

Julius Lee Engineering

RE: 318492 - RICHARD KEEN - LAMBERT RES.

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: RICHARD KEEN Project Name: 318492 Model: LAMBERT RES.
Lot/Block: Subdivision:
Address: 427 SE ORMOND WITT RD
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JAMES H. JOHNSTON License #: CRC1328128
Address: 650 SW MAIN BLVD
City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 26 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

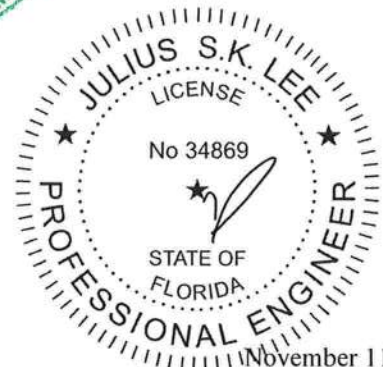
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4152986	PB01	11/11/09	18	I4153003	T11	11/11/09
2	I4152987	PB01G	11/11/09	19	I4153004	T11G	11/11/09
3	I4152988	PB02	11/11/09	20	I4153005	T12	11/11/09
4	I4152989	PB02G	11/11/09	21	I4153006	T13	11/11/09
5	I4152990	T01G	11/11/09	22	I4153007	T14	11/11/09
6	I4152991	T02	11/11/09	23	I4153008	T15	11/11/09
7	I4152992	T02G	11/11/09	24	I4153009	T16	11/11/09
8	I4152993	T03	11/11/09	25	I4153010	T17	11/11/09
9	I4152994	T04	11/11/09	26	I4153011	T17G	11/11/09
10	I4152995	T04G	11/11/09				
11	I4152996	T05	11/11/09				
12	I4152997	T06	11/11/09				
13	I4152998	T07	11/11/09				
14	I4152999	T08	11/11/09				
15	I4153000	T08G	11/11/09				
16	I4153001	T09	11/11/09				
17	I4153002	T10	11/11/09				

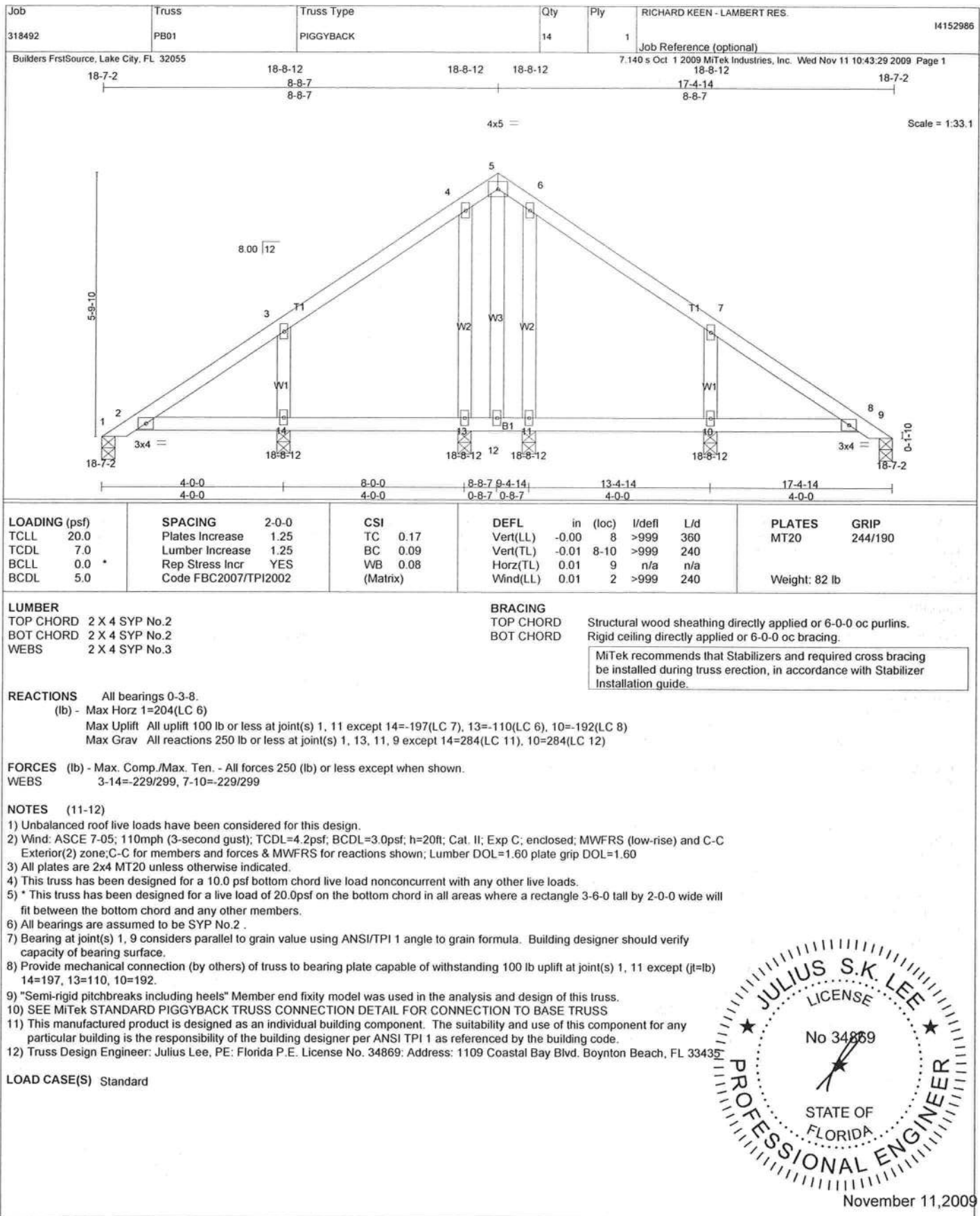
The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

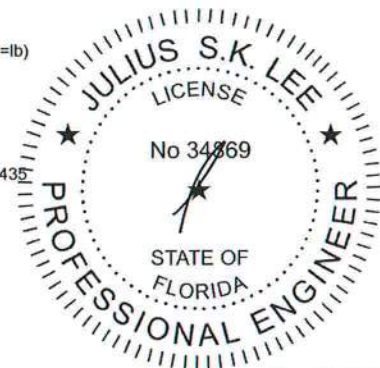
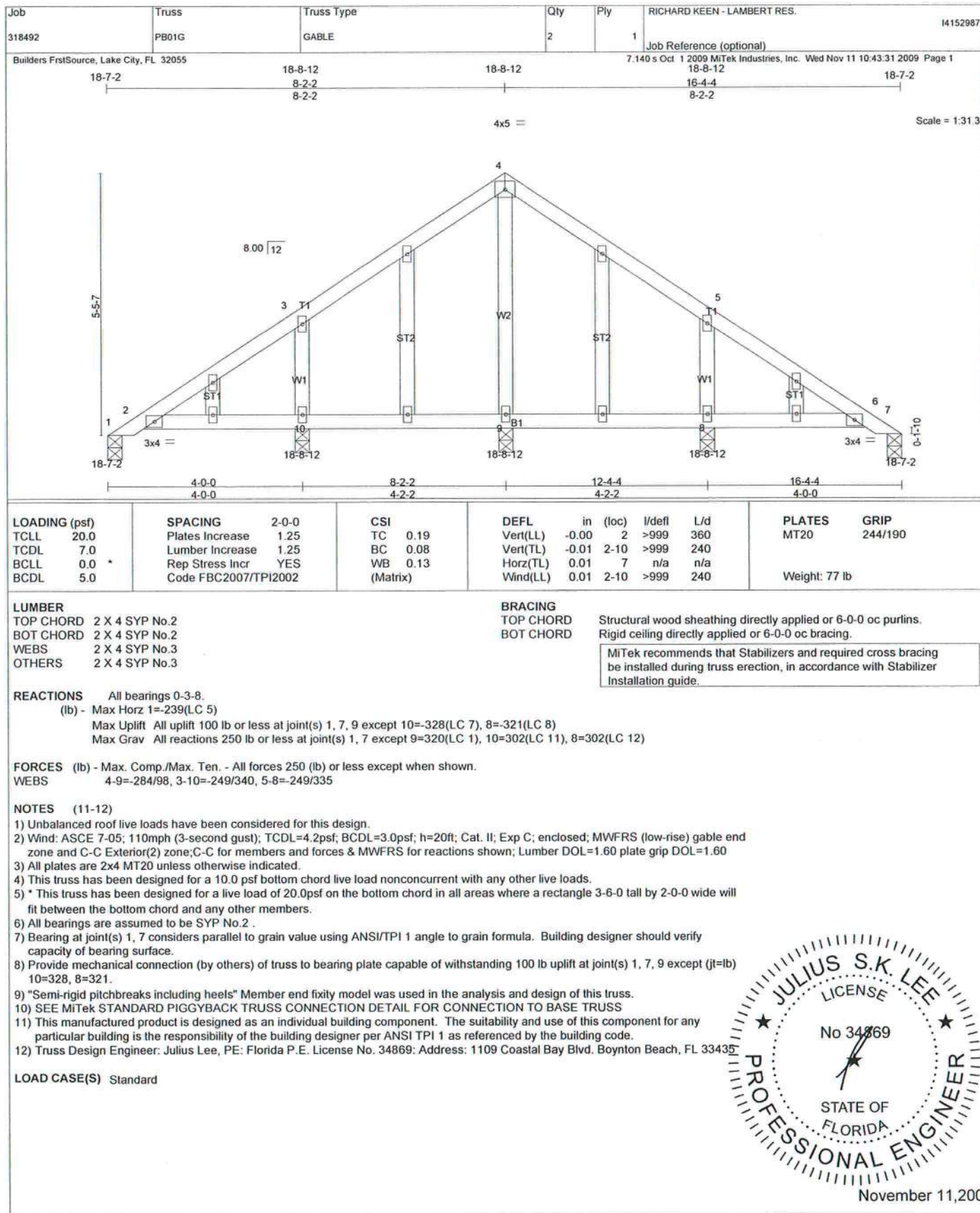
Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.







November 11, 2009

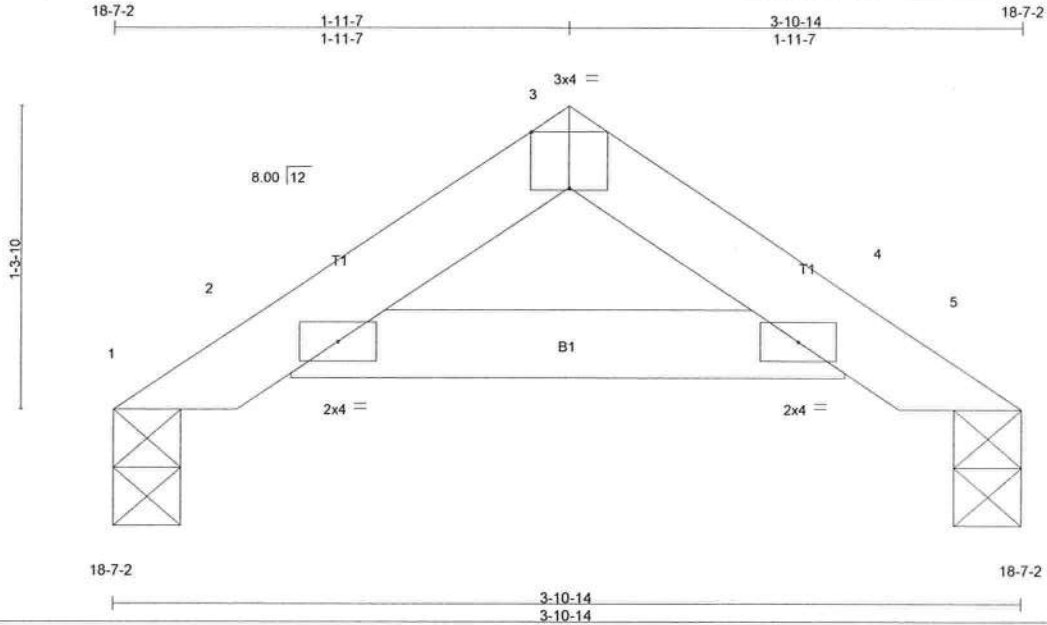
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	PB02	PIGGYBACK	9	1	
					Job Reference (optional)

Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:31 2009 Page 1



Scale = 1:9.3

Plate Offsets (X,Y): [3:0-2:0,Edge]

LOADING (psf)
TCLL 20.0
TCDL 7.0
BCLL 0.0
BCDL 5.0

SPACING 2-0-0
Plates Increase 1.25
Lumber Increase 1.25
Rep Stress Incr YES
Code FBC2007/TPI2002

CSI
TC 0.11
BC 0.06
WB 0.00
(Matrix)

DEFL in (loc) l/defl L/d
Vert(LL) -0.00 2-4 >999 360
Vert(TL) -0.01 2-4 >999 240
Horz(TL) -0.01 5 n/a n/a
Wind(LL) 0.01 2-4 >999 240

PLATES GRIP
MT20 244/190
Weight: 11 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-10-14 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

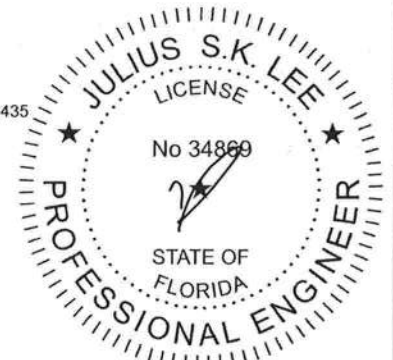
REACTIONS (lb/size) 1=117/0-3-8, 5=117/0-3-8
Max Horz 1=43(LC 6)
Max Uplift 1=38(LC 7), 5=38(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 11, 2009



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318492	Truss PB02G	Truss Type PIGGYBACK	Qty 1	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	14152989
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:32 2009 Page 1			

Plate Offsets (X,Y): [3:0-2:0,Edge]							
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.09	Vert(LL)	-0.00 2	>999	360	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(TL)	-0.01 2	>999	240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.01 5	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL)	0.00 2	>999	240	Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=91/0-3-8, 5=91/0-3-8

Max Horz 1=-42(LC 5)

Max Uplift 1=-53(LC 7), 5=-53(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2 .
- 6) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

November 11, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

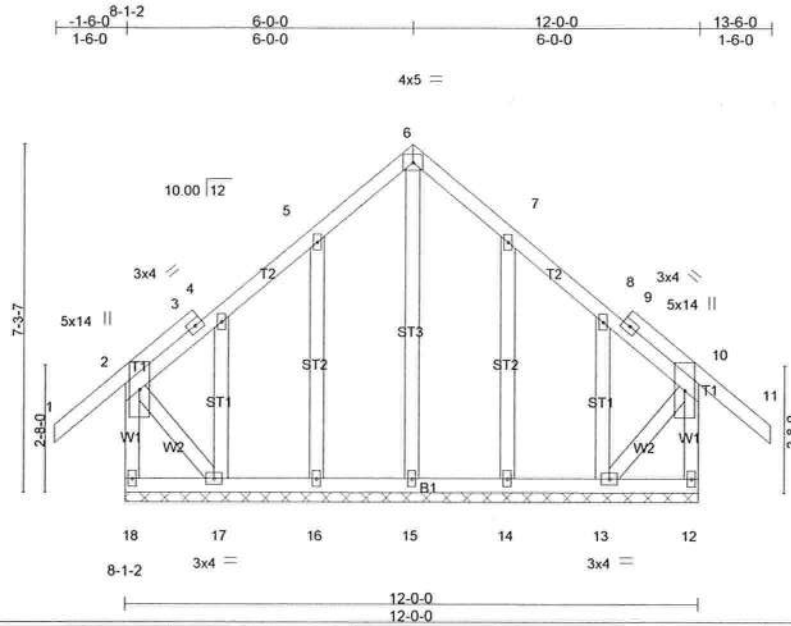
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Boynton, FL 33435

Job 318492	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	RICHARD KEEN - LAMBERT RES.	14152990
Job Reference (optional)						

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.31	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(LL) -0.02 11 n/r 120		
BCLL 0.0	Lumber Increase 1.25	WB 0.14	Vert(TL) -0.03 11 n/r 90		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 12 n/a n/a		
	Code FBC2007/TPI2002			Weight: 101 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2 *Except*
W2: 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 12-0-0.
(lb) - Max Horz 18=364(LC 6), 12=-221(LC 6), 16=-232(LC 7), 17=-381(LC 7), 14=-232(LC 8), 13=-366(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) except 18=-274(LC 5), 12=-221(LC 6), 16=-232(LC 7), 17=-381(LC 7), 14=-232(LC 8), 13=-366(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 15, 16, 17, 14, 13 except 18=274(LC 11), 12=274(LC 12)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

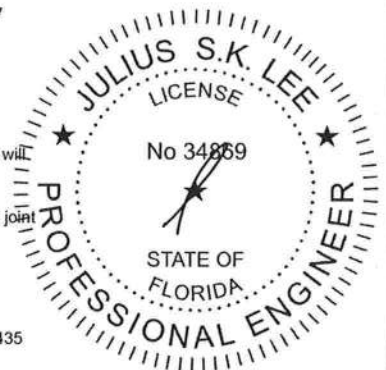
TOP CHORD 2-18=-267/273, 10-12=-267/222
BOT CHORD 17-18=-321/339
WEBS 2-17=-176/338, 10-13=-125/317

NOTES (14-15)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 18, 221 lb uplift at joint 12, 232 lb uplift at joint 16, 381 lb uplift at joint 17, 232 lb uplift at joint 14 and 366 lb uplift at joint 13.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.	I4152990
318492	T01G	GABLE	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

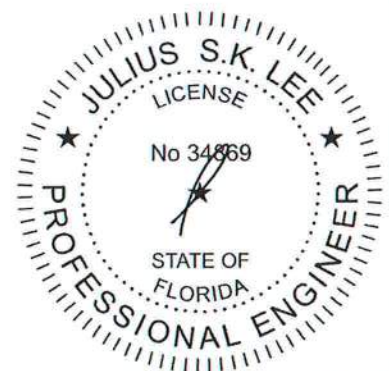
7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:33 2009 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-87(F=-33), 2-6=-87(F=-33), 6-10=-87(F=-33), 10-11=-87(F=-33), 12-18=-10



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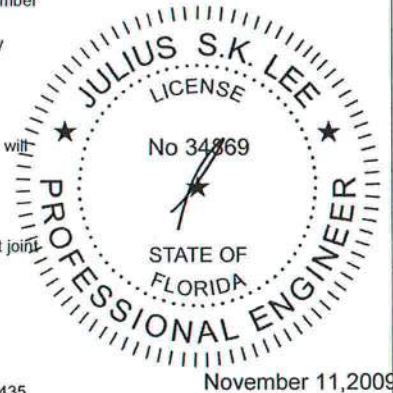
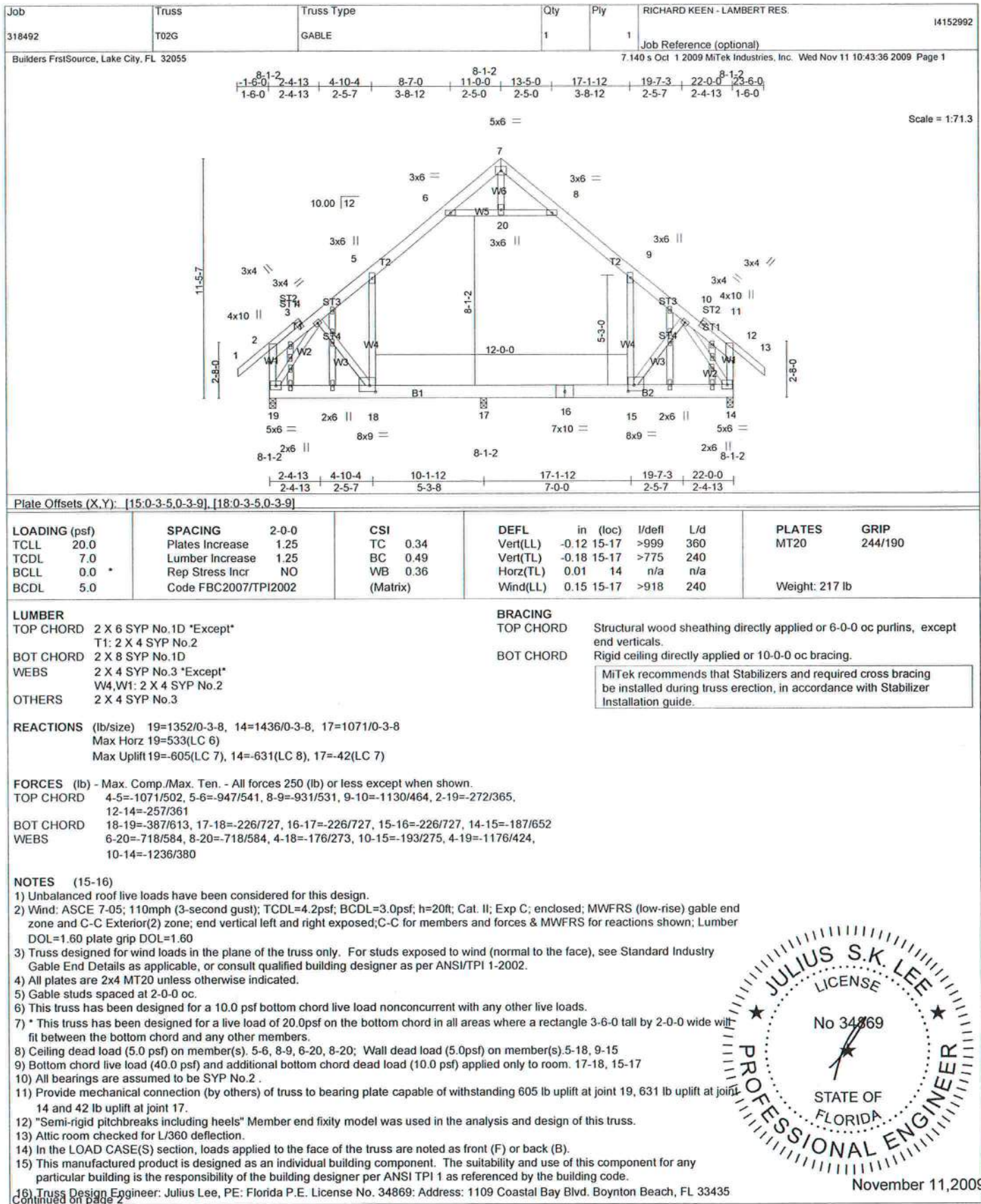


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Boynton, FL 33435

Job 318492	Truss T02G	Truss Type GABLE	Qty 1	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	4152992
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Builders FrstSource, Lake City, FL 32055

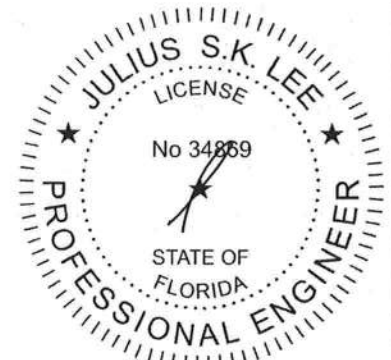
7.140 s Oct 1 2009 Mitek Industries, Inc. Wed Nov 11 10:43:37 2009 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 18-19=-10, 15-18=-110, 14-15=-10, 1-2=-87(F=-33), 2-5=-87(F=-33), 5-6=-97(F=-33), 6-7=-87(F=-33), 7-8=-87(F=-33), 8-9=-97(F=-33), 9-12=-87(F=-33),
12-13=-87(F=-33), 6-8=-10
Drag: 5-18=-10, 9-15=-10



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Safety Information available from Truss Plate Institute, 583 D'Oncofria Drive, Madison, WI 53719.

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Boynton, FL 33435

Job 318492	Truss T03	Truss Type ATTIC	Qty 9	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	14152993
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:38 2009 Page 1			

Scale = 1:70.7

Plate Offsets (X,Y): [4:0-4-5:0-0-2], [8:0-4-5:0-0-2], [12:0-3-8:0-4-0], [14:0-3-8:0-4-0]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.66	Vert(LL)	-0.31 12-14	>836	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	-0.51 12-14	>514	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.93	Horz(TL)	0.01 11	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.12 12-14	>999	240		
								Weight: 199 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP No.1D

WEBS 2 X 4 SYP No.3 *Except*

W4,W1: 2 X 4 SYP No.2

REACTIONS (lb/size) 15=1524/0-3-8, 11=1429/0-3-8

Max Horz 15=470(LC 6)

Max Uplift 15=-127(LC 7), 11=-42(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-1582/292, 4-5=-1016/388, 7-8=-1015/388, 8-9=-1588/296, 2-15=-129/332

BOT CHORD 14-15=-221/850, 13-14=-97/994, 12-13=-97/994, 11-12=-159/865

WEBS 5-16=-1176/435, 7-16=-1176/435, 4-14=0/843, 8-12=0/844, 3-14=-113/289, 9-12=-121/284, 3-15=-1786/62, 9-11=-1807/248

NOTES (11-12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-16, 7-16; Wall dead load (5.0psf) on member(s).4-14, 8-12
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 7) All bearings are assumed to be SYP No.2 .
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 15 and 42 lb uplift at joint 11.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Attic room checked for L/360 deflection.
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-3-4 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

November 11, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318492	Truss T04	Truss Type COMMON	Qty 2	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	14152994
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:39 2009 Page 1			

Scale = 1:71.2

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002	CSI TC 0.34 BC 0.32 WB 0.78 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.09 8-9 >999 360 Vert(TL) -0.16 8-9 >999 240 Horz(TL) 0.02 8 n/a n/a Wind(LL) 0.02 9-11 >999 240	PLATES MT20 GRIP 244/190 Weight: 167 lb
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LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 3-12, 5-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 12=988/0-3-8, 8=988/0-3-8
 Max Horz 12=-448(LC 5)
 Max Uplift 12=-298(LC 7), 8=-298(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-135/329, 3-4=-787/665, 4-5=-787/665, 5-6=-135/329, 2-12=-251/384, 6-8=-251/384

BOT CHORD 12-13=-307/542, 13-14=-307/542, 11-14=-307/542, 11-15=-137/443, 10-15=-137/443, 10-16=-137/443, 9-16=-137/443, 9-17=-190/542, 17-18=-190/542, 8-18=-190/542

WEBS 4-9=-309/336, 5-9=-95/367, 4-11=-309/336, 3-11=-95/367, 3-12=-830/281, 5-8=-830/281

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 12 and 298 lb uplift at joint 8.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

November 11, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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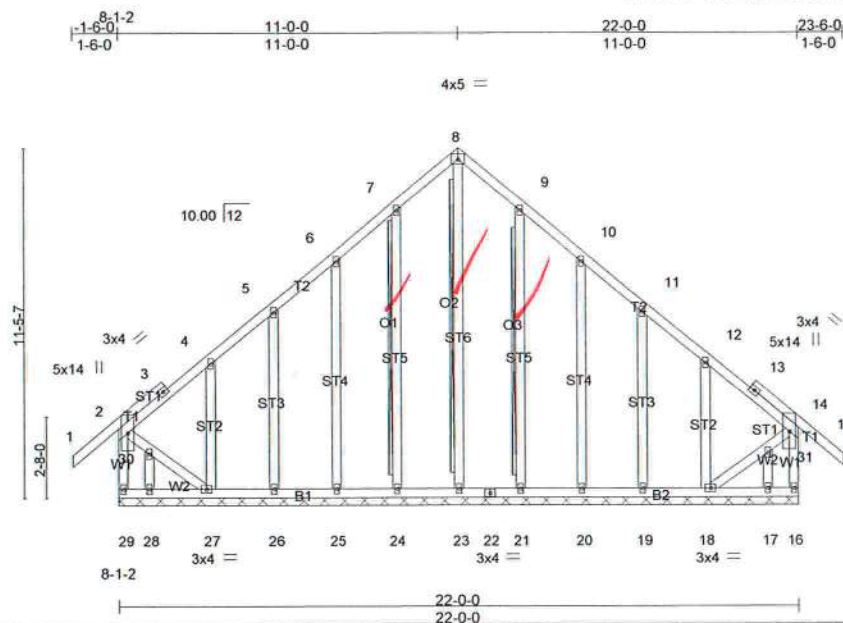
Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	T04G	GABLE	1	1	Job Reference (optional)

14152995

Builders FirstSource, Lake City, FL 32055

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Scale = 1:70.9

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.10	Vert(LL) -0.01 15 n/r 120		
BCLL 0.0	Lumber Increase 1.25	WB 0.20	Vert(TL) -0.02 14-15 n/r 90		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 18 n/a n/a		
	Code FBC2007/TPI2002			Weight: 203 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2 *Except*
 W2: 2 X 4 SYP No.3
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 Rigid ceiling directly applied or 6-0-0 oc bracing.
 T-Brace: 2 X 4 SYP No.3 - 8-23, 7-24, 9-21
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 22-0-0.
 (lb) - Max Horz 29=551(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) 28, 17 except 29=-356(LC 5),
 16=-250(LC 6), 24=-208(LC 7), 25=-226(LC 7), 26=-222(LC 7), 27=-485(LC 7),
 21=-206(LC 8), 20=-227(LC 8), 19=-222(LC 8), 18=-465(LC 8)
 Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 27, 28, 21, 20, 19, 18,
 17 except 29=367(LC 6), 16=304(LC 1), 23=287(LC 8)

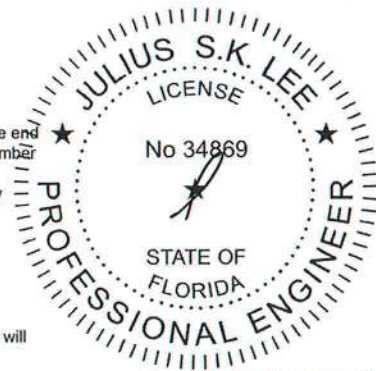
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-303/229, 3-4=-317/226, 6-7=-63/265, 7-8=-68/347, 8-9=-68/347, 2-29=-341/340,
 14-16=-302/241
 BOT CHORD 28-29=-507/523, 27-28=-507/523, 26-27=-140/394, 25-26=-140/394, 24-25=-140/394,
 23-24=-140/394, 22-23=-140/394, 21-22=-140/394, 20-21=-140/394, 19-20=-140/394,
 18-19=-140/394
 WEBS 8-23=-285/0, 2-30=-239/490, 27-30=-234/475, 18-31=-146/439, 14-31=-148/454

NOTES (15-16)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.

Continued on page 2



November 11, 2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	T04G	GABLE	1	1	Job Reference (optional)

I4152995

Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:41 2009 Page 2

NOTES (15-16)

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 17 except (jt=lb) 29=356, 16=250, 24=208, 25=226, 26=222, 27=485, 21=206, 20=227, 19=222, 18=465.
- 12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-87(F=-33), 2-8=-87(F=-33), 8-14=-87(F=-33), 14-15=-87(F=-33), 16-29=-10



November 11, 2009

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	T05	SPECIAL	5	1	Job Reference (optional)

I4152996

Builders FrstSource, Lake City, FL 32055

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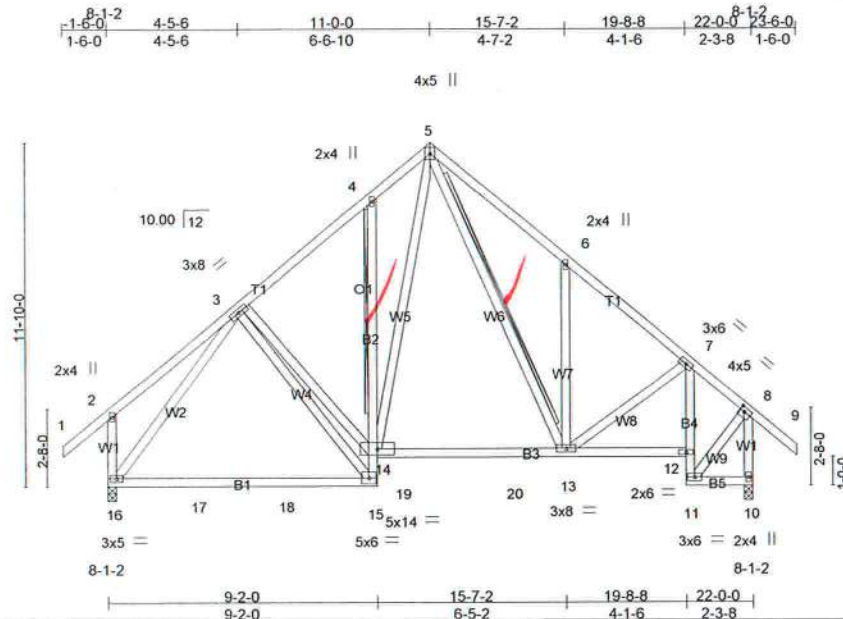


Plate Offsets (X,Y): [8:0-2-0-0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.27	Vert(LL)	-0.19 15-16	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.83	Vert(TL)	-0.33 15-16	>788	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.89	Horz(TL)	0.10 10	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL)	0.05 13-14	>999	240		
							Weight: 192 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B2,B4: 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3 *Except*
 W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
 T-Brace: 2 X 4 SYP No.3 - 4-14
 T-Brace: 2 X 4 SYP No.3 - 5-13
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

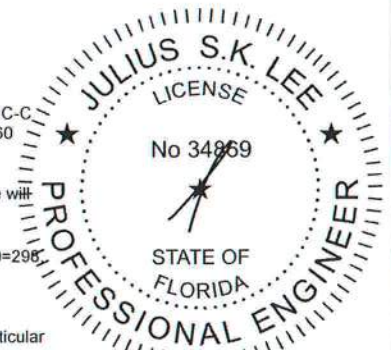
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=890/0-3-8, 16=949/0-3-8
 Max Horz 16=-448(LC 5)
 Max Uplift 10=-298(LC 8), 16=-298(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-141/291, 3-4=-757/566, 4-5=-716/754, 5-6=-853/825, 6-7=-830/528, 7-8=-534/393,
 2-16=-247/352, 8-10=-940/621
 BOT CHORD 16-17=-323/476, 17-18=-323/476, 15-18=-323/476, 14-15=-344/551, 4-14=-198/282,
 14-19=-146/440, 19-20=-146/440, 13-20=-146/440, 12-13=-203/436, 11-12=-381/167,
 7-12=-411/183
 WEBS 3-14=-251/567, 3-15=-565/467, 5-14=-445/471, 5-13=-469/391, 6-13=-266/425,
 3-16=-735/321, 8-11=-243/583

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=298, 16=298.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



November 11, 2009

LOAD CASE(S) Standard



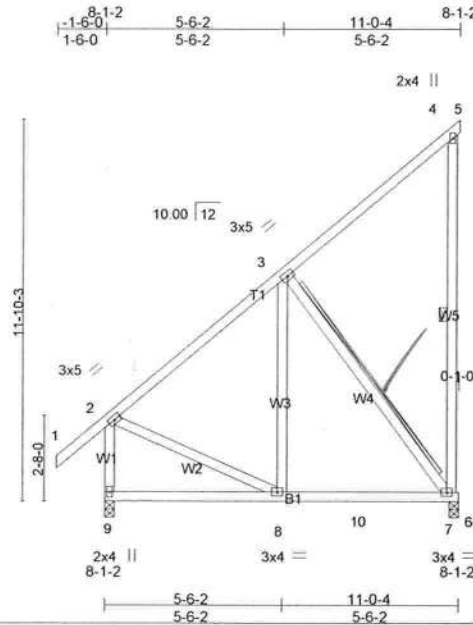
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 318492	Truss T06	Truss Type MONO TRUSS	Qty 5	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	14152997
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Builders FirstSource, Lake City, FL 32055

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Scale = 1:67.5

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.28	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.15	Vert(LL) -0.02 7-8 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.13	Vert(TL) -0.03 7-8 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.02 7 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.01 8-9 >999 240		
				Weight: 91 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.
WEBS 1 Row at midpt 4-7
T-Brace: 2 X 4 SYP No.3 - 3-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=397/0-3-8, 9=473/0-3-8
Max Horz 9=535(LC 7)
Max Uplift 7=407(LC 7)

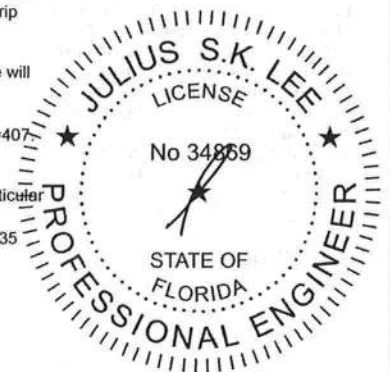
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-327/0, 2-9=-456/43
BOT CHORD 8-9=-698/29, 8-10=-326/185, 7-10=-326/185
WEBS 3-7=-298/529, 2-8=0/410

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=407
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 11, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318492	Truss T08	Truss Type HIP	Qty 7	Ply 1	RICHARD KEEN - LAMBERT RES.	4152999
Builders FrstSource, Lake City, FL 32055						Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:44 2009 Page 1

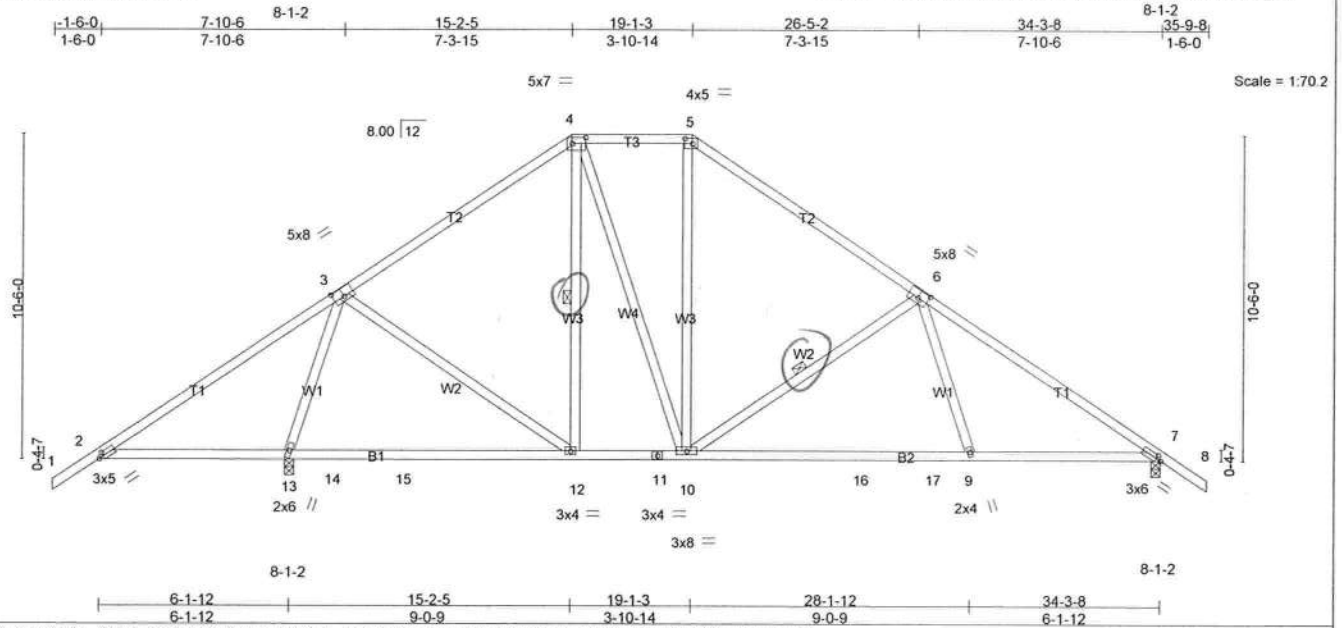


Plate Offsets (X,Y): [2.0-1-15.0-1-8], [3.0-4-0.0-3-4], [4.0-5-4.0-2-4], [5.0-3-0.0-2-0], [6.0-4-0.0-3-0], [7.0-1-15.0-1-8]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.53	Vert(LL)	-0.15 9-10 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.49	Vert(TL)	-0.29 9-10 >999 240
BCLL 0.0	Rep Stress Incr	YES	WB 0.75	Horz(TL)	0.03 7 n/a n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	-0.10 12-13 >999 240
					PLATES GRIP
					MT20 244/190
					Weight: 204 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

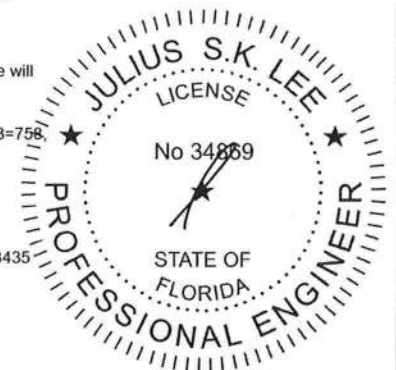
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 13=1542/0-3-8, 7=1000/0-3-8
Max Horz 13=362(LC 6)
Max Uplift 13=758(LC 7), 7=389(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-744/587, 3-4=-712/479, 4-5=-582/577, 5-6=-825/567, 6-7=-1404/690
BOT CHORD 2-13=-380/812, 13-14=-231/456, 14-15=-231/456, 15-16=-231/456, 16-17=-377/1010, 17-18=-377/1010, 18-19=-368/1071
WEBS 3-12=-292/525, 4-10=-246/325, 6-10=-523/530, 6-9=0/280, 3-13=-1411/1295

- NOTES** (10-11)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - 6) All bearings are assumed to be SYP No.2.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=758, 7=389.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 11, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318492	Truss T08G	Truss Type GABLE	Qty 1	Ply 1	RICHARD KEEN - LAMBERT RES.	14153000
Builders FrstSource, Lake City, FL 32055						Job Reference (optional)

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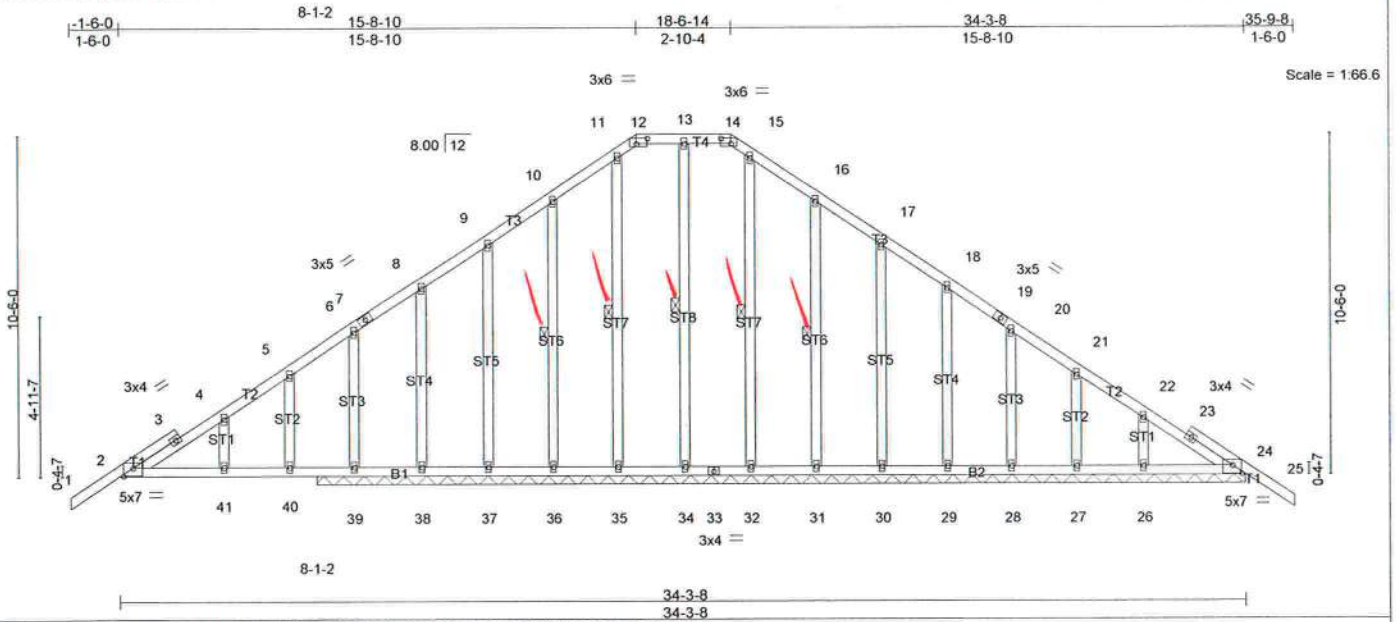


Plate Offsets (X,Y): [2:0-3-8,0-3-2], [12:0-3-12,0-2-0], [14:0-3-12,0-2-0], [24:0-3-8,0-3-2]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL)	-0.01	25	n/r	120	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.02	25	n/r	90	
BCLL 0.0	Rep Stress Incr	NO	WB 0.25	Horz(TL)	0.07	24	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)						Weight: 253 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 5-8-13 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 12-14.
Rigid ceiling directly applied or 6-0-0 oc bracing.
1 Row at midpt 13-34, 11-35, 10-36, 15-32, 16-31

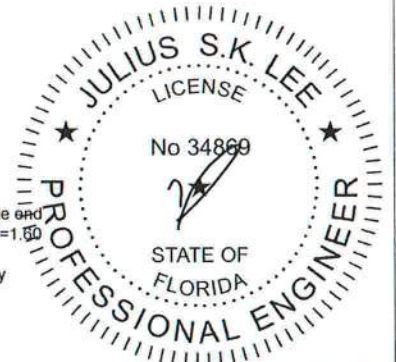
Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 28-3-8.
(lb) - Max Horz 39=-463(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) except 24=-215(LC 11), 34=-343(LC 5), 35=-289(LC 7), 36=-157(LC 7), 37=-279(LC 7), 38=-131(LC 1), 39=-800(LC 7), 32=-331(LC 5), 31=-192(LC 8), 30=-188(LC 8), 29=-190(LC 8), 28=-185(LC 8), 27=-206(LC 8), 26=-126(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 24, 36, 38, 31, 30, 29, 28, 27, 26 except 34=522(LC 1), 35=532(LC 11), 37=278(LC 11), 39=724(LC 1), 32=527(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-992/769, 3-4=-995/823, 4-5=-1015/886, 5-6=-969/935, 6-7=-698/723, 7-8=-713/810, 8-9=-722/885, 9-10=-611/867, 10-11=-537/894, 11-12=-333/630, 12-13=-369/685, 13-14=-369/685, 14-15=-333/630, 15-16=-536/894, 16-17=-614/869, 17-18=-705/872, 18-19=-795/872, 19-20=-779/803, 20-21=-883/870, 21-22=-981/876, 22-23=-1031/850, 23-24=-1010/789
BOT CHORD 2-41=-685/1023, 40-41=-685/1023, 39-40=-685/987, 37-38=-685/987, 36-37=-685/987, 35-36=-685/987, 34-35=-685/987, 33-34=-685/987, 32-33=-685/987, 31-32=-685/987, 30-31=-685/987, 29-30=-685/987, 28-29=-685/987, 27-28=-685/987, 26-27=-685/987, 24-26=-685/987
WEBS 13-34=-503/355, 11-35=-508/363, 6-39=-400/506, 15-32=-507/362

NOTES (15-16)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2



November 11, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	T08G	GABLE	1	1	14153000
Builders FrstSource, Lake City, FL 32055					Job Reference (optional)
					7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:47 2009 Page 2

NOTES (15-16)

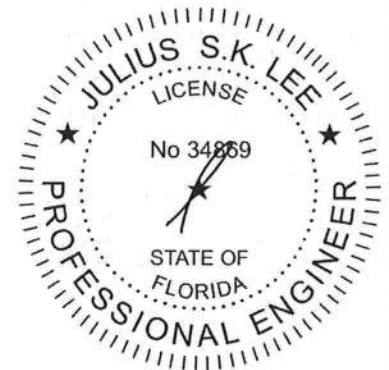
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SYP No.2 .
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 24, 343 lb uplift at joint 34, 289 lb uplift at joint 35, 157 lb uplift at joint 36, 279 lb uplift at joint 37, 131 lb uplift at joint 38, 800 lb uplift at joint 39, 331 lb uplift at joint 32, 192 lb uplift at joint 31, 188 lb uplift at joint 30, 190 lb uplift at joint 29, 185 lb uplift at joint 28, 206 lb uplift at joint 27 and 126 lb uplift at joint 26.
- 11) Non Standard bearing condition. Review required.
- 12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 13) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-12=-87(F=-33), 12-14=-87(F=-33), 14-25=-87(F=-33), 2-24=-10



November 11, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Boynton, FL 33435

Builders FirstSource, Lake City, FL 32055

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Job Reference (optional)

14153001



Plate Offsets (X,Y): [2:0-1-15,0-1-8], [3:0-4-0,0-3-4], [4:0-5-4,0-2-4], [5:0-3-0,0-2-0], [6:0-4-0,0-3-0], [7:0-0-11,0-0-2]

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 4-9-3 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
Rigid ceiling directly applied or 6-0-0 oc bracing.
1 Row at midpt 4-12, 6-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

(lb/size) 13=1521/0-3-8, 7=1033/0-3-8
Max Horz 13=-362(LC 5)
Max Uplift 13=-728(LC 7), 7=-395(LC 8)

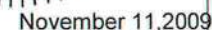
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=724/568, 3-4=777/531, 4-5=627/612, 5-6=879/608, 6-7=1479/730
BOT CHORD 2-13=364/97, 13-14=256/432, 14-15=256/432, 12-15=256/432, 11-12=150/537,
10-11=150/537, 10-16=414/1049, 16-17=414/1049, 9-17=414/1049, 7-9=403/1132
3-12=215/428, 4-10=238/309, 6-10=515/531, 6-9=0/296, 3-13=1422/298

NOTES (10-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCFL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) All bearings are assumed to be SYP No.2 .
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 728 lb uplift at joint 13 and 395 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10 nails.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

WARNING - Verify design parameters and READ NOTES on THIS and INCLUDED MIKEE REFERENCE PAGE MW-1743 BEFORE USE.
Design valid for use only with Mikee connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318492	Truss T10	Truss Type HIP	Qty 1	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	4153002
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Builders FirstSource, Lake City, FL 32055

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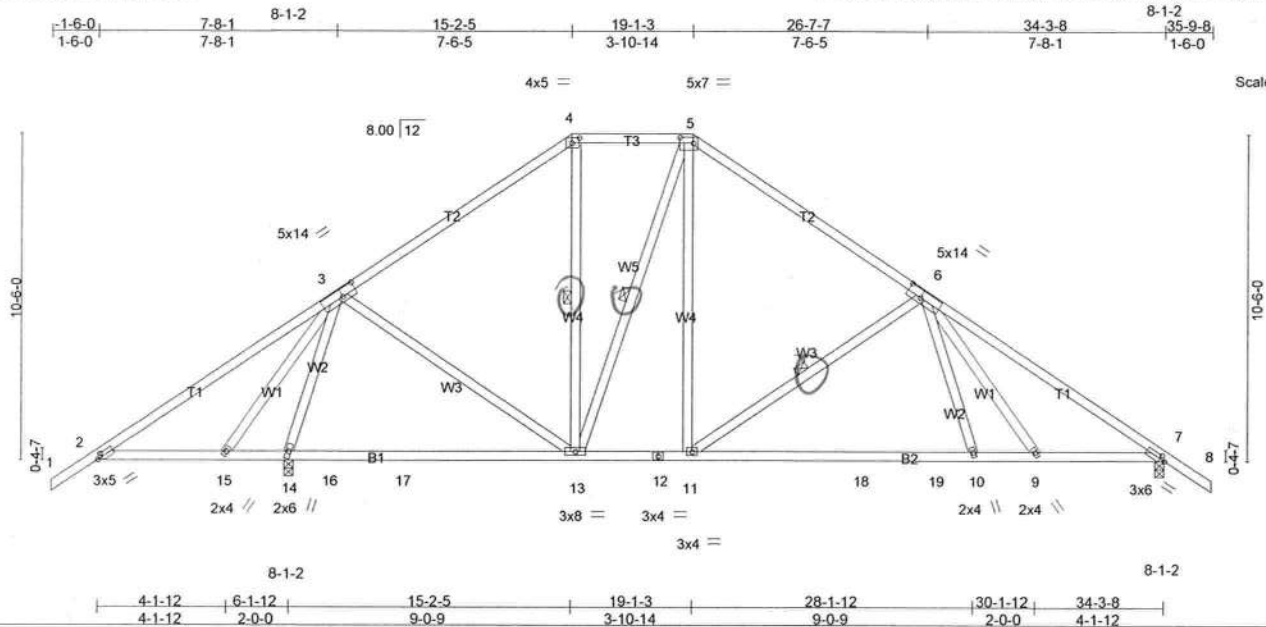


Plate Offsets (X,Y): [2.0-1.15,0-1-8], [3.0-5.8,0-3-0], [4.0-2.12,0-2-0], [5.0-5.4,0-2-4], [6.0-5.8,0-3-0], [7.0-1.15,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.53	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.50	Vert(LL) -0.15 10-11 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.72	Vert(TL) -0.29 10-11 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 7 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.06 10-11 >999 240		
				Weight: 221 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 5-13, 6-11

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

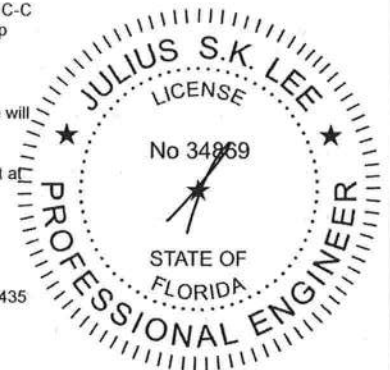
REACTIONS (lb/size) 14=1544/0-3-8, 7=1001/0-3-8
Max Horz 14=-362(LC 5)
Max Uplift 14=-759(LC 7), 7=-389(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-612/598, 3-4=-712/483, 4-5=-485/505, 5-6=-825/567, 6-7=-1390/716
BOT CHORD 2-15=-389/708, 14-15=-372/898, 14-16=-227/465, 16-17=-227/465, 13-17=-227/465,
12-13=-85/585, 11-12=-85/585, 11-18=-371/1013, 18-19=-371/1013, 10-19=-371/1013,
9-10=-348/1083, 7-9=-395/1060
WEBS 3-15=-347/1, 3-14=-1390/1507, 3-13=-323/524, 5-13=-325/241, 5-11=-262/382,
6-11=-528/523, 6-10=0/286

NOTES (10-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) All bearings are assumed to be SYP No.2.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 759 lb uplift at joint 14 and 389 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



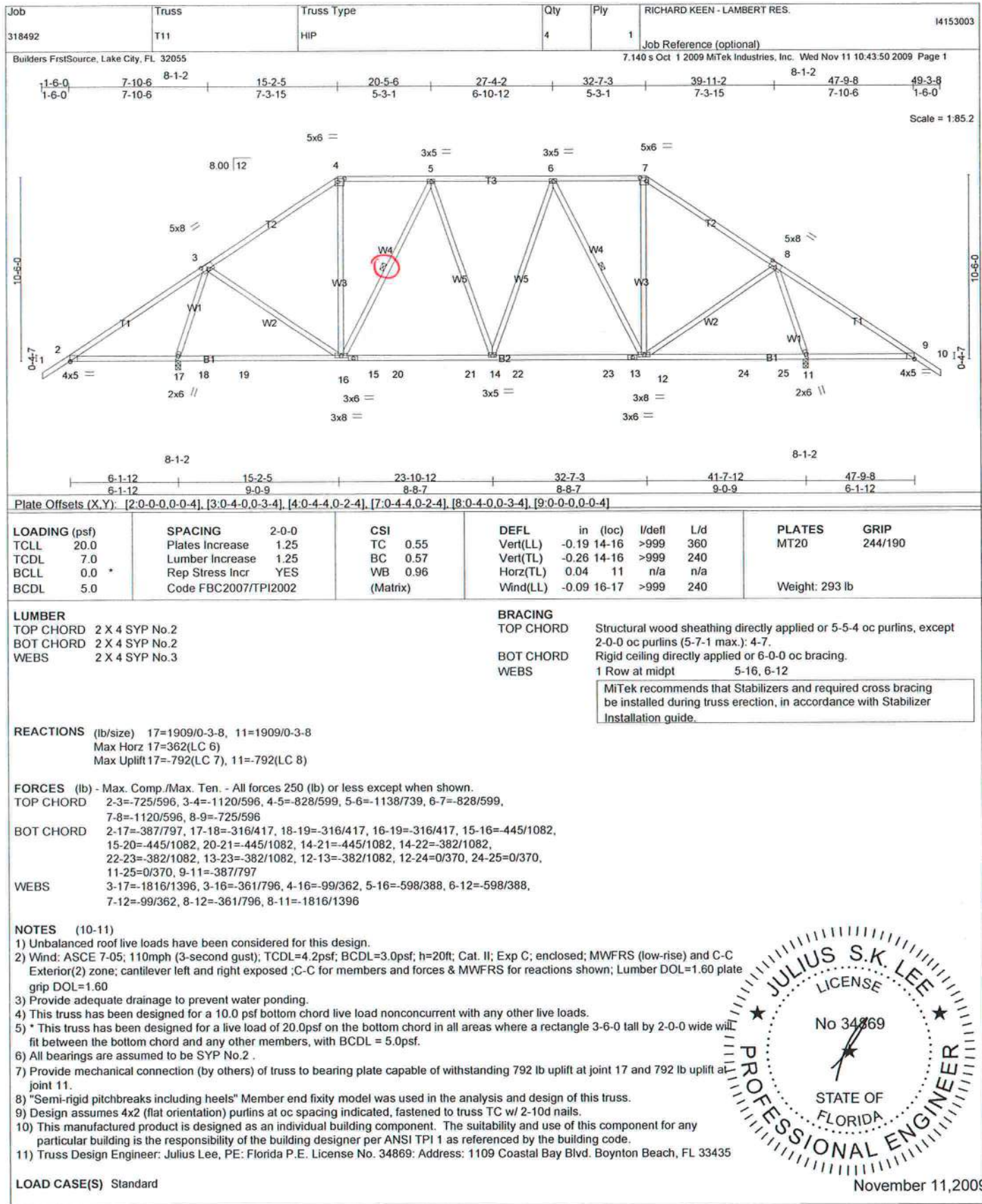
November 11, 2009



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	T11G	GABLE	1	1	14153004
Builders FrstSource, Lake City, FL 32055			Job Reference (optional)		
7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:52 2009 Page 2					

15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

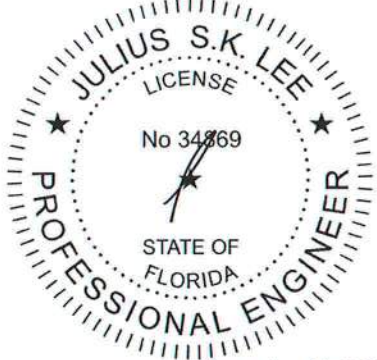
16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-87(F=-33), 7-12=-87(F=-33), 10-11=-33(F), 2-42=-10, 42-43=-50, 43-44=-10, 44-45=-50, 45-46=-10, 46-47=-50, 11-47=-10



November 11, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318492	Truss T12	Truss Type SPECIAL	Qty 1	Ply 1	RICHARD KEEN - LAMBERT RES. Job Reference (optional)	14153005	
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:54 2009 Page 1				
1-6-0 3-4-3 6-3-8 8-1-2 10-1-14 15-2-5 21-1-6 26-8-2 32-7-3 37-2-10 41-7-12 8-1-2 47-9-8 49-3-8 1-6-0 3-4-3 2-11-5 3-10-6 5-0-7 5-11-0 5-6-13 5-11-0 4-7-7 4-5-2 6-1-12 1-6-0 Scale = 1.86.7 Plate Offsets (X,Y): [2.0-0.0-0.0-4], [5.0-3.0-0.3-0], [6.0-4.4-0.2-4], [9.0-3.0-0.2-3], [11.0-3.0-0.3-0], [12.0-0.0-0.0-4], [15.0-3.12-0.3-0], [22.0-3.8-0.1-8]							
LOADING (psf)		SPACING	2-0-0	CSI	DEFL	PLATES	GRIP
TCDL	20.0	Plates Increase	1.25	TC	0.35	MT20	244/190
BCLL	0.0	Lumber Increase	1.25	BC	0.76		
BCDL	5.0	Rep Stress Incr	YES	WB	0.98		
		Code FBC2007/TPI2002		(Matrix)			
LUMBER		BRACING					
TOP CHORD		TOP CHORD		Structural wood sheathing directly applied or 4-4-11 oc purlins, except 2-0-0 oc purlins (5-0-2 max.): 6-9.			
BOT CHORD		BOT CHORD		Rigid ceiling directly applied or 3-9-14 oc bracing. Except: 10-0-0 oc bracing: 16-18			
WEBS		WEBS		1 Row at midpt 7-21, 8-18			
REACTIONS (lb/size)		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.					
24=1773/0-3-8, 14=1786/0-3-8							
Max Horz 24=-362(LC 4)							
Max Uplift 24=-788(LC 6), 14=-781(LC 7)							
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.							
TOP CHORD							
2-3=-600/403, 3-4=-2995/1700, 4-5=-1005/572, 5-6=-1346/582, 6-7=-1063/533, 7-8=-1510/730, 8-9=-1082/508, 9-10=-1351/519, 10-11=-675/262, 11-12=-675/543							
BOT CHORD							
2-24=-290/625, 23-24=-1944/1786, 4-23=-2266/2513, 22-23=-1367/2768, 21-22=-628/777, 21-26=-613/1448, 20-26=-613/1448, 20-27=-613/1448, 19-27=-613/1448, 19-28=-536/1454, 28-29=-536/1454, 18-29=-536/1454, 9-18=-99/490, 14-15=-379/744, 12-14=-369/736							
WEBS							
3-24=-751/330, 3-23=-1390/2804, 4-22=-2449/2236, 5-22=-623/728, 5-21=-445/352, 6-21=-144/479, 7-21=-640/396, 8-18=-617/406, 15-18=-48/600, 10-18=-292/761, 10-15=-1103/417, 11-15=-717/1186, 11-14=-1700/1265							
NOTES (10-11)							
1) Unbalanced roof live loads have been considered for this design.							
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60							
3) Provide adequate drainage to prevent water ponding.							
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.							
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.							
6) All bearings are assumed to be SYP No.2							
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 788 lb uplift at joint 24 and 781 lb uplift at joint 14.							
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.							
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.							
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.							
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435							

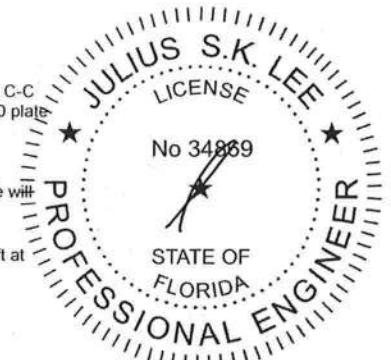
LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Boynton, FL 33435



November 11, 2009

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.	14153006
318492	T13	SPECIAL	4	1	Job Reference (optional)	

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LOAD CASE(S) Standard



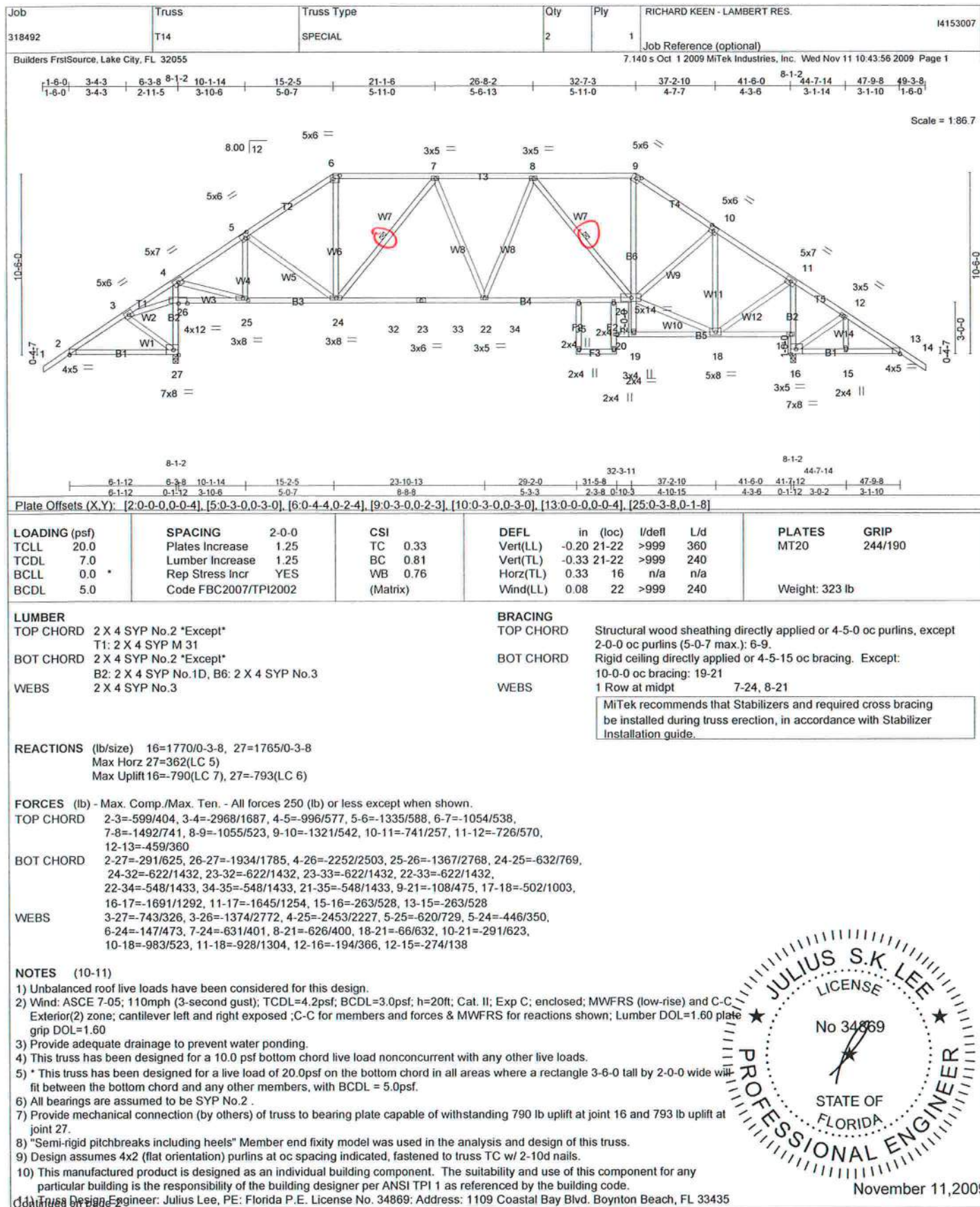
November 11, 2009



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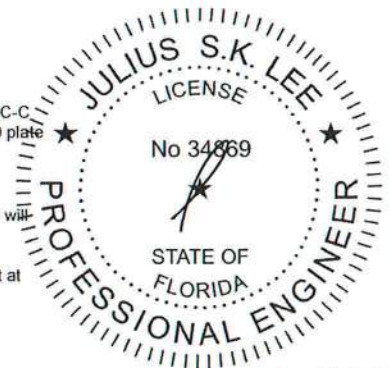
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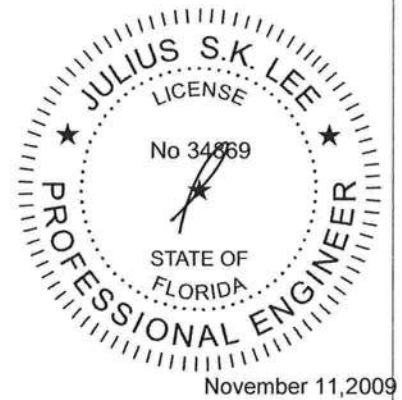
November 11, 2009

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.	I4153007
318492	T14	SPECIAL	2	1	Job Reference (optional)	

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LOAD CASE(S) Standard



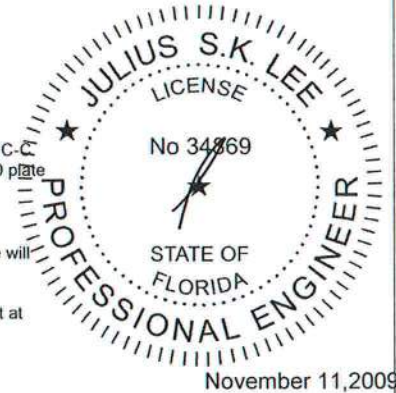
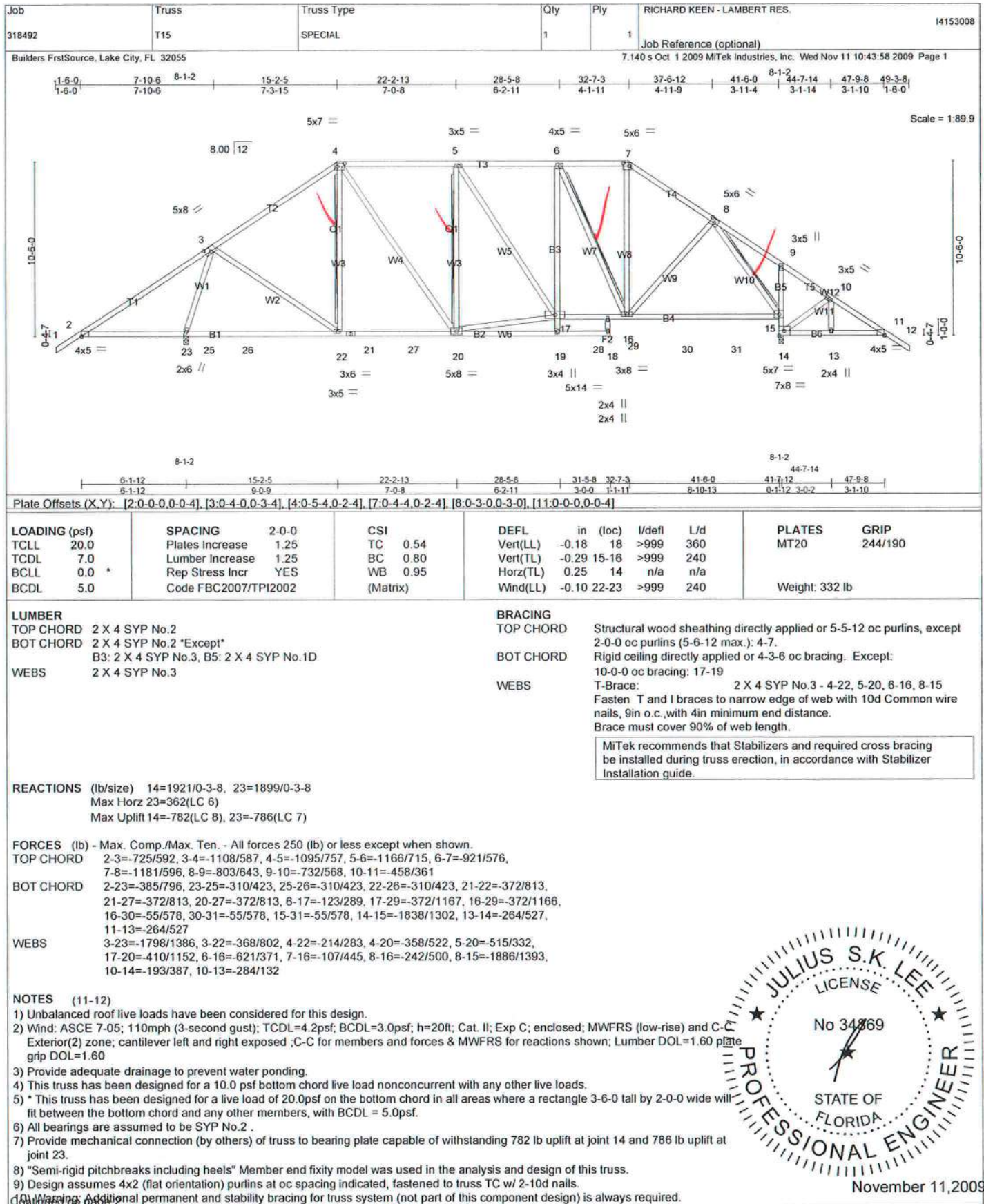
November 11, 2009



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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.	14153008
318492	T15	SPECIAL	1	1	Job Reference (optional)	

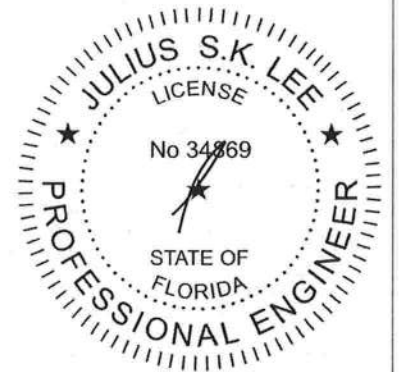
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7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:58 2009 Page 2

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



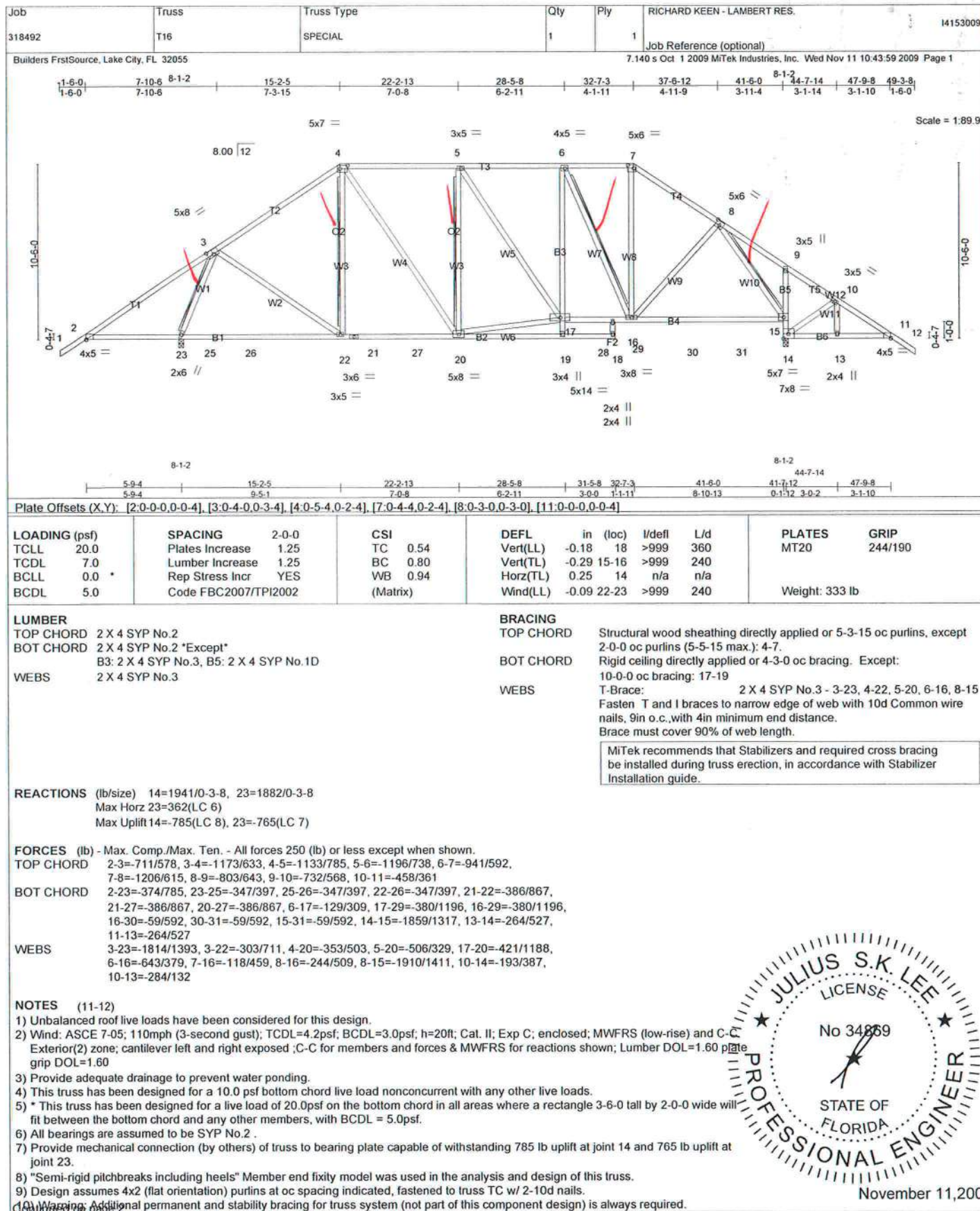
November 11, 2009



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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.
318492	T16	SPECIAL	1	1	
					Job Reference (optional)

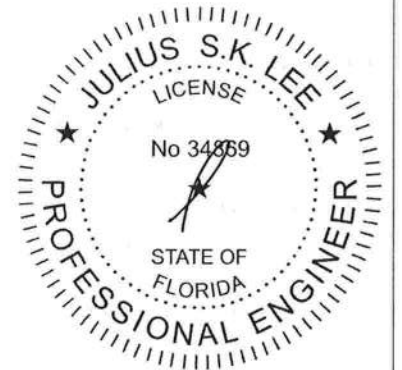
I4153009

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7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:43:59 2009 Page 2

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- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



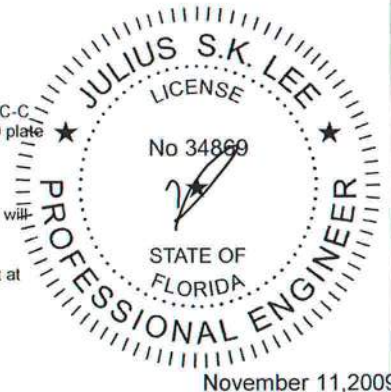
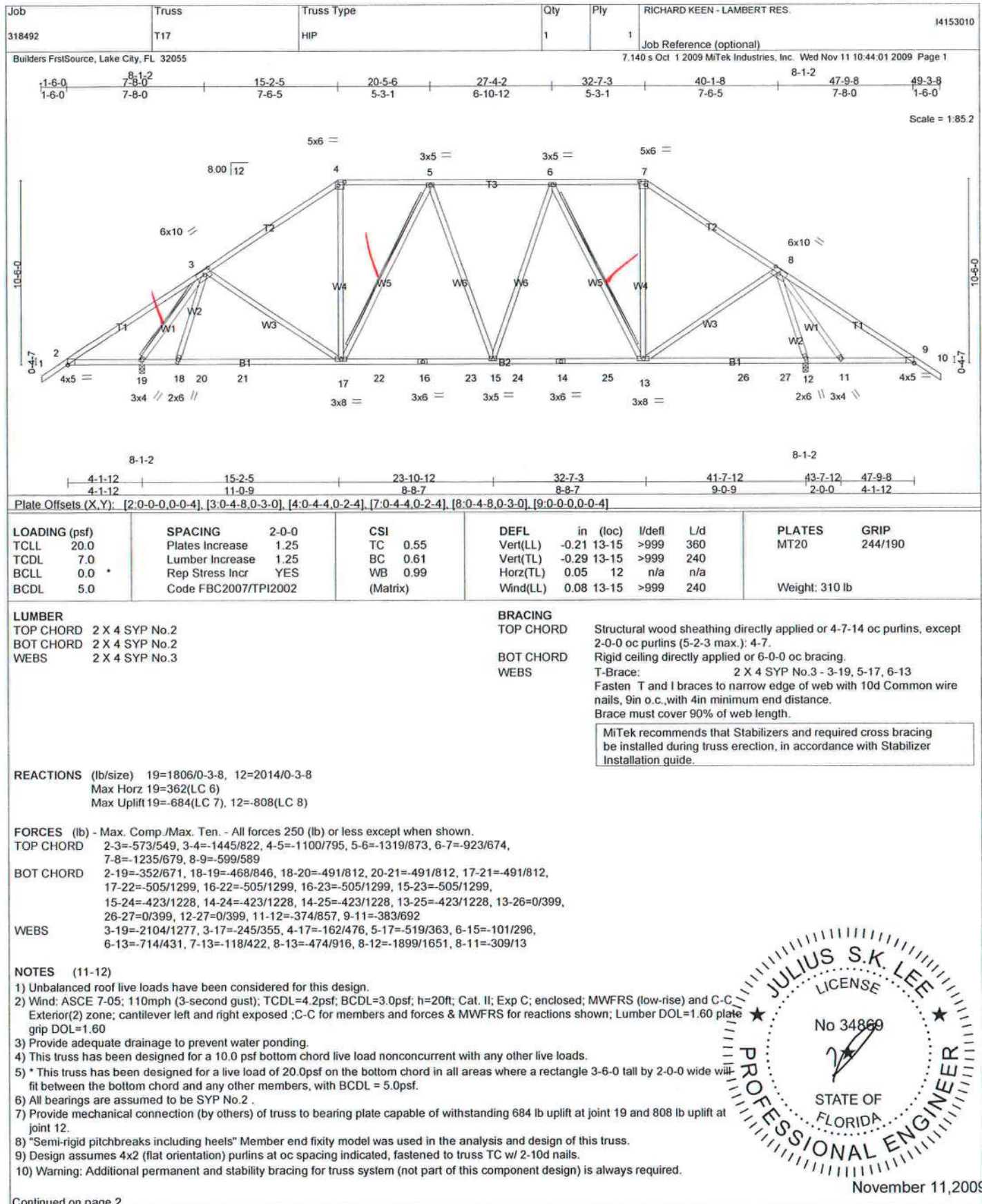
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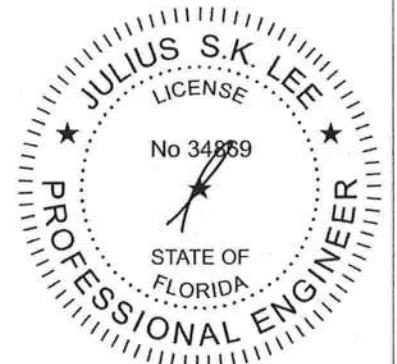
Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.	4153010
318492	T17	HIP	1	1	Job Reference (optional)	

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7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:44:01 2009 Page 2

- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard



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LUMBER TOP CHORD 2 X 4 SYP No.2 *Except* T4: 2 X 6 SYP No.1D BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 4-0-14 oc purlins, except 2-0-0 oc purlins (4-3-1 max.): 4-7. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. WEBS T-Brace: 2 X 4 SYP No.3 - 3-19, 5-17, 6-14, 8-14, 8-13 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
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FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

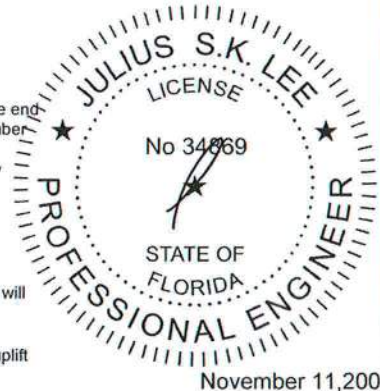
TOP CHORD 2-3=-571/545, 3-4=-1814/989, 4-5=-1410/935, 5-6=-1729/1060, 6-7=-1297/846,
7-8=-1776/878, 8-9=-831/1043, 9-10=-818/774

BOT CHORD 2-19=-348/670, 18-19=-773/1035, 18-43=-799/1003, 43-44=-799/1003, 17-44=-799/1003,
16-17=-1031/1714, 16-45=-1031/1714, 45-46=-1031/1714, 15-46=-1031/1714,
15-47=-917/1638, 47-48=-917/1638, 14-48=-917/1638, 14-49=-30/357, 49-50=-30/357,
13-50=-30/357, 12-13=-643/1015, 10-12=-703/876

WEBS 3-19=-2430/1425, 3-17=-425/495, 4-17=-302/614, 5-17=-754/659, 6-15=-153/302,
6-14=-889/373, 7-14=-246/487, 8-14=-690/1198, 8-15=-2759/2049

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 9) All bearings are assumed to be SYP No.2 .
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1212 lb uplift at joint 19 and 1629 lb uplift at joint 13.

(11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LAMBERT RES.	I4153011
318492	T17G	GABLE	1	1	Job Reference (optional)	

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7.140 s Oct 1 2009 MiTek Industries, Inc. Wed Nov 11 10:44:03 2009 Page 2

NOTES (15-16)

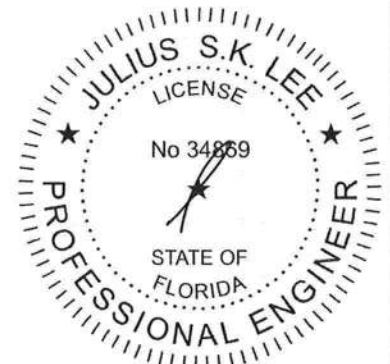
- 12) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-87(F=-33), 7-11=-87(F=-33), 2-43=-10, 43-44=-50, 44-45=-10, 45-46=-50, 46-47=-10, 47-48=-50, 48-49=-10, 49-50=-50, 10-50=-10



November 11, 2009

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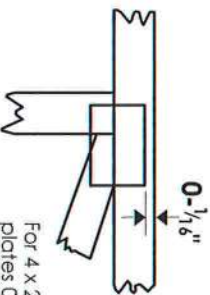
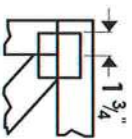
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Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **Mittek 20/20** software or upon request.

PLATE SIZE

4 X 4

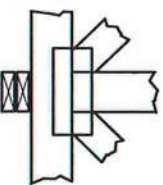
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



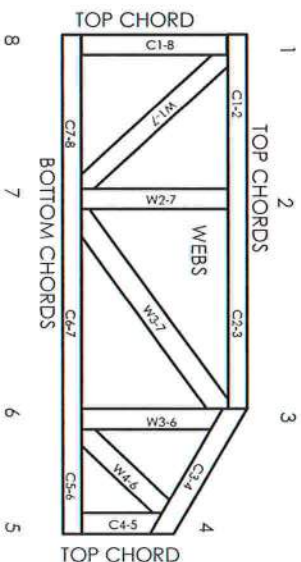
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by ANSI/FP11.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP11.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/FP11 Quality Criteria.

TOP	CHORD	2X4	SO.	PINE	#2	or	Better
BOT	CHORD	2X4	SO.	PINE	#2	or	Better
	WEBS	2X4	SO.	PINE	#3	or	Better

120 MPH MAX

Setback 7' or Less

2' TYP.
MAX

#1 HIP TRUSS

#2 HIP OR COMMON TRUSS

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

CJ's
2' TYP. —
MAX

I

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less

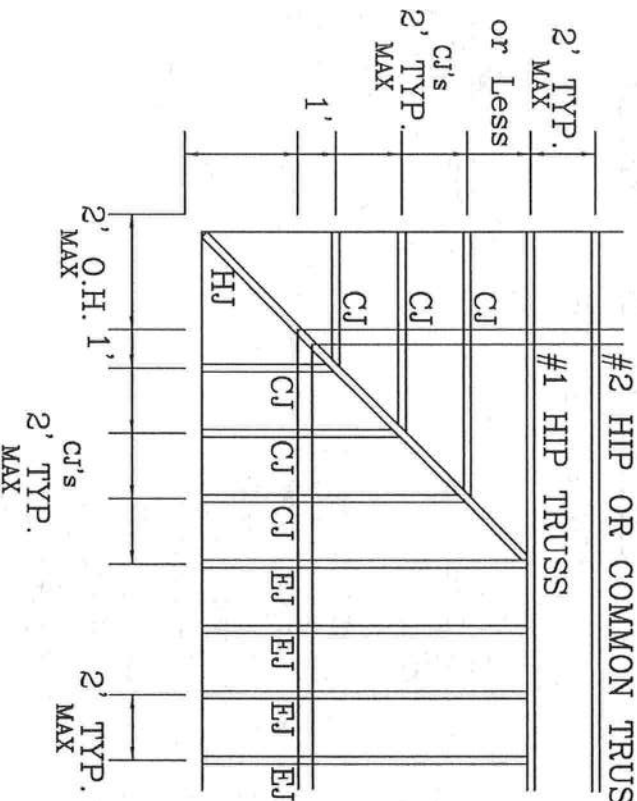
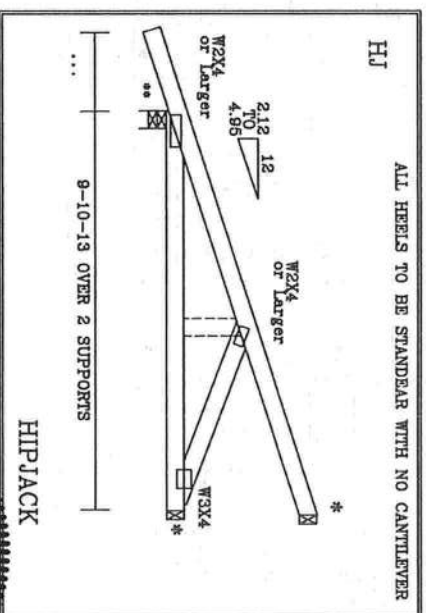
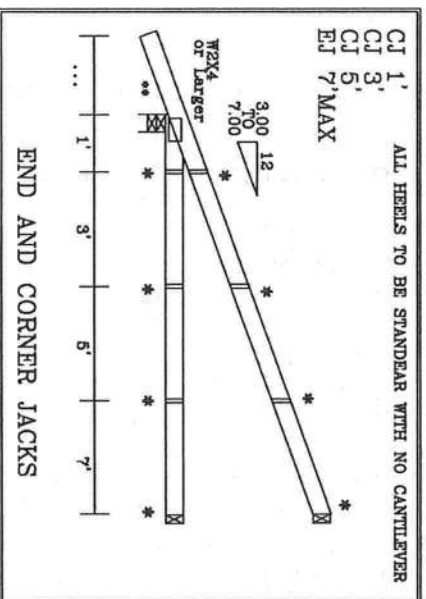
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

(3) 16d TOENAILS

SEE FOR THE DOWN
 看看

END AND CORNER JACKS

HIPJACK

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED

BC LIVE LOAD IS NON CONCURRENT 10%

CORNER SET
SETBACK

7'0" MAX

ALPINE ENGINEERED TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO THE FOLLOWING BUILDING CODES AND TRUSS SAFETY INFORMATION PUBLISHED BY THE TRUSS PLATE INSTITUTE: 968 THOMPSON DR., SUITE 200, MADISON, WI 53719 AND VITA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS SPECIFICATION AND ATTACHED DRAWINGS. THE USER SHALL BE RESPONSIBLE FOR THE PROPER SELECTION AND APPLICATION OF THE BRACING OF TRUSSES. DESIGN CONSIDERS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPECIFICATION FOR STEEL, ALPINE CONNECTER PLATES ARE MADE OF 2018/16GA U.S.A.S. ASTM A653 GRADE 50. 0/60 (A/C/A/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCALIZED ON THIS DESIGN, POSITION PER TYPICAL SECTION 1604-02. ANY INSPECTION OF PLATES ACCEPTED BY CD SHALL BE PER AMERICAN AS OF TYPICAL SECTION 3, A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE AVAILABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING OWNER AND USER.

CONS. ENGINEERS, P.A.

1465 SW 4th AVENUE
MIRRAY BEACH FL 33444-2181
NO. 34869

STATE OF

SPACING	2' MAX	REF	7' MAX STBK CS
DUR. FAC.	1.25	DATE	Jun./27/2008
		DRWG	
		-ENG	
		REVIEWED	
		By Julius Ise at 10:52 am, Jun 27, 2008	

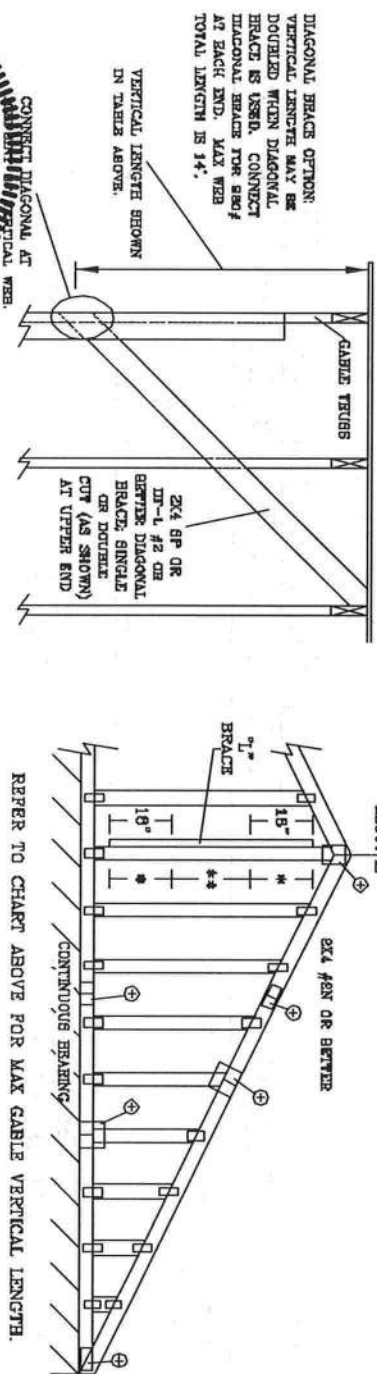
REVIEWED
By [Julius lee](#) at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																	
CABLE VERTICAL SPACING		2x4 SPECIES	BRACE		NO BRACES	(1) 1x4 "L" BRACE *											
			GRADE	GRADE		GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B		
12" O.C.		SPF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"			
		#3 <td>3' 3"</td> <td>4' 11"</td> <td>4' 11"</td> <td>6' 6"</td> <td>6' 6"</td> <td>8' 3"</td> <td>8' 3"</td> <td>10' 1"</td> <td>10' 1"</td> <td>12' 11"</td> <td>12' 11"</td>	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"				
		STUD <td>3' 3"</td> <td>4' 11"</td> <td>4' 11"</td> <td>6' 6"</td> <td>6' 6"</td> <td>8' 3"</td> <td>8' 3"</td> <td>10' 0"</td> <td>10' 0"</td> <td>12' 11"</td> <td>12' 11"</td>	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"				
		STANDARD <td>3' 3"</td> <td>4' 2"</td> <td>4' 2"</td> <td>5' 6"</td> <td>5' 6"</td> <td>7' 5"</td> <td>7' 5"</td> <td>8' 3"</td> <td>8' 3"</td> <td>10' 0"</td> <td>10' 0"</td>	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 3"	8' 3"	10' 0"	10' 0"				
16" O.C.		SP	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"			
		#2 <td>3' 7"</td> <td>5' 0"</td> <td>6' 0"</td> <td>6' 8"</td> <td>6' 8"</td> <td>8' 3"</td> <td>8' 3"</td> <td>9' 11"</td> <td>10' 4"</td> <td>12' 11"</td> <td>13' 7"</td>	3' 7"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	9' 11"	10' 4"	12' 11"	13' 7"				
		#3 <td>3' 6"</td> <td>5' 0"</td> <td>6' 0"</td> <td>6' 8"</td> <td>6' 8"</td> <td>8' 3"</td> <td>8' 3"</td> <td>10' 4"</td> <td>10' 4"</td> <td>12' 11"</td> <td>13' 7"</td>	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"				
		STUD <td>3' 6"</td> <td>5' 0"</td> <td>6' 0"</td> <td>6' 8"</td> <td>6' 8"</td> <td>8' 3"</td> <td>8' 3"</td> <td>10' 3"</td> <td>10' 3"</td> <td>12' 11"</td> <td>13' 7"</td>	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 3"	10' 3"	12' 11"	13' 7"				
24" O.C.		DFL	STANDARD	3' 4"	4' 3"	4' 8"	5' 8"	6' 6"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"				
		SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 6"	12' 9"	14' 0"				
		#3 <td>3' 9"</td> <td>6' 0"</td> <td>6' 0"</td> <td>7' 11"</td> <td>7' 11"</td> <td>9' 5"</td> <td>9' 5"</td> <td>12' 4"</td> <td>12' 4"</td> <td>14' 0"</td>	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"					
		STUD <td>3' 9"</td> <td>6' 0"</td> <td>6' 0"</td> <td>7' 11"</td> <td>7' 11"</td> <td>9' 5"</td> <td>9' 5"</td> <td>12' 4"</td> <td>12' 4"</td> <td>14' 0"</td>	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"					
12" O.C.		HF	STANDARD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"				
		#1 <td>3' 8"</td> <td>5' 2"</td> <td>6' 2"</td> <td>6' 10"</td> <td>6' 10"</td> <td>8' 2"</td> <td>8' 2"</td> <td>10' 7"</td> <td>10' 7"</td> <td>14' 0"</td>	3' 8"	5' 2"	6' 2"	6' 10"	6' 10"	8' 2"	8' 2"	10' 7"	10' 7"	14' 0"					
		#2 <td>4' 3"</td> <td>6' 8"</td> <td>7' 2"</td> <td>7' 11"</td> <td>8' 6"</td> <td>9' 5"</td> <td>10' 2"</td> <td>12' 5"</td> <td>13' 5"</td> <td>14' 0"</td>	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"					
		#3 <td>4' 0"</td> <td>6' 2"</td> <td>6' 2"</td> <td>7' 11"</td> <td>8' 2"</td> <td>9' 6"</td> <td>9' 11"</td> <td>12' 6"</td> <td>12' 6"</td> <td>14' 0"</td>	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 6"	12' 6"	14' 0"					
16" O.C.		DFL	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"				
		STANDARD <td>3' 10"</td> <td>5' 3"</td> <td>6' 11"</td> <td>6' 11"</td> <td>8' 11"</td> <td>9' 4"</td> <td>9' 4"</td> <td>10' 10"</td> <td>10' 10"</td> <td>14' 0"</td>	3' 10"	5' 3"	6' 11"	6' 11"	8' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"					
		SPF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 6"	13' 8"	14' 0"					
		#3 <td>4' 2"</td> <td>6' 11"</td> <td>6' 11"</td> <td>8' 9"</td> <td>8' 9"</td> <td>10' 5"</td> <td>10' 5"</td> <td>13' 8"</td> <td>13' 8"</td> <td>14' 0"</td>	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"					
24" O.C.		HF	STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"					
		STANDARD <td>4' 2"</td> <td>6' 11"</td> <td>6' 11"</td> <td>8' 9"</td> <td>8' 9"</td> <td>10' 5"</td> <td>10' 5"</td> <td>13' 8"</td> <td>13' 8"</td> <td>14' 0"</td>	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"					
		SPF	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"					
		#2 <td>4' 7"</td> <td>7' 4"</td> <td>7' 11"</td> <td>8' 9"</td> <td>9' 5"</td> <td>10' 6"</td> <td>11' 2"</td> <td>13' 8"</td> <td>14' 0"</td>	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"						
12" O.C.		SP	#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"					
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"						
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"					
		DFL	STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"				

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		2X4		BRACE		NO		(1) 1X4 T ¹ BRACE *		(1) 2X4 T ¹ BRACE *		(2) 2X4 T ¹ BRACE **		(1) 2X6 T ¹ BRACE *		(2) 2X6 T ¹ BRACE *		(2) 2X8 T ¹ BRACE **	
SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 2"	6' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				4' 5"	4' 5"	4' 5"	4' 5"	5' 10"	5' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				4' 6"	4' 6"	4' 6"	4' 6"	5' 10"	5' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				2' 11"	3' 9"	3' 9"	3' 9"	6' 0"	6' 0"	7' 10"	7' 10"	10' 7"	10' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
16" O.C.	SPF	#1 / #2	3' 3"	6' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				4' 6"	4' 6"	4' 6"	4' 6"	5' 11"	5' 11"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				4' 6"	4' 6"	4' 6"	4' 6"	5' 11"	5' 11"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				3' 3"	3' 3"	3' 3"	3' 3"	6' 0"	6' 0"	7' 10"	7' 10"	10' 7"	10' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
24" O.C.	SPF	#1 / #2	3' 3"	6' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				4' 6"	4' 6"	4' 6"	4' 6"	5' 11"	5' 11"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				4' 6"	4' 6"	4' 6"	4' 6"	5' 11"	5' 11"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
				3' 3"	3' 3"	3' 3"	3' 3"	6' 0"	6' 0"	7' 10"	7' 10"	10' 7"	10' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"



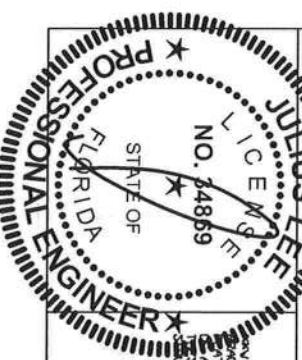
CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO. PLATES
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2X6

ATTACH EACH T¹ BRACE WITH 104 NAILS.
 * FOR (1) T¹ BRACE: SPACE NAILS AT 8" O.C.
 IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) T¹ BRACES: SPACE NAILS AT 3" O.C.
 IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
 T¹ BRACING MUST BE A MINIMUM OF 80% OF WEB
 MEMBER LENGTH.

BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPRUCE-PINE-FIR	GROUP B:
#1 / #2 STANDARD	#1 / #2 STUD
#3 STUD	#3 STUD
DOUGLAS FIR-LARCH	DOUGLAS FIR-LARCH
#1 / #2 STUD	#1 / #2 STUD
STANDARD	STANDARD

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 180 PSF OVER
 CONTINUOUS BRACING (6 PSF TO DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0"
 OUTLOWERS WITH 2' 0" OVERHANG, OR 12"
 PLTWOOD OVERHANG.



REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

PROVISIONS: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, DESTAILING AND
 BRACING. REFER TO BCST 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS
 ASSOCIATION, 1000 WEST 10TH AVENUE, SUITE 100, DENVER, CO 80202, FOR TRUSS DESIGN AND
 BRACING FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED
 STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBBED CEILING.

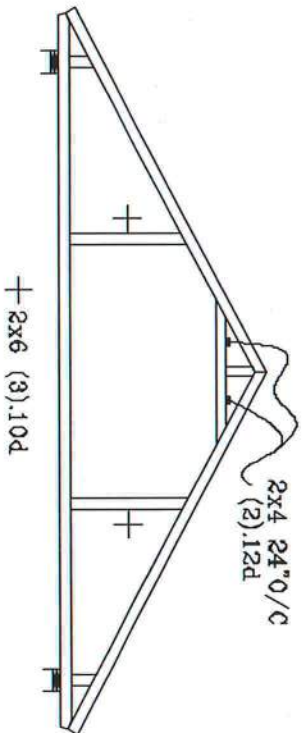
JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1466 SW 43rd AVENUE
 DELRAY BEACH, FL 33444-8161

No. 34869
 STATE OF FLORIDA

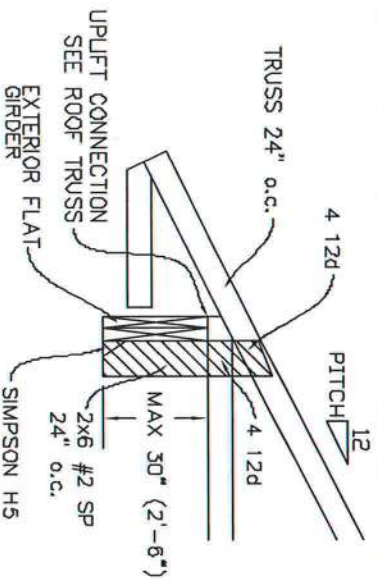
MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE7-02-CAB10000
 DATE 11/26/03
 DWG WERE STD GABLE 60' x 10'
 -ENG

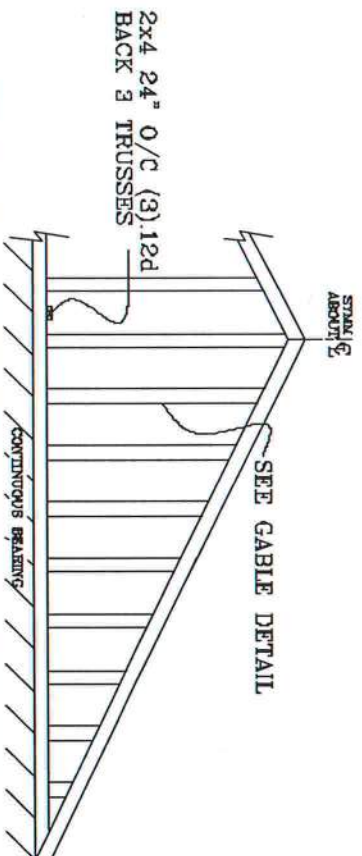
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

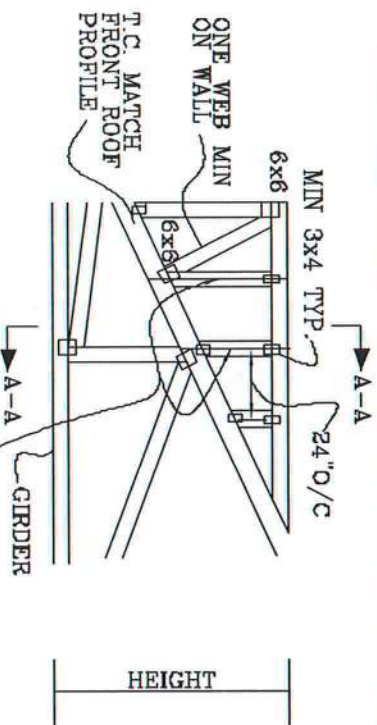


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

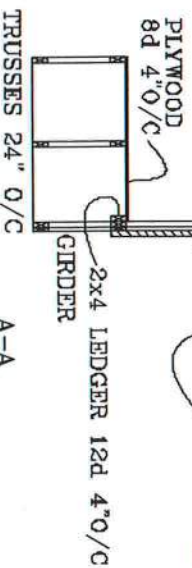
TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



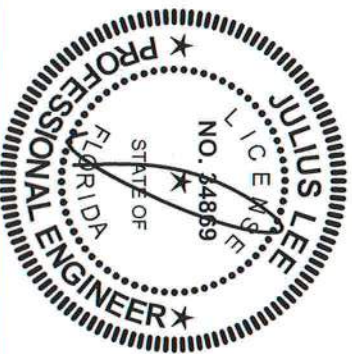
SEE ROOF TRUSSES FOR UPLIFT

ROOF 24" o/c

SEE CABL. END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



A-A



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1425 SW 4th AVENUE
DIKRAY BRIDGE, FL 33444-2161

No. 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
SPACE PIGGYBACK VERTICALS AT 4' OC MAX.
TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSSES.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

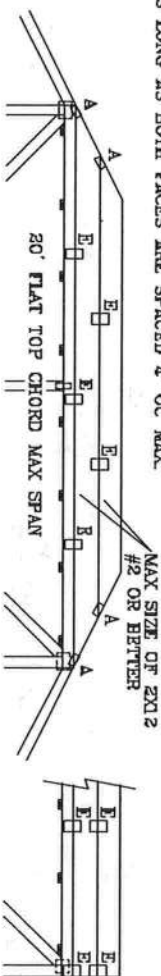
CAT I, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

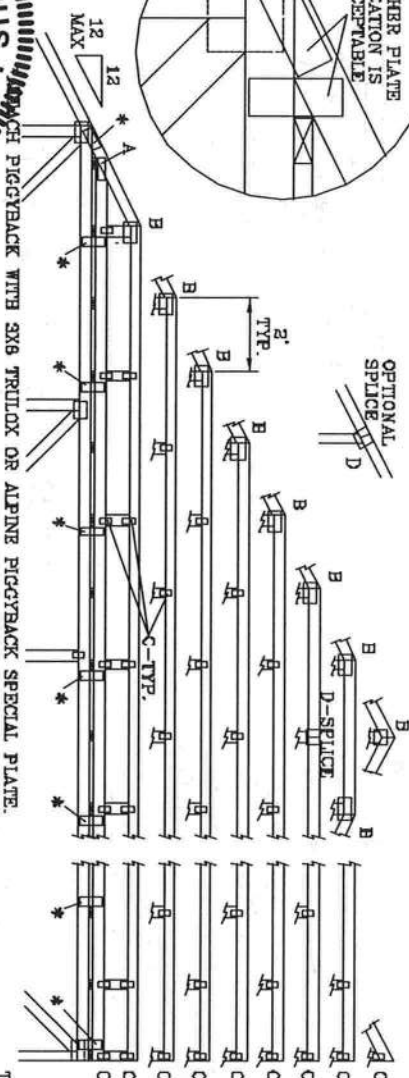
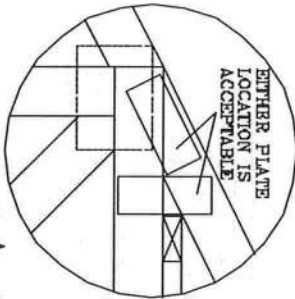
WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



OPTIONAL
SPLICE
D



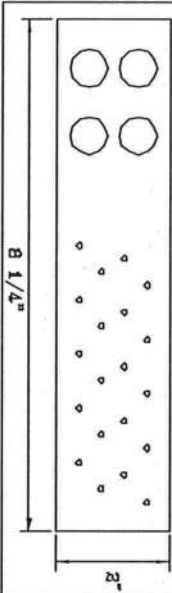
THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X3 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (B) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "P" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2X4 "P" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

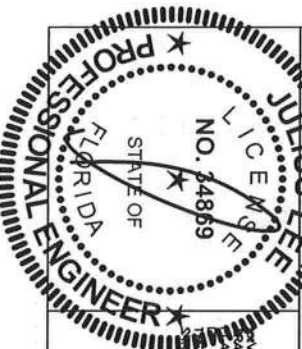
* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



JULIUS LEE'S
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1400 SW 4th AVENUE
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MAX LOADING
55 PSF AT
1.33 DUR. FAC.
50 PSF AT
1.25 DUR. FAC.
47 PSF AT
1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG/ITEK STD PIGGY
-ENG JL



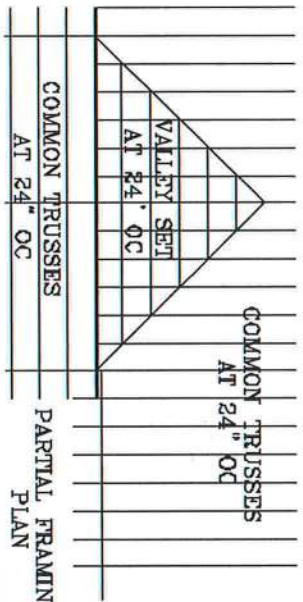
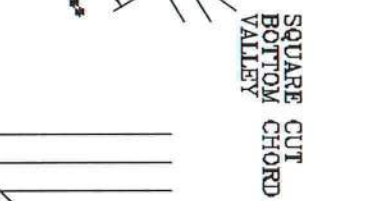
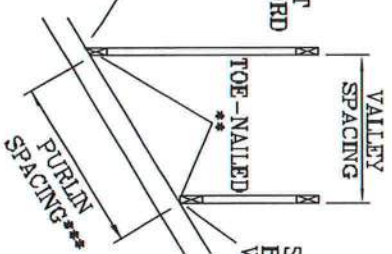
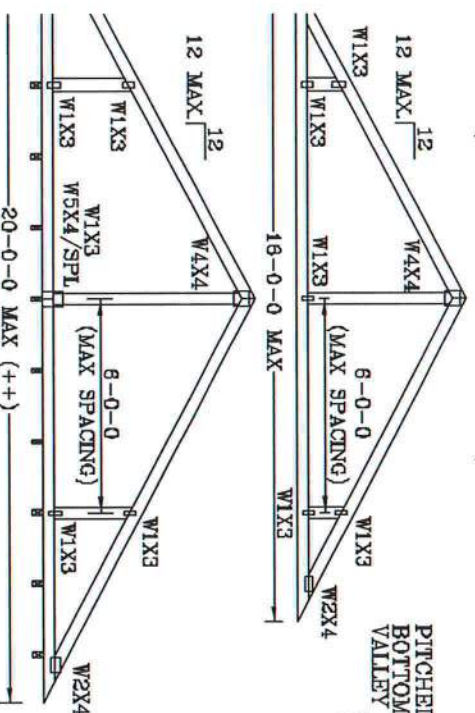
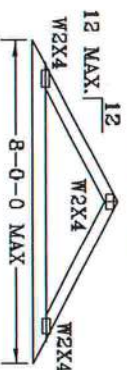
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By Julius Lee at 11:59 am, Jun 11, 2008

VALLEY TRUSS DETAIL

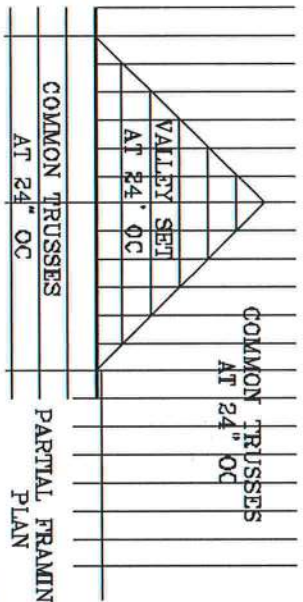
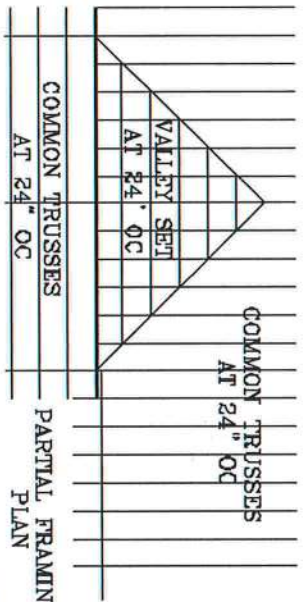
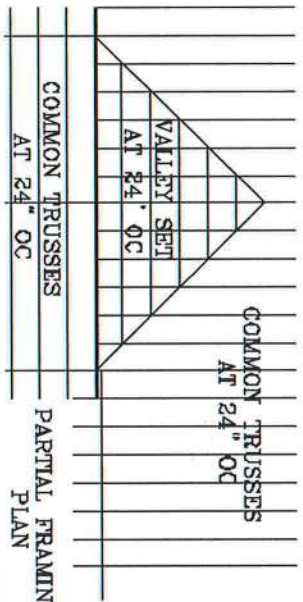
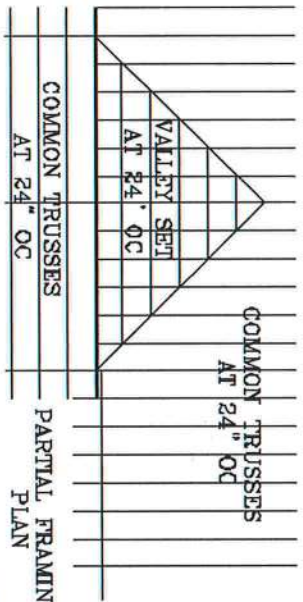
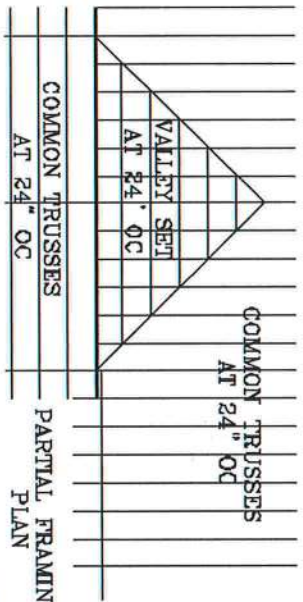
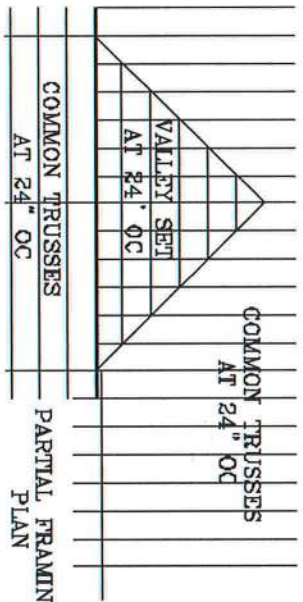
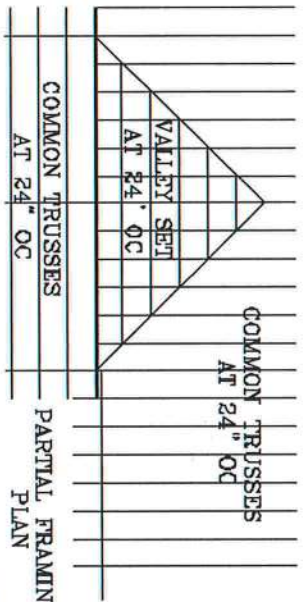
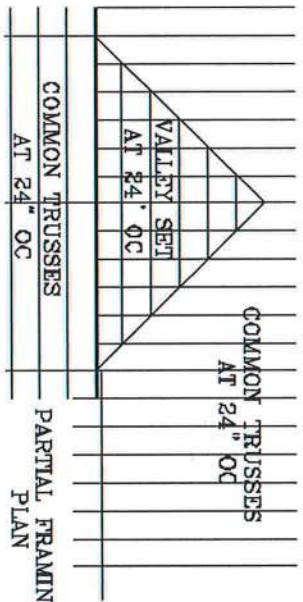
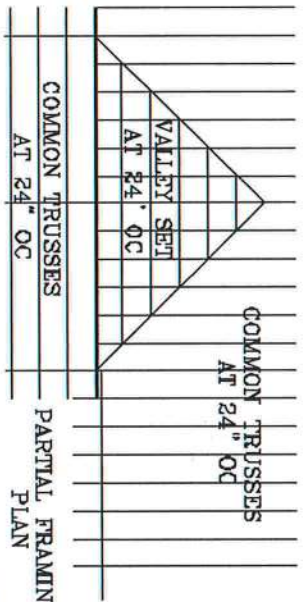
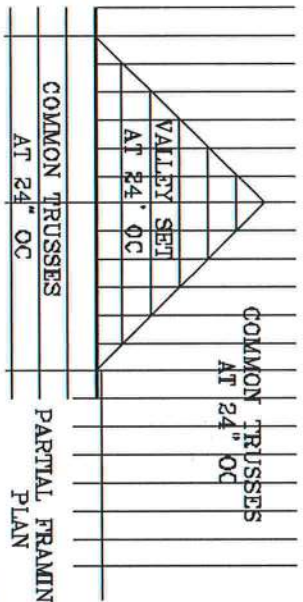
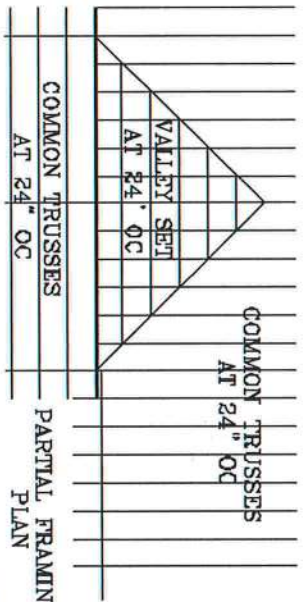
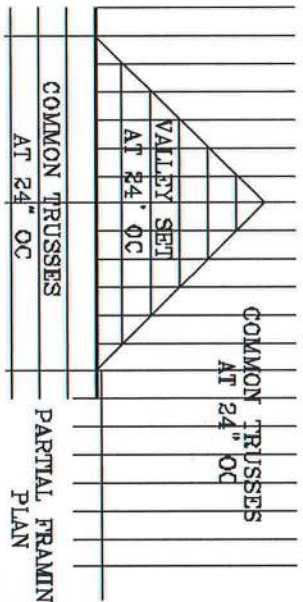
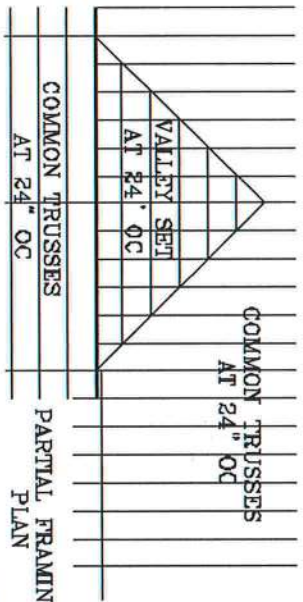
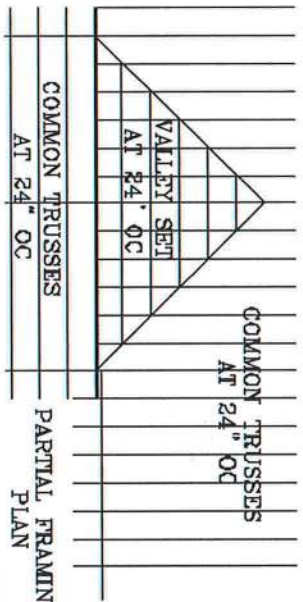
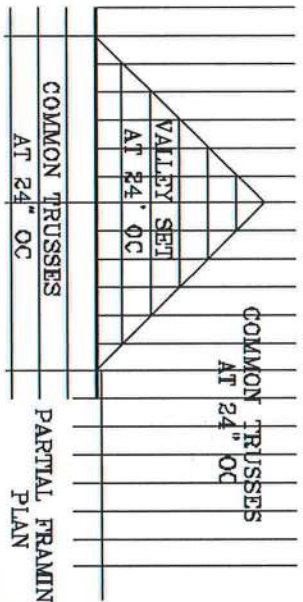
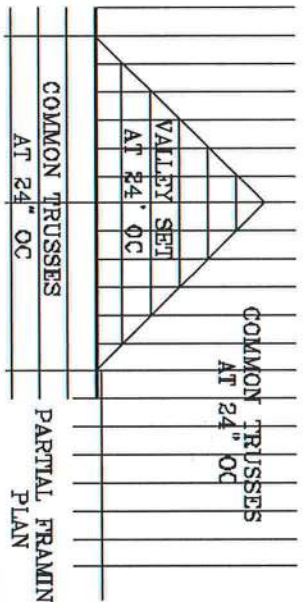
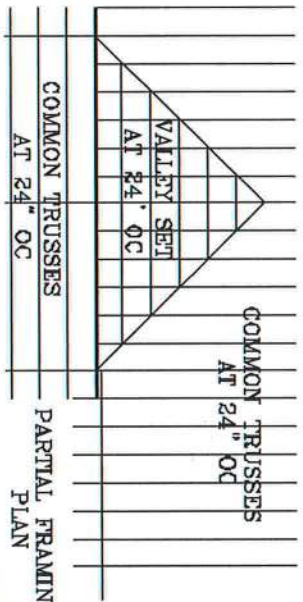
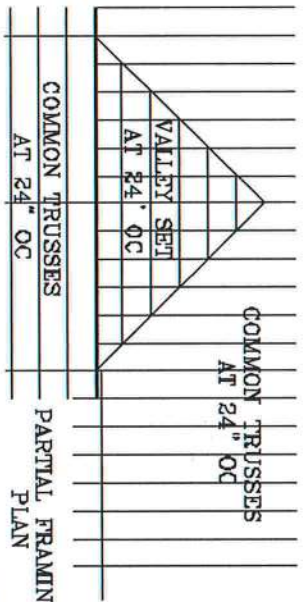
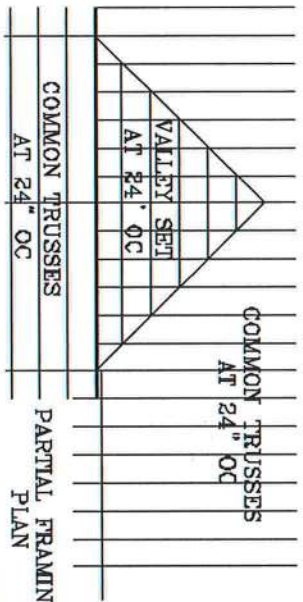
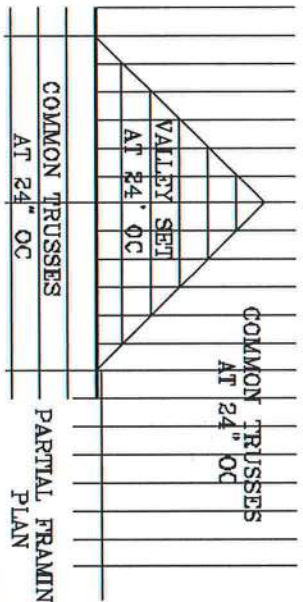
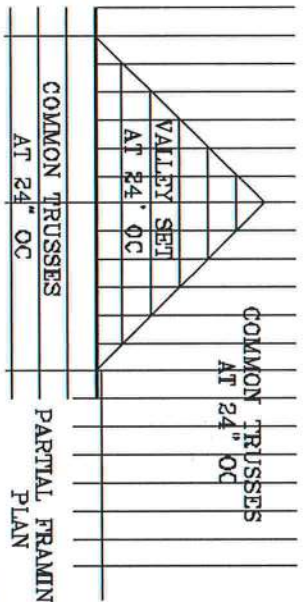
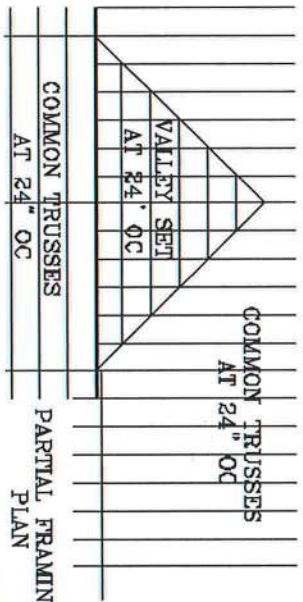
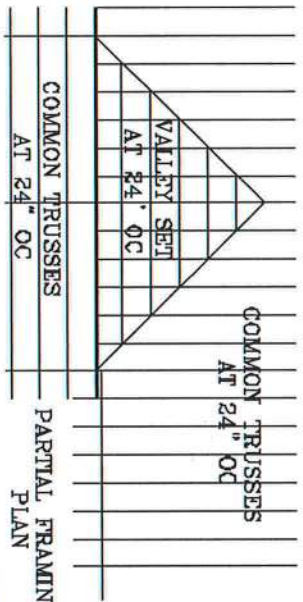
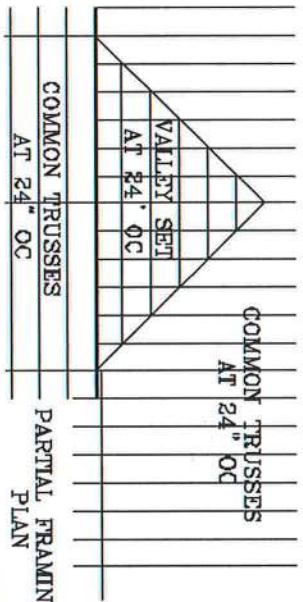
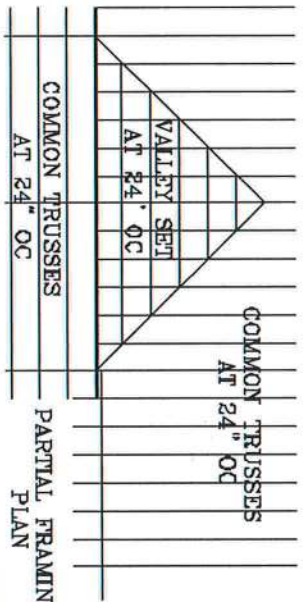
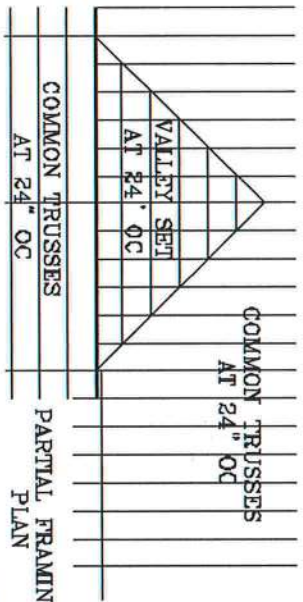
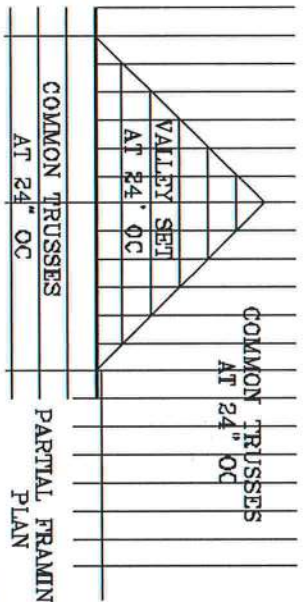
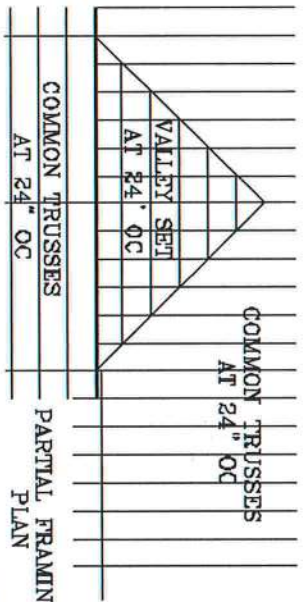
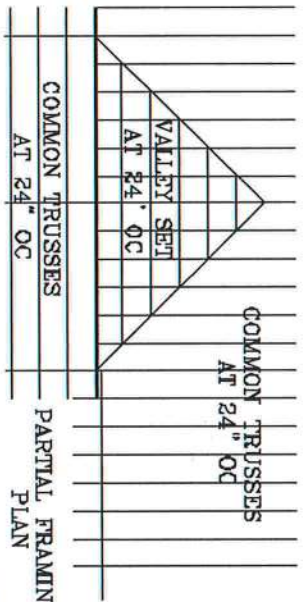
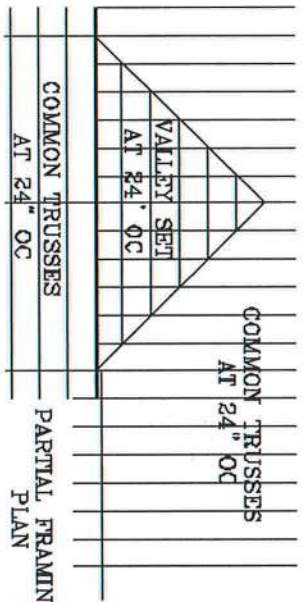
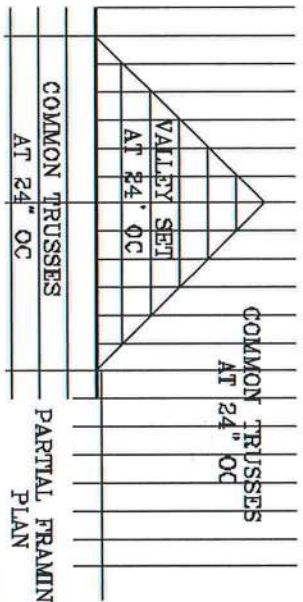
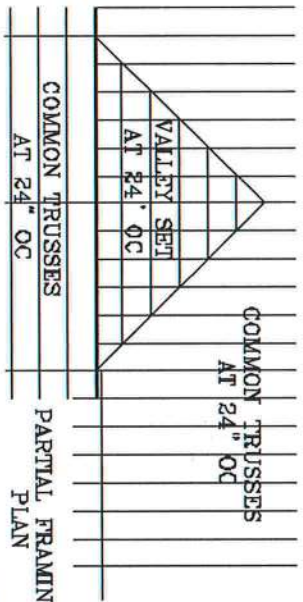
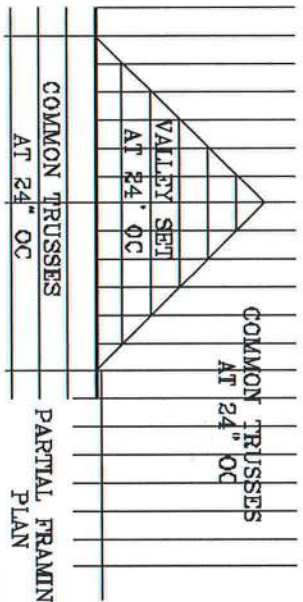
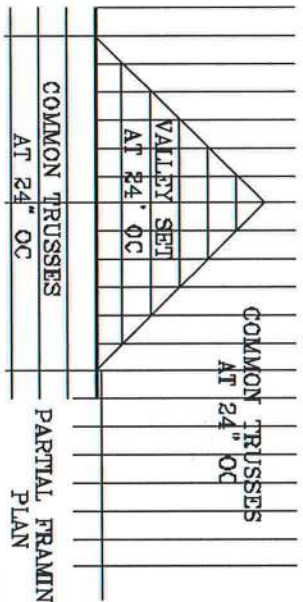
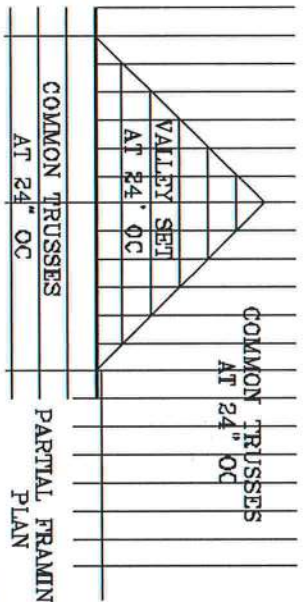
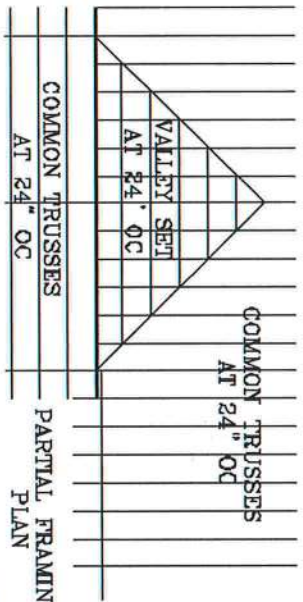
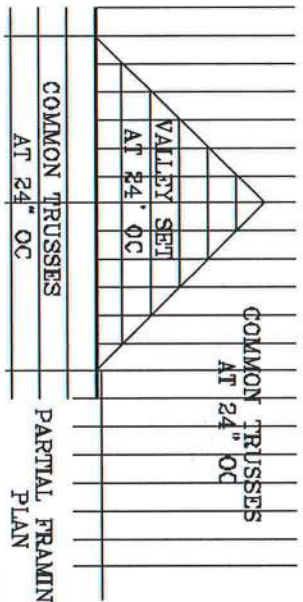
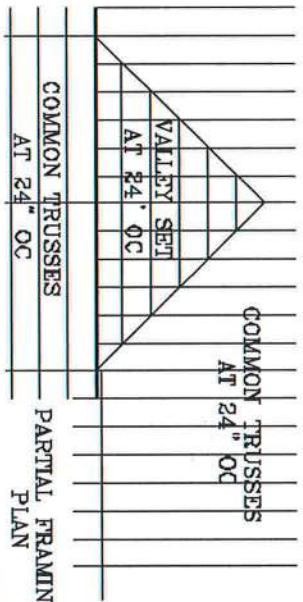
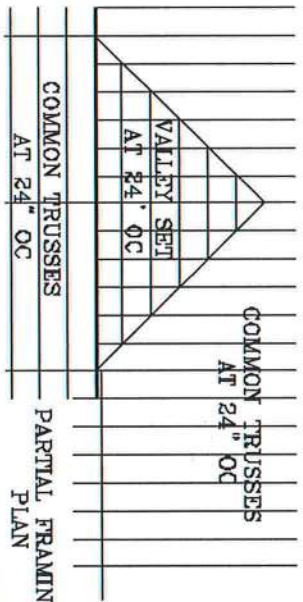
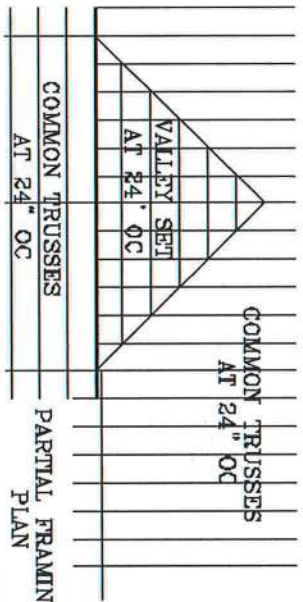
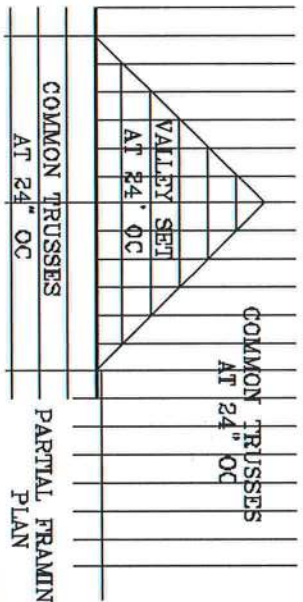
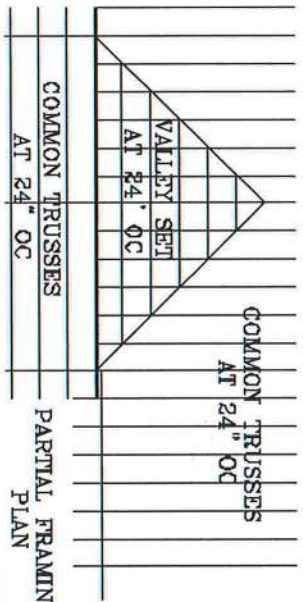
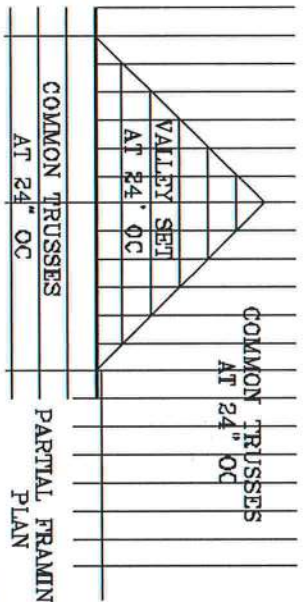
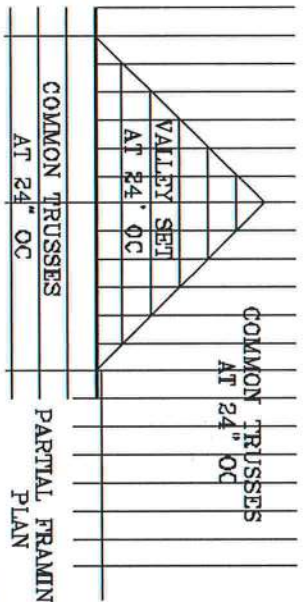
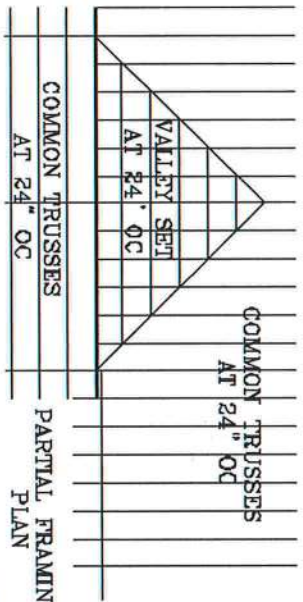
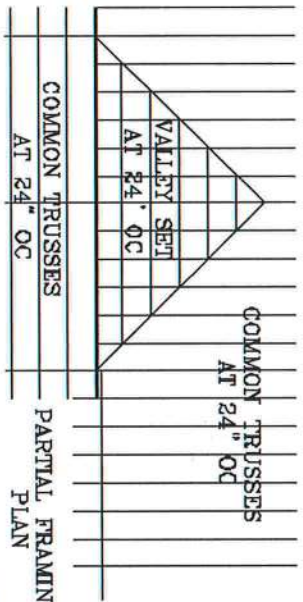
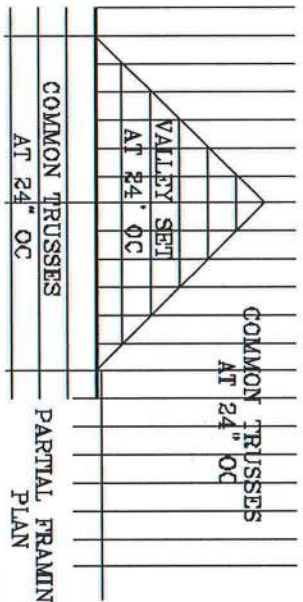
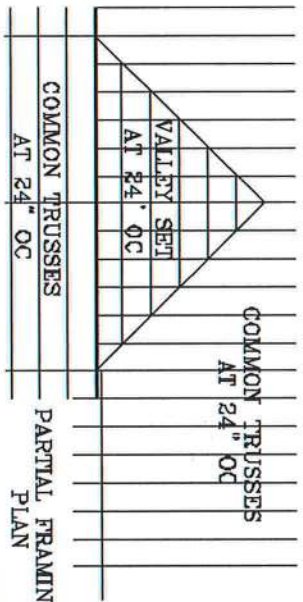
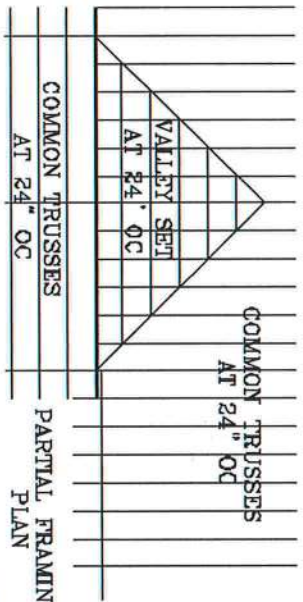
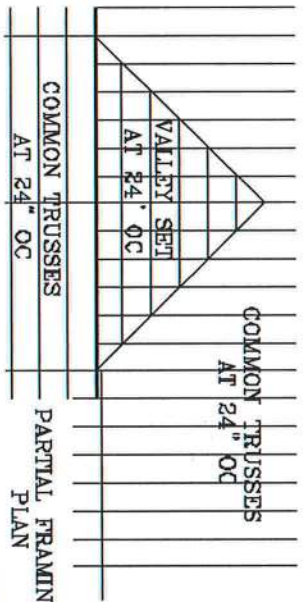
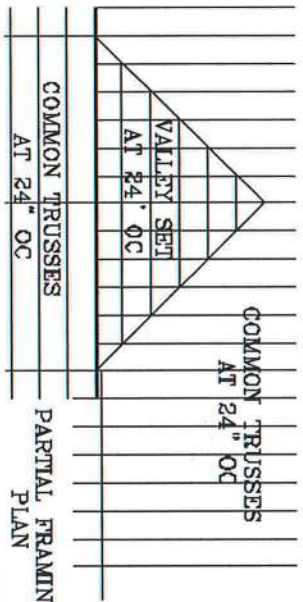
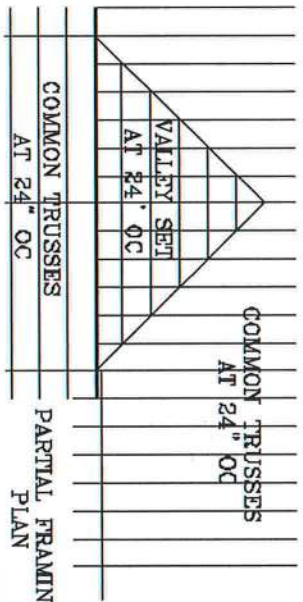
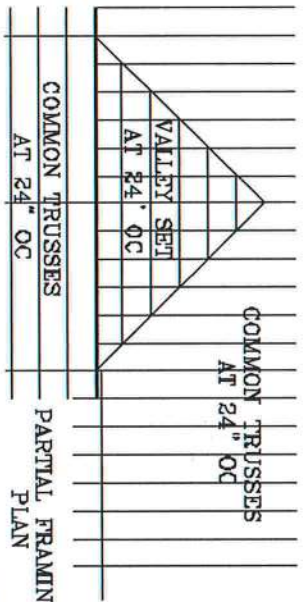
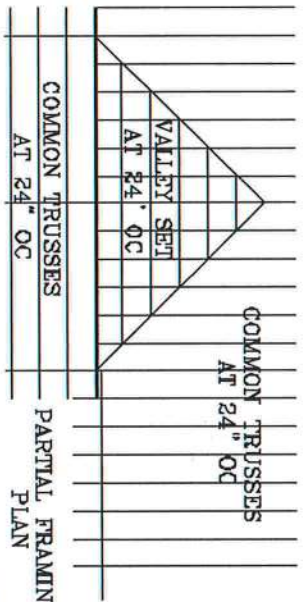
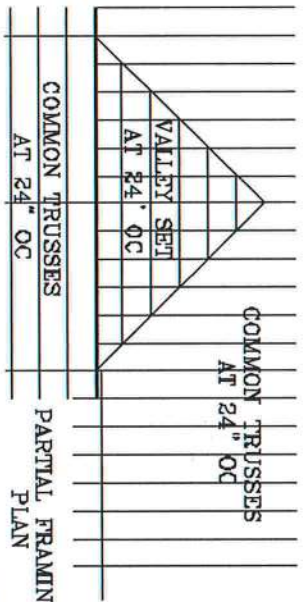
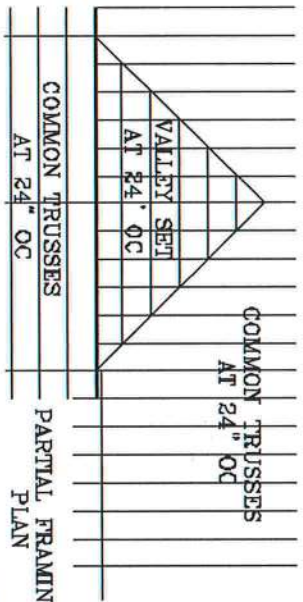
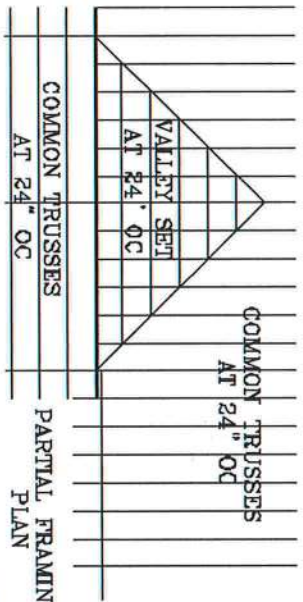
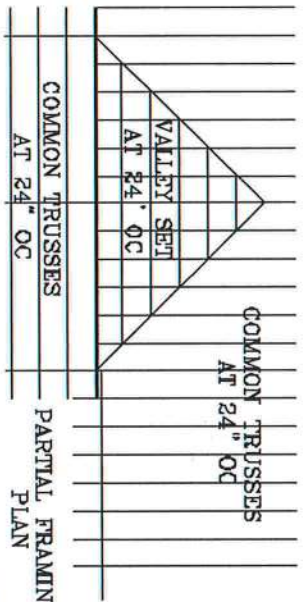
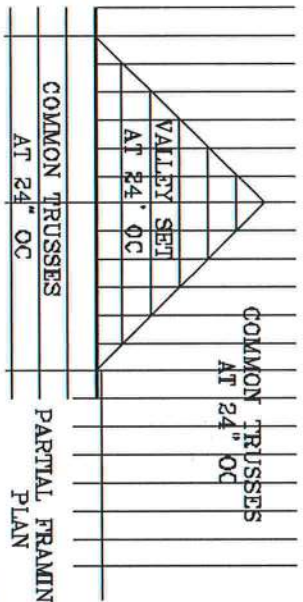
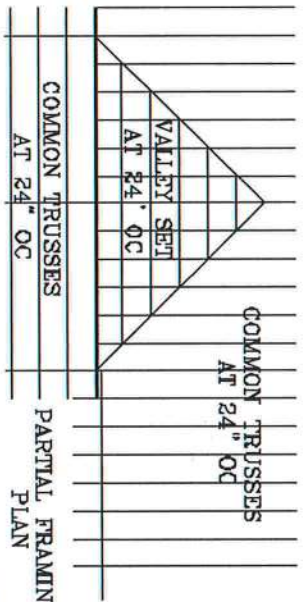
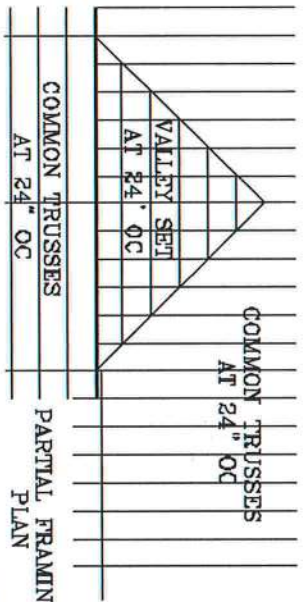
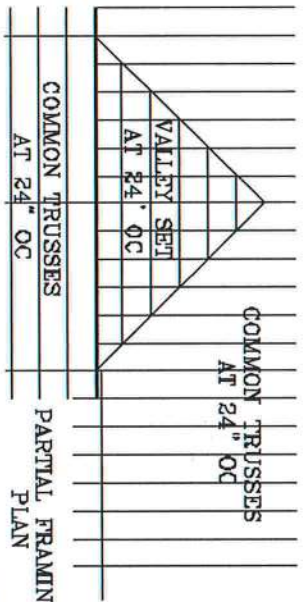
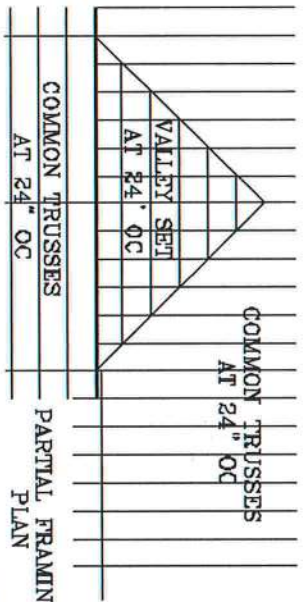
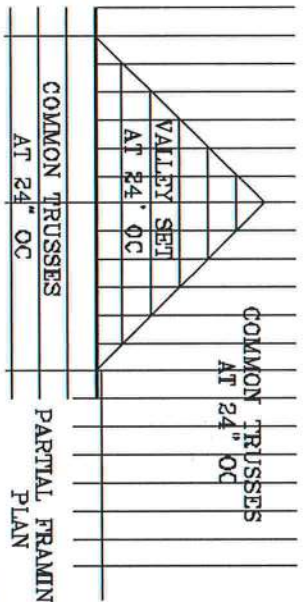
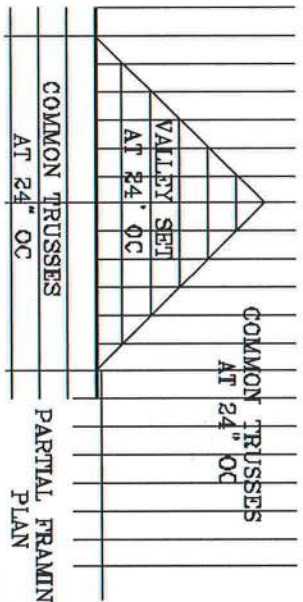
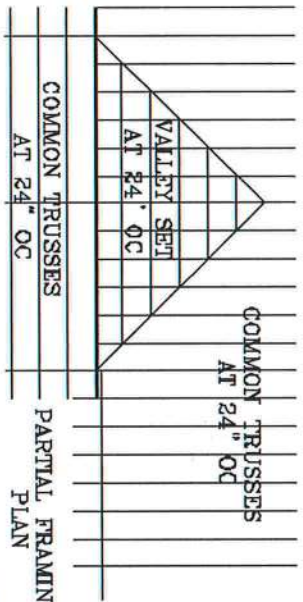
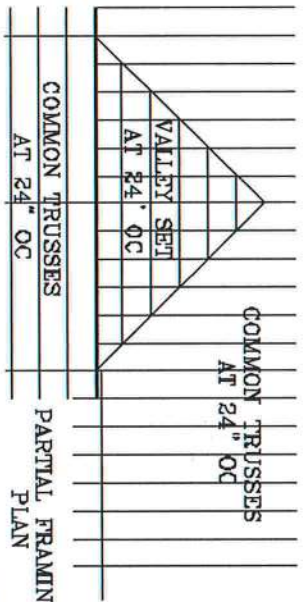
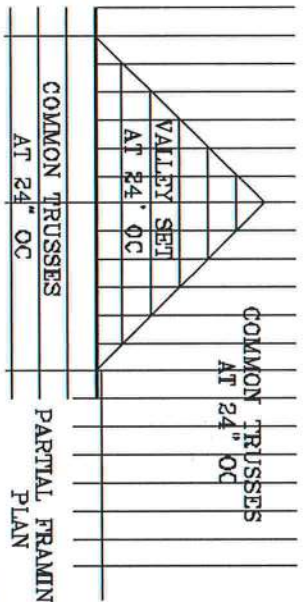
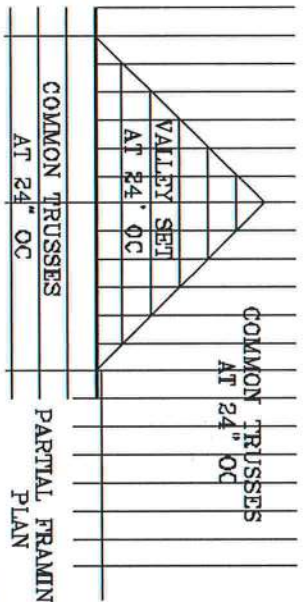
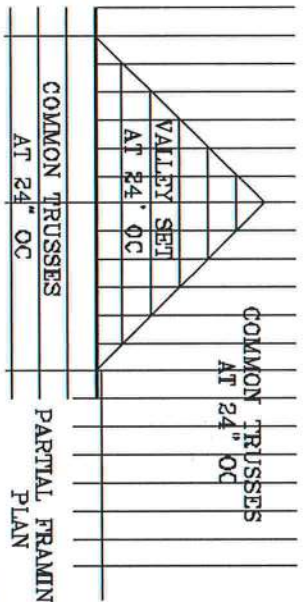
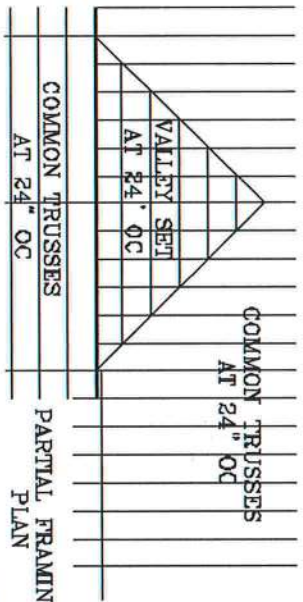
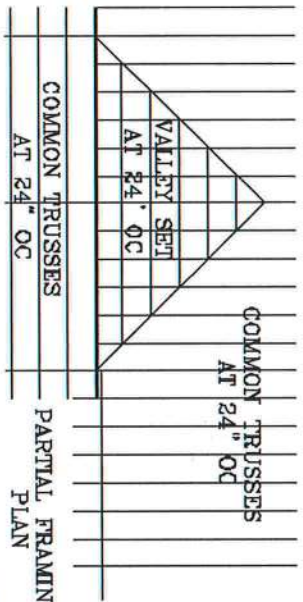
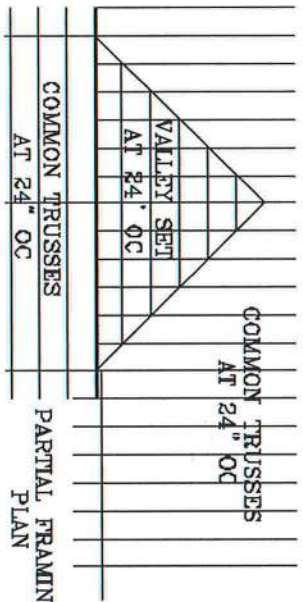
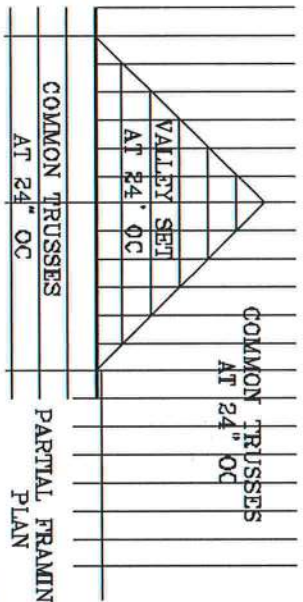
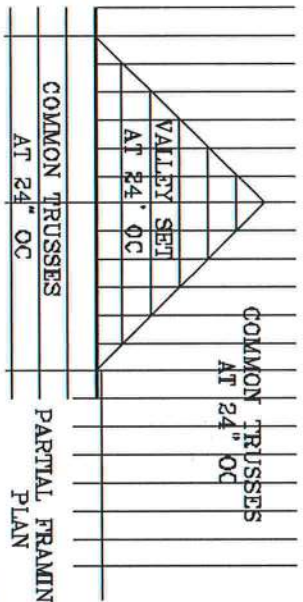
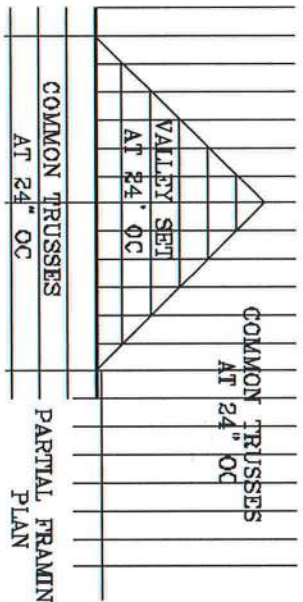
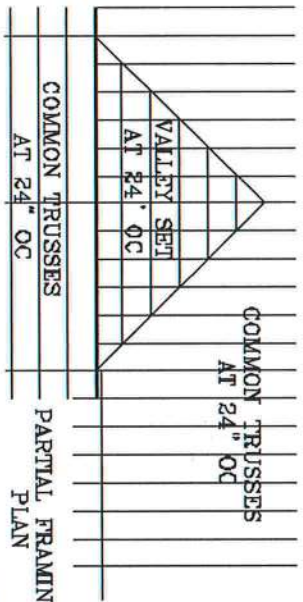
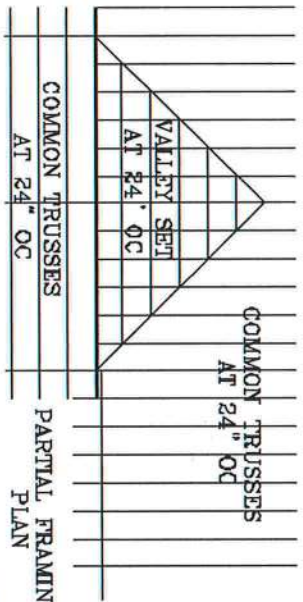
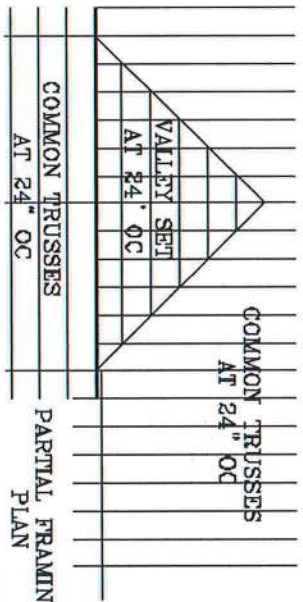
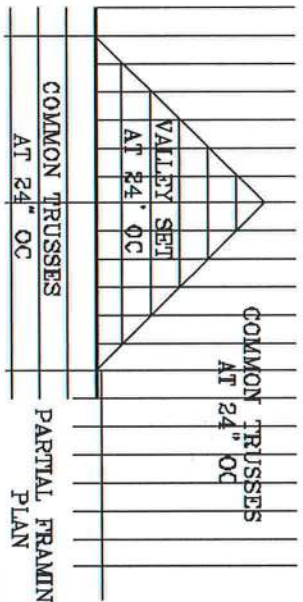
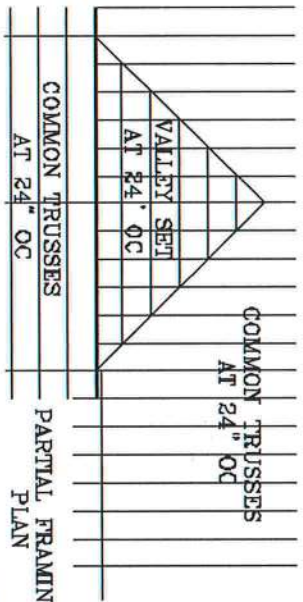
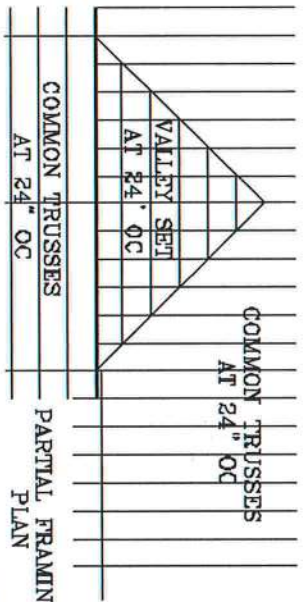
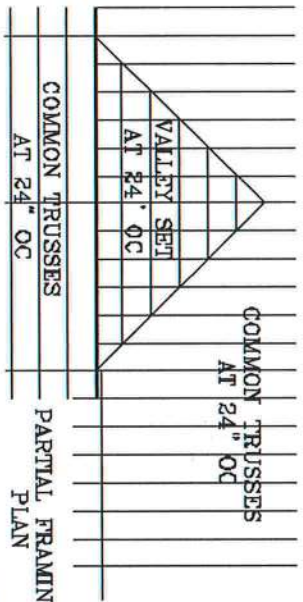
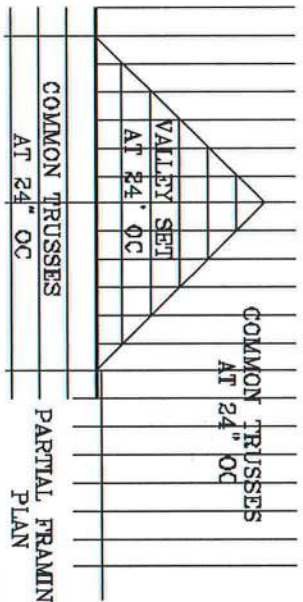
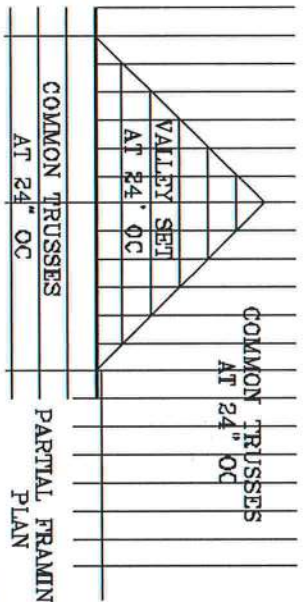
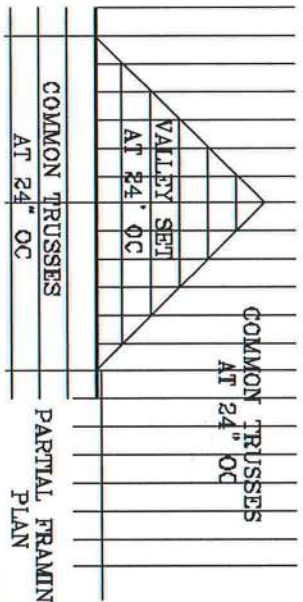
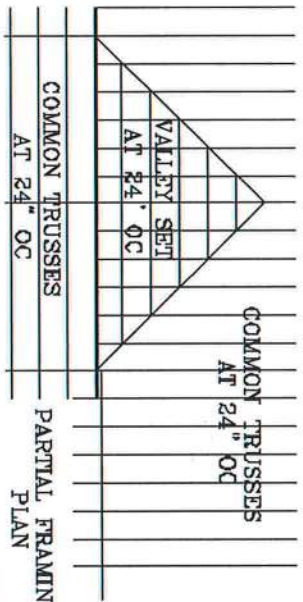
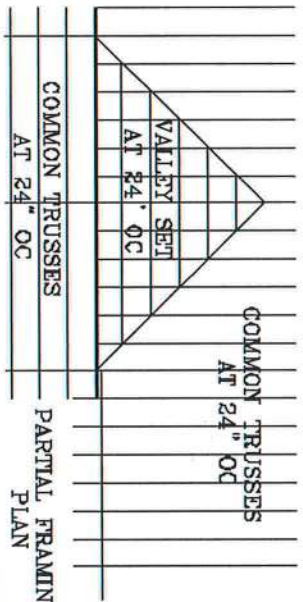
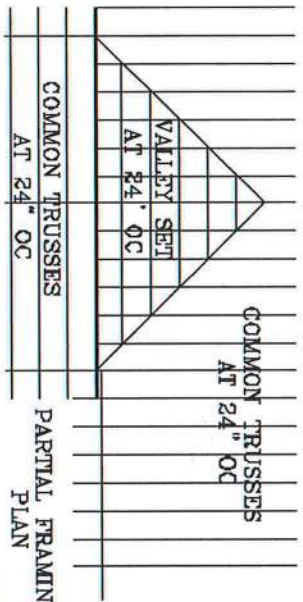
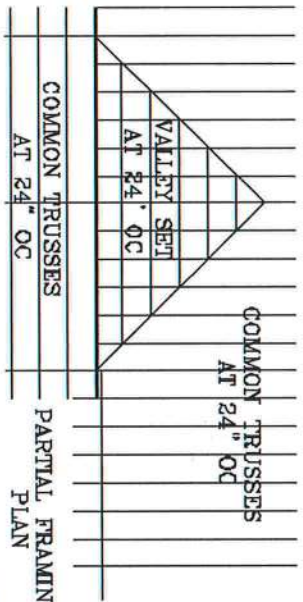
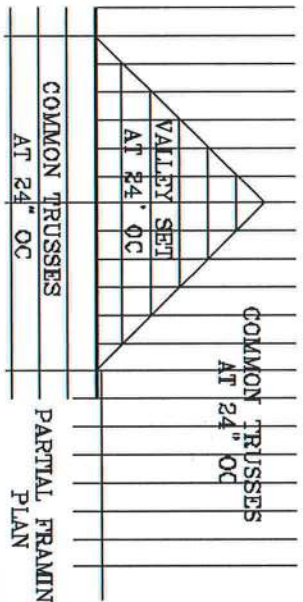
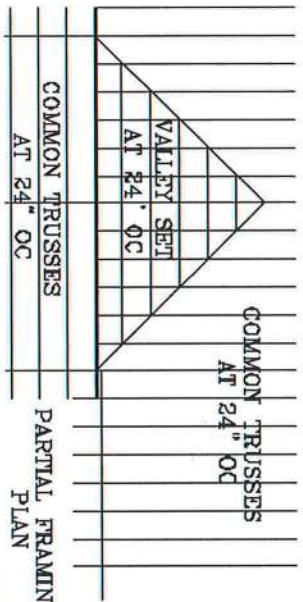
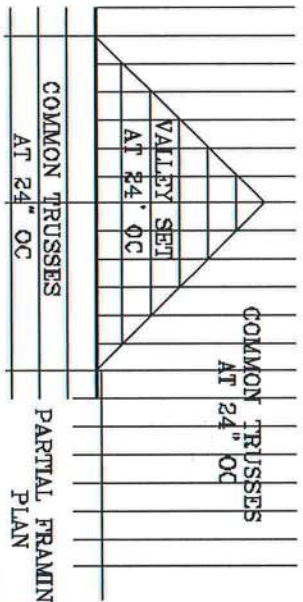
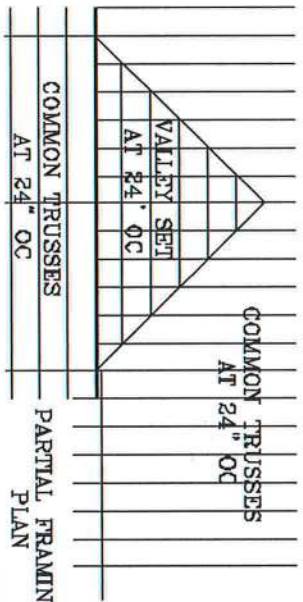
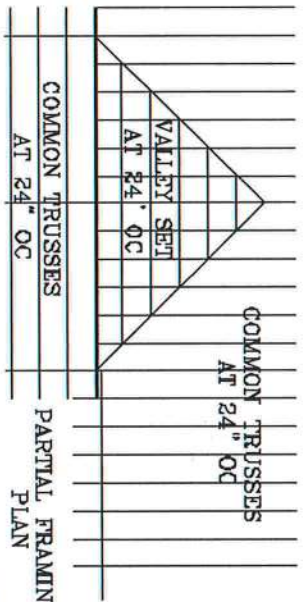
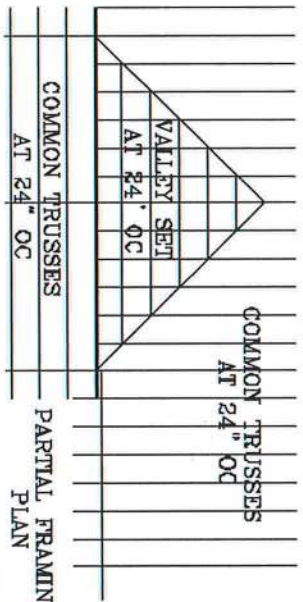
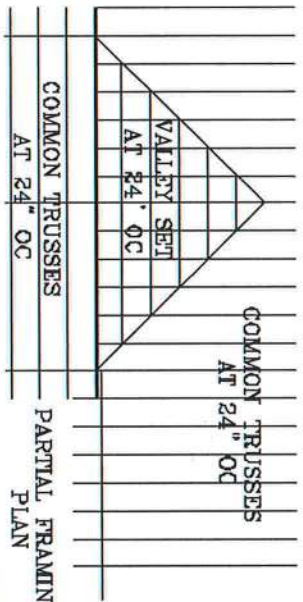
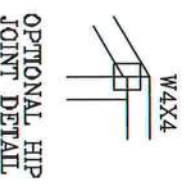
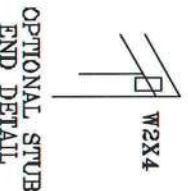
TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.



*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

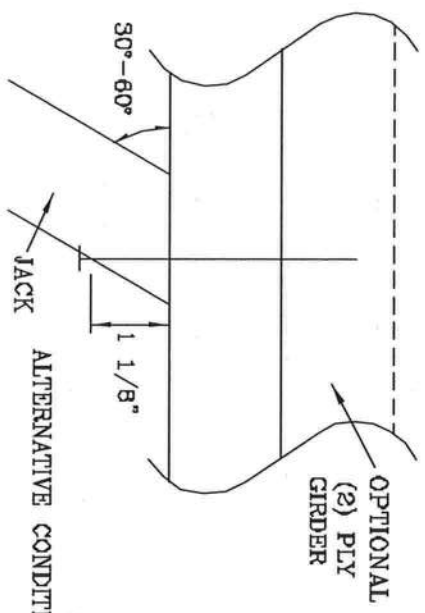
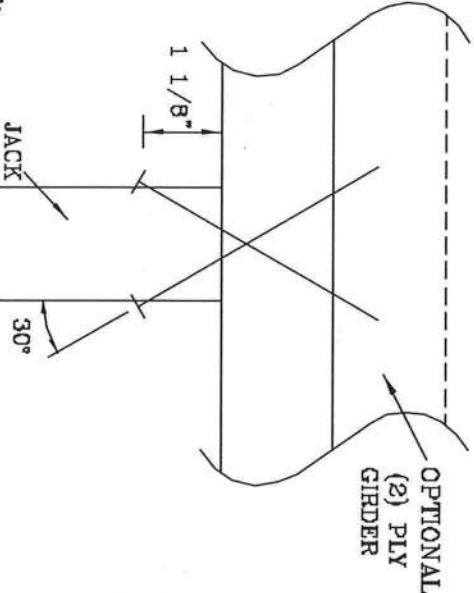
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

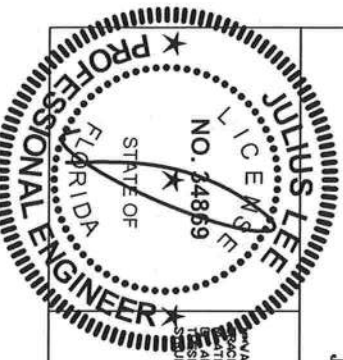
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	187#	256#	181#	234#	156#	203#	154#	189#
3	298#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES GUIDELINES FOR TRUSS CONSTRUCTION. FOR LUMBER TRUSSES, CONSULT THE MANUFACTURER'S INSTRUCTIONS. FOR STEEL TRUSSES, CONSULT THE MANUFACTURER'S INSTRUCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1405 ST. 4TH AVENUE
DELRAY BEACH, FL 33445-2161

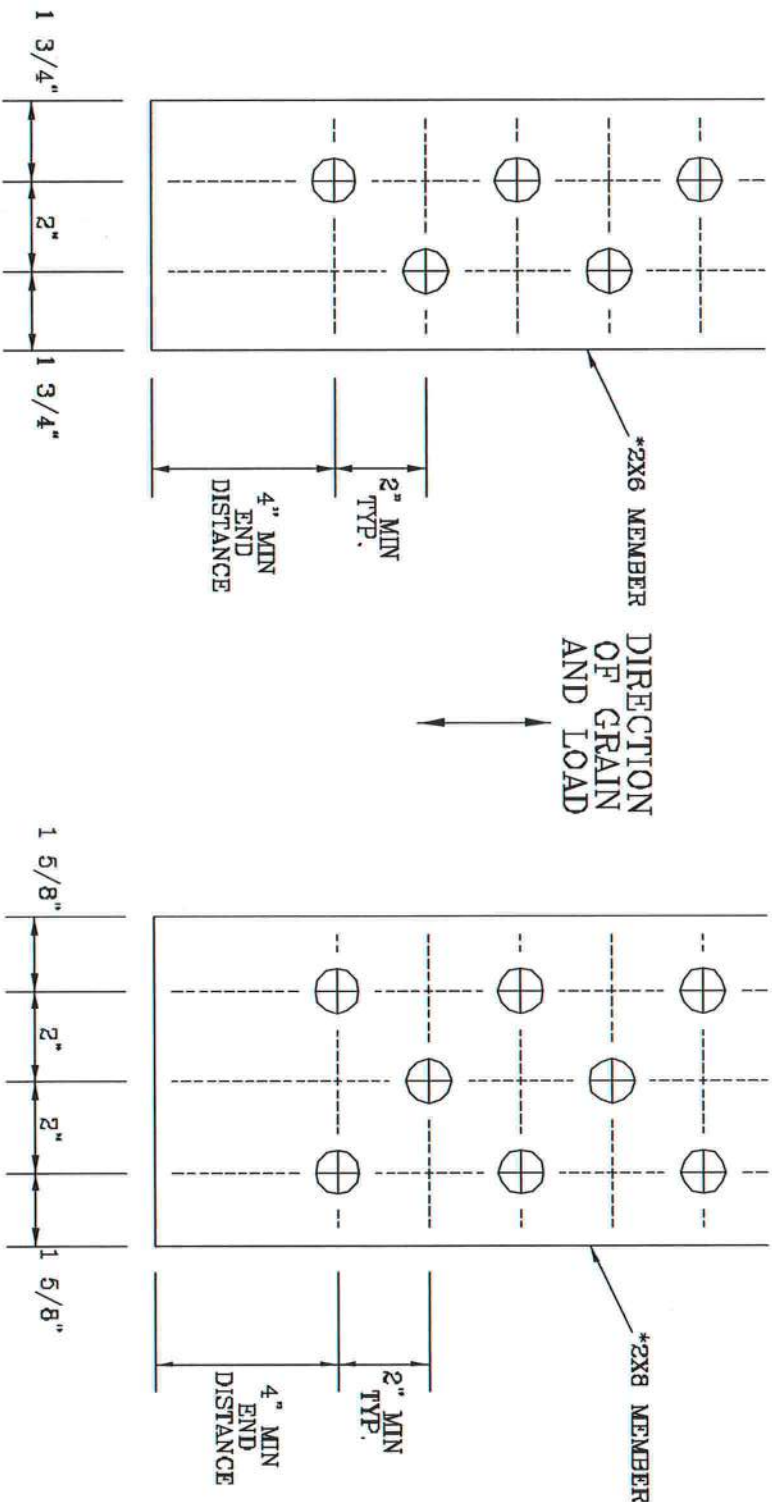
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT

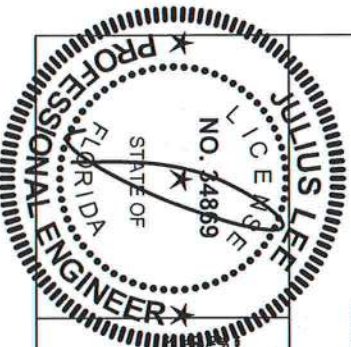


2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTAINING. SEE THE ALPINE DESIGN FOR DETAILED INFORMATION. PUBLISHED BY THE TRUSS ASSOCIATION, 380 DOWNSIDE DR., SUITE 200, MADISON, VT. 05759 AND WTC COUNCIL, AMERICA, 6300 ENTERPRISE LN, MADISON, VT. 05759 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 17th Avenue
Deerfield Beach, FL 33441-2161

No: 34869
STATE OF FLORIDA

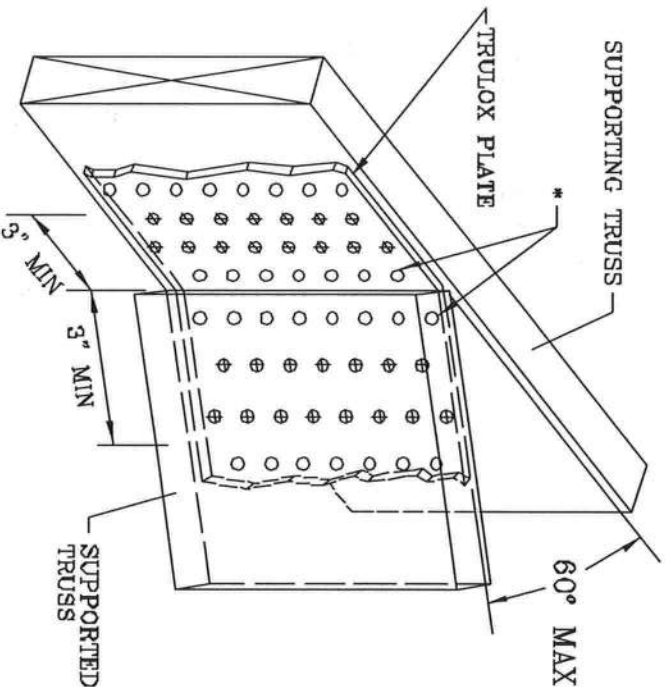
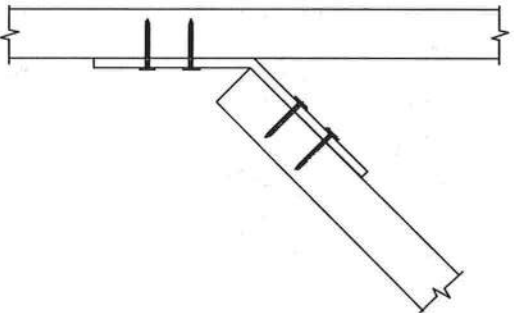
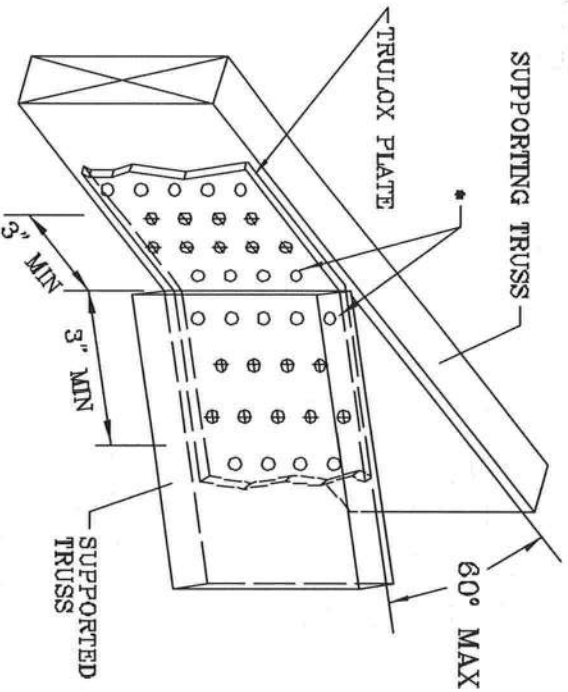
TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FULL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS. THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS. REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



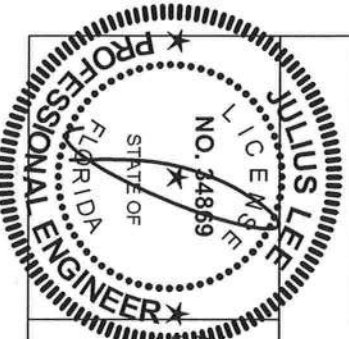
TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By Julius Lee at 11:56 am, Jun 11, 2008

MINIMUM 3X6 TRULOX PLATE

MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R
1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2031-1-03 BUILDING DEPARTMENT SAFETY INFORMATION PUBLISHED BY THE TRUSS MANUFACTURING INSTITUTE, 3661 POWERS DR., SUITE 200, MARLBOROUGH, VT 05759 AND VITA (VITA TRUSS CONSULTING) 1000 W. 10TH AVE., SUITE 100, DENVER, CO 80202 FOR TRUSS CONSTRUCTION DETAILS. TRUSSES SHOWN IN THIS DRAWING ARE FOR INFORMATION ONLY. TRUSSES SHOWN IN THIS DRAWING ARE NOT TO BE USED FOR ANY OTHER PURPOSES. TRUSSES SHOWN IN THIS DRAWING ARE NOT TO BE USED FOR ANY OTHER PURPOSES. TRUSSES SHOWN IN THIS DRAWING ARE NOT TO BE USED FOR ANY OTHER PURPOSES.

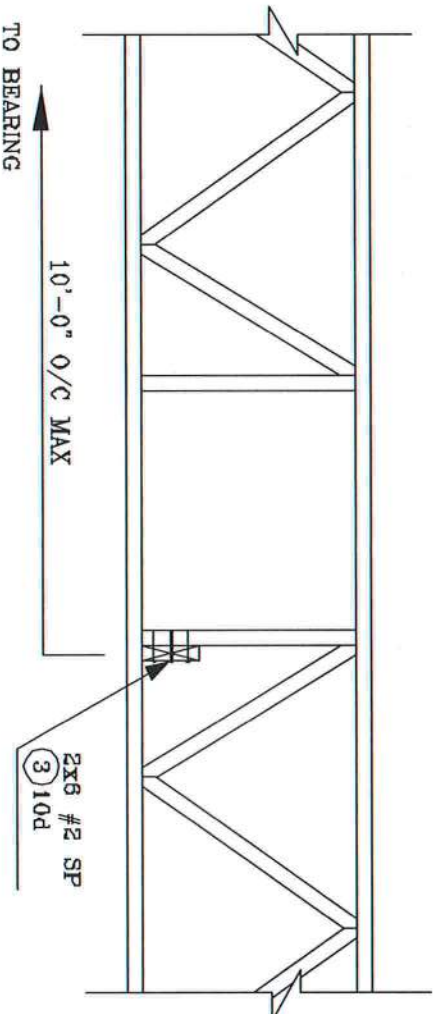
JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DEALAT BEACH, FL 33444-2181

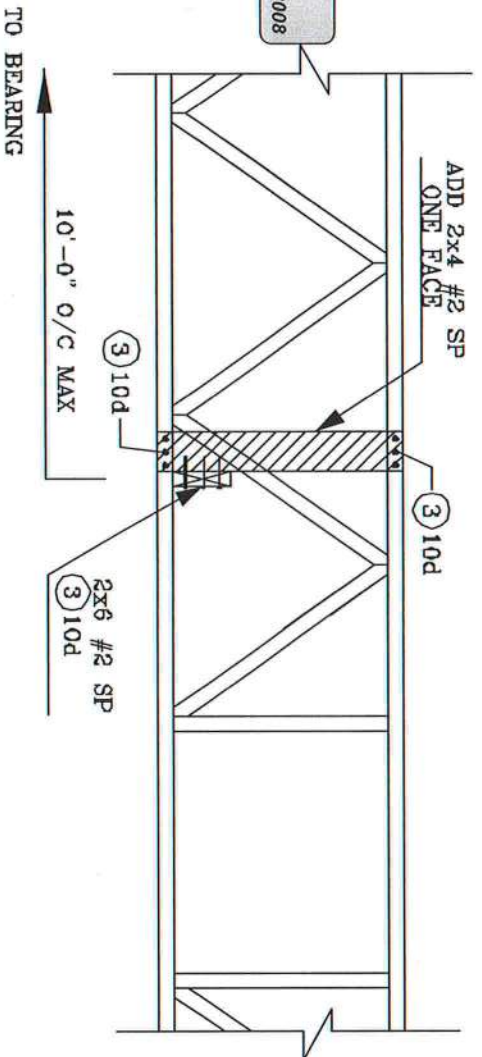
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

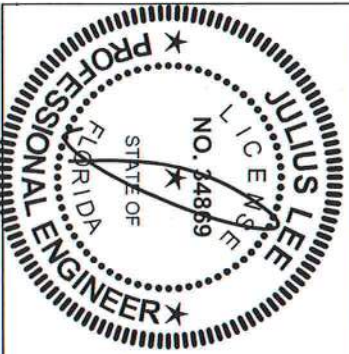
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

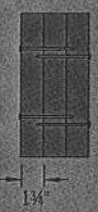
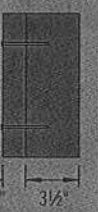
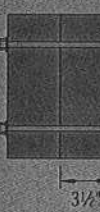


JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DIERLY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

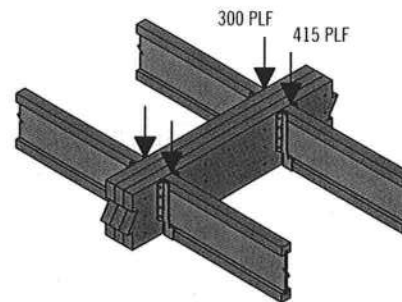
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 1/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

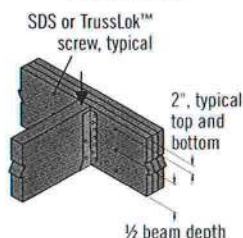
(2) 6" long screws required.

(3) 5" long screws required.

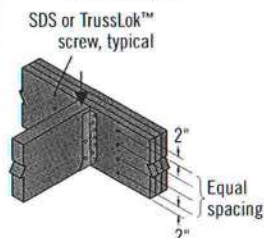
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

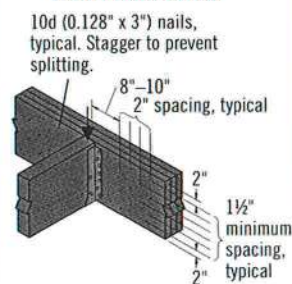
4 or 6 or Screw Connection



8 Screw Connection

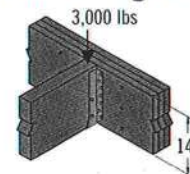


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 1/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 1/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

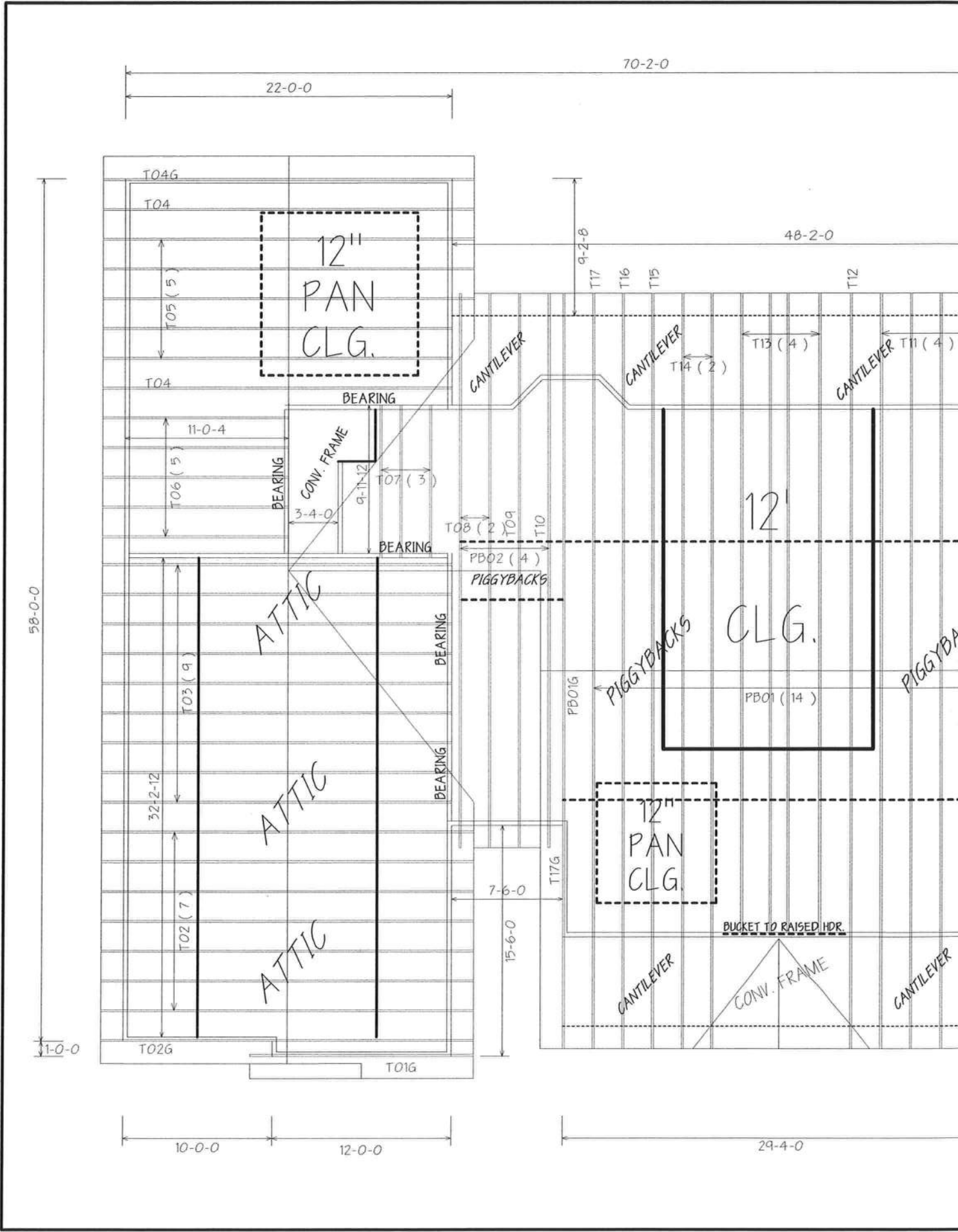
3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

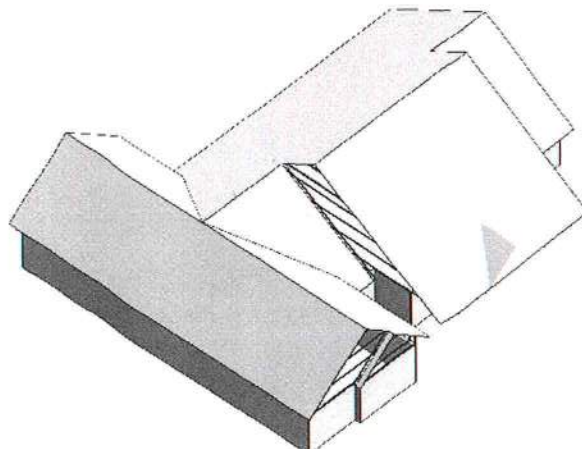
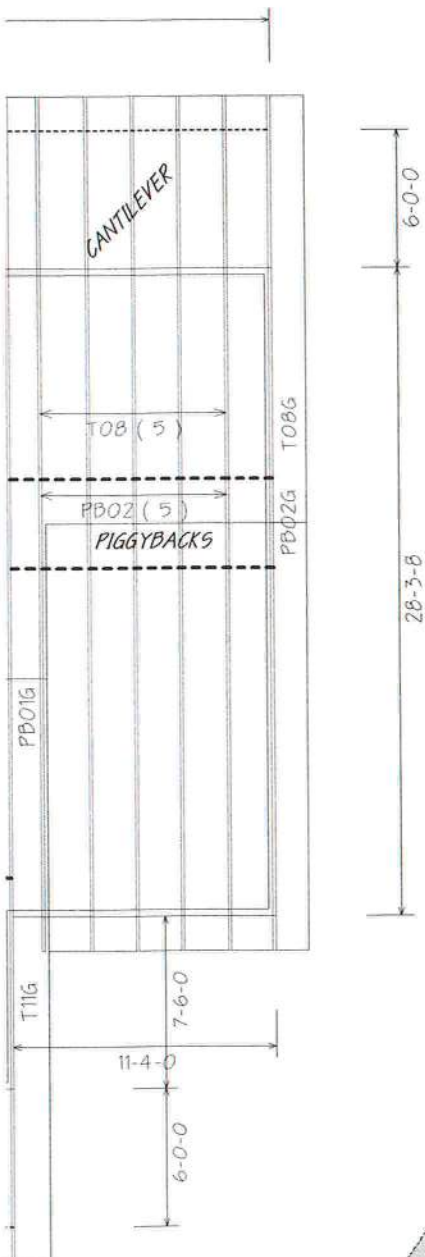
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"



8/12 PITCH HOUSE
10/12 GARAGE/MST BDRM
1'-6" O/H



BEARING HEIGHT SCHEDULE

9'-1 1/8"

HANGER SCHEDULE 4 - HTU26

NOTES:

- 1) REFER TO HD 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V109 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) BEAM/HEADER/INTEL (HOR) TO BE FURNISHED BY BUILDER.



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BUILDER:

RICHARD KEEN

LEGAL ADDRESS:

LAMBERT RES.

MODEL:

CUSTOM

Revision:

Rev. By:

DATE:

11-5-09

DRAWN BY:

K.L.H.

Original Reference #:

318492

1st Level Job#:

2nd Level Job#:

Roof Job#: