DATE 10/20/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025146

APPLICANT SCOTT ROSENBOOM PHONE 352 538-3877

ADDRESS 14802 NW 190TH AVE HIGH SPRINGS FL 32643

OWNER MICHAEL MCALHANY PHONE 454-8848

ADDRESS 420 SW MARYNIK DRIVE HIGH SPRINGS FL 32643

CONTRACTOR SCOTT ROSENBOOM PHONE 352 538-3877

LOCATION OF PROPERTY 441S, TO CR 778, CORNER OF 441S AND CR 778, 1ST
ENTRANCE OFF OF CR 778, TR, 3/4 OF THE WAY DOWN

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 125400.00

HEATED FLOOR AREA 2508.00 TOTAL AREA 3874.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 16

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 16-7S-17-10006-212 SUBDIVISION RIVER RISE

LOT 12 BLOCK PHASE UNIT TOTAL ACRES 5.00

000001245		CRC057796	Scott Rosenboom	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
CULVERT	06-0883-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 8483

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 630.00 CERTIFICATION FEE $ 19.37 SURCHARGE FEE $ 19.37

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 768.74

INSPECTORS OFFICE Hale Tedder CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

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This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

After Recording Return To:
COUNTRYWIDE HOME LOANS, INC.
MS SV-79 DOCUMENT PROCESSING
P.O.Box 10423
Van Nuys, CA 91410-0423

This document was prepared by:
ROBERT CASTILLO
COUNTRYWIDE HOME LOANS, INC.

6400 LEGACY DR
PLANO, TX 75024

Inst:200623486 Date:10/02/2006 Time:10:38
DC,P.DeWitt Cason,Columbia County B:1097 P:2274

[Space Above This Line For Recording Data]

144541522
[Loan #]

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: (legal description of property, and street address if available)

Tax Parcel #16-7S-17-10006-212

Lot 12 of RIVER RISE RESIDENTIAL SUBDIVISION, UNIT 1, according to the Plat thereof as recorded in Plat Book 8, Pages 51 through 53, of the Public Records of Columbia County, Florida.

Parcel ID Number: 16-7S-17-10006-212

2. General description of improvement: Residential

3. Owner information:
   a. Name and address: Michael J. McAlhany and Patricia S. McAlhany
   25103 NW 122nd Avenue, High Springs, FL 32643
   b. Interest in property: Fee Simple
   c. Name and address of fee simple titleholder (if other than owner): Not Applicable

Initials:

23991

1445415220000002D537

Record & Return To:
Darryl J. Tompkins, P.A.
P.O. Box 519
Alachua, FL 32616

Parcel ID Number: 16-7s-17-10006-001 Portion of

Inst:2006006514 Date:03/16/2006 Time:12:35
Doc Stamp-Deed : 543.90
DC,P.DeWitt Cason,Columbia County B:1077 P:146

Warranty Deed

This Indenture, Made this 7th day of MARCH, 2006 A.D., **Between**

Nevin G. Summers, a married man

of the Borough of Anchorage, State of Alaska, **Grantor**, and

Michael J. McAlhany and Patricia S. McAlhany, husband and wife

whose post office address is : 25103 NW 122nd Avenue, High Springs, Florida 32643

of the County of Alachua, State of Florida, **Grantee**

Witnesseth that the GRANTOR, for and in consideration of the sum of TEN & NO/100 ($10.00), and other good and valuable consideration to GRANTOR in hand paid by GRANTEE, the receipt of which is hereby acknowledged, has granted, bargained and sold to the said GRANTEE and GRANTEE'S successors and assigns forever, the following described land, situate, lying and being in the County of Columbia, State of Florida to wit:

LOT 12, RIVER RISE RESIDENTIAL SUBDIVISION, UNIT 1, A SUBDIVISION ACCORDING TO PLAT THEREOF RECORDED IN PLAT BOOK 8, PAGES 51 THROUGH 53 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

SUBJECT TO THE FOLLOWING:

A. Zoning restrictions, prohibitions and other requirements imposed by governmental authority;
B. Restrictions and matters appearing on the plat and/or common to the subdivision;
C. Taxes for the year 2006 and subsequent years.

The land described herein is not the homestead of the grantor(s), and neither the grantor(s) nor the grantor(s) spouse, nor anyone for whose support the grantor(s) is responsible, resides on or adjacent to said land

and the grantor does hereby fully warrant the title to said land, and will defend the same against lawful claims of all persons whomsoever.

In Witness Whereof, the grantor has hereunto set his hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Kristine Gillham
Printed Name: Kristine Gillham

Danielle Tucker
Printed Name: Danielle Tucker

NEVIN G. SUMMERS

STATE OF ALASKA
BOROUGH OF Kenai Peninsula

The foregoing instrument was acknowledged before me this 7th day of March, 2006, by NEVIN G. SUMMERS, who is personally known to me or has produced his AK Drivers License as identification.

DANIELLE TUCKER
My Commission Expires
NOTARY PUBLIC
July 9, 2009
STATE OF ALASKA

Danielle Tucker
Notary Public State of Alaska
Printed Name: Danielle Tucker
My Commission Expires: July 9, 2009

DATE 10/20/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
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APPLICANT SCOTT ROSENBOOM PHONE 352 538-3877

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LOCATION OF PROPERTY 441S, TO CR 778, CORNER OF 441S AND CR 778, 1ST
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TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 125400.00

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000001245	____	CRC057796	Scott Rosenboom	
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Check # or Cash 8483

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Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

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INSPECTORS OFFICE Gale Eddy CLERKS OFFICE CH

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This document was prepared by:
ROBERT CASTILLO
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6400 LEGACY DR
PLANO, TX 75024

Inst:200623486 Date:10/02/2006 Time:10:38
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   a. Name and address: Michael J. McAlhany and Patricia S. McAlhany
   25103 NW 122nd Avenue, High Springs, FL 32643
   b. Interest in property: Fee Simple
   c. Name and address of fee simple titleholder (if other than owner): Not Applicable

Witness:

23991

144841522000002D537

Record & Return To:
Darryl J. Tompkins, P.A.
P.O. Box 519
Alachua, FL 32616

Parcel ID Number: 16-7s-17-10006-001 Portion of

Inst:2006006514 Date:03/16/2006 Time:12:35
Doc Stamp-Deed : 543.90
S-2 DC,P.DeWitt Cason,Columbia County B:1077 P:146

Warranty Deed

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Nevin G. Summers, a married man

of the Borough of Anchorage, State of Alaska, **Grantor**, and

Michael J. McAlhany and Patricia S. McAlhany, husband and wife

whose post office address is : 25103 NW 122nd Avenue, High Springs, Florida 32643

of the County of Alachua, State of Florida, **Grantee**

Witnesseth that the GRANTOR, for and in consideration of the sum of TEN & NO/100 ($10.00), and other good and valuable consideration to GRANTOR in hand paid by GRANTEE, the receipt of which is hereby acknowledged, has granted, bargained and sold to the said GRANTEE and GRANTEE'S successors and assigns forever, the following described land, situate, lying and being in the County of Columbia, State of Florida to wit:

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and the grantor does hereby fully warrant the title to said land, and will defend the same against lawful claims of all persons whomsoever.

In Witness Whereof, the grantor has hereunto set his hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Kristine Gillham
Printed Name: Kristine

Danielle Tucker
Printed Name: Danielle Tucker

NEVIN G. SUMMERS

STATE OF ALASKA
BOROUGH OF Kenai Peninsula

The foregoing instrument was acknowledged before me this 7th day of March, 2006, by NEVIN G. SUMMERS, who is personally known to me or has produced his AK Drivers License as identification.

DANIELLE TUCKER My Commission Expires NOTARY PUBLIC July 9, 2009 STATE OF ALASKA

Danielle Tucker
Notary Public State of Alaska
Printed Name: Danielle Tucker
My Commission Expires: July 9, 2009

One copy of windows & Door specs they are the same for the SFD and the Garage.

NEED copy of Deed

Columbia County Building Permit Application

For Office Use Only Application # 0610-37 Date Received 10/11/06 By LH Permit # 25146/245
Application Approved by - Zoning Official BLK Date 10.10.06 Plans Examiner OK JTH Date 10-19-06
Flood Zone X pw pkt Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
Comments
NOC EH Deed or PA Site Plan MA State Road Info Parent Parcel # Development Permit

Name Authorized Person Signing Permit Scott Rosenboom Phone 352-538-3877 Fax 386-454-2666
Address 14802 NW 190th AVE High Springs FL 32643
Owners Name MICHAEL McALHANY Phone 386-454-8848
911 Address 420 SW MARYNIK Drive, High Springs, FL 32643
Contractors Name Scott Rosenboom Phone 352-538-3877
Address 14802 NW 190th AVE High Springs FL 32643
X Fee Simple Owner Name & Address Michael J. & Patricia S McAlhany 25103 NW 122 Avenue, High Springs, FL 32643
Bonding Co. Name & Address
Architect/Engineer Name & Address MARK GSKRIDGE ALACHUA FL 32615
X Mortgage Lenders Name & Address Countrywide Home Loans Inc. 4505 La Paloma Ave, Calabrisas, CA 91302
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
Property ID Number 16-7S-17-10006-212 Estimated Cost of Construction 250,500.00
Subdivision Name Riven Rise Lot ___ Block ___ Unit 1 Phase ___
Driving Directions 441 South to 778 Corner of 778 + 441
1st entrance off 778, on (R) 3/4 of the way down.

Type of Construction FRAME Number of Existing Dwellings on Property 0
Total Acreage 5 Lot Size ___ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 210 Side 126 Side 125 Rear 357
Total Building Height 16.21 Number of Stories 1 Heated Floor Area 2508 TOTAL 3874 Roof Pitch 7/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

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Patricia S McAlhany
Owner Builder or Authorized Person by Notarized Letter

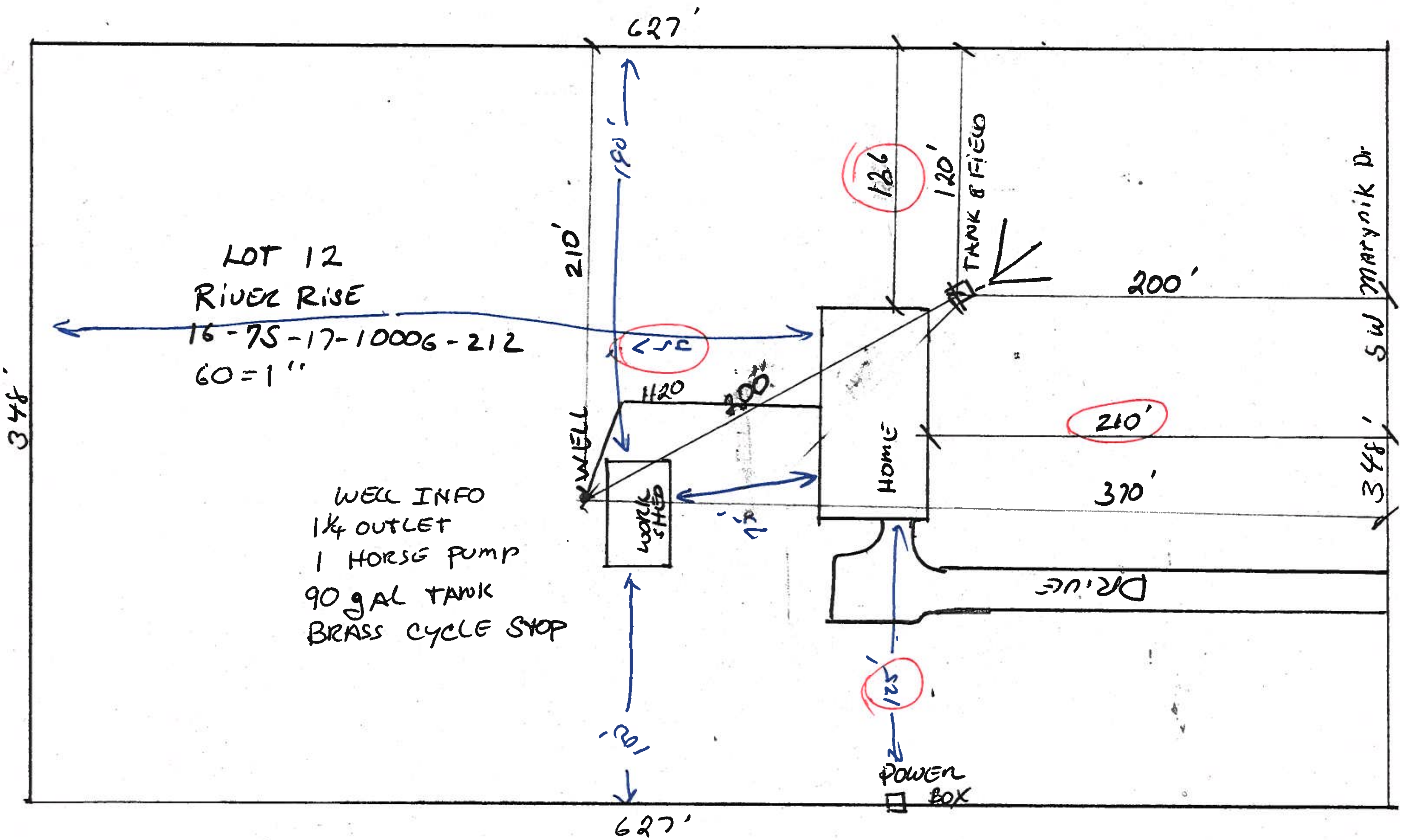
Scott Rosenboom
Contractor Signature
Contractors License Number CRC057786
Competency Card Number
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA
Sworn to (or affirmed) and subscribed before me this 10 day of October 20 06.
Personally known X or Produced Identification X

Jeff Wolfe
Commission # DD285878
Expires January 28, 2008
Bonded Thru Atlantic Bonding Insurance, Inc. 800-385-7019

Notary Signature

(Revised Sept. 2006)



627'
LOT 12
River Rise
16-75-17-1000G-212
60=1"
348'
210'
190'
126
120'
TANK & FIELD
200'
WELL
1120
200'
HOME
210'
370'
348'
SW Marynik Dr
WELL INFO
1¼ OUTLET
1 HORSE PUMP
90 gAL TANK
BRASS CYCLE STOP
WORK SHED
DRIVE
125'
POWER BOX
627'

STATE OF FLORIDA
DEPARTMENT OF HEALTH

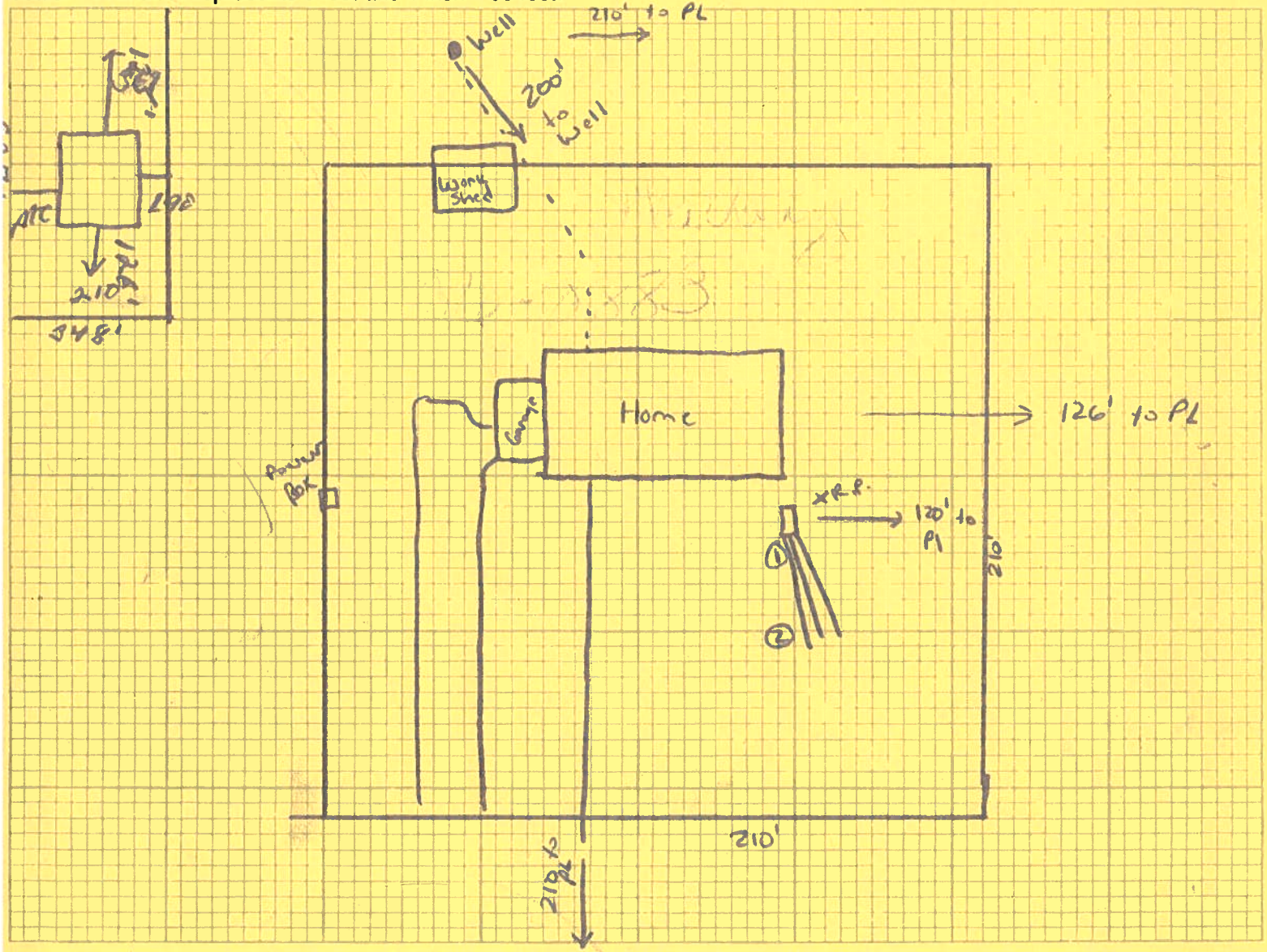
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

10/2/06

Permit Application Number 06-0883U

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: OK per MSC

3 trees

Keep system high due to possible I.S. in area

Site Plan submitted by: Manuel Bernard (Signature) Agent (Title)

Plan Approved ✓ Not Approved _____ Date 10/9/06

By _____ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001245

DATE 10/20/2006 PARCEL ID # 16-7S-17-10006-212

APPLICANT SCOTT ROSENBOOM PHONE 352 538-3877

ADDRESS 14802 NW 190TH AVE HIGH SPRINGS FL 32643

OWNER MICHAEL MCALHANY PHONE 454-8848

ADDRESS 420 SW MARYNIK DRIVE HIGH SPRINGS FL 32643

CONTRACTOR SCOTT ROSENBOOM PHONE 352 538-3877

LOCATION OF PROPERTY 441S, TO 778, CORNER OF 441S AND CR 778, 1ST ENTRANCE,

ON RIGHT, 3/4 OF WAY DOWN

SUBDIVISION/LOT/BLOCK/PHASE/UNIT RIVER RISE 12

SIGNATURE Scott Rosenboom

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Rosenboom- McAlhany Res.**	Builder:	Rosenboom
Address:		Permitting Office:	Columbia
City, State:	,	Permit Number:	25146
Owner:	**Rosenboom Construction**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2508 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	Description Area 7a. (Dble Default) 241.0 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 241.0 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 213.3(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1523.0 ft²	
b.	Frame, Wood, Adjacent	R=13.0, 184.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1409.0 ft²	
b.	Under Attic	R=30.0, 1145.0 ft²	
c.	Under Attic	R=19.0, 138.0 ft²	
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 186.0 ft	
b.	N/A		
12.	Cooling systems		
a.	Central Unit	Cap: 60.0 kBtu/hr SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 60.0 kBtu/hr HSPF: 8.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.89	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.10

Total as-built points: 29277
Total base points: 33704

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 10/24/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2508.0	20.04	9046.9	Double, Clear	E	0.0	0.0	110.0	42.06	1.00	4627.0	
				Double, Clear	W	0.0	0.0	92.0	38.52	1.00	3544.2	
				Double, Clear	N	0.0	0.0	39.0	19.20	1.00	748.8	
				As-Built Total:				**241.0**			**8920.0**	
WALL TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM	=	Points	
Adjacent	184.0	0.70	128.8	Frame, Wood, Exterior			13.0	1523.0	1.50		2284.5	
Exterior	1523.0	1.70	2589.1	Frame, Wood, Adjacent			13.0	184.0	0.60		110.4	
Base Total:	**1707.0**		**2717.9**	**As-Built Total:**				**1707.0**			**2394.9**	
DOOR TYPES	Area X	BSPM	= Points	Type				Area X	SPM	=	Points	
Adjacent	20.0	1.60	32.0	Exterior Insulated				30.0	4.10		123.0	
Exterior	30.0	4.10	123.0	Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**50.0**		**155.0**	**As-Built Total:**				**50.0**			**155.0**	
CEILING TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM X SCM	=	Points	
Under Attic	2508.0	1.73	4338.8	Under Attic			30.0	1409.0	1.73 X 1.00		2437.6	
				Under Attic			30.0	1145.0	1.73 X 1.00		1980.8	
				Under Attic			19.0	138.0	2.34 X 1.00		322.9	
Base Total:	**2508.0**		**4338.8**	**As-Built Total:**				**2692.0**			**4741.3**	
FLOOR TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM	=	Points	
Slab	213.3(p)	-37.0	-7893.2	Slab-On-Grade Edge Insulation			0.0	213.3(p	-41.20		-8789.2	
Raised	0.0	0.00	0.0									
Base Total:			**-7893.2**	**As-Built Total:**				**213.3**			**-8789.2**	
INFILTRATION	Area X	BSPM	= Points					Area X	SPM	=	Points	
	2508.0	10.21	25606.7					2508.0	10.21		25606.7	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 33972.1			**Summer As-Built Points:**					**33028.7**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 60000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			33029	1.00	(1.09 x 1.147 x 1.00)	0.263	1.000	10841.1
33972.1	**0.4266**	**14492.5**	**33028.7**	**1.00**	**1.250**	**0.263**	**1.000**	**10841.1**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2508.0	12.74	5751.3	Double, Clear	E	0.0	0.0	110.0	18.79	1.00	2067.3	
				Double, Clear	W	0.0	0.0	92.0	20.73	1.00	1907.0	
				Double, Clear	N	0.0	0.0	39.0	24.58	1.00	958.5	
				As-Built Total:				**241.0**			**4932.8**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	184.0	3.60	662.4	Frame, Wood, Exterior			13.0	1523.0	3.40		5178.2	
Exterior	1523.0	3.70	5635.1	Frame, Wood, Adjacent			13.0	184.0	3.30		607.2	
Base Total:	**1707.0**		**6297.5**	**As-Built Total:**				**1707.0**			**5785.4**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	8.00	160.0	Exterior Insulated				30.0	8.40		252.0	
Exterior	30.0	8.40	252.0	Adjacent Insulated				20.0	8.00		160.0	
Base Total:	**50.0**		**412.0**	**As-Built Total:**				**50.0**			**412.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	2508.0	2.05	5141.4	Under Attic			30.0	1409.0	2.05 X	1.00	2888.4	
				Under Attic			30.0	1145.0	2.05 X	1.00	2347.3	
				Under Attic			19.0	138.0	2.70 X	1.00	372.6	
Base Total:	**2508.0**		**5141.4**	**As-Built Total:**				**2692.0**			**5608.3**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	213.3(p)	8.9	1898.6	Slab-On-Grade Edge Insulation			0.0	213.3(p	18.80		4010.6	
Raised	0.0	0.00	0.0									
Base Total:			**1898.6**	**As-Built Total:**				**213.3**			**4010.6**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	2508.0	-0.59	-1479.7					2508.0	-0.59		-1479.7	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**18021.2**	**Winter As-Built Points:**					**19269.4**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 60000 btuh ,EFF(8.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			19269.4	1.000	(1.069 x 1.169 x 1.00)	0.426	1.000	10264.2
18021.2	**0.6274**	**11306.5**	**19269.4**	**1.00**	**1.250**	**0.426**	**1.000**	**10264.2**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	40.0	0.89	3	1.00	2723.82	1.00	8171.5	
				As-Built Total:						**8171.5**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
14492	**11306**	**7905**	**33704**	**10841**	**10264**	**8171**	**29277**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.5

The higher the score, the more efficient the home.

Rosenboom Construction, , , ,

1.	New construction or existing	New			
2.	Single family or multi-family	Single family			
3.	Number of units, if multi-family	1			
4.	Number of Bedrooms	3			
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	2508 ft²			
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)				
a.	U-factor:	Description Area			
	(or Single or Double DEFAULT)	7a. (Dble Default) 241.0 ft²			
b.	SHGC:				
	(or Clear or Tint DEFAULT)	7b. (Clear) 241.0 ft²			
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 213.3(p) ft			
b.	N/A				
c.	N/A				
9.	Wall types				
a.	Frame, Wood, Exterior	R=13.0, 1523.0 ft²			
b.	Frame, Wood, Adjacent	R=13.0, 184.0 ft²			
c.	N/A				
d.	N/A				
e.	N/A				
10.	Ceiling types				
a.	Under Attic	R=30.0, 1409.0 ft²			
b.	Under Attic	R=30.0, 1145.0 ft²			
c.	Under Attic	R=19.0, 138.0 ft²			
11.	Ducts				
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 186.0 ft			
b.	N/A				
12.	Cooling systems				
a.	Central Unit		Cap: 60.0 kBtu/hr		
			SEER: 13.00		
b.	N/A				
c.	N/A				
13.	Heating systems				
a.	Electric Heat Pump		Cap: 60.0 kBtu/hr		
			HSPF: 8.00		
b.	N/A				
c.	N/A				
14.	Hot water systems				
a.	Electric Resistance		Cap: 40.0 gallons		
			EF: 0.89		
b.	N/A				
c.	Conservation credits				
	(HR-Heat recovery, Solar DHP-Dedicated heat pump)				
15.	HVAC credits				
	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)				

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

#25147
25146

ARROW EXTERMINATORS
TERMITE AND PEST CONTROL

5602 N.W. 13th STREET
GAINESVILLE, FLORIDA 32653-2198

P.O. BOX 5875
GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642
FAX (352) 373-9037

CERTIFICATE OF PROTECTIVE TREATMENT

Builder: Rosenboom

Date: 11-17-06 Time: _____ AM _____ PM

Site Location: 402 Macinit Dr

Area Treated: Detached Garage, Garage, Entry, Porch, Living

Product Used: BifenIT Chemical Used: Bifenthrin

% Concentration: .06% # Gallons Used: 850

Applicator: Jerry

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 16-7S-17-10006-212

Building permit No. 000025146

Use Classification SFD,UTILITY

Fire: 33.48

Permit Holder SCOTT ROSENBOOM

Waste: 100.50

Owner of Building MICHAEL MCALHANY

Total: 133.98

Location: 420 SW MAYNIK DRIVE(RIVER RISE, LOT 12)

Date: 04/16/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 16-7S-17-10006-212 **Building permit No.** 000025147

Use Classification DETACHED GARAGE **Fire:** 0.00

Permit Holder SCOTT ROSENBOOM **Waste:**

Owner of Building MICHAEL MCALHANY **Total:** 0.00

Location: 420 SW MARYNIK DRIVE(RIVER RISE,LOT 12)

Date: 04/12/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

ROSENBOOM

Scott & Kim Rosenboom
386.454.2894

CUSTOM HOMES • REMODELING • ADDITIONS

LIC CR-C057796

To : Columbia Co. building department .

Please allow Manuel Bernardo to pick up the certificate of occupancy and any other documents necessary for the McAhaney job. I would like this letter on file for any future documents if that is OK.

Thank you Scott Rosenboom

Scott Rosenboom

4-17-07

Bonnie P. Presnell

BONNIE P. PRESNELL
Notary Public, State of Florida
My comm. exp. Mar. 1, 2008
Comm. No. DD 277528

MARTY R. ESKRIDGE

14952 MAIN STREET ALACHUA FL 32615 PH: 386-462-1340

2514
2514

October 17, 2006

Job: Rosenboom Construction

Re: McAlhany Residence

Dear Sir:

The windload analysis has been reviewed as to the anchorages of the roof dormers. Install Simpson H6 @ 32" o.c. anchoring the walls to the roof trusses. Install Simpson SP2 @ 32" o.c. for the top plate to walls. Install Simpson H10 for the trusses to double top plates. See the original windload analysis for the nail size and pattern.

All 36" openings will have 1 jack and 2 full length spf studs on each side.

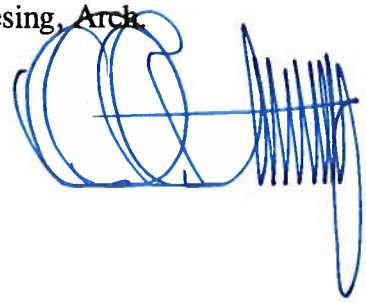
All openings from 36" to 78" will have 2 jack and 2 full length spf studs.

All headers will be 2 ply 2 x 12 syp #2 or better.

The garage door header will be pre-engineered. Install a 4 ply spf studs under each end of the garage door header. Install Simpson SPH4 and ST24 for each jack to header connection.

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

Paul R. Stresing, Arch.
AR0013985



06.10
06.10

ATTN: Joe

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0610-37

Scott Rosenboom Contractor Owners Michael McAlhany Property ID 16-7s-17-10006-212

On the date of October 12, 2006 application 0610-37 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0610-37 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. This is to clarify the 2004 Florida Residential Building Code requirements as the code relates to residential garages. Section R309 garages and carports.

A. R309.1 Opening protection:

Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

R309.1.1 Duct penetration:

B. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.

R309.2 Separation required:

C. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

R309.3 Floor surface:

D. Garage floor surfaces shall be of approved noncombustible material. The area of floor used for parking of automobiles or other vehicles shall be sloped to facilitate the movement of liquids to a drain or toward the main vehicle entry doorway.

E. The attic access opening (pull down ladder type attic egress door) in the garage ceiling shall have the same protection requirements of FRC-2004 C: R309.2 Separation required. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

F. The 2004 Florida Mechanical Code, Sections: 303.4 Protection from damage: Appliances shall not be installed in a location where subject to mechanical damage unless protected by approved barriers. Show the method which used

G. The garage has a sixteen foot opening for an overhead garage door; show a structural design which will detail the header beam size and the

attachment method including the total number of jack and king studs. Include in this design a detail showing the attachment of the beam and supporting members to the foundation.

Structural load bearing walls or supporting headers

2. On the exterior load bearing walls which have opening for door, windows or headers which support roof trusses, show the headers size for each span and the required number of supporting members (jack and king studs) needed to support these headers.

Dormer structural design

3. For construction of the false dormers provide a drawing, which will include design and construction information, including, material species, spacing, attachment to roof system and uplift requirements for the false dormers.

Electrical requirements

4. The electrical plans show that the electrical panel will be located in the garage area. At the electrical service entrance point an overcurrent protection device shall be installed on the exterior of structure which will provide overcurrent protection for the total service amperage rating and a means of disconnecting electrical service from the serving utility company. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Thank You:

Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

MARTY R. ESKRIDGE
14952 MAIN STREET ALACHUA FL 32615 PH 386-462-1340

October 17, 2006

Job: Rosenboom Construction

Re: McAlhany Residence

Dear Sir:

The windload analysis has been reviewed as to the anchorages of the roof dormers. Install Simpson H6 @ 32" o.c. anchoring the walls to the roof trusses. Install Simpson SP2 @ 32" o.c. for the top plate to walls. Install Simpson H10 for the trusses to double top plates. See the original windload analysis for the nail size and pattern.

All 36" openings will have 1 jack and 2 full length spf studs on each side.

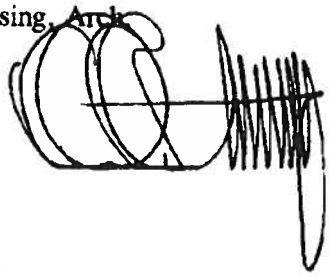
All openings from 36" to 78" will have 2 jack and 2 full length spf studs.

All headers will be 2 ply 2 x 12 syp #2 or better.

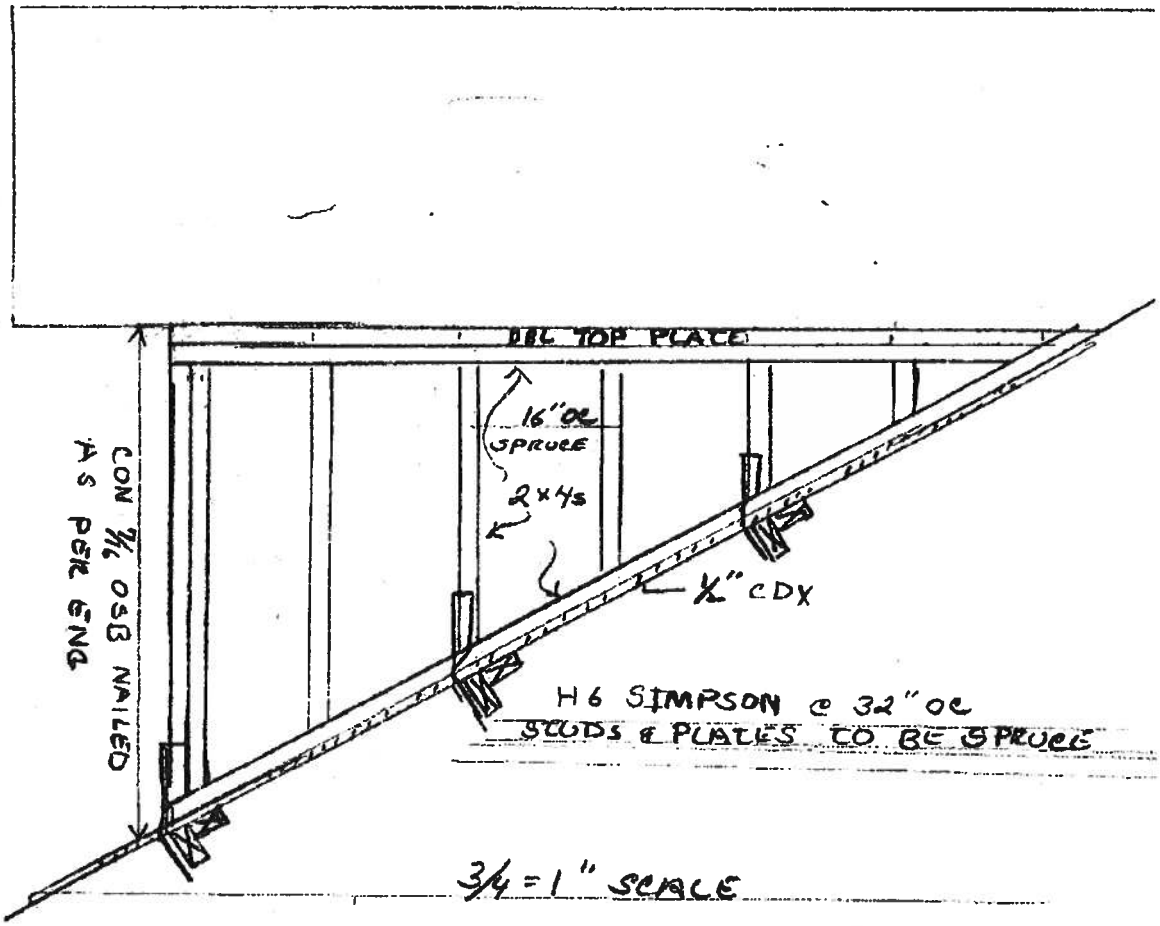
The garage door header will be pre-engineered. Install a 4 ply spf studs under each end of the garage door header. Install Simpson SPH4 and ST24 for each jack to header connection.

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

Paul R. Stresing, Arch.
AR0013985



ATTN: Joe

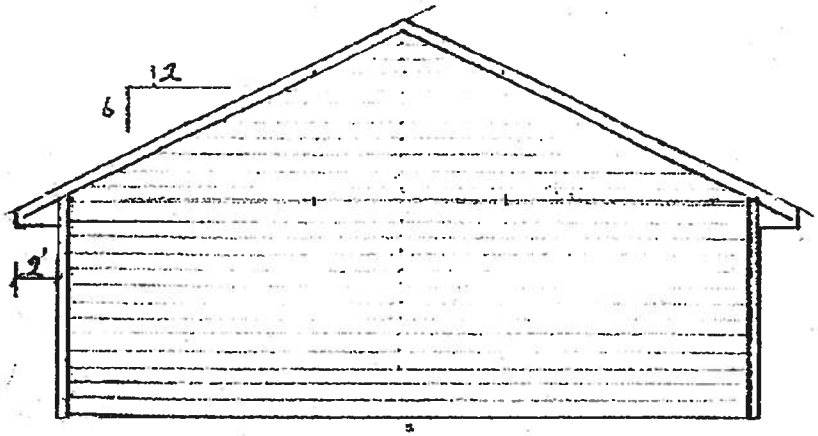


ALL OF THE 36" OPENINGS SHALL HAVE 1 JACK AND 2 KING STUDS ON EITHER SIDE OF THE OPENINGS THE FRONT DOOR REAR FRENCH DOORS AND THE 3'0" TWIN SHALL HAVE 2 KING & 2 JACKS ON EITHER SIDE OF THE OPENING ALL HEADERS SHALL BE 2x12 YELLOW PINE JACK AND STUD SHALL BE SPRUCE GARAGE DOOR SHALL BE ENGLED BEAM SUPPORTED BY 4 2x4x7' SPRUCE 2x4s ON EACH END EACH JACK SHALL BE ANCHORED AT THE BOTTOM WITH AN SPH4 AND A ST24 FOR JACK TO HEADER CON FOR EACH JACK

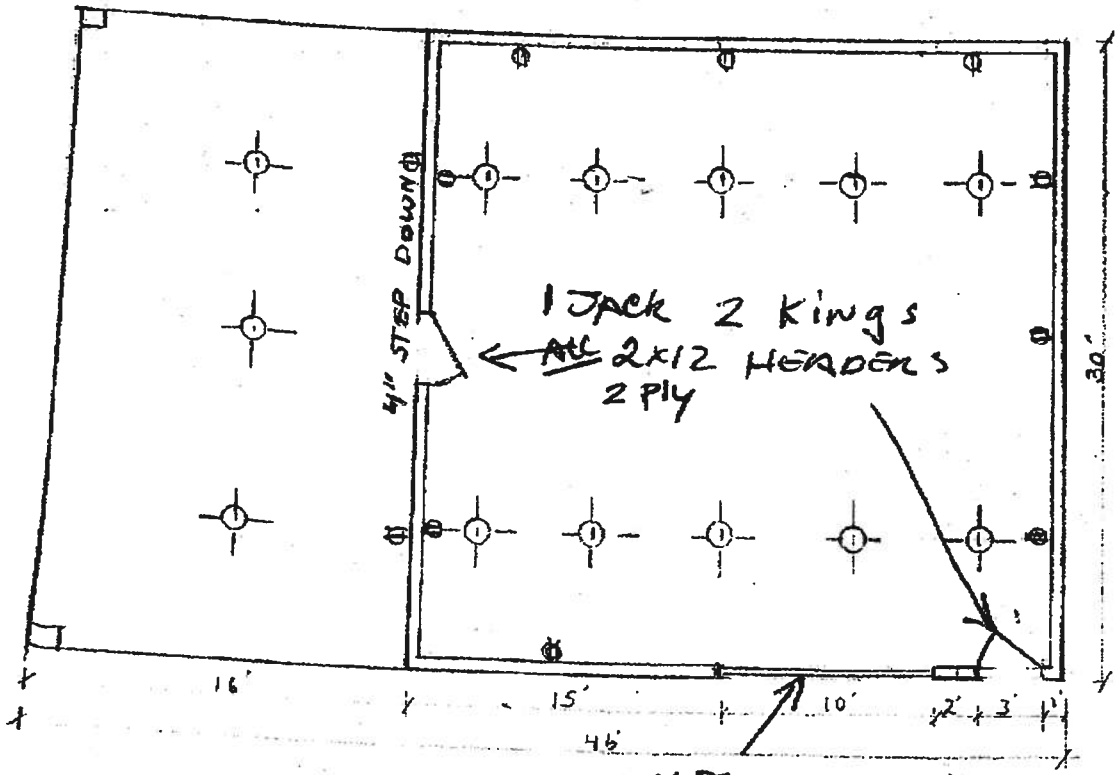
PMTS # 0610-37 AND 38

CELL # 352-538-3877 Scott

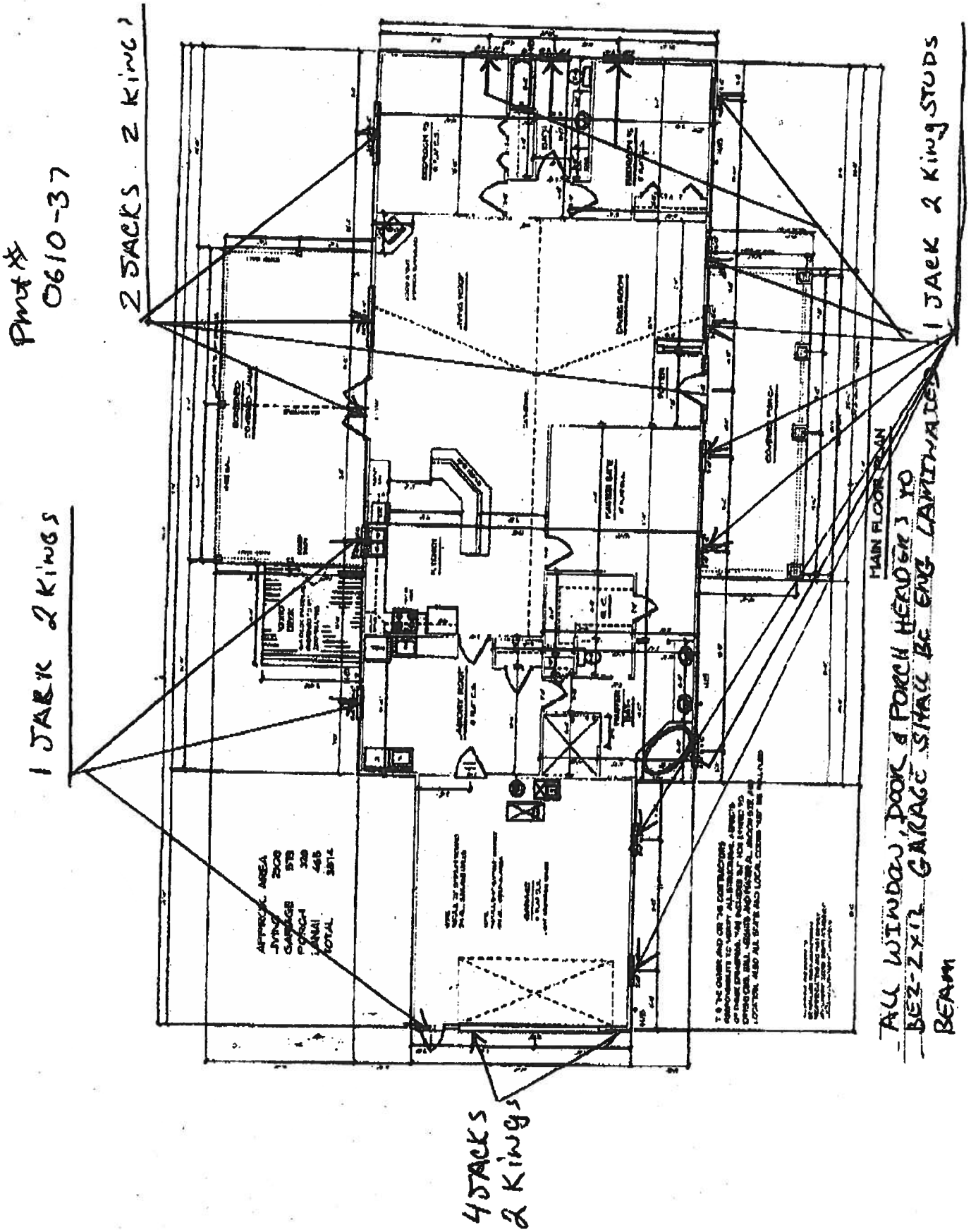
FROM : ROSENBOOM INC. PHONE NO. : 904 454 2894 Oct. 16 2006 01:15PM P1



RIGHT ELEVATION



0610-38
McALHANY GARAGE
ROSENBOOM CON.


Pmt # 0610-37
2 JACKS 2 KINGS
1 JACK 2 KINGS
1 JACK 2 KingSTUDS
ALL WINDOW DOOR & PORCH HEADERS TO BE 2x12 GARAGE SHALL BE ENG LAMINATED BEAM
4 JACKS 2 KINGS
MAIN FLOOR PLAN

RESIDENTIAL WIND DESIGN & ANALYSIS

FBC SECTION 1609 \ ***NO COPIES ARE TO BE PERMITTED***

PREPARED FOR:

ROSENBOOM CONSTRUCTION \ THE McALHANY RESIDENCE

PREPARED BY:

MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329

SUMMARY
OF WIND DESIGN & ANALYSIS

Trusses: Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/10d/16d
Interior zone spacing: Interior 8 in. Periphery 4 in.
Edge and end zone spacing: Interior 8 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
Interior stud spacing 16 in. Composite (yes or no) Y
End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.
61' — Trans: Fastener 8d/10d Spacing: Int 8 in. Edge 4 in.
103' — Long: Fastener 8d/10d Spacing: Int 8 in. Edge 4 in.

Allowable unit shear on shearwalls: 320 **pounds per linear foot**

Wall tension transferred by: Siding nails 8d/10d @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
Size 1/2 in. Shape L Washer 2" Embedment 7 in.
Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C. Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 96 in.
Monolithic footing: Depth 20 in. Bottom width 12 in.

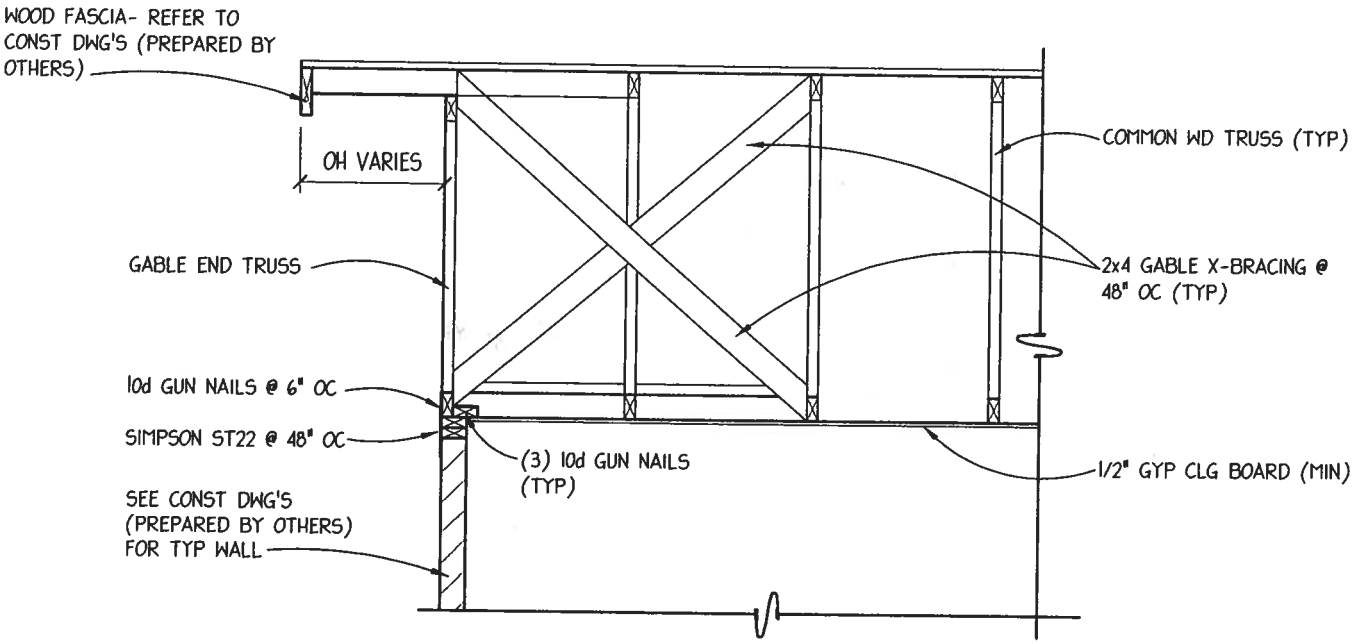
Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
Interior Footings: 16" W X 10" D

Porch Columns: 4X4X8' SYP #2 or # 1 @ 156" OC max

Porch Column Fasteners: Simpson CBY4/CCY4 or equal

NOTE:
1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

McALHANY
ANN 13285
9/20/06



THIS WIND ANALYSIS ASSUMES IDEAL SOIL CONDITIONS W/MIN 2500 PSI SOIL BRG CAPACITY & 95% DRY PROCTOR DENSITIES

GABLE END DETAIL

SCALE: 1/2" = 1'-0"

NOTE:
INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WOTH THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN ERECTION & BRACING SHEET.

AM053985
9/29/06

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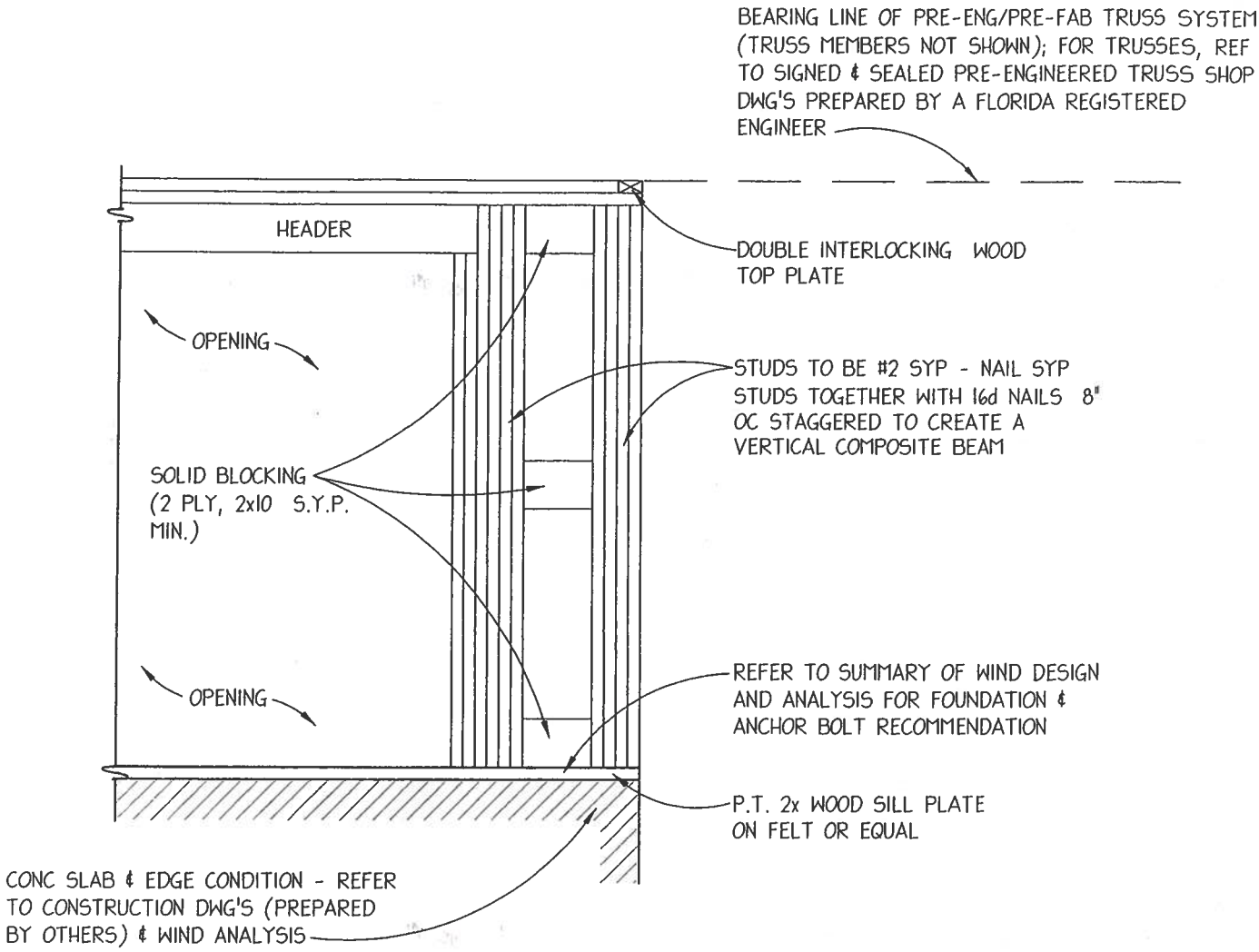
TYPICAL GABLE END DETAILS	PAUL STRESING ASSOCIATES, INC. 14017 MAIN STREET ALACHUA, FLORIDA 32615 E-MAIL: psaarch@bellsouth.net TELEPHONE: (386) 462-9407 FAX (386) 462-6408 REGISTRATION NO. AR0013888 - CA NO. AA0003377	DRAWN BY: C.A.D. DATE: 2006 PROJECT FILE: WINDLOADS	SHEET NO. of



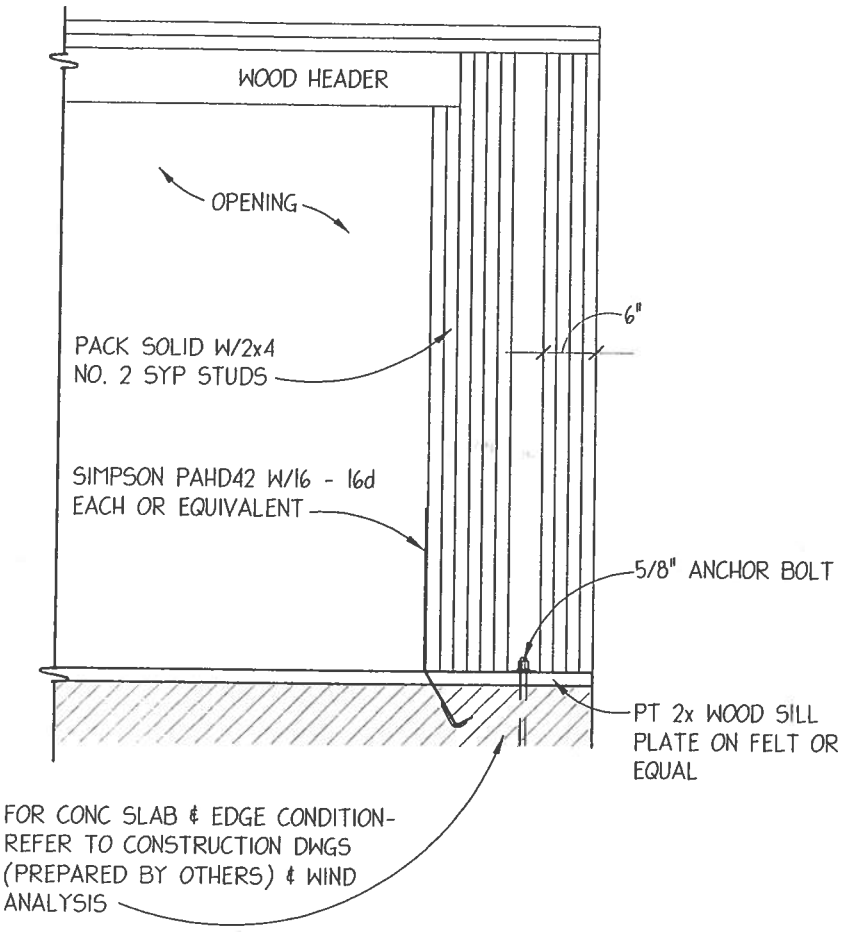
MINIMUM 2'-0" SHEAR WALL SEGMENT

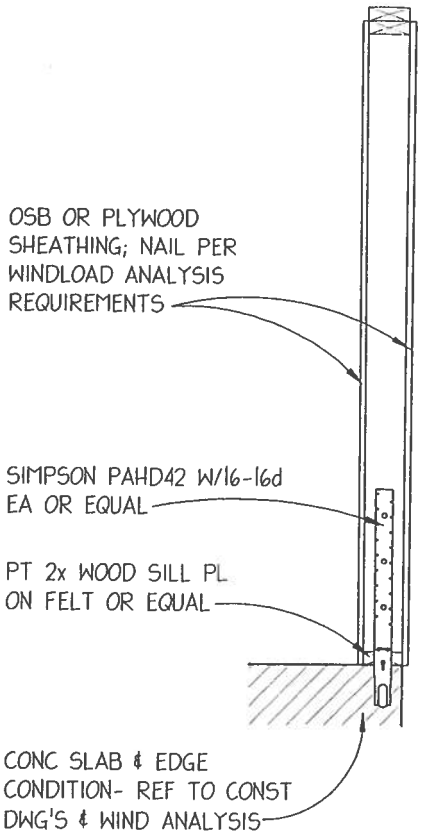
NOTE: ALLOWABLE EQUIVALENT SHEAR WALL EQUAL TO 1.5 TIMES THE ACTUAL WALL SEGMENT LENGTH. MINIMUM WALL SEGMENT LENGTH OF 2'-0" WHICH EQUATES TO A 3'-0" SHEAR WALL SEGMENT. SEE WINDLOAD ANALYSIS FOR NAIL SIZES AND SPACING ON SHEATHING.

AR0013988
9/20/06

O:\SUPPORT\DETAILS\WINDLOADS\SHEAR WALL.DWG 06/20/06 16:01

TYPICAL SHEAR WALL DETAIL	PAUL STRESING ASSOCIATES, INC. 14617 MAIN STREET ALACHUA, FLORIDA 32615 E-MAIL: prsarch@bellsouth.net TELEPHONE: (386) 462-9407 FAX (386) 462-9408 REGISTRATION NO. AR0013988 - CA NO. AA0003377	DRAWN BY: CAD DATE: 2006 PROJECT FILE: WINDLOAD	SHEET NO. OF





FRONT VIEW

SIDE VIEW

EQUIVALENT 3'-0" SHEAR WALL SEGMENT

9/20/06

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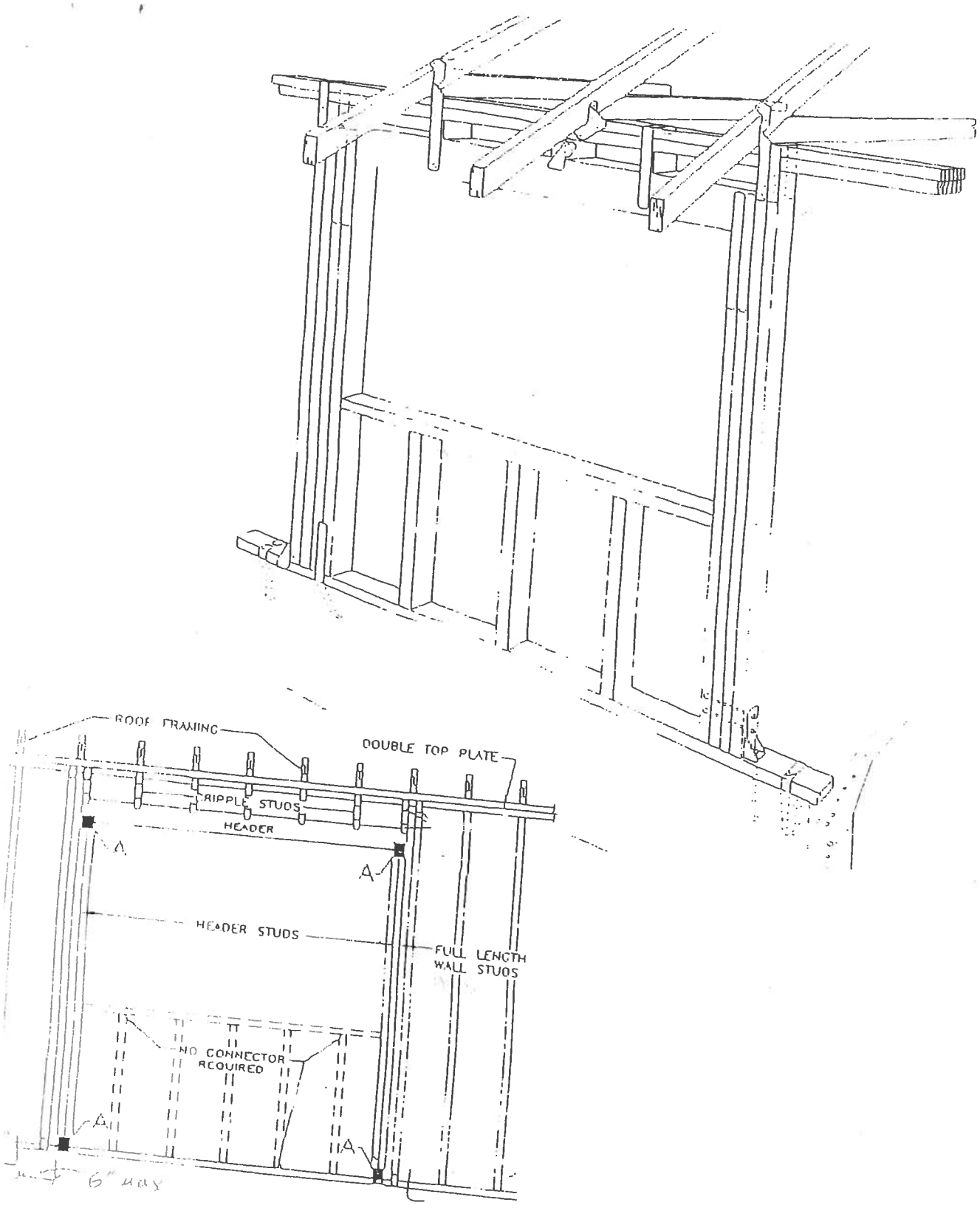
SHEAR WALL DETAIL / TYP DOUBLE SHEATH CORNER	PAUL STRESING ASSOCIATES, INC. 14617 MAIN STREET ALACHUA, FLORIDA 32615 E-MAIL: psearchi@bellsouth.net TELEPHONE: (386) 462-6407 FAX (386) 462-6408 REGISTRATION NO. AR0013868 - CA NO. AA0003377 DRAWN BY: C.A.D. DATE: 2006 PROJECT FILE: WINDLOAD SHEET NO. 01



Total each truss uplift on the header divide by 2 for header anchorage

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force.
Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate.
Use proper bolt anchors.
Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.
Strap ridge beam at each end with minimum uplift resistance of 1000#.
It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300

*or per truss engineering
Use proper bolt anchors
All beams to be sheathed or strapped to Double Top Plate when applicable.

CRIPPLES	Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Wall sheathing nailing Adequate exterior walls top w/8d nails @ 3" O.C., as long as sheathing covers top plate, otherwise use SP2 @ 32" O.C. in addition to sheathing nailing.
Use SP2 top and SP1 bottom each stud for all interior load bearing walls and anchor bolts @ 32" O.C.
Interior anchor bolts to be ½" x 8" A307 or ½" x 6" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.
NOTE: For nailing into SPF members, multiply table values by .86

ASCE 7-02

9/19/06

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.3	Deg
Type of Roof	Gabled	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	20.58	ft
Mean Roof Height (Ht)	16.21	ft
Width Perp. to Wind (B)	60.58	ft
Width Parallel to Wind (L)	96.67	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.27	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8903	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8603	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	1.05	
Nb	4.6*NatFreq*B/Vzm	3.93	
Nd	15.4*NatFreq*Depth/Vzm	21.00	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.5539	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2220	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0465	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.6120	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.02	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

ASCE 7-02

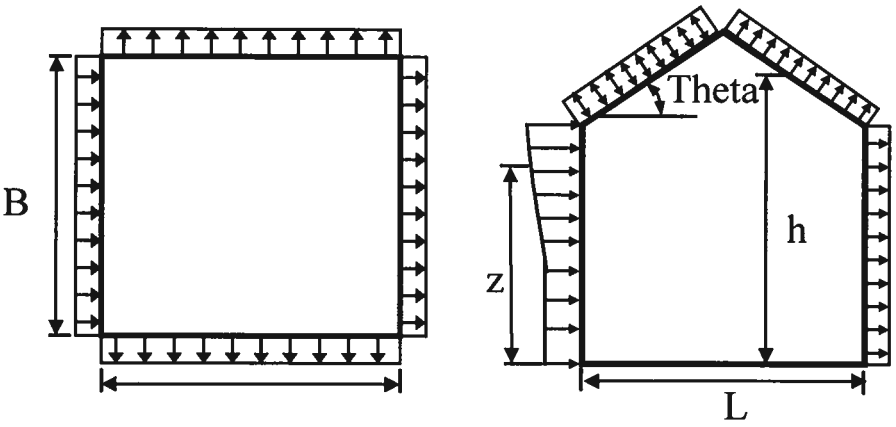
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd 1.00	qz lb/ft^2	Pressure (lb/ft^2) Windward Wall* +GCpi	-GCpi
20.58	0.70	1.00	1.00	21.70	11.66	18.21
20	0.70	1.00	1.00	21.70	11.66	18.21
16.21	0.70	1.00	1.00	21.70	11.66	18.21
15	0.70	1.00	1.00	21.70	11.66	18.21

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.59	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	18.20	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf) +GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 60.58 ft wall)	-0.38	-9.24	-2.69
Leeward Walls (Wind Dir Parallel to 96.67 ft wall)	-0.50	-11.11	-4.55
Side Walls	-0.70	-14.24	-7.68
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.19	-6.22	0.33
Windward - Max Positive	0.31	1.52	8.07
Leeward Normal to Ridge	-0.60	-12.67	-6.12
Overhang Top	-0.19	-2.94	-2.94
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 8.105 ft	-0.90	-17.37	-10.82

ASCE 7-02

Wind Load Design per ASCE 7-02

Dist from Windward Edge: 8.105 ft to 16.21 ft	-0.90	-17.37	-10.82
Dist from Windward Edge: 16.21 ft to 32.42 ft	-0.50	-11.11	-4.55
Dist from Windward Edge: > 32.42 ft	-0.30	-7.97	-1.42

* Horizontal distance from windward edge

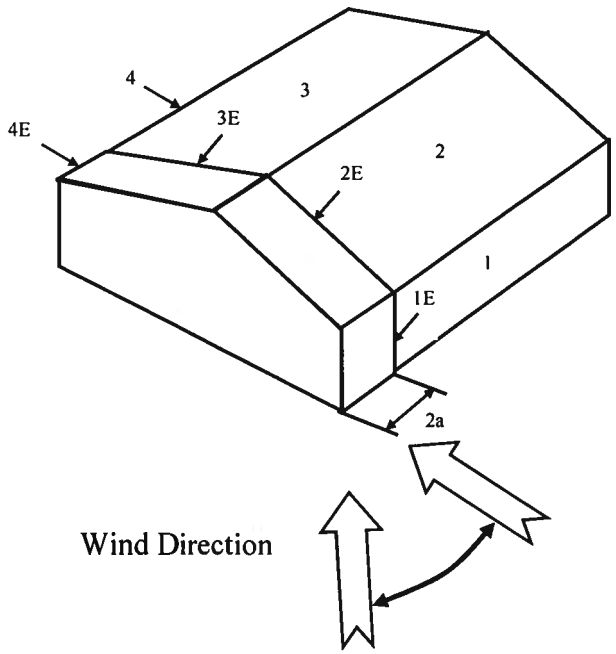
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.59
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.20

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-02

Wind Load Design per ASCE 7-02

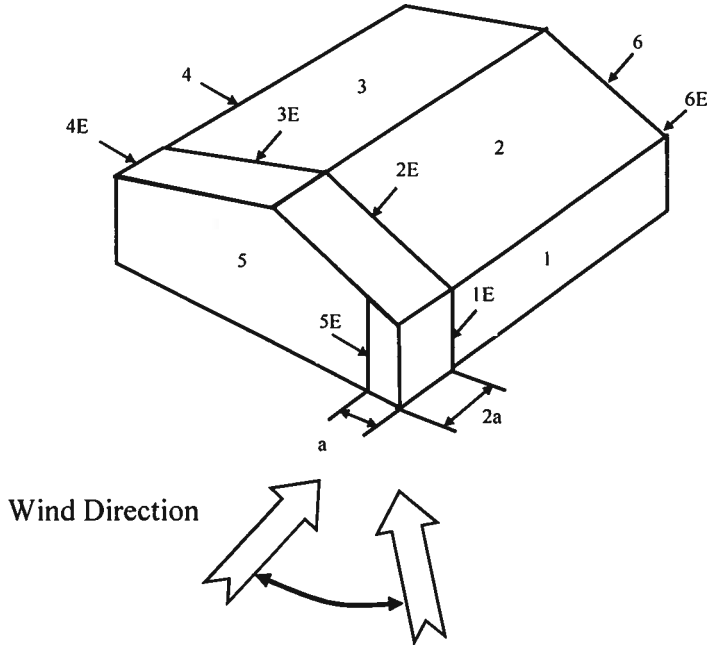
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.59
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.20

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

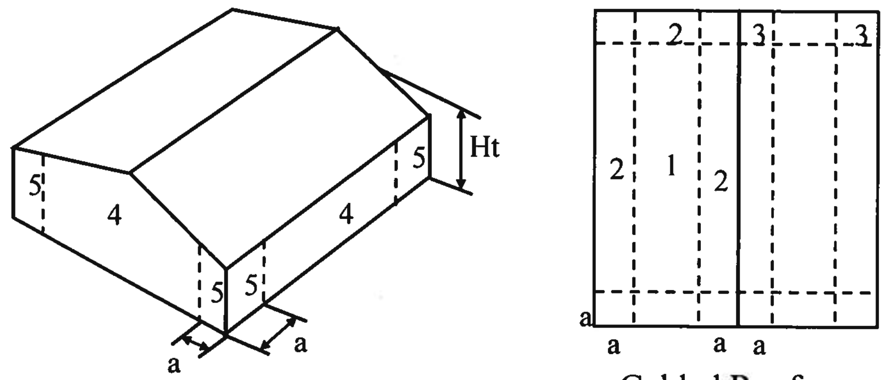


ASCE 7-02

Wind Load Design per ASCE 7-02

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft



Gabled Roof

10 < Theta <= 45

a = 6.058 ==> 6.06 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp Max	GCp Min	Wind Press (lb/ft^2) Max	Wind Press (lb/ft^2) Min
	16	7	112.00	5	0.81	-1.03	18.11	-22.01
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

ASCE 7-02

Wind Load Design per ASCE 7-02

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.32	-18.93	-11.11
	P (-GCpi) -psf	5.23	-12.38	-4.55
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.30	-18.93	-11.11
	P (-GCpi) -psf	3.30	-18.93	-11.11

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	96.67	ft
B	Roof dimension parallel to wind direction	60.58	ft
L/B	Ratio of L to B	1.596	
Theta	Slope of Roof	30.3	Deg
Cf	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft

CRP 650

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.02
Test Dates: 05/13/02
And: 05/16/02
Report Date: 11/12/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title	Summary of Results
AAMA Rating	H-R35 112 x 72
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Deflection Test Pressure	+35.3 psf -35.0 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.02 for complete test specimen description and data.

Revision Log

Rev. #	Date	Page(s)	Revision(s)
0	06/15/05	N/A	Original report issue
1	04/18/06	Cover Page, pages 1, 2, 4, 5, and 6	Change rating to LC instead of R

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Test Specimen #2: H-LC40* 36 x 72		
	Optional Performance		
4.3	Water Resistance per ASTM E 547 (with and without screen) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	50.0 psf (positive)	0.18"	See Note #2
	50.0 psf (negative)	0.15"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	75.0 psf (positive)	0.02"	0.12" max.
	75.0 psf (negative)	0.01"	0.12" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Mark A. Hess

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MAH:vlm/tla

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-LC35 48 x 84 (Oriel) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Right stile	0.06"/12%	0.50"/100%
	Left stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen)		
	6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	35.3 psf (positive)	0.27"	See Note #2
	47.2 psf (negative)	0.35"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	53.0 psf (positive)	0.02"	0.17" max.
	70.8 psf (negative)	0.06"	0.17" max.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-LC35 48 x 84 (Oriel)			
2.2.1.6.1	Operating Force	23 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.16 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547 (with and without screen) 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	15.0 psf (positive)	0.14"	See Note #2
	15.0 psf (negative)	0.14"	See Note #2

Note #2: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	22.5 psf (positive)	0.01"	0.17" max.
	22.5 psf (negative)	0.01"	0.17" max.

Test Specimen Description: (Continued)

Sash Construction: The sash was constructed of thermally broken extruded aluminum with coped, and butted corners fastened with one #8 x 3/4" screws per corner through the rails into the stile screw boss.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible wrap around vinyl spline.

Hardware:

Description	Quantity	Location
Metal cam lock	1 per sash	Interior meeting rail midspan
Plastic tilt latch	2 per sash	Interior meeting rail ends
Metal tilt pin	2 per sash	Bottom rail ends
Balance assembly	2 per sash	One per jamb
Spring loaded retainer pin	2 per screen	4" from stiles on bottom screen rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill

Installation: The specimens were installed into a #2 Spruce-Pine-Fir wood buck. The nail fins were back bedded in silicone and secured with #8 x 1-5/8" drywall screws located 2-1/2" from corners and 14" on center around nail fin perimeter. Silicone was utilized around the exterior perimeter.

Test Specimen Description: (Continued)

Test Specimen #2: H-LC40* 36 x 72

Overall Size: 3' 0-1/4" wide by 6' 0-1/4" high

Interior Sash Size: 2' 9-3/4" wide by 2' 11-7/8" high

Fixed Daylight Opening Size: 2' 7" wide by 2' 9" high

Screen Size: 2' 8-3/16" wide by 2' 11-3/8" high

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The specimen utilized 5/8" thick sealed insulating glass constructed from two sheets of 3/16" thick clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with a flexible vinyl snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail
0.230" high by 0.187" backed polypile with center fin	2 Rows	Sash stiles
3/4" wide by 5/8" long polypile pad	4 pieces	All corners of sash
1/4" foam filled vinyl bulb seal	1 Row	Bottom rail

Frame Construction: The frame was constructed of thermally broken extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two #6 x 1" screws per cap. Meeting rail was then secured to the frame utilizing two #6 x 1" screws.

CAP 650

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-42487.01
Test Date: 08/14/02
And: 08/15/02
Report Date: 10/02/02
Expiration Date: 08/15/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 450/650/850 Drop In Glazing, aluminum single hung window at their facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-LC30 53 x 90 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 450/650/850 Drop In Glazing

Type: Aluminum Single Hung Window

Overall Size: 4' 5-1/8" wide by 7' 5-5/8" high

Interior Sash Size: 4' 2-3/4" wide by 3' 8-7/8" high

Fixed Daylight Opening Size: 4' 0" wide by 3' 5-3/8" high

Screen Size: 4' 0-3/4" wide by 3' 8-3/4" high

Finish: The unit was white.

Glazing Details: The specimen utilized 5/8" thick, sealed insulating glass constructed from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450/650/850 Drop In Glazing
TYPE: Aluminum Single Hung Window

Title	Summary of Results
AAMA Rating	H-LC30 53 x 90
Operating Force	24 lb max.
Air Infiltration	0.11 cfm/ft^2
Water Resistance Test Pressure	6.75 psf
Uniform Load Deflection Test Pressure	+32.8 psf -47.2 psf
Uniform Load Structural Test Pressure	+49.2 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-42487.01 for complete test specimen description and data.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Optional Performance		
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 35.3 psf (positive) @ 35.0 psf (negative)	 0.46" 0.41"	 See Note #2 See Note #2
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 53.0 psf (positive) @ 52.5 psf (negative)	 0.03" 0.02"	 0.29" max. 0.29" max.

Note #2: The Uniform Load Deflection test is not an AAMA/NWWDA 101/I.S.2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without written approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

David A. Kranz
Director - Product Physical Testing

MAH:dm
01-41641.02

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2.1.6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

McALHANY JOB

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

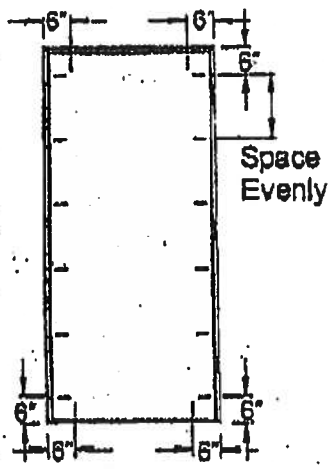
Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	CAPTIVOL	650 SERIES	FL675 FL685
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			FL881
E. MULLION			FL880
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL	GULF COAST	FL 6V 26 yr	#2632
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER FELT	30# WANTON	30# FELT	FL 2346
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

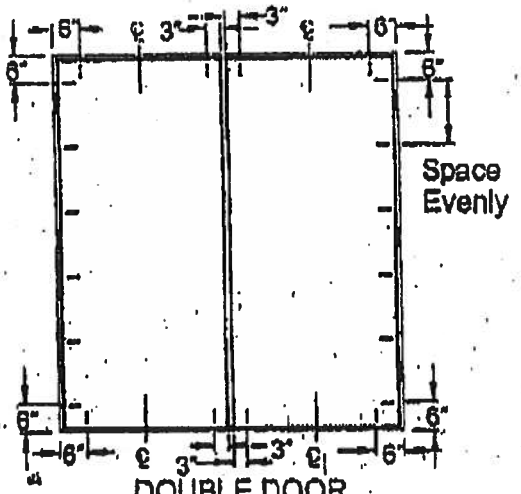
Scott Rourke
APPLICANT SIGNATURE

DATE

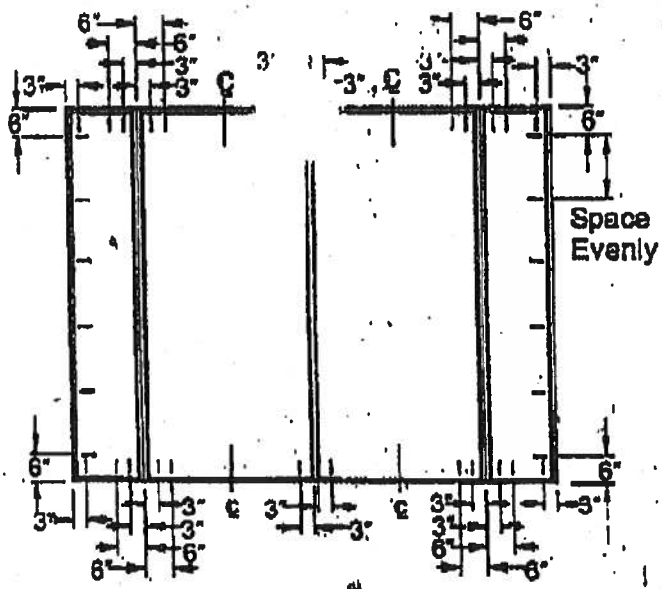
R-1305 01-04



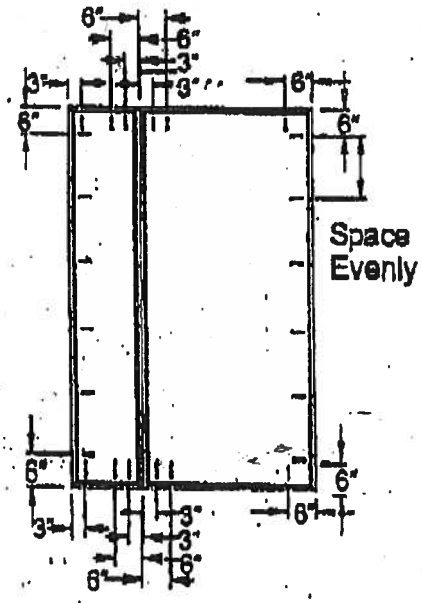
SINGLE DOOR



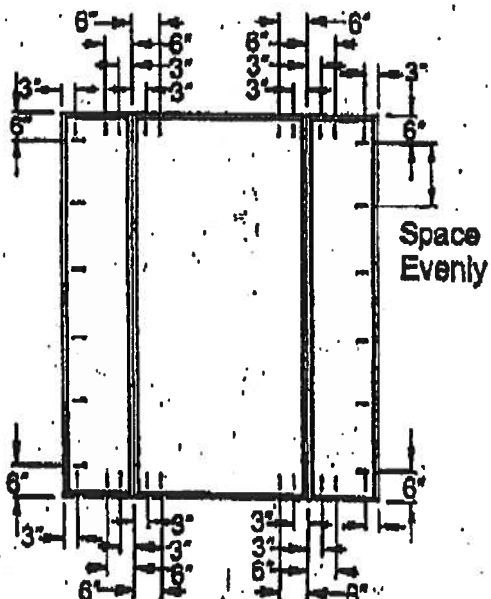
DOUBLE DOOR



DOUBLE DOOR W/SIDELITES



SINGLE DOOR W/SIDELITE



SINGLE DOOR W/SIDELITES

ATTACHMENTS FOR THERMOTRUE DOORS

3" x #8 SCREWS 2% TAPCONS IN SILL

5V CRIMP 26 GA.
LOAD TABLE OVER PLYWOOD

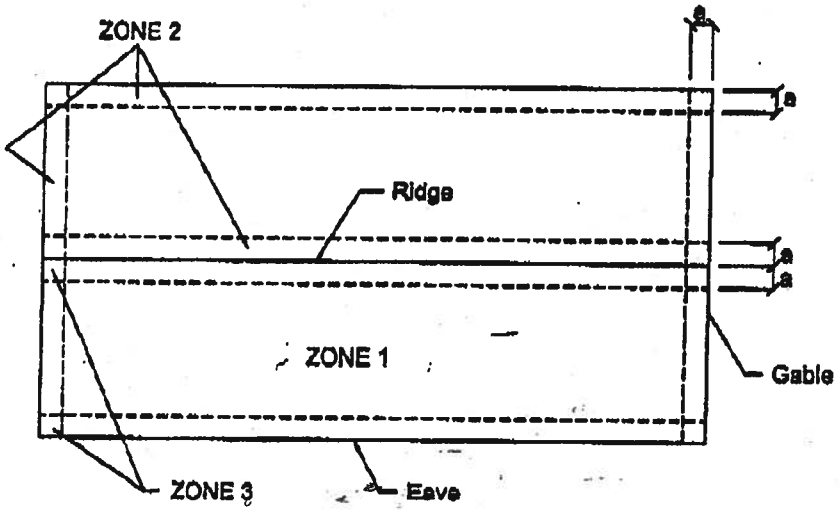
GULF COAST

Buildings having a Roof Mean Height ≤ 20'-0"; Roof Slope: 3"/12" -12"/12"
Wind Speeds 110-140 mph, Exp C, I = 1.0, based on FLORIDA BUILDING CODE 2004

5 V CRIMP 26 GA. FASTENER SPACING						
ZONE	FASTENER	SUBSTRATE	WIND SPEED ZONE			
			110	120	130	140
			ON CENTER SPACING	ON CENTER SPACING	ON CENTER SPACING	ON CENTER SPACING
ZONE 1	#9-15 x 1-1/2"	15/32" CDX/ 19/32" CDX	16"	16"	16"	16"
ZONE 2	#9-15 x 1-1/2"	15/32" CDX/ 19/32" CDX	16"	16"	16"	16"
ZONE 3	#9-15 x 1-1/2"	15/32" CDX/ 19/32" CDX	16"	16"	16"	16"

STANDARD PATTERN

PANEL END/END LAP PATTERN



Note: Dimension (a) is defined as 10% of the minimum width of the building or 40% of the mean height of the roof, whichever is smaller, however, (a) cannot be less than either 4% of the minimum width of the building or 3 feet.

FL # 2632

JUN 2 8 2005

Project Summary
Entire House
HOGLE'S HEATING & AIR

Job: McALHANY RESIDENCE
Date: 09/18/06
By: W.D.HOGLE

Project Information

For: SCOTT ROSENBOOM CONSTRUCTION INC.

Notes:

Design Information

Weather: Gainesville, FL , US

Winter Design Conditions

Outside db	33	°F
Inside db	70	°F
Design TD	37	°F

Summer Design Conditions

Outside db	92	°F
Inside db	75	°F
Design TD	17	°F
Daily range	M	
Relative humidity	50	%
Moisture difference	52	gr/lb

Heating Summary

Building heat loss	37242	Btuh
Ventilation air	0	cfm
Ventilation air loss	0	Btuh
Design heat load	37242	Btuh

Sensible Cooling Equipment Load Sizing

Structure	40262	Btuh
Ventilation	0	Btuh
Design temperature swing	3.0	°F
Use mfg. data	n	
Rate/swing multiplier	0.97	
Total sens. equip. load	39054	Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1 (Average)

	Heating	Cooling
Area (ft²)	2502	2502
Volume (ft³)	22292	22292
Air changes/hour	0.70	0.35
Equiv. AVF (cfm)	260	130

Latent Cooling Equipment Load Sizing

Internal gains	1200	Btuh
Ventilation	0	Btuh
Infiltration	4572	Btuh
Total latent equip. load	7143	Btuh
Total equipment load	46197	Btuh
Req. total capacity at 0.70 SHR	4.6	ton

Heating Equipment Summary

Make YORK
Trade
E1RD060S06

Efficiency	8	HSPF
Heating input		
Heating output	54500	Btuh @ 47°F
Heating temp rise	25	°F
Actual heating fan	2000	cfm
Heating air flow factor	0.054	cfm/Btuh

Space thermostat

Cooling Equipment Summary

Make YORK
Trade
E1RD060S06
F2FP060N06

Efficiency	13	EER
Sensible cooling	38150	Btuh
Latent cooling	16350	Btuh
Total cooling	54500	Btuh
Actual cooling fan	2000	cfm
Cooling air flow factor	0.050	cfm/Btuh
Load sensible heat ratio	85	%

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Short Form
Entire House
HOGLE'S HEATING & AIR

Job: McALHANY RESIDENCE
Date: 09/18/06
By: W.D.HOGLE

Project Information

For: SCOTT ROSENBOOM CONSTRUCTION INC.

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1 (Average)

HEATING EQUIPMENT

Make YORK
Trade
E1RD060S06

Efficiency	8 HSPF	
Heating input		
Heating output	54500	Btuh @ 47°F
Heating temperature rise	25	°F
Actual heating fan	2000	cfm
Heating air flow factor	0.054	cfm/Btuh

Space thermostat

COOLING EQUIPMENT

Make YORK
Trade
E1RD060S06
F2FP060N06

Efficiency	13 EER	
Sensible cooling	38150	Btuh
Latent cooling	16350	Btuh
Total cooling	54500	Btuh
Actual cooling fan	2000	cfm
Cooling air flow factor	0.050	cfm/Btuh
Load sensible heat ratio	85	%

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
MASTER BATHROOM	224	3794	3027	204	150
MAST.BED/WIC	378	4207	5208	226	259
DINING ROOM	204	3021	2426	162	121
FOYER	136	2293	1622	123	81
BEDROOM #2	192	4444	4694	239	233
HALL BATHROOM	72	1418	758	76	38
BEDROOM #3	208	4719	4925	253	245
LIVING ROOM	437	5668	7532	304	374
KITCHEN	361	2941	6546	158	325
LAUNDRY ROOM	266	4685	3458	252	172
HALL AREA	24	51	66	3	3

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House Ventilation air Equip. @ 0.97 RSM Latent cooling	d	2502	37242 0	40262 0 39054 7143	2000	2000
TOTALS		2502	37242	46197	2000	2000

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

MiTek

RE: RSNMCA -

MiTek Industries, Inc.
1801 Massaro Blvd.
Tampa, Fl 33619
Phone: 813/675-1200
Fax: 813/675-1148

Site Information:
Project Customer: Project Name:
Lot/Block: 12 Subdivision: RIVER RISE
Address:
City: HIGH SPRINGS State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.3
Wind Code: ASCE 7/02 Wind Speed: 110 mph Design Method: User defined
Roof Load: 40 psf, nonconcurrent BCLL=10 psf Floor Load: 55.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	T2381466	RSNMCA	A	10/3/06
2	T2381467	RSNMCA	A1	10/3/06
3	T2381468	RSNMCA	A1ET	10/3/06
4	T2381469	RSNMCA	A2	10/3/06
5	T2381470	RSNMCA	A3	10/3/06
6	T2381471	RSNMCA	A4	10/3/06
7	T2381472	RSNMCA	AET	10/3/06
8	T2381473	RSNMCA	B	10/3/06
9	T2381474	RSNMCA	BET	10/3/06
10	T2381475	RSNMCA	C	10/3/06
11	T2381476	RSNMCA	CET	10/3/06
12	T2381477	RSNMCA	D	10/3/06
13	T2381478	RSNMCA	DET	10/3/06
14	T2381479	RSNMCA	E	10/3/06
15	T2381480	RSNMCA	EET	10/3/06

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie
My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

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Zhang, Guo-jie

Job	Truss	Truss Type	Qty	Ply	
RSNMCA	A	ROOF TRUSS	13	1	T2381466
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:37:52 2006 Page 1

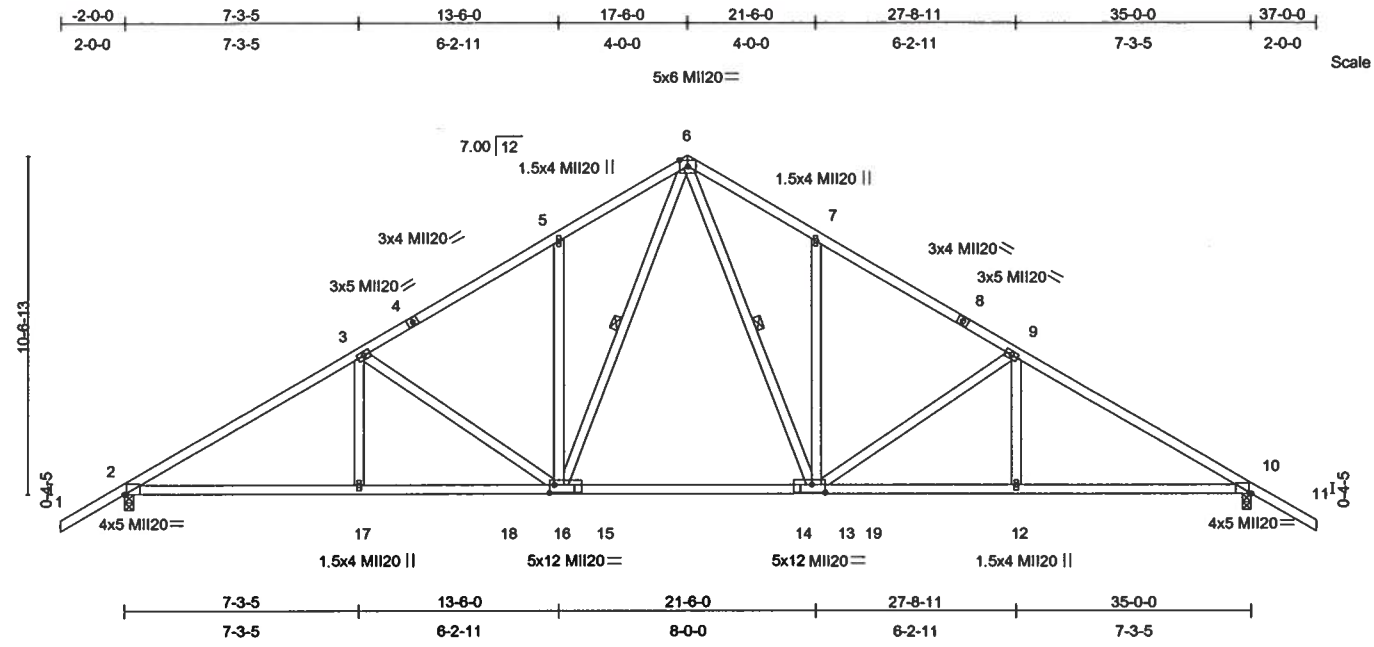

Plate Offsets (X,Y): [2:0-0-13,0-0-2], [10:0-0-13,0-0-2], [14:0-5-4,0-3-0], [15:0-1-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.28	13-16	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.58	13-16	>716	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.12	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 208 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-11 oc bracing.
WEBS 1 Row at midpt 6-16, 6-13

REACTIONS (lb/size) 2=1687/0-3-8, 10=1687/0-3-8
Max Horz 2=456(load case 4)
Max Uplift2=-997(load case 5), 10=-997(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-2621/1213, 3-4=-2126/898, 4-5=-2020/926, 5-6=-2085/1047, 6-7=-2085/1046, 7-8=-2020/926, 8-9=-2126/898, 9-10=-2621/1213, 10-11=0/60
BOT CHORD 2-17=-978/2157, 17-18=-978/2157, 16-18=-978/2157, 15-16=-371/1392, 14-15=-371/1392, 13-14=-371/1392, 13-19=-865/2157, 12-19=-865/2157, 10-12=-865/2157
WEBS 3-17=-219/255, 3-16=-506/545, 5-16=-300/215, 6-16=-539/982, 6-13=-539/982, 7-13=-300/214, 9-13=-506/545, 9-12=-220/255

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 997 lb uplift at joint 2 and 997 lb uplift at joint 10.
6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-60, 6-11=-60, 2-18=-20, 18-19=-50(F=-30), 10-19=-20
2) IBC BC Live: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-20, 6-11=-20, 2-18=-40, 18-19=-70(F=-30), 10-19=-40

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Continued on page 2

LOAD CASE(S)

3) MWFRS Wind Left: Lumber Increase=1.33, Plate Increase=1.33
   Uniform Loads (plf)
   Vert: 1-2=12, 2-6=-15, 6-10=29, 10-11=19, 2-18=27, 18-19=-3(F=-30), 10-19=27
   Horz: 1-2=-22, 2-6=5, 6-10=39, 10-11=29
4) MWFRS Wind Right: Lumber Increase=1.33, Plate Increase=1.33
   Uniform Loads (plf)
   Vert: 1-2=19, 2-6=29, 6-10=-15, 10-11=12, 2-18=27, 18-19=-3(F=-30), 10-19=27
   Horz: 1-2=-29, 2-6=-39, 6-10=-5, 10-11=22
5) MWFRS 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
   Uniform Loads (plf)
   Vert: 1-2=71, 2-3=48, 3-6=30, 6-10=30, 10-11=22, 2-18=22, 18-19=-8(F=-30), 10-19=22
   Horz: 1-2=-81, 2-3=-58, 3-6=-40, 6-10=40, 10-11=32
6) MWFRS 2nd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
   Uniform Loads (plf)
   Vert: 1-2=22, 2-6=30, 6-9=30, 9-10=48, 10-11=71, 2-18=22, 18-19=-8(F=-30), 10-19=22
   Horz: 1-2=-32, 2-6=-40, 6-9=40, 9-10=58, 10-11=81
7) MWFRS 3rd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
   Uniform Loads (plf)
   Vert: 1-2=46, 2-3=23, 3-6=16, 6-10=16, 10-11=7, 2-18=22, 18-19=-8(F=-30), 10-19=22
   Horz: 1-2=-56, 2-3=-33, 3-6=-26, 6-10=26, 10-11=17
8) MWFRS 4th Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
   Uniform Loads (plf)
   Vert: 1-2=7, 2-6=16, 6-9=16, 9-10=23, 10-11=46, 2-18=22, 18-19=-8(F=-30), 10-19=22
   Horz: 1-2=-17, 2-6=-26, 6-9=26, 9-10=33, 10-11=56
9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-6=-60, 6-11=-20, 2-18=-20, 18-19=-50(F=-30), 10-19=-20
10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-6=-20, 6-11=-60, 2-18=-20, 18-19=-50(F=-30), 10-19=-20

Job	Truss	Truss Type	Qty	Ply	T2381467
RSNMCA	A1	ROOF TRUSS	2	1	Job Reference (optional)

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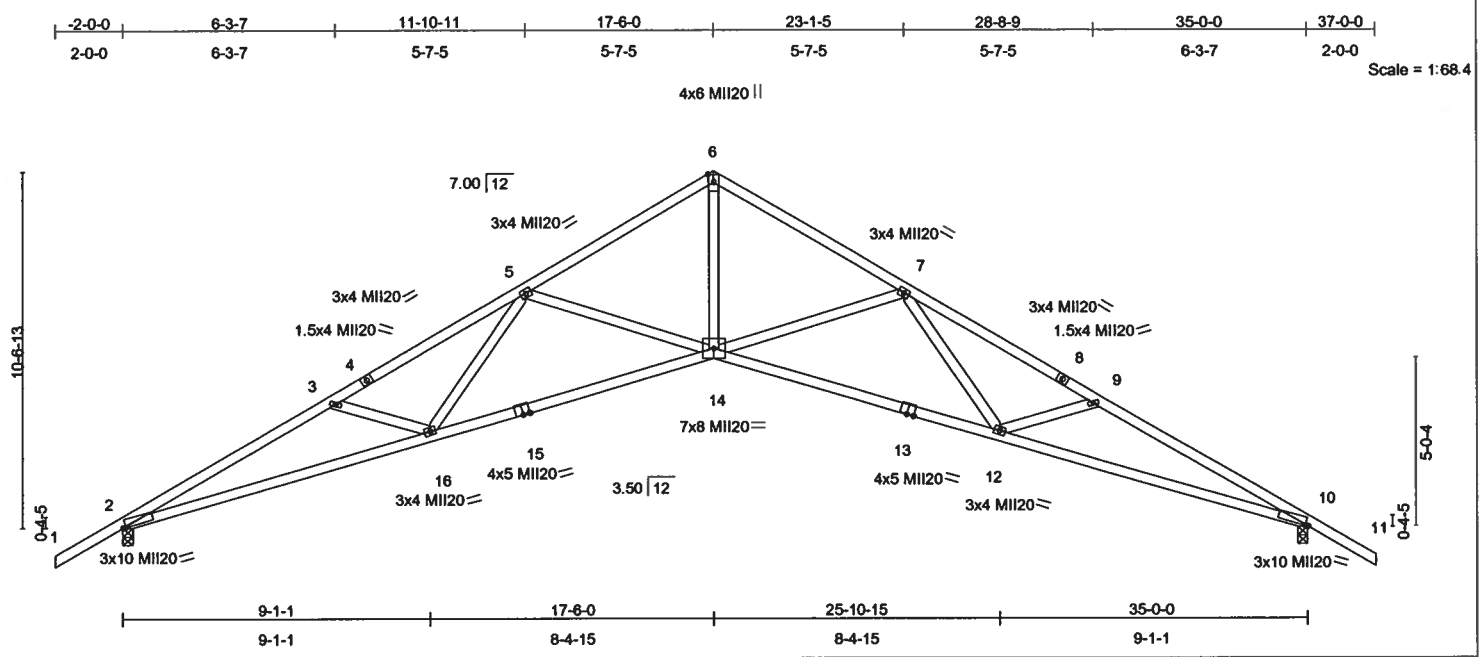

Plate Offsets (X,Y): [2:0-1-1,0-0-2], [10:0-1-1,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.66	14-16	>636	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.90	14-16	>461	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.71	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-10-3 oc bracing.

REACTIONS (lb/size) 2=1517/0-3-8, 10=1517/0-3-8
Max Horz 2=457(load case 4)
Max Uplift2=-1167(load case 5), 10=-1167(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-4248/2779, 3-4=-3871/2559, 4-5=-3784/2585, 5-6=-2780/1641, 6-7=-2780/1641, 7-8=-3784/2492, 8-9=-3871/2466, 9-10=-4248/2644, 10-11=0/58
BOT CHORD 2-16=-2402/3723, 15-16=-1706/3107, 14-15=-1694/3139, 13-14=-1535/3139, 12-13=-1547/3107, 10-12=-2189/3723
WEBS 3-16=-312/346, 5-16=-623/601, 5-14=-725/663, 6-14=-1463/2375, 7-14=-725/673, 7-12=-640/601, 9-12=-312/351

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1167 lb uplift at joint 2 and 1167 lb uplift at joint 10.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	T2381468
RSNMCA	A1ET	GABLE	1	1	Job Reference (optional)

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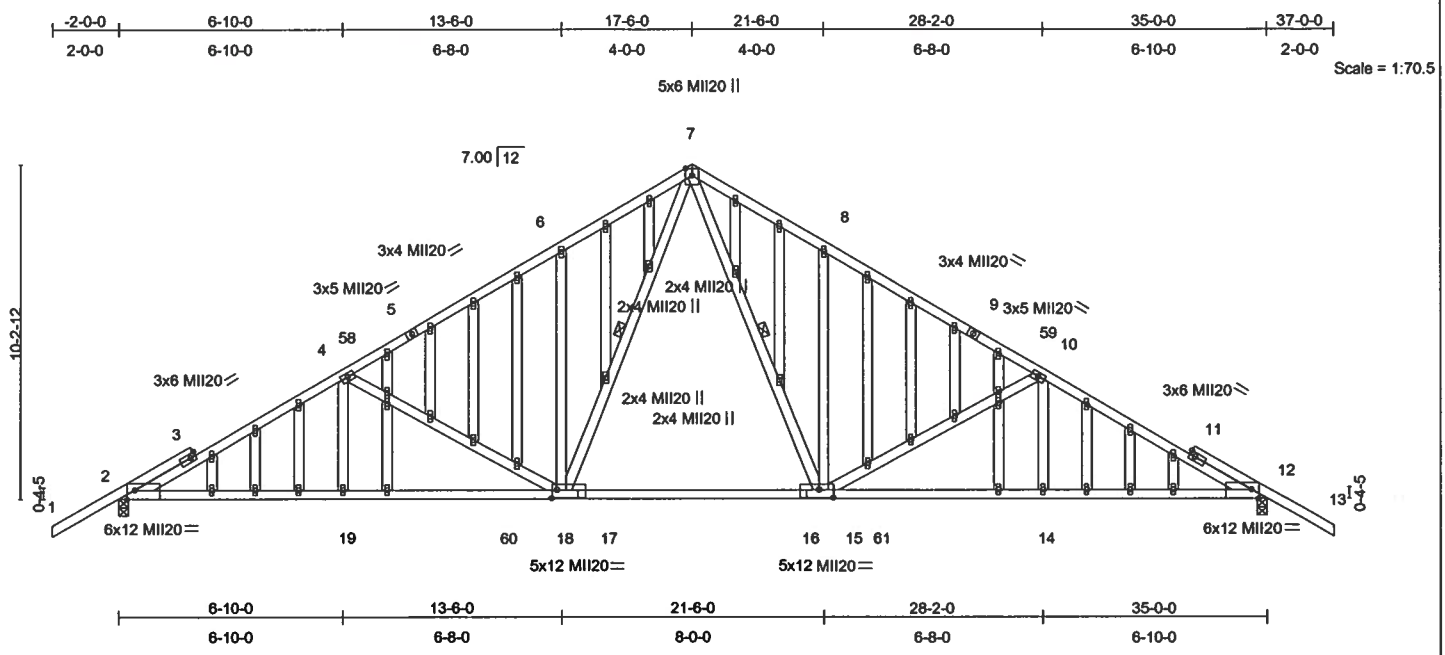

Plate Offsets (X,Y): [2:0-2-11,Edge], [12:0-2-11,Edge], [16:0-5-4,0-3-0], [17:0-1-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	0.32	15-18	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.62	15-18	>668	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.13	12	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 300 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-8 oc bracing.
WEBS 1 Row at midpt 7-18, 7-15

REACTIONS (lb/size) 2=1687/0-3-8, 12=1687/0-3-8
Max Horz 2=441(load case 4)
Max Uplift2=-1000(load case 5), 12=-1000(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-2827/1302, 3-4=-2774/1331, 4-58=-2227/917, 5-58=-2208/921, 5-6=-2112/951, 6-7=-2178/1083, 7-8=-2178/1083, 8-9=-2112/951, 9-59=-2208/921, 10-59=-2227/917, 10-11=-2774/1331, 11-12=-2827/1302, 12-13=0/60
BOT CHORD 2-19=-1082/2404, 19-60=-1082/2404, 18-60=-1082/2404, 17-18=-376/1439, 16-17=-376/1439, 15-16=-376/1439, 15-61=-995/2404, 14-61=-995/2404, 12-14=-995/2404
WEBS 4-19=-216/266, 4-18=-663/627, 6-18=-312/232, 7-18=-554/1032, 7-15=-553/1032, 8-15=-312/230, 10-15=-663/628, 10-14=-216/266

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MII20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 1-4-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1000 lb uplift at joint 2 and 1000 lb uplift at joint 12.
9) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

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Continued on page 2

LOAD CASE(S)

Uniform Loads (plf)
Vert: 1-7=-60, 7-13=-60, 2-60=-20, 60-61=-50(F=-30), 12-61=-20

2) IBC BC Live: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-20, 7-13=-20, 2-60=-40, 60-61=-70(F=-30), 12-61=-40

3) MWFRS Wind Left: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=12, 2-7=-15, 7-12=29, 12-13=19, 2-60=27, 60-61=-3(F=-30), 12-61=27
Horz: 1-2=-22, 2-7=5, 7-12=39, 12-13=29

4) MWFRS Wind Right: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=19, 2-7=29, 7-12=-15, 12-13=12, 2-60=27, 60-61=-3(F=-30), 12-61=27
Horz: 1-2=-29, 2-7=-39, 7-12=-5, 12-13=22

5) MWFRS 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=71, 2-58=48, 7-58=30, 7-12=30, 12-13=22, 2-60=22, 60-61=-8(F=-30), 12-61=22
Horz: 1-2=-81, 2-58=-58, 7-58=-40, 7-12=40, 12-13=32

6) MWFRS 2nd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=22, 2-7=30, 7-59=30, 12-59=48, 12-13=71, 2-60=22, 60-61=-8(F=-30), 12-61=22
Horz: 1-2=-32, 2-7=-40, 7-59=40, 12-59=58, 12-13=81

7) MWFRS 3rd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=46, 2-58=23, 7-58=16, 7-12=16, 12-13=7, 2-60=22, 60-61=-8(F=-30), 12-61=22
Horz: 1-2=-56, 2-58=-33, 7-58=-26, 7-12=26, 12-13=17

8) MWFRS 4th Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=7, 2-7=16, 7-59=16, 12-59=23, 12-13=46, 2-60=22, 60-61=-8(F=-30), 12-61=22
Horz: 1-2=-17, 2-7=-26, 7-59=26, 12-59=33, 12-13=56

9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-60, 7-13=-20, 2-60=-20, 60-61=-50(F=-30), 12-61=-20

10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-20, 7-13=-60, 2-60=-20, 60-61=-50(F=-30), 12-61=-20

Job	Truss	Truss Type	Qty	Ply	
RSNMCA	A2	ROOF TRUSS	9	1	T2381469

Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:37:56 2006 Page 1

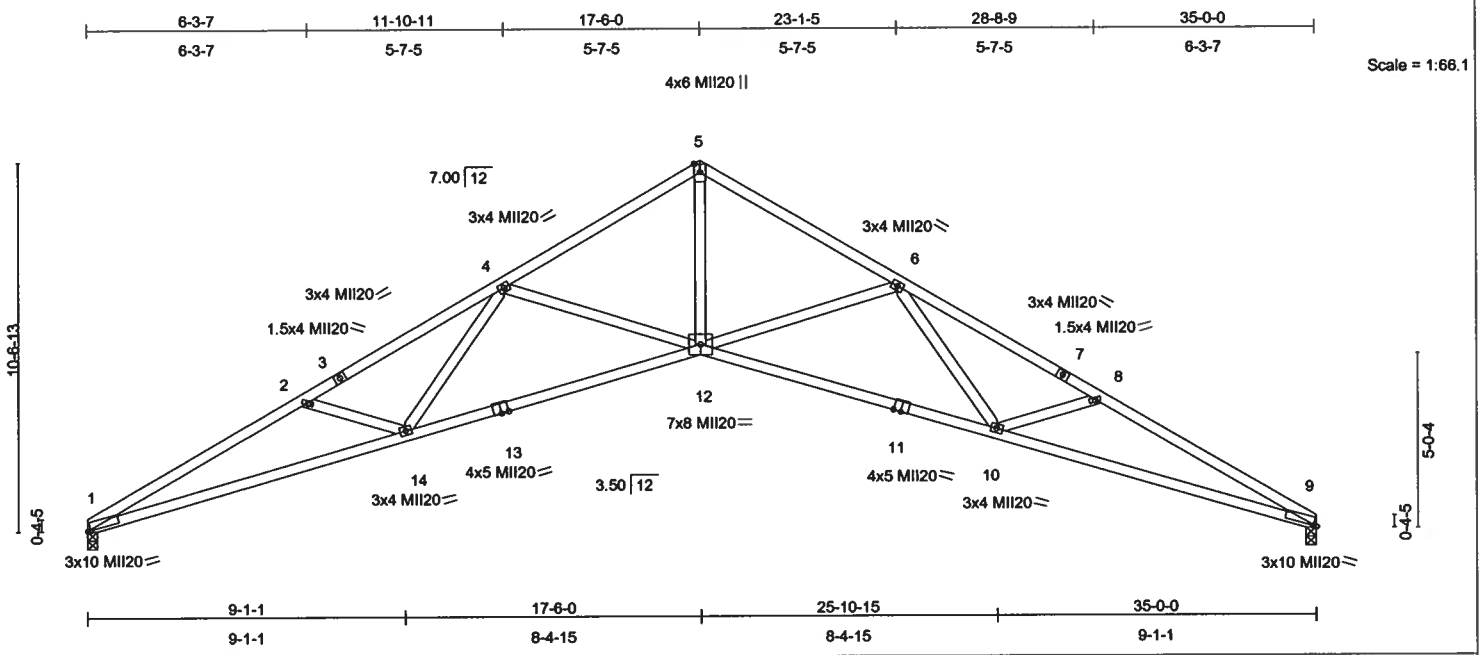

Plate Offsets (X,Y): [1:0-1-1,0-0-2], [9:0-1-1,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.67	10-12	>625	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.91	10-12	>458	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.72	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-6-0 oc bracing.

REACTIONS (lb/size) 1=1388/0-3-8, 9=1388/0-3-8
Max Horz 1=-450(load case 3)
Max Uplift1=-1010(load case 5), 9=-1010(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4313/2928, 2-3=-3940/2687, 3-4=-3851/2714, 4-5=-2809/1709, 5-6=-2809/1709, 6-7=-3851/2632, 7-8=-3940/2606, 8-9=-4313/2849
BOT CHORD 1-14=-2544/3807, 13-14=-1771/3136, 12-13=-1759/3179, 11-12=-1639/3179, 10-11=-1651/3136, 9-10=-2395/3807
WEBS 2-14=-334/378, 4-14=-673/613, 4-12=-740/670, 5-12=-1529/2402, 6-12=-740/679, 6-10=-676/613, 8-10=-334/381

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1010 lb uplift at joint 1 and 1010 lb uplift at joint 9.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	T2381470
RSNMCA	A3	ROOF TRUSS	8	1	Job Reference (optional)

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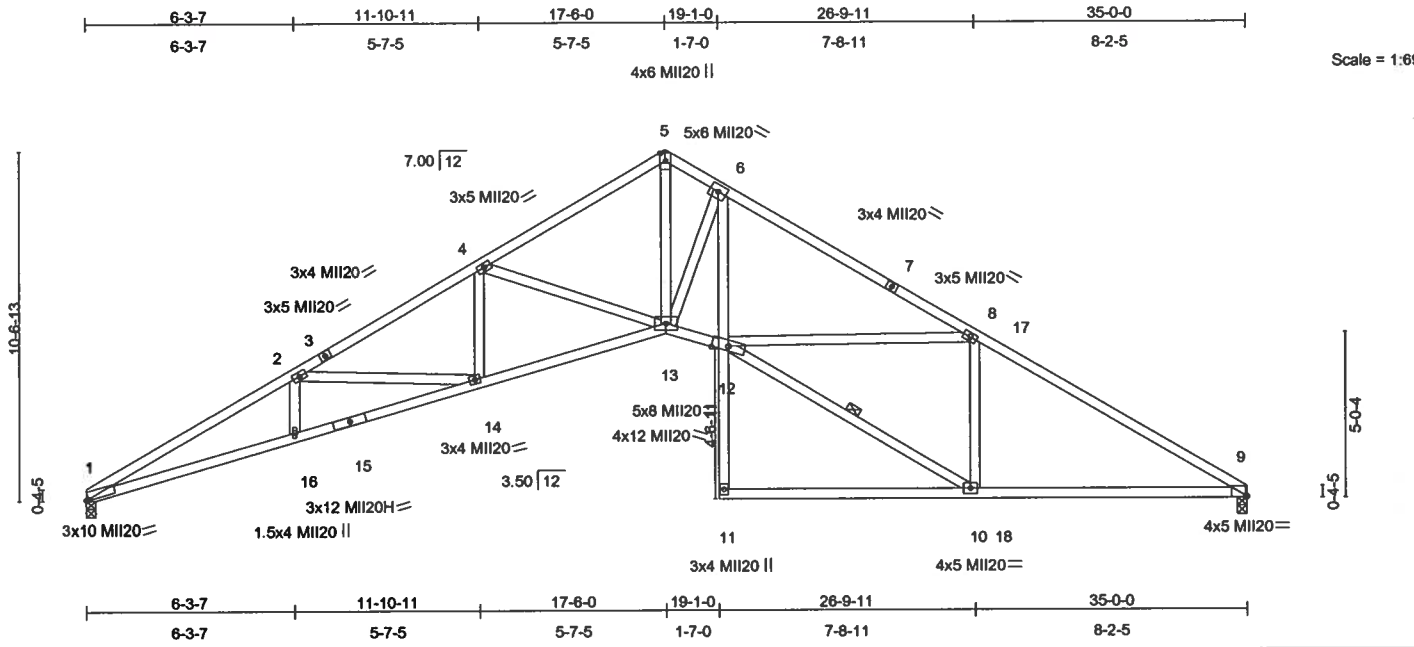


Plate Offsets (X,Y): [1:0-1-1,0-0-2], [9:0-0-13,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	0.57	13-14	>728	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.72	13-14	>579	180	MII20H	187/143
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.55	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
6-11 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-4-7 oc bracing.
WEBS 1 Row at midpt 10-12

REACTIONS (lb/size) 9=1388/0-3-8, 1=1388/0-3-8
Max Horz 1=795(load case 4)
Max Uplift9=-1043(load case 6), 1=-990(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4385/3240, 2-3=-3620/2550, 3-4=-3535/2575, 4-5=-2795/1993, 5-6=-2707/2013, 6-7=-3017/2140, 7-8=-3144/2106, 8-17=-2061/1627, 9-17=-2309/1623
BOT CHORD 1-16=-3172/3837, 15-16=-3104/3814, 14-15=-3096/3836, 13-14=-2405/3197, 12-13=-1807/2664, 11-12=-90/134, 6-12=-366/515, 10-11=0/107, 10-18=-1249/1888, 9-18=-1249/1888
WEBS 2-16=-190/241, 2-14=-645/735, 4-14=-389/389, 4-13=-765/707, 5-13=-1860/2460, 6-13=-649/502, 10-12=-1589/2177, 8-12=-658/718, 8-10=-932/590

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1043 lb uplift at joint 9 and 990 lb uplift at joint 1.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 3,2006

Job	Truss	Truss Type	Qty	Ply	T2381471
RSNMCA	A4	ROOF TRUSS	3	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:37:58 2006 Page 1

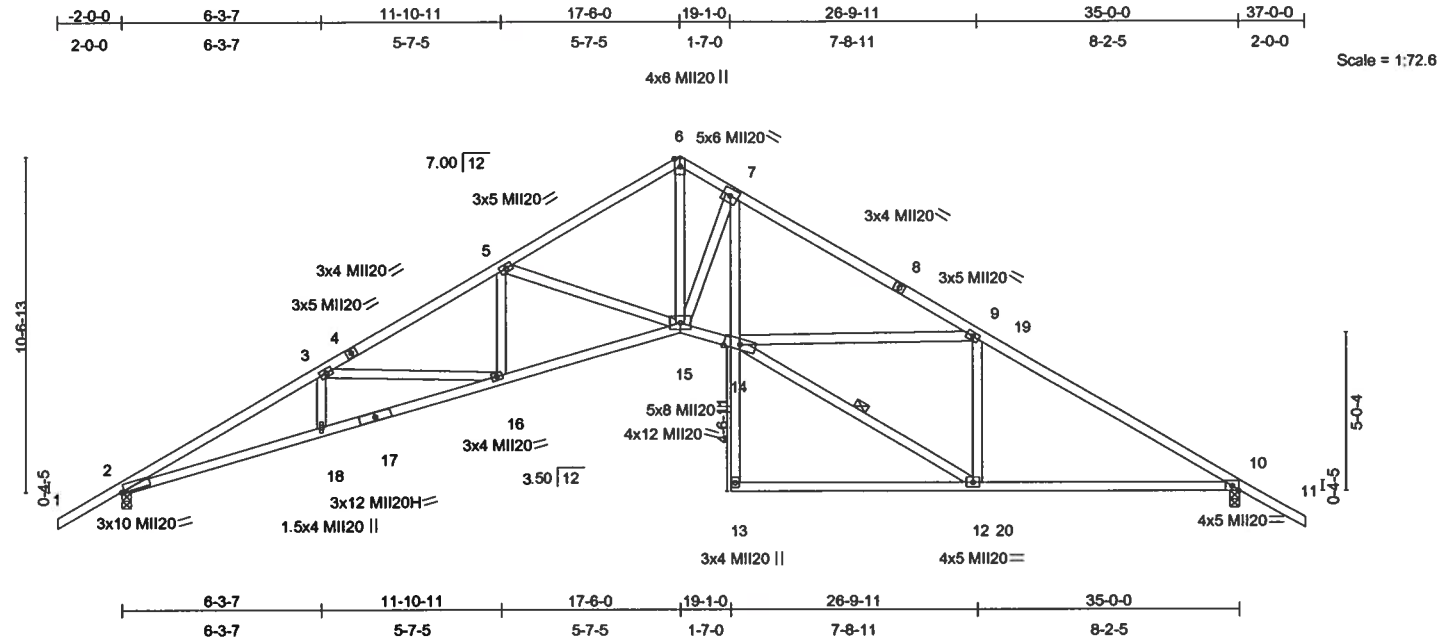


Plate Offsets (X,Y): [2:0-1-1,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	0.55	15-16	>760	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.71	15-16	>589	180	MII20H	187/143
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.54	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
7-13 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-6-12 oc bracing.
WEBS 1 Row at midpt 12-14

REACTIONS (lb/size) 10=1517/0-3-8, 2=1517/0-3-8
Max Horz 2=801(load case 4)
Max Uplift10=-1200(load case 6), 2=-1147(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-4288/3069, 3-4=-3576/2459, 4-5=-3494/2485, 5-6=-2764/1876, 6-7=-2683/1904, 7-8=-2984/2027, 8-9=-3110/1993, 9-19=-2024/1577, 10-19=-2272/1573, 10-11=0/60
BOT CHORD 2-18=-3082/3740, 17-18=-3016/3723, 16-17=-3008/3745, 15-16=-2339/3159, 14-15=-1598/2635, 13-14=-91/135, 7-14=-347/502, 12-13=0/106, 12-20=-1152/1851, 10-20=-1152/1851
WEBS 3-18=-182/234, 3-16=-592/656, 5-16=-379/384, 5-15=-756/696, 6-15=-1758/2435, 7-15=-644/497, 12-14=-1421/2133, 9-14=-634/726, 9-12=-924/526

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1200 lb uplift at joint 10 and 1147 lb uplift at joint 2.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 3,2006

MiTek
POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	T2381472
RSNMCA	AET	GABLE	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:00 2006 Page 1

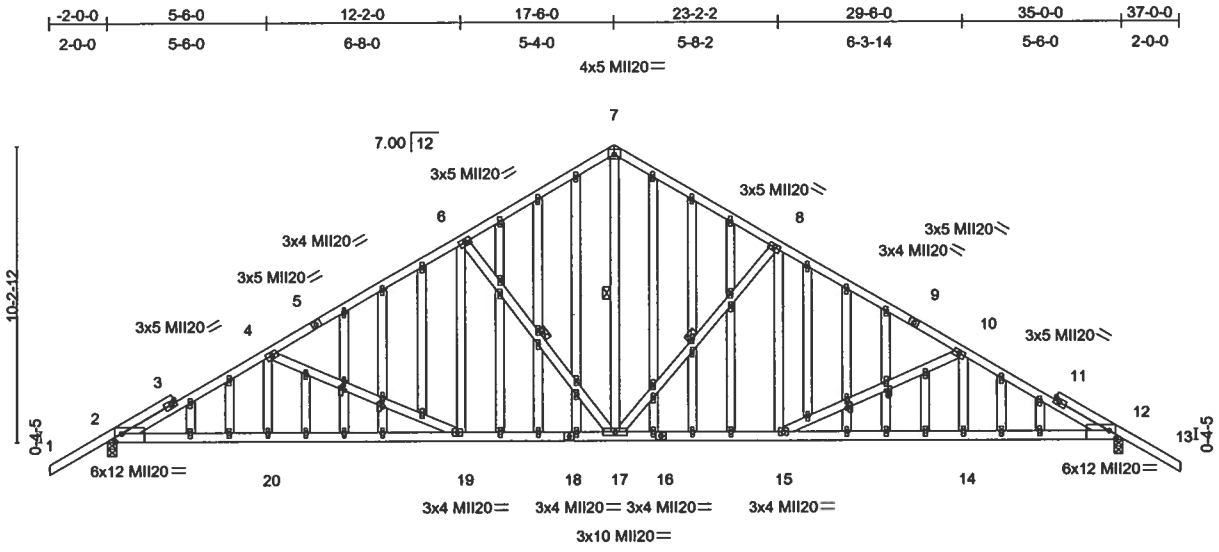


Plate Offsets (X,Y): [2:0-2-11,Edge], [12:0-2-11,Edge], [32:0-2-0,0-0-12], [35:0-2-0,0-0-12], [57:0-2-0,0-0-12], [60:0-2-0,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	0.24	19-20	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.30	19-20	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.12	12	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 341 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-15 oc bracing.
WEBS 1 Row at midpt 6-17, 7-17, 8-17

REACTIONS (lb/size) 2=1517/0-3-8, 12=1517/0-3-8
Max Horz 2=-441(load case 3)
Max Uplift2=-1168(load case 5), 12=-1168(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-2580/1743, 3-4=-2523/1761, 4-5=-1977/1333, 5-6=-1872/1363, 6-7=-1496/1124, 7-8=-1501/1120, 8-9=-1902/1383, 9-10=-2001/1355, 10-11=-2516/1756, 11-12=-2573/1737, 12-13=0/60
BOT CHORD 2-20=-1486/2210, 19-20=-1486/2210, 18-19=-978/1617, 17-18=-978/1617, 16-17=-929/1643, 15-16=-929/1643, 14-15=-1382/2201, 12-14=-1382/2201
WEBS 4-20=-182/266, 4-19=-647/609, 6-19=-377/447, 6-17=-650/566, 7-17=-953/1101, 8-17=-652/565, 8-15=-377/447, 10-15=-615/582, 10-14=-176/254

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MII20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 1-4-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1168 lb uplift at joint 2 and 1168 lb uplift at joint 12.

LOAD CASE(S) Standard

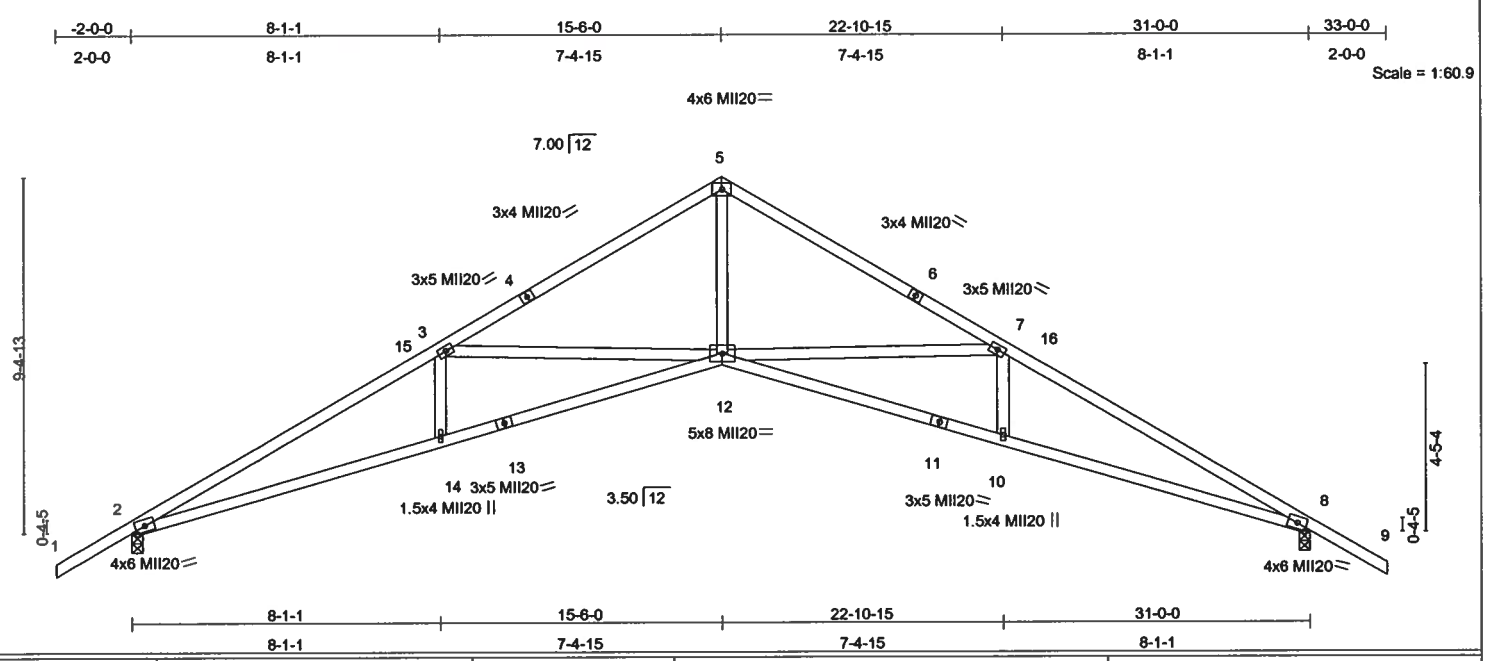
Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 3,2006

Job	Truss	Truss Type	Qty	Ply	
RSNMCA	B	ROOF TRUSS	7	1	T2381473

Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:01 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	0.49	12-14	>751	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.69	12-14	>536	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.56	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-6 oc bracing.

REACTIONS (lb/size) 2=1357/0-3-8, 8=1357/0-3-8
Max Horz 2=-406(load case 3)
Max Uplift2=-1064(load case 5), 8=-1064(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-15=-3631/2342, 3-15=-3385/2345, 3-4=-2515/1421, 4-5=-2402/1455, 5-6=-2402/1451, 6-7=-2515/1417, 7-16=-3385/2241, 8-16=-3631/2238, 8-9=0/58
BOT CHORD 2-14=-2012/3160, 13-14=-1920/3117, 12-13=-1909/3154, 11-12=-1737/3154, 10-11=-1748/3117, 8-10=-1827/3160
WEBS 3-14=-258/327, 3-12=-983/980, 5-12=-1238/1981, 7-12=-983/987, 7-10=-261/327

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1064 lb uplift at joint 2 and 1064 lb uplift at joint 8.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 3,2006

Job	Truss	Truss Type	Qty	Ply	
RSNMCA	BET	GABLE	1	1	T2381474
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:03 2006 Page 1

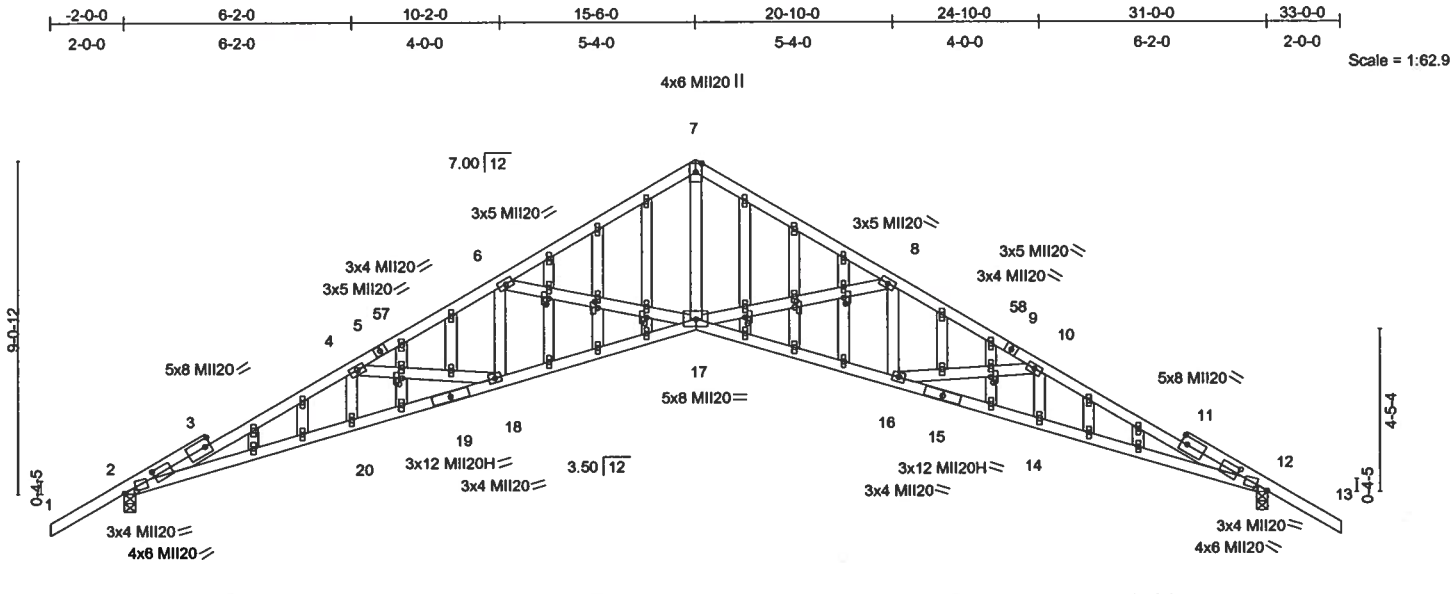


Plate Offsets (X,Y): [2:0-3-14,0-0-4], [2:0-11-3,0-1-12], [12:0-11-3,0-1-12], [12:0-3-14,0-0-4], [21:0-2-0,0-0-12], [24:0-2-0,0-0-12], [27:0-2-0,0-0-12], [32:0-2-0,0-0-12], [41:0-2-0,0-0-12], [44:0-2-0,0-0-12], [47:0-2-0,0-0-12], [52:0-2-0,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.65	17-18	>568	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.91	17-18	>405	180	MII20H	187/143
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.73	12	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-9-4 oc bracing.

REACTIONS (lb/size) 2=1357/0-3-8, 12=1357/0-3-8
Max Horz 2=-391(load case 3)
Max Uplift2=-1065(load case 5), 12=-1065(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-4394/2916, 3-4=-4339/2940, 4-5=-3537/2246, 5-57=-3513/2248, 6-57=-3498/2264, 6-7=-2628/1543, 7-8=-2628/1543, 8-58=-3498/2193, 9-58=-3513/2177, 9-10=-3536/2175, 10-11=-4339/2783, 11-12=-4395/2759, 12-13=0/58
BOT CHORD 2-20=-2580/3944, 19-20=-2527/3926, 18-19=-2520/3938, 17-18=-1713/3131, 16-17=-1616/3131, 15-16=-2326/3938, 14-15=-2333/3926, 12-14=-2386/3944
WEBS 4-20=-165/208, 4-18=-783/780, 6-18=-416/420, 6-17=-810/737, 7-17=-1353/2211, 8-17=-810/733, 8-16=-413/420, 10-16=-783/776, 10-14=-168/208

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 1.5x4 MII20 unless otherwise indicated.
7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
8) Gable studs spaced at 1-4-0 oc.
9) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1065 lb uplift at joint 2 and 1065 lb uplift at joint 12.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 3,2006

MiTek
POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	T2381475
RSNMCA	C	ROOF TRUSS	5	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:05 2006 Page 1

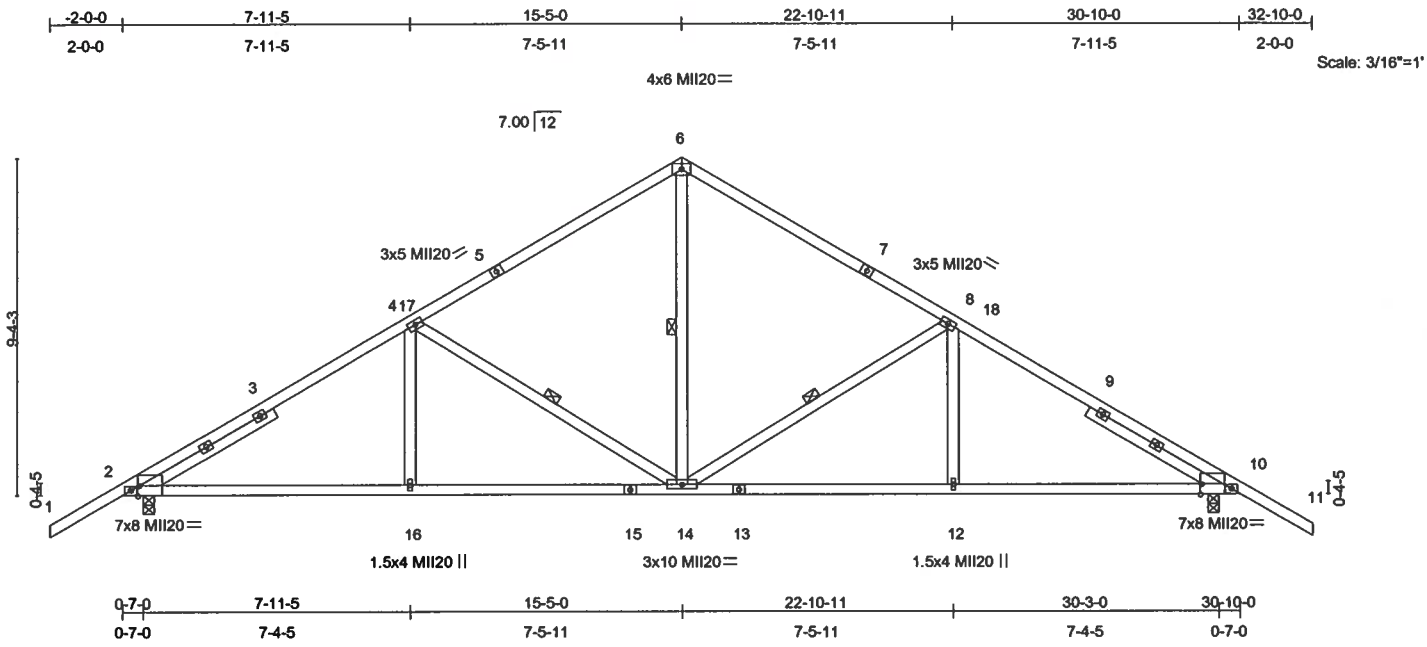


Plate Offsets (X,Y): [2:0-0-8,Edge], [10:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.18	2-16	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.20	2-16	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.08	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 4-2-15, Right 2 X 4 SYP No.3 4-2-15

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-10 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 8-14

REACTIONS (lb/size) 2=1350/0-3-8, 10=1350/0-3-8
Max Horz 2=-402(load case 3)
Max Uplift2=-1060(load case 5), 10=-1060(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-1938/1274, 3-17=-1813/1293, 4-17=-1698/1296, 4-5=-1337/934, 5-6=-1225/968, 6-7=-1225/968, 7-8=-1337/934, 8-18=-1698/1296, 9-18=-1813/1293, 9-10=-1938/1274, 10-11=0/60
BOT CHORD 2-16=-1035/1566, 15-16=-1035/1566, 14-15=-1035/1566, 13-14=-938/1566, 12-13=-938/1566, 10-12=-938/1566
WEBS 4-16=-220/315, 4-14=-624/587, 6-14=-736/782, 8-14=-624/587, 8-12=-221/315

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x4 MII20 unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1060 lb uplift at joint 2 and 1060 lb uplift at joint 10.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 3,2006

MiTek
POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	T2381476
RSNMCA	CET	GABLE	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:07 2006 Page 1

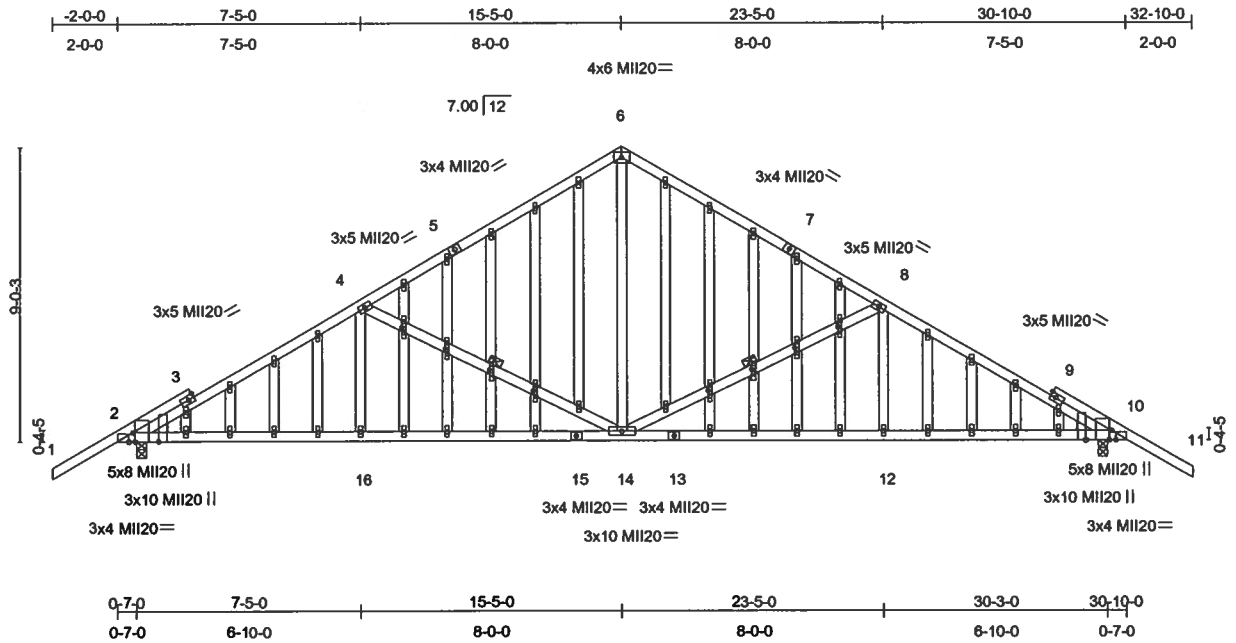


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-3-8,Edge], [2:0-1-9,Edge], [10:0-1-9,Edge], [10:0-3-8,Edge], [10:0-3-8,Edge], [19:0-1-12,0-0-12], [22:0-1-12,0-0-12], [25:0-1-12,0-0-12], [28:0-1-12,0-0-12], [43:0-1-12,0-0-12], [46:0-1-12,0-0-12], [49:0-1-12,0-0-12], [52:0-1-12,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.21	14-16	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.26	14-16	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.08	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 272 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-11 oc bracing.
WEBS 1 Row at midpt 4-14, 8-14

REACTIONS (lb/size) 2=1344/0-3-8, 10=1344/0-3-8
Max Horz 2=-383(load case 3)
Max Uplift2=-1063(load case 5), 10=-1063(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/67, 2-3=-1979/1299, 3-4=-1912/1332, 4-5=-1347/916, 5-6=-1226/952, 6-7=-1226/954, 7-8=-1347/917, 8-9=-1912/1338, 9-10=-1979/1304, 10-11=0/67
BOT CHORD 2-16=-1085/1653, 15-16=-1087/1652, 14-15=-1087/1652, 13-14=-1011/1652, 12-13=-1011/1652, 10-12=-1009/1653
WEBS 4-16=-206/301, 4-14=-683/619, 6-14=-702/785, 8-14=-683/622, 8-12=-210/301

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MII20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 1-4-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1063 lb uplift at joint 2 and 1063 lb uplift at joint 10.

LOAD CASE(S) Standard

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October 3,2006

MiTek
POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	T2381477
RSNMCA	D	FINK	12	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:08 2006 Page 1

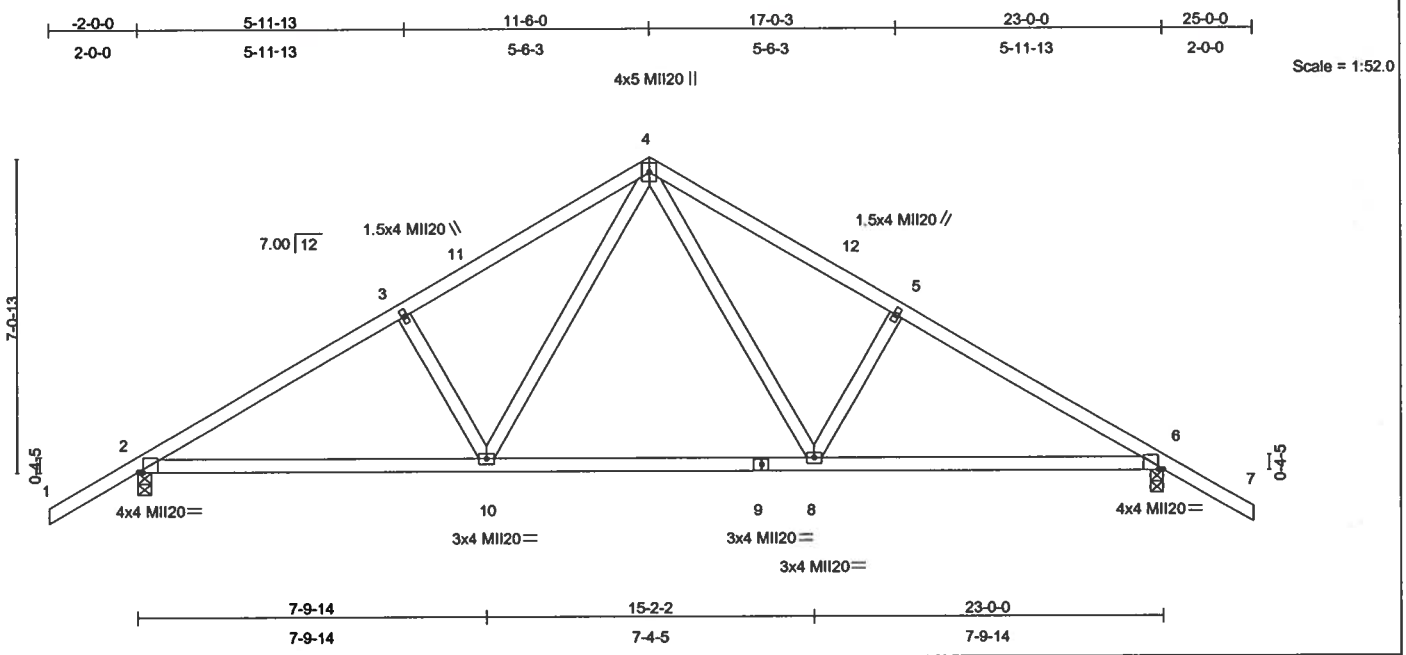

Plate Offsets (X,Y): [2:0-1-5,0-0-2], [6:0-1-5,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.16	2-10	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.37	8-10	>746	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.33	Horz(TL)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-9-8 oc bracing.

REACTIONS (lb/size) 2=1184/0-3-8, 6=1184/0-3-8
Max Horz 2=303(load case 4)
Max Uplift2=-703(load case 5), 6=-703(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-1701/638, 3-11=-1531/652, 4-11=-1448/678, 4-12=-1448/678, 5-12=-1531/653, 5-6=-1701/638, 6-7=0/60
BOT CHORD 2-10=-478/1380, 9-10=-195/945, 8-9=-195/945, 6-8=-407/1380
WEBS 3-10=-286/315, 4-10=-315/654, 4-8=-315/654, 5-8=-286/316

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 703 lb uplift at joint 2 and 703 lb uplift at joint 6.
6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-10=-20, 8-10=-60(F=-40), 6-8=-20, 1-4=-60, 4-7=-60
2) IBC BC Live: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-10=-40, 8-10=-80(F=-40), 6-8=-40, 1-4=-20, 4-7=-20
3) MWFRS Wind Left: Lumber Increase=1.33, Plate Increase=1.33

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Continued on page 2

LOAD CASE(S) Standard

Uniform Loads (plf)
Vert: 2-10=27, 8-10=-13(F=-40), 6-8=27, 1-2=12, 2-4=-15, 4-6=29, 6-7=19
Horz: 1-2=-22, 2-4=5, 4-6=39, 6-7=29

4) MWFRS Wind Right: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 2-10=27, 8-10=-13(F=-40), 6-8=27, 1-2=19, 2-4=29, 4-6=-15, 6-7=12
Horz: 1-2=-29, 2-4=-39, 4-6=-5, 6-7=22

5) MWFRS 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 2-10=22, 8-10=-18(F=-40), 6-8=22, 1-2=71, 2-11=48, 4-11=30, 4-6=30, 6-7=22
Horz: 1-2=-81, 2-11=-58, 4-11=-40, 4-6=40, 6-7=32

6) MWFRS 2nd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 2-10=22, 8-10=-18(F=-40), 6-8=22, 1-2=22, 2-4=30, 4-12=30, 6-12=48, 6-7=71
Horz: 1-2=-32, 2-4=-40, 4-12=40, 6-12=58, 6-7=81

7) MWFRS 3rd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 2-10=22, 8-10=-18(F=-40), 6-8=22, 1-2=46, 2-11=23, 4-11=16, 4-6=16, 6-7=7
Horz: 1-2=-56, 2-11=-33, 4-11=-26, 4-6=26, 6-7=17

8) MWFRS 4th Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 2-10=22, 8-10=-18(F=-40), 6-8=22, 1-2=7, 2-4=16, 4-12=16, 6-12=23, 6-7=46
Horz: 1-2=-17, 2-4=-26, 4-12=26, 6-12=33, 6-7=56

9) Attic Floor: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
Uniform Loads (plf)
Vert: 2-10=-20, 8-10=-60(F=-40), 6-8=-20, 1-4=-20, 4-7=-20

10) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-10=-20, 8-10=-60(F=-40), 6-8=-20, 1-4=-60, 4-7=-20

11) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-10=-20, 8-10=-60(F=-40), 6-8=-20, 1-4=-20, 4-7=-60

Job	Truss	Truss Type	Qty	Ply	T2381478
RSNMCA	DET	GABLE	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:09 2006 Page 1

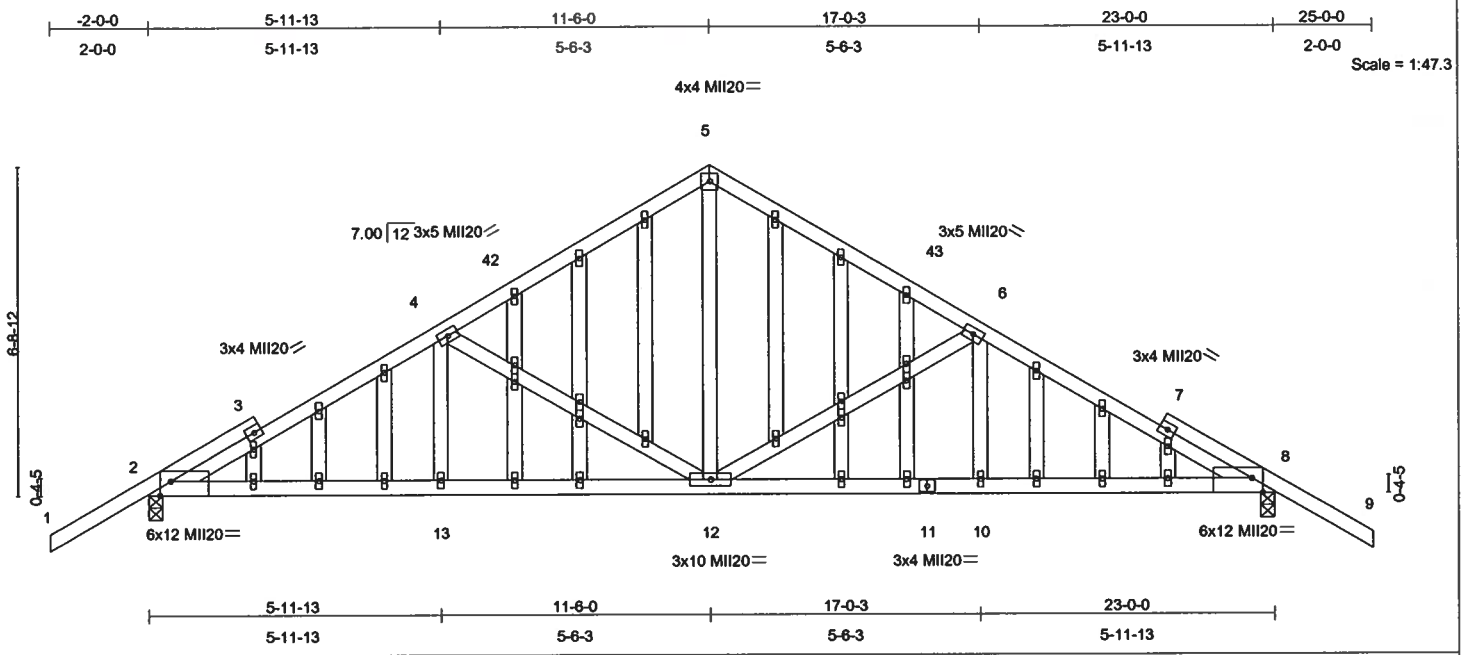

Plate Offsets (X,Y): [2:0-2-11,Edge], [8:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.11	2-13	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.13	10-12	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.05	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-3 oc bracing.

REACTIONS (lb/size) 2=1037/0-3-8, 8=1037/0-3-8
Max Horz 2=288(load case 4)
Max Uplift2=-852(load case 5), 8=-852(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-1521/1031, 3-4=-1465/1054, 4-42=-1026/729, 5-42=-938/754, 5-43=-938/754, 6-43=-1026/729, 6-7=-1465/1054, 7-8=-1521/1032, 8-9=0/60
BOT CHORD 2-13=-855/1269, 12-13=-855/1269, 11-12=-783/1269, 10-11=-783/1269, 8-10=-783/1269
WEBS 4-13=-170/245, 4-12=-534/519, 5-12=-592/635, 6-12=-534/520, 6-10=-171/245

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MII20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 1-4-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 852 lb uplift at joint 2 and 852 lb uplift at joint 8.

LOAD CASE(S) Standard

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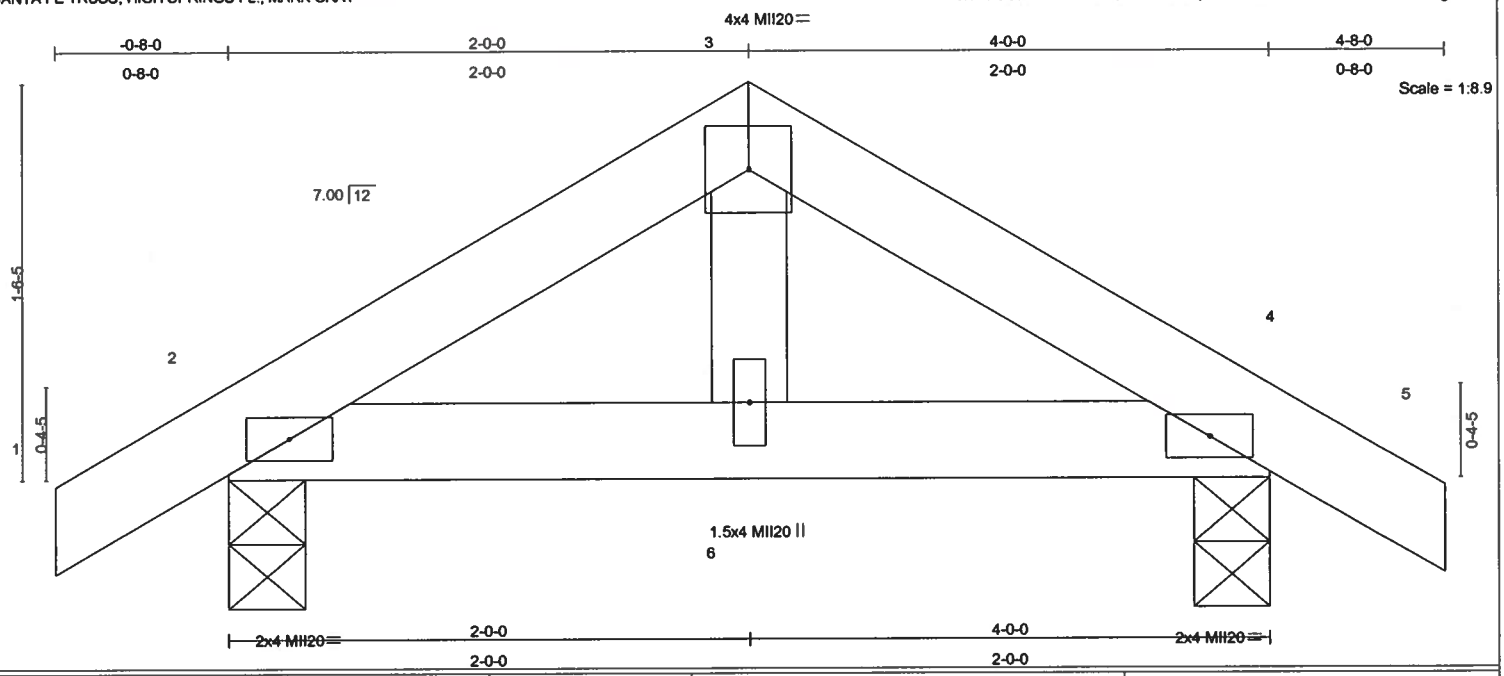
October 3,2006

Job	Truss	Truss Type	Qty	Ply	T2381479
RSNMCA	E	ROOF TRUSS	8	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:10 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.05	Vert(LL)	-0.00	6	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	6	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	4	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=197/0-3-8, 4=197/0-3-8
Max Horz 2=-55(load case 3)
Max Uplift2=-190(load case 5), 4=-190(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/19, 2-3=-156/114, 3-4=-156/114, 4-5=0/19
BOT CHORD 2-6=-58/100, 4-6=-58/100
WEBS 3-6=-61/87

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 2 and 190 lb uplift at joint 4.

LOAD CASE(S) Standard

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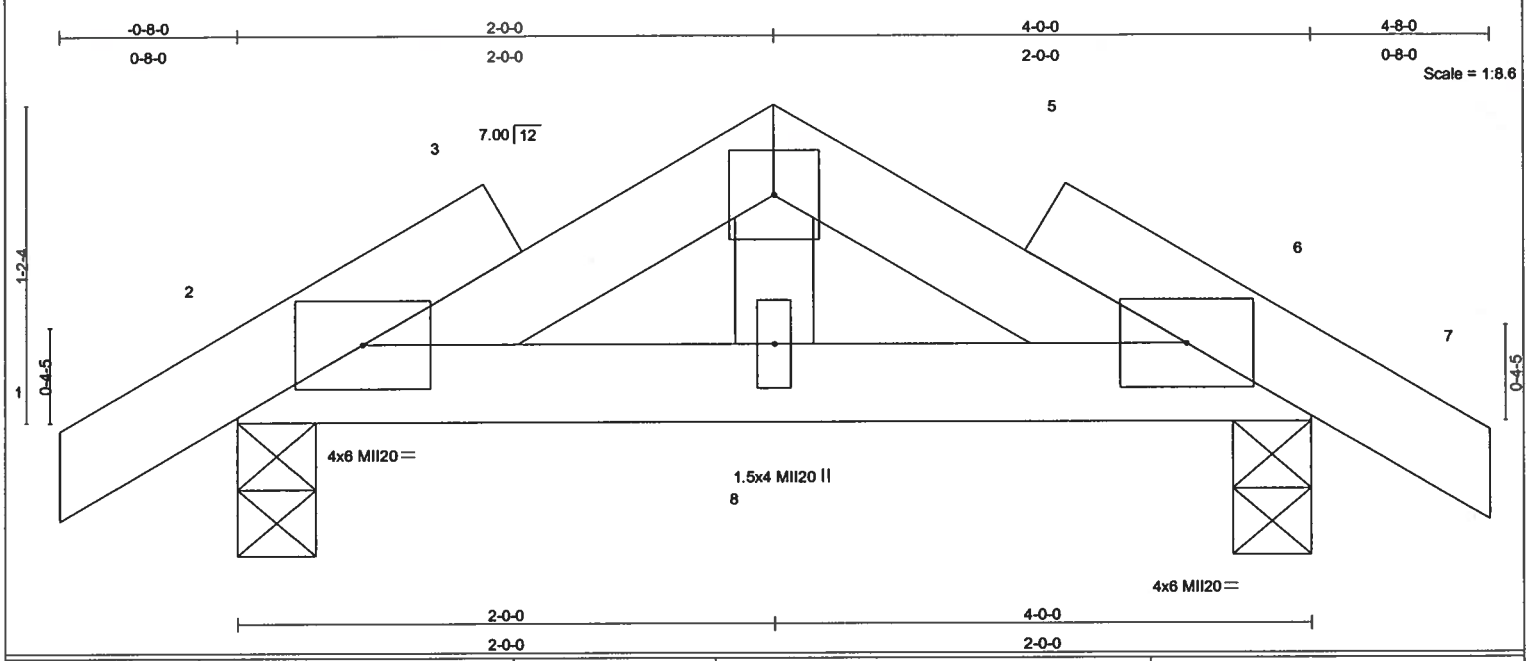
FL Cert.#6634

October 3,2006

Job	Truss	Truss Type	Qty	Ply	T2381480
RSNMCA	EET	KINGPOST	2	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY — 4x4 MII20 — 6.300 s Jul 11 2006 MiTek Industries, Inc. Tue Oct 03 07:38:11 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	-0.00	2	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	8	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=197/0-3-8, 6=197/0-3-8
Max Horz 2=40(load case 4)
Max Uplift2=-190(load case 5), 6=-190(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/19, 2-3=-171/126, 3-4=-151/132, 4-5=-151/132, 5-6=-171/127, 6-7=0/19
BOT CHORD 2-8=-72/134, 6-8=-72/134
WEBS 4-8=-59/84

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 2 and 190 lb uplift at joint 6.

LOAD CASE(S) Standard

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MiTek Industries, Inc.
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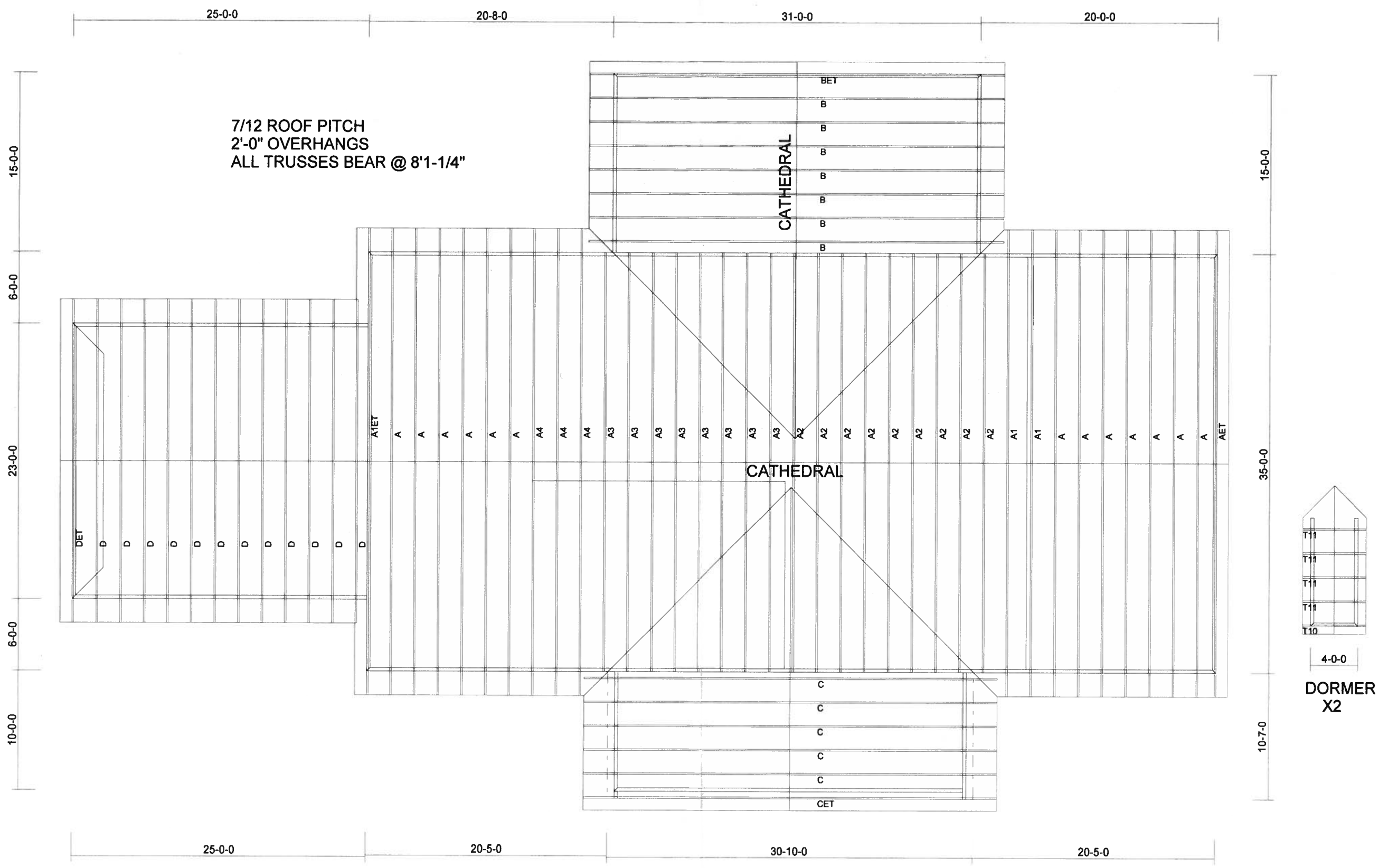
Tampa FL 33619
FL Cert #6634

October 3,2006



Santa Fe Truss

410 SW POE SPRINGS RD.
HIGH SPRINGS, FL 32655
FX#(386)454-1055 PH#(386)454-7711

CUSTOMER	ROSENBOOM CONSTRUCTIOM
DATE	9/28/2006
JOB NAME	McALHANY (RSNMCALHANY)
DESIGNER	P.A.C.