30046

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1UK8487-Z0108161937

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

Truss Fabricator:	Anderson Truss Company
Job Identification:	12-049--Fill in later BRYAN ZECHER/KOCH -- , **
Truss Count:	45
Model Code:	Florida Building Code
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Douglas Fleming
-Truss Design Engineer-

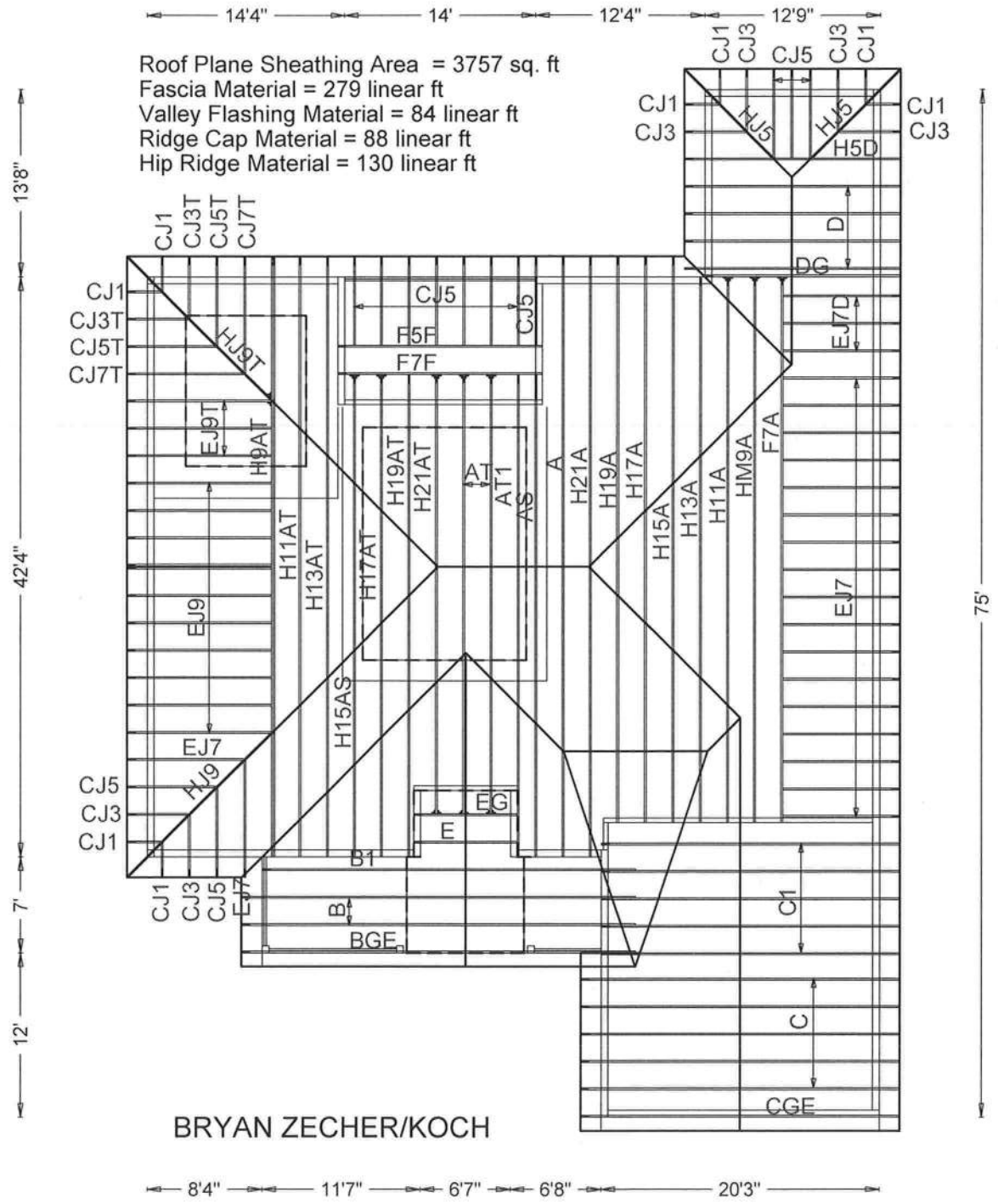
1950 Marley Drive
Haines City, FL 33844

Details: CNNAILSP-BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref Description	Drawing#	Date
1	44470--F7A	12068001	03/08/12
2	44471--HM9A	12068002	03/08/12
3	44472--H11A	12068003	03/08/12
4	44473--H13A	12068004	03/08/12
5	44474--H15A	12068005	03/08/12
6	44475--H17A	12068006	03/08/12
7	44476--H19A	12068007	03/08/12
8	44477--H21A	12068008	03/08/12
9	44478--A	12068009	03/08/12
10	44479--AS	12068010	03/08/12
11	44480--H13AT	12068011	03/08/12
12	44481--H15AS	12068012	03/08/12
13	44482--H9AT	12068013	03/08/12
14	44483--H11AT	12068014	03/08/12
15	44484--H17AT	12068015	03/08/12
16	44485--H19AT	12068016	03/08/12
17	44486--H21AT	12068017	03/08/12
18	44487--AT	12068018	03/08/12
19	44488--AT1	12068019	03/08/12
20	44489--B	12068020	03/08/12
21	44490--B1	12068021	03/08/12
22	44491--BGE	12068022	03/08/12
23	44492--CGE	12068023	03/08/12
24	44493--C	12068024	03/08/12
25	44494--C1	12068025	03/08/12
26	44495--H5D	12068026	03/08/12
27	44496--D	12068027	03/08/12
28	44497--DG	12068028	03/08/12
29	44498--E	12068029	03/08/12
30	44499--EG	12068030	03/08/12
31	44500--F5F	12068031	03/08/12
32	44501--F7F	12068032	03/08/12
33	44502--CJ1	12068033	03/08/12
34	44503--CJ3T	12068034	03/08/12
35	44504--CJ5T	12068035	03/08/12
36	44505--CJ7T	12068036	03/08/12

#	Ref Description	Drawing#	Date
37	44506--EJ9T	12068037	03/08/12
38	44507--EJ9	12068038	03/08/12
39	44508--CJ3	12068039	03/08/12
40	44509--CJ5	12068040	03/08/12
41	44510--EJ7	12068041	03/08/12
42	44511--EJ7D	12068042	03/08/12
43	44512--HJ9	12068043	03/08/12
44	44513--HJ9T	12068044	03/08/12
45	44514--HJ5	12068045	03/08/12

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER



JOB DESCRIPTION:: Fill in later
/: BRYAN ZECHER/KOCH

JOB NO:
12-049

PAGE NO:
1 OF 1

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - F7A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x6 SP SS :T1 2x6 SP #2: :T2 2x4 SP M-30:
:T5 2x6 SP #1 Dense:
Bot chord 2x6 SP SS :B1 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W2, W6, W12, W14 2x4 SP M-30:

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

Max JT VERT DEFL: LL: 0.58" DL: 0.92" recommended camber 1 1/2"

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

Brg blocks:0.131"x3" nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	39.500'	1	12"	4	Rigid Surface

Brg block to be same size and species as bottom chord.
Refer to drawing CNNAILSP0109 for more information.

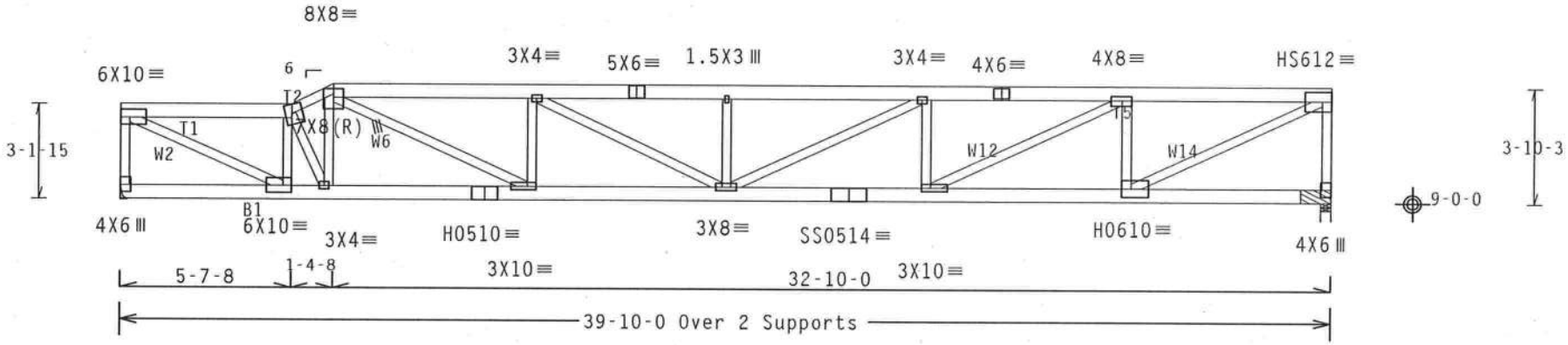
Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.10" due to live load and 0.16" due to dead load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Girder supports 7-0-0 span to TC/BC split one face and 2-0-0 span to TC/BC split opposite face.

Calculated vertical deflection is 0.58" due to live load and 0.92" due to dead load at X = 19-11-14.



R=3691 U=0 H=Simpson HGUS26
Supported Member Face: (8) 0.148"x3" nails
Supporting Member Face: (20) 0.148"x3" nails
Supporting Member : (2) 2x6 SP #2

R=3691 U=0 W=4"

PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44470
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068001
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274464
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - HM9A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

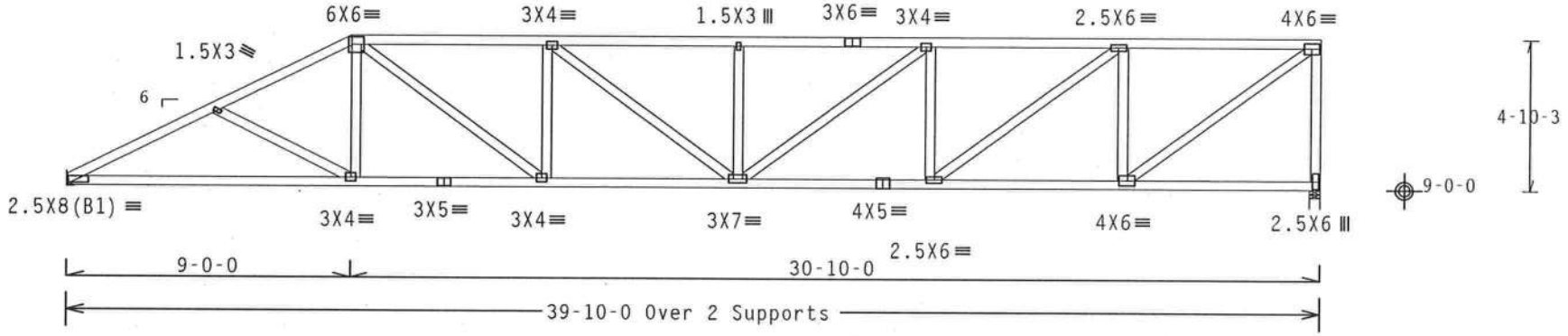
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1646 U=0
RL=74/-20 H=Simpson HUS26
Supported Member Face: (4) 0.162"x3.5" nails
Supporting Member Face: (14) 0.162"x3.5" nails
Supporting Member : (1) 2x6 SP #2

R=1635 U=11 W=4"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.05.11 0209.20 QTY: 1

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

ALPINE
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FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 44471
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068002
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274472
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H11A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

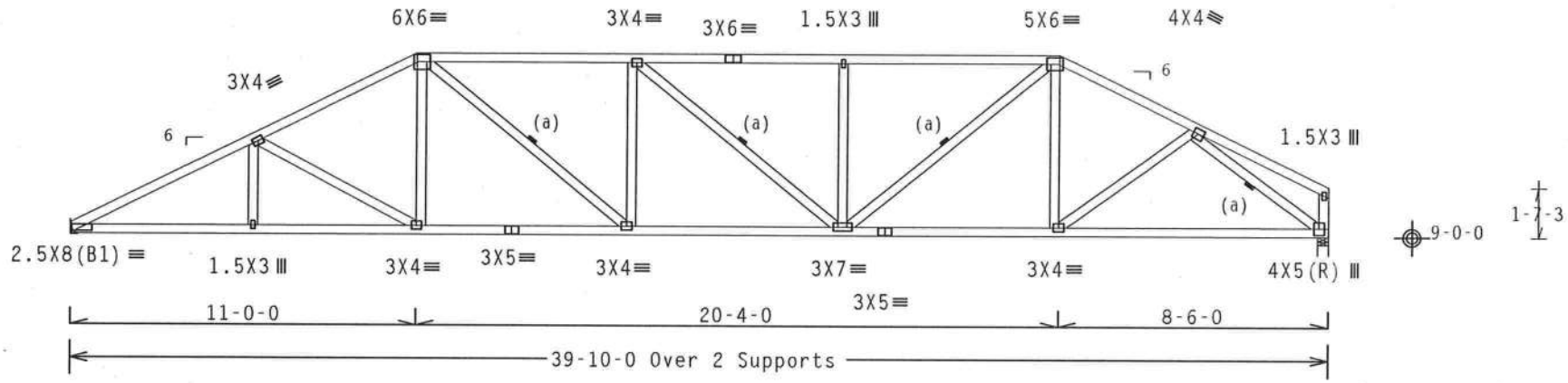
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1646 U=0
RL=80/-72 H=Simpson HUS26
Supported Member Face: (4) 0.162"x3.5" nails
Supporting Member Face: (14) 0.162"x3.5" nails
Supporting Member : (1) 2x6 SP #2

R=1635 U=0 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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TC LL	20.0 PSF	REF	R487-- 44472
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068003
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274482
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H13A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30 :B3 2x4 SP #1:
Webs 2x4 SP #3

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

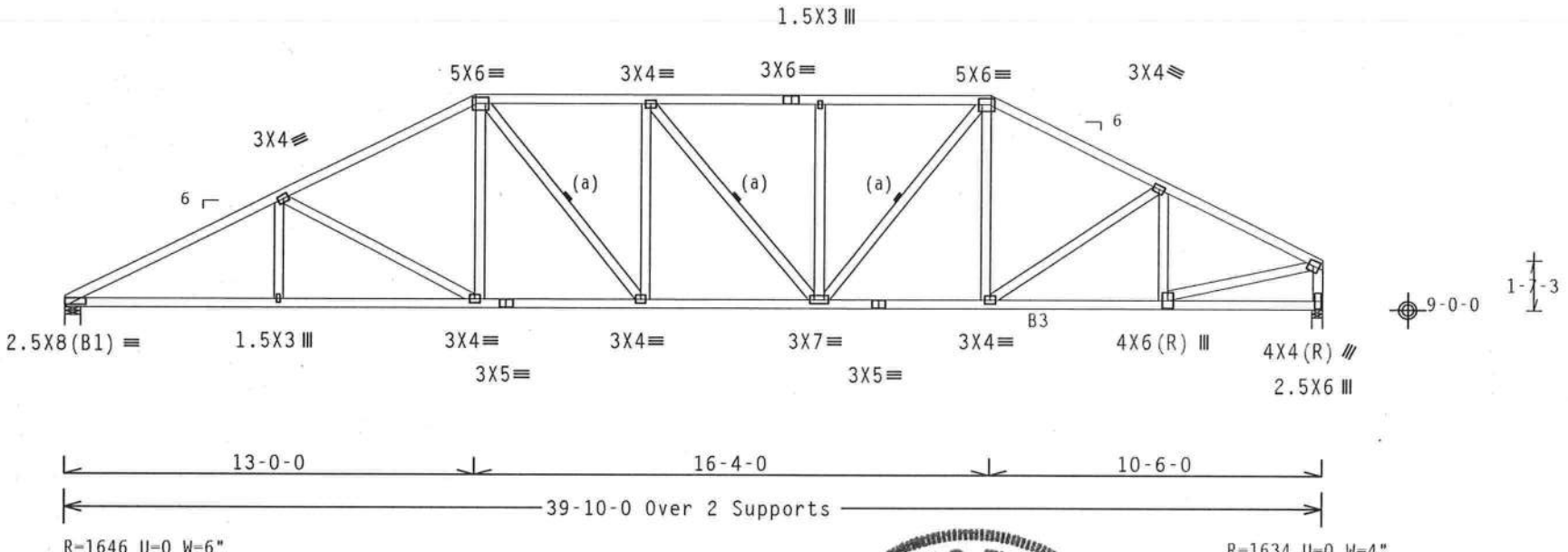
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.20

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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03/08/2012

TC LL	20.0 PSF	REF R487-- 44473
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068004
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274497
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H15A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

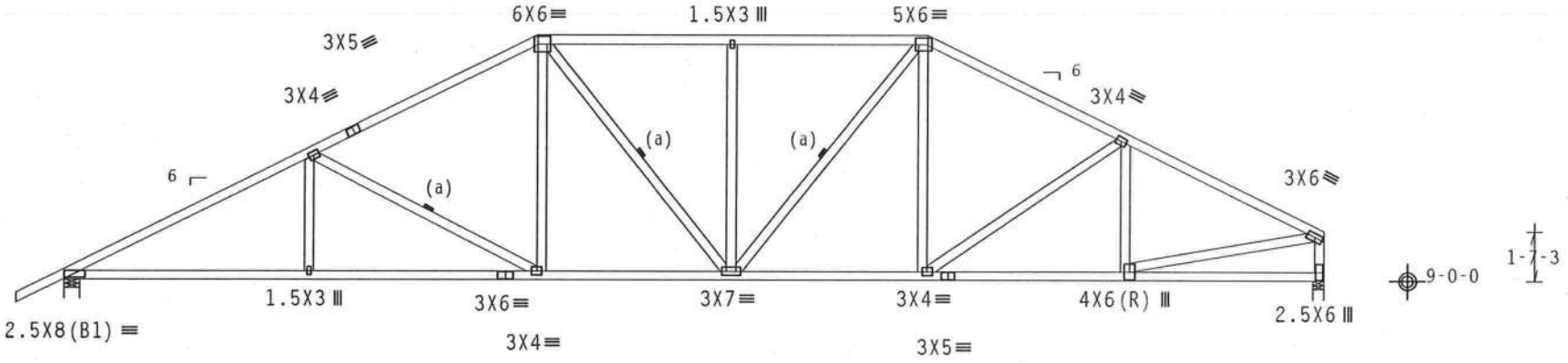
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

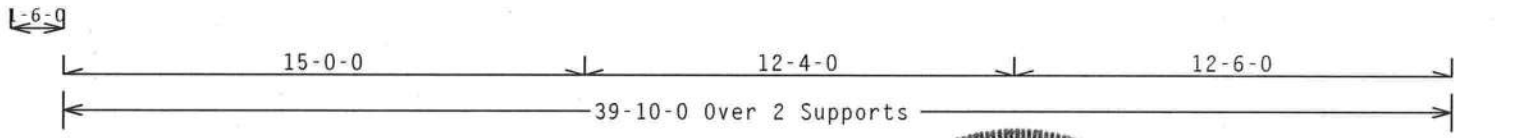
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.05.11.0209.20

QTY: 1

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44474
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068005
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274519
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H17A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30 :T4 2x4 SP #1:
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

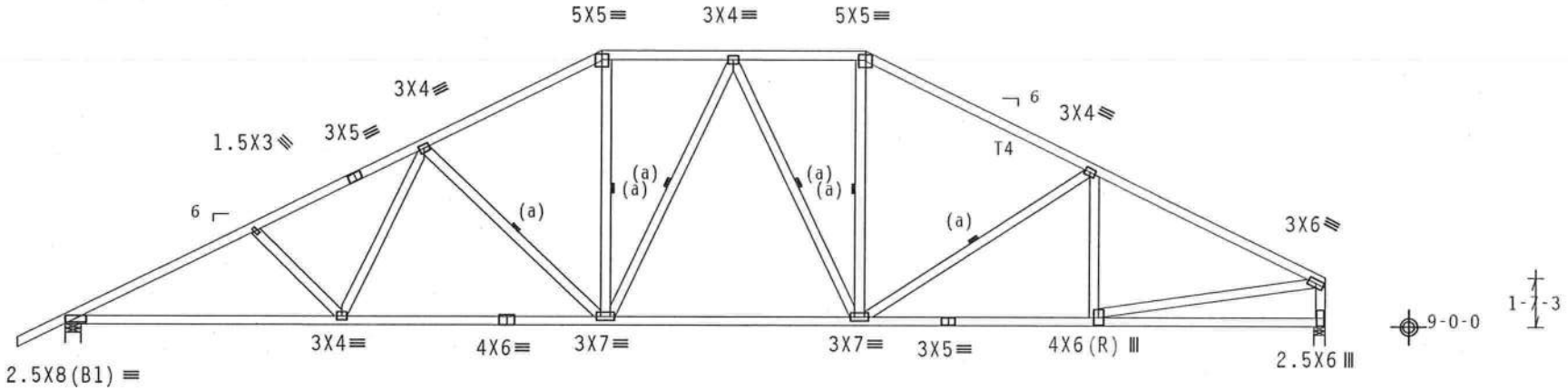
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

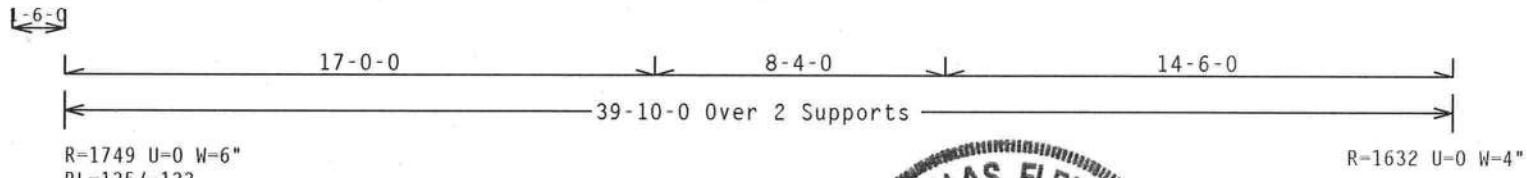
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44475
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068006
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274529
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H19A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.08" due to live load and 0.21" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

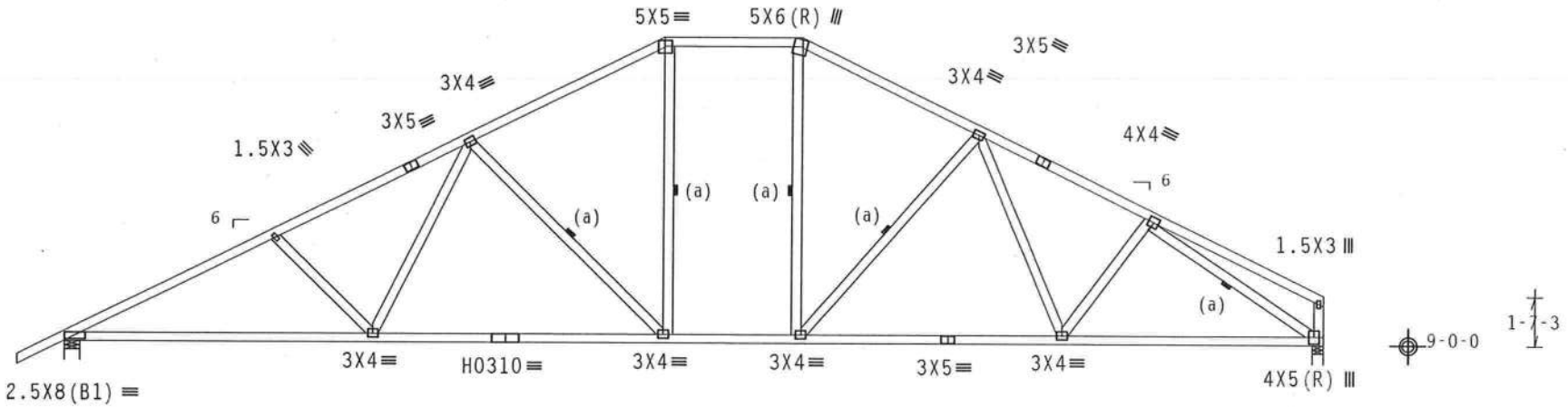
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

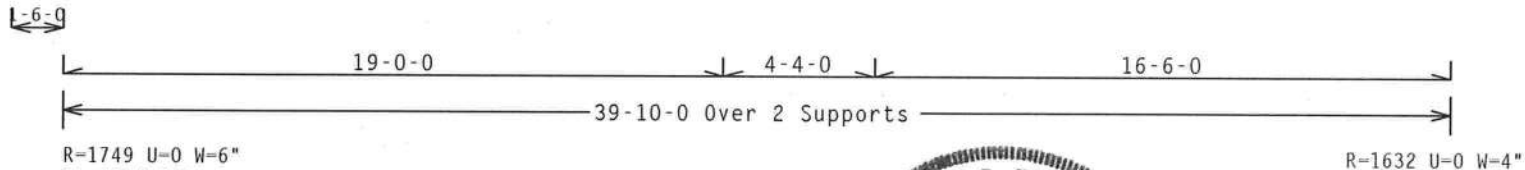
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





PLT TYP. 20 Gauge HS,Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44476
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068007
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274548
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H21A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

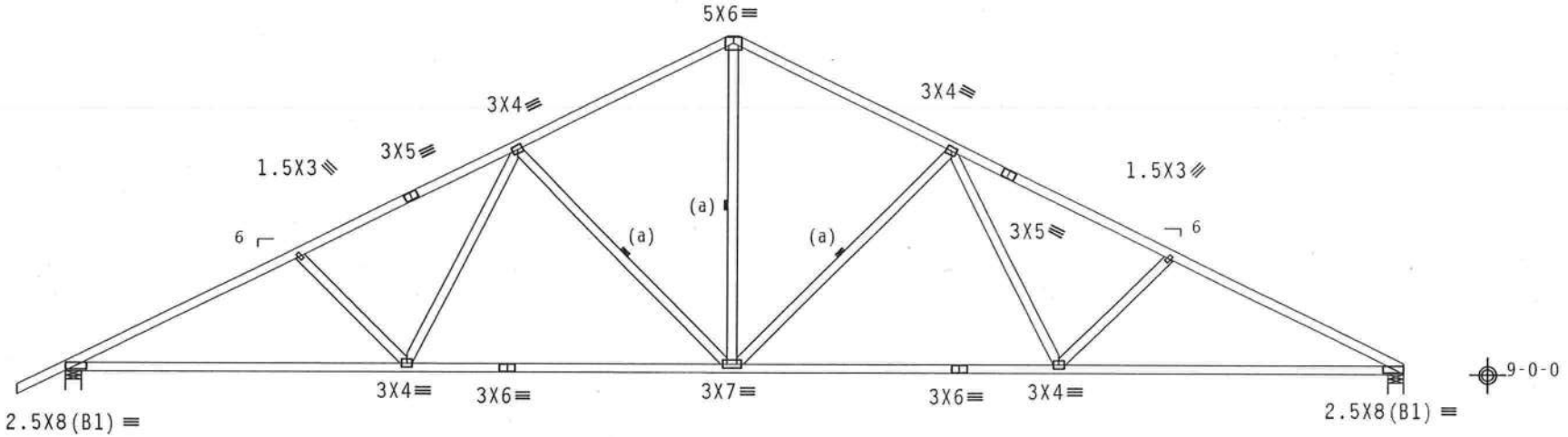
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

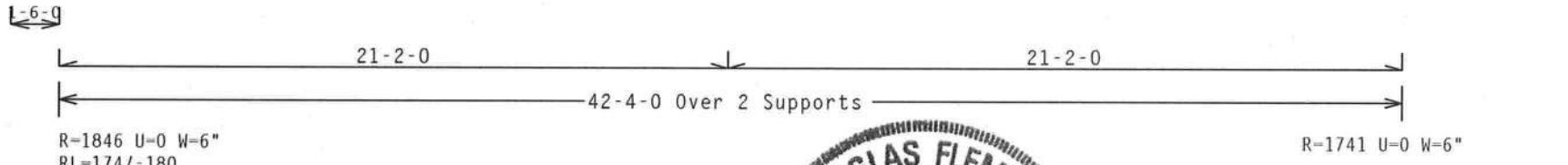
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.00.11 0208.20

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/08/2012

TC LL	20.0 PSF	REF	R487-- 44477
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068008
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274555
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

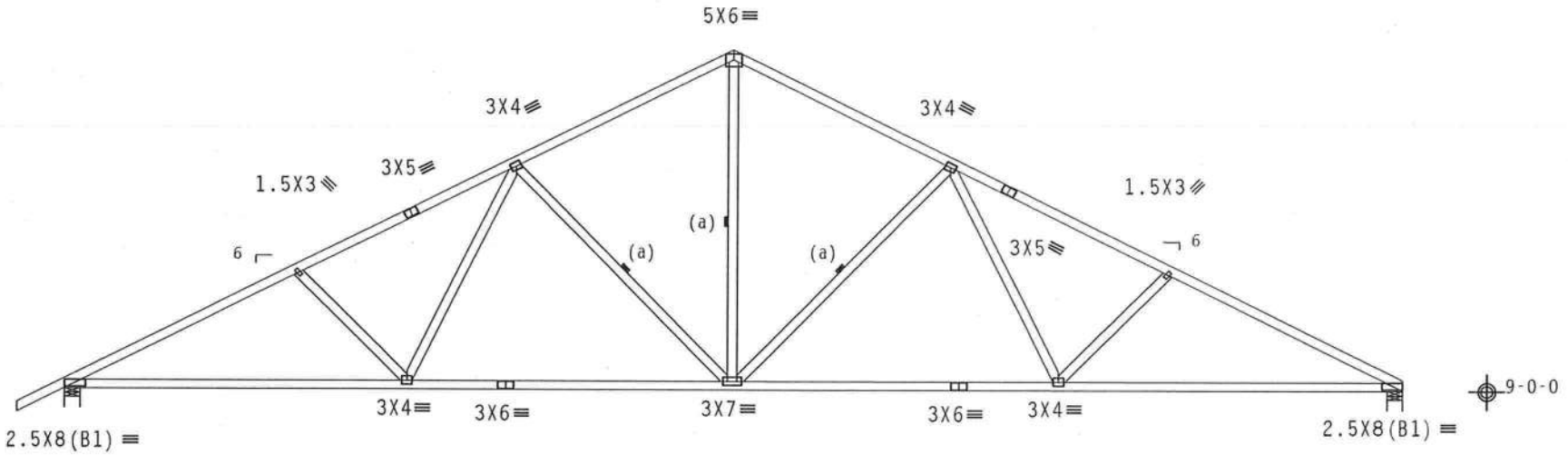
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

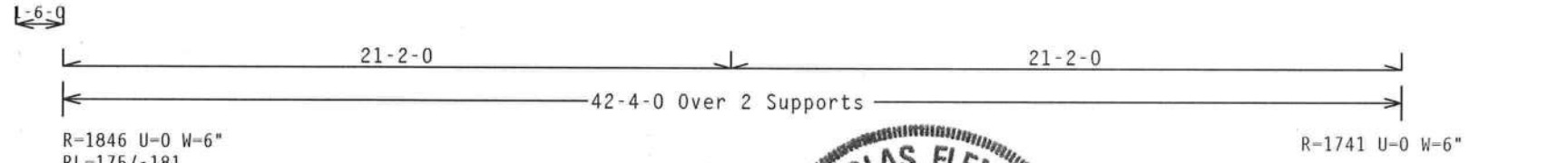
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.20

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/08/2012

TC LL	20.0 PSF	REF R487-- 44478
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068009
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274564
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - AS)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

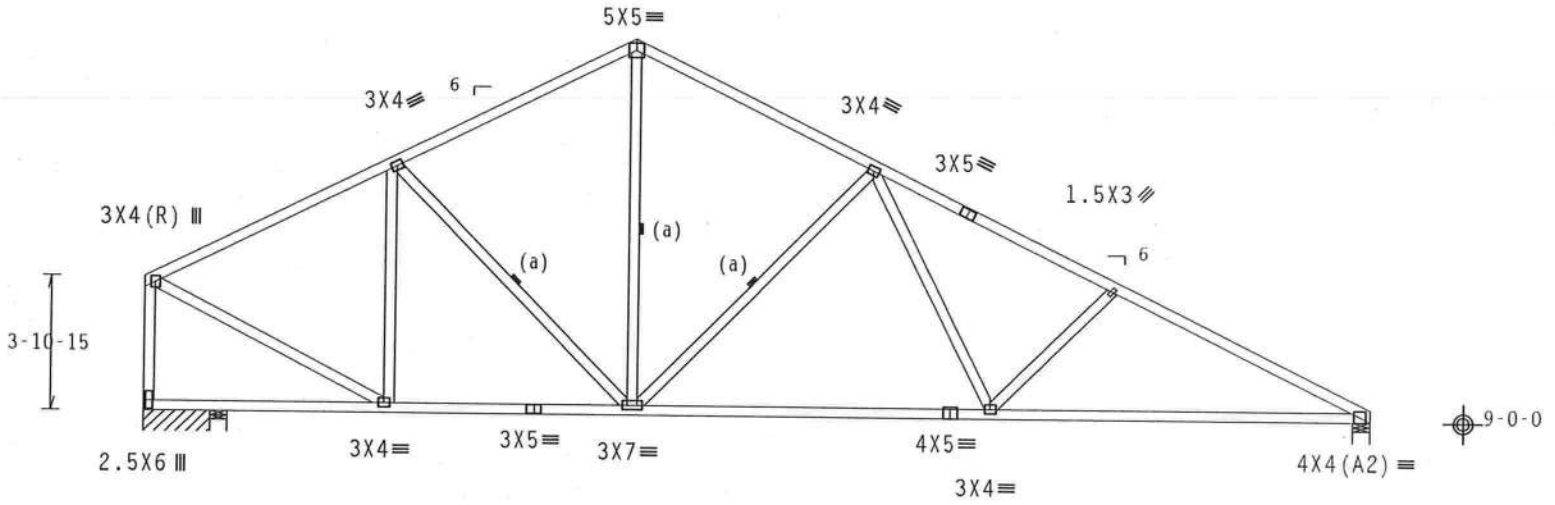
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



2-1-8

14-0-8

21-2-0

35-2-8 Over 3 Supports

R=712 PLF U=0 PLF W=1-10-8
RL=65/R=787 PLF U=0 W=6"

R=1449 U=0 W=6"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44479
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068010
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274751
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H13AT)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

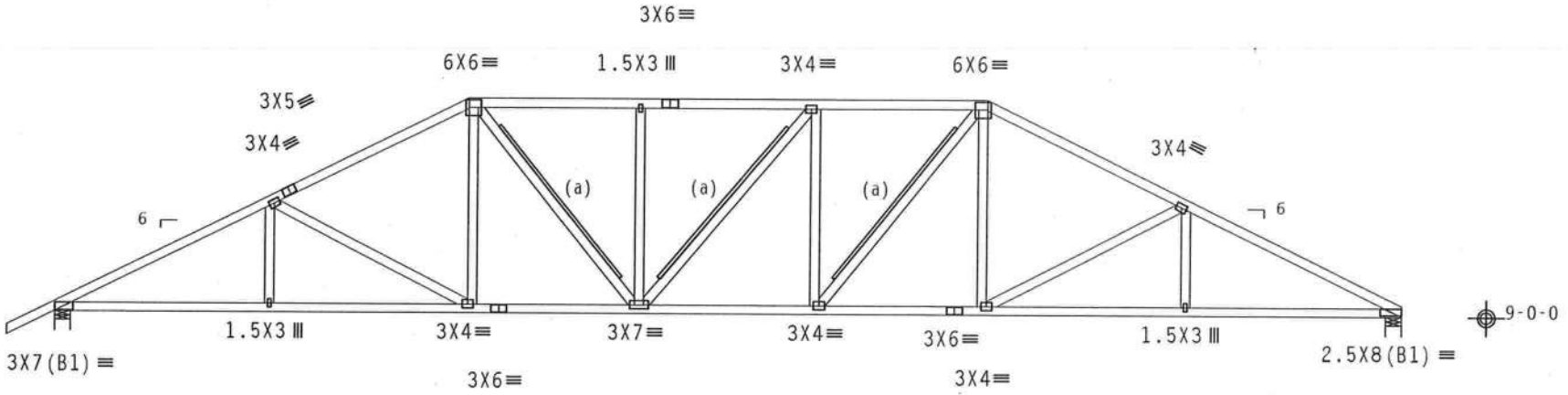
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

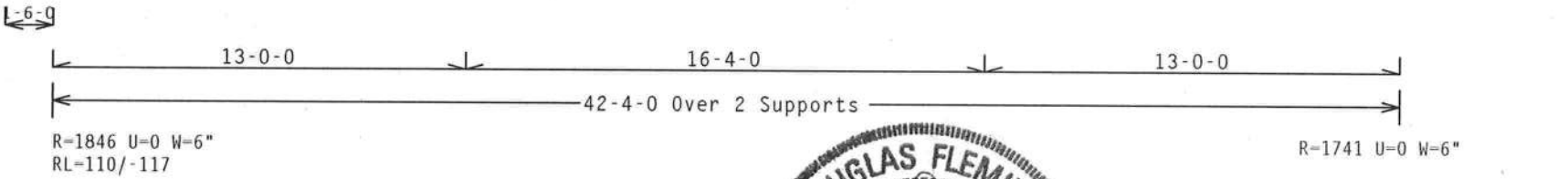
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.06.11 0209.20

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44480
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068011
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274614
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H15AS)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

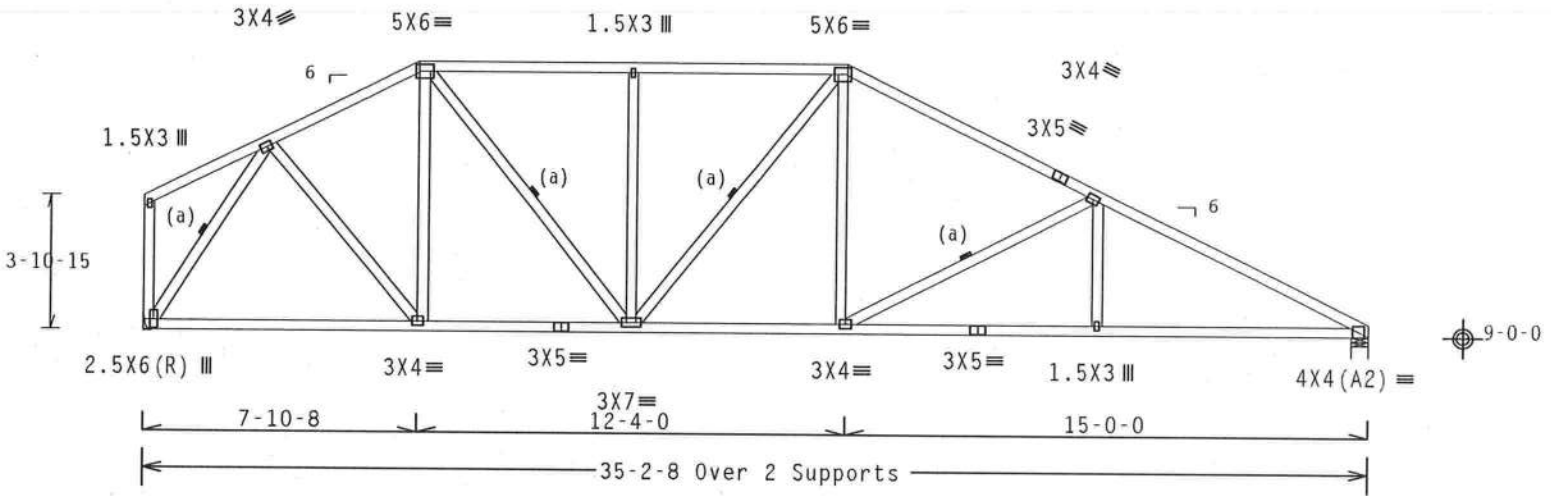
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1444 U=0
RL=75/-99 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

R=1456 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY: FL/-/4/-/-/R/- Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 44481
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068012
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274622
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H9AT)

Top chord 2x4 SP M-30 :T2, T3 2x6 SP #2:
Bot chord 2x6 SP #2 :B1 2x4 SP M-30: :B2 2x6 SP SS:
Webs 2x4 SP #3 :W10, W11 2x4 SP M-30:

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

#1 hip supports 9-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

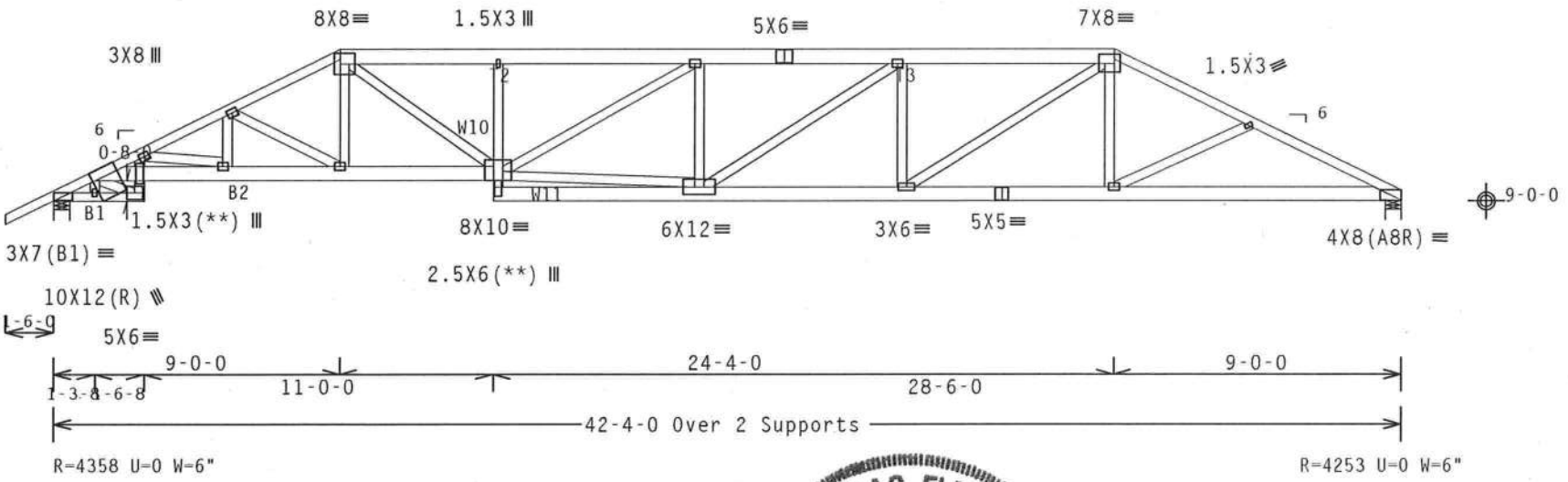
Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.15" due to live load and 0.24" due to dead load.

Left side jacks have 9-0-0 setback with 0-0-0 cant and 1-6-0 overhang. End jacks have 9-0-0 setback with 0-0-0 cant and 1-6-0 overhang. Right side jacks have 9-0-0 setback with 0-0-0 cant and 1-6-0 overhang.

Calculated vertical deflection is 0.40" due to live load and 0.64" due to dead load at X = 20-4-0.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44482
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068013
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274449
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H11AT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3 :W9 2x4 SP M-30:

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.14" due to live load and 0.21" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

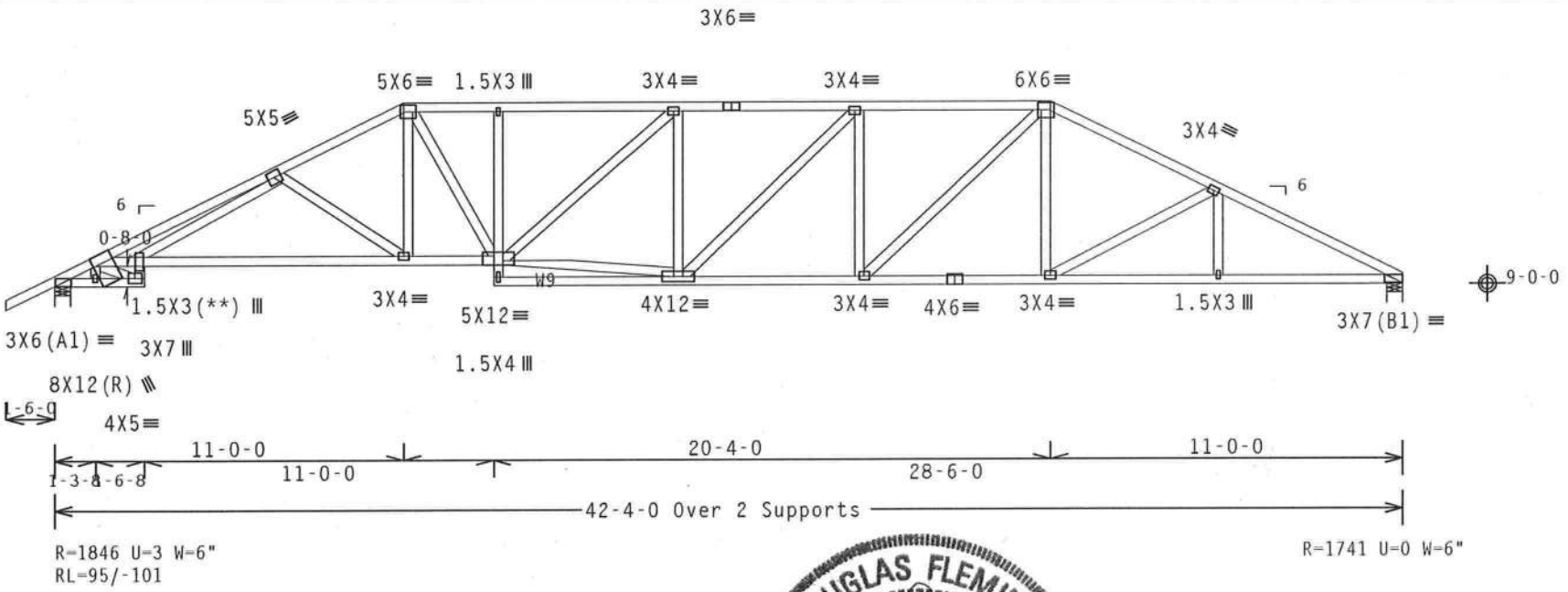
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20 QTY 1

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44483
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068014
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274602
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H17AT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

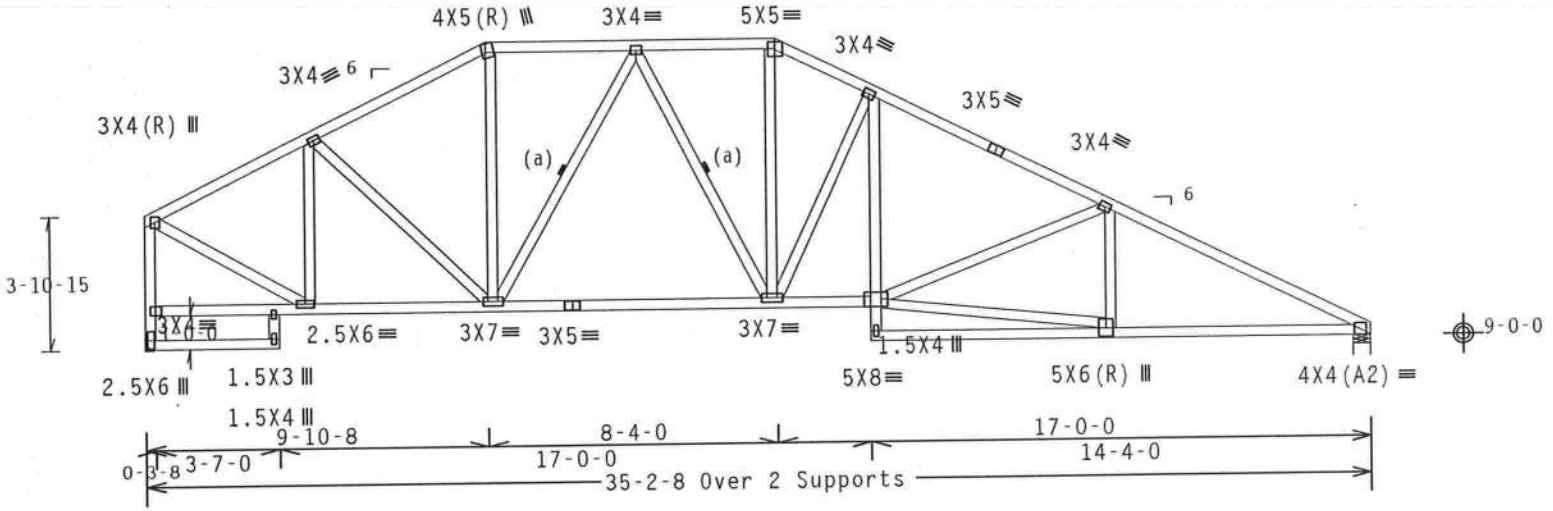
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1444 U=0
RL=90/-114 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

R=1456 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

PLT TYP. Wave

10.03.11.0209.20 QTY:

FL/-/4/-/-/R/- Scale =.1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 44484
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068015
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274657
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H19AT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

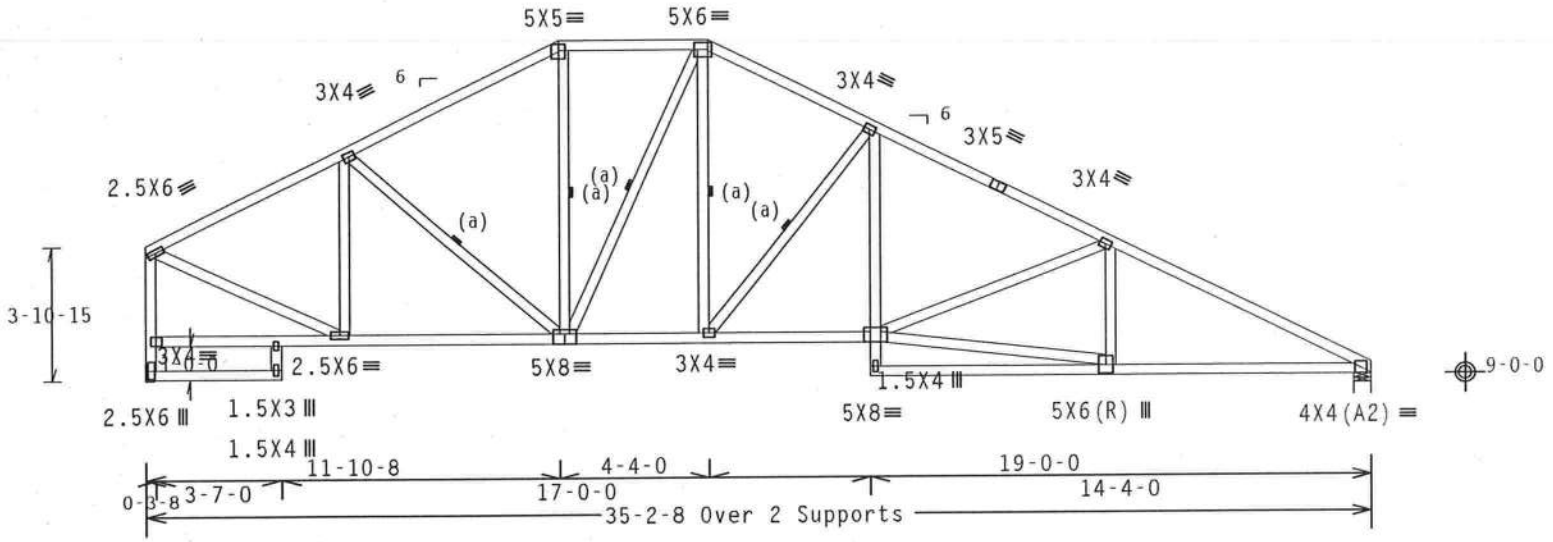
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1444 U=0
RL=105/-129 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

R=1456 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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TC LL	20.0 PSF	REF	R487-- 44485
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068016
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274676
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H21AT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(J) hanger connection not found in inventory file for this condition. Provide connection.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.38 ft. from roof edge.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

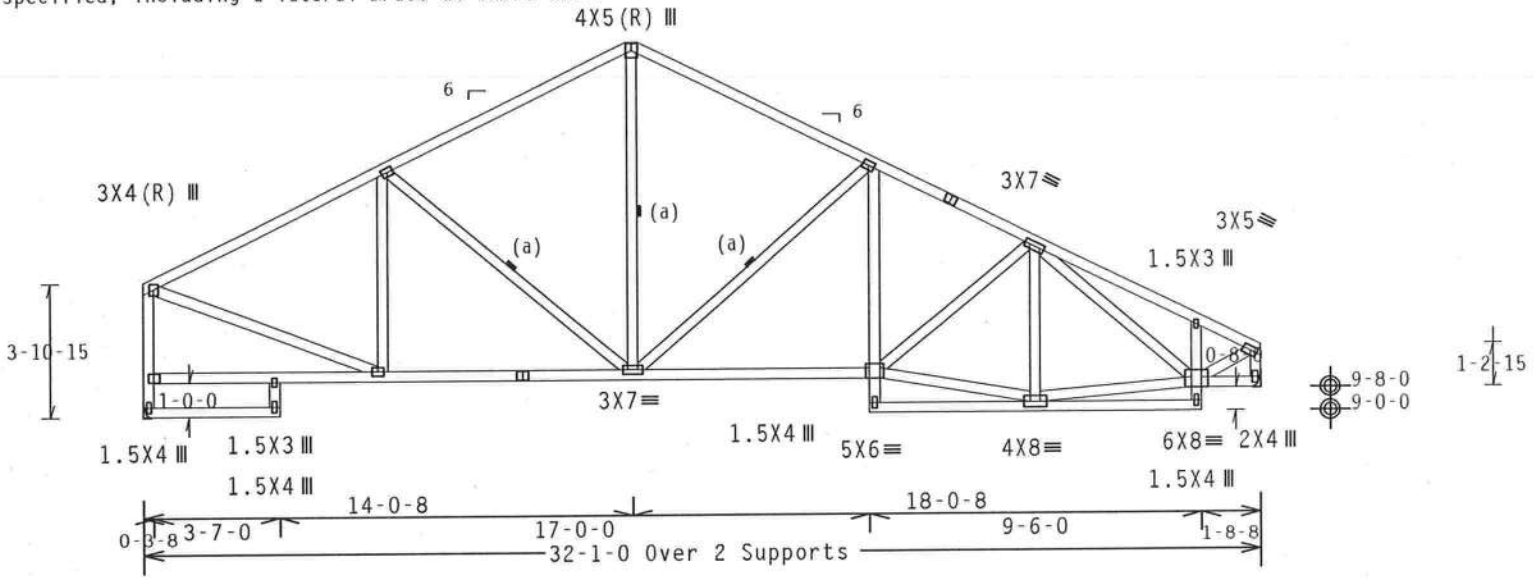
120 mph wind, 15.38 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1321 U=0
RL=111/-126 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.09.11.0209.20 QTY:

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

FL/-/4/-/-/R/-		Scale =.1875"/Ft.	
TC LL	20.0 PSF	REF	R487-- 44486
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068017
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274690
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - AT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(H1) = (J) Hanger not calculated (1)2x6 SP SS supporting member.
(H2) = (J) Hanger not calculated (1)2x6 SP M-26 supporting member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

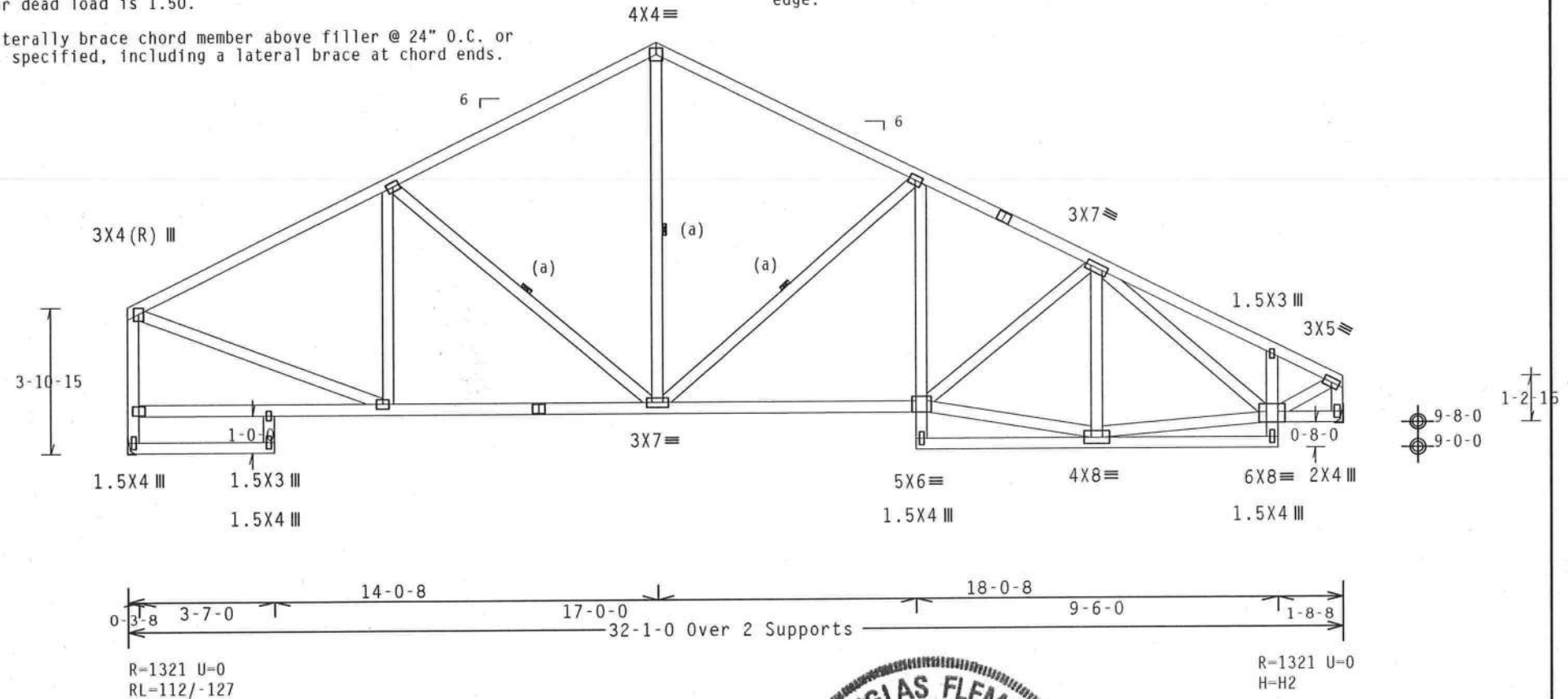
120 mph wind, 15.42 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.42 ft. from roof edge.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY 2 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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DOUGLAS FLEMING LICENSE No.66848 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44487
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068018
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274698
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - AT1)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

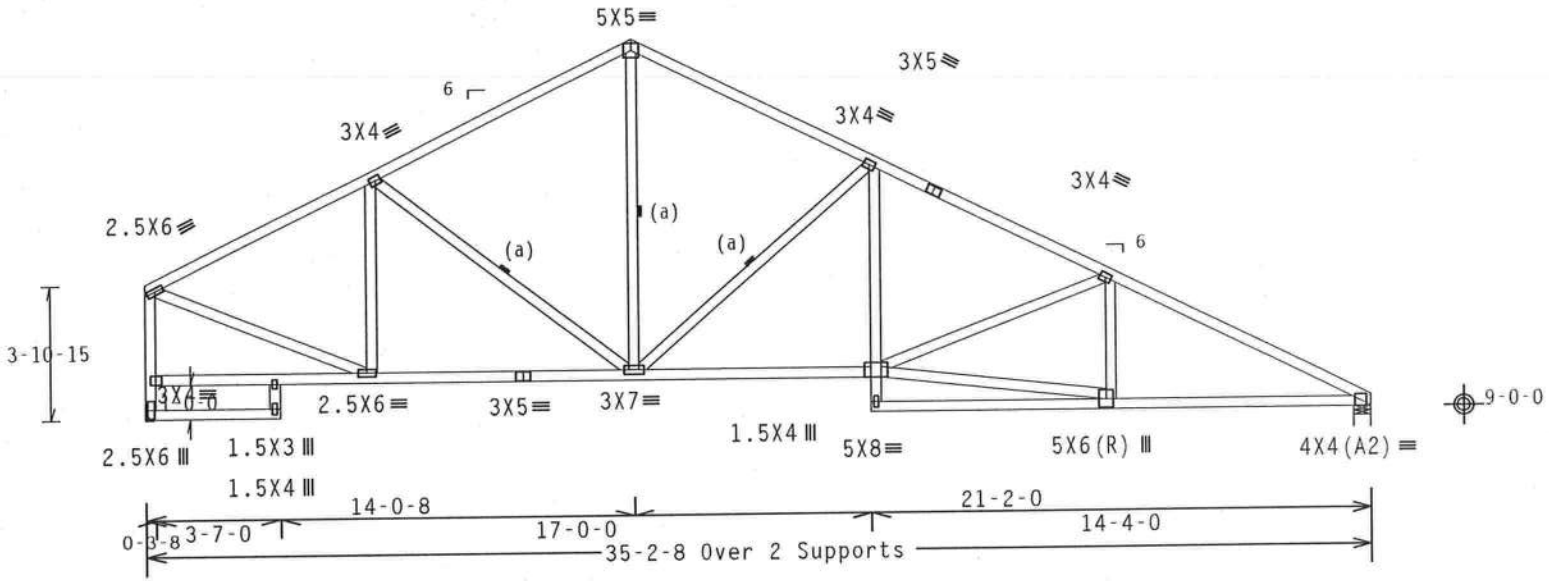
Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1444 U=0
RL=122/-146 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

R=1456 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
10.04.11.0209.20
03/08/2012

PLT TYP. Wave QTY: FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 44488
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068019
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274714
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - B)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

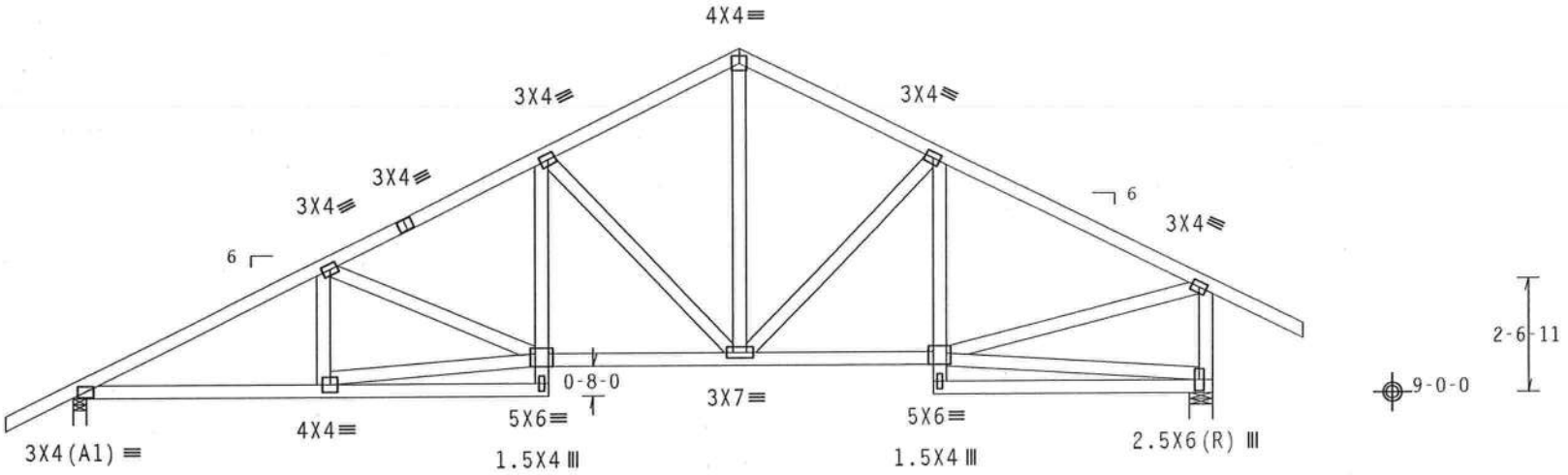
Bottom chord checked for 10.00 psf non-concurrent live load.

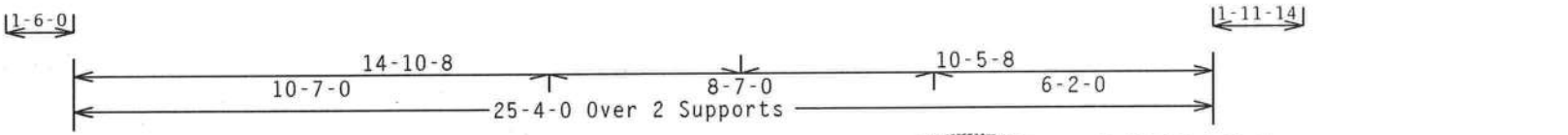
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.





R=1148 U=0 W=3.5"
RL=127/-109

R=1172 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.08.11.0209.20 QTY 12

FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/08/2012

TC LL	20.0 PSF	REF	R487-- 44489
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068020
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	274405
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - B1)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

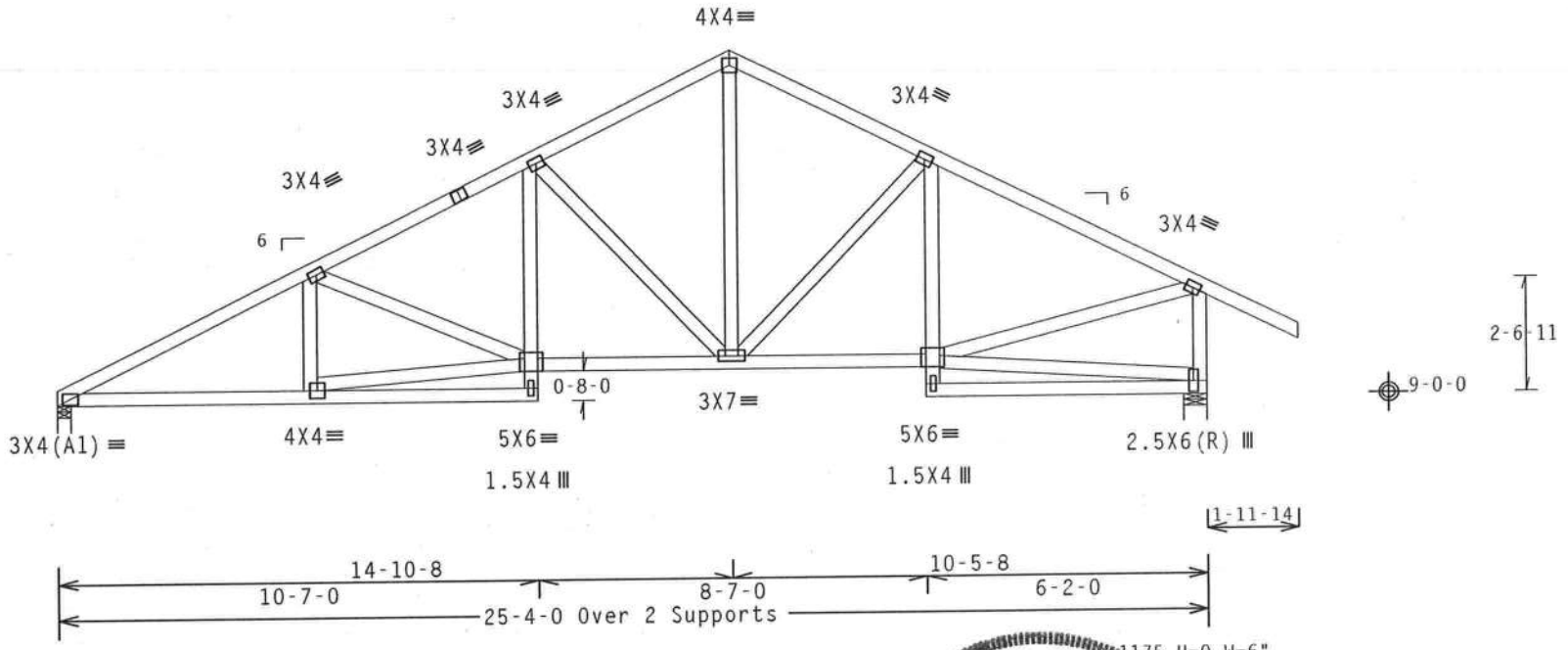
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20 QTY:

FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66848 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44490
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068021
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274398
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - BGE)

Top chord 2x4 SP #1 :T2 2x4 SP M-30:
Bot chord 2x4 SP M-30
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP M-30::Stack Chord SC2 2x4 SP M-30:

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.10" due to live load and 0.24" due to dead load.

Gable end supports 8" max rake overhang.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

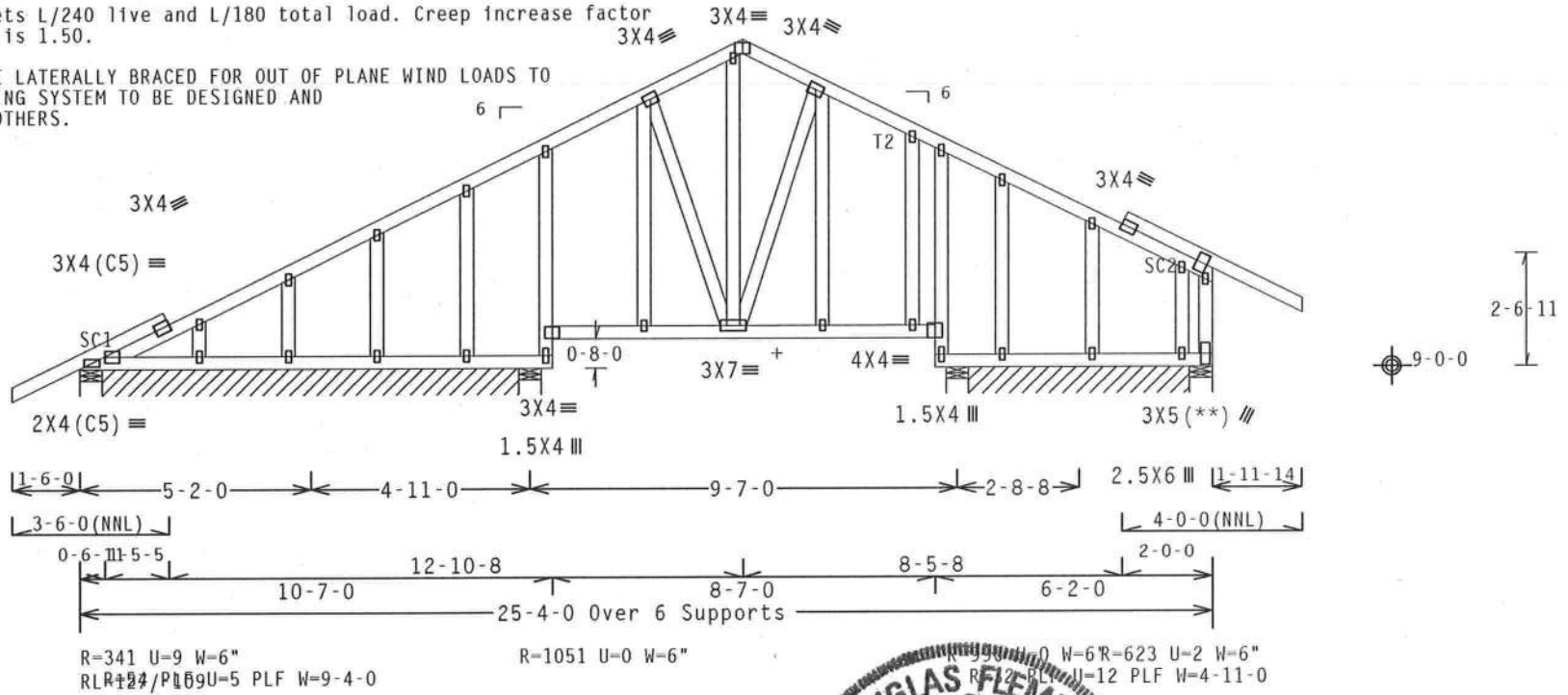
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.2D QTY 1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44491
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068022
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274748
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - CGE)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP M-30::Stack Chord SC2 2x4 SP M-30:

Roof overhang supports 2.00 psf soffit load.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

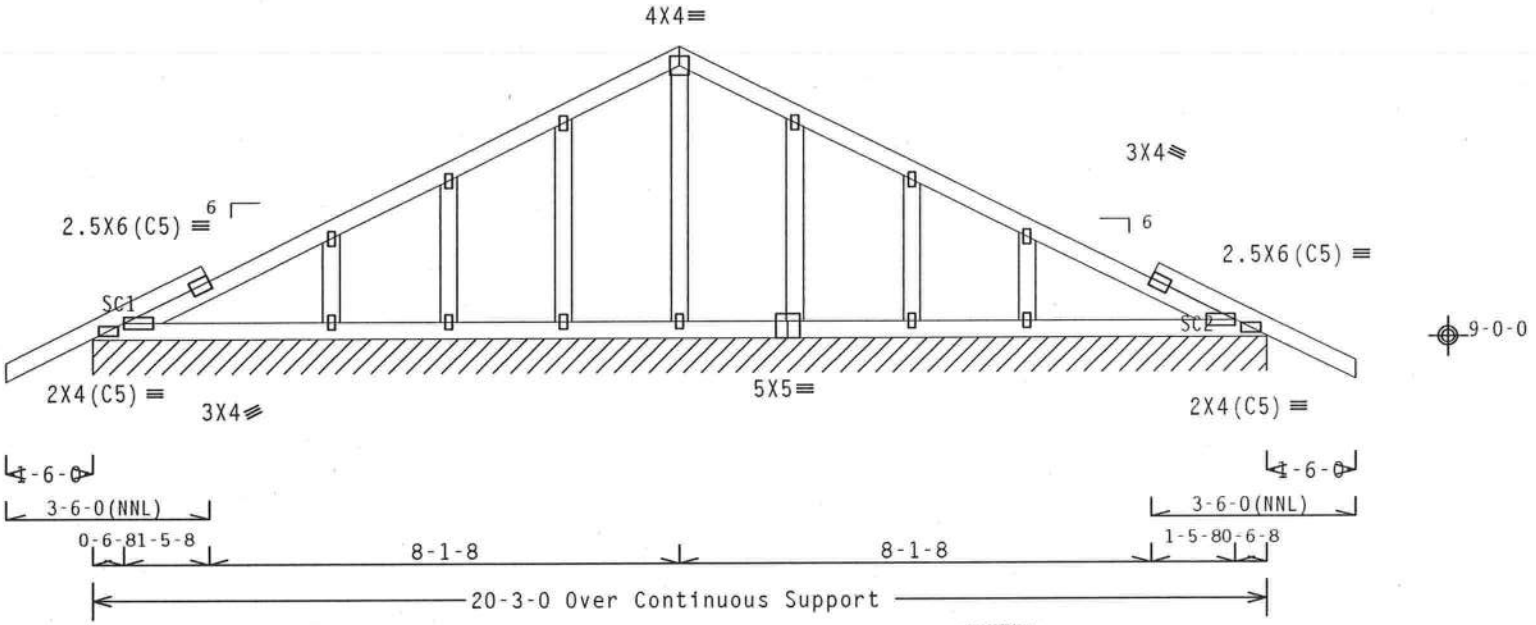
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



R=122 PLF U=0 PLF W=20-3-0
RL=5/-5 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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03/08/2012

TC LL	20.0 PSF	REF	R487-- 44492
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068023
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	274359
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - C)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

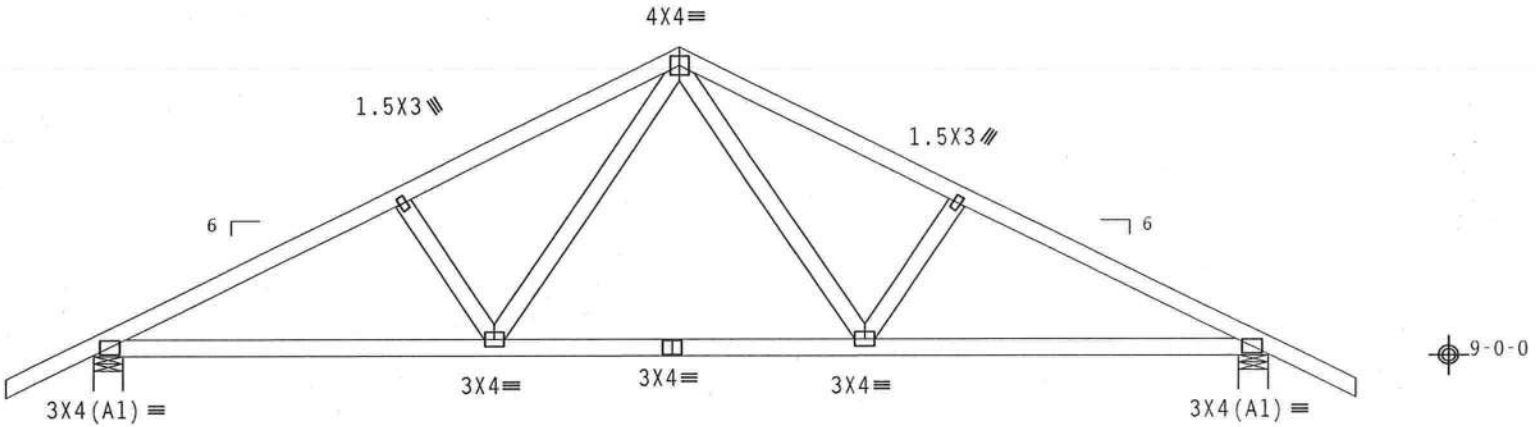
Roof overhang supports 2.00 psf soffit load.

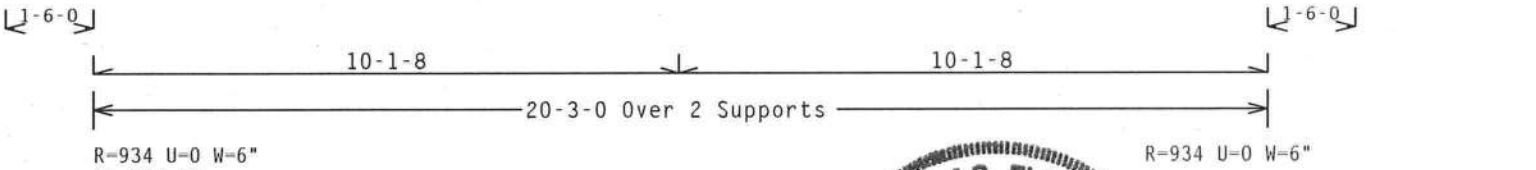
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=934 U=0 W=6"
RL=96/-96

R=934 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.2D QTY 5 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44493
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068024
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274369
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - C1)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

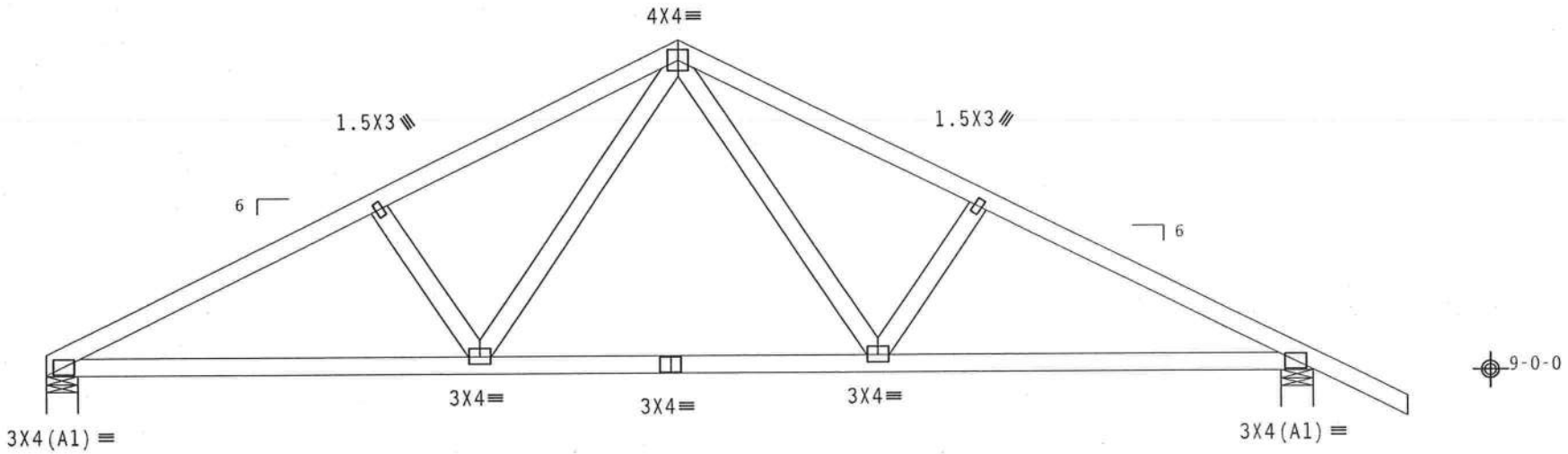
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.00.11.0209.20 QTY:

FL/-/4/-/-/R/-

Scale =.375"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 44494
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068025
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274421
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - H5D)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

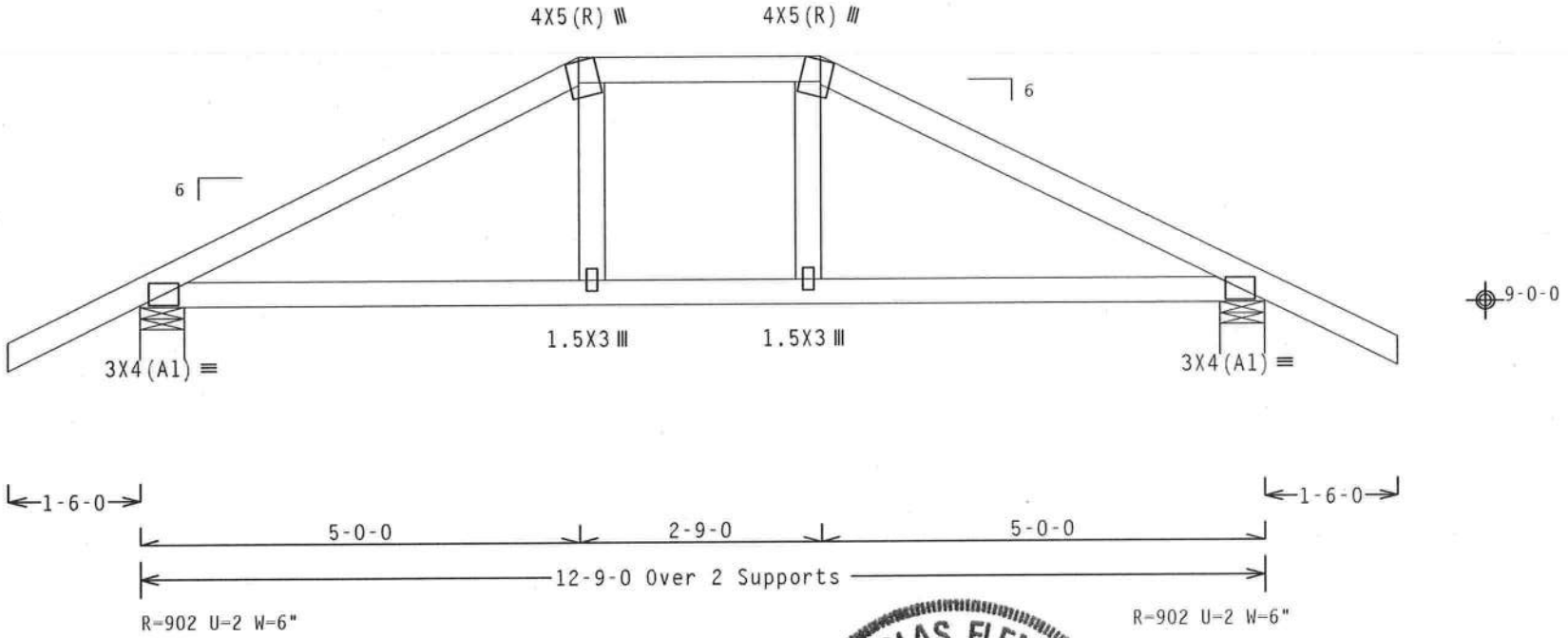
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

#1 hip supports 5-0-0 jacks with no webs.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.20 QTY: 6

FL/-/4/-/-/R/- Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 44495
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068026
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274350
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - D)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

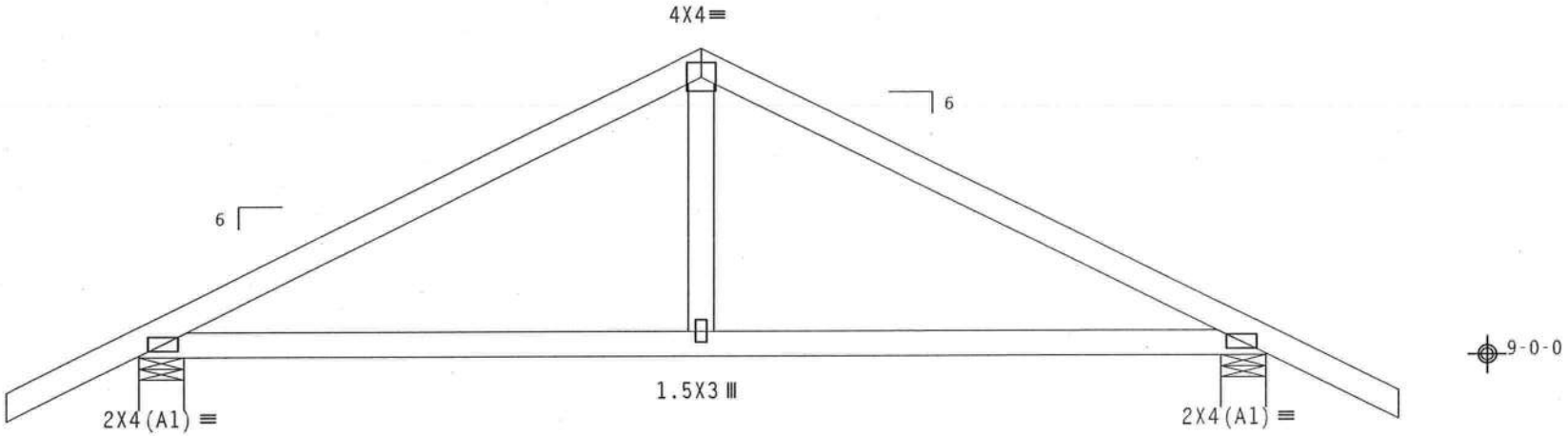
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20

QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 44496
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068027
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274353
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - DG)

Top chord 2x4 SP M-30
Bot chord 2x6 SP M-26
Webs 2x4 SP #3 :W3 2x4 SP M-30:
:Lt Wedge 2x4 SP #3:

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

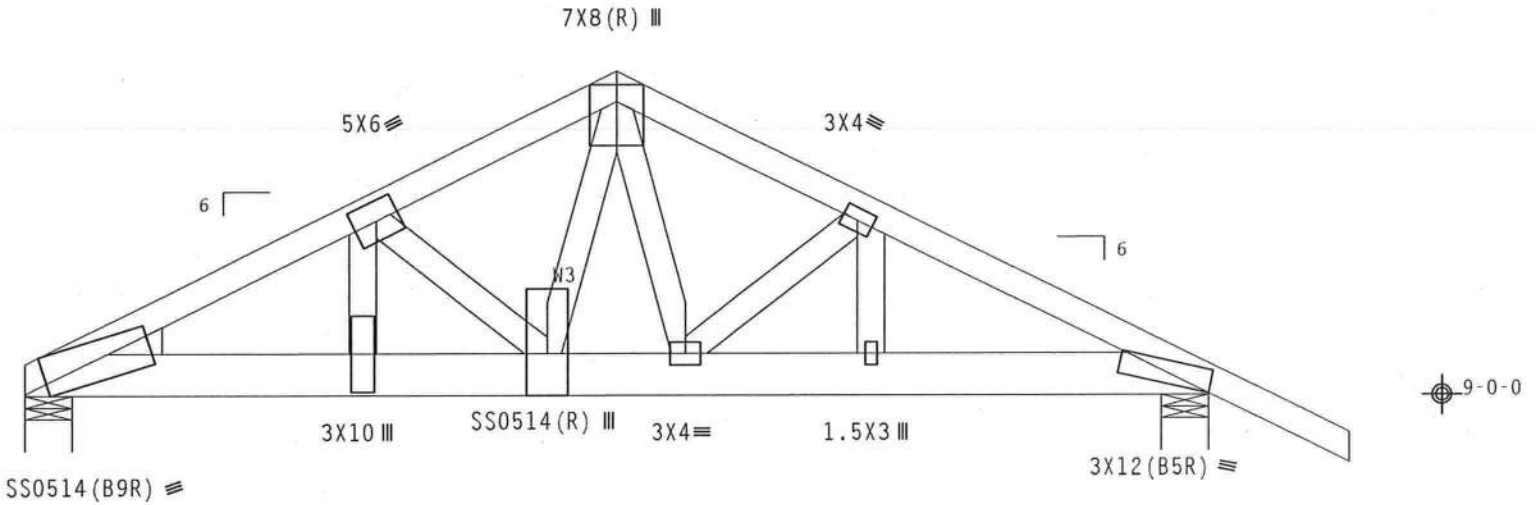
Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at 0.00 to 62 plf at 6.38
TC- From 62 plf at 6.38 to 62 plf at 14.25
BC- From 10 plf at 0.00 to 10 plf at 5.69
BC- From 20 plf at 5.69 to 20 plf at 12.75
BC- From 4 plf at 12.75 to 4 plf at 14.25
BC- 1645.51 lb Conc. Load at 1.69, 3.69
BC- 3690.78 lb Conc. Load at 5.69

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.20 | QTY 1 | FL/-/4/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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DOUGLAS FLEMING LICENSE No.66848 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44497
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068028
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274509
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - E)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

Calculated horizontal deflection is 0.06" due to live load and 0.20" due to dead load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

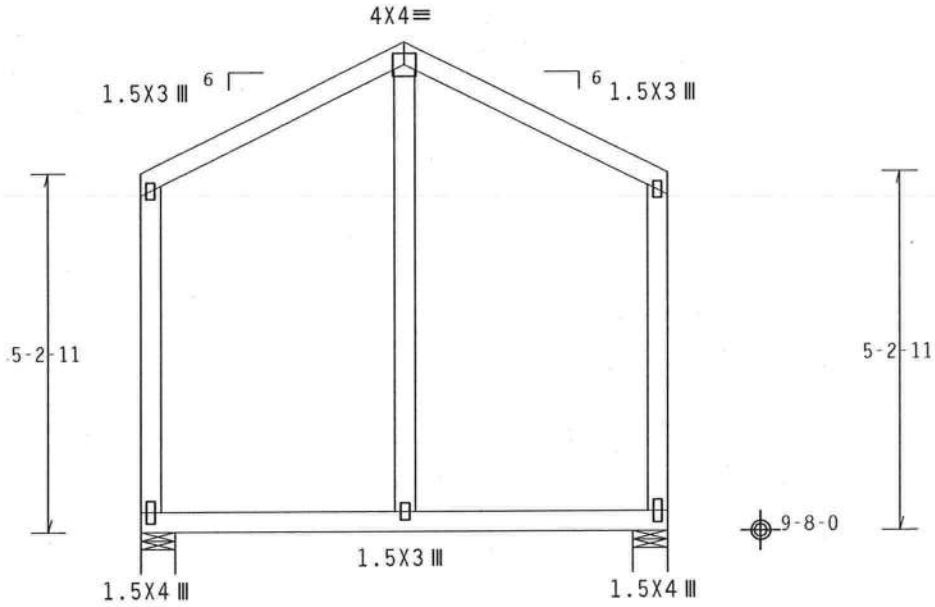
120 mph wind, 15.84 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Fasten rated sheathing to one face of this frame.

MWFRS loads based on trusses located at least 7.92 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20 QTY:

FL/-/4/-/-/R/- Scale =.375"/Ft.

TC LL	20.0 PSF	REF	R487-- 44498
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068029
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274413
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - EG)

Top chord 2x4 SP M-30
Bot chord 2x6 SP M-26
Webs 2x4 SP #3

120 mph wind, 15.84 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

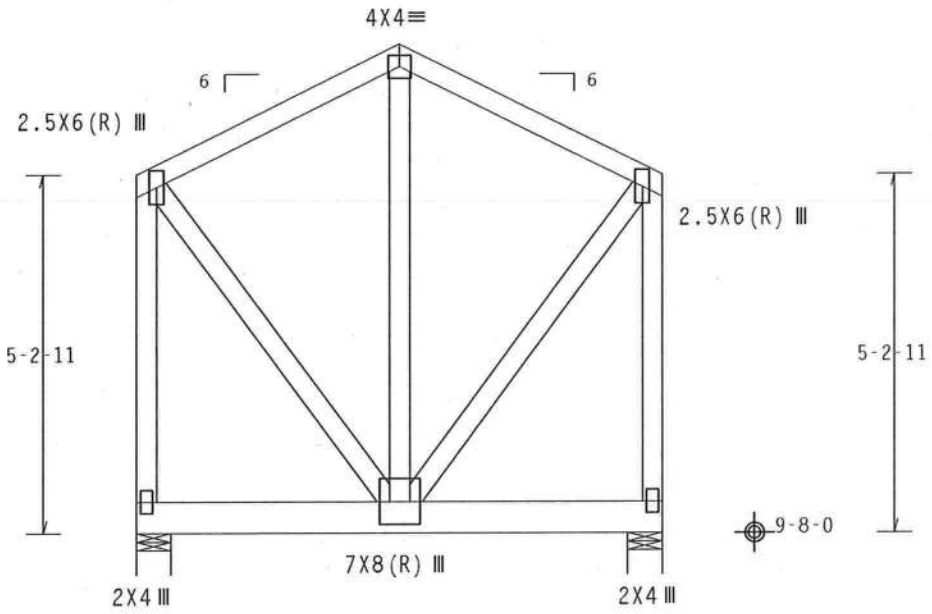
Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at 0.00 to 62 plf at 3.79
TC- From 62 plf at 3.79 to 62 plf at 7.58
BC- From 10 plf at 0.00 to 10 plf at 7.58
BC- 1321.20 lb Conc. Load at 1.65, 3.65, 5.65

End verticals not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF	R487-- 44499
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274732
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - F5F)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

Girder supports 5-0-0 span to BC one face and 2-0-0 span to TC/BC split opposite face.

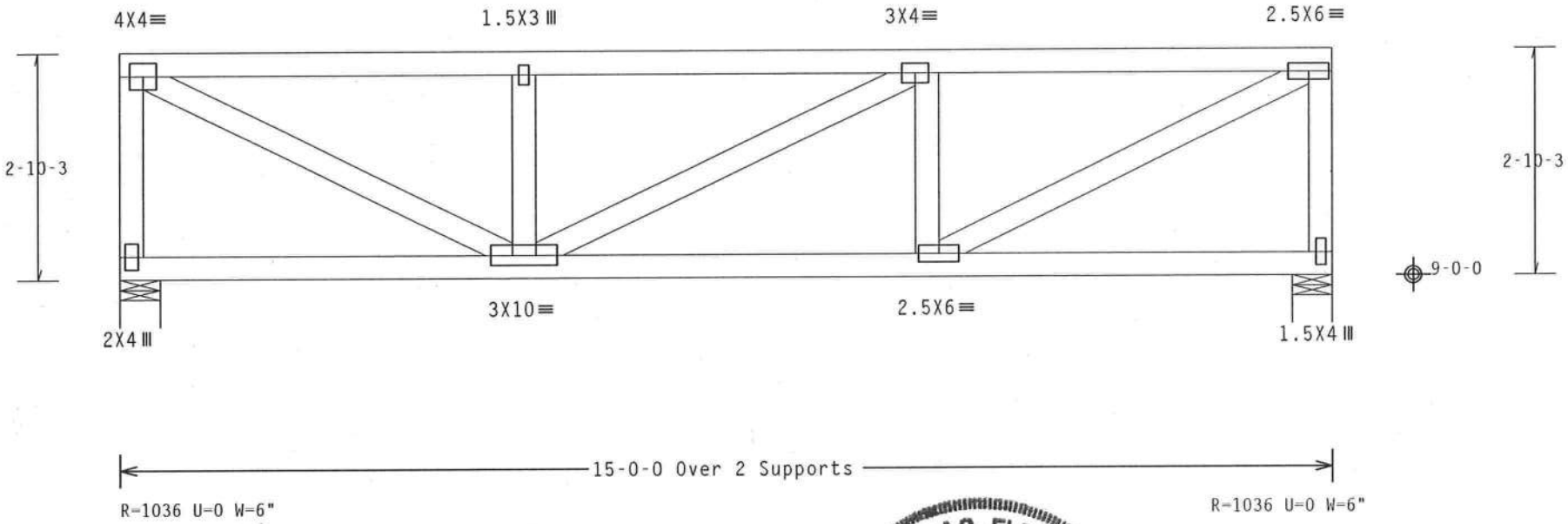
Truss must be installed as shown with top chord up.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.20 QTY 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

TC LL	20.0 PSF	REF R487-- 44500
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068031
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274718
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - F7F)

Top chord 2x4 SP M-30
Bot chord 2x6 SP SS
Webs 2x4 SP #3 :W2, W8 2x4 SP M-30:

Brg blocks:0.131"x3" nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0.000'	1	12"	5	Rigid Surface
2	14.500'	1	12"	4	Rigid Surface

Brg block to be same size and species as bottom chord.
Refer to drawing CNNAILSP0109 for more information.

Wind loads and reactions based on MWFRS with additional C&C member design.

Max JT VERT DEFL: LL: 0.12" DL: 0.17" recommended camber 1/4"

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at 0.00 to 60 plf at 15.00
BC- From 10 plf at 0.00 to 10 plf at 15.00
BC- 1443.86 lb Conc. Load at 1.23, 3.23, 5.23,13.23
BC- 1321.20 lb Conc. Load at 7.23, 9.23,11.23

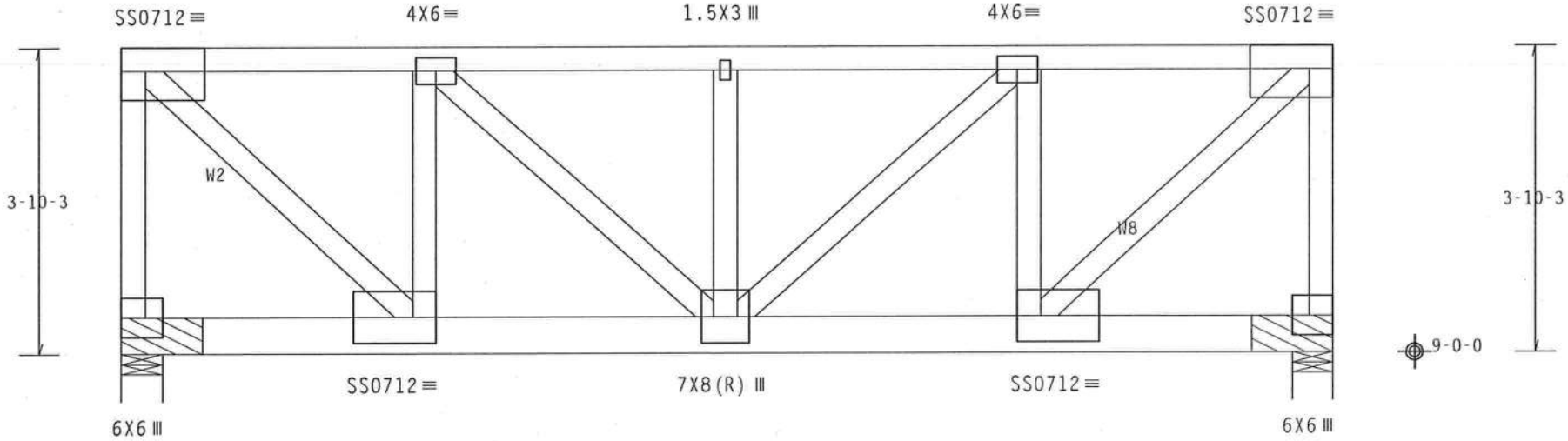
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. 18 Gauge HS,Wave

10.03.11.0209.20 QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/08/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 44501
TC DL	10.0 PSF	DATE 03/08/12
BC DL	10.0 PSF	DRW HCUSR487 12068032
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 274724
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UK8487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-049--Fill in later BRYAN ZECHER/KOCH -- , ** - CJ1)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30

Roof overhang supports 2.00 psf soffit load.

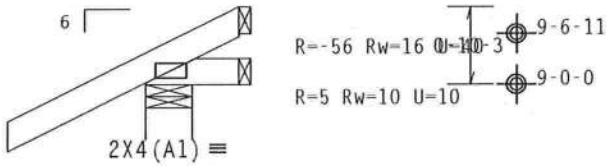
Bottom chord checked for 10.00 psf non-concurrent live load.

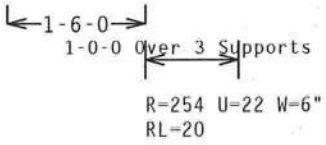
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11 0209.20

QTY:8

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/08/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 44502
TC DL	10.0 PSF	DATE	03/08/12
BC DL	10.0 PSF	DRW	HCUSR487 12068033
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	274223
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UK8487_Z01