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This document was prepared by:  
ROBERT CASTILLO  
COUNTRYWIDE HOME LOANS, INC.

6400 LEGACY DR  
PLANO, TX 75024

Inst:2006023486 Date:10/02/2006 Time:10:38  
DC, P. Dewitt Cason, Columbia County B:1097 P:2274

[Space Above This Line For Recording Data]

144541522  
(Loan #)

**NOTICE OF COMMENCEMENT**

STATE OF FLORIDA  
COUNTY OF COLUMBIA

TITLE UNDERGROUND hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 715, Florida Statutes, the following information is provided in this Notice of Commencement.

- 1. Description of property: (legal description of property, and street address if available)

Tax Parcel #16-7S-17-10006-212

Lot 12 of RIVER RISE RESIDENTIAL SUBDIVISION, UNIT 1, according to the Plat thereof as recorded in Plat Book 8, Pages 51 through 53, of the Public Records of Columbia County, Florida.

Parcel ID Number: 16-7S-17-10006-212

- 2. General description of improvement: Residential

- 3. Owner information:

- a. Name and address: Michael J. McAlhany and Patricia S. McAlhany  
25103 NW 122nd Avenue, High Springs, FL 32643
- b. Interest in property: Fee Simple
- c. Name and address of fee simple titleholder (if other than owner): Not Applicable



Record & Return To:  
Darryl J. Tompkins, P.A.  
P.O. Box 519  
Alachua, FL 32616

Parcel ID Number: 16-7s-17-10006-001 Portion of

Inst:2006006514 Date:03/16/2006 Time:12:35

Doc Stamp-Deed : 543.90

S-7 DC, P. DeWitt Cason, Columbia County B:1077 P:146:

# Warranty Deed

This Indenture, Made this 7<sup>th</sup> day of MARCH, 2006 A.D., **Between**

Nevin G. Summers, a married man

of the Borough of Anchorage, State of Alaska, **Grantor**, and

Michael J. McAlhany and Patricia S. McAlhany, husband and wife

whose post office address is : 25103 NW 122<sup>nd</sup> Avenue, High Springs, Florida 32643

of the County of Alachua, State of Florida, **Grantee**

**Witnesseth** that the GRANTOR, for and in consideration of the sum of TEN & NO/100 (\$10.00), and other good and valuable consideration to GRANTOR in hand paid by GRANTEE, the receipt of which is hereby acknowledged, has granted, bargained and sold to the said GRANTEE and GRANTEE'S successors and assigns forever, the following described land, situate, lying and being in the County of Columbia, State of Florida to wit:

LOT 12, RIVER RISE RESIDENTIAL SUBDIVISION, UNIT 1, A SUBDIVISION ACCORDING TO PLAT THEREOF RECORDED IN PLAT BOOK 8, PAGES 51 THROUGH 53 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

SUBJECT TO THE FOLLOWING:

- A. Zoning restrictions, prohibitions and other requirements imposed by governmental authority;
- B. Restrictions and matters appearing on the plat and/or common to the subdivision;
- C. Taxes for the year 2006 and subsequent years.

The land described herein is not the homestead of the grantor(s), and neither the grantor(s) nor the grantor(s) spouse, nor anyone for whose support the grantor(s) is responsible, resides on or adjacent to said land

and the grantor does hereby fully warrant the title to said land, and will defend the same against lawful claims of all persons whomsoever.

**In Witness Whereof**, the grantor has hereunto set his hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

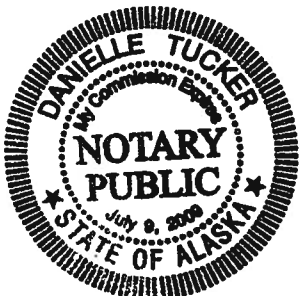
Kristine Gillham  
Printed Name: Kristine Gillham

Nevin G. Summers  
NEVIN G. SUMMERS

Danielle Tucker  
Printed Name: Danielle Tucker

STATE OF ALASKA  
BOROUGH OF Kenai Peninsula

The foregoing instrument was acknowledged before me this 7<sup>th</sup> day of March, 2006, by NEVIN G. SUMMERS, who is personally known to me or has produced his AK Drivers License as identification.



Danielle Tucker  
Notary Public State of Alaska  
Printed Name: Danielle Tucker  
My Commission Expires: July 9, 2009

One copy of windows & Door Specs  
they are the same for the SPD  
and the Garage.

WEEK COPY OF DECED

### Columbia County Building Permit Application

**For Office Use Only** Application # 0610-38 Date Received 10/11/06 By LH Permit # 25147  
 Application Approved by - Zoning Official BLK Date 10-10-06 Plans Examiner DK JTH Date 10-19-06  
 Flood Zone A-3 Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3  
 Comments Accessory use  
 NOC  EH  Deed or PA  Site Plan  State Road Info  Parent Parcel #  Development Permit

Name Authorized Person Signing Permit Scott Rosenboom Phone 352-538-3877  
 Address 19802 NW 190th AVE High Springs, FL 32643  
 Owners Name MICHAEL McALHANY Phone 386-454-8848  
 911 Address 420 SW Marynik Drive, High Springs, FL 32649  
 Contractors Name Scott Rosenboom Phone 352-538-3877  
 Address 19802 NW 190th AVE High Springs, FL 32643  
 Fee Simple Owner Name & Address Michael S. & Patricia S. McAlhany 25103 NW 122 Avenue High Springs, FL 32643  
 Bonding Co. Name & Address \_\_\_\_\_

Architect/Engineer Name & Address MARY ESKRIDGE ALACHUA FL 32615  
 Mortgage Lenders Name & Address Countrywide Home Loans, Inc. 4505 La Paloma Ave., Calabasas, CA 91302

Circle the correct power company -  FL Power & Light -  Clay Elec. -  Suwannee Valley Elec. -  Progressive Energy  
 Property ID Number 16-75-17-10006-212 Estimated Cost of Construction 250,500.00  
 Subdivision Name River Rise Lot \_\_\_\_\_ Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase 42,000  
 Driving Directions 441 South to 778 CORNER OF 778 + 441  
1st entrance off 778, on (R) 3/4 of the way down.

Type of Construction FRAME-DETACHED GARAGE Number of Existing Dwellings on Property 0  
 Total Acreage 5 Lot Size \_\_\_\_\_ Do you need a  Culvert Permit or  Culvert Waiver or  Have an Existing Drive  
 Actual Distance of Structure from Property Lines - Front 210 Side 126 Side 125 Rear 357  
 Total Building Height 16.21 Number of Stories 1 Heated Floor Area 2508 Roof Pitch 7/12  
 TOTAL 7,380

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

**OWNERS AFFIDAVIT:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Patricia S. McAlhany  
Owner Builder or Authorized Person by Notarized Letter

Scott Rosenboom  
Contractor Signature

STATE OF FLORIDA  
 COUNTY OF COLUMBIA  
 Sworn to (or affirmed) and subscribed to before me this \_\_\_\_\_ day of \_\_\_\_\_, 2006.  
 Personally known  or Produced Identification

**NOTARY PUBLIC**  
**Jeff Wolfe**  
 Commission # DD285878  
 Expires January 28, 2008

Contractors License Number \_\_\_\_\_  
 Competency Card Number \_\_\_\_\_  
**NOTARY STAMP/SEAL**  
Jeff Wolfe  
 Notary Signature (Revised Sept. 2006)



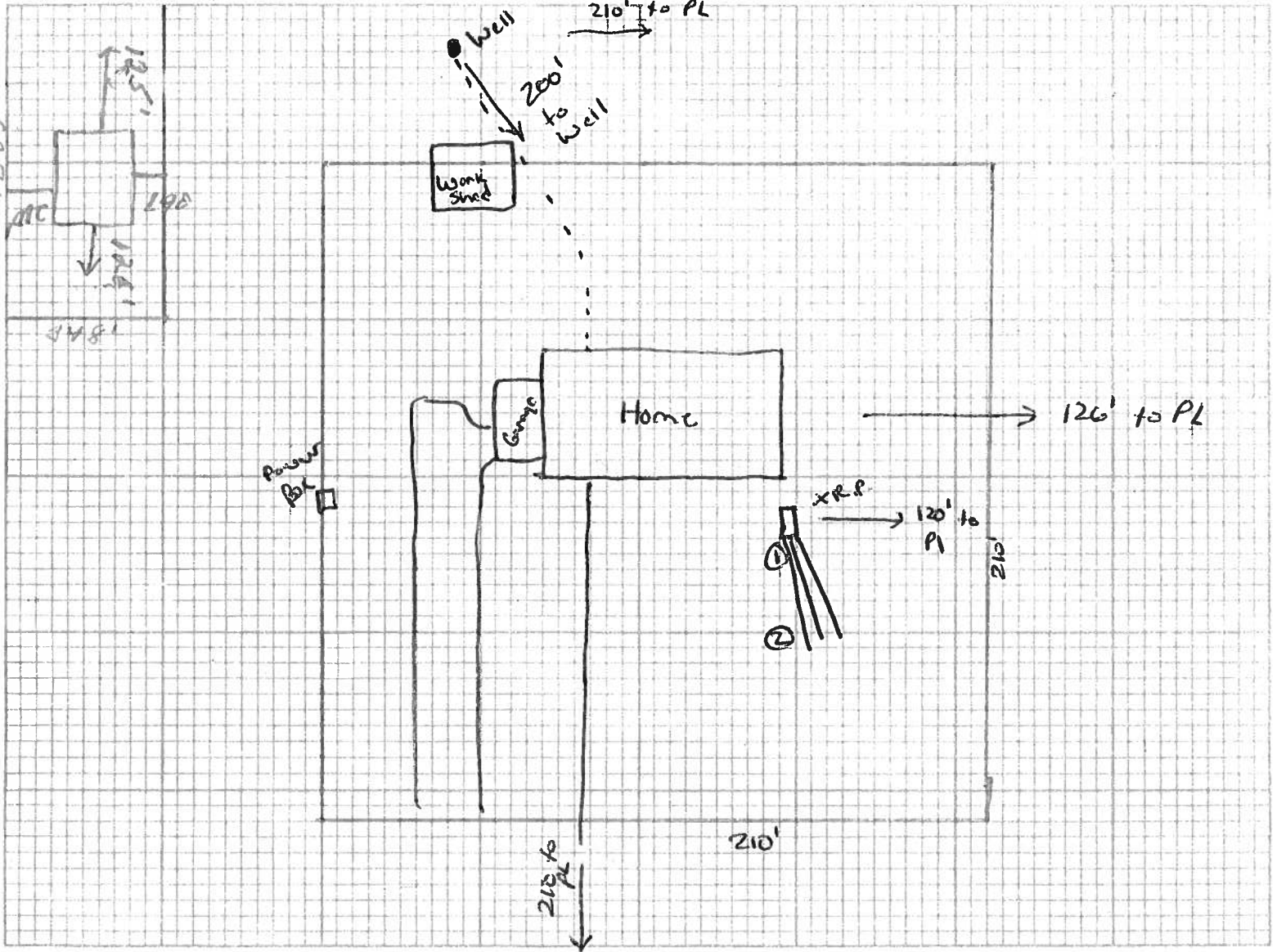
STATE OF FLORIDA  
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0883N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: \_\_\_\_\_

*OK per M.S.C.*

Site Plan submitted by: *Manuel B...*  
Signature

*Agent*  
Title

Plan Approved  Not Approved \_\_\_\_\_

Date 10/9/06

By *M...* *Columbia* County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



STATE OF FLORIDA  
DEPARTMENT OF HEALTH

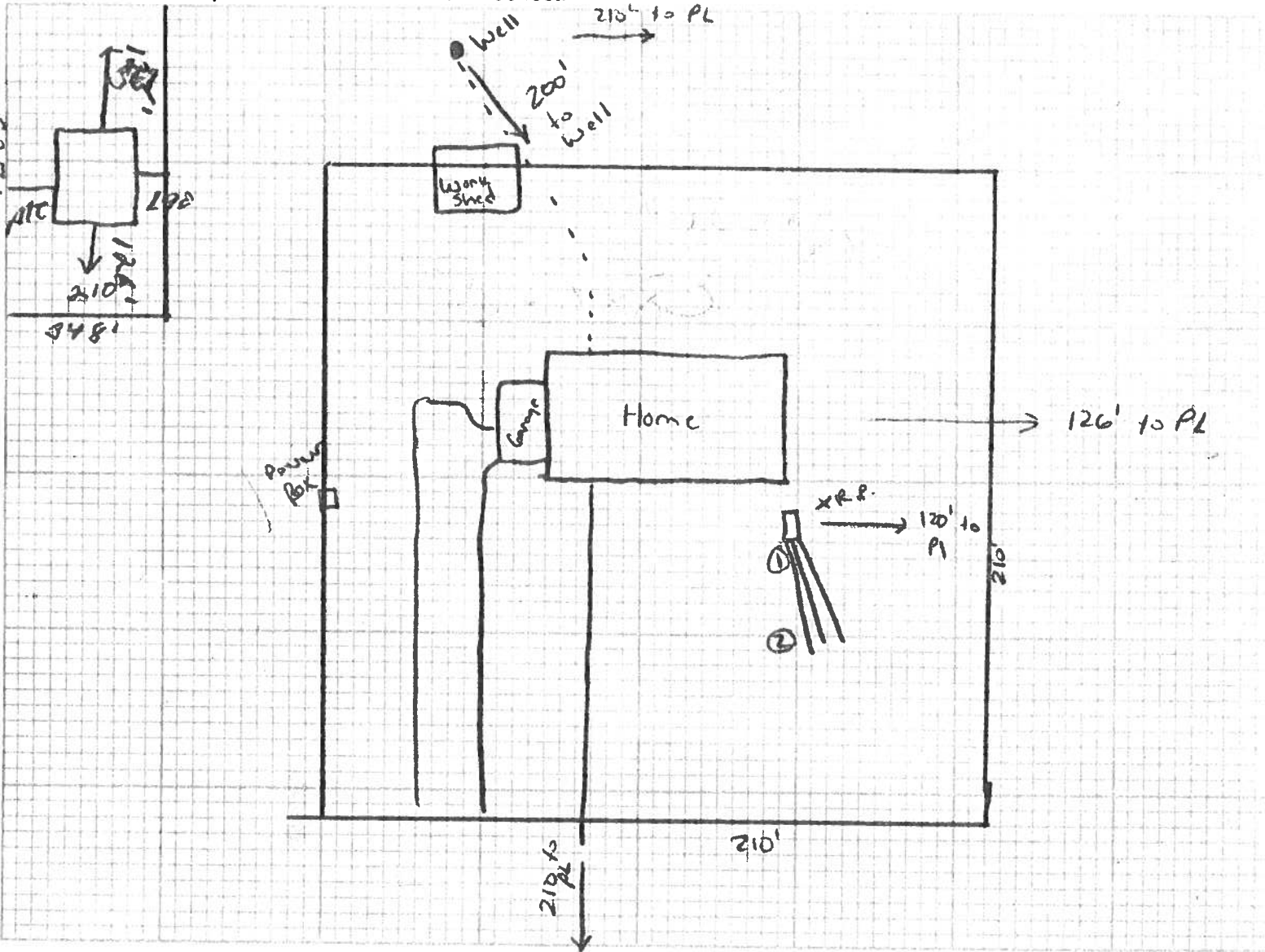
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number

06-08831V

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: \_\_\_\_\_

OK per MSC

Site Plan submitted by: Manuel Gonzalez Signature

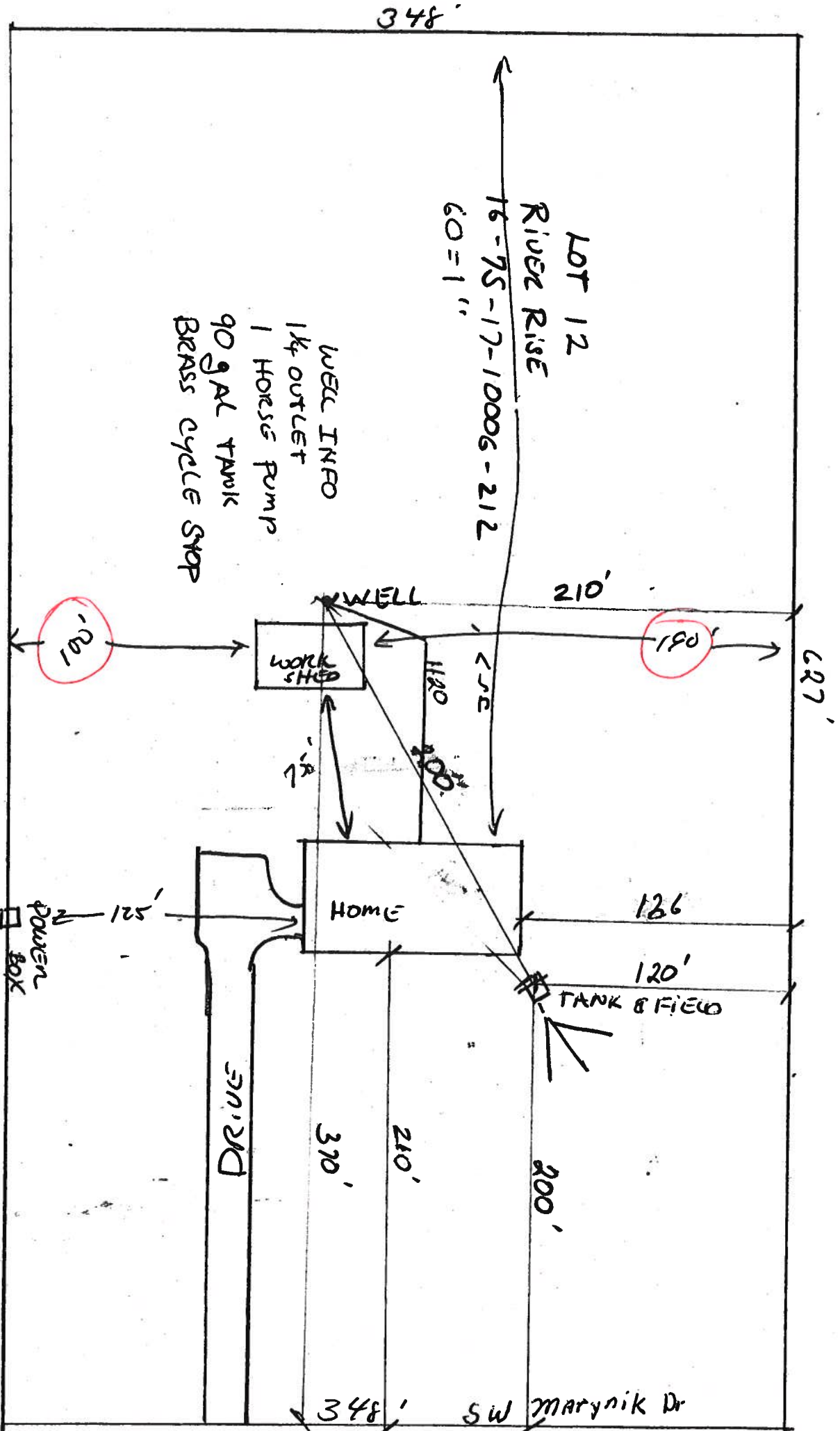
Agent Title

Plan Approved  Not Approved \_\_\_\_\_

Date 10/9/06

By [Signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



**MARTY R. ESKRIDGE**14952 MAIN STREET ALACHUA FL 32615 PH. 386-462-1340

October 17, 2006

Job: Rosenboom Construction

Re: McAlhany Residence

Dear Sir:

The windload analysis has been reviewed as to the anchorages of the roof dormers. Install Simpson H6 @ 32" o.c. anchoring the walls to the roof trusses. Install Simpson SP2 @ 32" o.c. for the top plate to walls. Install Simpson H10 for the trusses to double top plates. See the original windload analysis for the nail size and pattern.

All 36" openings will have 1 jack and 2 full length spf studs on each side.

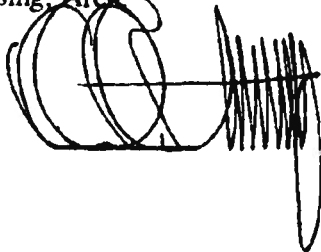
All openings from 36" to 78" will have 2 jack and 2 full length spf studs.

All headers will be 2 ply 2 x 12 syp #2 or better.

The garage door header will be pre-engineered. Install a 4 ply spf studs under each end of the garage door header. Install Simpson SPH4 and ST24 for each jack to header connection.

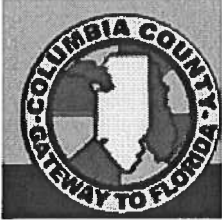
If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

Paul R. Stresing, Arch  
AR0013985



ATTN: JOR

From: The Columbia County Building & Zoning Department  
Plan Review  
135 NE Hernando Av.  
P.O. Box 1529  
Lake City Florida 32056-1529



Phone Number 386-758-1163  
Fax Number 386-754-7088

**FAX TRANSMITTAL**

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To: Scott Rosenboom

From: Joe Haltiwanger

Date Sent: 10/11/06

CC: Building permits applications **0610-37 and 38**

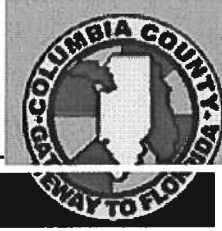
Number of Pages: *Eight pages* including the cover page

Fax: (386) 454-2666

*Message:* Building department review of applications for building permits.

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**To the review of the party to whom it is addressed. It may contain proprietary and/or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you have received this transmission in error, please immediately telephone the sender above to arrange for its return.**



From: The Columbia County Building & Zoning Department  
Plan Review  
135 NE Hernando Av.  
P.O. Box 1529  
Lake City Florida 32056-1529

Reference to a building permit application Number: **0610-38**  
Scott Rosenboom Contractor Owners Michael McAlhany Property ID 16-7s-17-10006-212

On the date of October 12, 2006 application 0610-38 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

**Please include application number 0610-37 and when making reference to this application.**

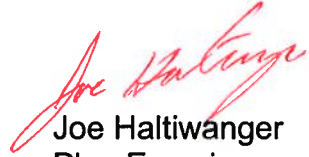
***This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.***

**Structural load bearing walls or supporting headers**

1. On the exterior load bearing walls of the detached garage which have opening for door, windows or headers which support roof trusses, show the headers size for each span and the required number of supporting

members (jack and king studs) or columns needed to support these headers.

Thank You:



Joe Haltiwanger  
Plan Examiner  
Columbia County Building  
Department

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**RESIDENTIAL WIND DESIGN & ANALYSIS**

FBC SECTION 1609 \ *NO COPIES ARE TO BE PERMITTED*

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**PREPARED FOR:**

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ROSENBOOM CONSTRUCTION \ THE McALHANY GARAGE

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**PREPARED BY:**

**MARTY R. ESKRIDGE  
14952 MAIN ST  
ALACHUA FL 32615  
386-462-1340 / 352-375-6329**

*OFFICE COPY*

# SUMMARY

## OF WIND DESIGN & ANALYSIS

**Trusses:** Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

**Roof sheathing:** Type OSB Size 7/16 Fastener type Nails Size 8d/131 GA  
 Interior zone spacing: Interior 8 in. Periphery 4 in.  
 Edge and end zone spacing: Interior 8 in. Periphery 4 in.

**Top double pl:** Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

**Studs:** Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4  
 Interior stud spacing 16 in. Composite (yes or no) Y  
 End stud spacing 16 in. Composite (yes or no) Y

**Shearwall siding:** Type OSB Thickness 7/16 in.  
54' - Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.  
39' - Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

**Allowable unit shear on shearwalls:** 326 pounds per linear foot

**Wall tension transferred by:** Siding nails 8d/131 @ 4 O.C. edges

**Foundation anchor bolts:** Concrete strength 3000 psi  
 Size 1/2 in. Shape L Washer 2" Embedment 7 in.  
 Location of first anchor bolt from corner 8 in.

**Anchor Bolts @ 48" O.C.** Model A307 Loc. from corner 8 in.

**Type of foundation:** 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 96 in.  
 Monolithic footing: Depth 18 in. Bottom width 12 in.

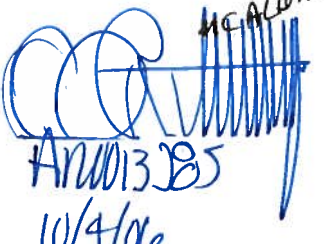
**Footing:** Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars  
 Interior Footings: 16" W X 10" D

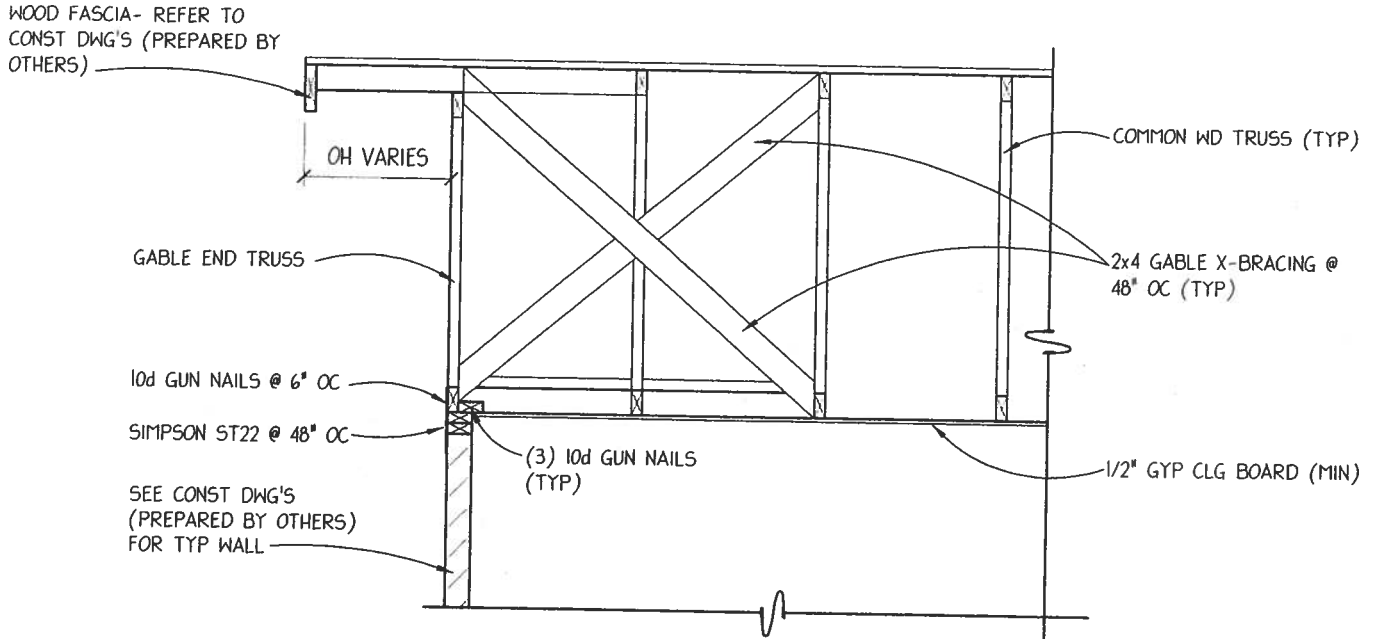
**Porch Columns:** 6x6x8' spr #4 PT @ 192" OC

**Porch Column Fasteners:** 5/16" - C666 / C666 OR EQUAL

**NOTE:**

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

  
 H. CALVERT  
 APR 13 2005  
 10/4/06



THIS WIND ANALYSIS ASSUMES IDEAL SOIL CONDITIONS W/MIN 2500 PSI SOIL BRG CAPACITY & 95% DRY PROCTOR DENSITIES

# GABLE END DETAIL

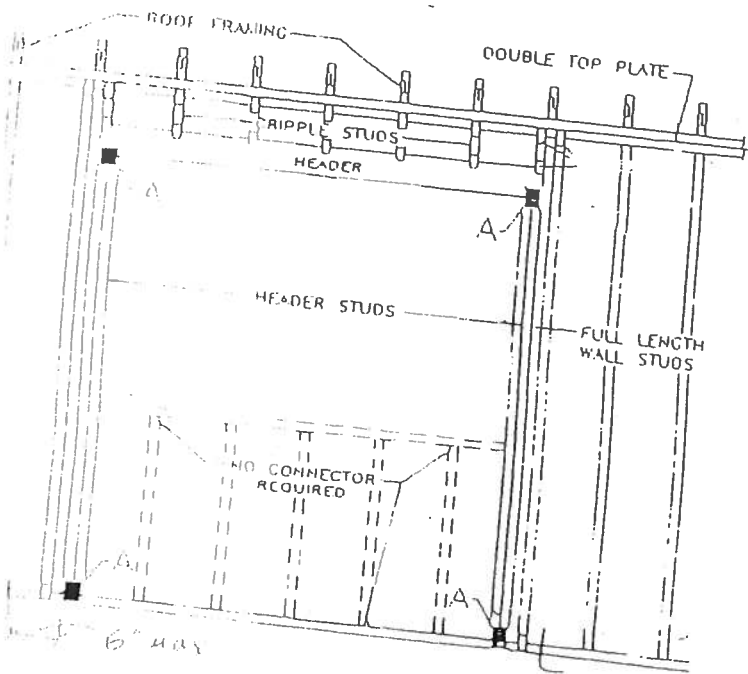
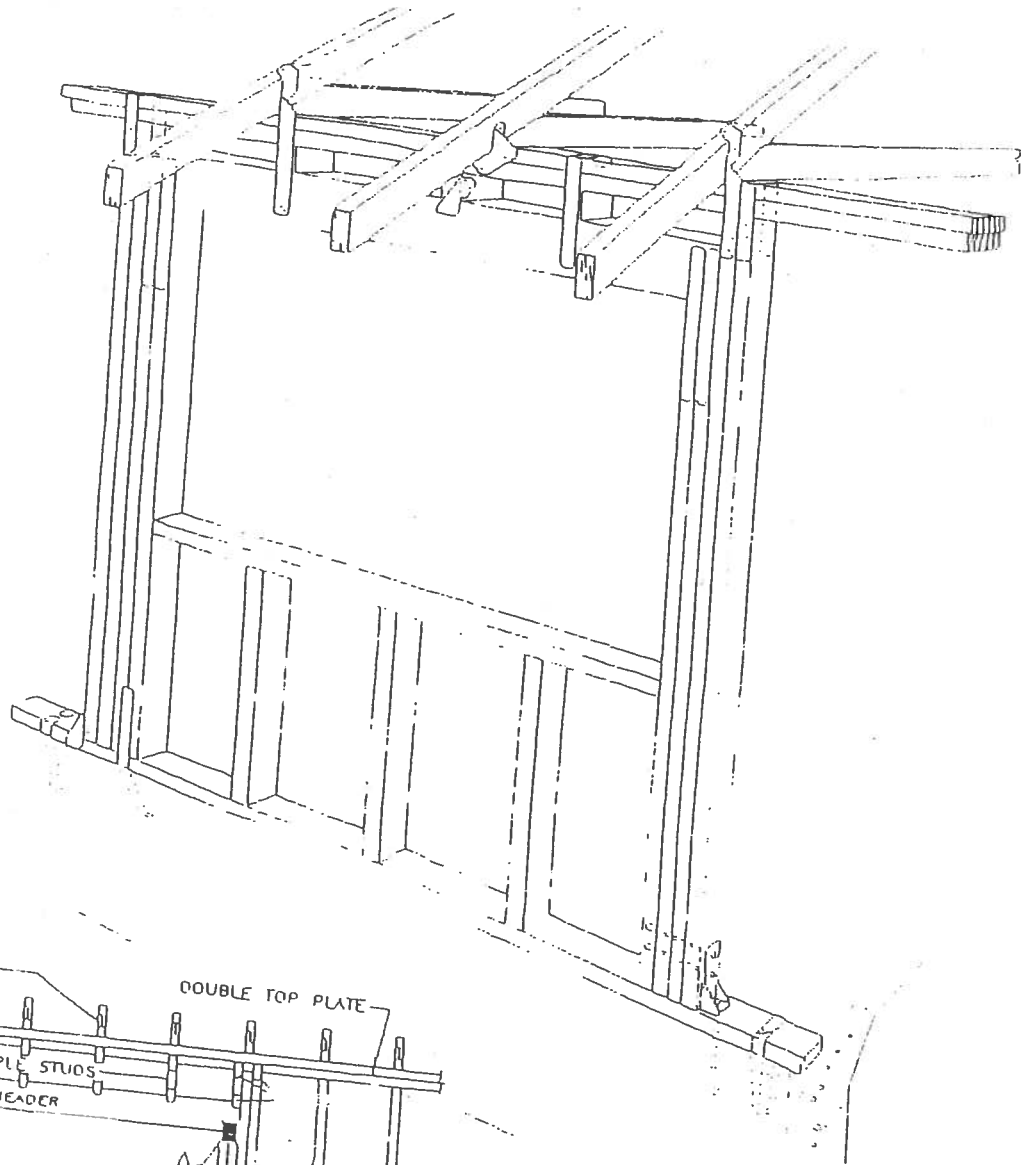
SCALE: 1/2" = 1'-0"

**NOTE:**  
 INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WITH THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN ERECTION & BRACING SHEET.

*[Handwritten signature]*  
 AR2003285  
 10/4/06

Q:\SUPPORT\DETAILS\WINDLOADS\02-GABLE END.DWG 06/22/06 15:27

TYPICAL GABLE END DETAILS	<b>PAUL STRESING ASSOCIATES, INC.</b>		DRAWN BY: C.A.D.	SHEET NO.
	14077 MAIN STREET ALACHUA, FLORIDA 32010 E-MAIL: paul@psa-usa.com TELEPHONE: (386) 962-9407 FAX: (386) 962-9408 REGISTRATION NO. AN000886 - CA NO. AA000337		DATE: 2006	OF
			PROJECT FILE: WINDLOADS	



Total each truss uplift on the header divide by 2 for header anchorage

### TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate.  
Use proper bolt anchors.  
Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.  
Strap ridge beam at each end with minimum uplift resistance of 1000#.  
It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
<b>BEAM SEATS</b>	LSTA18*	1200	LTT19*	1250
<b>POSTS</b> (max 17' spacing)	2-LSTA18	2400	ABU44	2300

\*or per truss engineering  
Use proper bolt anchors  
All beams to be sheathed or strapped to Double Top Plate when applicable.

**CRIPPLES** | Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Wall sheathing nailing Adequate exterior walls top w/8d nails @ 3" O.C., as long as sheathing covers top plate, otherwise use SP2 @ 32" O.C. in addition to sheathing nailing.
Use SP2 top and SP1 bottom each stud for all interior load bearing walls and anchor bolts @ 32" O.C.
Interior anchor bolts to be 1/2" x 8" A307 or 1/2" x 6" wedge anchor or equivalent.

\*\* Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE: For nailing into SPF members, multiply table values by .86

# ASCE 7-02

10/3/06

## Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Gabled	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	16.67	ft
Mean Roof Height (Ht)	13.28	ft
Width Perp. to Wind (B)	30.00	ft
Width Parallel to Wind (L)	46.00	ft
Damping Ratio (beta)	0.01	

*Red values should be changed only through "Main Menu"*

Calculated Parameters		
Importance Factor	1	
<i>Hurricane Prone Region (V&gt;100 mph)</i>		
<b>Table C6-4 Values</b>		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.44
Flexible Structure	No

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
lzm	$Cc * (33/z)^{0.167}$	0.3048	
Lzm	$l*(zm/33)^{Epsilon}$	309.99	ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.9197	
Gust2	$0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))$	0.8776	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	$V*(5280/3600)$	161.33	ft/s
Vzm	$bm*(zm/33)^{Am}*Vhref$	70.89	ft/s
NF1	$NatFreq*Lzm/Vzm$	4.37	Hz
Rn	$(7.47*Nf1)/(1+10.302*Nf1)^{1.667}$	0.0552	
Nh	$4.6*NatFreq*Ht/Vzm$	0.86	
Nb	$4.6*NatFreq*B/Vzm$	1.95	
Nd	$15.4*NatFreq*Depth/Vzm$	9.99	
Rh	$1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))$	0.6073	
Rb	$1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))$	0.3844	
Rd	$1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))$	0.0951	
RR	$((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^{0.5}$	0.8604	
gg	$+(2*LN(3600*n1))^{0.5}+0.577/(2*LN(3600*n1))^{0.5}$	4.19	
Gust3	$0.925*((1+1.7*lzm*(3.4^2*Q^2+GG^2*RR^2)^{0.5})/(1+1.7*3.4*lzm))$	1.16	

Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.88	Gust Factor (G)	0.88

# ASCE 7-02

10/3/06

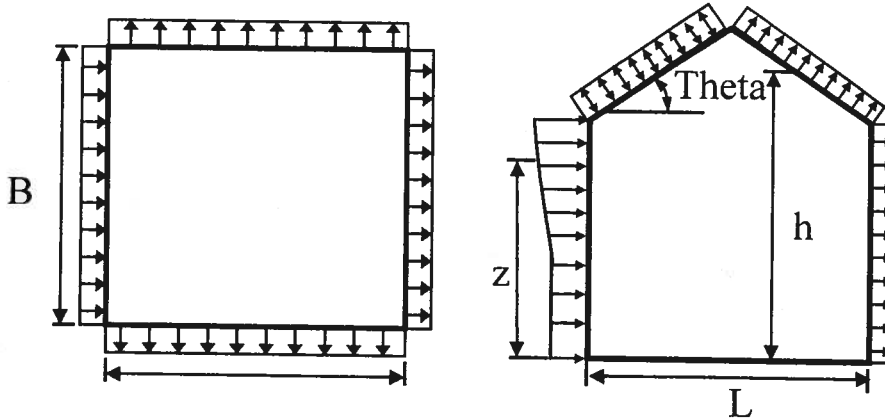
## Wind Load Design per ASCE 7-02

### 6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft <sup>2</sup>	Pressure (lb/ft <sup>2</sup> )	
					Windward Wall*	
					+GCpi	-GCpi
16.67	0.70	1.00	1.00	21.70	12.03	18.44
15	0.70	1.00	1.00	21.70	12.03	18.44

**Figure 6-3 - External Pressure Coefficients, Cp**

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (15/z_g)^{2/\alpha}$	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d$	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 30 ft wall)	-0.39	-9.35	-2.94
Leeward Walls (Wind Dir Parallel to 46 ft wall)	-0.50	-11.02	-4.61
Side Walls	-0.70	-14.14	-7.73
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.21	-6.49	-0.08
Windward - Max Positive	0.28	1.24	7.65
Leeward Normal to Ridge	-0.60	-12.58	-6.17
Overhang Top	-0.21	-3.29	-3.29
Overhang Bottom	0.80	0.70	0.70
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 6.64 ft	-0.90	-17.27	-10.86
Dist from Windward Edge: 6.64 ft to 13.28 ft	-0.90	-17.27	-10.86
Dist from Windward Edge: 13.28 ft to 26.56 ft	-0.50	-11.02	-4.61

# ASCE 7-02

10/3/06

## Wind Load Design per ASCE 7-02

Dist from Windward Edge: > 26.56 ft	-0.30	-7.89	-1.48
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\* Horizontal distance from windward edge

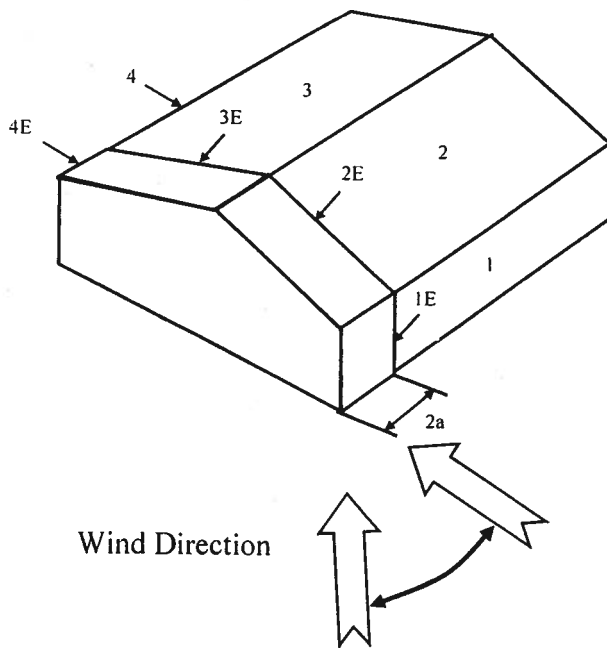
### Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

\* p = qh \* (GCpf - GCpi)



# ASCE 7-02

10/3/06

## Wind Load Design per ASCE 7-02

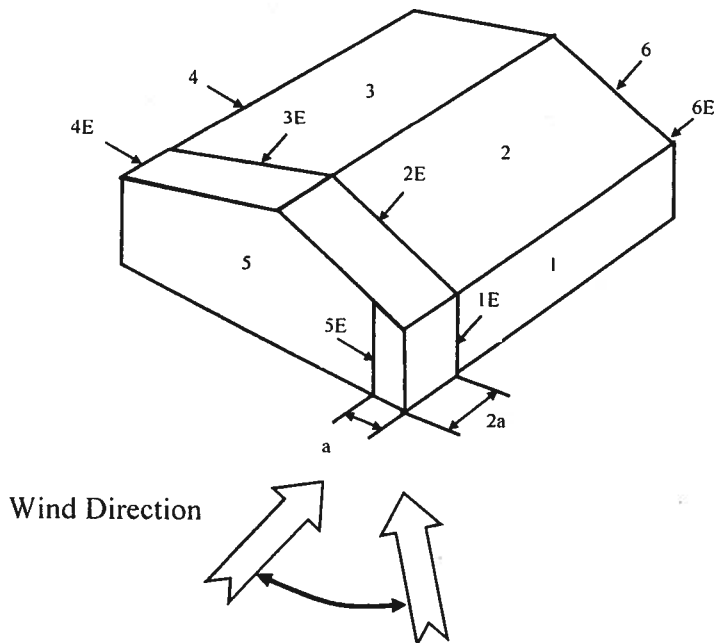
### Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (15/z_g)^{2/\alpha} &= & 0.57 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 17.80
 \end{aligned}$$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

\*  $p = q_h \cdot (GC_{pf} - GC_{pi})$



### Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft



**ASCE 7-02**

10/3/06

**Wind Load Design per ASCE 7-02**

Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
<b>Enclosed Buildings</b>	<b>0.18</b>	<b>-0.18</b>

**Table 6-8 External Pressure Coefficients for Arched Roofs,  $C_p$** 

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	$C_p$		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	$C_p$	0.13	-1	-0.5
	P (+GCpi) - psf	-1.25	-18.83	-11.02
	P (-GCpi) - psf	5.16	-12.42	-4.61
Roof Springing from Ground	$C_p$	0.42	-1	-0.5
	P (+GCpi) - psf	3.36	-18.83	-11.02
	P (-GCpi) - psf	3.36	-18.83	-11.02

**Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings,  $C_f$** 

Variable	Description	Value	
L	Roof dimension normal to wind direction	46.00	ft
B	Roof dimension parallel to wind direction	30.00	ft
L/B	Ratio of L to B	1.533	
Theta	Slope of Roof	26.6	Deg
$C_f$	Force Coefficient	1.11	
X	Distance to center of pressure from windward edge	0.39	ft



# MiTek®

RE: RSNMCA -

**MiTek Industries, Inc.**

1801 Massaro Blvd.

Tampa, FL 33619

Phone: 813/675-1200

Fax: 813/675-1148

**Site Information:**

Project Customer:      Project Name:

Lot/Block: 12

Subdivision: RIVER RISE

Address:

City: HIGH SPRINGS

State: FLORIDA

**Name Address and License # of Structural Engineer of Record, if there is one, for the building.**

Name:

License #:

Address:

City:

State:

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.3

Wind Code: ASCE 7/02    Wind Speed: 110 mph

Design Method: User defined

Roof Load: 40 psf, nonconcurrent BCLL=10 psf

Floor Load: N/A psf

This package includes 2 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	T2384722	RSNMCA	F1	10/6/06
2	T2384723	RSNMCA	FET	10/6/06

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie

My license renewal date for the state of is February 28, 2007.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Guo-Jie Zhang, FL Lic #47744  
MiTek Industries, Inc.  
1801 Massaro Blvd  
Tampa FL 33619  
FL Cert.#6634

October 6, 2006

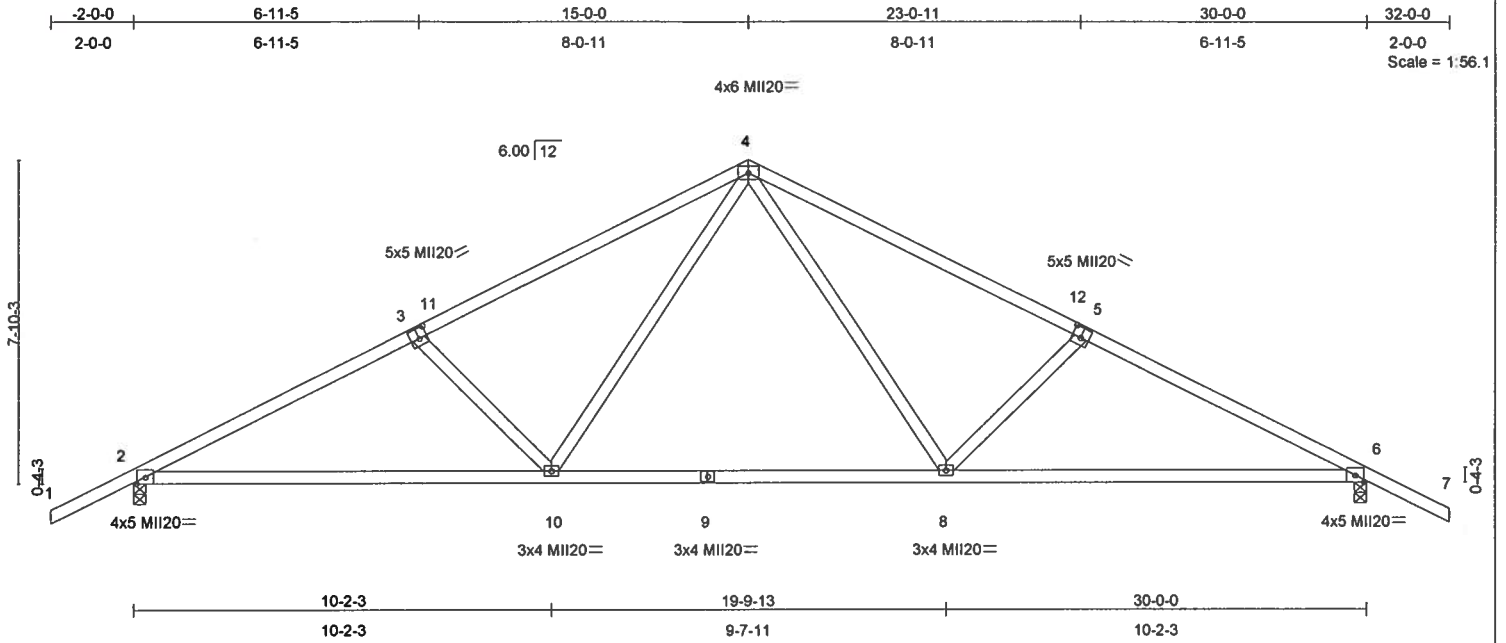


Plate Offsets (X,Y): [3:0-2-8,0-3-0], [5:0-2-8,0-3-0]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.41	Vert(LL) -0.20 6-8 >999 240	MII20	249/190
TCDL 10.0	Lumber Increase 1.25	BC 0.62	Vert(TL) -0.55 6-8 >654 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.20	Horz(TL) 0.08 6 n/a n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)			Weight: 141 lb

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2D	TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins.
BOT CHORD 2 X 4 SYP No.2D	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

**REACTIONS** (lb/size) 2=1317/0-3-8, 6=1317/0-3-8  
 Max Horz 2=99(load case 4)  
 Max Uplift 2=-387(load case 5), 6=-387(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/53, 2-3=-2104/428, 3-11=-1800/354, 4-11=-1784/390, 4-12=-1784/390, 5-12=-1800/354, 5-6=-2104/428, 6-7=0/53  
 BOT CHORD 2-10=-311/1800, 9-10=-77/1190, 8-9=-77/1190, 6-8=-247/1800  
 WEBS 3-10=-443/259, 4-10=-103/625, 4-8=-103/625, 5-8=-443/259

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
  - \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2 and 387 lb uplift at joint 6.

**LOAD CASE(S)** Standard

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 Tampa FL 33619  
 FL Cert.#6634

October 6, 2006

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



14515 N. Outer Forty, Suite #300  
 Chesterfield, MO 63017

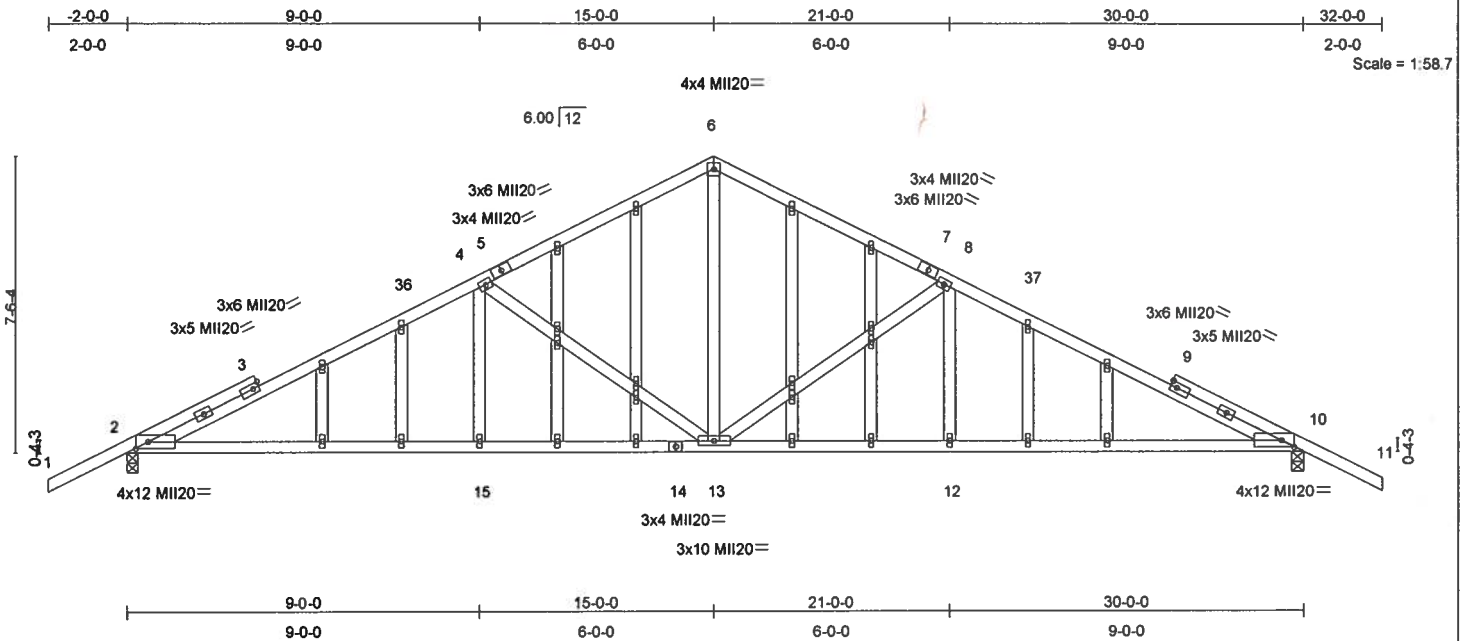


Plate Offsets (X,Y): [2:0-3-12,0-2-0], [10:0-3-12,0-2-0]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.82	Vert(LL) 0.17 2-15 >999 240	MI20	249/190
TCDL 10.0	Lumber Increase 1.25	BC 0.64	Vert(TL) -0.50 2-15 >714 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.74	Horz(TL) 0.09 10 n/a n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)			Weight: 200 lb

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2D	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2D	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

**REACTIONS** (lb/size) 2=1317/0-3-8, 10=1317/0-3-8  
 Max Horz 2=95(load case 4)  
 Max Uplift 2=-388(load case 5), 10=-388(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/53, 2-3=-2156/368, 3-36=-2078/377, 4-36=-1955/396, 4-5=-1464/303, 5-6=-1450/330, 6-7=-1450/330, 7-8=-1464/303, 8-37=-1955/396, 9-37=-2078/377, 9-10=-2156/368, 10-11=0/53  
 BOT CHORD 2-15=-266/1859, 14-15=-266/1859, 13-14=-266/1859, 12-13=-219/1859, 10-12=-219/1859  
 WEBS 4-15=0/346, 4-13=-787/247, 6-13=-190/971, 8-13=-787/248, 8-12=0/346

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
  - \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - All plates are 1.5x4 MII20 unless otherwise indicated.
  - This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
  - Gable studs spaced at 2-0-0 oc.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 2 and 388 lb uplift at joint 10.

**LOAD CASE(S)** Standard

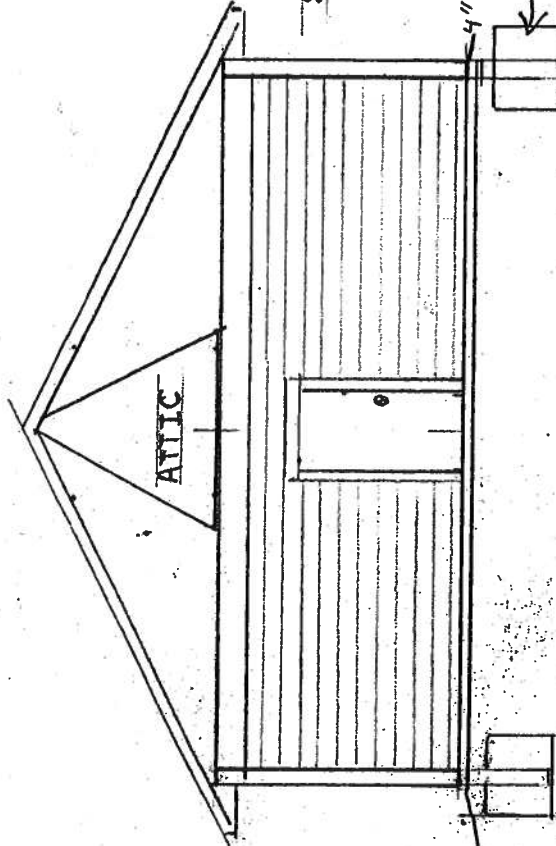
Guo-Jie Zhang, FL Lic #47744  
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October 6, 2006

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8x8 PT POST

4" STEP DOWN

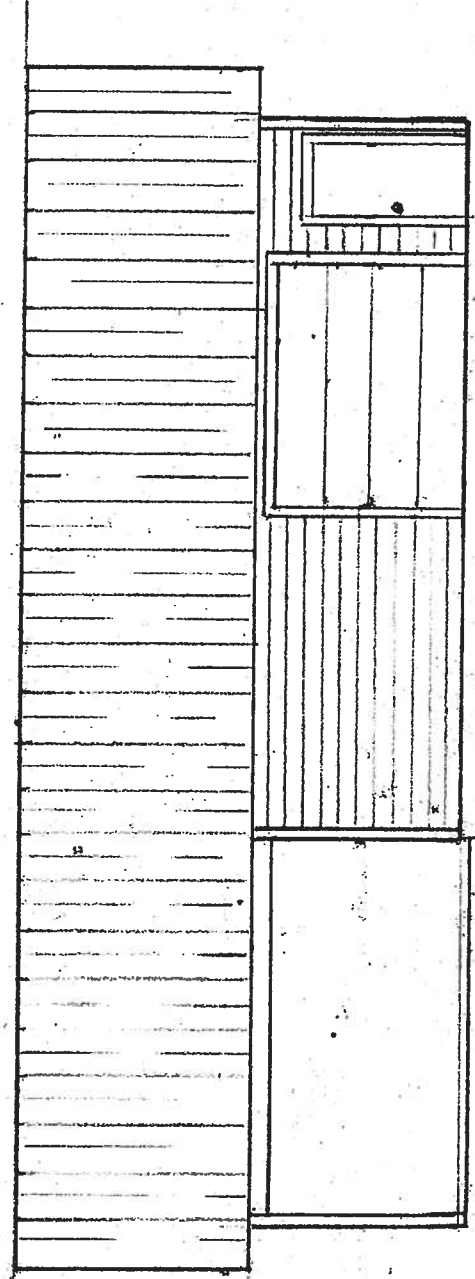
CONC AS PER ENG

26 GAGE GALVALUME

26 GAGE GALVALUME OVER  
K" PLYWOOD OVER 2x4 ENG

BRUSS

VENED ALUMINUM  
SOFFIT



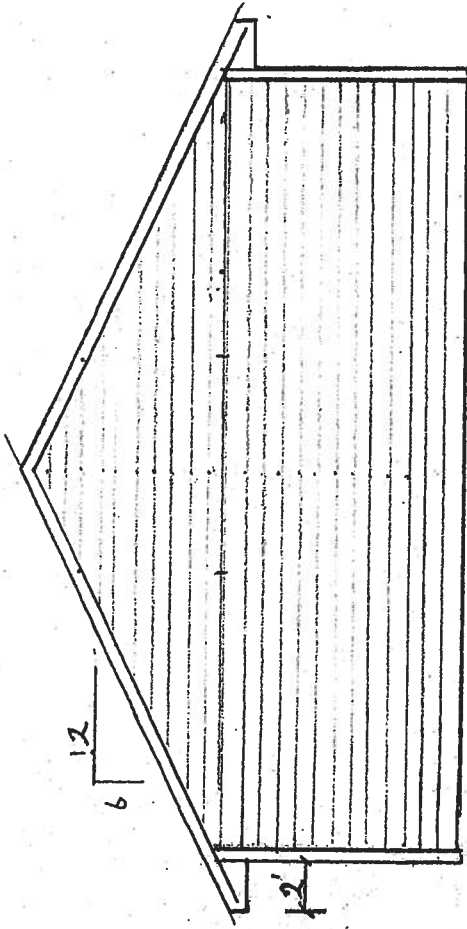
2x4 SPRUCE STUD  
3/4 OSB, TYVEC, CEM PLANK

3000 PSI CONC OVER 6 MILL  
W BRASS 75% POISON &  
CLEAN FILL

18"

1000\* PSF MIN  
BEARING CAP

4-1/2"



RIGHT ELEVATION

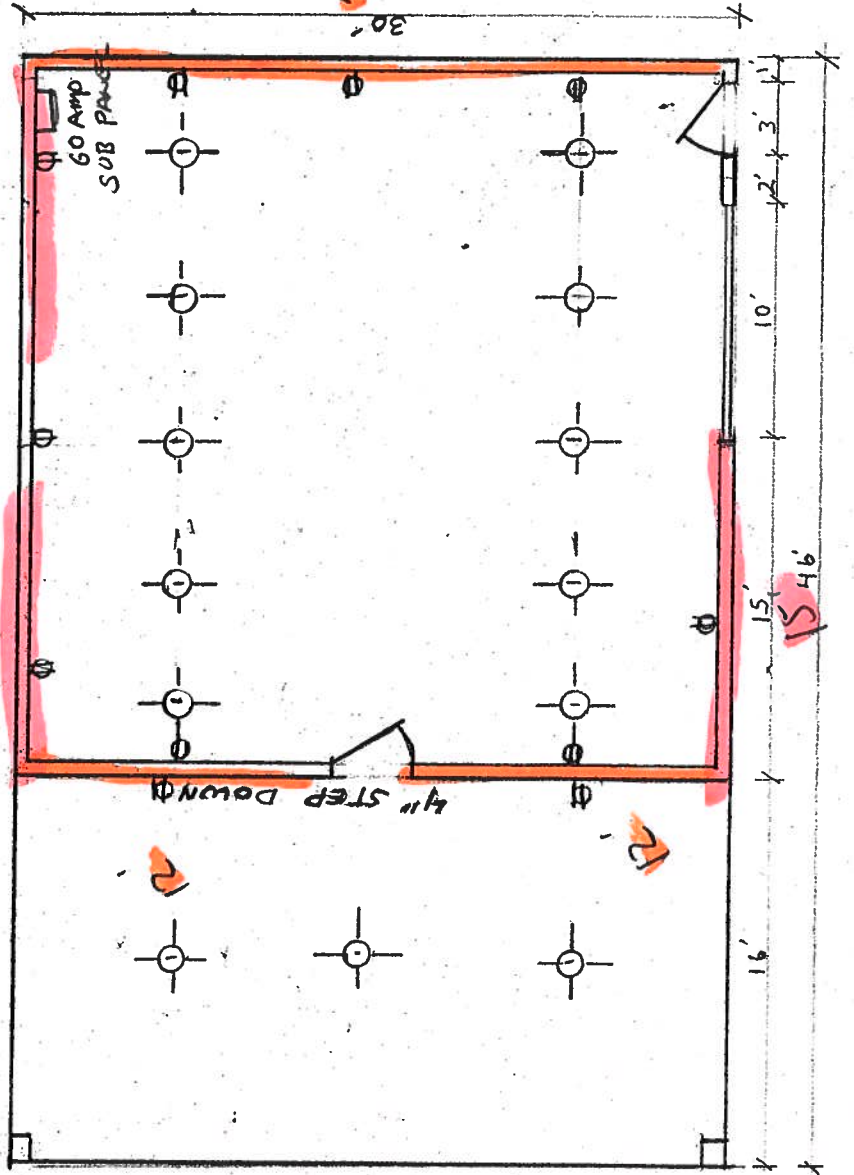
115-54'

1016-39'

McALHANY GARAGE  
ROSENBOOM CON.

12'

12'



12'