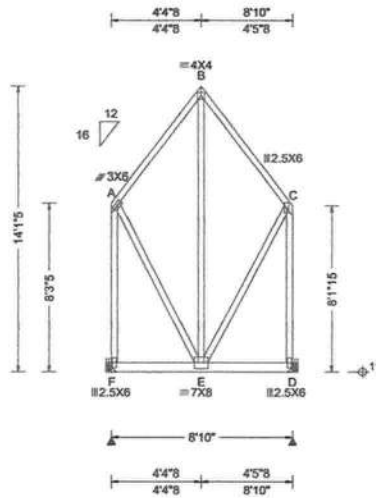


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→ 2 Complete Trusses Required



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.14 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.018 E 999 240 VERT(CL): 0.036 E 999 180 HORZ(LL): -0.006 C - - HORZ(TL): 0.011 C - - Creep Factor: 2.0 Max TC CSI: 0.236 Max BC CSI: 0.469 Max Web CSI: 0.724  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 4408 /- /- /- /528 /- D 2758 /- /- /- /365 /- Wind reactions based on MWFRS F Brg Wid = - Min Req = - D Brg Wid = - Min Req = - Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 95 -661 B - C 96 -662
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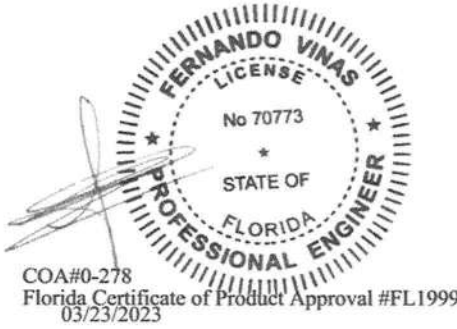
**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x6 SP 2400f-2.0E;  
Webs: 2x4 SP #3;

**Nailnote**  
Nail Schedule: 0.131"x3", min. nails  
Top Chord: 1 Row @ 12.00" o.c.  
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)  
Webs : 1 Row @ 4" o.c.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC: From 73 plf at 0.00 to 73 plf at 8.83  
BC: From 10 plf at 0.00 to 10 plf at 8.83  
BC: 2199 lb Conc. Load at 1.02, 3.02  
BC: 1021 lb Conc. Load at 5.02  
BC: 1012 lb Conc. Load at 7.02

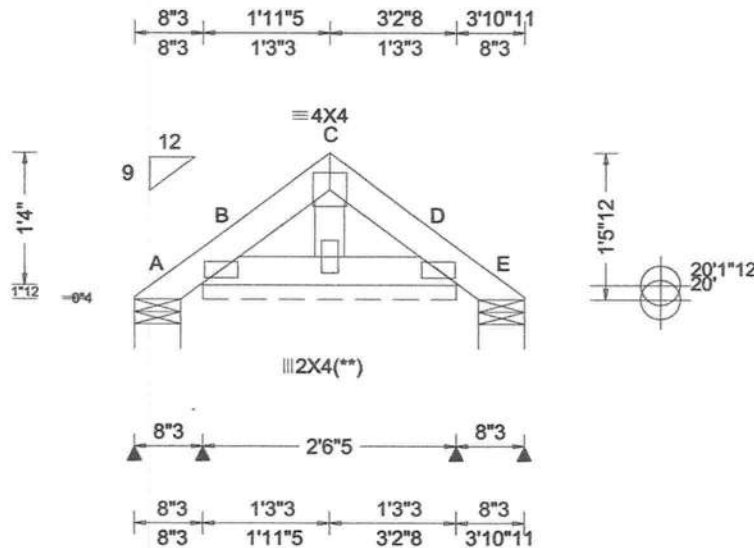
**Hangers / Ties**  
(J) Hanger Support Required, by others

**Wind**  
Wind loads and reactions based on MWFRS.  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.37 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.013 Max BC CSI: 0.007 Max Web CSI: 0.007  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 16 /- /- /32 /20 /38 B* 82 /- /- /65 /21 /- E 16 /- /- /15 /6 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) B Brg Wid = 30.3 Min Req = - E Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(A1) except as noted.  
(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**

Wind loads based on MWFRS.  
Wind loading based on both gable and hip roof types.

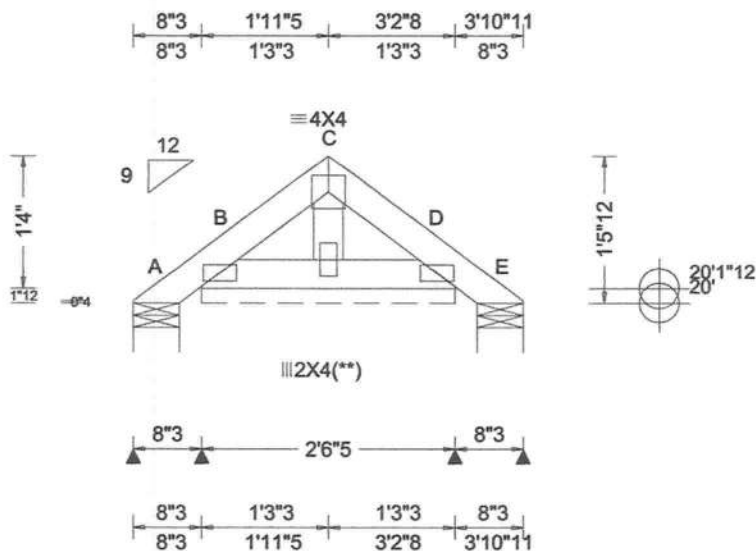
**Additional Notes**

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.37 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.013 Max BC CSI: 0.007 Max Web CSI: 0.007  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or * = PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 16 /- /- /32 /20 /38 B* 82 /- /- /65 /7 /- E 16 /- /- /15 /3 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) B Brg Wid = 30.3 Min Req = - E Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(A1) except as noted.  
(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

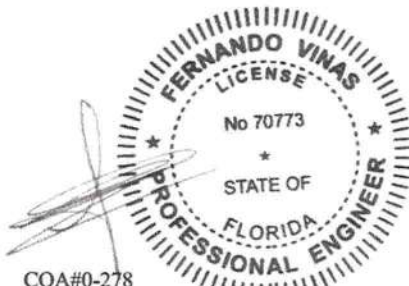
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

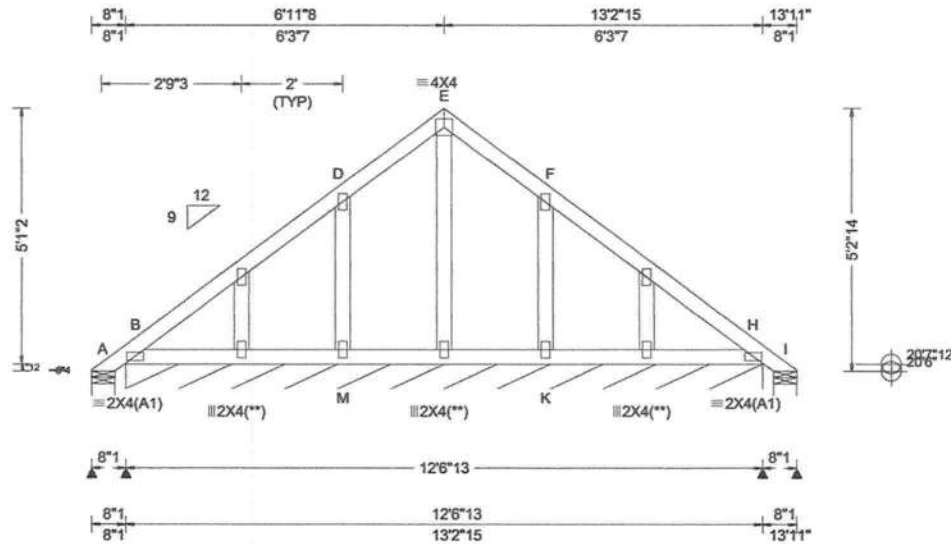
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.68 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.000 F 999 240 VERT(CL): 0.001 F 999 180 HORZ(LL): 0.001 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.051 Max BC CSI: 0.028 Max Web CSI: 0.039  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 3 /- /- /110 /103 /157 B* 74 /- /- /59 /8 /- I 3 /- /- /113 /6 /- M /-122 K /-122 Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) B Brg Wid = 150 Min Req = - I Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4 except as noted.  
(\*\*) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

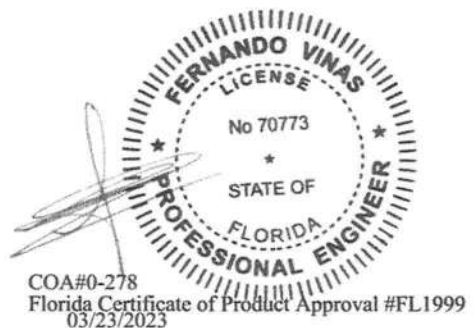
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

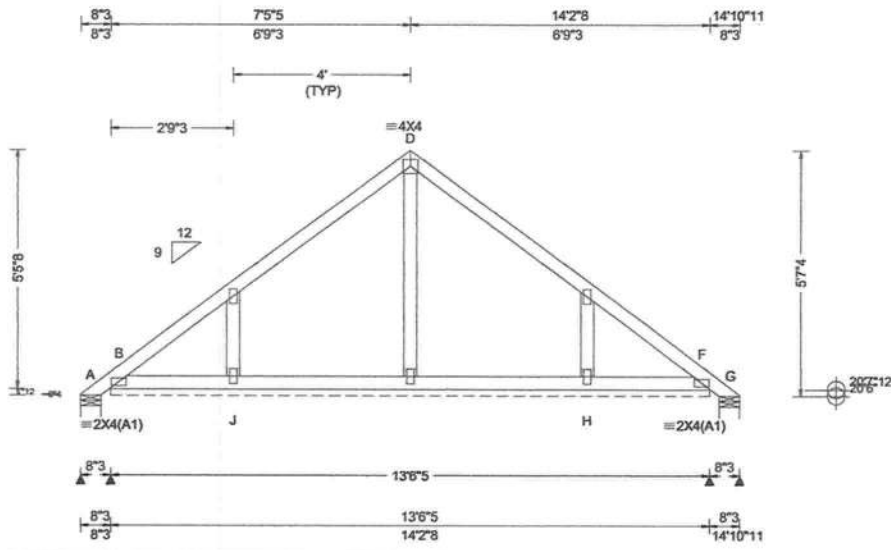
**Additional Notes**

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or * = PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.68 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): 0.002 E - - HORZ(TL): 0.003 E - - Creep Factor: 2.0 Max TC CSI: 0.223 Max BC CSI: 0.061 Max Web CSI: 0.088  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 29 /-3 /- /115 /102 /169 B* 71 /- /- /157 /8 /- G 19 /- /- /112 /- /- J /-107 H /-109 Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) B Brg Wid = 162 Min Req = - G Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(\*\*) except as noted.  
(\*\*) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

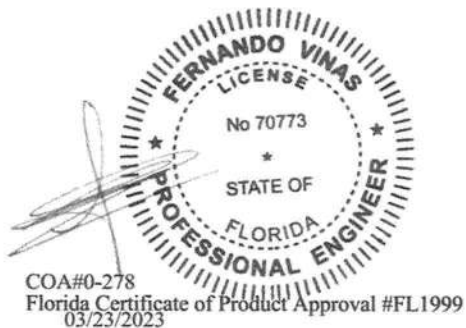
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

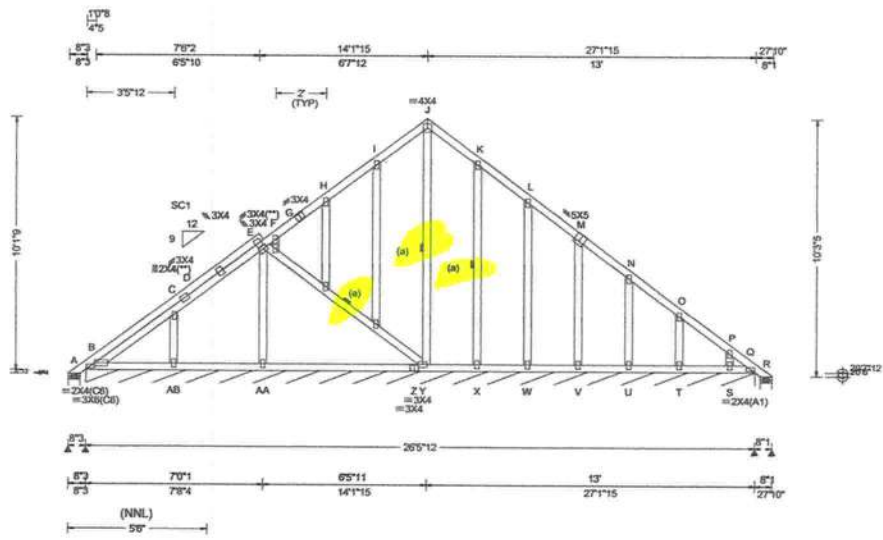
**Additional Notes**

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.20 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.080   999 240 VERT(CL): 0.138   999 180 HORZ(LL): 0.060   - - HORZ(TL): 0.104   - - Creep Factor: 2.0 Max TC CSI: 0.410 Max BC CSI: 0.109 Max Web CSI: 0.658  VIEW Ver: 21.02.00.1005.17	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL A - / -110 / - / 209 / 266 / 316 B* 75 / - / - / 57 / 5 / - R 19 / - / - / 14 / - / - B / -160 Y / -103 X / -126 W / -107  <b>Non-Gravity</b> Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) B Brg Wid = 317 Min Req = - R Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A, B, & R are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp.  A - B 415 -354

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 2X4 except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

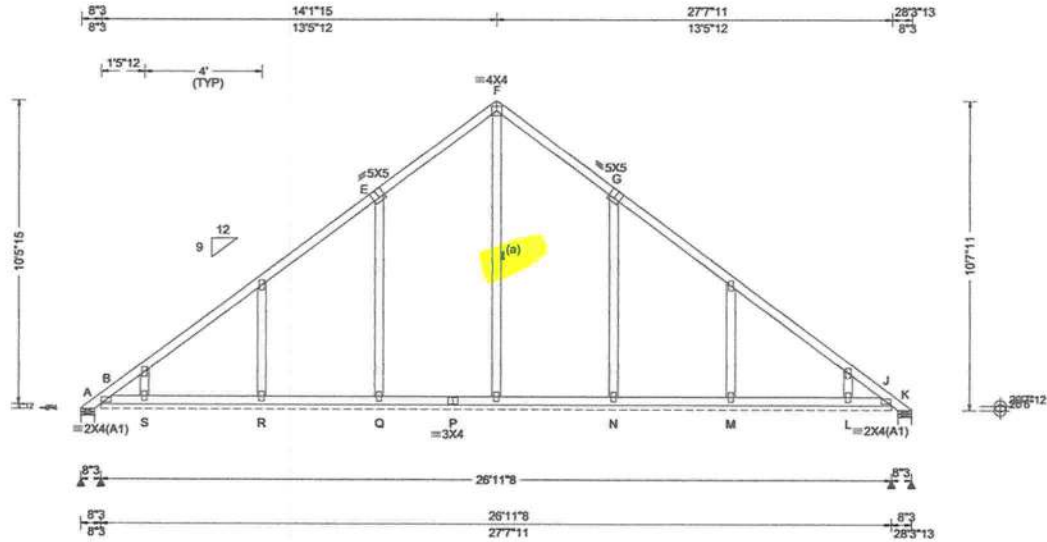
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
Refer to DWG PB160160118 for piggyback details.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or * = PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.20 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 G 999 240 VERT(CL): 0.004 E 999 180 HORZ(LL): 0.007 G - - HORZ(TL): 0.010 G - - Creep Factor: 2.0 Max TC CSI: 0.217 Max BC CSI: 0.077 Max Web CSI: 0.295  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 29 /- /- /228 /206 /337 B* 69 /- /- /56 /8 /- K 29 /- /- /19 /- /- S /-156 R /-174 Q /-201 N /-227 M /-174 L /-156  Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) B Brg Wid = 323 Min Req = - K Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A, B, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. A - B 400 -406

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**

All plates are 2X4 except as noted.

**Loading**

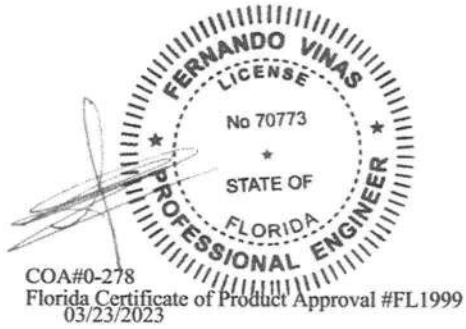
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

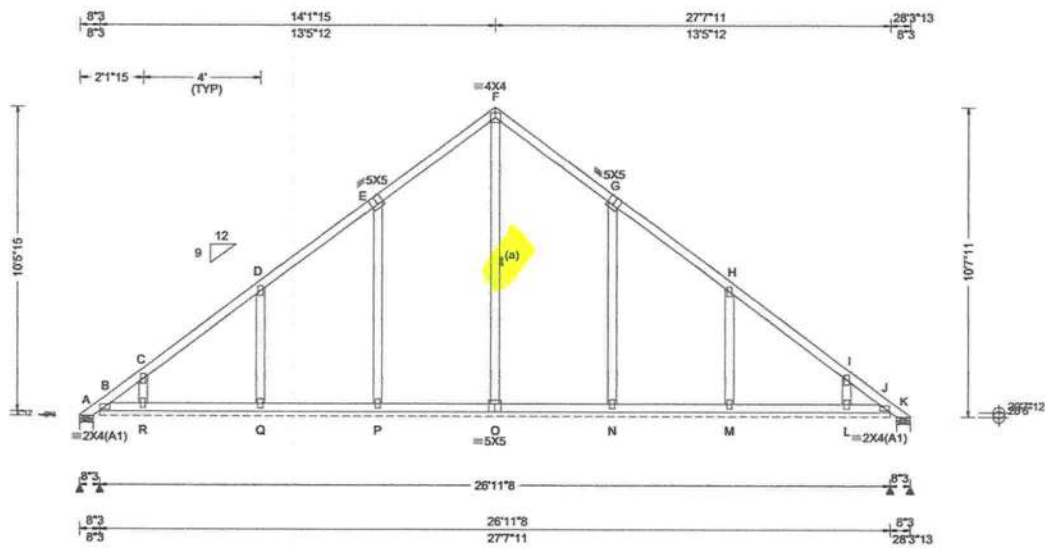
**Additional Notes**

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Refer to DWG PB160160118 for piggyback details.



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**Loading Criteria (psf)**

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

**Wind Criteria**

Wind Std: ASCE 7-16  
 Speed: 130 mph  
 Enclosure: Closed  
 Risk Category: II  
 EXP: C Kzt: NA  
 Mean Height: 25.83 ft  
 TCDL: 5.0 psf  
 BCDL: 2.0 psf  
 MWFRS Parallel Dist: h to 2h  
 C&C Dist a: 4.25 ft  
 Loc. from endwall: not in 9.00 ft  
 GCpi: 0.18  
 Wind Duration: 1.60

**Snow Criteria (Pg,Pf in PSF)**

Pg: NA Ct: NA CAT: NA  
 Pf: NA Ce: NA  
 Lu: NA Cs: NA  
 Snow Duration: NA

Building Code:  
 FBC 7th Ed. 2020 Res.  
 TPI Std: 2014  
 Rep Fac: Yes  
 FT/RT:20(0)/10(0)  
 Plate Type(s):  
 WAVE

**Defl/CSI Criteria**

PP Deflection in loc L/defl L/#

VERT(LL):	0.002 G	999	240
VERT(CL):	0.004 G	999	180
HORZ(LL):	0.007 G	-	-
HORZ(TL):	0.010 G	-	-

Creep Factor: 2.0  
 Max TC CSI: 0.214  
 Max BC CSI: 0.132  
 Max Web CSI: 0.295

VIEW Ver: 21.02.00.1005.17

**▲ Maximum Reactions (lbs), or \*=PLF**

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
A	27	/-	/-	/209	/194	/316
B*	91	/-	/-	/54	/15	/-
K	27	/-	/-	/13	/-	/-
R		/-143				
Q		/-159				
P		/-183				
N		/-183				
M		/-159				
L		/-143				

Wind reactions based on MWFRS  
 A Brg Wid = 5.5 Min Req = 1.5 (Truss)  
 B Brg Wid = 323 Min Req = -  
 K Brg Wid = 5.5 Min Req = 1.5 (Truss)  
 Bearings A, B, & K are a rigid surface.  
 Members not listed have forces less than 375#

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

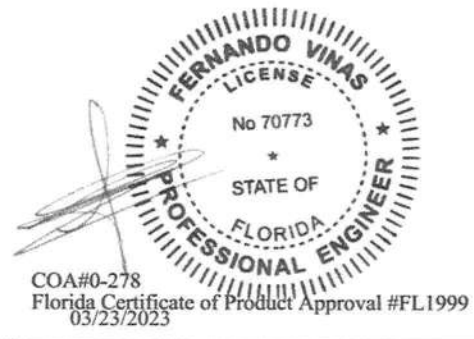
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 2X4 except as noted.

**Loading**  
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**  
 Wind loads based on MWFRS.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 Refer to DWG PB160160118 for piggyback details.



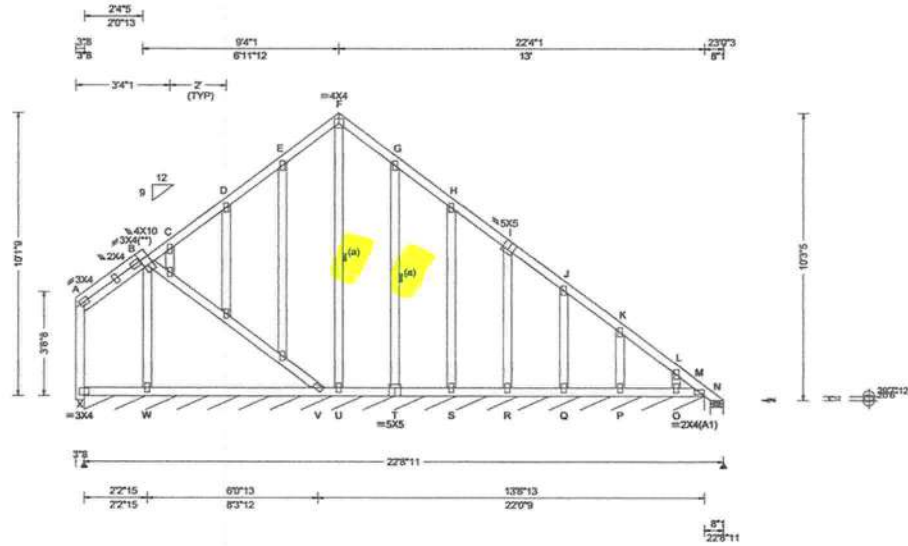
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 25.83 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.078 D 999 240 VERT(CL): 0.134 D 999 180 HORZ(LL): 0.050 D - - HORZ(TL): 0.087 D - - Creep Factor: 2.0 Max TC CSI: 0.419 Max BC CSI: 0.110 Max Web CSI: 0.813  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL X* 73 /- /- /52 /10 /14 N 12 /- /- /15 /0 /- Wind reactions based on MWFRS X Brg Wid = 264 Min Req = - N Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings X & N are a rigid surface. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**

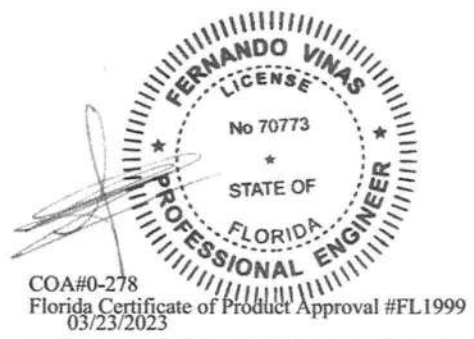
All plates are 2X4 except as noted.  
(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Left cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.

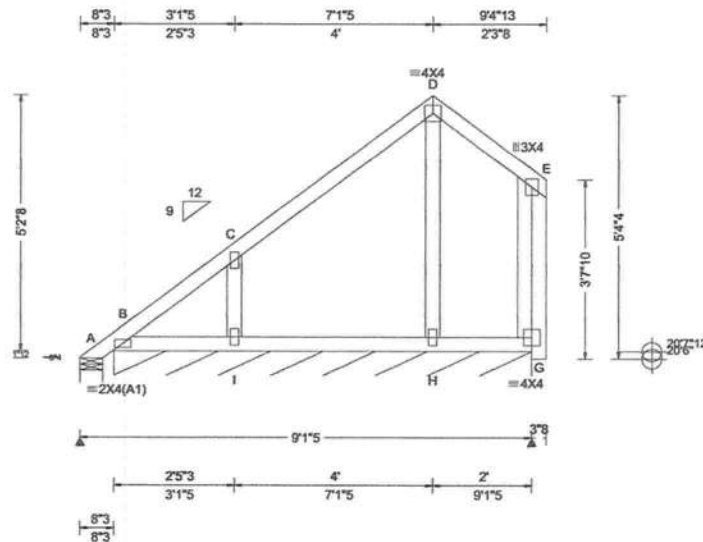
**Additional Notes**

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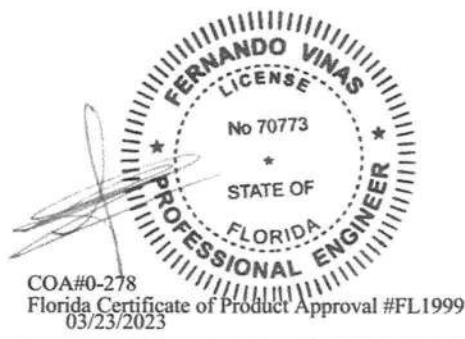
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 2X4 except as noted.

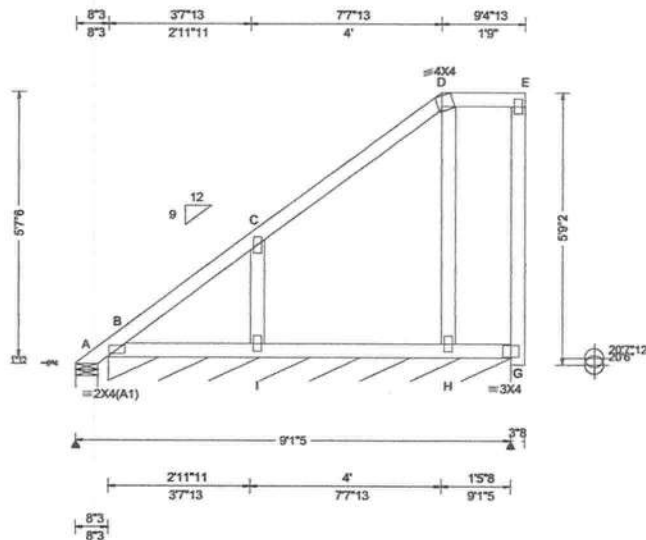
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Right cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
Refer to DWG PB160160118 for piggyback details.



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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.047 Max Web CSI: 0.089 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-1</td> <td>/-</td> <td>/140</td> <td>/67</td> <td>/198</td> </tr> <tr> <td>B*</td> <td>75</td> <td>/-</td> <td>/-</td> <td>/69</td> <td>/38</td> <td>/-</td> </tr> <tr> <td>I</td> <td></td> <td></td> <td>/-107</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS          A Brg Wid = 5.5 Min Req = 1.5 (Truss)          B Brg Wid = 101 Min Req = -          Bearings A &amp; B are a rigid surface.          Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-1	/-	/140	/67	/198	B*	75	/-	/-	/69	/38	/-	I			/-107			
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
A	-	/-1	/-	/140	/67	/198																																
B*	75	/-	/-	/69	/38	/-																																
I			/-107																																			

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Plating Notes**  
 All plates are 2X4 except as noted.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS.  
 Right end vertical not exposed to wind pressure.  
 Right cantilever is exposed to wind  
 Wind loading based on both gable and hip roof types.

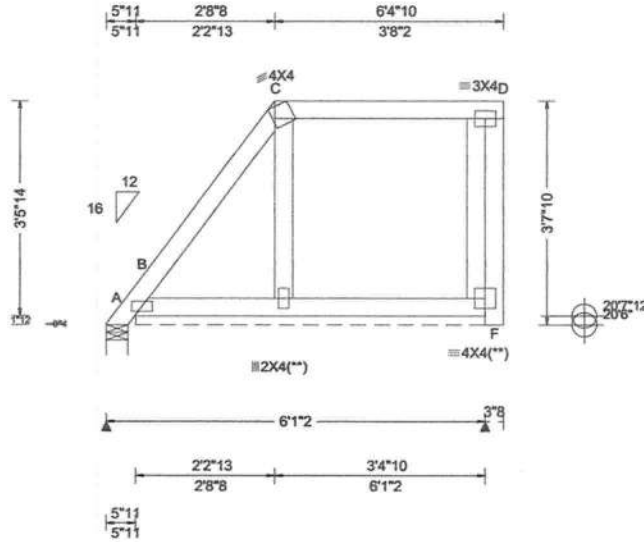
**Additional Notes**  
 Refer to DWG PB160160118 for piggyback details.



COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

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<b>Loading Criteria</b> (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.64 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria</b> (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.268 Max BC CSI: 0.042 Max Web CSI: 0.083  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or * = PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-42 /- /149 /155 /93 B* 93 /- /- /89 /47 /- Wind reactions based on MWFRS A Brg Wid = 4.2 Min Req = 1.5 (Truss) B Brg Wid = 67.5 Min Req = - Bearings A & B are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. A - B 186 -390
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(A1) except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**

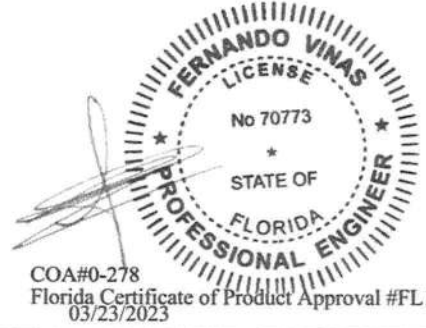
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

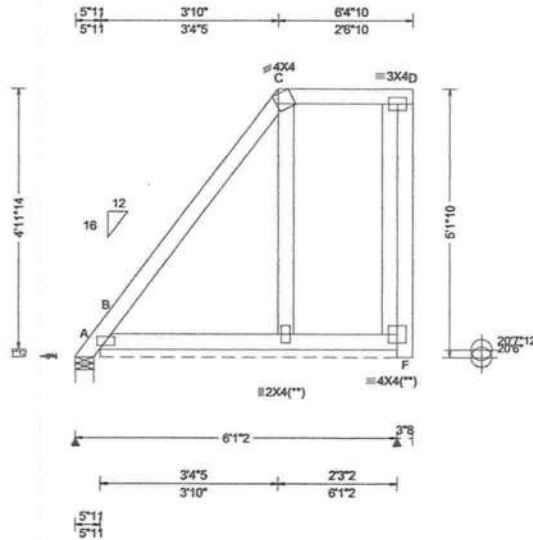
**Additional Notes**

See DWGS A14030ENC160118 & GBULLETIN0118 for gable wind bracing and other requirements.  
Refer to DWG PB160160118 for piggyback details.



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.14 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.255 Max BC CSI: 0.047 Max Web CSI: 0.083  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-143 /- /283 /347 /135 B* 111 /- /- /131 /68 /- Wind reactions based on MWFRS A Brg Wid = 4.2 Min Req = 1.5 (Truss) B Brg Wid = 67.5 Min Req = - Bearings A & B are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. A - B 362 -679
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(A1) except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**

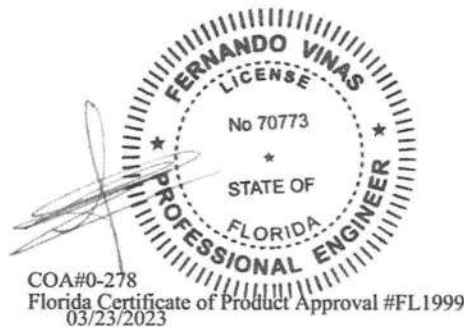
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

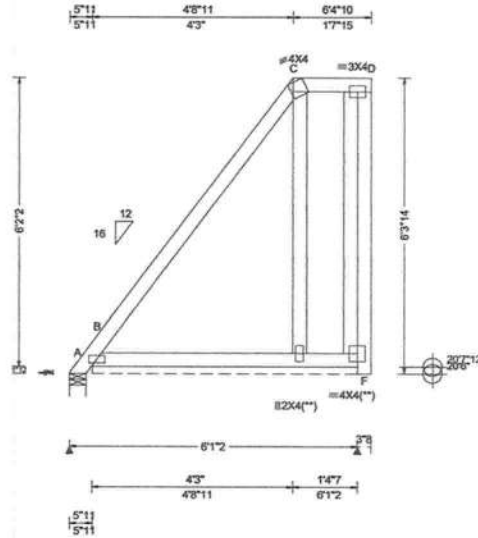
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.32 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): -0.005 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.408 Max BC CSI: 0.075 Max Web CSI: 0.121  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-259 /- /422 /553 /170 B* 132 /- /- /173 /90 /- Wind reactions based on MWFRS A Brg Wid = 4.2 Min Req = 1.5 (Truss) B Brg Wid = 67.5 Min Req = - Bearings A & B are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. A - B 544 -945
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(A1) except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**

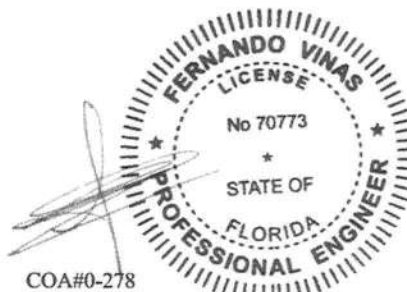
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

Negative reaction(s) of -259# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.  
See DWGS A14030ENC160118 & GBULLETIN0118 for gable wind bracing and other requirements.  
Refer to DWG PB160160118 for piggyback details.



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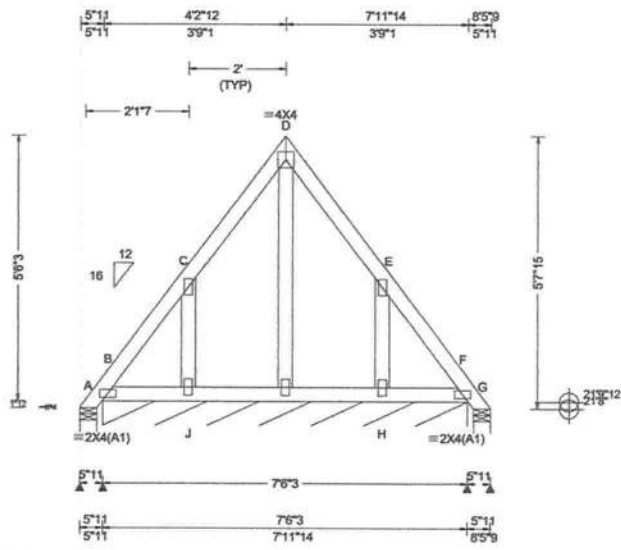
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155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.003 E - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.084 Max BC CSI: 0.034 Max Web CSI: 0.077  VIEW Ver: 21.02.00.1005.17	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL A 11 /- /- /222 /209 /198 B* 81 /- /- /87 /29 /- G 11 /- /- /38 /25 /- J /-113 H /-123 <b>Wind reactions based on MWFRS</b> A Brg Wid = 4.2 Min Req = 1.5 (Truss) B Brg Wid = 90.2 Min Req = - G Brg Wid = 4.2 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 2X4 except as noted.

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

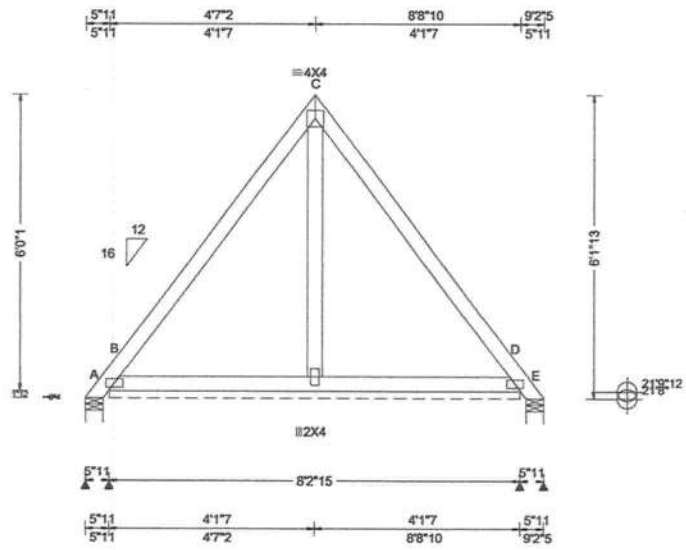
**Additional Notes**  
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Refer to DWG PB160160118 for piggyback details.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or * = PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): -0.003 D - - HORZ(TL): 0.005 D - - Creep Factor: 2.0 Max TC CSI: 0.279 Max BC CSI: 0.089 Max Web CSI: 0.052  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A - /-217 /- /431 /557 /216 B* 136 /- /- /126 /34 /- E - /-217 /- /232 /358 /- D /-217 Wind reactions based on MWFRS A Brg Wid = 4.2 Min Req = 1.5 (Truss) B Brg Wid = 98.9 Min Req = - E Brg Wid = 4.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp.  A - B 588 -474

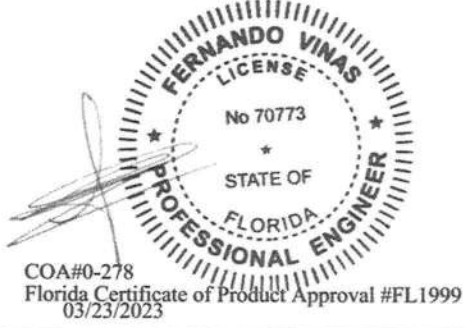
**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 2X4(A1) except as noted.

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

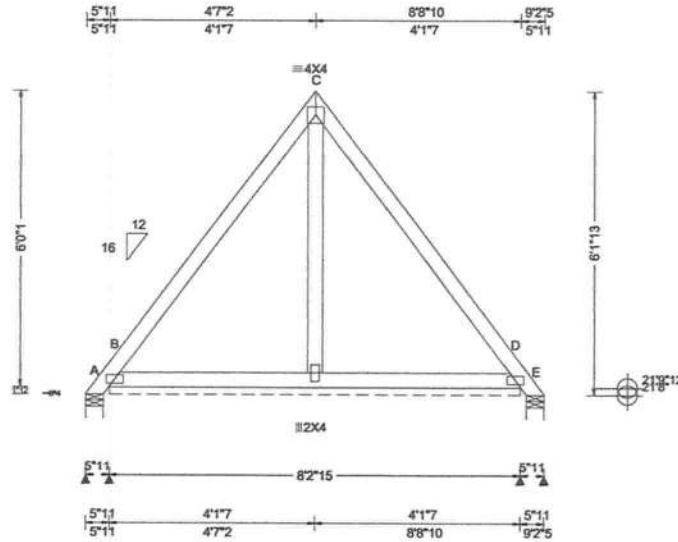
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
Negative reaction(s) of -217# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.  
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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): -0.003 D - - HORZ(TL): 0.005 D - - Creep Factor: 2.0 Max TC CSI: 0.279 Max BC CSI: 0.089 Max Web CSI: 0.052 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-217</td> <td>/-</td> <td>/431</td> <td>/557</td> <td>/216</td> </tr> <tr> <td>B*</td> <td>136</td> <td>/-</td> <td>/-</td> <td>/126</td> <td>/51</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-217</td> <td>/-</td> <td>/232</td> <td>/358</td> <td>/-</td> </tr> <tr> <td>D</td> <td>-</td> <td>/-226</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS          A Brg Wid = 4.2 Min Req = 1.5 (Truss)          B Brg Wid = 98.9 Min Req = -          E Brg Wid = 4.2 Min Req = 1.5 (Truss)          Bearings A, B, &amp; E are a rigid surface.          Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b>          Chords Tens.Comp.          A - B 588 -474</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-217	/-	/431	/557	/216	B*	136	/-	/-	/126	/51	/-	E	-	/-217	/-	/232	/358	/-	D	-	/-226	-	-	-	-
Loc	Gravity			Non-Gravity																																									
	R+	/R-	/Rh	/Rw	/U	/RL																																							
A	-	/-217	/-	/431	/557	/216																																							
B*	136	/-	/-	/126	/51	/-																																							
E	-	/-217	/-	/232	/358	/-																																							
D	-	/-226	-	-	-	-																																							

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4(A1) except as noted.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**

Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

**Additional Notes**

Negative reaction(s) of -217# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA#0-278  
 Florida Certificate of Product Approval #FL1999  
 03/23/2023

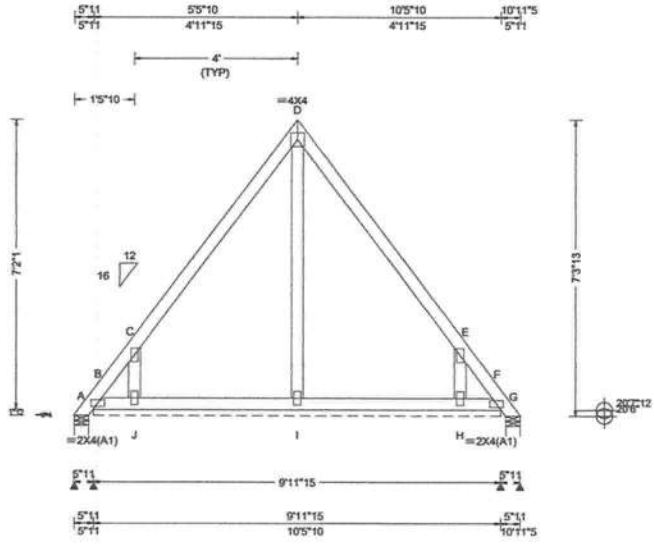
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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



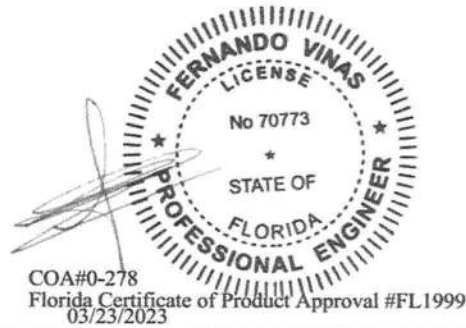
<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 24.17 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): 0.004 E - - HORZ(TL): 0.006 E - - Creep Factor: 2.0 Max TC CSI: 0.412 Max BC CSI: 0.074 Max Web CSI: 0.193  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 41 /- /- /283 /256 /269 B* 74 /- /- /82 /15 /- G 41 /- /- /31 /4 /- J /-290 H /-290 F /-261  Wind reactions based on MWFRS A Brg Wid = 4.2 Min Req = 1.5 (Truss) B Brg Wid = 119 Min Req = - G Brg Wid = 4.2 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 370 -388 E - F 284 -417 B - C 335 -431  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. C - J 647 -467 H - E 647 -466
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 2X4(\*\*) except as noted.  
(\*\*) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

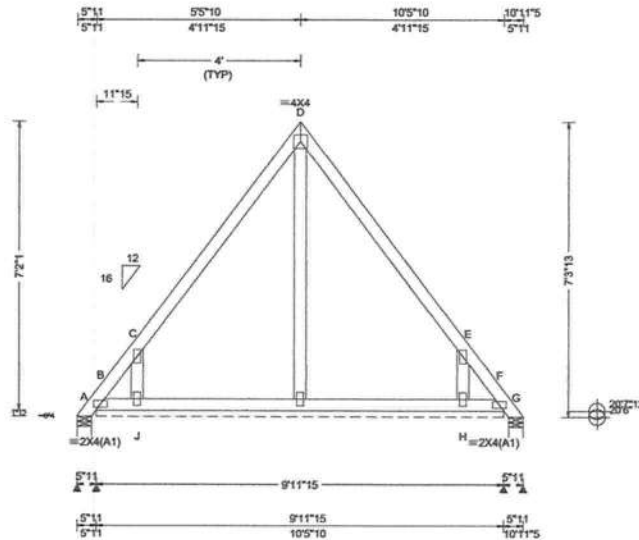
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
Refer to DWG PB160160118 for piggyback details.



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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwalk: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): 0.004 E - - HORZ(TL): 0.005 E - - Creep Factor: 2.0 Max TC CSI: 0.301 Max BC CSI: 0.080 Max Web CSI: 0.139 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>42</td> <td>-</td> <td>-</td> <td>1277</td> <td>1244</td> <td>1258</td> </tr> <tr> <td>B*</td> <td>74</td> <td>-</td> <td>-</td> <td>185</td> <td>113</td> <td>-</td> </tr> <tr> <td>G</td> <td>42</td> <td>-</td> <td>-</td> <td>134</td> <td>13</td> <td>-</td> </tr> <tr> <td>J</td> <td></td> <td></td> <td>-248</td> <td></td> <td></td> <td></td> </tr> <tr> <td>H</td> <td></td> <td></td> <td>-270</td> <td></td> <td></td> <td></td> </tr> <tr> <td>F</td> <td></td> <td></td> <td>-150</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS          A Brg Wid = 4.2 Min Req = 1.5 (Truss)          B Brg Wid = 119 Min Req = -          G Brg Wid = 4.2 Min Req = 1.5 (Truss)          Bearings A, B, &amp; G are a rigid surface.          Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b>          Chords Tens.Comp.          A - B 356 -377</p> <p><b>Maximum Gable Forces Per Ply (lbs)</b>          Gables Tens.Comp. Gables Tens. Comp.          C - J 468 -457 H - E 468 -457</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	42	-	-	1277	1244	1258	B*	74	-	-	185	113	-	G	42	-	-	134	13	-	J			-248				H			-270				F			-150			
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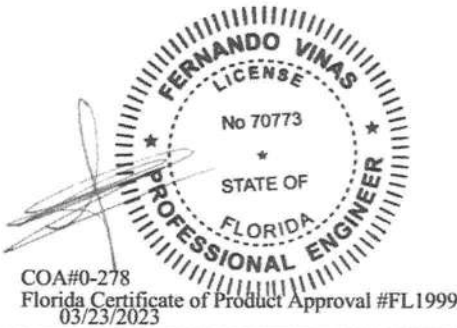
**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Plating Notes**  
 All plates are 2X4(\*\*) except as noted.  
 (\*\*) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

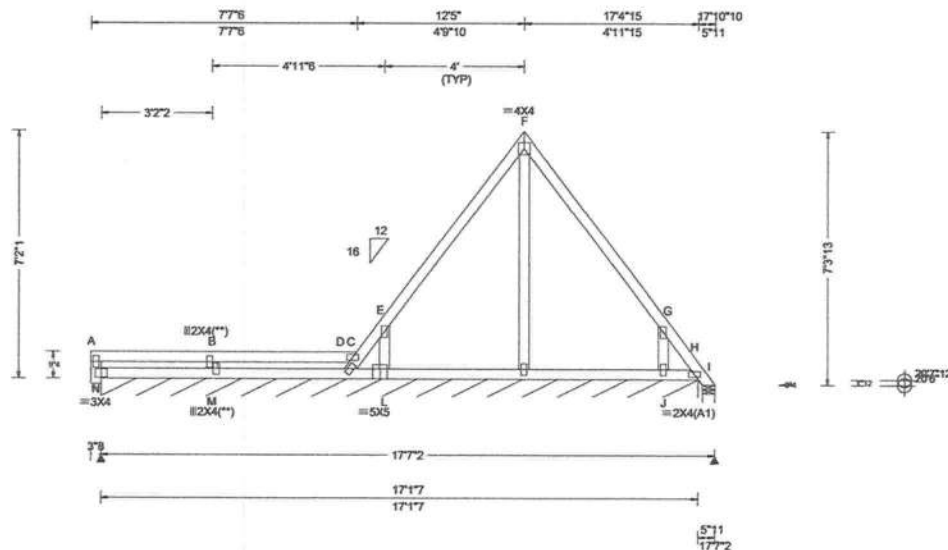
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.34 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 F 999 240 VERT(CL): 0.002 F 999 180 HORZ(LL): 0.004 G - - HORZ(TL): 0.004 G - - Creep Factor: 2.0 Max TC CSI: 0.309 Max BC CSI: 0.074 Max Web CSI: 0.151  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL N* 76 /- /- /44 /12 /15 I 41 /- /- /34 /3 /- Wind reactions based on MWFRS N Brg Wid = 205 Min Req = - I Brg Wid = 4.2 Min Req = 1.5 (Truss) Bearings N & I are a rigid surface. Members not listed have forces less than 375# <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. E - L 477 -453 J - G 509 -466

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4 except as noted.  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**

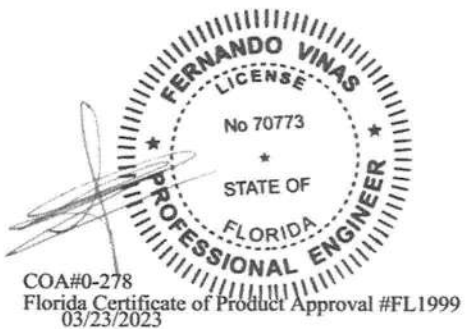
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

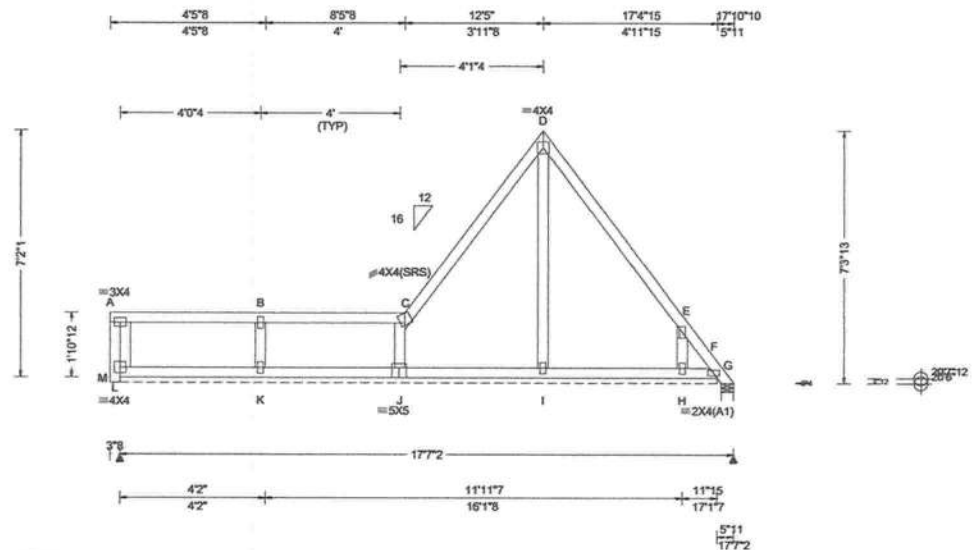
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.90 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.004 D 999 180 HORZ(LL): -0.016 C - - HORZ(TL): 0.022 C - - Creep Factor: 2.0 Max TC CSI: 0.415 Max BC CSI: 0.088 Max Web CSI: 0.171  VIEW Ver: 21.02.00.1005.17	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL M* 78 /- /- /54 /15 /15 G 40 /- /- /66 /39 /- <b>Non-Gravity</b> Wind reactions based on MWFRS M Brg Wid = 205 Min Req = - G Brg Wid = 4.2 Min Req = 1.5 (Truss) Bearings M & G are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. E - F 463 -448

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

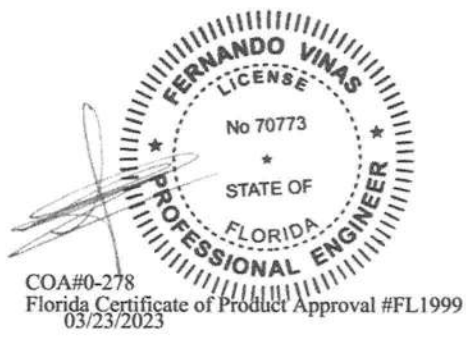
**Plating Notes**  
All plates are 2X4 except as noted.

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
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**Maximum Web Forces Per Ply (lbs)**

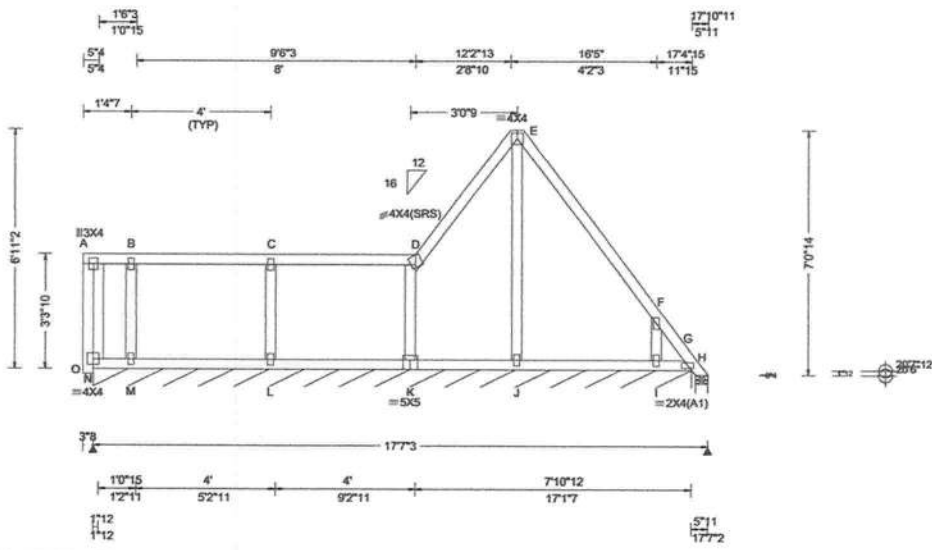
Webs	Tens.Comp.	Webs	Tens. Comp.
B - K	371 -381	H - E	489 -472

**Maximum Gable Forces Per Ply (lbs)**

Gables	Tens.Comp.
D - I	221 -375

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.14 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 E 999 240 VERT(CL): 0.002 E 999 180 HORZ(LL): -0.005 A - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.260 Max BC CSI: 0.030 Max Web CSI: 0.161  VIEW Ver: 21.02.00.1005.17	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL O* 71 /- /- /- /15 /4 H 36 /- /- /- /18 /- K /-105 <b>Non-Gravity</b> Wind reactions based on MWFRS O Brg Wid = 205 Min Req = - H Brg Wid = 4.2 Min Req = 1.5 (Truss) Bearings O & H are a rigid surface. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**

All plates are 2X4 except as noted.

**Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads and reactions based on MWFRS.  
Left end vertical not exposed to wind pressure.  
Left cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.

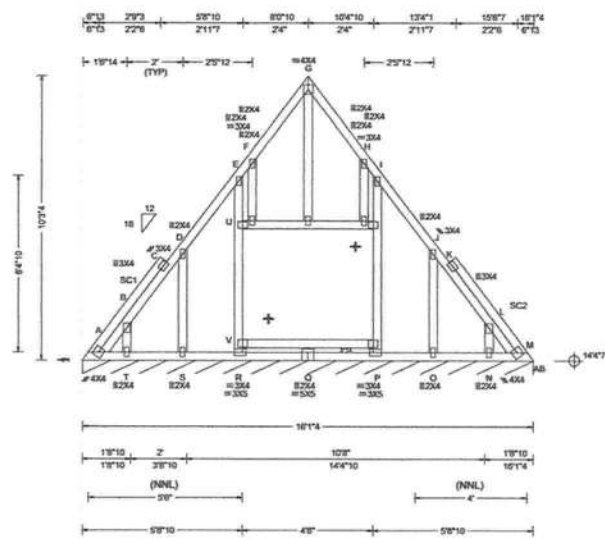
**Additional Notes**

Refer to DWG PB160160118 for piggyback details.



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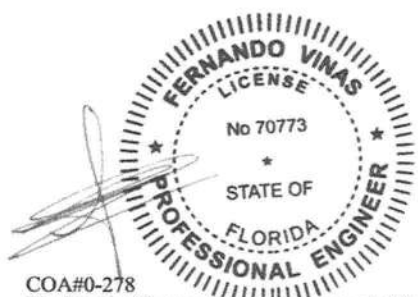


<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or * = PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.65 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 F 999 240 VERT(CL): 0.010 F 999 180 HORZ(LL): -0.015 F - - HORZ(TL): 0.018 F - - Creep Factor: 2.0 Max TC CSI: 0.174 Max BC CSI: 0.124 Max Web CSI: 0.320  VIEW Ver: 21.02.00.1005.17	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL AB*87 /- /- /80 /16 /39 Wind reactions based on MWFRS AB Brg Wid = 193 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 281 -394 D - E 262 -445 A - C 265 -406 J - K 264 -382 C - D 461 -594

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

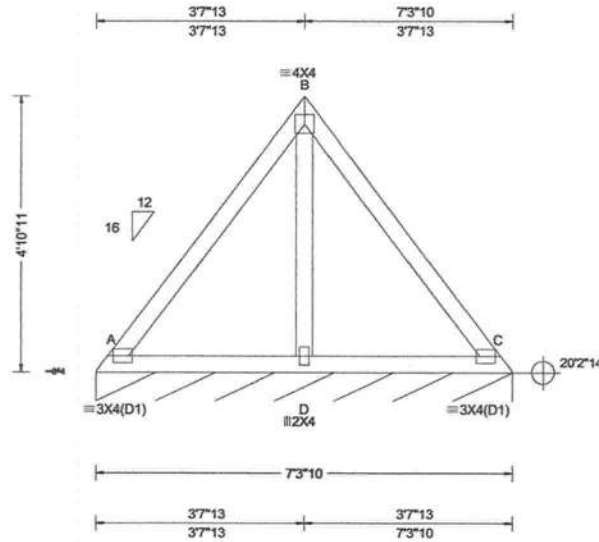
**Additional Notes**  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
See DWGS VALTN160118 and VAL180160118 for valley details.  
See DWGS A14030ENC160118 & GBLETIN0118 for gable wind bracing and other requirements.  
+ MEMBER TO BE Laterally Braced for HORIZONTAL WIND LOADS.



COA#0-278  
Florida Certificate of Product Approval #FL1999  
03/23/2023

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.84 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.008 A 999 180 HORZ(LL): -0.004 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.414 Max BC CSI: 0.242 Max Web CSI: 0.112  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity C* 93 /- /- /56 /6 /23 Wind reactions based on MWFRS C Brg Wid = 87.6 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

**Lumber**

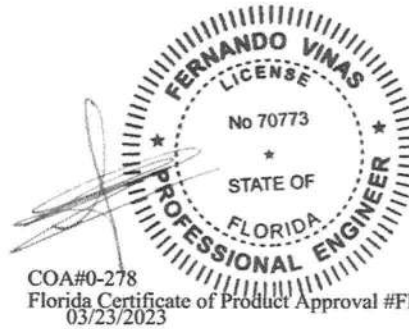
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

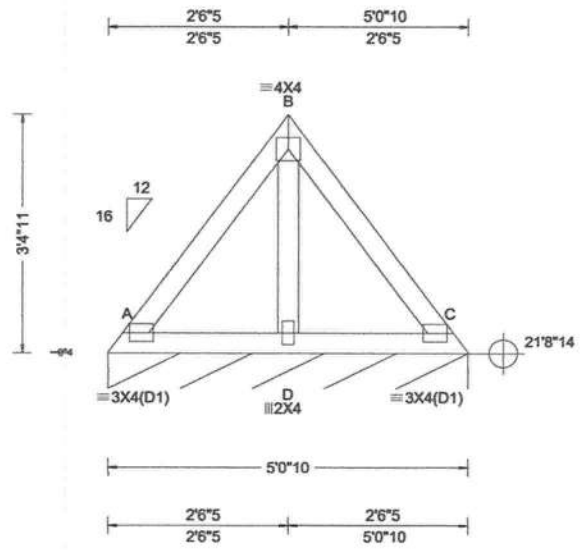
**Additional Notes**

See DWGS VALTN160118 and VAL180160118 for valley details.



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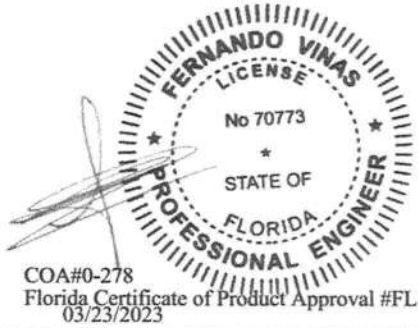


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 23.59 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.003 A 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.203 Max BC CSI: 0.119 Max Web CSI: 0.060  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity C* 92 /- /- /55 /5 /22 Wind reactions based on MWFRS C Brg Wid = 60.6 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

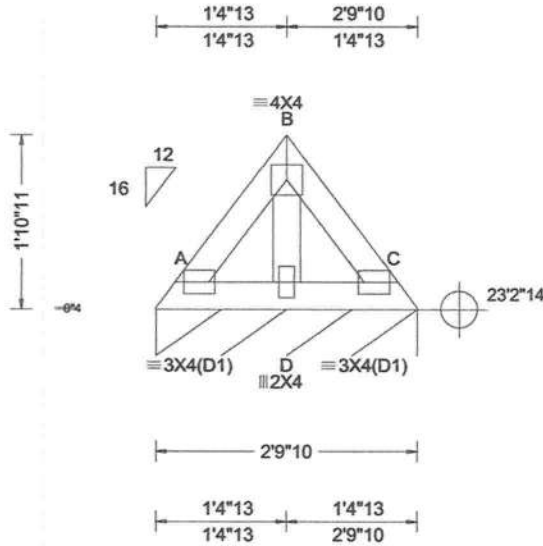
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
See DWGS VALTN160118 and VAL180160118 for valley details.



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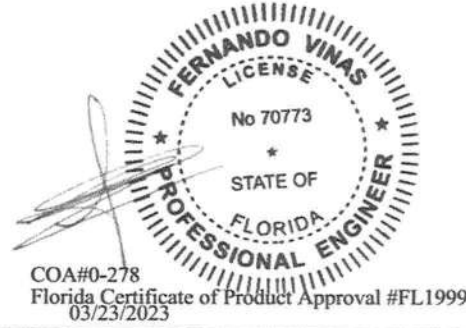


<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 24.34 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 240 VERT(CL): 0.000 C 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.045 Max BC CSI: 0.024 Max Web CSI: 0.022  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL C* 91 /- /- /52 /2 /19 Wind reactions based on MWFRS C Brg Wid = 33.6 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

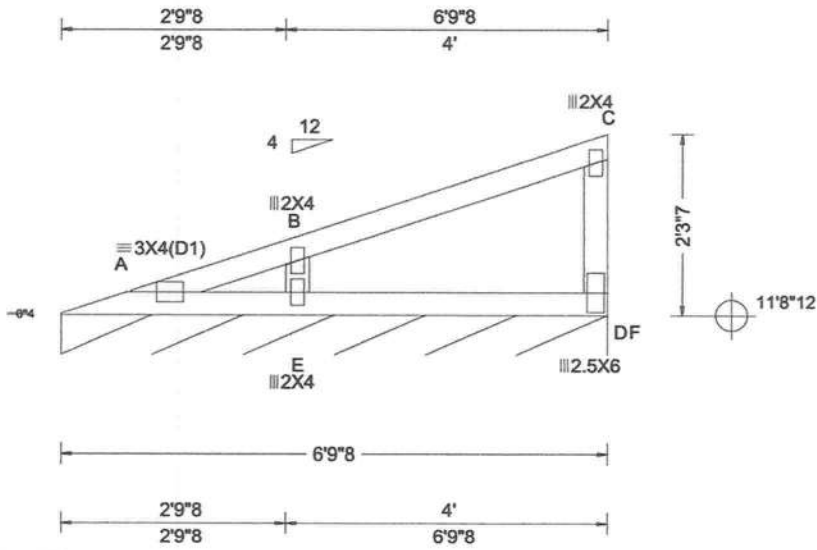
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 15.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 240 VERT(CL): 0.002 A 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.109 Max Web CSI: 0.090  VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th colspan="2">Gravity</th> <th colspan="4">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+ / R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>F*</td> <td>91 / -</td> <td>/ -</td> <td>/ 57</td> <td>/ 12</td> <td>/ 17</td> </tr> </tbody> </table>	Gravity		Non-Gravity				Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	F*	91 / -	/ -	/ 57	/ 12	/ 17
				Gravity		Non-Gravity																
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL																	
F*	91 / -	/ -	/ 57	/ 12	/ 17																	
Wind reactions based on MWFRS F Brg Wid = 81.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#																						

**Lumber**

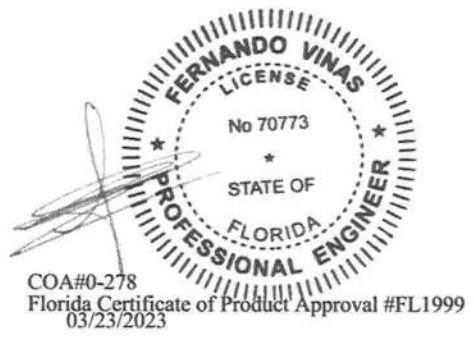
Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

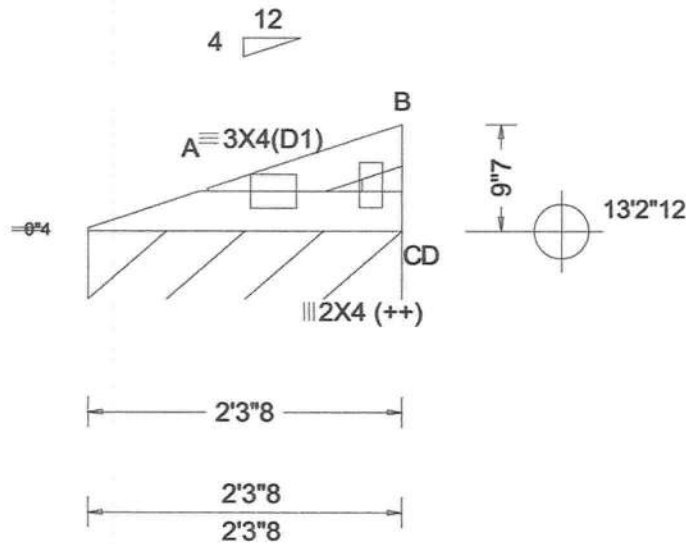
**Additional Notes**

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<b>Loading Criteria (psf)</b> TCCL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.057 Max BC CSI: 0.058 Max Web CSI: 0.021 VIEW Ver: 21.02.00.1005.17	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>D* 91</td> <td>/-</td> <td>/-</td> <td>/46</td> <td>/-</td> <td>/5</td> <td></td> </tr> </tbody> </table> Wind reactions based on MWFRS D Brg Wid = 27.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	D* 91	/-	/-	/46	/-	/5	
Loc	Gravity			Non-Gravity																				
	R+	/R-	/Rh	/Rw	/U	/RL																		
D* 91	/-	/-	/46	/-	/5																			

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Plating Notes**

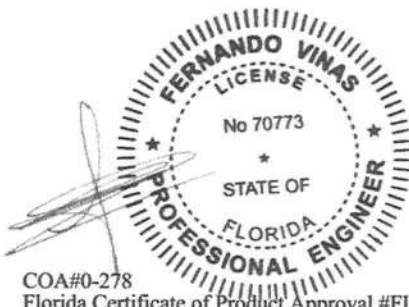
(++) - This plate works for both joints covered.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

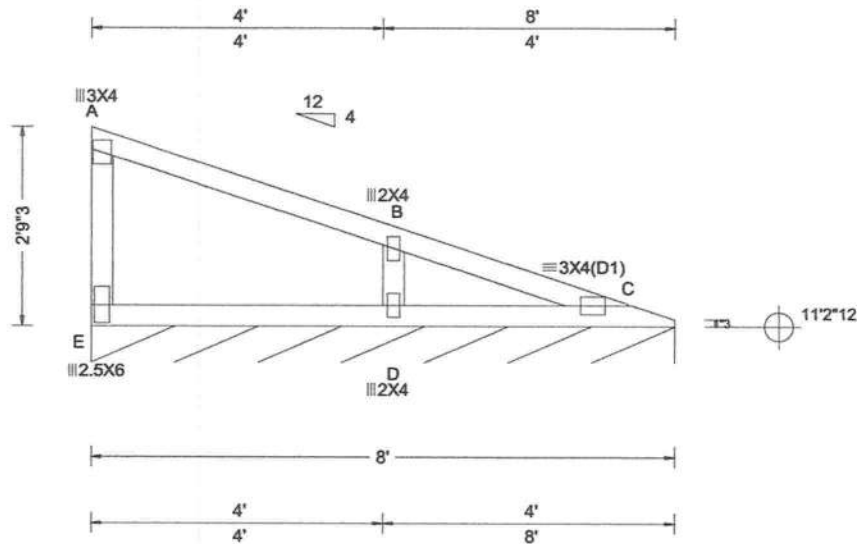
**Additional Notes**

See DWGS VALTN160118 and VAL180160118 for valley details.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.012 C 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.308 Max BC CSI: 0.141 Max Web CSI: 0.101  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL C* 91 /- /- /59 /3 /8 Wind reactions based on MWFRS C Brg Wid = 96.0 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

**Lumber**

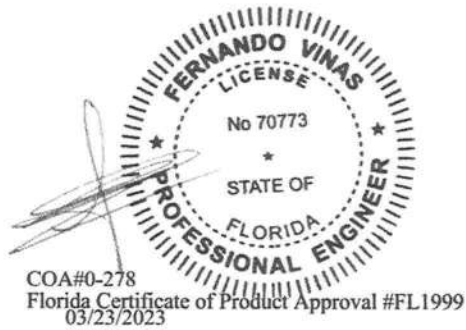
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

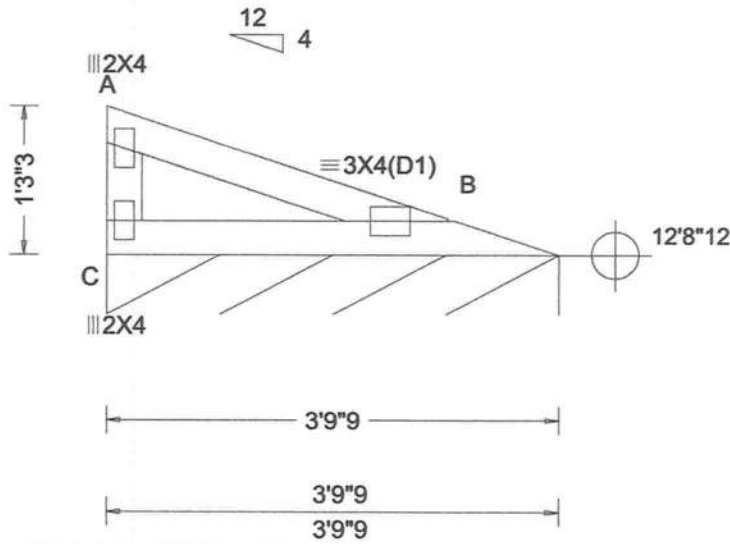
**Additional Notes**

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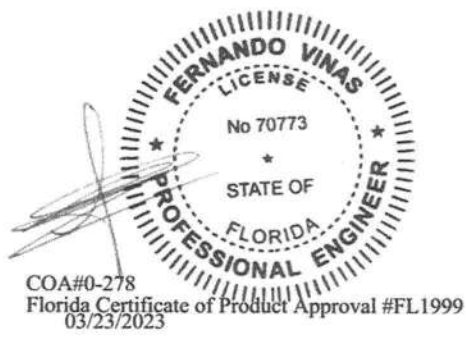


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.011 B 999 240 VERT(CL): 0.026 B 999 180 HORZ(LL): -0.003 B - - HORZ(TL): 0.007 B - - Creep Factor: 2.0 Max TC CSI: 0.175 Max BC CSI: 0.159 Max Web CSI: 0.071  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 91 /- /- /52 /0 /6 Wind reactions based on MWFRS B Brg Wid = 45.5 Min Req = - Bearing C is a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
See DWGS VALTN160118 and VAL180160118 for valley details.



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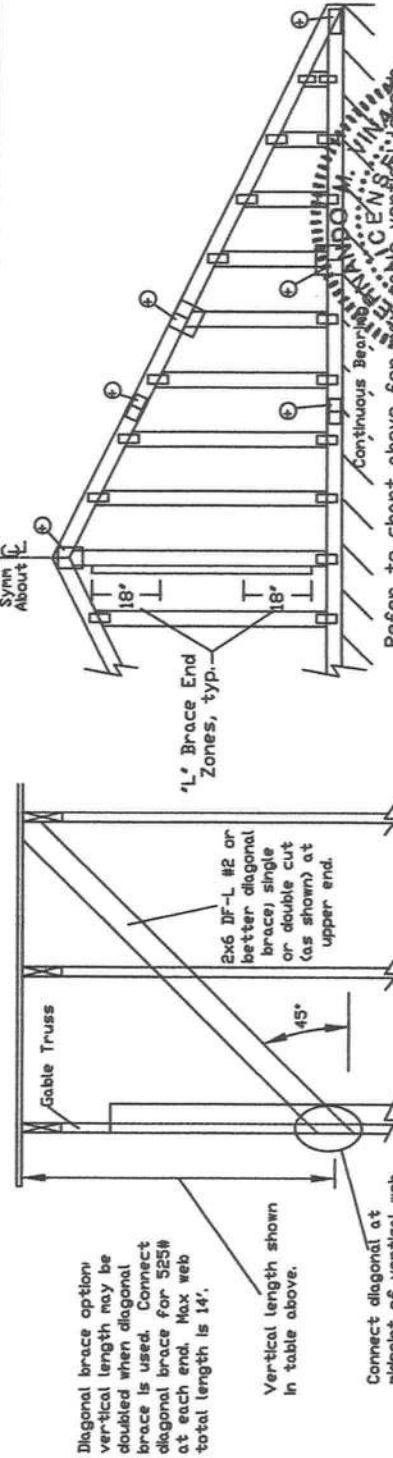


# Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00  
 Dr: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00  
 Dr: 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Vertical Species		Brace Grade		No Braces		(1) 1x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace	
	Spacing	Species	#1 / #2	#3	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	HF	#1 / #2	#3	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
	HF		6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	9' 8"	10' 0"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
	SP	DFL	Standard	#1	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
			#2	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	10' 2"	10' 2"	12' 10"	13' 4"	14' 0"
16" O.C.	SPF	HF	#3	4' 0"	5' 7"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	11' 7"	12' 5"	14' 0"	14' 0"
			Standard	3' 9"	4' 11"	5' 13"	6' 6"	8' 10"	9' 6"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
	SP	DFL	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	HF	Standard	4' 5"	7' 6"	8' 0"	8' 10"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"
			#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	6' 10"	7' 3"	8' 1"	9' 1"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	HF	Standard	4' 7"	6' 10"	7' 3"	8' 1"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"
			#1 / #2	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	11' 6"	12' 7"	13' 15"	14' 0"
	SP	DFL	#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 9"	12' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	7' 5"	7' 11"	10' 2"	10' 7"	12' 2"	12' 8"	12' 8"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	HF	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"



**Bracing Group Species and Grades:**

Group A:	
Spruce-Pine-Fir	Hem-Fir
#1 / #2 Standard	#2 Stud
#3 Standard	#3 Standard
Douglas Fir-Larch	Southern Pine
#3 Standard	#3 Stud
	Standard

**Group B:**

Hem-Fir	#1
Douglas Fir-Larch	#1
	#2
	#2

1x4 Braces shall be SRB (Stress-Rated Board).  
 For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.  
**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 100 plf over continuous bearing (3 psf TC Dead Load).  
 Gable end supports load from 4' 0" outlofters with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.  
 \* For (1) 'L' brace space nails at 2' o.c.  
 In 18" end zones and 4' o.c. between zones.  
 \*\* For (2) 'L' braces: space nails at 3' o.c.  
 In 18" end zones and 6' o.c. between zones.  
 'L' bracing must be a minimum of 80% of web member length.

**Gable Vertical Plate Sizes**

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-16-GABI4030
DATE	01/26/2018
DRWG	A14030ENC160118

MAX. TOT. L.D.	60 PSF
MAX. SPACING	24.0"

NO 70773

STATE OF FLORIDA

PROFESSIONAL ENGINEER

003122023

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 ALPINE: www.alpineitw.com TPI: www.tpi-truss.com SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

**ALPINE**  
ANITW COMPANY

155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025

# CLR Reinforcing

# Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

### Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

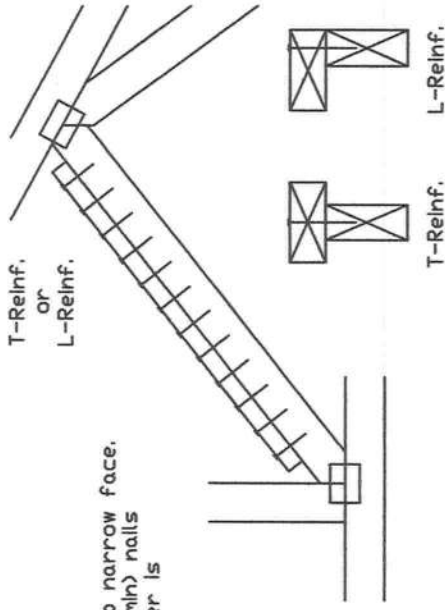
Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(\*) Center scab on wide face of web. Apply (1) scab to each face of web.

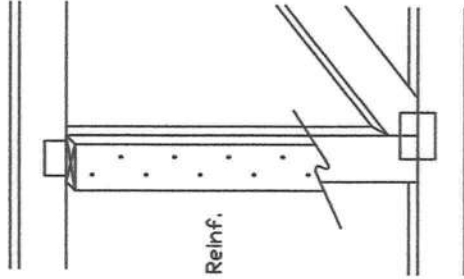
T-Reinforcement  
Or  
L-Reinforcement:



Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.

Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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**ALPINE**  
AN ITW COMPANY

155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025

FERNANDO M. VINAR  
LICENSE  
No 70773  
STATE OF FLORIDA  
PROFESSIONAL ENGINEER  
03/23/2023

PSF	PSF	PSF	PSF	PSF	REF	CLR	Subst.
TOT. L.D.	TOT. L.D.	TOT. L.D.	TOT. L.D.	TOT. L.D.	DATE	01/02/19	
DUR. FAC.	DUR. FAC.	DUR. FAC.	DUR. FAC.	DUR. FAC.	DRWG	BRCLBSUB0119	
SPACING	SPACING	SPACING	SPACING	SPACING			

# NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

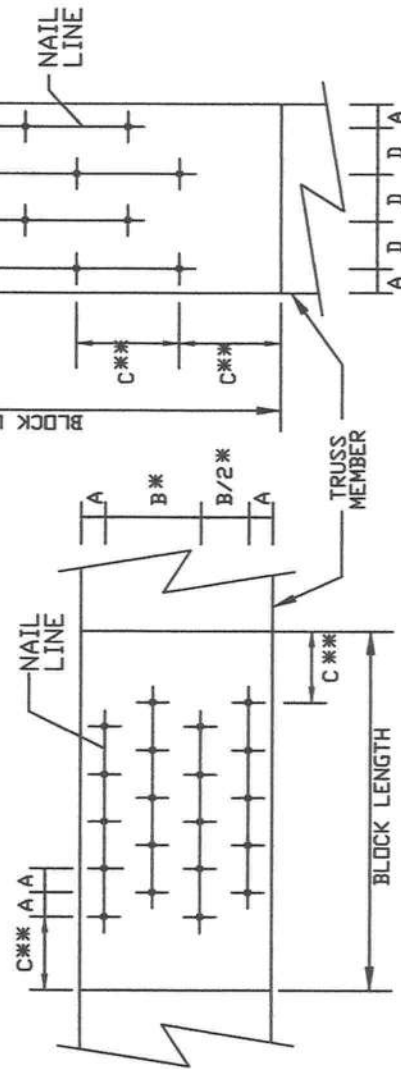
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

\* SPACING MAY BE REDUCED BY 50%

\*\* SPACING MAY BE REDUCED BY 33%

DIRECTION OF LOAD AND NAIL ROWS



## MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113" X 2.5", MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128" X 3", MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128" X 3.25", MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135" X 3.5", MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148" X 4", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148" X 3", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148" X 3.25", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162" X 3.5", MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120" X 2.5", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120" X 3", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 3", MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025

**ALPINE**  
ANITW COMPANY

For more information see this job's general notes page and these web sites:  
ALPINE: [www.alpinetw.com](http://www.alpinetw.com) TPI: [www.tpinet.org](http://www.tpinet.org) SBCA: [www.sbcacomponents.com](http://www.sbcacomponents.com) ICC: [www.iccsafe.org](http://www.iccsafe.org)

**WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING**

INSTALLERS MUST FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of SCS Building Component Safety Information, by TPI and SBCA for safety practices pertaining to trusses. Trusses are not to be used for any other purpose. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per SCS sections 33, 37 or 310, as applicable. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise.

Refer to drawings 104A-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of the drawing for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2.

NO 70773

FERNANDO M. VILAR

LIICENSE

STATE OF FLORIDA

PROFESSIONAL ENGINEER

06/23/2013

REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSPI014

## Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (Inches)  
D = Depth of Truss at Deflection Point (Inches)

### Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75' (horizontal)	1.25' (horizontal)

### Recommended Camber

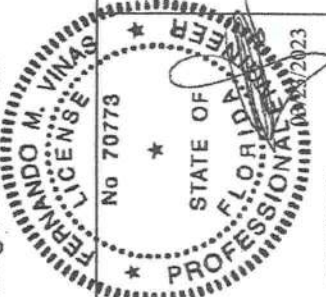
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.

**IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING TO THE INSTALLERS.**  
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for safety instructions and other information. Installers shall provide appropriate bracing per BCSI. Unless noted otherwise, all trusses shall be installed with the top chord braced. All trusses shall have bracing installed per BCSI sections 33, 37 or 210, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.  
Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, the loading or unloading of trusses.  
A valid engineering seal is required for the design stamp. The suitability and use of the drawing engineering responsibility solely for the design stamp. The suitability and use of the drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
For more information see this job's general notes page and these web sites:  
ALPINE: [www.alpineitw.com](http://www.alpineitw.com) TPI: [www.tpi.org](http://www.tpi.org) SBCA: [www.sbcccomponents.com](http://www.sbcccomponents.com) ICD: [www.icdcsfa.org](http://www.icdcsfa.org)



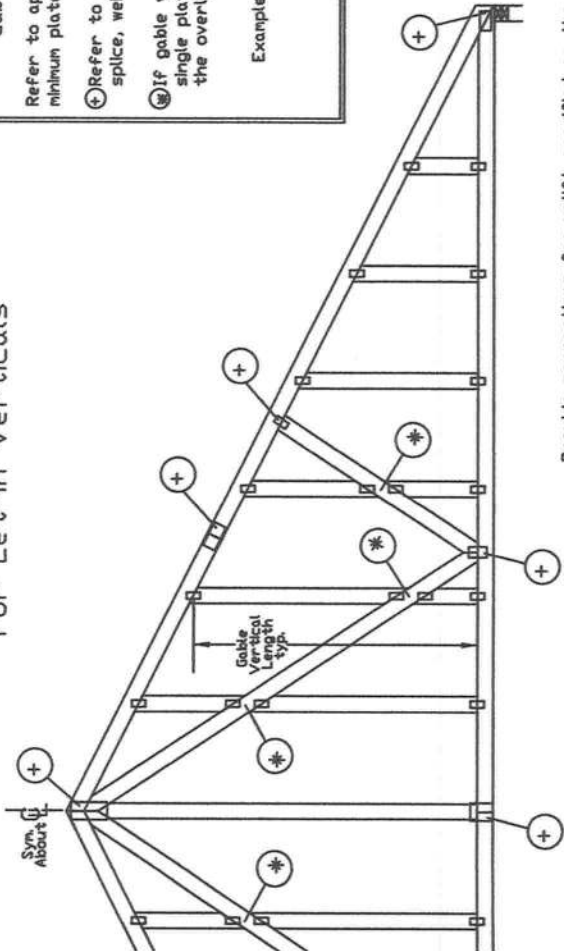
**ALPINE**  
ANITW COMPANY  
155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



COA#0-278  
Florida Certificate of Product Approval #E1.1999

REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014

# Gable Detail For Let-in Verticals



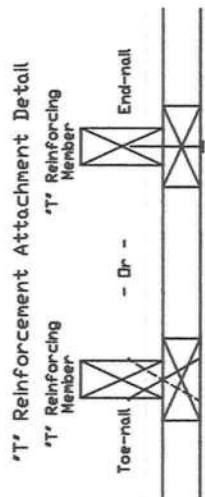
**Gable Truss Plate Sizes**

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

**Web Length Increase w/ 'T' Brace**

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:  
 ASCE 7-10 Wind Speed = 120 mph  
 Mean Roof Height = 30 ft, Kzt = 1.00  
 Gable Vertical = 24' o.c. SP #3  
 'T' Reinforcing Member Size = 2x4  
 'T' Brace Increase (From Above) = 30% = 1.30  
 (1) 2x4 'L' Brace Length = 8' 7"  
 Maximum 'T' Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with  
 End Driven Nails:  
 10d Common (0.148"x3.75") Nails at 4' o.c. plus  
 (4) nails in the top and bottom chords.

Toenailed Nails:  
 10d Common (0.148"x3.75") Toenails at 4' o.c. plus  
 (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings  
 A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,  
 A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings  
 A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,  
 A18015ENC100118, A20015ENC100118, A20015ENDI00118, A20015PEDI00118,  
 A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,  
 A18030ENC100118, A20030ENC100118, A20030ENDI00118, A20030PEDI00118,  
 S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,  
 S18015ENC100118, S20015ENC100118, S20015ENDI00118, S20015PEDI00118,  
 S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,  
 S18030ENC100118, S20030ENC100118, S20030ENDI00118, S20030PEDI00118

See appropriate Alpine gable detail for maximum unreinforced length.

ALPINE  
 AN ITW COMPANY

155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

MAX. TOT. LD. 60 PSF  
 DUR. FAC. ANY  
 MAX. SPACING 24.0'

STATE OF FLORIDA  
 PROFESSIONAL ENGINEER  
 No 70773  
 License No. 70773  
 01/23/2023

**IMPORTANT! READ AND FOLLOW ALL NOTES ON THIS DRAWING BEFORE INSTALLING.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of SCSI Building Component Safety Information, by TPI and SCSA for safety practices prior to performing these functions. Installers shall provide appropriate bracing per SCSI instructions. All connections shall be made in accordance with the instructions provided. All connections shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160M-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses.

Alpine and its representatives shall not be responsible for the design shown. The liability and use of this drawing engineering responsibility solely for the design shown. The responsibility of the building designer per ANSI/TPI 1 Sec.2 for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:  
 ALPINE: www.alphinetw.com TPI: www.tpinetw.com SCSA: www.scsacomponents.com ICD: www.icdusa.org

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

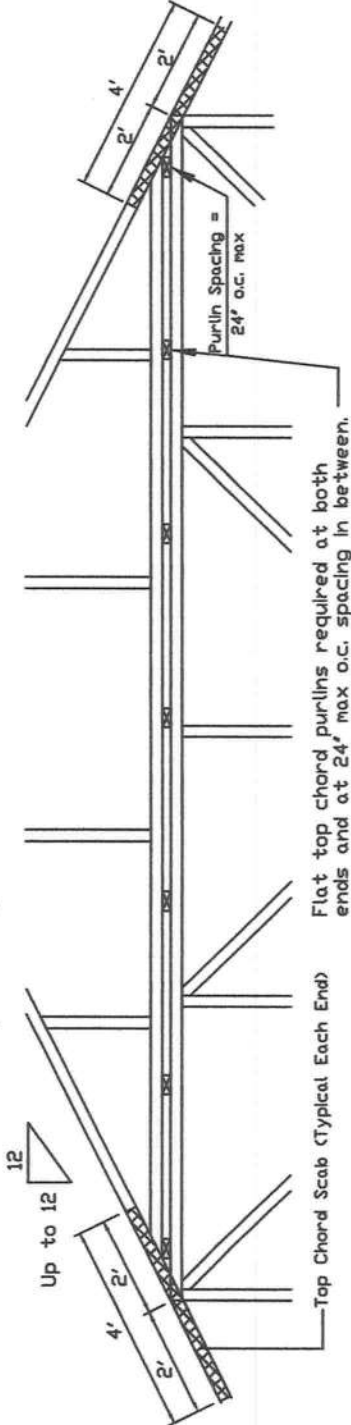
# Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, $K_z t = 1.00$

160 mph Wind, 30.00 Ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (min),  $K_z t = 1.00$   
 Or 140 mph Wind, 30.00 Ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (min),  $K_z t = 1.00$

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends. Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

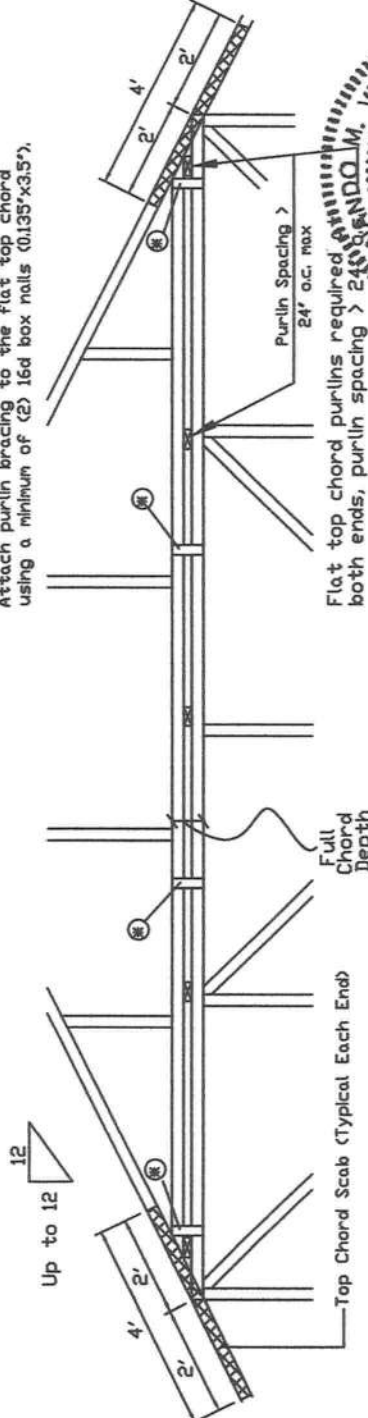
Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

## Detail A : Purlin Spacing = 24" o.c. or less



## Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3x8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

In addition, provide connection with one of the following methods:

### Trulox

Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

### APA Rated Gusset

8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 0.113"x2" nails per gusset. (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

### 2x4 Vertical Scabs

2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

### 28PB Wave Piggyback Plate

One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.



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 Refer to drawings 150A-Z for standard plate positions.  
 Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, or failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.  
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineer for any structure is the responsibility of the Building Designer per ANSI/TPI 1, SBCA.  
 For more information see this job's general notes page and these web sites:  
 ALPINE: [www.alpineitv.com](http://www.alpineitv.com) TPI: [www.tpi.net](http://www.tpi.net) SBCA: [www.sbcacomponents.com](http://www.sbcacomponents.com) ICC: [www.iccsafe.org](http://www.iccsafe.org)

**ALPINE**  
 AN ITV COMPANY  
 155 Hadem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118

# Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better,  
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better,  
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better,

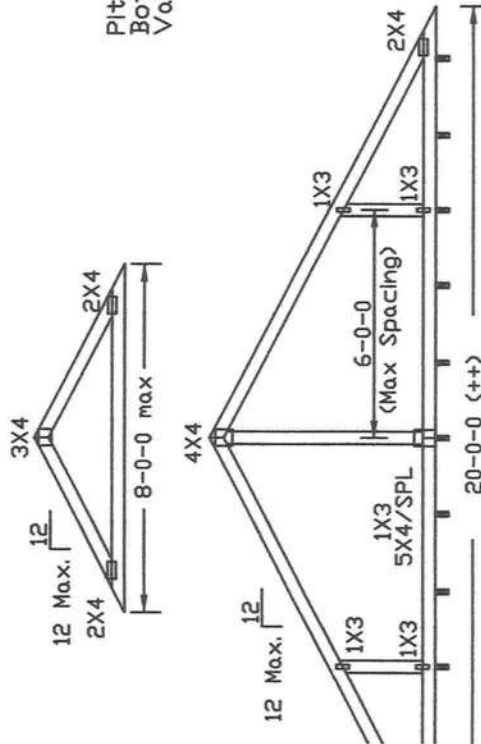
\*\* Attach each valley to every supporting truss with  
 535# connection or with (1) Simpson H2.5A or  
 equivalent connector for  
 ASCE 7-16 180 mph, 30' Mean Height, Part. Enc.  
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00

Or  
 ASCE 7-16 160 mph, 30' Mean Height, Part. Enc.  
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut  
 as shown.

Valleys short enough to be cut as solid triangular  
 members from a single 2x6, or larger as required,  
 shall be permitted in lieu of fabricating from  
 separate 2x4 members.

All plates shown are Alpine Wave Plates.



Supporting trusses at 24' o.c. maximum spacing.

Unless specified otherwise on engineer's sealed design, for vertical  
 valley webs taller than 7'-9" apply 2x4 'T' reinforcement, 80% length of  
 web, same species and grade or better, attached with 10d box  
 (0.128" x 3.0") nails at 6' o.c. In lieu of 'T' reinforcement, 2x4 Continuous  
 Lateral Restraint applied at mid-length of web is permitted with diagonal  
 bracing as shown in DRWG BRCLBANC1014.

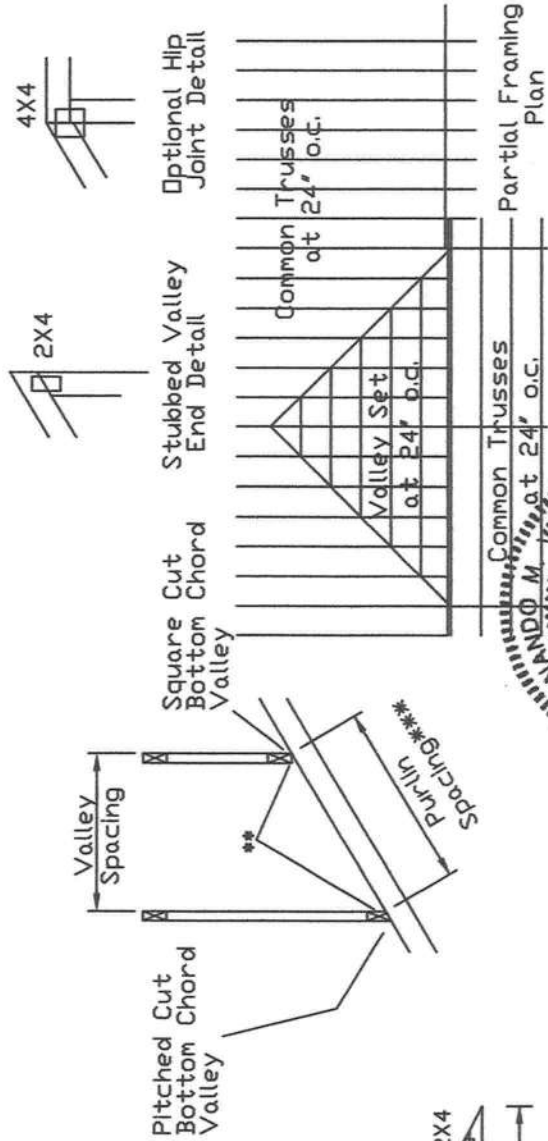
Top chord of truss beneath valley set must be braced with  
 properly attached, rated sheathing applied prior to valley truss  
 installation.

Or

Purlins at 24' o.c. or as otherwise specified on engineer's sealed design  
 Or  
 By valley trusses used in lieu of purlin spacing as specified on  
 Engineer's sealed design.

\*\*\* Note that the purlin spacing for bracing the top chord of the truss  
 beneath the valley is measured along the slope of the top chord.

\*\* Larger spans may be built as long as the vertical height does  
 not exceed 14'-0".



**IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLER.**  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and  
 follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for safety  
 practices prior to performing these functions. Installers shall provide temporary bracing per BCSI  
 instructions. Trusses shall be installed in the locations shown for permanent lateral restraint of webs  
 shall have bracing installed per BCSI sections 33, 37 or 310, as applicable. Apply plates to each face  
 of truss and position as shown above and on the Joint Details, unless noted otherwise.  
 Refer to drawings 160A-2 for standard plate positions.  
 Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from  
 this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping,  
 installation or bracing of trusses.  
 The engineer shall be responsible for the design, including the accuracy of professional  
 engineering responsibility solely for the design shown. The liability and use of this drawing  
 for any structure is the responsibility of the Building Designer per ANSI/TPI 1, Sec.2.  
 For more information see this job's general notes page and these web sites:  
 ALPINE: www.alpineitw.com TPI: www.tpi.net SBCA: www.sbcacomponents.com, ICC: www.iccsafe.org

REF	VALLEY DETAIL	DATE	01/26/2018
DRWG	VAL180160118		
40PSF	30	15	7PSF
LL	30	20	15
DL	10	10	10PSF
LL	0	0	0PSF
TOT. LD.	60	55	57PSF
SPACING		1.15	1.15
DUR.FAC.		1.25	1.33
COA#0-278		24.0'	
Florida Certificate of Product Approval #FT1999			

ALPINE  
 AN ITW COMPANY  
 155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

# Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.  
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.  
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

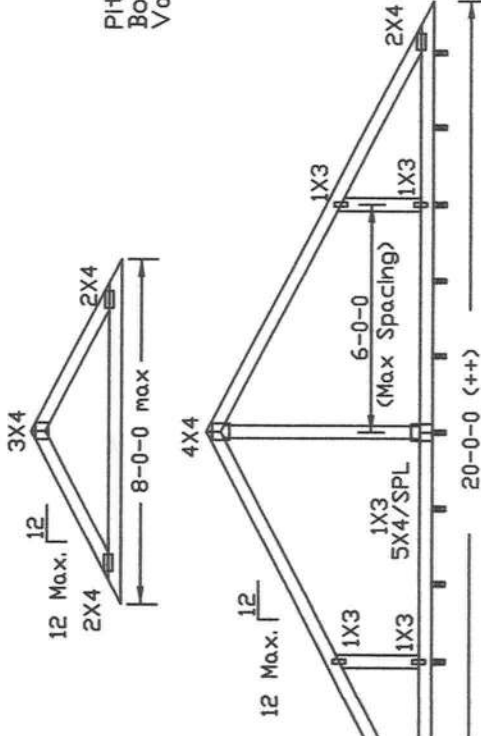
\*\* Attach each valley to every supporting truss with (2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location: 170 mph for SP (G = 0.55, min.), 155 mph for DF-L (G = 0.50, min.), or 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.



Supporting trusses at 24' o.c. maximum spacing.

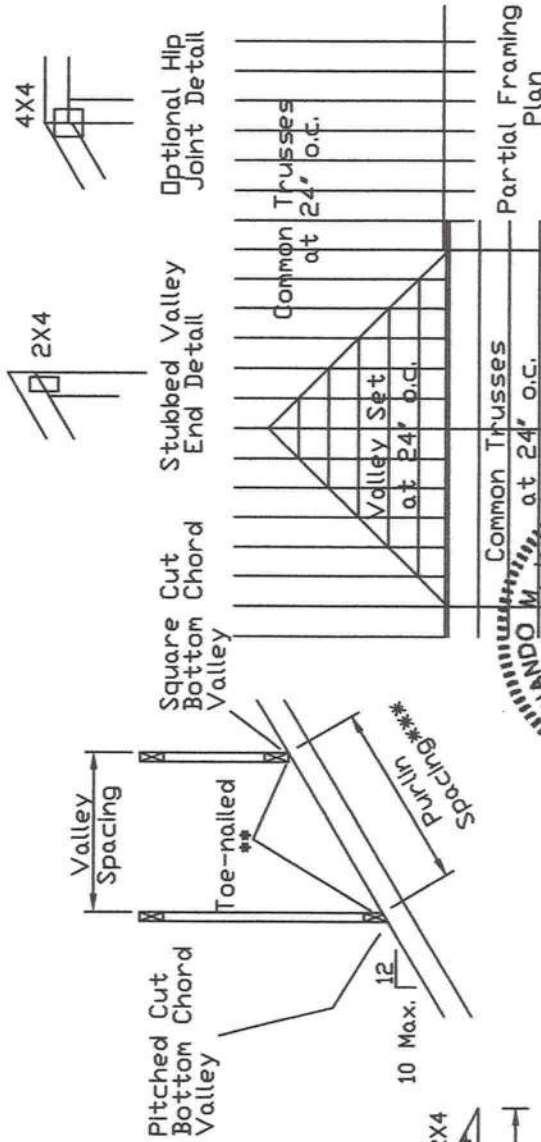
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6' o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

Or  
 Purlins at 24' o.c. or as otherwise specified on engineer's sealed design  
 Or  
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

\*\*\* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

\*\* Larger spans may be built as long as the vertical height does not exceed 14'-0".



**IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLER'S COMMENTS.**  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for safety information. The manufacturer's instructions for the trusses shall be followed. The contractor shall have a properly attached ridge cap. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Job Details, unless noted otherwise.  
 Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, or installation. The contractor shall seal on this drawing or cover page listing the design, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see this job's general notes page and these web sites:  
 ALPINE www.alphetw.com TPI www.tpi.net SBCA www.sbcacomponents.com ICC www.iccsafe.org



TC	LL	30	40	PSF	REF	VALLEY	DETAIL
TC	DL	20	15	7	PSF	DATE	01/26/2018
BC	DL	10	10	10	PSF	DRWG	VALTNI60118
BC	LL	0	0	0	PSF		
TOT. LD.		60	55	57	PSF		
DUR.FAC.		1.25	1.33	1.15			
SPACING		24.0'					

COA#0-278  
 Florida Certificate of Product Approval #PL1999

