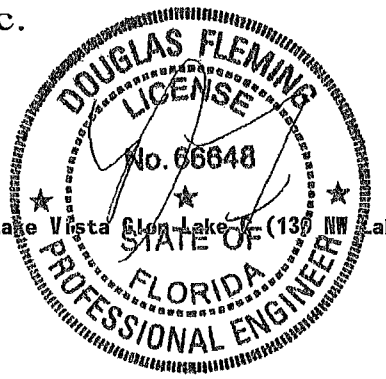
ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V4G487-Z0106092837



Truss Fabricator	**Anderson Truss Company**
Job Identification	**14-034--Don Reed Construction /Kerry Yates Add. -- 130 NW Lake Vista Glen-Lake C (130 NW Lake**
Truss Count	**2**
Model Code	**Florida Building Code 2010**
Truss Criteria	**FBC2010Res/TPI-2007(STD)**
Engineering Software	**Alpine Software,Version 12.03.**
Structural Engineer of Record	**The identity of the structural EOR did not exist as of**
Address	**the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads	**Roof - 37.0 PSF @ 1.25 Duration**
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Open Clear Wind

03/06/2014

Notes

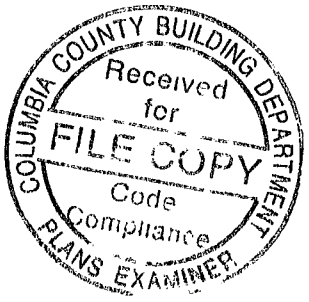
1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	79134--S1	25'1" Common	14065001	03/06/14
2	79135--DG1	25'1" Gable	14065002	03/06/14



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-034--Don Reed Construction /Kerry Yates Add. -- 130 NW Lake Vista Glen Lake C - S1 25'1" Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

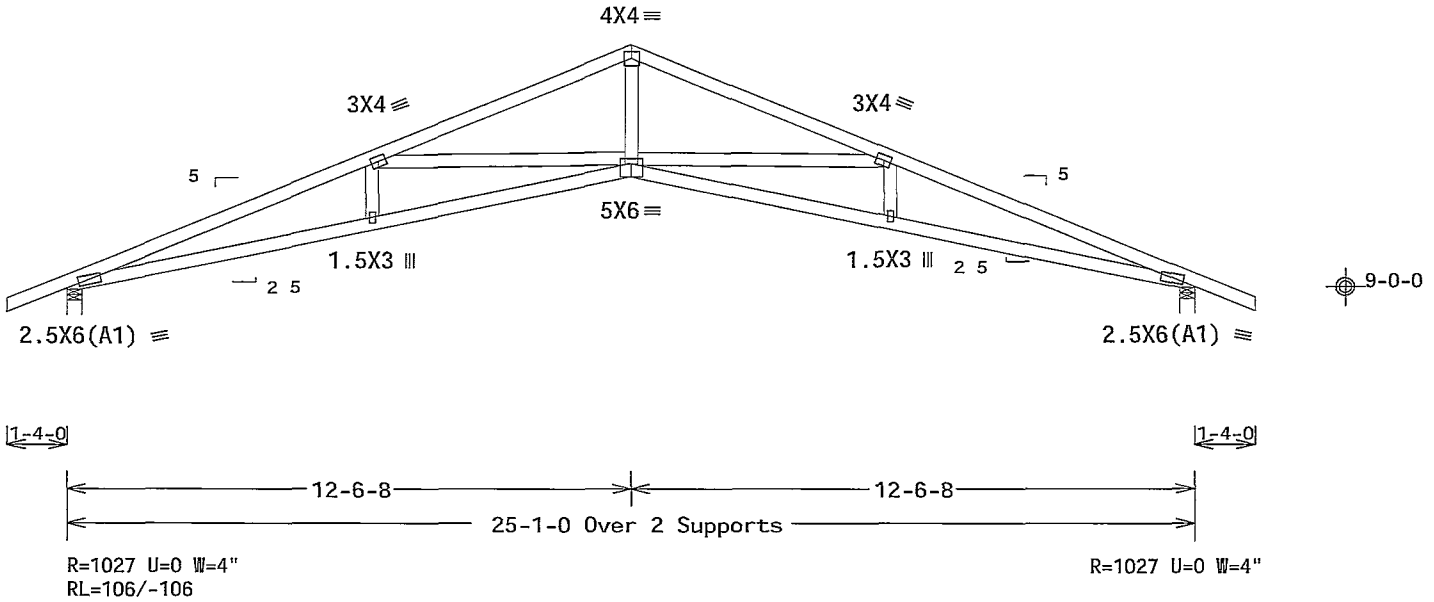
Calculated horizontal deflection is 0.18" due to live load and 0.22" due to dead load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.00

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04-0006.13 | QTY:6 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering respons bility solely for the des gn shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 79134
TC DL	7.0 PSF	DATE	03/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14065001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	28313
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4G487_Z01

(14-034--Don Reed Construction /Kerry Yates Add. -- 130 NW Lake Vista Glen Lake C - DG1 25'1" Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B·:Stack Chord SC2 2x4 SP #1-13B·

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCpi(+/-)=0.00

Wind loads and reactions based on MWFRS with additional C&C member design.

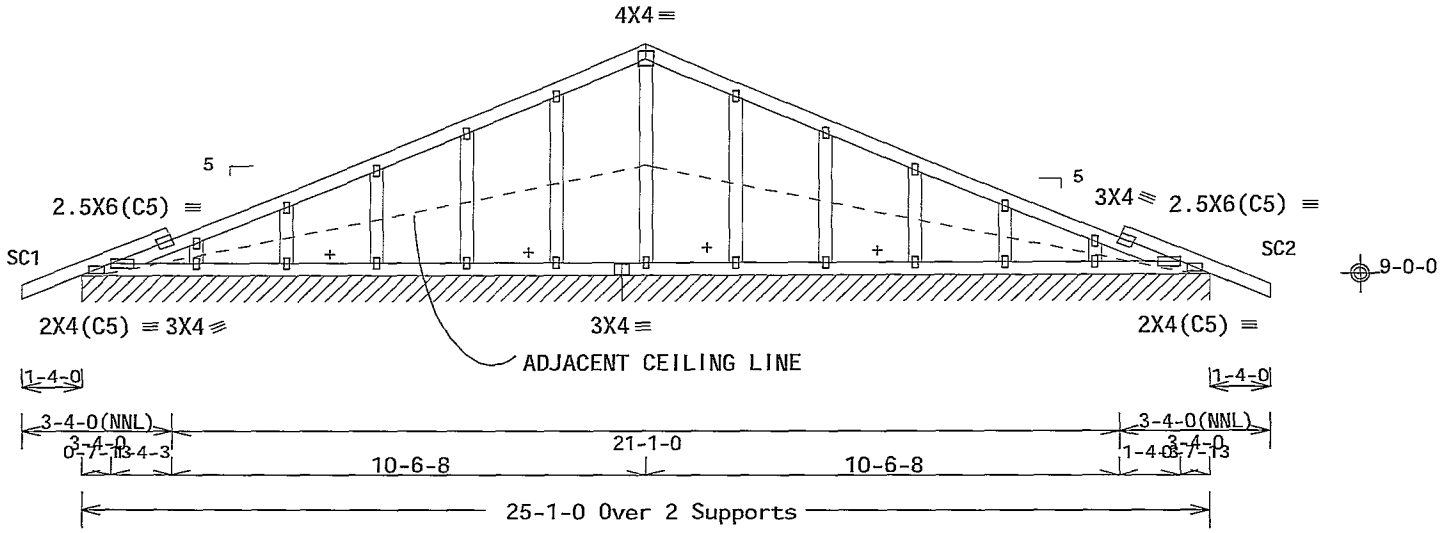
Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50.

THE PROJECT ENGINEER SHALL PROVIDE FOR ENDWALL STABILITY PER SECTION 2304.3.4.1 OF THE 2010 FLORIDA BUILDING CODE. THE TOP OF THE WALL BELOW THIS TRUSS SHALL BE LATERALLY BRACED AS SPECIFIED BY THE PROJECT ENGINEER. THIS TRUSS WILL NOT PROVIDE LATERAL SUPPORT OF THE ENDWALL.



R=120 PLF U=3 PLF W=12-0-0
RL=12/-12 PLF

R=129 PLF U=0 PLF W=13-1-0

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

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TC DL	7.0 PSF	DATE	03/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14065002
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	28319
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4G487_Z01

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

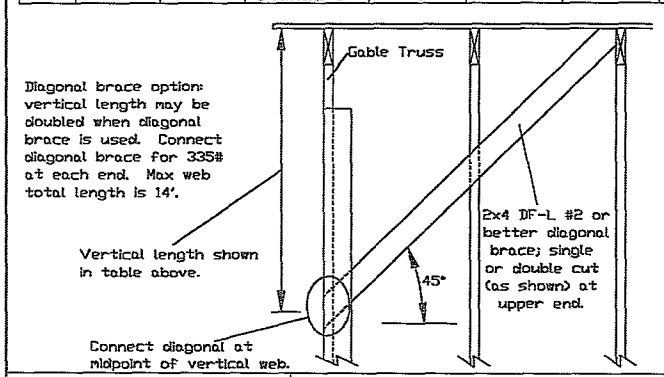
** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

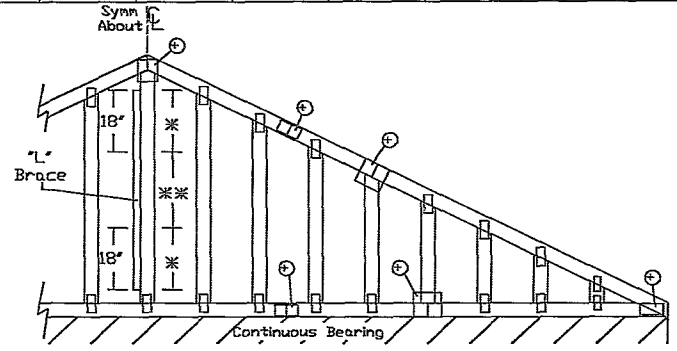
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

TW Building Components Group Inc.

Earth City MO 63045

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DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

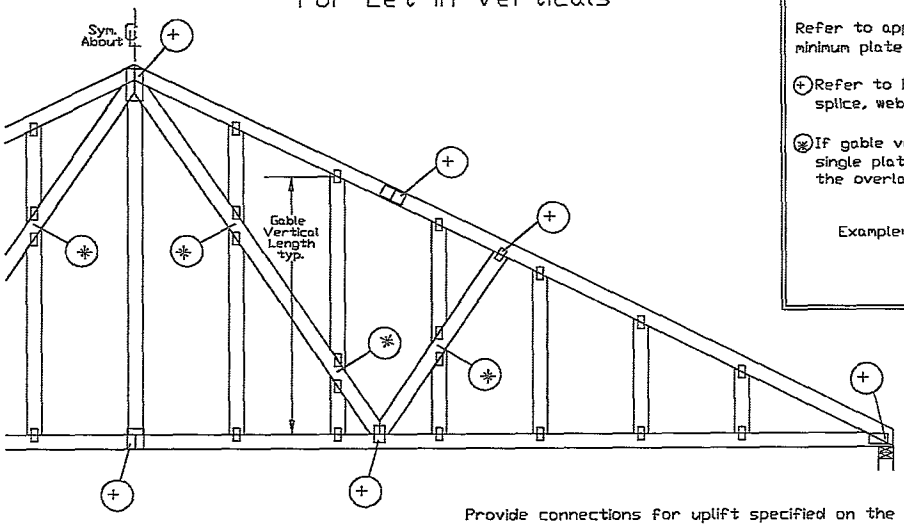
06 '14

MAX. TOT L.D. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



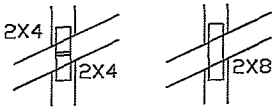
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

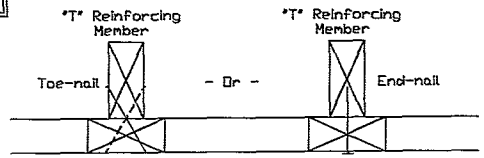
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

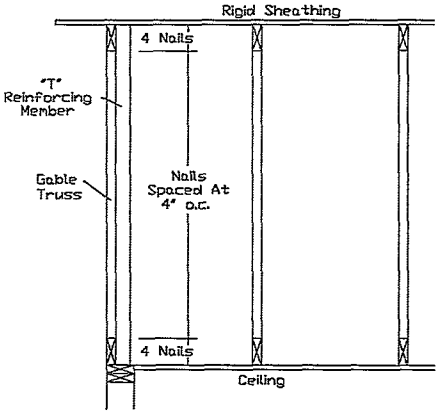
⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail





Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212, A16015ENC100212,
A18015ENC100212, A20015ENC100212, A20015END100212, A20015PED100212,
A11530ENC100212, A12030ENC100212, A14030ENC100212, A16030ENC100212,
A18030ENC100212, A20030ENC100212, A20030END100212, A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Building Components Group Inc.

Earth City, MO 63045

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DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '14

MAX. TOT LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

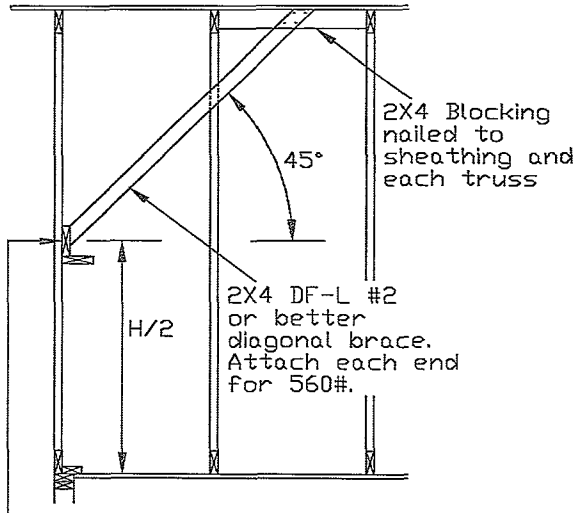
Nails: 10d box or gun (0.128"x3",min) nails.

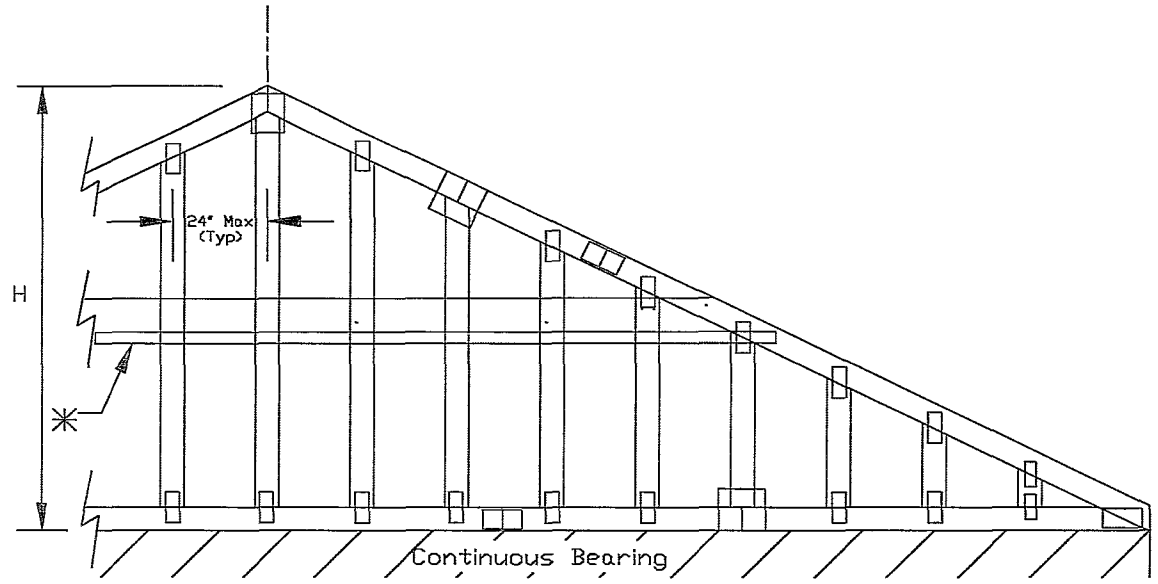
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.





Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

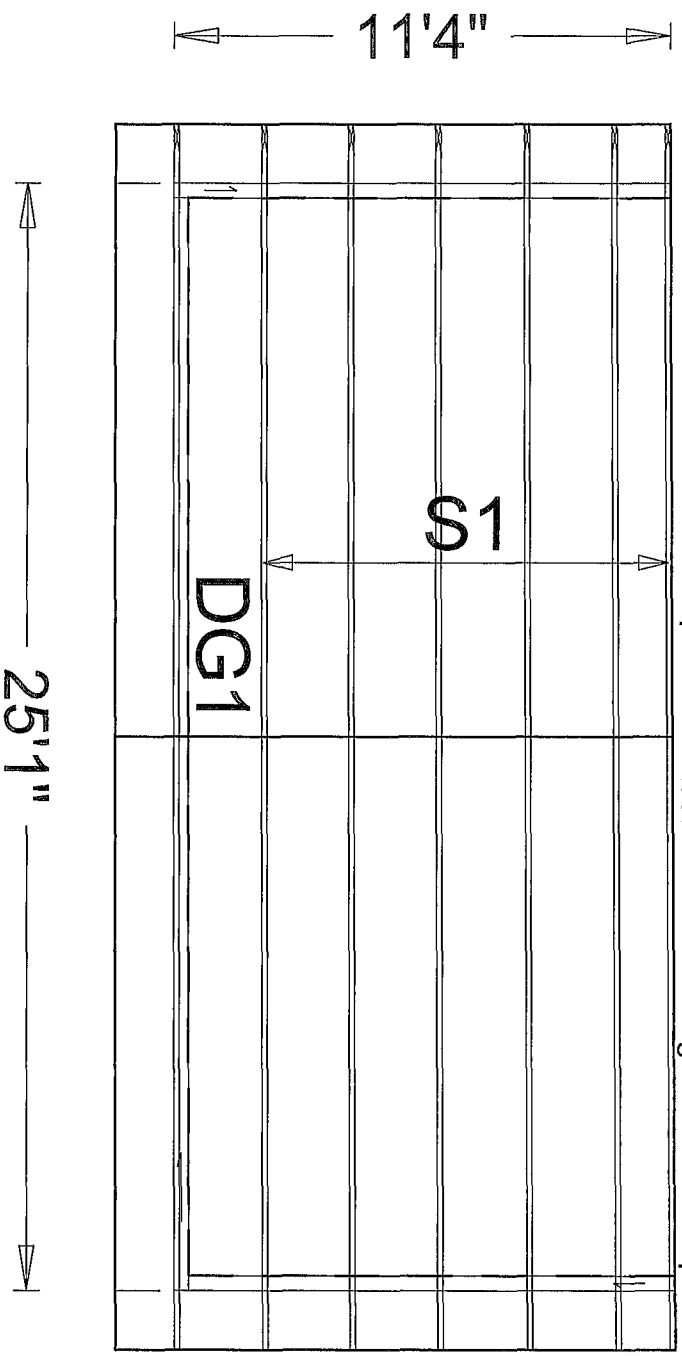
ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2.
For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Feb '14

MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212



Yates Add.

Total Truss Quantity = 7.

Total Plan Area with OHs = 351 sq.ft Roof Plane Sheathing Area = 381 sq. ft

PAGE NO 1 OF 1	JOB NO 14-034	Customer: Don Reed Construction Job Name: Kerry Yates Add 130 NW Lake Vista Glen Job Numb: 14-034 Designer: Curt V Burlingame Salesman: Curt V Burlingame	Created 03-04-2014 <Not Found>	ANDERSON TRUSS COMPANY "QUALITY TRUSS ENGINEERED WOOD TRUSSES"