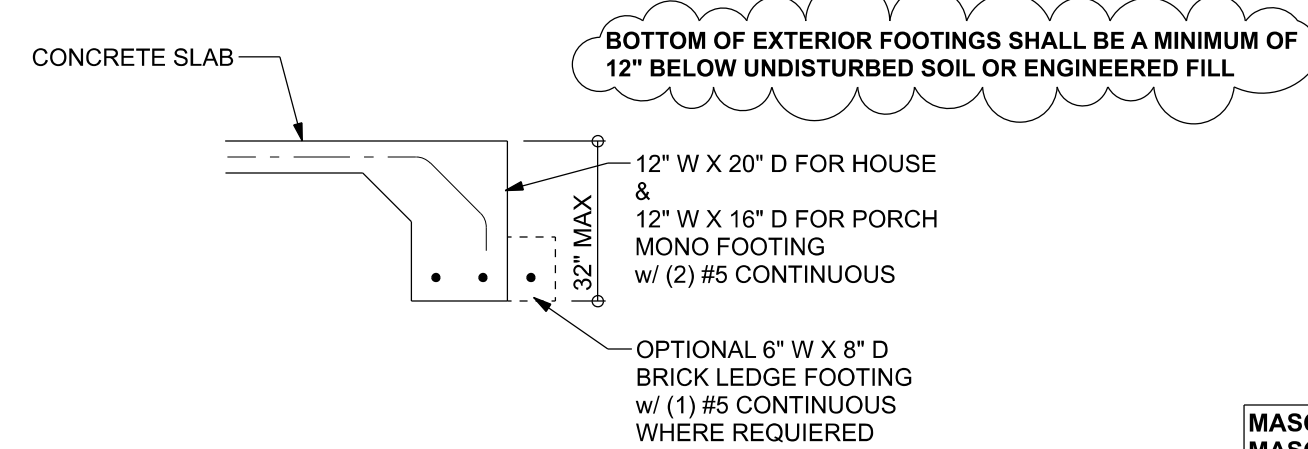
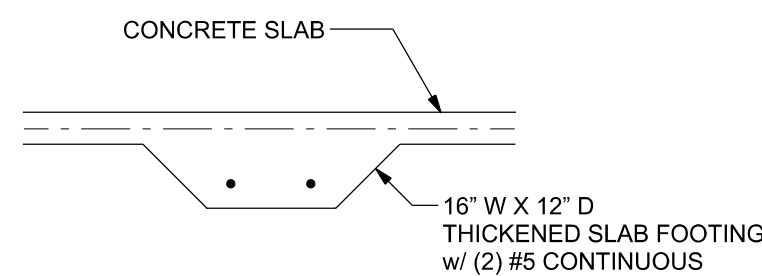


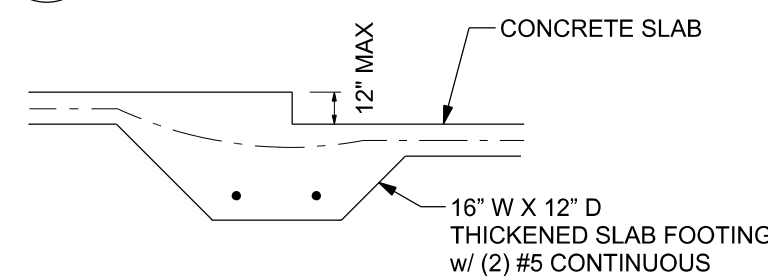
F1 S-2 OPTIONAL STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

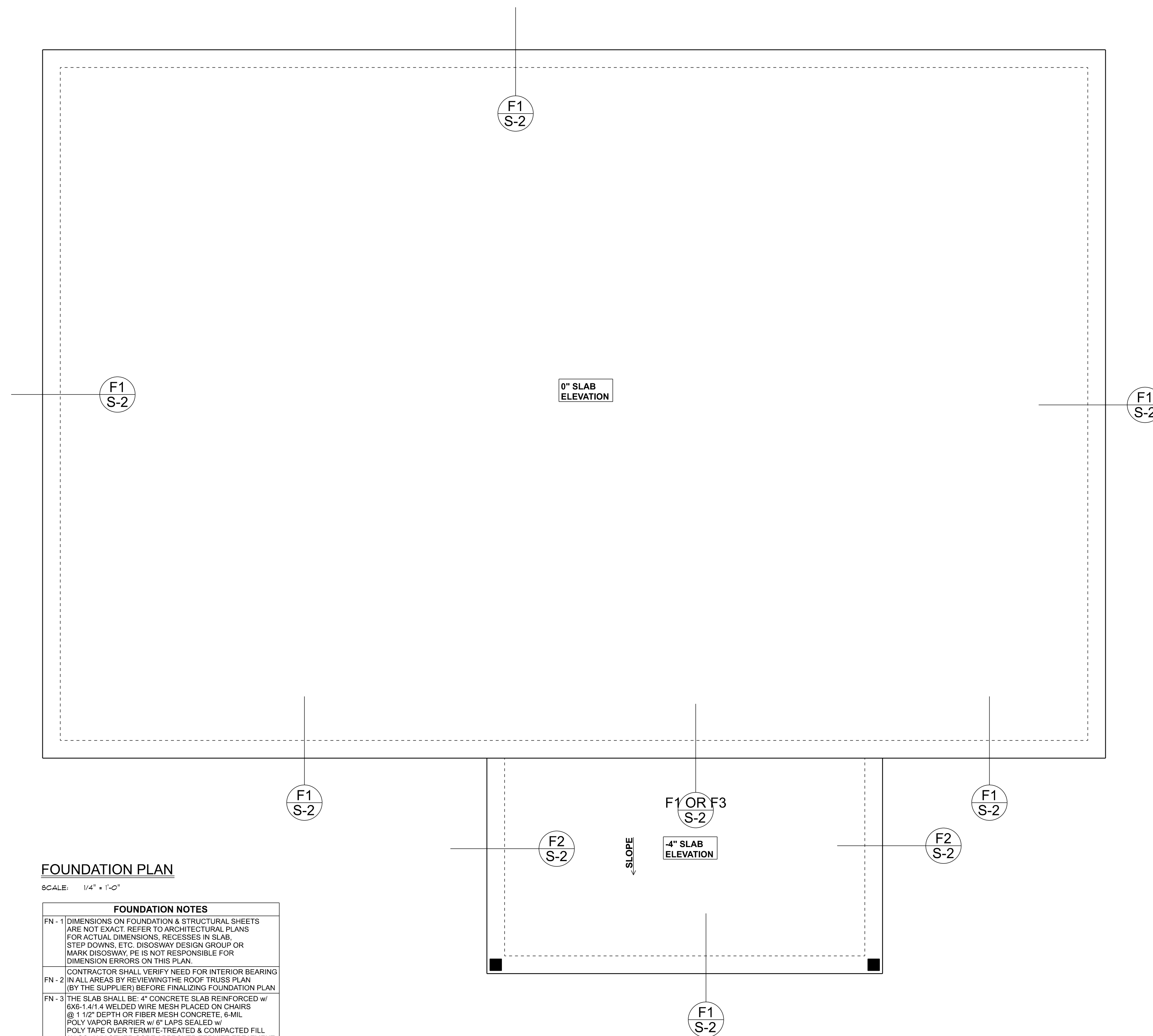
TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength 8" block bearing walls F _m = 1500 psi
2.1	Mortar ASTM C 270, Type N, UNO
2.2	Grout ASTM C 476, admixtures require approval
2.3	CMU standard ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 40, F _y = 40 ksi, Lap splices min 40 bar dia. (20" for #5)
2.4F	Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class Q95, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

FN - 2 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-14/14 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY W/PPR BARRIER w/ 7 LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES TREATMENT METHOD CAN BE USED INSTEAD).

Hunter & Ashley Faulkner Res.
PROJECT ADDRESS:
FL White, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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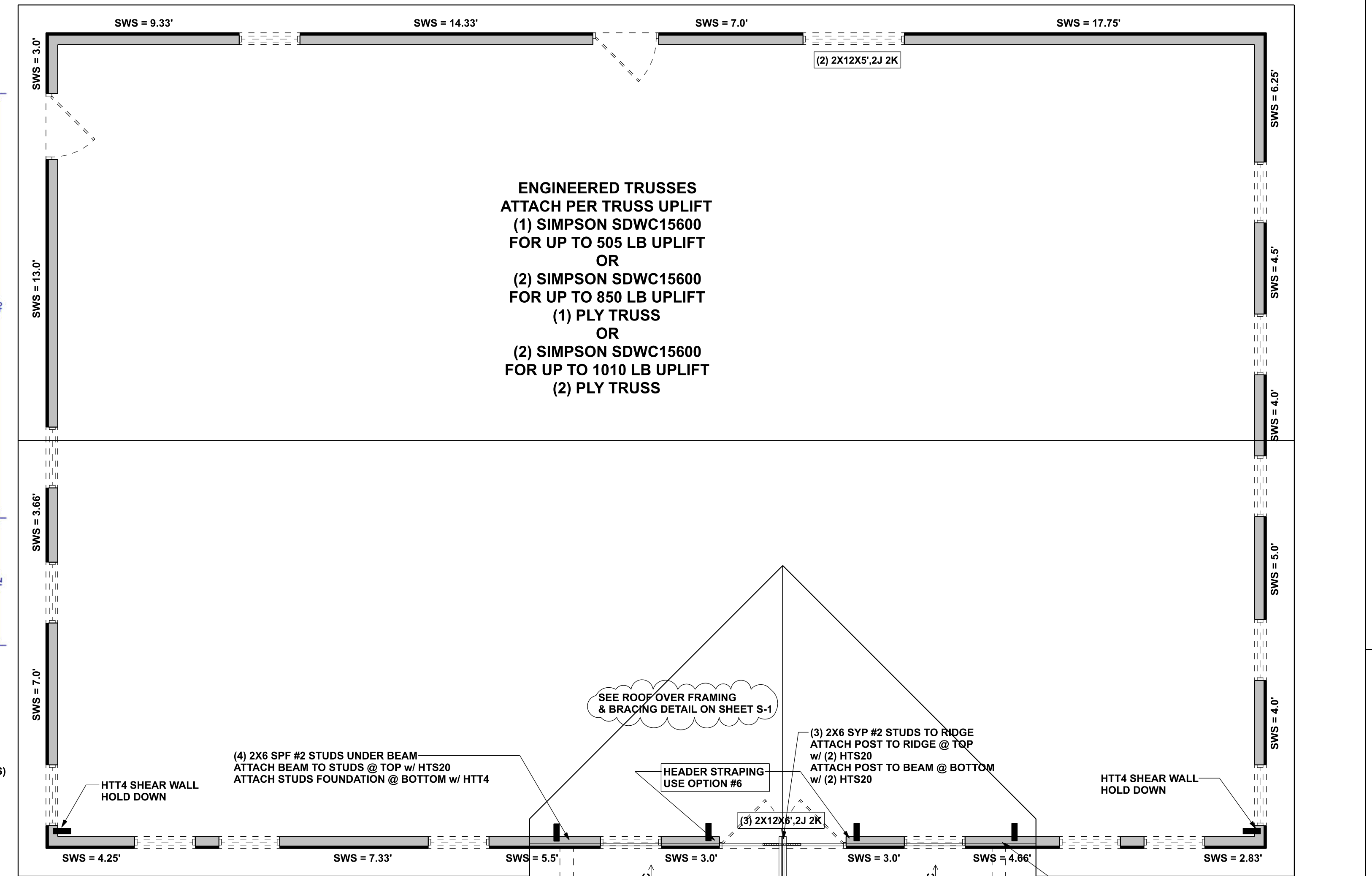
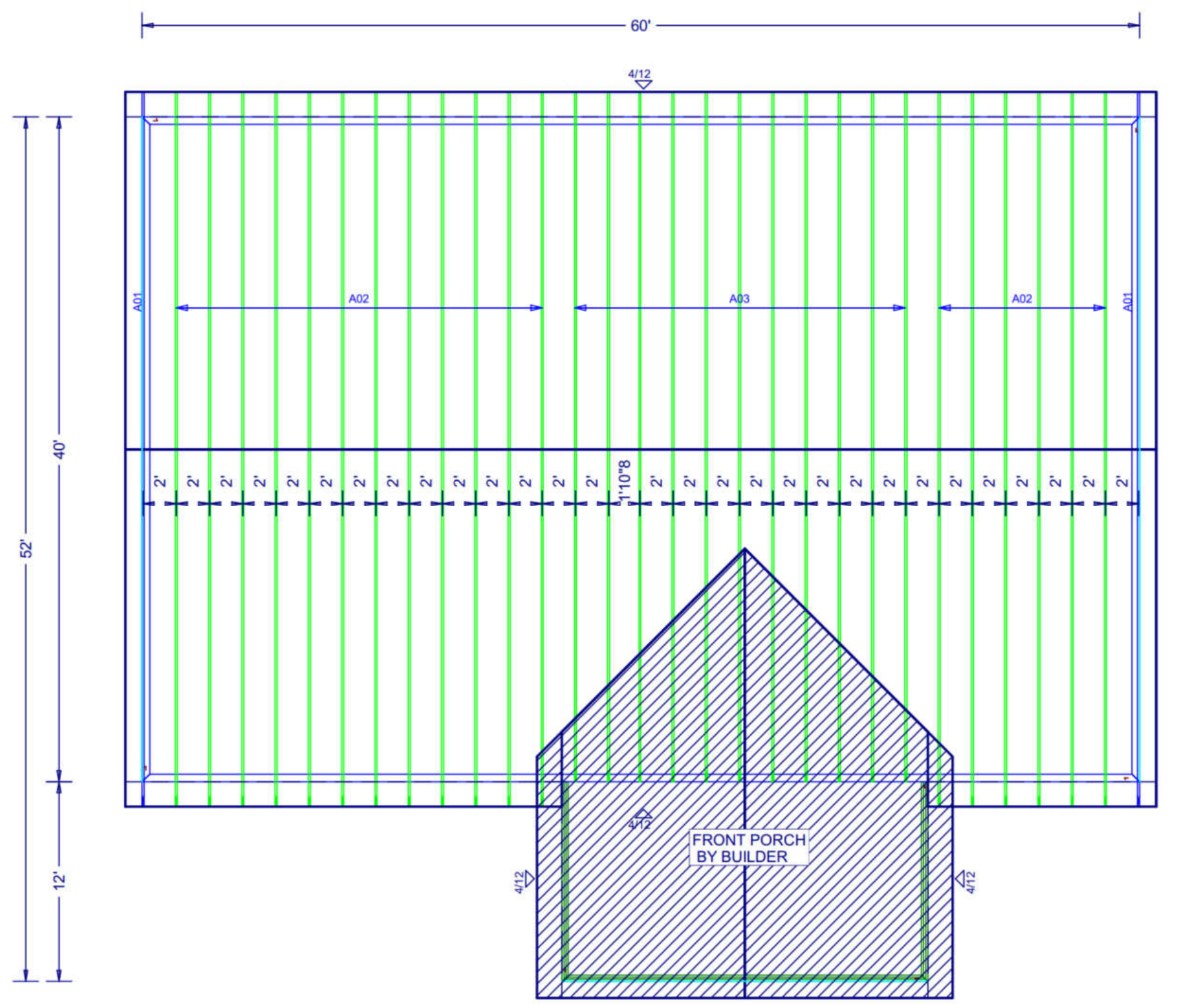
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

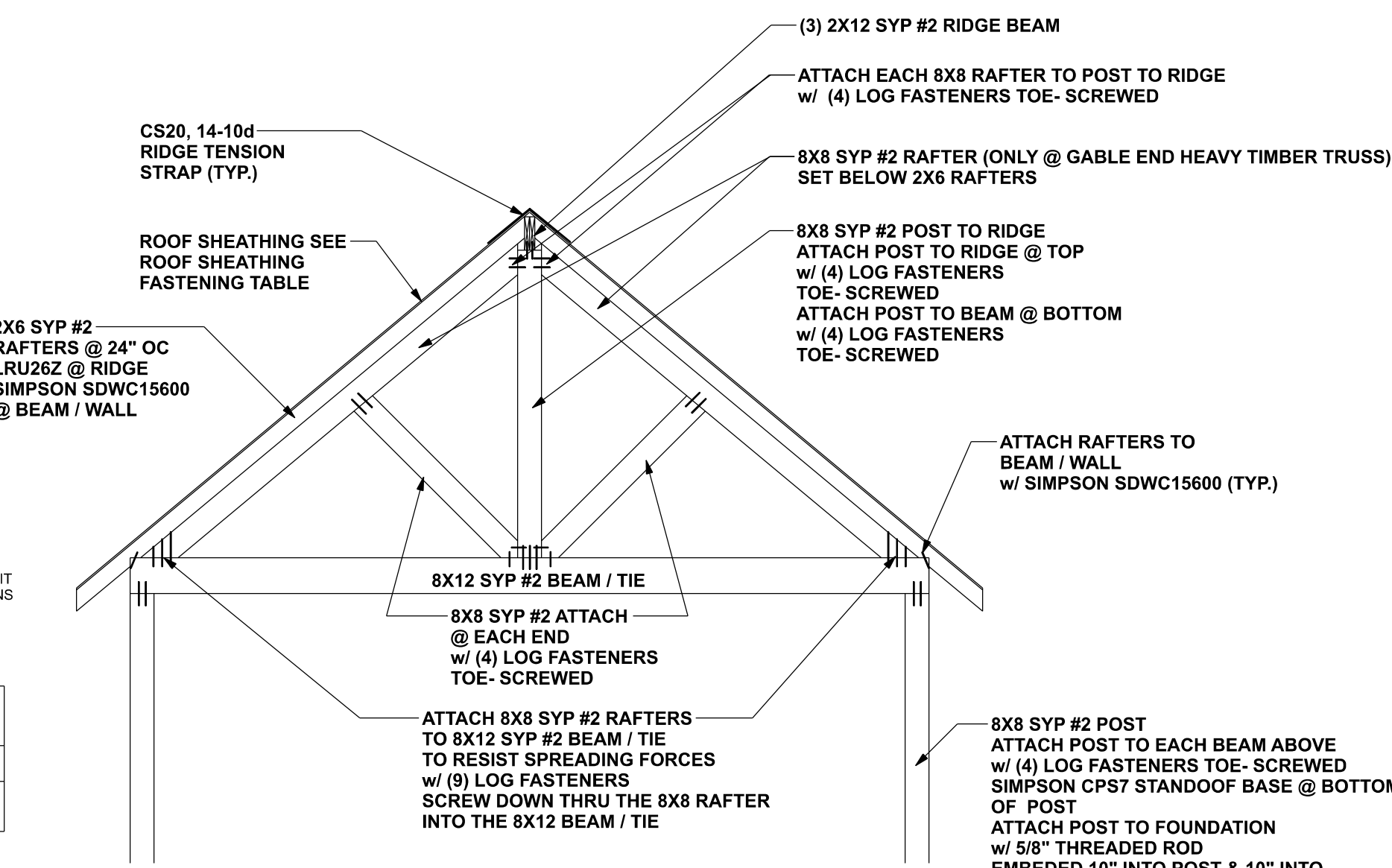
Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
250613
S-2
OF 3 SHEETS



**ENGINEERED TRUSSES
ATTACH PER TRUSS UPLIFT**
**(1) SIMPSON SDWC15600
FOR UP TO 505 LB UPLIFT
OR**
**(2) SIMPSON SDWC15600
FOR UP TO 850 LB UPLIFT
(1) PLY TRUSS
OR**
**(2) SIMPSON SDWC15600
FOR UP TO 1010 LB UPLIFT
(2) PLY TRUSS**

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"



HEAVY TIMBER ROOF SECTION @ PORCH
SCALE: 1/4" = 1'-0"

LOG HOG FASTENERS ARE SELF DRILLING, HIGH STRENGTH, F1 = 111ksi, STEEL WOOD SCREWS WITH 0.228 SHANK DIAMETER. THE FOLLOWING DESIGN VALUES ARE BASED ON CODE TEST REPORT BY ICC EVALUATION SERVICE, INC. AND NDS 2001:

LOG HOG FASTENERS ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT FASTENER OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. LISTED LOADS HAVE BEEN INCREASED FOR WIND DURATION, AND MUST BE ADJUSTED FOR OTHER DURATIONS OR SPECIES.

LOG HOG FASTENERS DESIGN VALUE FOR 1.6 WIND DURATION (LB) BASED ON 3" MIN. PENETRATION INTO MAIN MEMBER & 2.5" MIN. SIDE MEMBER

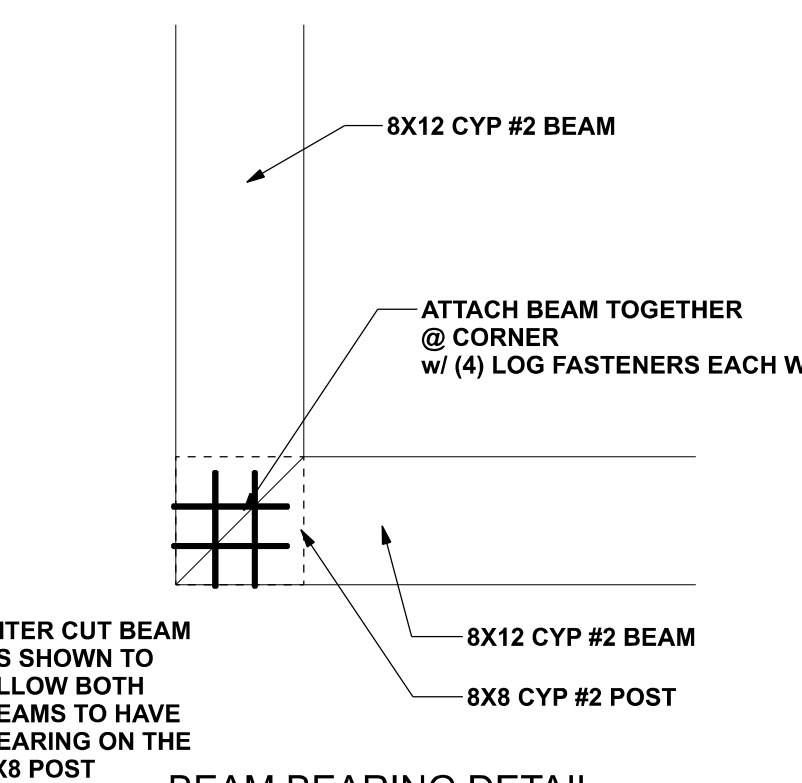
WOOD SPECIES (SPECIFIC GRAVITY)	SHINGLE SHEAR DESIGN VALUE PARALLEL TO GRAIN	SHINGLE SHEAR DESIGN VALUE PERPENDICULAR TO GRAIN	WITHDRAWAL DESIGN VALUE
SYP (.55 SG)	502	477	1060
CYP (.43 SG) SPF (.42 SG)	438	387	710

ALLOWABLE WITHDRAWAL STRENGTH AND SINGLE SHEAR RESISTANCE BASED ON VALUES LISTED IN ICC ES REPORT (ESR-1078 REISSUED FEB. 1, 2007)

ADJUSTMENT FACTORS:
 WITHDRAWAL VALUES ARE BASED ON 3" PENETRATION INTO MAIN MEMBER.
 WHEN PENETRATION (D) IS LESS THAN 3" VALUE SHALL BE MULTIPLIED BY: p3"
 SHEAR VALUES ARE BASED ON 100 MIN. PENETRATION INTO MAIN MEMBER.
 WHEN PENETRATION (D) IS 60-99/100 SHEAR VALUE SHALL BE MULTIPLIED BY: p1/100
 TABLE VALUES ARE FOR 1.6 WIND LOAD-DURATIONS. ADJUST FOR OTHER DURATIONS.
 END GRAIN FACTOR (NDS 11.5.2)
 WITHDRAWAL LOADS MULTIPLY BY: = .75, LATERAL LOADS MULTIPLY BY: .67
 TOE-SCREW FACTOR (NDS 11.5.4)
 WITHDRAWAL LOADS MULTIPLY BY: = .67, LATERAL LOADS MULTIPLY BY: .83

CONNECTION GEOMETRY MINIMUM CRITERIA (PER ER REPORT):
 - MIN. EDGE DISTANCE (LOADED EDGE) = 8d + 1.5d
 - MIN. END DISTANCE, TENSION LOAD PARALLEL TO GRAIN = 16d + 3.3d
 - COMPRESSION LOAD PARALLEL TO GRAIN = 10d + 2.3d
 - SPACING BETWEEN FASTENERS IN A ROW, PARALLEL TO GRAIN = 15d + 3.12"
 - PERPENDICULAR TO GRAIN = 10d + 2.3d
 - SPACING BETWEEN ROWS OF FASTENERS, IN LINE = 5d + 1.1d
 - SPACING BETWEEN ROWS OF FASTENERS, STAGGERED = 2.5d + 5/8"

N21 LOG HOG FASTENER DATA
TYPICAL DESIGN VALUES



BEAM BEARING DETAIL
SCALE: 1" = 1'-0"

NOTE:
ALL MUST HAVE 3" MIN. PENETRATION INTO MAIN MEMBER & 2.5" MIN. SIDE MEMBER
SEE N21 LOG HOG FASTENER DATA THIS SHEET FOR MORE DETAILS

Simpson Strong-Tie® Wood Construction Connectors
CPS/PBV

Standoff Bases

The PBV is a hidden standoff post base. Two different shapes fit a variety of post sizes. The CPS is a composite plastic standoff designed for increased concrete surface area.

Material: PBV – Galvanized with black powder coat; CPS – engineered composite polymer
Finish: PBV – Galvanized with black powder coat
To Order: For black powder coat, order PBV/PC or PBV/OPC. For galvanized coating, order PBV/G or PBV/10.

Installation:
PBV and CPS:
Post:
 • Drill a 1/2" diameter hole, 10" into the center of the post.
 • Clean out dust. Fill hole halfway with Simpson Strong-Tie SET-3G™ anchoring adhesive.
 • Insert all-thread rod and allow epoxy to set and cure.
 • Secure standoff to post using four 0.148" x 3" nails except PBV which uses four Strong-Drive® SDS Heavy-Duty Connector screws (sold separately).

Concrete:
 • Drill a 1/2" diameter hole per anchor design (see footnote 2 below).
 • Clean out dust. Fill hole halfway with Simpson Strong-Tie SET-3G, ET-3G™ or AT-3G™ anchoring adhesive. Insert post subassembly into hole and allow epoxy to set and cure.
 • Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non-top-supported installations (such as fences or unbraced carports).

Codes: See p. 13 for Code Reference Key Chart

ACTUAL vs REQUIRED SHEAR WALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	15123 LBF	18955 LBF
REQUIRED	12249 LBF	7832 LBF

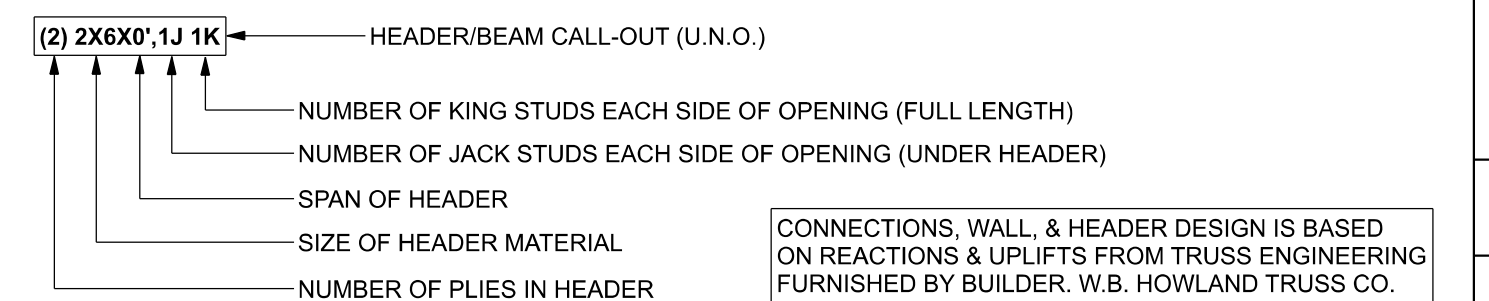
STRUCTURAL PLAN NOTES

- SN-1** DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2** PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSD-03, BCSD-01, BCSD-02, & BCSD-03. BCSD-01, BCSD-02, & BCSD-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS)
*****SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS*****

BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCHEDULED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



Hunter & Ashley Faulkner Res.
 PROJECT ADDRESS:
 Ft. White, FL

FL PE 53915
 This item has been digitally signed and sealed by Mark Disosway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
 163 SW Midtown Place
 Suite 103
 Lake City, Florida 32025
 386.754.5419
 disoswaydesign@gmail.com

JOB NUMBER:
 250613
S-3
 OF 3 SHEETS