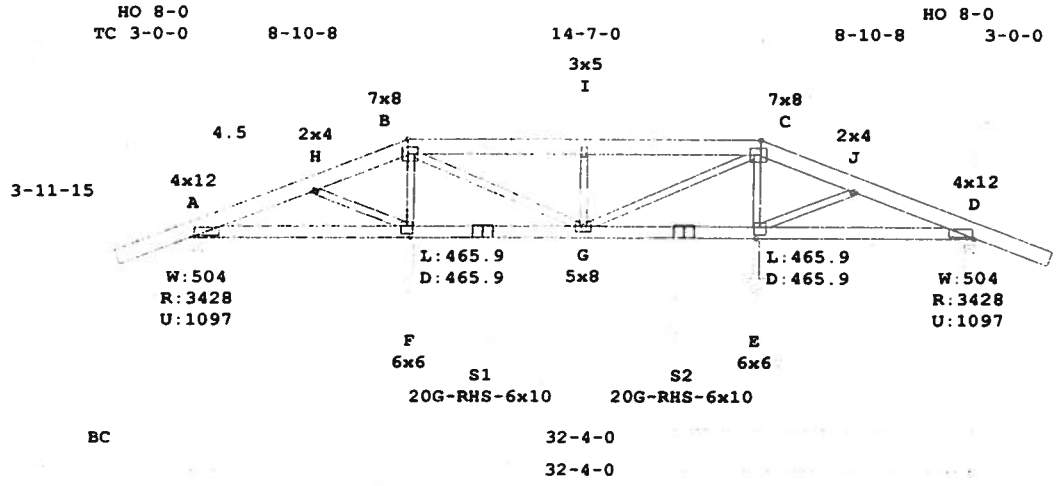
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R1*	1	HIPP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 305.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 8	SP-2400
BC	0.49	2x 6	SP-2400
WB	0.76	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	32.3'
BC V 0	20	0.0'	32.3'
TC V 69	34	8.9'	23.5'
BC V 0	34	9.0'	23.3'
BC V 466	466	9.0'	CL-LB
BC V 466	466	23.3'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	3428	1097	5- 4	2-13
			Hz =	-38
D	3428	1097	5- 4	2-13
			Hz =	39

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -H	0.28	7499 C	0.07	0.21
H -B	0.29	7741 C	0.08	0.21
B -I	0.35	9072 C	0.12	0.23
I -C	0.35	9072 C	0.12	0.23
C -J	0.29	7741 C	0.08	0.21
J -D	0.28	7499 C	0.07	0.21
Bottom Chords				
A -F	0.39	6904 T	0.34	0.05
F -S1	0.48	7272 T	0.36	0.12
S1-G	0.49	7272 T	0.36	0.13
G -S2	0.49	7272 T	0.36	0.13
S2-E	0.48	7272 T	0.36	0.12
E -D	0.39	6904 T	0.34	0.05
Webs				
H -F	0.14	440 T		
F -B	0.32	1074 T		
B -G	0.76	1972 T		
G -I	0.24	1201 C		
G -C	0.76	1972 T		
E -C	0.32	1074 T		
E -J	0.14	440 T		

LL Defl -0.31" in S1-G L/999
TL Defl -0.64" in S1-G L/586
Shear // Grain in B -I 0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

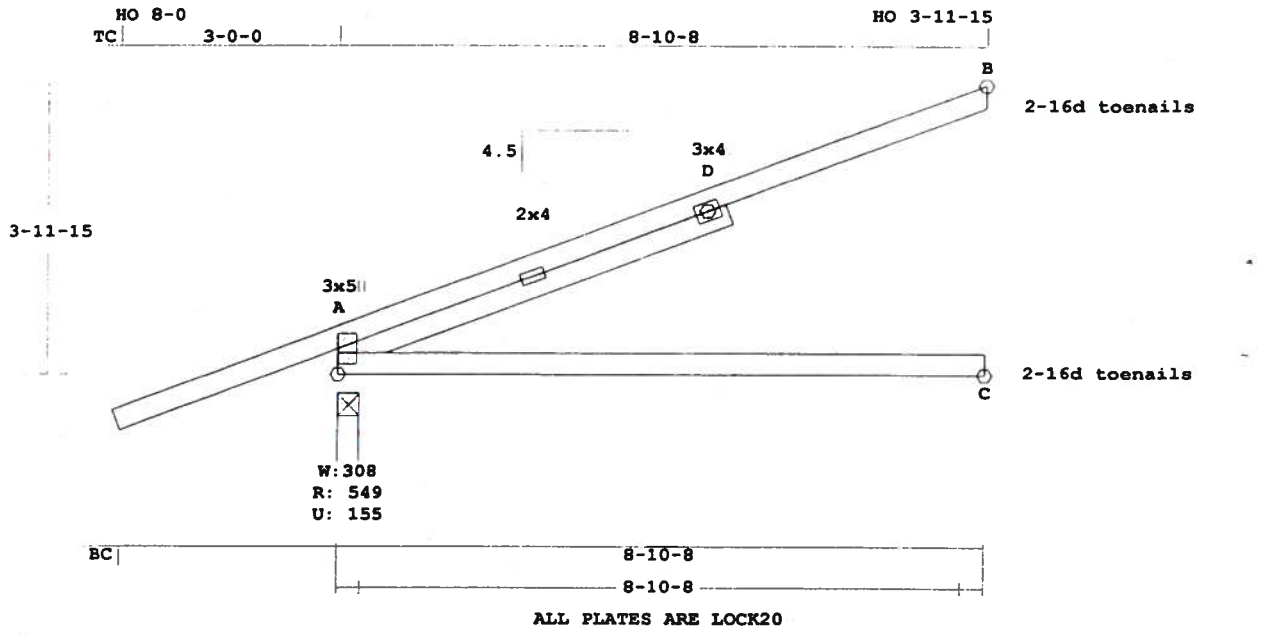
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x12.0	Ctr	Ctr	0.95
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	7.0x 8.0-0.5	Ctr		0.91
I	LOCK	3.0x 5.0	Ctr	1.1	0.50
C	LOCK	7.0x 8.0 0.5	Ctr		0.91
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
D	LOCK	4.0x12.0	Ctr	Ctr	0.95
F	LOCK	6.0x 6.0	Ctr	-0.8	0.54
S1	RHS	6.0x10.0	Ctr	0.2	0.97
G	LOCK	5.0x 8.0	Ctr	-0.2	0.84
S2	RHS	6.0x10.0	Ctr	0.2	0.97
E	LOCK	6.0x 6.0	Ctr	-0.8	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 8-10- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9072 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R2*	9	JACK	81008	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.88	2x 4	SP-#2
BC	0.51	2x 4	SP-#2
SL	0.19	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8-10- 8
BC	Cont.	0- 0- 0	8-10- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	549	155	3- 8	1- 0
			Hz =	150
B	262	170	3- 8	1- 0
C	150	0G	3- 8	1- 0
			Hz =	80

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
A -D	0.88	687 C	0.00 0.88
D -B	0.88	130 C	0.00 0.88
A -C	0.51	80 T	0.00 0.51
A -D	0.19	690 T	

Top Chords
Bottom Chords
Sliders

LL Defl -0.14" in A -C L/691
TL Defl -0.35" in A -C L/281
Shear // Grain in A -D 0.37

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.72
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

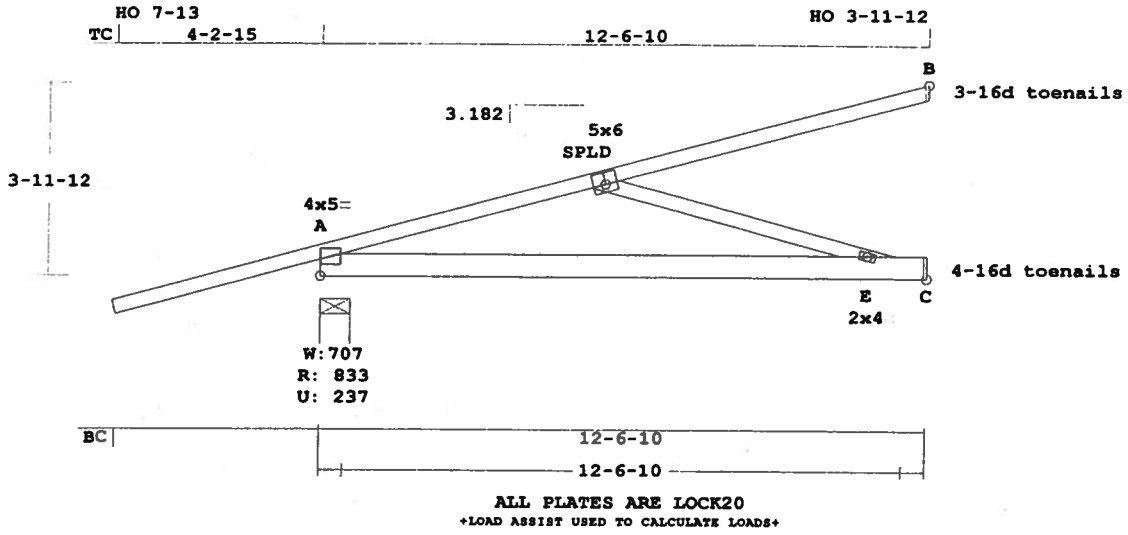
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 687 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R3	2	KING	120610	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 80.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	1.00	2x 4	SP-#2
BC	0.81	2x 6	SP-#2
WB	0.81	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 6-10
BC	Cont.	0- 0- 0	12- 6-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	12.6'
BC V	0	20	0.0'	12.6'
TC V	-40	-20	0.0'	
	71	36		12.6'
BC V	0	-20	0.0'	
	0	36		12.6'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	833	238	7- 7	1- 0
			Hz = 209	
B	434	298	3- 8	1- 0
C	572	41	3- 8	1- 0

Hz = 112

Membr	CSI	P Lbs	Axl-CSI-Bnd	
		Top Chords		
A -D	0.85	1197 C	0.01	0.84
D -B	1.00	143 C	0.00	1.00
		Bottom Chords		
A -E	0.81	1187 T	0.06	0.75
E -C	0.11	112 T	0.00	0.11
		Webs		
D -E	0.81	1229 C		

LL Defl -0.32" in A -C L/436
TL Defl -0.62" in A -C L/225
Shear // Grain in D -B 0.51

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.68
D	LOCK	5.0x 6.0-0.1	0.5		0.87
E	LOCK	2.0x 4.0	Ctr	Ctr	0.62

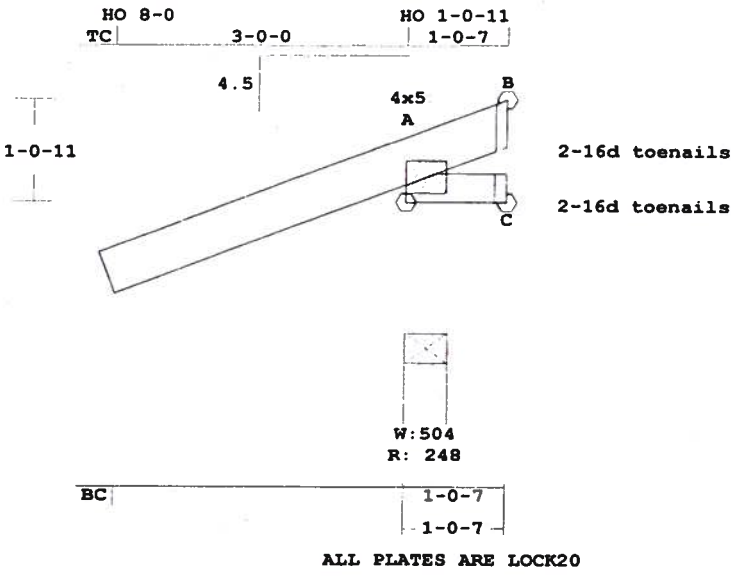
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 8-10- 8
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 1- 7. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1229 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R4*	41	JACK1	10007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 14.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 6	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	248	0	3- 8	1- 0
			Hz =	42
B	24	57	1- 8	1- 0
C	14	3	1- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	21 C	
--------Bottom Chords---------			
A -C	0.00	8 T	

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.03

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

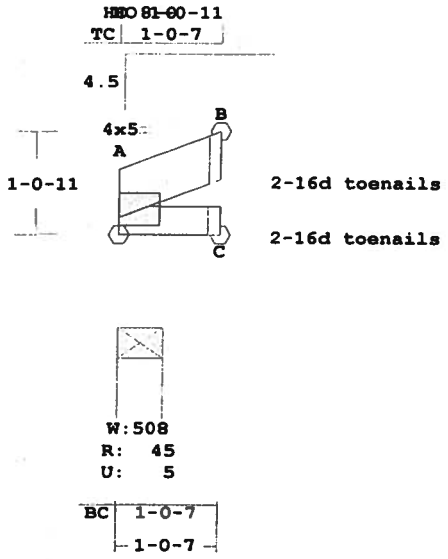
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 21 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R4A*	1	JACK1	10007	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 5.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 6	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	46	5	3- 8	1- 0
			Hz =	16
B	24	20	1- 8	1- 0
C	14	0	1- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	21 C	
--------Bottom Chords---------			
A -C	0.00	8 T	

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.03

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

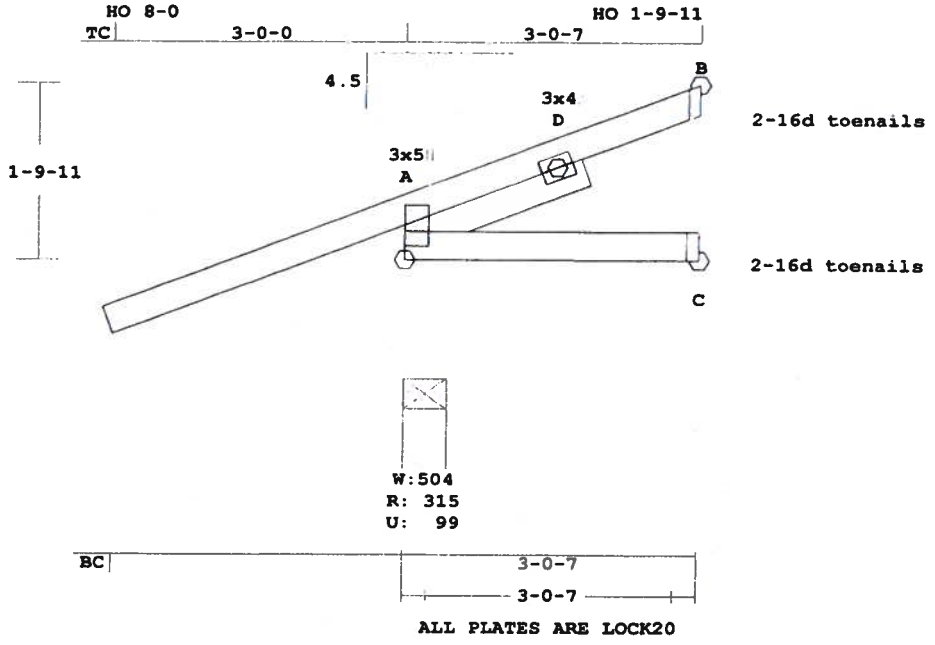
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 21 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R5	30	JACK1	30007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
SL	0.01	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	315	100	5- 4	1- 0
			Hz =	51
B	92	64	3- 8	1- 0
C	51	0	3- 8	1- 0
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.12	60 C	0.00 0.12
D -B	0.12	67 C	0.00 0.12
Bottom Chords			
A -C	0.09	27 T	0.00 0.09
Sliders			
A -D	0.01	88 C	

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in D -D 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

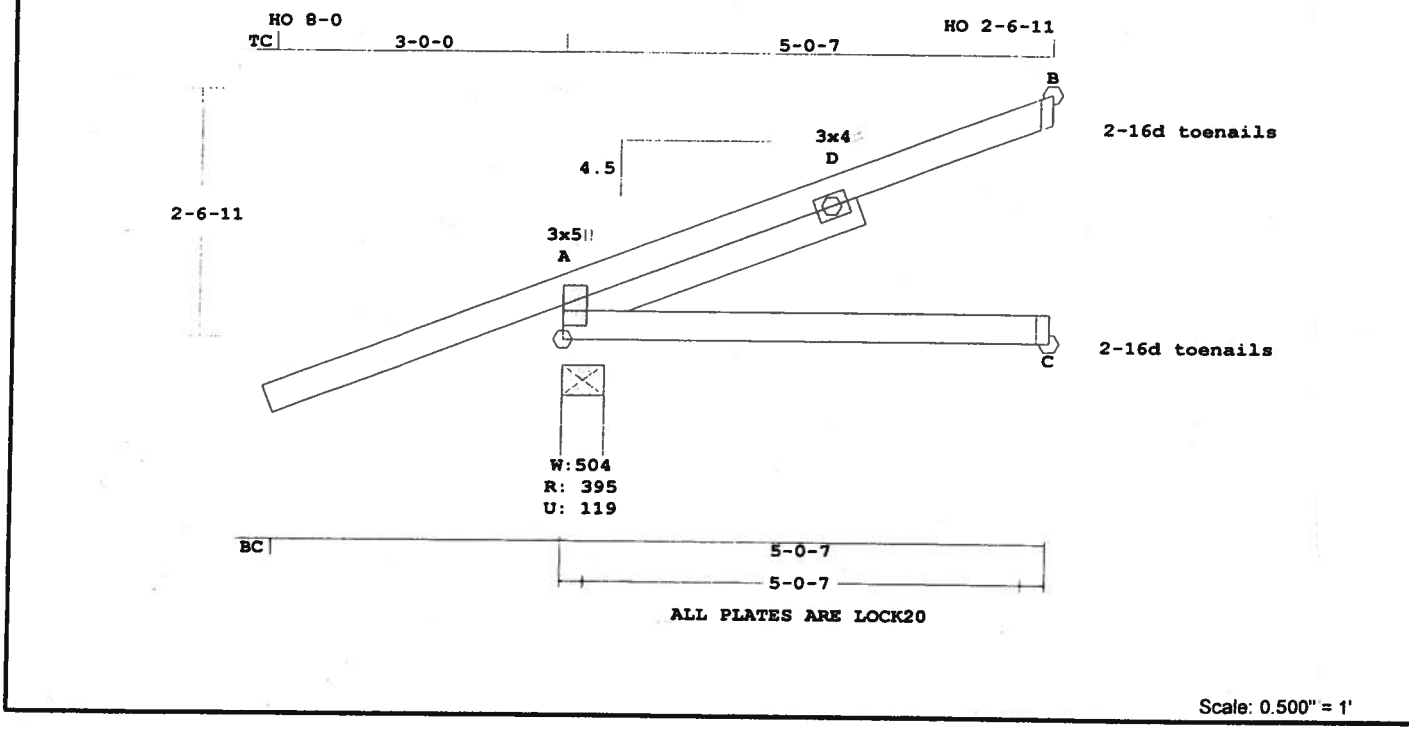
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 88 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R6	26	JACK1	50007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.38	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
SL	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	395	119	5- 4	1- 0
			Hz =	85
B	150	100	3- 8	1- 0
C	85	0	3- 8	1- 0
			Hz =	46

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

A -D	0.38	207	C	0.01	0.37
D -B	0.37	105	C	0.00	0.37

--------Bottom Chords---------

A -C	0.25	45	T	0.00	0.25

----------Sliders----------

A -D 0.05 258 C

LL Defl -0.01" in A -C L/999
TL Defl -0.03" in A -C L/999
Shear // Grain in A -D 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	3.0x 5.0	1.5	0.3	0.65
D LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

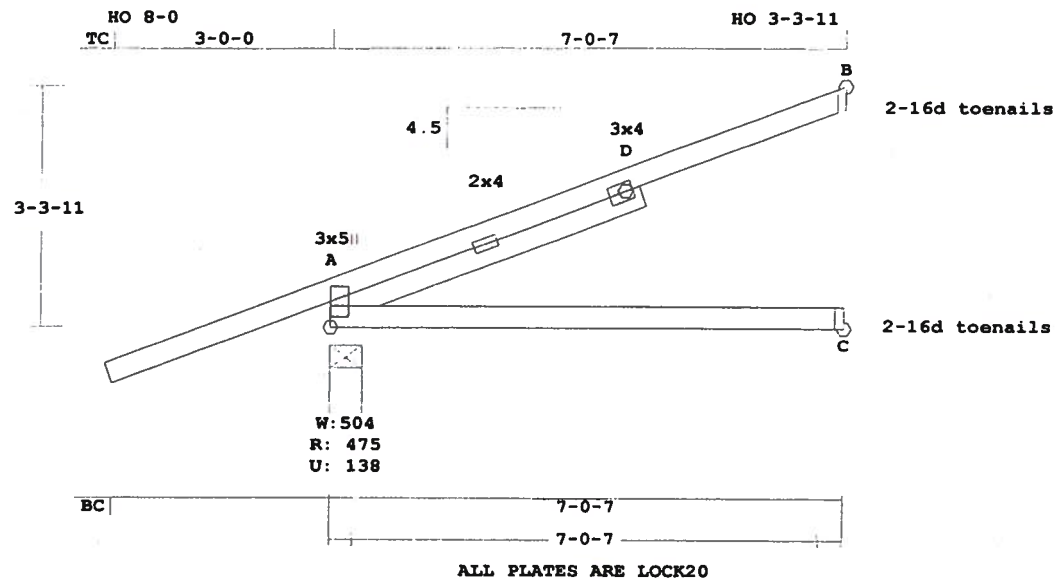
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 258 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R7*	8	JACK1	70007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
SL	0.12	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 7
BC Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	476	138	5- 4	1- 0
			Hz =	119
B	208	137	3- 8	1- 0
C	119	0	3- 8	1- 0
			Hz =	64

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -D	0.53	424 C	0.00	0.53
D -B	0.53	117 C	0.00	0.53

--------Bottom Chords---------

A -C	0.36	63 T	0.00	0.36

----------Sliders-----------

A -D	0.12	429 T

LL Defl -0.06" in A -C L/999
TL Defl -0.14" in A -C L/558
Shear // Grain in A -D 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

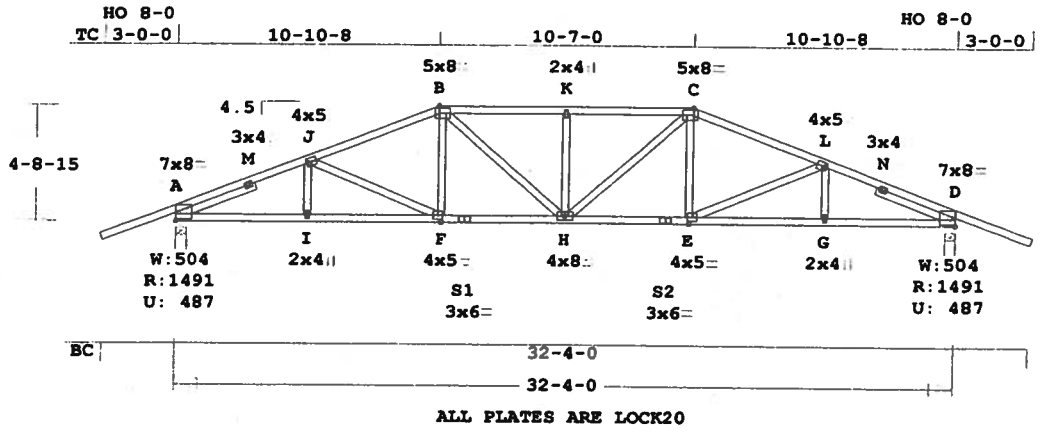
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 424 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R8	1	HIPP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 228.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.39	2x 4	SP-#2
BC	0.63	2x 4	SP-#2
WB	0.14	2x 4	SP-#3
SL	0.27	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	1492	487	5- 4	1-12
			Hz =	-47
D	1492	487	5- 4	1-12
			Hz =	48

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.31	1237	C	0.01	0.30
M -J	0.34	2531	C	0.04	0.30
J -B	0.39	2234	C	0.06	0.33
B -K	0.28	2341	C	0.03	0.25
K -C	0.28	2341	C	0.03	0.25
C -L	0.39	2234	C	0.06	0.33
L -N	0.34	2531	C	0.04	0.30
N -D	0.31	1237	C	0.01	0.30
--------Bottom Chords---------					
A -I	0.63	2347	T	0.39	0.24
I -F	0.50	2347	T	0.39	0.11
F -S1	0.46	2094	T	0.35	0.11
S1-H	0.39	2094	T	0.35	0.04
H -S2	0.39	2094	T	0.35	0.04
S2-E	0.46	2094	T	0.35	0.11
E -G	0.50	2347	T	0.39	0.11
G -D	0.63	2347	T	0.39	0.24
----------Webs----------					
I -J	0.04	177	T		
J -F	0.14	269	C		
F -B	0.08	307	T		
B -H	0.10	324	T		
H -K	0.10	331	C		
H -C	0.10	324	T		
E -C	0.08	307	T		
E -L	0.14	269	C		
G -L	0.04	177	T		
----------Sliders----------					
A -M	0.27	1370	C		
N -D	0.27	1370	C		

LL Defl -0.14" in H -S2 L/999
TL Defl -0.30" in H -S2 L/999
Shear // Grain in J -B 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

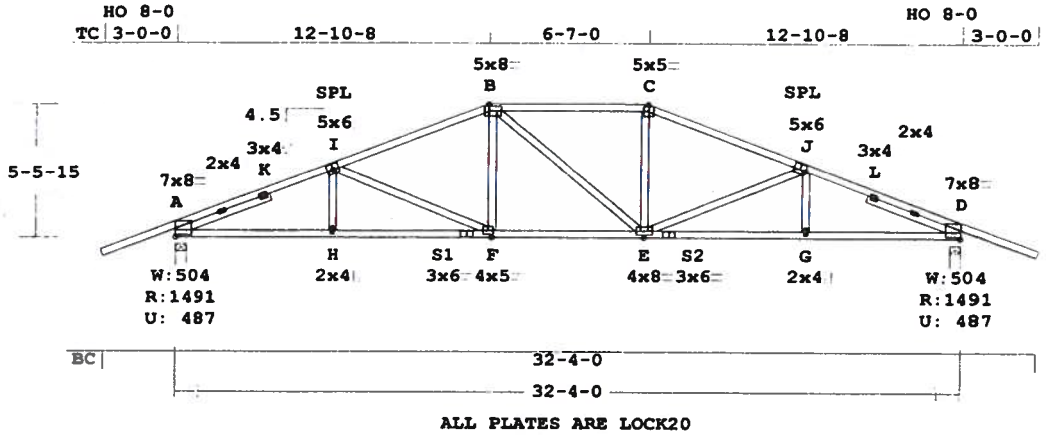
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
M	LOCK	3.0x 4.0	Ctr	Ctr	0.57
J	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 8.0	Ctr	Ctr	0.85
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
C	LOCK	5.0x 8.0	Ctr	Ctr	0.85
L	LOCK	4.0x 5.0	Ctr	Ctr	0.64
N	LOCK	3.0x 4.0	Ctr	Ctr	0.57
D	LOCK	7.0x 8.0-4.0	0.6		0.85
I	LOCK	2.0x 4.0	Ctr	Ctr	0.43
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2531 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R9*	1	HIPP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 222.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 4	SP-#2
BC	0.61	2x 4	SP-#2
WB	0.33	2x 4	SP-#3
SL	0.32	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1492	487	5- 4	1-12
			Hz =	-56
D	1492	487	5- 4	1-12
			Hz =	57

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -K	0.23	1274 C	0.01	0.22
K -I	0.37	2531 C	0.15	0.22
I -B	0.47	2064 C	0.07	0.40
B -C	0.40	1937 C	0.04	0.36
C -J	0.47	2064 C	0.07	0.40
J -L	0.37	2531 C	0.15	0.22
L -D	0.23	1276 C	0.01	0.22
--------Bottom Chords---------				
A -H	0.61	2364 T	0.39	0.22
H -S1	0.51	2364 T	0.39	0.12
S1-F	0.52	2364 T	0.39	0.13
F -E	0.46	1932 T	0.32	0.14
E -S2	0.53	2364 T	0.39	0.14
S2-G	0.51	2364 T	0.39	0.12
G -D	0.61	2364 T	0.39	0.22
----------Webs----------				
H -I	0.06	231 T		
I -F	0.33	461 C		
F -B	0.10	393 T		
B -E	0.05	61 T		
E -C	0.10	392 T		
E -J	0.33	463 C		
G -J	0.06	229 T		
----------Sliders----------				
A -K	0.32	1334 C		
L -D	0.32	1333 C		

LL Defl -0.13" in S1-F L/999
TL Defl -0.29" in H -S1 L/999
Shear // Grain in I -B 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

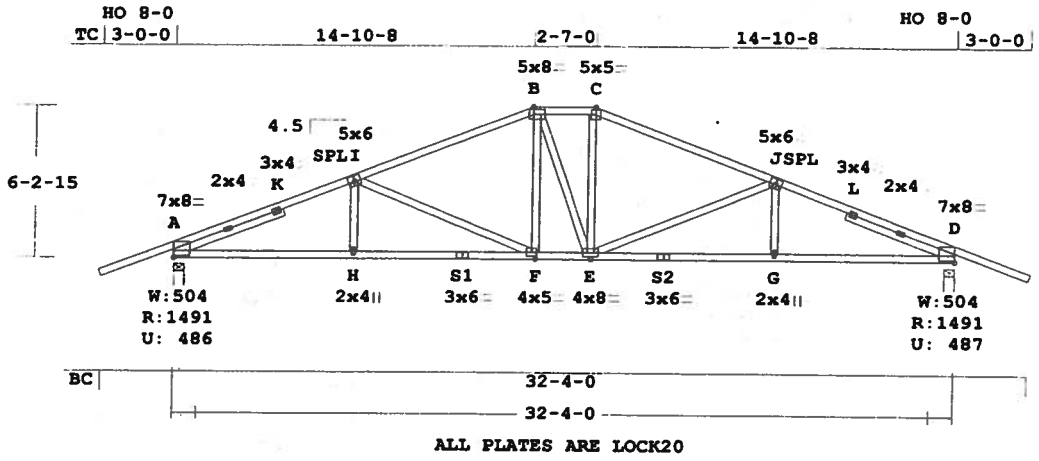
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	5.0x 6.0-0.2	0.5		0.87
B	LOCK	5.0x 8.0	Ctr	Ctr	0.85
C	LOCK	5.0x 5.0	Ctr	Ctr	0.85
J	LOCK	5.0x 6.0	0.2	0.5	0.87
L	LOCK	3.0x 4.0	Ctr	Ctr	0.57
D	LOCK	7.0x 8.0-4.0	0.6		0.85
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 8.0	Ctr	Ctr	0.58
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
G	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2531 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R10	1	HIPP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 229.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.65	2x 4	SP-#2
BC	0.60	2x 4	SP-#2
WB	0.64	2x 4	SP-#3
SL	0.38	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	32- 4- 0
BC	Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1492	487	5- 4	1-12
			Hz =	-65
D	1492	487	5- 4	1-12
			Hz =	66

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -K	0.24	1335	C	0.08 0.16
K -I	0.49	2514	C	0.15 0.34
I -B	0.65	1870	C	0.08 0.57
B -C	0.29	1748	C	0.12 0.17
C -J	0.65	1869	C	0.08 0.57
J -L	0.49	2514	C	0.15 0.34
L -D	0.24	1335	C	0.08 0.16
--------Bottom Chords---------				
A -H	0.60	2364	T	0.39 0.21
H -S1	0.53	2364	T	0.39 0.14
S1-F	0.53	2364	T	0.39 0.14
F -E	0.43	1743	T	0.29 0.14
E -S2	0.53	2365	T	0.39 0.14
S2-G	0.53	2365	T	0.39 0.14
G -D	0.60	2365	T	0.39 0.21
----------Webs----------				
H -I	0.07	288	T	
I -F	0.64	667	C	
F -B	0.11	380	T	
B -E	0.03	67	C	
E -C	0.11	370	T	
E -J	0.64	668	C	
G -J	0.07	287	T	
----------Sliders----------				
A -K	0.38	1262	C	
L -D	0.38	1262	C	

LL Defl -0.12" in S2-G L/999
TL Defl -0.29" in S2-G L/999
Shear // Grain in I -B 0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

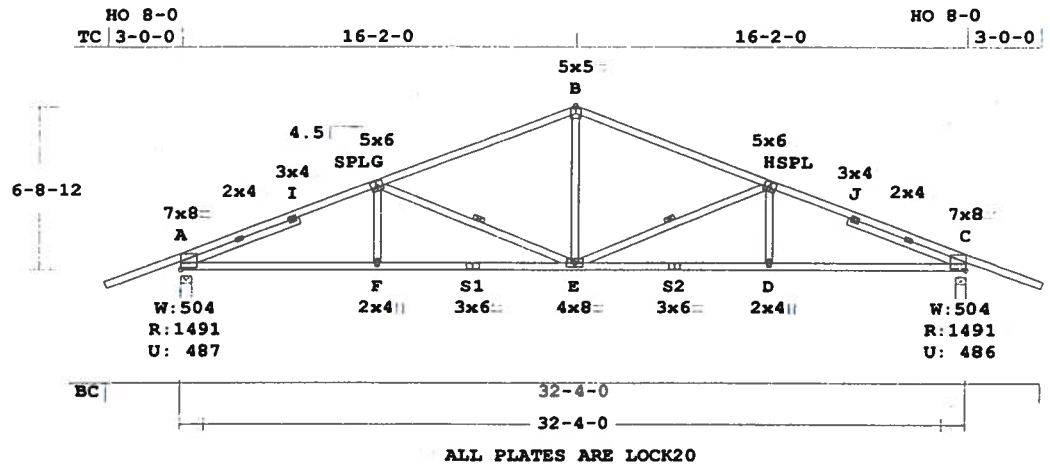
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	5.0x 6.0	-0.2	0.5	0.87
B	LOCK	5.0x 8.0	Ctr	Ctr	0.85
C	LOCK	5.0x 5.0	Ctr	Ctr	0.85
J	LOCK	5.0x 6.0	0.2	0.5	0.87
L	LOCK	3.0x 4.0	Ctr	Ctr	0.57
D	LOCK	7.0x 8.0	-4.0	0.6	0.85
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 8.0	Ctr	Ctr	0.62
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
G	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2514 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R11*	1	TR	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 213.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.67	2x 4	SP-#2
BC	0.62	2x 4	SP-#2
WB	0.23	2x 4	SP-#3
SL	0.42	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

WB 1 rows CLB on G -E
WB 1 rows CLB on E -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1492	487	5- 4	1-12
		Hz =	-70	
C	1492	487	5- 4	1-12
		Hz =	71	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.30	1383 C	0.09	0.21
I -G	0.52	2481 C	0.14	0.38
G -B	0.67	1772 C	0.09	0.58
B -H	0.67	1771 C	0.09	0.58
H -J	0.52	2481 C	0.14	0.38
J -C	0.30	1383 C	0.09	0.21
--------Bottom Chords---------				
A -F	0.57	2341 T	0.39	0.18
F -S1	0.52	2341 T	0.39	0.13
S1-E	0.62	2341 T	0.39	0.23
E -S2	0.62	2341 T	0.39	0.23
S2-D	0.52	2341 T	0.39	0.13
D -C	0.57	2341 T	0.39	0.18
----------Webs----------				
F -G	0.08	301 T		
G -E	0.23	739 C		1 Br
E -B	0.23	757 T		
E -H	0.23	739 C		1 Br
D -H	0.08	301 T		
----------Sliders----------				
A -I	0.42	1185 C		
J -C	0.42	1185 C		

LL Defl -0.12" in S1-E L/999
TL Defl -0.28" in S1-E L/999
Shear // Grain in G -B 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	3.0x 4.0	Ctr	Ctr	0.57
G	LOCK	5.0x 6.0	-0.2	0.5	0.87
B	LOCK	5.0x 5.0	Ctr	Ctr	0.66
H	LOCK	5.0x 6.0	0.2	0.5	0.87
J	LOCK	3.0x 4.0	Ctr	Ctr	0.57
C	LOCK	7.0x 8.0	-4.0	0.6	0.85
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.43
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.58
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
D	LOCK	2.0x 4.0	Ctr	Ctr	0.43

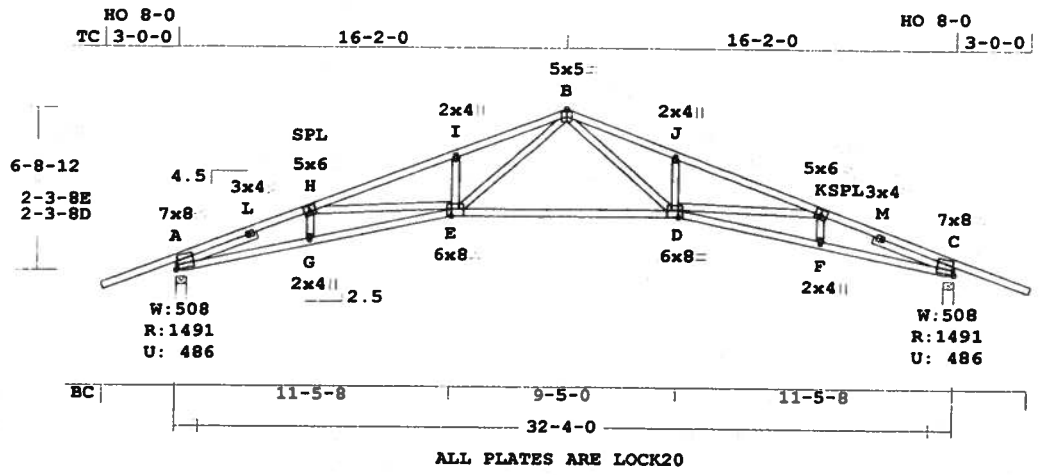
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2481 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R12	9	SP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 210.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.78	2x 4	SP-#2
EX H -B		2x 4	SP-2400
EX B -K		2x 4	SP-2400
BC	0.64	2x 4	SP-2400
EX E -D		2x 4	SP-#2
WB	0.72	2x 4	SP-#3
SL	0.45	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	32- 4- 0
BC Cont.		0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1492	487	5- 8	1- 4
			Hz =	-70
C	1492	487	5- 8	1- 4
			Hz =	71

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.68	2009	C	0.02 0.66
L -H	0.78	4257	C	0.12 0.66
H -I	0.59	4380	C	0.16 0.43
I -B	0.28	4404	C	0.09 0.19
B -J	0.28	4404	C	0.09 0.19
J -K	0.59	4381	C	0.16 0.43
K -M	0.78	4257	C	0.12 0.66
M -C	0.68	2009	C	0.02 0.66
Bottom Chords				
A -G	0.64	3980	T	0.28 0.36
G -E	0.50	4014	T	0.28 0.22
E -D	0.73	2356	T	0.39 0.34
D -F	0.50	4014	T	0.28 0.22
F -C	0.64	3980	T	0.28 0.36
Webs				
G -H	0.03	132	T	
H -E	0.06	202	T	
E -I	0.08	390	C	
E -B	0.72	2264	T	
B -D	0.72	2265	T	
D -J	0.08	390	C	
D -K	0.06	202	T	
F -K	0.03	132	T	
Sliders				
A -L	0.45	2353	C	
M -C	0.45	2353	C	

LL Defl -0.49" in E -D L/761
TL Defl -1.19" in E -D L/317
Shear // Grain in L -H 0.34

Hz Disp	LL	DL	TL
Jt C	0.25"	0.26"	0.51"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

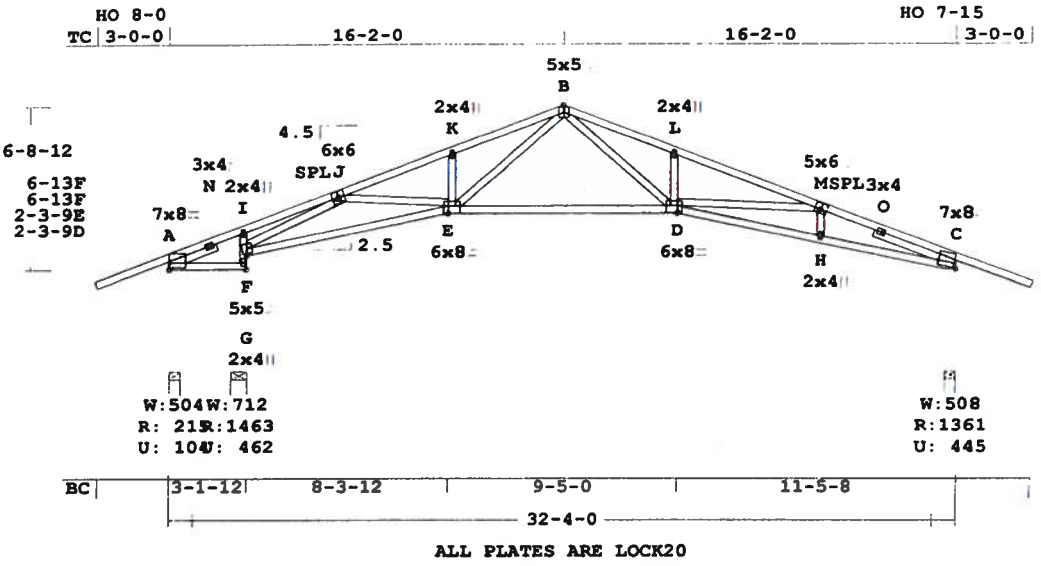
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.6	1.2	0.91
L	LOCK	3.0x 4.0	Ctr	Ctr	0.87
H	LOCK	5.0x 6.0	-0.2	0.5	0.87
I	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.95
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
K	LOCK	5.0x 6.0	0.2	0.5	0.87
M	LOCK	3.0x 4.0	Ctr	Ctr	0.87
C	LOCK	7.0x 8.0	-4.6	1.2	0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.43
E	LOCK	6.0x 8.0	Ctr	-0.5	0.88
D	LOCK	6.0x 8.0	Ctr	-0.5	0.88
F	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 4404 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R13*	1	SP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 213.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.90	2x 4	SP-#2
BC	0.63	2x 4	SP-#2
EX D -C		2x 4	SP-2400
CW	0.68	2x 4	SP-#3
WB	0.99	2x 4	SP-#3
SL	0.38	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	32- 4- 0
BC	Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	216	104	5- 4	1- 0
			Hz =	-70
G	1463	462	7-12	1-10
C	1361	446	5- 8	1- 2
			Hz =	71

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.05	45	C	0.00	0.05
N -I	0.54	348	T	0.06	0.48
I -J	0.51	249	T	0.03	0.48
J -K	0.35	3167	C	0.10	0.25
K -B	0.36	3179	C	0.19	0.17
B -L	0.43	3707	C	0.22	0.21
L -M	0.90	3694	C	0.21	0.69
M -O	0.66	3776	C	0.09	0.57
O -C	0.59	1855	C	0.02	0.57
Bottom Chords					
A -G	0.32	256	C	0.00	0.32
F -E	0.63	2264	T	0.22	0.41
E -D	0.60	1885	T	0.31	0.29
D -H	0.44	3568	T	0.25	0.19
H -C	0.57	3537	T	0.25	0.32
Chord-Webs					
G -F	0.68	1533	C	0.05	0.63
F -I	0.07	437	C		
Webs					
F -J	0.99	2871	C		
J -E	0.24	753	T		
E -K	0.07	315	C		
E -B	0.44	1395	T		
B -D	0.65	2036	T		
D -L	0.08	358	C		
D -M	0.04	184	T		
H -M	0.03	127	T		
Sliders					
A -N	0.12	379	T		
O -C	0.38	2019	C		

LL Defl -0.33" in E -D L/999
TL Defl -0.82" in E -D L/422
Shear // Grain in G -F 0.33

Hz Disp	LL	DL	TL
Jt C	0.15"	0.15"	0.30"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

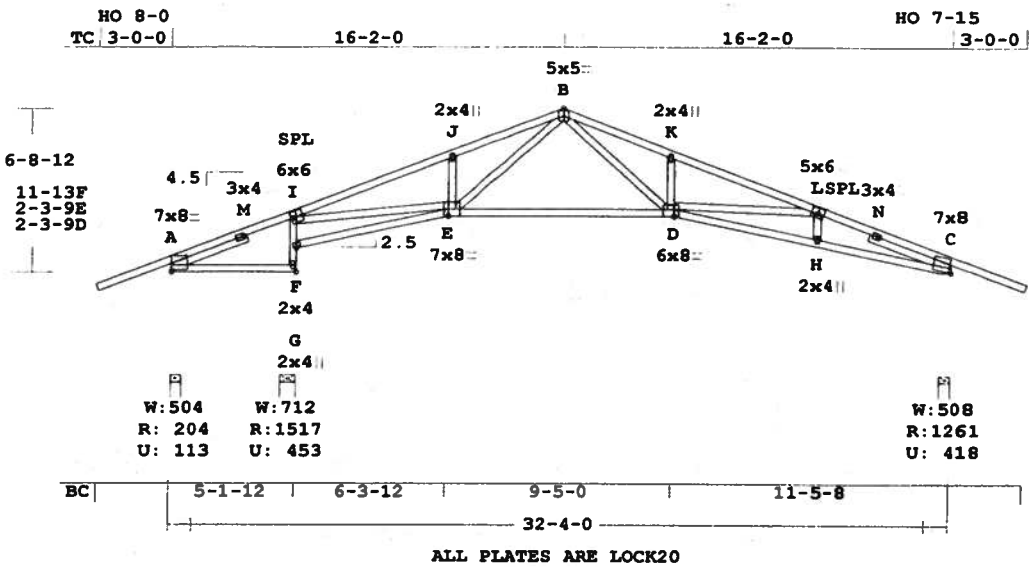
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
N	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	2.0x 4.0	Ctr	Ctr	0.26
J	LOCK	6.0x 6.0	Ctr	Ctr	0.89
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.85
L	LOCK	2.0x 4.0	Ctr	Ctr	0.43
M	LOCK	5.0x 6.0	0.2	0.5	0.87
O	LOCK	3.0x 4.0	Ctr	Ctr	0.75
C	LOCK	7.0x 8.0-4.6	1.3		0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr	Ctr	0.88
E	LOCK	6.0x 8.0	Ctr-0.5		0.66
D	LOCK	6.0x 8.0	Ctr-0.5		0.78
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3776 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R14*	1	SP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 212.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.76	2x 4	SP-#2
BC	0.95	2x 4	SP-#2
CW	0.60	2x 4	SP-#2
WB	0.79	2x 4	SP-#3
SL	0.34	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	204	114	5- 4	1- 0
			Hz =	-70
G	1517	453	7-12	1-11
C	1262	418	5- 8	1- 8
			Hz =	71

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -M	0.14	106 T	0.00 0.14
M -I	0.60	381 T	0.06 0.54
I -J	0.57	2307 C	0.03 0.54
J -B	0.37	2309 C	0.15 0.22
B -K	0.40	3172 C	0.19 0.21
K -L	0.76	3163 C	0.15 0.61
L -N	0.57	3395 C	0.07 0.50
N -C	0.51	1662 C	0.01 0.50
--------Bottom Chords---------			
A -G	0.11	300 C	0.00 0.11
F -E	0.49	338 C	0.00 0.49
E -D	0.67	1525 T	0.25 0.42
D -H	0.82	3210 T	0.54 0.28
H -C	0.95	3184 T	0.53 0.42
----------Chord-Webs----------			
G -F	0.60	1474 C	0.01 0.59
F -I	0.12	1281 C	0.12 0.00
------------Webs------------			
I -E	0.79	2472 T	
E -J	0.08	342 C	
E -B	0.25	802 T	
B -D	0.59	1859 T	
D -K	0.08	347 C	
D -L	0.08	247 T	
H -L	0.02	115 T	
-----------Sliders-----------			
A -M	0.13	425 T	
N -C	0.34	1825 C	

LL Defl -0.28" in E -D L/999
TL Defl -0.68" in E -D L/470
Shear // Grain in G -F 0.39

Hz Disp	LL	DL	TL
Jt C	0.13"	0.12"	0.24"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

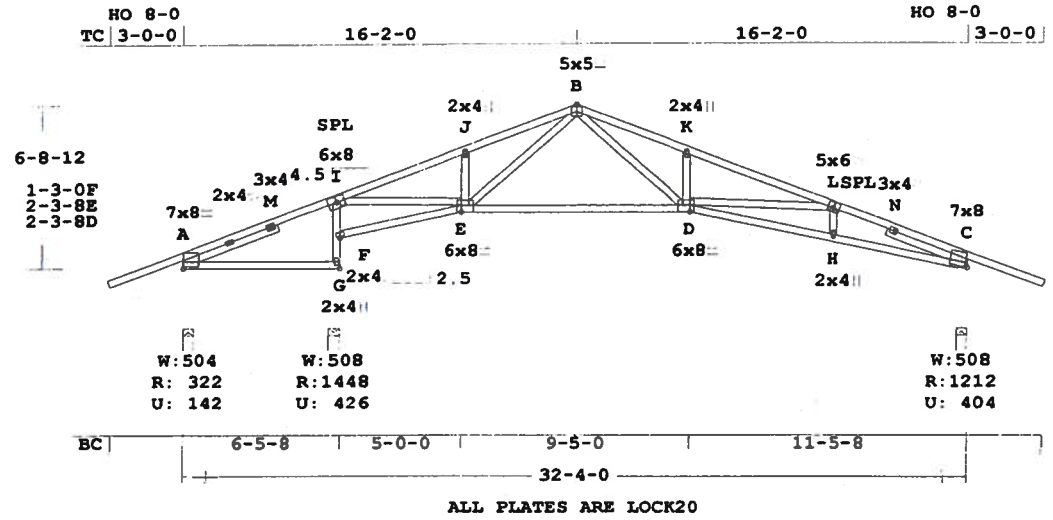
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
M	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	6.0x 6.0	Ctr	Ctr	0.91
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.78
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
L	LOCK	5.0x 6.0	0.2	0.5	0.87
N	LOCK	3.0x 4.0	Ctr	Ctr	0.68
C	LOCK	7.0x 8.0	-4.6	1.3	0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.58
F	LOCK	2.0x 4.0	Ctr	Ctr	0.81
E	LOCK	7.0x 8.0	Ctr	Ctr	0.79
D	LOCK	6.0x 8.0	Ctr	Ctr	0.71
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3395 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R15*	3	SP	320400	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 211.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.70	2x 4	SP-#2
BC	0.91	2x 4	SP-#2
CW	0.95	2x 4	SP-#3
WB	0.61	2x 4	SP-#3
SL	0.33	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	322	143	5- 4	1- 0
			Hz =	-70
G	1448	426	5- 8	1-10
C	1213	405	5- 8	1- 7
			Hz =	71

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.30	272	C	0.00	0.30
M -I	0.55	274	T	0.05	0.50
I -J	0.52	1810	C	0.02	0.50
J -B	0.30	1807	C	0.03	0.27
B -K	0.39	2906	C	0.05	0.34
K -L	0.70	2902	C	0.12	0.58
L -N	0.52	3188	C	0.06	0.46
N -C	0.47	1544	C	0.01	0.46
Bottom Chords					
A -G	0.19	202	C	0.00	0.19
F -E	0.37	254	C	0.00	0.37
E -D	0.60	1342	T	0.22	0.38
D -H	0.79	3016	T	0.50	0.29
H -C	0.91	2991	T	0.50	0.41
Chord-Webs					
G -F	0.95	1388	C	0.04	0.91
F -I	0.20	1228	C	0.20	0.00
Webs					
I -E	0.61	1922	T		
E -J	0.07	291	C		
E -B	0.13	435	T		
B -D	0.57	1779	T		
D -K	0.07	334	C		
D -L	0.11	271	T		
H -L	0.02	113	T		
Sliders					
A -M	0.14	493	T		
N -C	0.33	1734	C		

LL Defl -0.27" in E -D L/999
TL Defl -0.63" in E -D L/484
Shear // Grain in M -I 0.30

Hz Disp	LL	DL	TL
Jt C	0.10"	0.09"	0.19"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

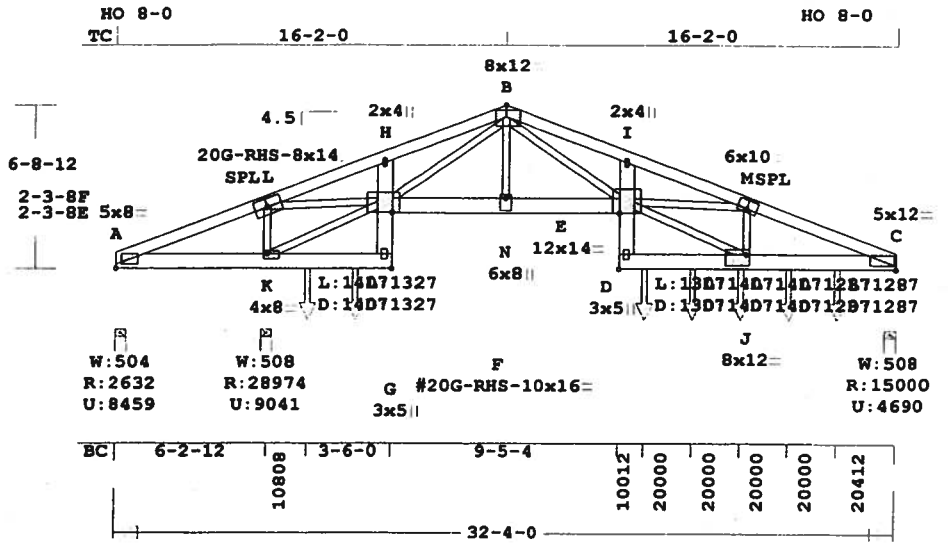
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	6.0x 8.0	Ctr	Ctr	0.87
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.75
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
L	LOCK	5.0x 6.0	0.2	0.5	0.87
N	LOCK	3.0x 4.0	Ctr	Ctr	0.64
C	LOCK	7.0x 8.0-4.6	1.2		0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.58
F	LOCK	2.0x 4.0	Ctr	Ctr	0.77
E	LOCK	6.0x 8.0	Ctr-0.5		0.89
D	LOCK	6.0x 8.0	Ctr-0.5		0.66
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3188 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R16*	1*4P	BOXTRAY	320400	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 321.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 4-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.91	2x 6	SP-#2
EX B -M		2x 6	SP-2400
BC	0.93	2x 8	SP-2400
CW	0.86	2x 8	SP-2400
WB	0.84	2x 4	SP-#3
EX L -F		2x 4	SP-2400
EX B -E		2x 4	SP-2400
EX E -J		2x 4	SP-2400

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	32- 4- 0
BC Cont.		0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To	
TC V	40	20	0.0'	32.3'	
BC V	0	20	0.0'	32.3'	
BC V	704	704	10.9'	20.9'	
BC V	1407	1407	7.9'	CL-LB	
BC V	1327	1327	9.9'	CL-LB	
BC V	1307	1307	21.9'	CL-LB	
BC V	1407	1407	23.9'	CL-LB	
BC V	1407	1407	25.9'	CL-LB	
BC V	1287	1287	27.9'	CL-LB	
BC V	1287	1287	29.9'	CL-LB	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2633	8459G	5- 4	1- 0
		Hz =	-71	

K	28975	9042	5- 8	5-10 **
C	15001	4690	5- 8	3- 2
			Hz =	71

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -L	0.85	20115 T	0.61	0.24
L -H	0.32	18084 C	0.06	0.26
H -B	0.91	21126 C	0.09	0.82
B -I	0.75	42358 C	0.22	0.53
I -M	0.81	43232 C	0.25	0.56
M -C	0.37	33091 C	0.35	0.02
Bottom Chords				
A -K	0.42	18749 C	0.02	0.40
K -G	0.40	329 C	0.00	0.40
F -N	0.58	21926 T	0.19	0.39
N -E	0.93	21926 T	0.19	0.74
D -J	0.31	4810 T	0.04	0.27
J -C	0.45	30998 T	0.26	0.19
Chord-Webs				
G -F	0.07	2170 T	0.02	0.05
F -H	0.02	2243 C	0.02	0.00
D -E	0.86	3409 T	0.03	0.83
E -I	0.02	2646 T	0.02	0.00
Webs				
K -L	0.59	15114 C		
L -F	0.70	35710 T		
K -F	0.84	20344 C		
F -B	0.12	3001 C		
N -B	0.81	10233 T		
B -E	0.44	22348 T		
E -M	0.73	9174 T		
E -J	0.57	28822 T		
J -M	0.19	4989 C		

LL Defl -0.55" in D -E L/558
TL Defl -1.09" in D -E L/278
Shear // Grain in N -E 0.97

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	Ctr	0.90
L	RHS	8.0x14.0	0.4	0.5	0.95
H	LOCK	2.0x 4.0	Ctr	Ctr	0.22
B	LOCK	8.0x12.0	0.5	-0.6	0.93
I	LOCK	2.0x 4.0	Ctr	Ctr	0.44
M	LOCK	6.0x10.0	0.5	1.2	0.95
C	LOCK	5.0x12.0	Ctr	Ctr	0.93
K	LOCK	4.0x 8.0	0.5	-0.5	0.88
G	LOCK	3.0x 5.0	Ctr	Ctr	0.96
F	RHS	10.0x16.0	-0.8	1.4	0.95
N	LOCK	6.0x 8.0	Ctr	-2.1	0.92
E	LOCK	12.0x14.0	Ctr	2.4	0.97
D	LOCK	3.0x 5.0	Ctr	Ctr	0.96
J	LOCK	8.0x12.0	-2.5	-1.4	0.83

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	24
BC 2	4	8	10
WB 1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 43232 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054

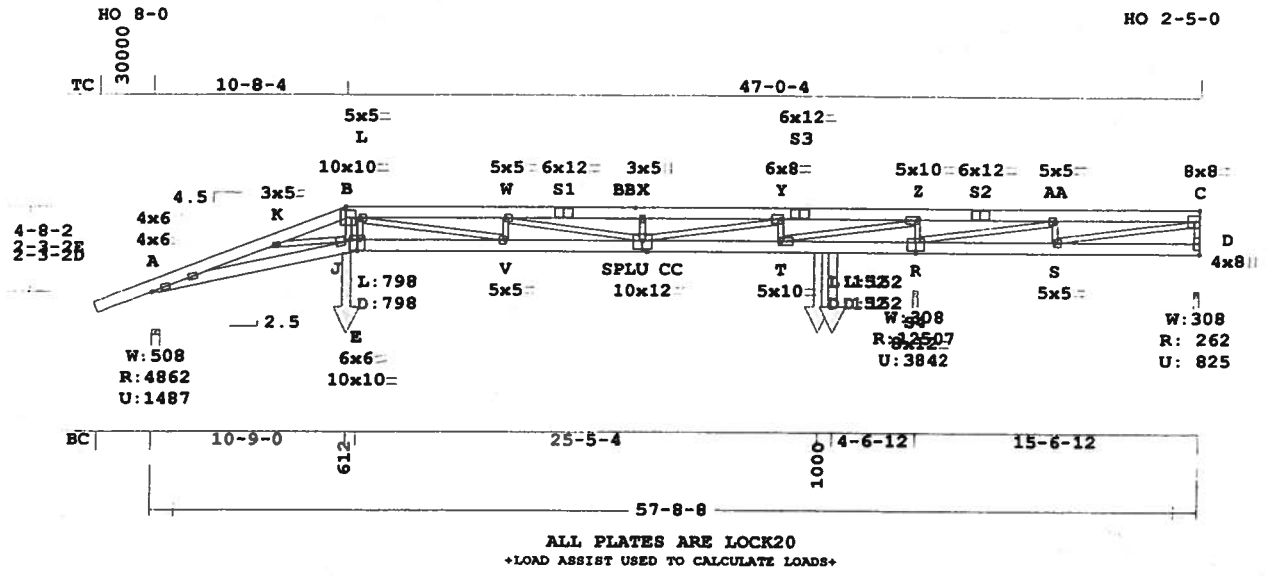
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R16*	1*4P	BOXTRAY	320400	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish

Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R17*	1*4P	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 566.6 LBS

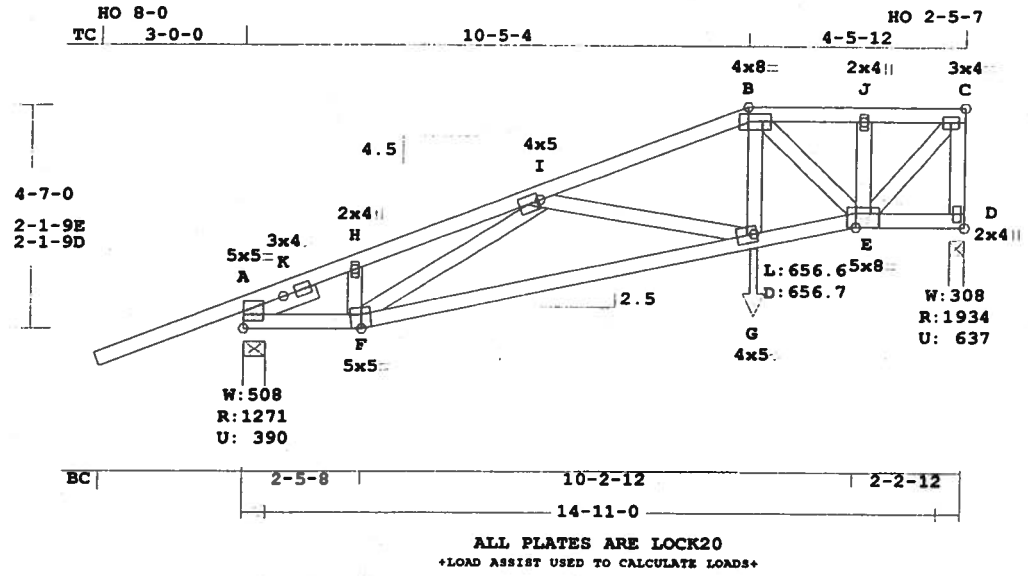
```
Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
***************
* 4-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.38  2x 8  SP-2400
BC  0.41  2x 8  SP-2400
WB  0.67  2x 4  SP-#3
EX U -Y   2x 4  SP-#2
EX T -Z   2x 4  SP-2400

Brace truss as follows:
      O.C.      From      To
 TC Cont.      0- 0- 0 57- 8- 8
 BC Cont.      0- 0- 0 57- 8- 8

Loading   Live   Dead  (psf)
TC         20.0   10.0
BC          0.0   10.0
Total      20.0   20.0    40.0
Spacing                   24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor   1.25
Plate  Duration Factor   1.25
plf - Live  Dead  From    To
TC V    40    20   0.0'  57.7'
BC V     0    20   0.0'  57.7'
BC V   107   107  11.8'  35.8'
BC V   119   119  38.6'  54.7'
BC V   798   798  10.8'  CL-LB
BC V   152   152  36.8'  CL-LB
BC V   152   152  37.6'  CL-LB


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A    4862  1488  5- 8  1- 0
                 Hz =   240
S4  12507  3842  3- 8  2- 5
D     262   826G 3- 8  1- 0
                 Hz =   134
G = Gravity Uplift

Membr  CSI   P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -K  0.21 20616 C  0.02  0.19
K -B  0.23 24991 C  0.04  0.19
B -L  0.06 25873 C  0.04  0.02
L -W  0.15 27405 C  0.06  0.09
W -S1 0.13 20360 C  0.03  0.10
S1-X  0.12 20360 C  0.03  0.09
X -Y  0.10 20360 C  0.03  0.07
Y -S3 0.03  3645 C  0.00  0.03
S3-Z  0.23  3645 C  0.00  0.23
Z -S2 0.38 18079 T  0.15  0.23
S2-AA 0.23 18079 T  0.15  0.08
AA-C  0.09  6387 T  0.05  0.04
--------Bottom Chords---------
A -E  0.41 19609 T  0.17  0.24
E -E  0.28 24441 C  0.04  0.24
E -V  0.35 25953 T  0.22  0.13
V -U  0.35 27406 T  0.23  0.12
U -T  0.13  3646 T  0.03  0.10
T -S4 0.33 18079 C  0.02  0.31
S4-S  0.31  6387 C  0.00  0.31
S -D  0.08   134 T  0.00  0.08
------------Webs-------------
K -E  0.37  4635 T
B -E  0.67  8359 T
E -L  0.03   819 C
L -V  0.11  1488 T
V -W  0.11  1629 T
W -U  0.33  7241 C
U -X  0.02   510 C
U -Y  0.79 17183 T
T -Y  0.15  3913 C
T -Z  0.44 22334 T
S4-Z  0.26  6695 C
S4-AA 0.55 12019 C
S -AA 0.23  3016 T
S -C  0.30  6560 C
D -C  0.09  1243 T

LL Defl -0.63" in J -V  L/795
TL Defl -1.26" in J -V  L/396
Shear // Grain in A -J   0.28
Hz Disp   LL      DL      TL
  Jt S4  0.17"    0.17"   0.33"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  4.0x 6.0 Ctr Ctr 0.75
A  LOCK  4.0x 6.0 Ctr Ctr 0.32
K  LOCK  3.0x 5.0 Ctr Ctr 0.59
B  LOCK 10.0x10.0-0.5 Ctr 0.85
L  LOCK  5.0x 5.0 Ctr Ctr 0.60
W  LOCK  5.0x 5.0 Ctr Ctr 0.60
S1 LOCK  6.0x12.0 Ctr Ctr 0.91
X  LOCK  3.0x 5.0 Ctr Ctr 0.39
Y  LOCK  6.0x 8.0-0.5 Ctr 0.93
S3 LOCK  6.0x12.0 Ctr Ctr 0.91
Z  LOCK  5.0x10.0-1.5 Ctr 0.83
S2 LOCK  6.0x12.0 Ctr Ctr 0.91
AA LOCK  5.0x 5.0 Ctr Ctr 0.89
C  LOCK  8.0x 8.0-0.5 Ctr 0.88
E  LOCK  6.0x 6.0-3.5-0.7 0.37
E  LOCK 10.0x10.0 Ctr-2.2 0.67
V  LOCK  5.0x 5.0 Ctr Ctr 0.60
U  LOCK 10.0x12.0 Ctr-0.5 0.99
T  LOCK  5.0x10.0 1.5 Ctr 0.83
S4 LOCK  8.0x12.0 Ctr-1.5 0.99
S  LOCK  5.0x 5.0 Ctr Ctr 0.60
D  LOCK  4.0x 8.0-0.2 Ctr 0.85


NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS6   screws -OR- 16d nails
  as each layer is applied.)
                ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  2    12      24      24
  BC  2    12      24      24
  WB  1     4       4
  No bolts in 2x4s or smaller.
Plus clusters of nails where
  shown.
Plus use 1/2 In (ASTM-A307)
  thru bolts at each panel
  point and on each side of
  splices in 2x6 or larger
  chords only.
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
  bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:    B
  Occupancy Factor   : 1.00
  Building Type: Enclosed
  TC Dead Load:          4.0 psf
  BC Dead Load:          6.0 psf
Max comp. force    27405 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R17*	1*4P	SP	570808	4.5	3- 0- 0	0	
Office and Care Center, Hospice, M.M. Parrish								

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R17A	1*2P	SP	141100	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 107.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.40	2x 4	SP-#3
SL	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14-11- 0
BC Cont.	0- 0- 0	14-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.9'
BC V	0	20	0.0'	14.9'
TC V	84	42	10.4'	13.9'
TC V	-40	-20	13.9'	14.9'
BC V	0	42	10.6'	13.9'
BC V	0	-20	13.9'	14.9'
BC V	657	657	10.6'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1272	391	5- 8	1- 0
			Hz =	181
D	1934	638	3- 8	1- 2
			Hz =	98

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -K	0.20	1152 C	0.00	0.20
K -H	0.20	1834 C	0.00	0.20
H -I	0.11	1766 C	0.00	0.11
I -B	0.18	2803 C	0.01	0.17
B -J	0.16	1769 C	0.00	0.16
J -C	0.10	1769 C	0.06	0.04
Bottom Chords				
A -F	0.30	1651 T	0.15	0.15
F -G	0.35	2615 T	0.24	0.11
G -E	0.40	2648 T	0.24	0.16
E -D	0.18	98 T	0.00	0.18
Webs				
F -H	0.02	155 T		
F -I	0.09	1087 C		
I -G	0.02	179 T		
G -B	0.21	1360 T		
B -E	0.08	1058 C		
E -J	0.04	492 C		
E -C	0.40	2529 T		
D -C	0.14	1819 C		
Sliders				
A -K	0.06	827 C		

LL Defl -0.07" in F -G L/999
TL Defl -0.15" in F -G L/999
Shear // Grain in B -J 0.19

Hz Disp	LL	DL	TL
Jt D	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.94
K	LOCK	3.0x 4.0	Ctr	Ctr	0.50
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
I	LOCK	4.0x 5.0	0.5	0.2	1.00
B	LOCK	4.0x 8.0	Ctr	Ctr	0.76
J	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	3.0x 4.0	Ctr	Ctr	0.81
F	LOCK	5.0x 5.0-0.3	2.7		0.53
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
E	LOCK	5.0x 8.0	Ctr-1.0		0.72
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 10- 5- 4
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

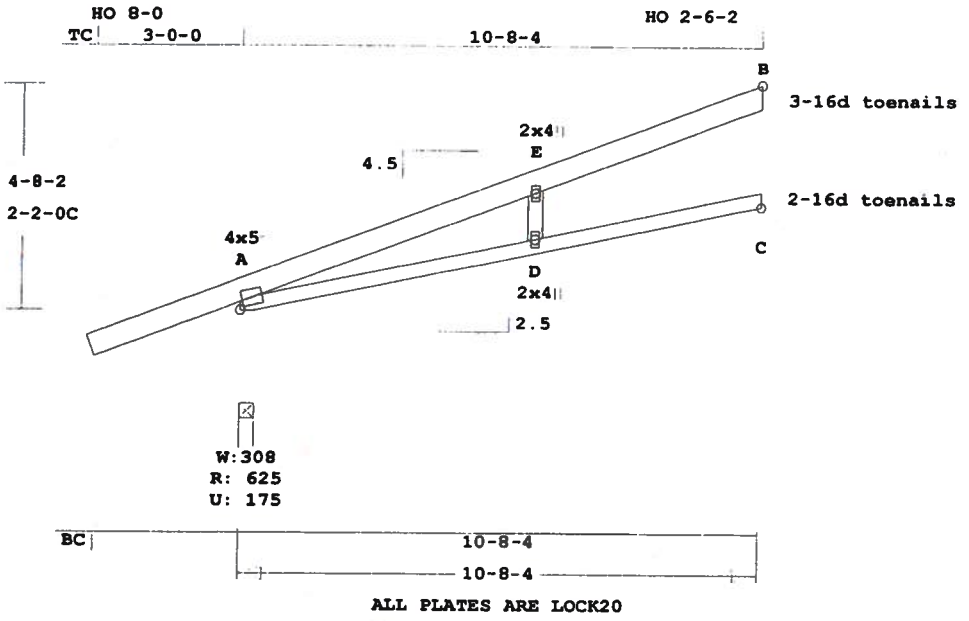
	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	1	12	24	0
BC	1	12	24	0
WB	1	4	4	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2803 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R18*	24	VJACK	100804	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.74	2x 6	SP-#2
BC	0.41	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 8- 4
BC Cont.	0- 0- 0	10- 8- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	625	176	3- 8	1- 0
			Hz =	180
B	338	175	3- 8	1- 0
C	116	2	3- 8	1- 0
			Hz =	97

Membr	CSI	P Lbs	Axl-CSI-Bnd
**----------Top Chords----------			
A -E	0.74	189 C	0.00 0.74
E -B	0.73	129 C	0.00 0.73
**--------Bottom Chords---------			
A -D	0.41	99 T	0.00 0.41
D -C	0.39	96 T	0.00 0.39
**----------Webs----------			
D -E	0.04	158 T	

LL Defl -0.20" in A -D L/611
TL Defl -0.41" in A -D L/293
Shear // Grain in E -B 0.24

Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.76
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

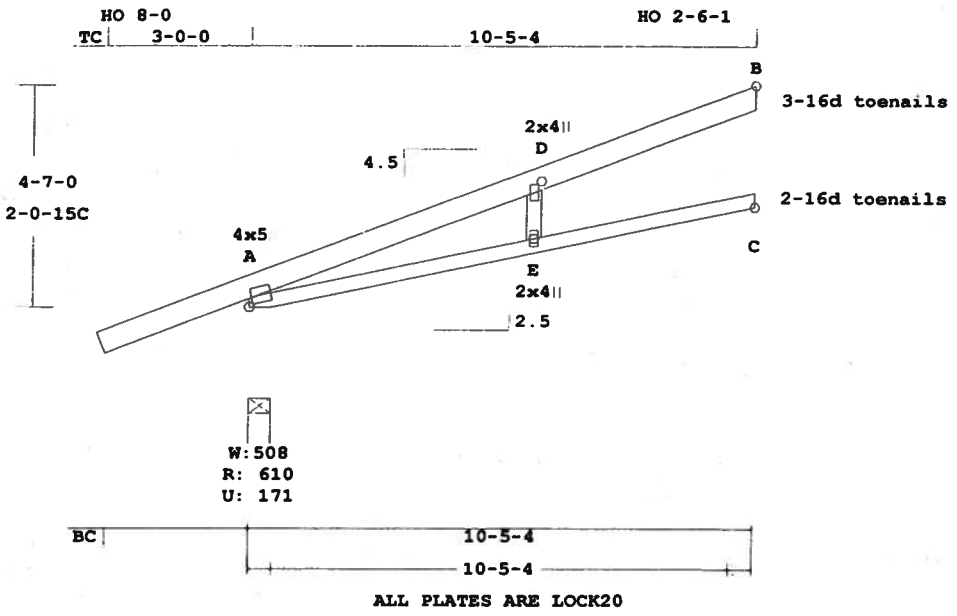
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 189 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R18A	2	HFSC	100504	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 63.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 6	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 5- 4
BC Cont.	0- 0- 0	10- 5- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	611	171	5- 8	1- 0
			Hz =	176
B	334	172	3- 8	1- 0
			Hz =	95
C	114	2	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.71	188	C 0.00 0.71
D -B	0.70	128	C 0.00 0.70
Bottom Chords			
A -E	0.40	7	C 0.00 0.40
E -C	0.38	9	C 0.00 0.38
Webs			
E -D	0.04	157	T

LL Defl -0.18" in A -E L/628
TL Defl -0.39" in A -E L/300
Shear // Grain in D -B 0.24

Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

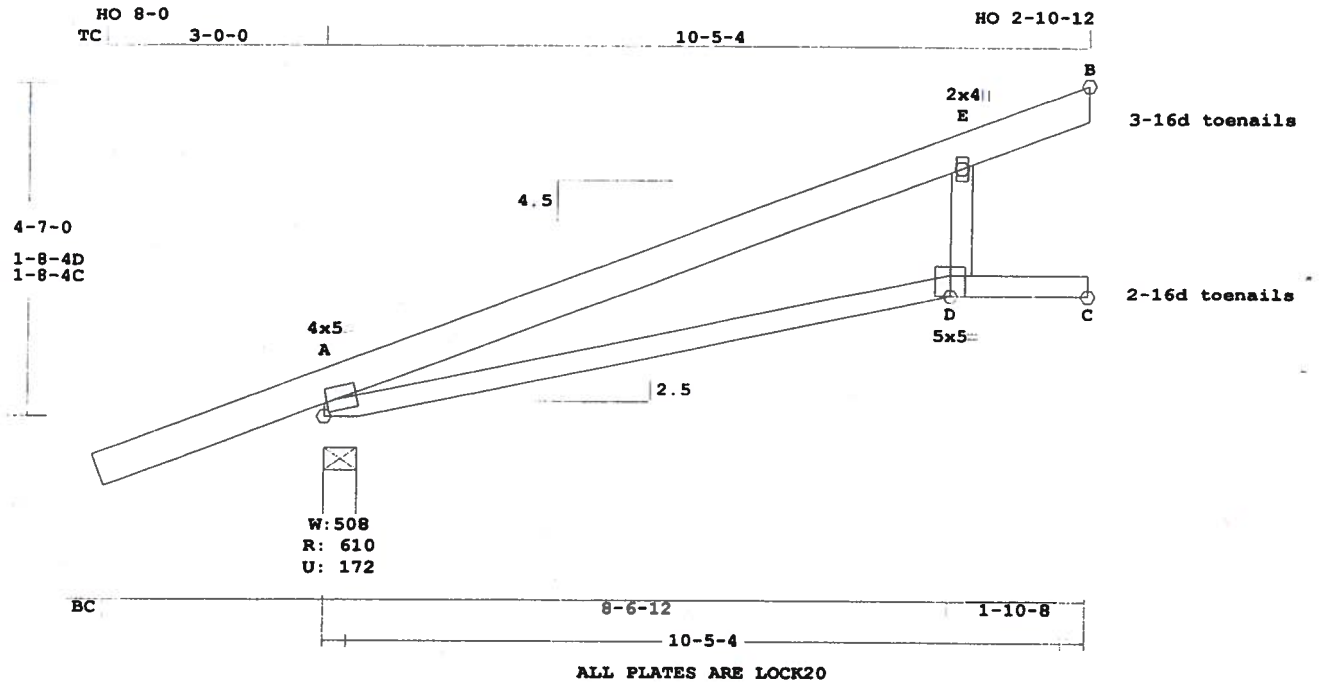
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 188 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R18B*	1	SP	100504	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 6	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 5- 4
BC Cont.	0- 0- 0	10- 5- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	610	172	5- 8	1- 0
			Hz =	176
B	374	116	3- 8	1- 0
			Hz =	95
C	53	57	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.71	183 C	0.00 0.71
E -B	0.40	109 T	0.01 0.39
--------Bottom Chords---------			
A -D	0.40	17 T	0.00 0.40
D -C	0.25	0 T	0.00 0.25
------------Webs------------			
D -E	0.06	245 T	

LL Defl -0.19" in A -D L/612
TL Defl -0.45" in A -D L/260
Shear // Grain in E -B 0.27

Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.46

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

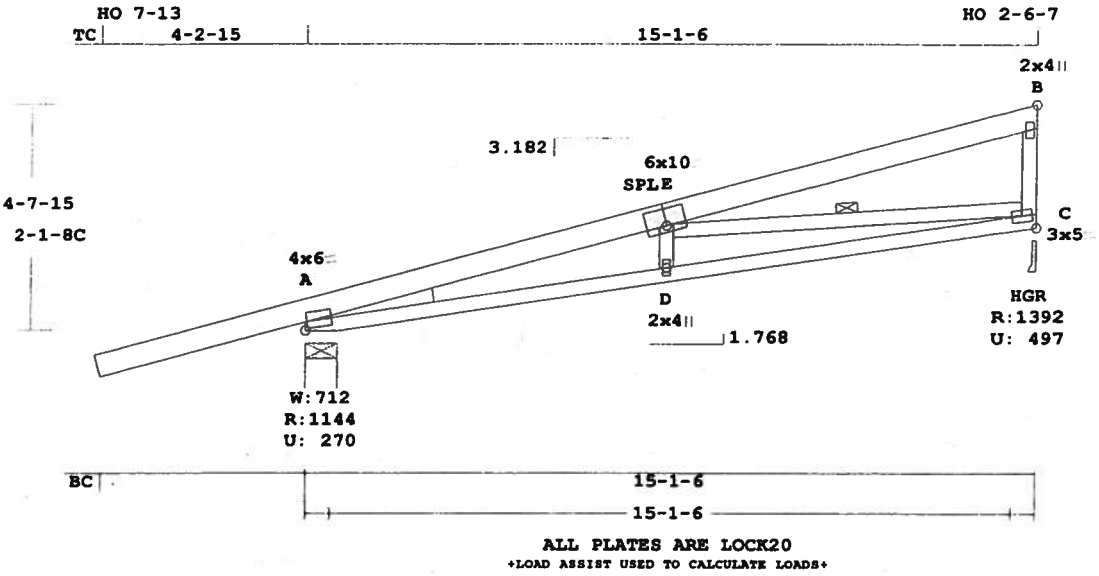
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 183 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R19*	1	HFSC	150106	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 109.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.90	2x 6	SP-#2
EX E -B		2x 6	SP-2400
BC	0.77	2x 4	SP-2400
WB	0.78	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	15- 1- 6
BC Cont.	0- 0- 0	15- 1- 6

WB 1 rows CLB on E -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	15.1'
BC V	0	20	0.0'	15.1'
TC V	-40	-20	0.0'	
	97	49		15.1'
BC V	0	-20	0.0'	
	0	49		15.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1144	270	7-12	1- 0
			Hz =	309
C	1392	498	3- 8	1- 2
			Hz =	166

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.90	3165 C	0.04	0.86
E -B	0.57	181 C	0.00	0.57
--------Bottom Chords---------				
A -D	0.59	3076 T	0.24	0.35
D -C	0.77	3080 T	0.15	0.62
-------------Webs-------------				
D -E	0.15	566 T		
E -C	0.78	3046 C		1 Br
C -B	0.15	666 C		

LL Defl -0.32" in D -C L/536
TL Defl -0.73" in D -C L/234
Shear // Grain in E -B 0.47

Hz Disp	LL	DL	TL
Jt C	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.6	0.6	0.82
E	LOCK	6.0x10.0	-0.3	1.2	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.46
D	LOCK	2.0x 4.0	Ctr	Ctr	0.51
C	LOCK	3.0x 5.0	Ctr	Ctr	0.95

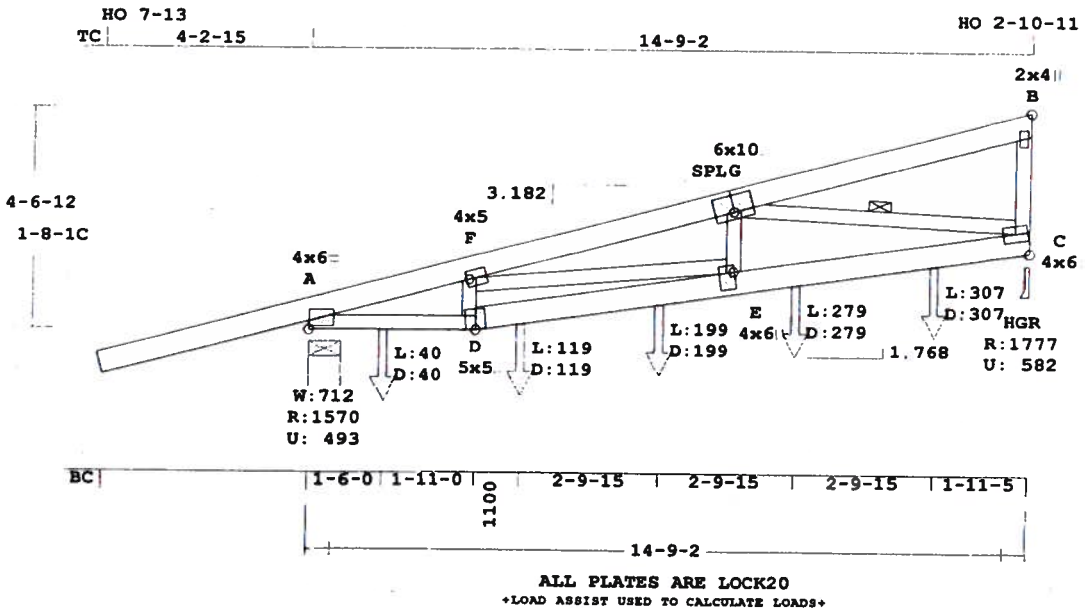
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack

Loading TC and BC
Setback 10- 8- 4
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 4- 3- 9. Support end or provide level return.
Provide connection to bearing for 308 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3165 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R20*	1	SP	140902	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 123.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 6	SP-#2
BC	0.81	2x 4	SP-#2
EX D -C		2x 6	SP-2400
WB	0.76	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 9- 2
BC Cont.	0- 0- 0	14- 9- 2

WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.8'
BC V	0	20	0.0'	14.8'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.2'	CL-LB
BC V	279	279	10.0'	CL-LB
BC V	307	307	12.8'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1571	493	7-12	1-14
			Hz =	176
C	1777	583	3- 8	1- 8
			Hz =	95

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -F	0.51	3135	C	0.03	0.48
F -G	0.41	3868	C	0.06	0.35
G -B	0.23	59	C	0.00	0.23
Bottom Chords					
A -D	0.81	2958	T	0.54	0.27
D -E	0.45	3009	T	0.15	0.30
E -C	0.64	3773	T	0.19	0.45
Webs					
D -F	0.03	154	T		
F -E	0.25	800	T		
E -G	0.36	1266	T		
G -C	0.76	3790	C		1 Br
C -B	0.05	208	T		

LL Defl -0.09" in D -E L/999
TL Defl -0.19" in D -E L/884
Shear // Grain in E -C 0.59

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

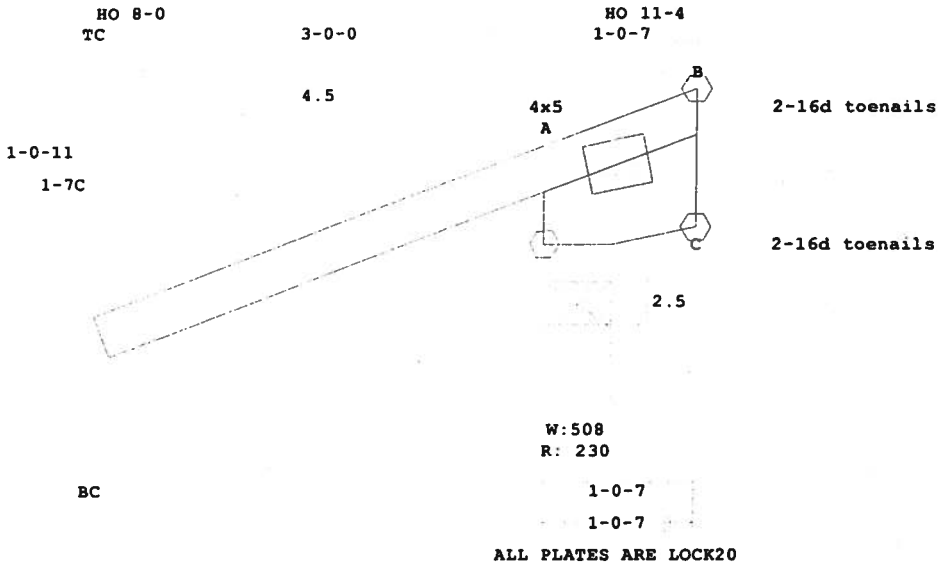
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.4	Ctr	0.74
F	LOCK	4.0x 5.0	Ctr	Ctr	0.73
G	LOCK	6.0x10.0	-0.3	1.2	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.46
D	LOCK	5.0x 5.0	-0.2	2.7	0.55
E	LOCK	4.0x 6.0	0.2	-1.0	0.77
C	LOCK	4.0x 6.0	Ctr	Ctr	0.96

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3868 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R21*	10	VJACK	10007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 12.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 4	SP-#2
BC	0.00	2x 8	SP-2400

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	231	0	3- 8	1- 0
		Hz =	42	
B	33	37G	1- 8	1- 0
C	84	107	1- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
--------Bottom Chords---------			
A -A	0.00	35	C
A -C	0.00	4	C

LL Defl 0.00" in D -C L/999
TL Defl 0.00" in D -C L/999
Shear // Grain in C -A 0.01

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	4.0x 5.0 Ctr	Ctr	1.00

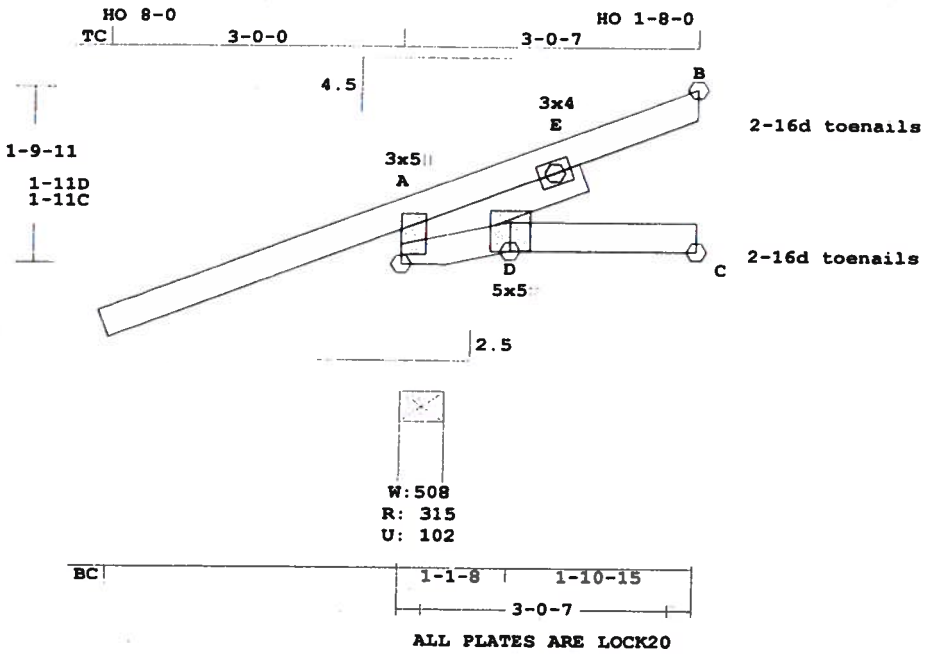
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 35 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R22*	1	SP	30007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 22.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
SL	0.01	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	315	102	5- 8	1- 0
			Hz =	51
E	91	62	3- 8	1- 0
			Hz =	28
C	50	0	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.11	58 C	0.00 0.11
E -E	0.11	65 C	0.00 0.11
E -B	0.00	3 C	
--------Bottom Chords---------			
A -D	0.09	8 C	0.00 0.09
D -C	0.08	0 T	0.00 0.08
-----------Sliders-----------			
A -E	0.01	98 C	

LL Defl 0.00" in D -C L/999
TL Defl 0.00" in D -D L/999
Shear // Grain in E -B 0.13

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.64
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	5.0x 5.0	Ctr	0.8	0.33

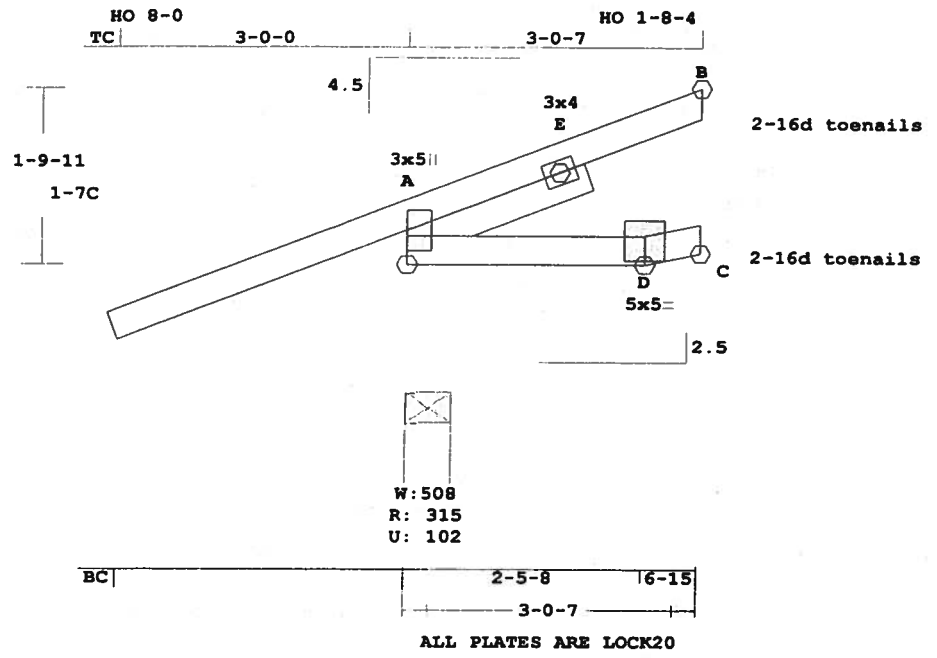
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 98 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R23	1	SP	30007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 22.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
SL	0.01	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	315	102	5- 8	1- 0
			Hz =	51
E	92	64	3- 8	1- 0
			Hz =	28
C	51	0	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
A -E	0.12	60	C 0.00 0.12
E -E	0.12	67	C 0.00 0.12
E -B	0.00	3	C
A -D	0.09	0	T 0.00 0.09
D -C	0.02	7	T 0.00 0.02
A -E	0.01	88	C

Top Chords
Bottom Chords
Sliders

LL Defl 0.00" in A -D L/999
TL Defl 0.00" in A -D L/999
Shear // Grain in E -E 0.14

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	5.0x 5.0	Ctr	1.2	0.39

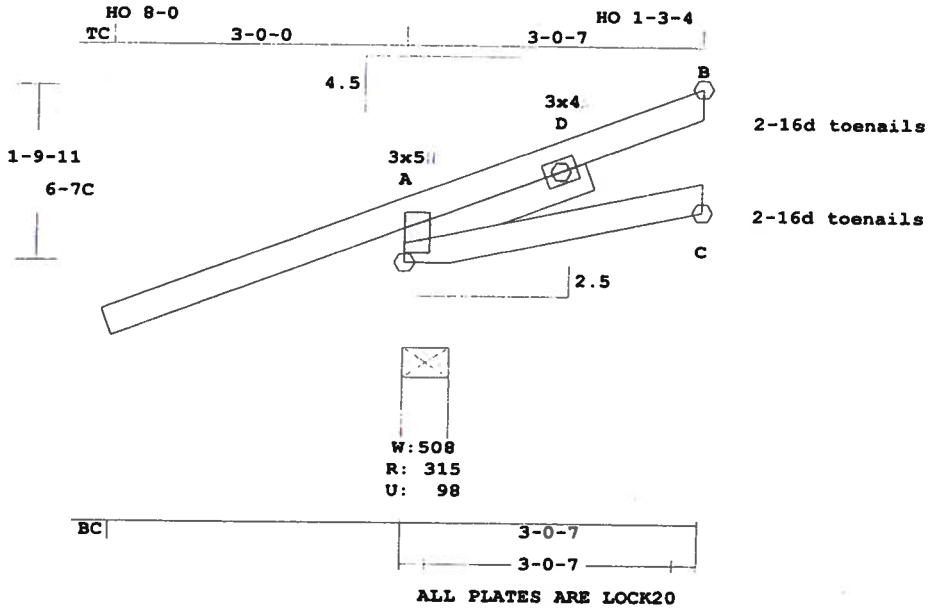
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 88 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R24	10	VJACK	30007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 22.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
SL	0.01	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	315	99	5- 8	1- 0
			Hz =	51
B	91	62	3- 8	1- 0
C	50	0	3- 8	1- 0
			Hz =	28

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords					
A -D	0.11	58	C	0.00	0.11
D -B	0.11	65	C	0.00	0.11
Bottom Chords					
A -C	0.09	26	T	0.00	0.09
Sliders					
A -D	0.01	99	C		

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in D -D 0.13

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.64
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

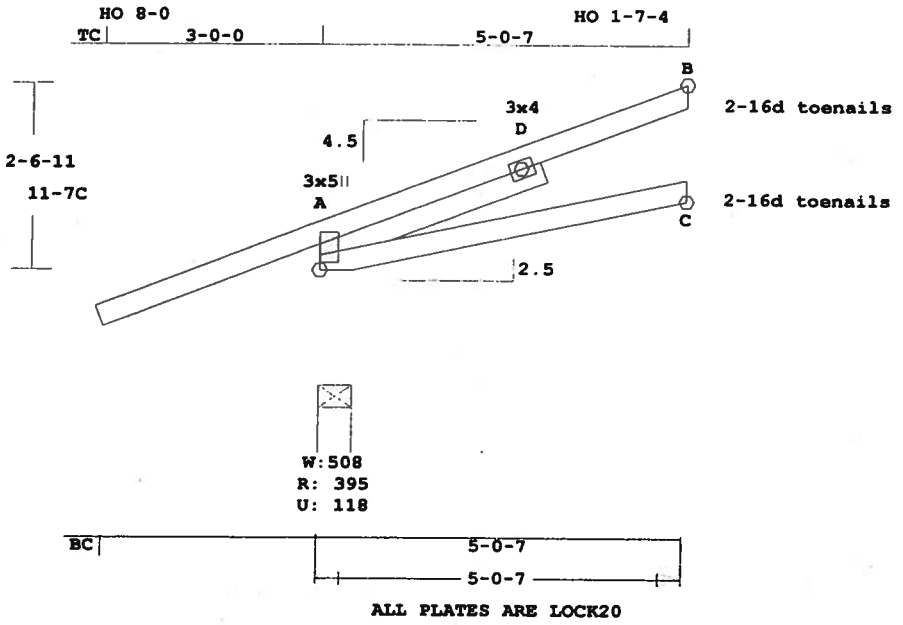
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 99 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R25*	10	VJACK	50007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.38	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
SL	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	395	118	5- 8	1- 0
			Hz =	85
B	150	99	3- 8	1- 0
C	83	0	3- 8	1- 0
			Hz =	46

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.38	219 C	0.02 0.36
D -B	0.36	105 C	0.00 0.36
Bottom Chords			
A -C	0.25	44 T	0.00 0.25
Sliders			
A -D	0.06	310 C	

LL Defl -0.01" in A -C L/999
TL Defl -0.03" in A -C L/999
Shear // Grain in A -D 0.28

Hz Disp	LL	DL	TL
Jt B	0.02"	0.00"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.65
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

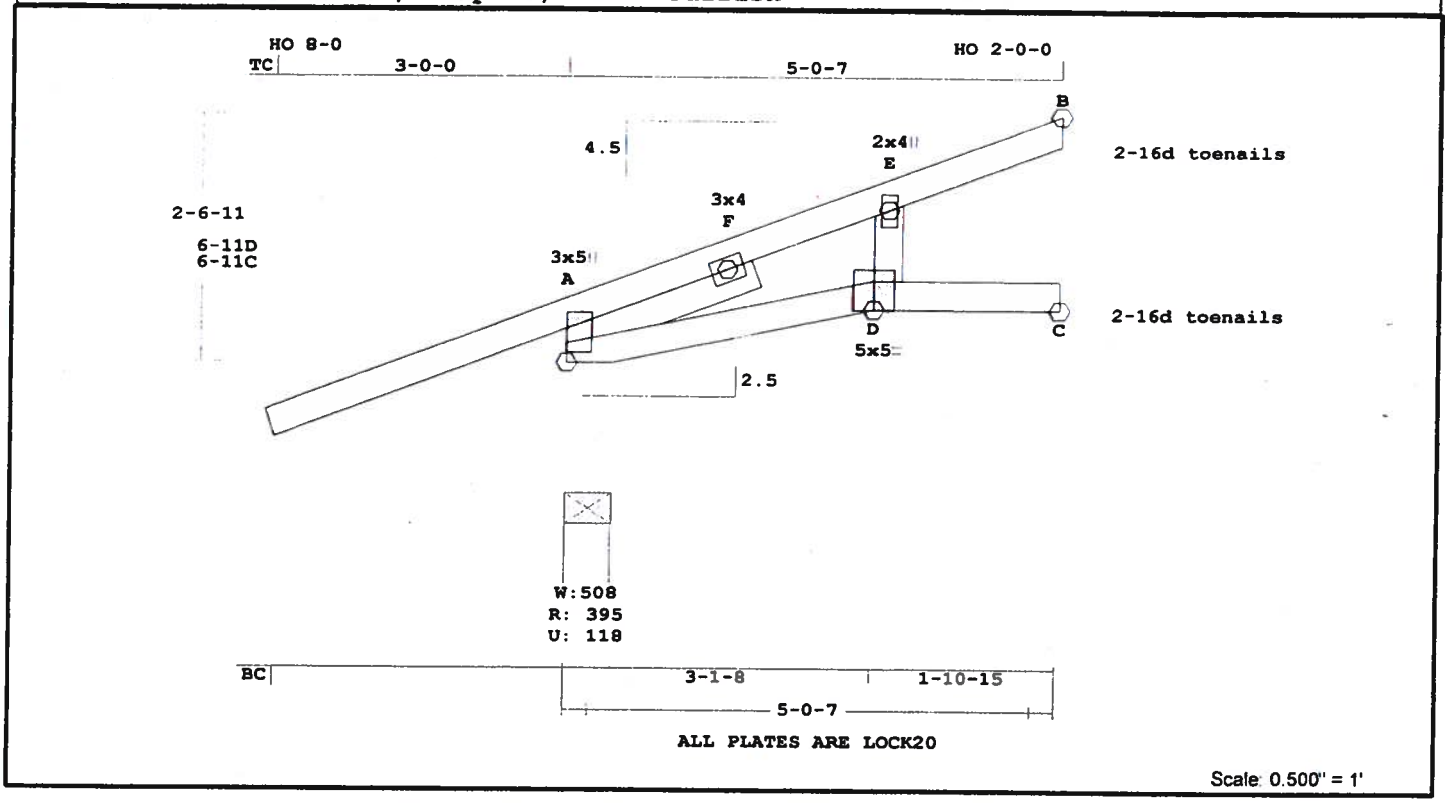
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 310 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R26*	1	SP	50007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 4	SP-#2
BC	0.23	2x 4	SP-#2
WB	0.03	2x 4	SP-#3
SL	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	396	119	5- 8	1- 0
			Hz =	85
B	132	73	3- 8	1- 0
			Hz =	46
C	76	16	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.26	95 C	0.00	0.26
F -E	0.29	145 C	0.00	0.29
E -B	0.20	82 C	0.00	0.20
--------Bottom Chords---------				
A -D	0.23	18 T	0.00	0.23
D -C	0.23	0 T	0.00	0.23
------------Webs------------				
D -E	0.03	123 T		
----------Sliders----------				
A -F	0.02	193 C		

LL Defl -0.02" in A -D L/999
TL Defl -0.05" in A -D L/999
Shear // Grain in F -F 0.25

Hz Disp	LL	DL	TL
Jt B	0.03"	0.00"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.65
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.46

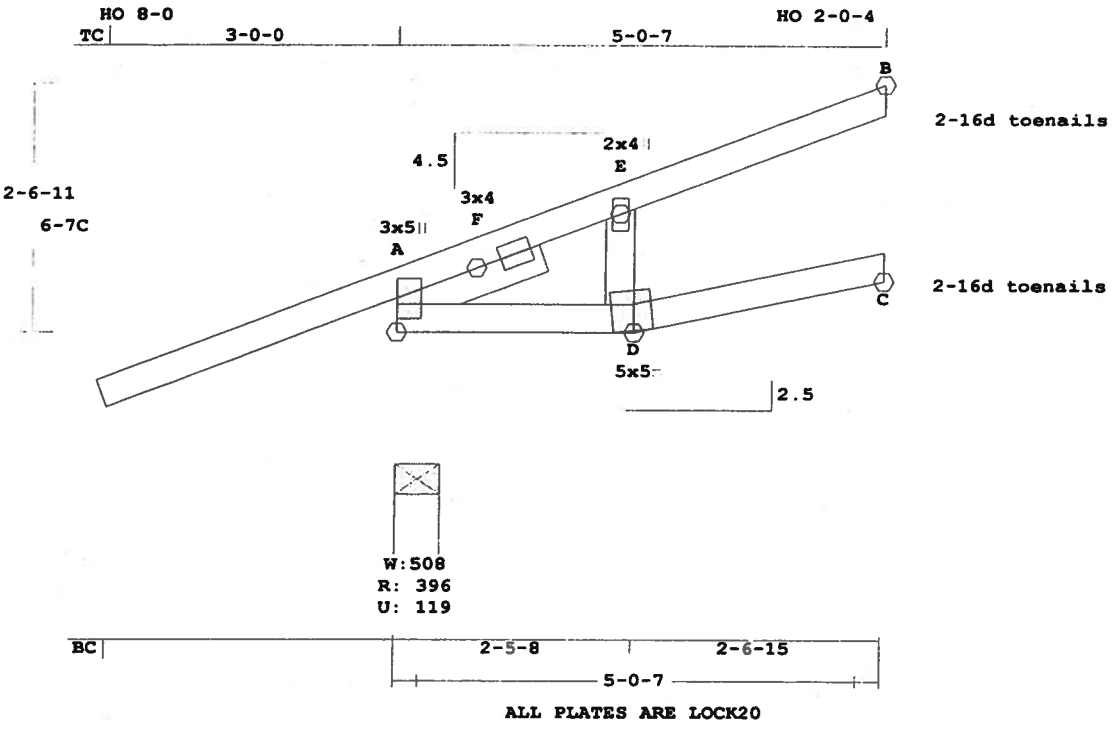
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 193 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R27	1	SP	50007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.30	2x 4	SP-#2
WB	0.04	2x 4	SP-#3
SL	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	396	120	5- 8	1- 0
			Hz =	85
B	128	78	3- 8	1- 0
			Hz =	46
C	89	10	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -F	0.21	91 C	0.00 0.21
F -E	0.22	136 C	0.00 0.22
E -B	0.22	86 C	0.00 0.22
Bottom Chords			
A -D	0.30	22 C	0.00 0.30
D -C	0.30	17 C	0.00 0.30
Webs			
D -E	0.04	188 T	
Sliders			
A -F	0.02	191 C	

LL Defl -0.02" in A -D L/999
TL Defl -0.04" in A -D L/999
Shear // Grain in F -F 0.20

Hz Disp	LL	DL	TL
Jt B	0.03"	0.00"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0-0.3	2.7		0.38

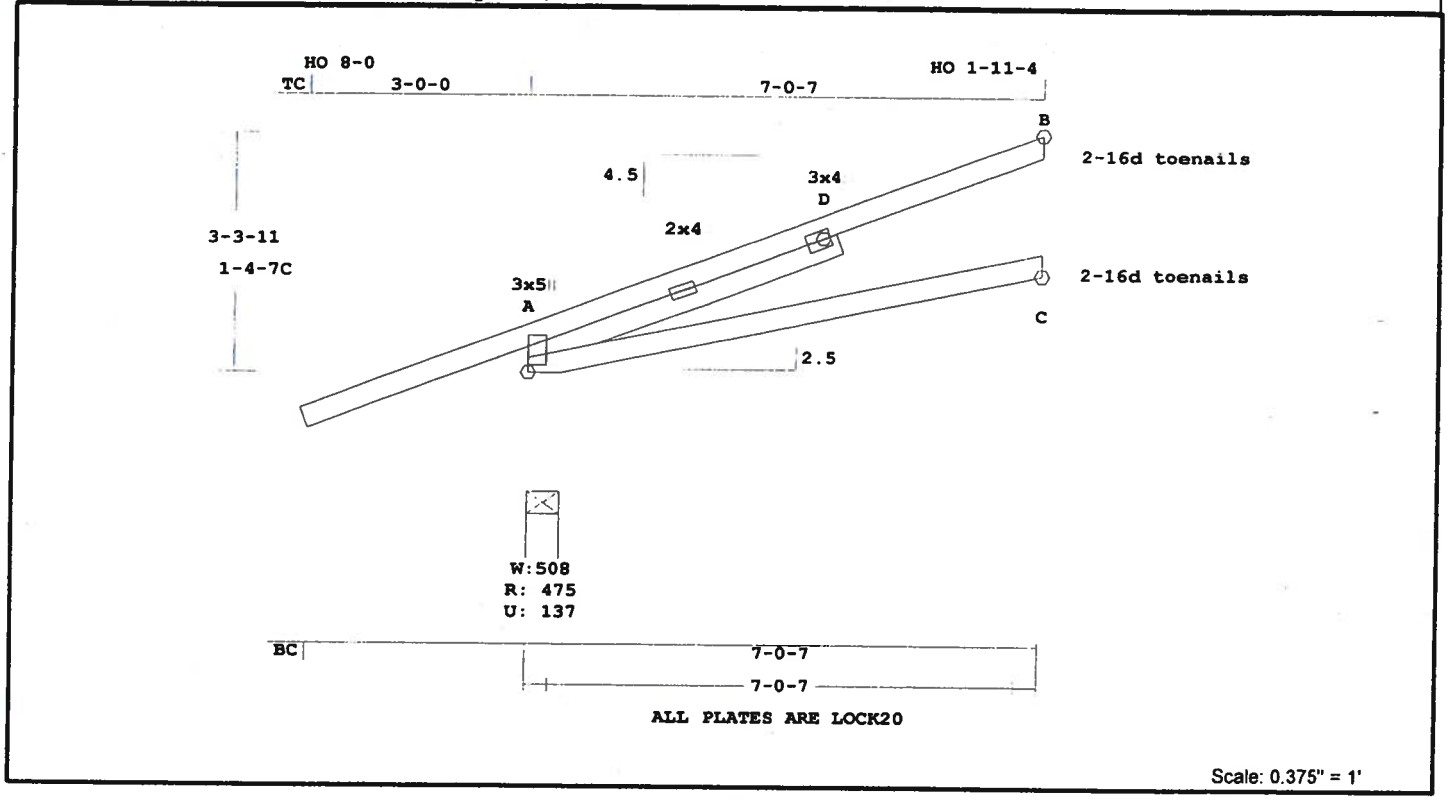
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 191 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R28	7	VJACK	70007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.13	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 7
BC	Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	476	137	5- 8	1- 0
			Hz =	119
B	209	136	3- 8	1- 0
C	116	0	3- 8	1- 0
			Hz =	64

Membr CSI P Lbs Axl-CSI-Bnd OH Loading

Top Chords				
A -D	0.53	448 C	0.00	0.53
D -B	0.53	117 C	0.00	0.53
Bottom Chords				
A -C	0.35	62 T	0.00	0.35
Sliders				
A -D	0.13	456 T		

LL Defl -0.06" in A -C L/999
TL Defl -0.13" in A -C L/559
Shear // Grain in A -D 0.28

Hz Disp	LL	DL	TL
Jt B	0.04"	-0.01"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

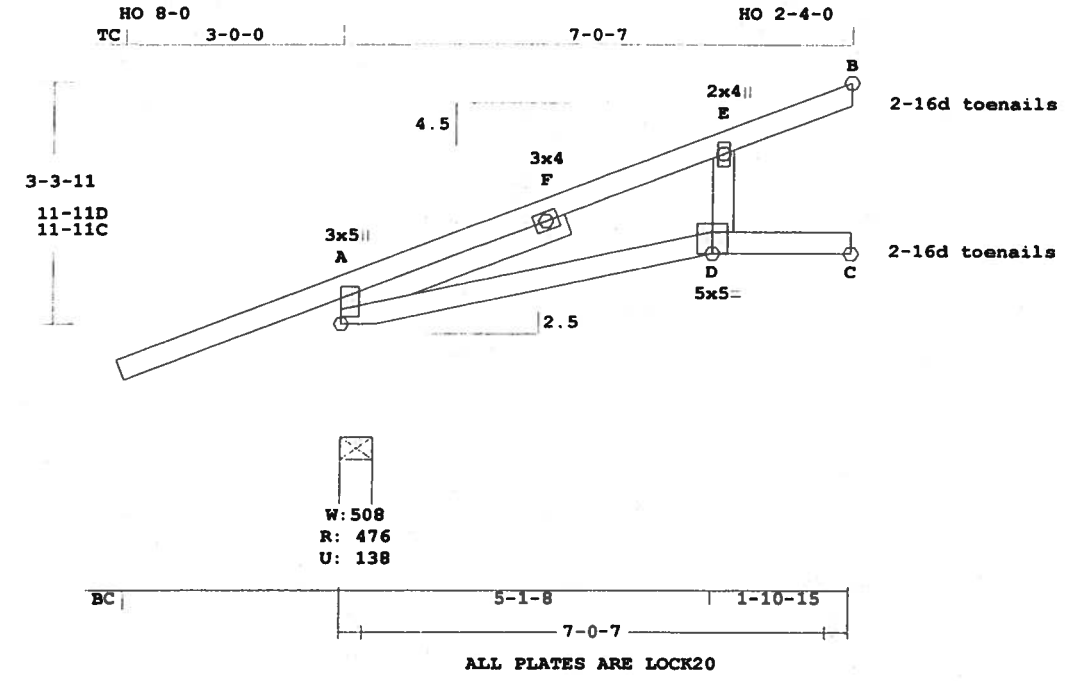
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 448 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R29*	1	SP	70007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.48	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.04	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 7
BC Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	476	138	5- 8	1- 0
			Hz =	119
B	165	76	3- 8	1- 0
			Hz =	64
C	120	44	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -F	0.47	336 C	0.00 0.47
F -E	0.48	163 C	0.00 0.48
E -B	0.27	76 C	0.00 0.27
Bottom Chords			
A -D	0.35	23 T	0.00 0.35
D -C	0.33	0 T	0.00 0.33
Webs			
D -E	0.04	182 T	
Sliders			
A -F	0.10	346 C	

LL Defl -0.09" in A -D L/835
TL Defl -0.18" in A -D L/415
Shear // Grain in A -F 0.27

Hz Disp	LL	DL	TL
Jt B	0.05"	-0.01"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.69
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr-1.0		0.46

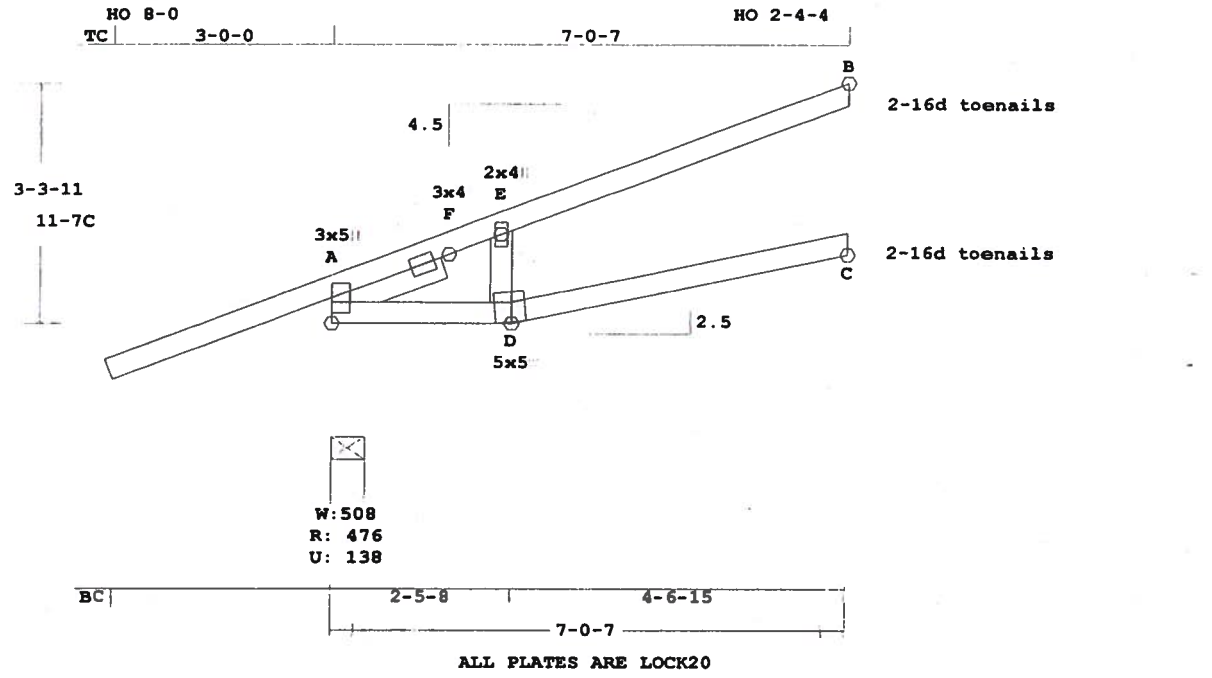
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 346 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R30*	1	SP	70007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 38.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.41	2x 4	SP-#2
BC	0.45	2x 4	SP-#2
WB	0.06	2x 4	SP-#3
SL	0.04	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 7
BC	Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	476	139	5- 8	1- 0
			Hz =	119
B	184	116	3- 8	1- 0
			Hz =	64
C	126	4	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.22	153 C	0.00	0.22
F -E	0.26	172 C	0.00	0.26
E -B	0.41	107 C	0.00	0.41
--------Bottom Chords---------				
A -D	0.45	28 C	0.00	0.45
D -C	0.45	12 C	0.00	0.45
-------------Webs-------------				
D -E	0.06	240 T		
-----------Sliders------------				
A -F	0.04	251 C		

LL Defl -0.08" in D -C L/920
TL Defl -0.18" in D -C L/420
Shear // Grain in E -B 0.24

Hz Disp	LL	DL	TL
Jt B	0.02"	0.02"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	-0.3	2.7	0.38

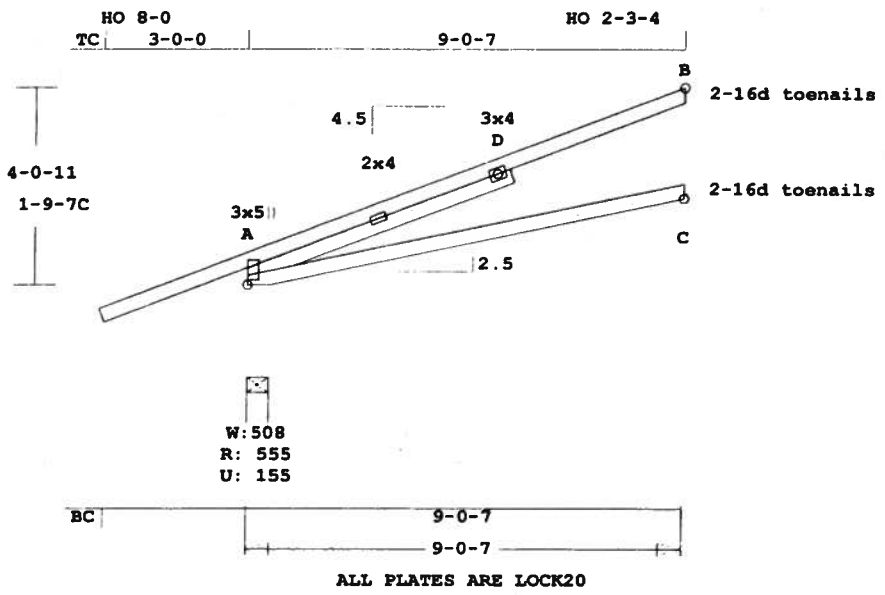
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 251 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R31*	9	HFSC	90007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 53.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.91	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
SL	0.22	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	556	156	5- 8	1- 0
			Hz =	152
B	267	172	3- 8	1- 0
			Hz =	82
C	150	0	3- 8	1- 0

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

A -D	0.91	746	C	0.00	0.91
D -B	0.91	130	C	0.00	0.91

--------Bottom Chords---------

A -C	0.52	9	C	0.00	0.52

----------Sliders-----------

A -D 0.22 753 T

LL Defl -0.16" in A -C L/622
TL Defl -0.38" in A -C L/260
Shear // Grain in A -D 0.37

Hz Disp	LL	DL	TL
Jt B	0.08"	-0.01"	0.07"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.73
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.52

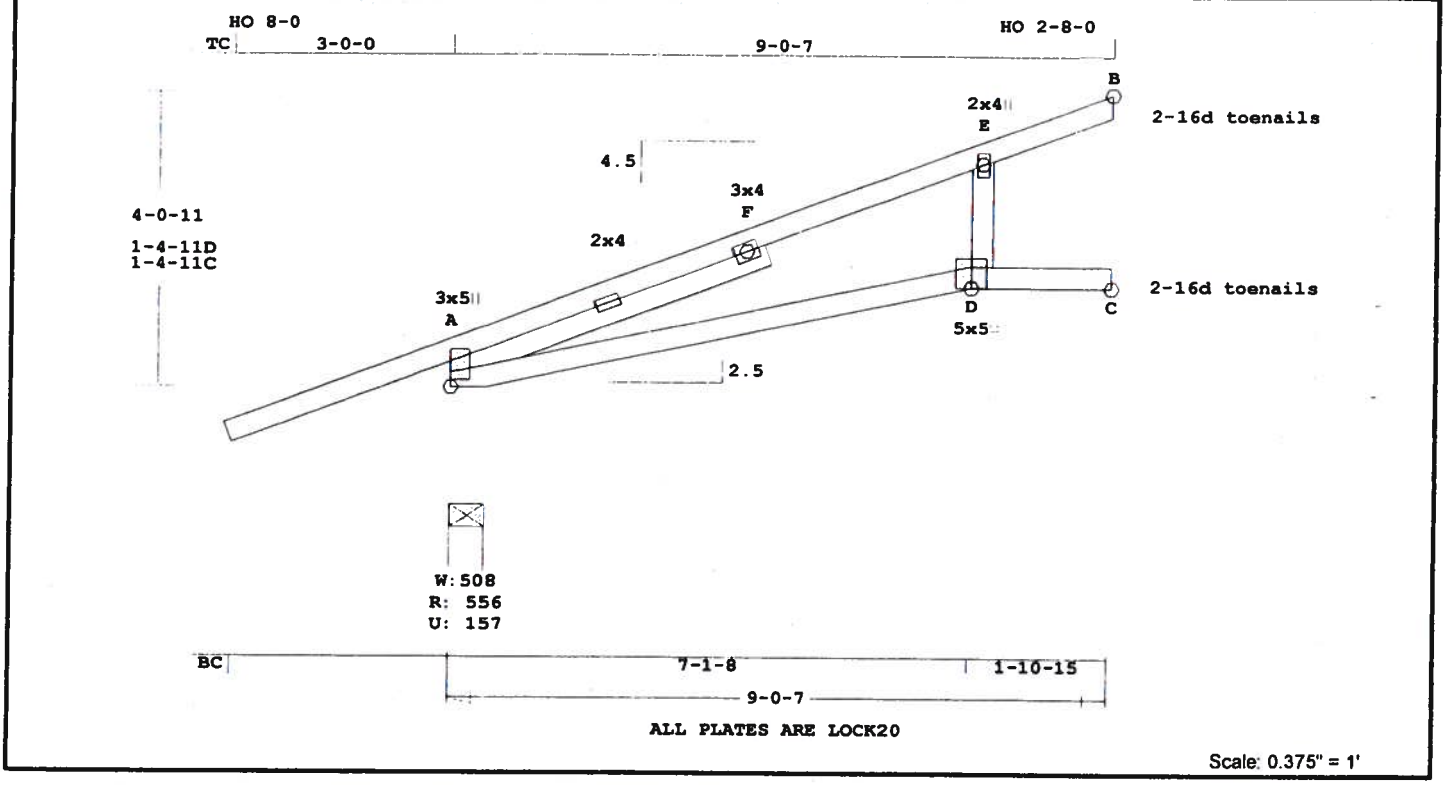
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 746 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R32	1	SP	90007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 53.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.85	2x 4	SP-#2
BC	0.57	2x 4	SP-#2
WB	0.06	2x 4	SP-#3
SL	0.19	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	557	158	5- 8	1- 0
			Hz =	152
B	189	68	3- 8	1- 0
			Hz =	82
C	176	83	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -F	0.85	640 C	0.00	0.85
F -E	0.85	180 C	0.00	0.85
E -B	0.33	80 T	0.00	0.33
Bottom Chords				
A -D	0.57	25 T	0.00	0.57
D -C	0.52	0 T	0.00	0.52
Webs				
D -E	0.06	243 T		
Sliders				
A -F	0.19	621 T		

LL Defl -0.25" in A -D L/405
TL Defl -0.50" in A -D L/199
Shear // Grain in A -F 0.37

Hz Disp	LL	DL	TL
Jt B	0.04"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.73
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.46

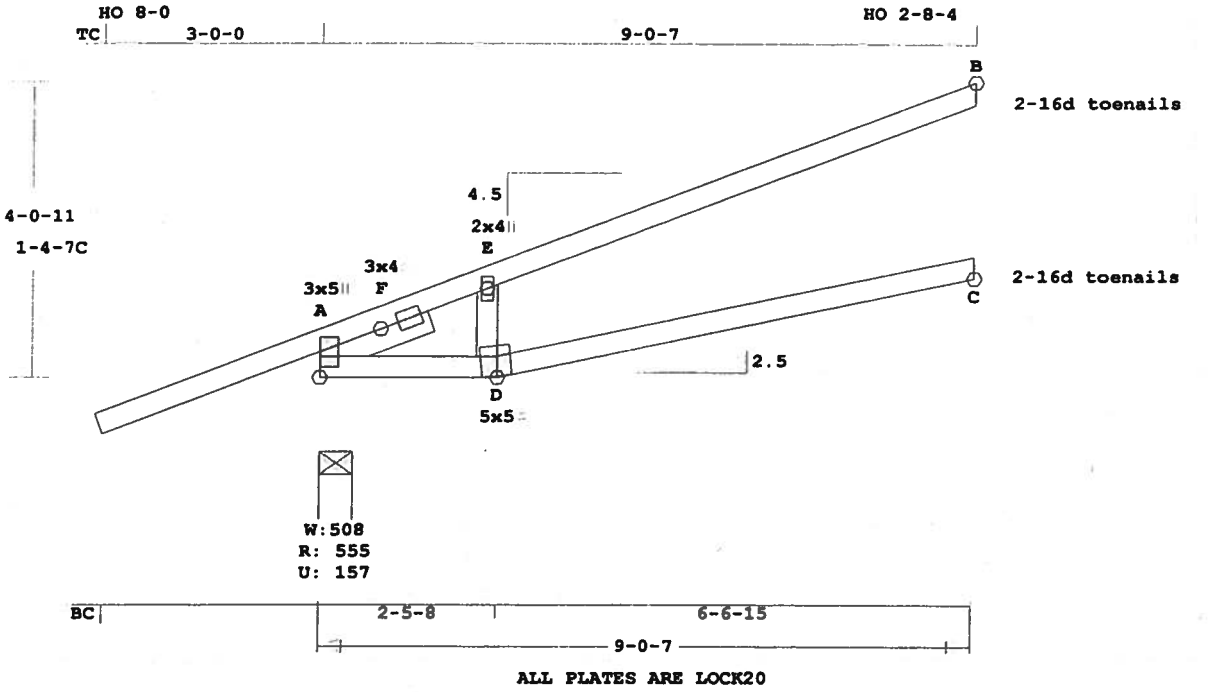
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 640 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R33	1	SP	90007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.74	2x 4	SP-#2
BC	0.71	2x 4	SP-#2
WB	0.08	2x 4	SP-#3
SL	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	556	157	5- 8	1- 0
			Hz =	153
B	240	153	3- 8	1- 0
			Hz =	82
C	164	0	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -F	0.29	227 C	0.00	0.29
F -E	0.37	213 C	0.00	0.37
E -B	0.74	123 C	0.00	0.74
Bottom Chords				
A -D	0.71	38 C	0.00	0.71
D -C	0.71	12 C	0.00	0.71
Webs				
D -E	0.08	332 T		
Sliders				
A -F	0.06	358 C		

LL Defl -0.24" in D -C L/422
TL Defl -0.53" in D -C L/188
Shear // Grain in E -B 0.28

Hz Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.10"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.73
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0-0.3	2.7		0.38

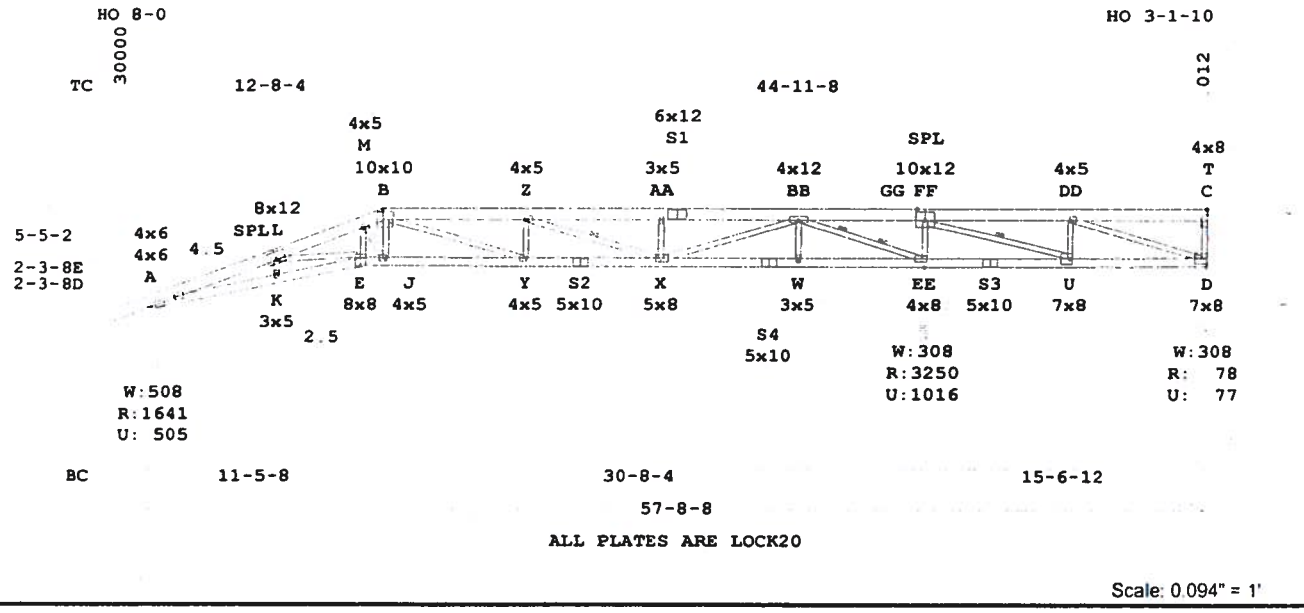
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 358 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R34*	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 527.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 8	SP-2400
BC	0.88	2x 6	SP-#2
WB	0.90	2x 4	SP-#3
EX X -BB		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8

WB 1 rows CLB on Z -X
WB 2 rows CLB on BB-EE
WB 1 rows CLB on FF-U
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	1641	506	5- 8	1-15
			Hz =	212
EE	3250	1017	3- 8	3- 7
D	78	78	3- 8	1- 0
			Hz =	115

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.17	5566	C	0.03	0.14
L -M	0.19	5315	C	0.03	0.16
M -B	0.11	4656	C	0.02	0.09
B -Z	0.12	4810	C	0.02	0.10
Z -AA	0.09	3441	C	0.01	0.08
AA-S1	0.05	3441	C	0.01	0.04
S1-BB	0.08	3441	C	0.01	0.07
BB-FF	0.34	3678	T	0.12	0.22
FF-DD	0.25	981	T	0.03	0.22
DD-T	0.08	0	T	0.00	0.08
--------Bottom Chords---------					
A -K	0.88	5260	T	0.64	0.24
K -E	0.88	5286	T	0.64	0.24
E -J	0.62	4942	T	0.60	0.02
J -Y	0.65	4463	T	0.54	0.11
Y -S2	0.67	4810	T	0.58	0.09
S2-X	0.67	4810	T	0.58	0.09
X -S4	0.13	273	T	0.01	0.12
S4-W	0.09	273	T	0.01	0.08
W -EE	0.33	273	T	0.03	0.30
EE-S3	0.35	3678	C	0.05	0.30
S3-U	0.20	3678	C	0.13	0.07
U -D	0.14	981	C	0.00	0.14
----------Webs----------					
K -L	0.03	132	T		
L -E	0.06	175	C		
E -M	0.38	1197	T		
M -J	0.14	868	C		
J -B	0.27	847	T		
B -Y	0.15	363	T		
Y -Z	0.06	230	T		
Z -X	0.38	1450	C	1 Br	
X -AA	0.08	467	C		
X -BB	0.98	3356	T		
W -BB	0.06	242	T		
BB-EE	0.76	4227	C	2 Br	
EE-FF	0.26	1514	C		
FF-U	0.90	2834	T	1 Br	
U -DD	0.13	747	C		
DD-D	0.39	1040	T		
D -T	0.03	206	C		

LL Defl -0.43" in J -Y L/999
TL Defl -0.87" in J -Y L/572
Shear // Grain in M -B 0.27

Hz Disp	LL	DL	TL
Jt EE	0.15"	0.15"	0.29"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

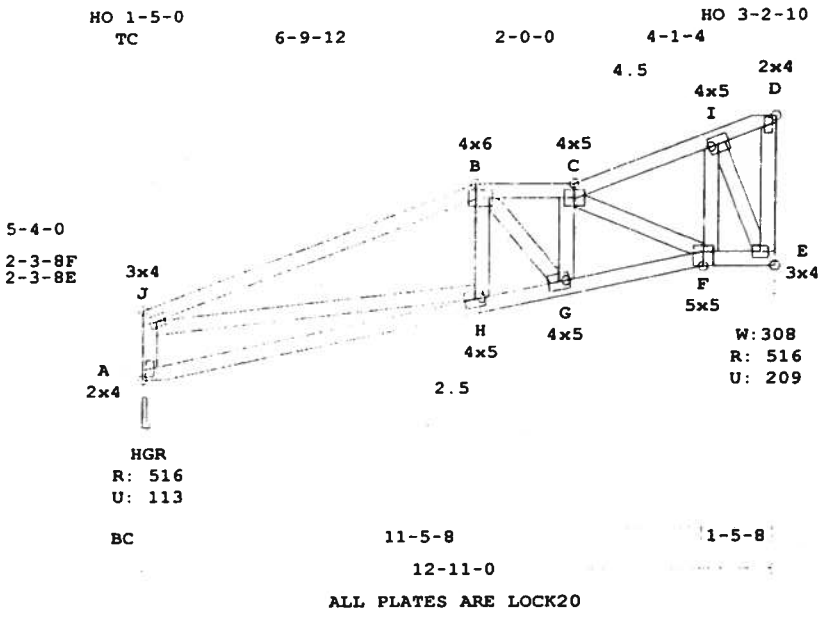
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.75
A	LOCK	4.0x 6.0	Ctr	Ctr	0.36
L	LOCK	8.0x12.0-0.6	1.5		0.97
M	LOCK	4.0x 5.0-0.5	0.2		0.59
B	LOCK	10.0x10.0-0.5	Ctr		0.88
Z	LOCK	4.0x 5.0	Ctr	0.5	0.70
AA	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
BB	LOCK	4.0x12.0	Ctr	0.5	0.90
FF	LOCK	10.0x12.0	Ctr	0.5	0.99
DD	LOCK	4.0x 5.0	Ctr	0.5	0.70
T	LOCK	4.0x 8.0-0.2	Ctr		0.85
K	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr-1.5		0.84
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.62
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
X	LOCK	5.0x 8.0	0.5	Ctr	0.93
S4	LOCK	5.0x10.0	Ctr	Ctr	0.82
W	LOCK	3.0x 5.0	Ctr	Ctr	0.39
EE	LOCK	4.0x 8.0-0.5	Ctr		0.75
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
U	LOCK	7.0x 8.0-0.5	Ctr		0.93
D	LOCK	7.0x 8.0-0.5	Ctr		0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5566 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R34A*	1	SP	121100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 95.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.54	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.25	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	12-11- 0
BC Cont.		0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
J -B	0.54	823	C	0.01	0.53
B -C	0.27	711	C	0.06	0.21
C -I	0.12	252	C	0.01	0.11
I -D	0.11	16	C	0.00	0.11
--------Bottom Chords---------					
A -H	0.27	343	C	0.00	0.27
H -G	0.35	776	T	0.08	0.27
G -F	0.19	731	T	0.12	0.07
F -E	0.08	232	T	0.03	0.05
------------Webs------------					
A -J	0.09	458	C	WindLd	
J -H	0.25	776	T		
H -B	0.05	200	T		
B -G	0.03	120	T		
G -C	0.00	54	C		
C -F	0.10	494	C		
F -I	0.12	374	T		
I -E	0.12	535	C		
E -D	0.00	6	T		

LL Defl -0.07" in A -H L/999
TL Defl -0.14" in A -H L/999
Shear // Grain in J -B 0.27

Hz Disp	LL	DL	TL
Jt E	0.01"	0.00"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 6.0	-0.5	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.74
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr	-1.0	0.46
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

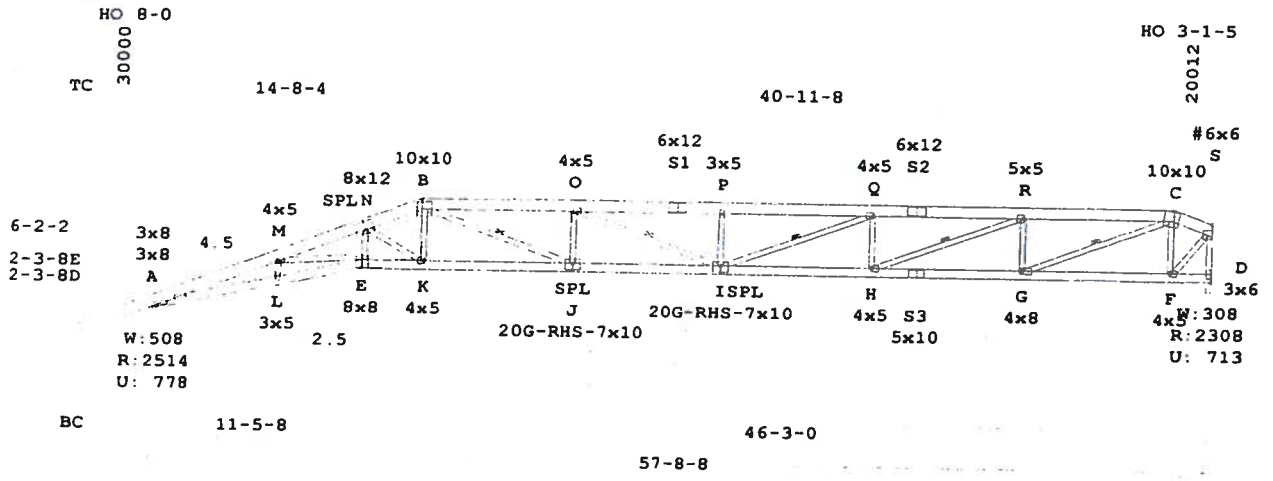
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 823 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R35	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 540.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S3-D	2x 6	SP-#2
WB	0.68	2x 4	SP-#3
EX	H -R	2x 4	SP-#2
EX	G -C	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8

WB 1 rows CLB on B -J
WB 1 rows CLB on O -I
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -R
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	2514	779	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	119

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.34	9819	C	0.11	0.23
M -N	0.37	10200	C	0.12	0.25
N -B	0.21	7912	C	0.07	0.14
B -O	0.22	9415	C	0.11	0.11
O -S1	0.25	9811	C	0.12	0.13
S1-P	0.21	9811	C	0.12	0.09
P -Q	0.22	9811	C	0.12	0.10
Q -S2	0.18	8615	C	0.09	0.09
S2-R	0.24	8615	C	0.09	0.15
R -C	0.20	5666	C	0.03	0.17
C -S	0.13	1459	C	0.00	0.13
--------Bottom Chords---------					
A -L	0.63	9265	T	0.42	0.21
L -E	0.63	9308	T	0.42	0.21
E -K	0.49	9493	T	0.43	0.06
K -J	0.39	7415	T	0.33	0.06
J -I	0.51	9415	T	0.43	0.08
I -H	0.45	8614	T	0.39	0.06
H -S3	0.31	5666	T	0.25	0.06
S3-G	0.85	5666	T	0.68	0.17
G -F	0.44	1326	T	0.16	0.28
F -D	0.28	118	T	0.00	0.28
------------Webs------------					
L -M	0.03	157	T		
M -E	0.15	473	T		
E -N	0.65	2053	T		
N -K	0.57	2431	C		
K -B	0.46	1464	T		
B -J	0.68	2137	T		1 Br
J -O	0.12	610	C		
O -I	0.13	427	T		1 Br
I -P	0.09	493	C		
I -Q	0.41	1293	T		1 Br
H -Q	0.20	1032	C		
H -R	0.58	3189	T		1 Br
G -R	0.31	1544	C		
G -C	0.85	4639	T		1 Br
F -C	0.26	1309	C		
F -S	0.65	2051	T		
D -S	0.42	2435	C		

LL Defl -1.09" in J -I L/627
TL Defl -2.20" in J -I L/310
Shear // Grain in A -M 0.23

Hz Disp	LL	DL	TL
Jt D	0.34"	0.34"	0.68"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

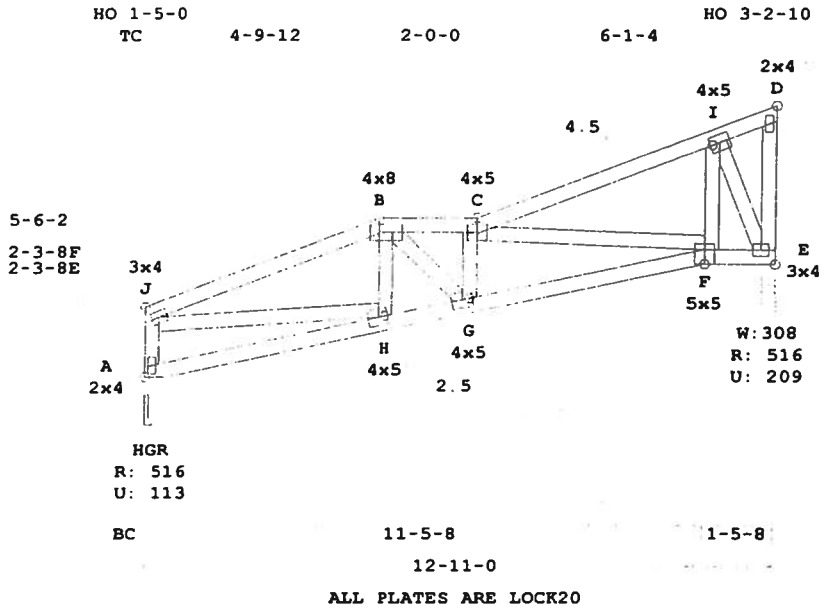
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.80
M	LOCK	4.0x 5.0	-0.5	0.2	0.63
N	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	-0.5	Ctr	0.90
O	LOCK	4.0x 5.0	Ctr	0.5	0.66
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	3.0x 5.0	Ctr	Ctr	0.39
Q	LOCK	4.0x 5.0	Ctr	0.5	0.66
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
R	LOCK	5.0x 5.0	Ctr	Ctr	0.96
C	LOCK	10.0x10.0	-2.1	-5.8	0.98
S	LOCK	6.0x 6.0	0.4	Ctr	0.87
L	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
K	LOCK	4.0x 5.0	Ctr	Ctr	0.69
J	RHS	7.0x10.0	Ctr	-0.8	0.97
I	RHS	7.0x10.0	Ctr	-0.8	0.97
H	LOCK	4.0x 5.0	0.5	Ctr	0.91
S3	LOCK	5.0x10.0	Ctr	Ctr	0.85
G	LOCK	4.0x 8.0	0.5	Ctr	0.92
F	LOCK	4.0x 5.0	Ctr	Ctr	0.80
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10200 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R35A	1	SP	121100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 4	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.28	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	12-11- 0
BC Cont.		0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
J -B	0.37	845 C	0.07	0.30
B -C	0.20	1006 C	0.08	0.12
C -I	0.27	270 C	0.00	0.27
I -D	0.27	45 C	0.00	0.27
--------Bottom Chords---------				
A -H	0.12	354 C	0.00	0.12
H -G	0.20	807 T	0.13	0.07
G -F	0.31	1052 T	0.17	0.14
F -E	0.15	257 T	0.04	0.11
----------Webs----------				
A -J	0.10	473 C	WindLd	
J -H	0.25	794 T		
H -B	0.02	110 T		
B -G	0.09	280 T		
G -C	0.03	239 C		
C -F	0.28	759 C		
F -I	0.09	306 T		
I -E	0.12	593 C		
E -D	0.02	85 T		

LL Defl -0.04" in H -G L/999
TL Defl -0.07" in H -G L/999
Shear // Grain in C -I 0.22

Hz Disp	LL	DL	TL
Jt E	0.01"	0.01"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.74
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr	-1.0	0.81
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

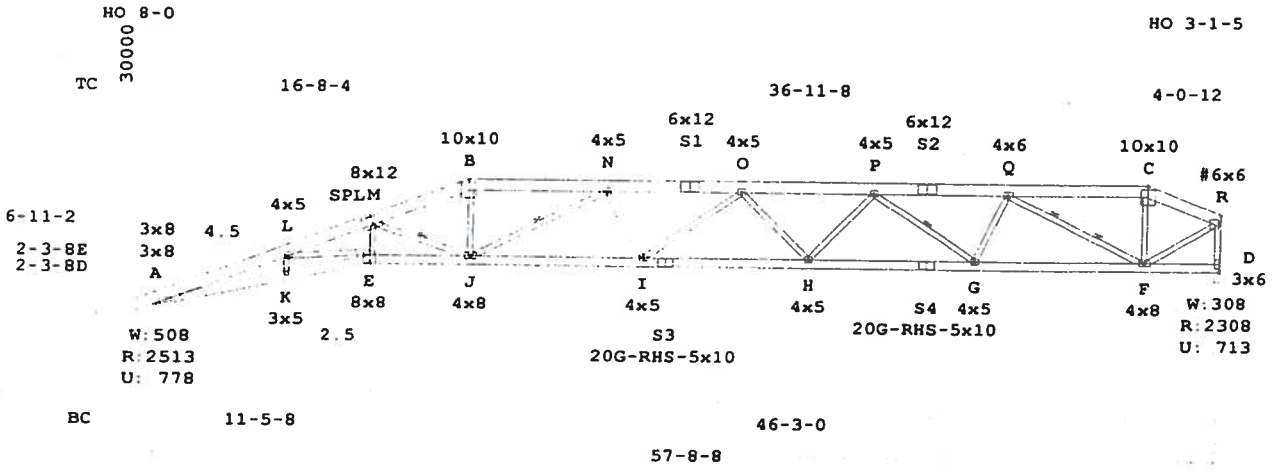
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1006 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R36	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 539.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.38	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S4-D	2x 6	SP-#2
WB	0.82	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	57- 8- 8
BC Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on M -J
WB 1 rows CLB on J -N
WB 1 rows CLB on P -G
WB 2 rows CLB on Q -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2514	779	5- 8	2- 1
			Hz =	211
D	2309	714	3- 8	2-12
			Hz =	124

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.34	9837	C	0.11	0.23
L -M	0.38	10168	C	0.12	0.26
M -B	0.21	7054	C	0.05	0.16
B -N	0.14	6657	C	0.05	0.09
N -S1	0.15	7934	C	0.07	0.08
S1-O	0.14	7934	C	0.07	0.07
O -P	0.15	7577	C	0.07	0.08
P -S2	0.10	5668	C	0.04	0.06
S2-Q	0.11	5668	C	0.04	0.07
Q -C	0.08	2150	C	0.00	0.08
C -R	0.04	2290	C	0.02	0.02
--------Bottom Chords---------					
A -K	0.63	9285	T	0.42	0.21
K -E	0.63	9328	T	0.42	0.21
E -J	0.48	9461	T	0.43	0.05
J -I	0.42	7834	T	0.35	0.07
I -S3	0.39	7965	T	0.36	0.03
S3-H	0.42	7965	T	0.36	0.06
H -S4	0.38	7061	T	0.32	0.06
S4-G	0.93	7061	T	0.85	0.08
G -F	0.72	5167	T	0.62	0.10
F -D	0.16	124	T	0.00	0.16
----------Webs----------					
K -L	0.03	152	T		
L -E	0.13	421	T		
E -M	0.66	2058	T		
M -J	0.58	3035	C		1 Br
J -B	0.62	1958	T		
J -N	0.40	1343	C		1 Br
N -I	0.09	357	T		
I -O	0.04	180	T		
O -H	0.25	569	C		
H -P	0.25	791	T		
P -G	0.40	1733	C		1 Br
G -Q	0.42	1309	T		
Q -F	0.67	3444	C		2 Br
F -C	0.14	477	T		
F -R	0.82	2558	T		
D -R	0.40	2293	C		

LL Defl -0.82" in I -S3 L/833
TL Defl -1.66" in J -I L/411
Shear // Grain in A -L 0.23

Hz Disp	LL	DL	TL
Jt D	0.32"	0.33"	0.65"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

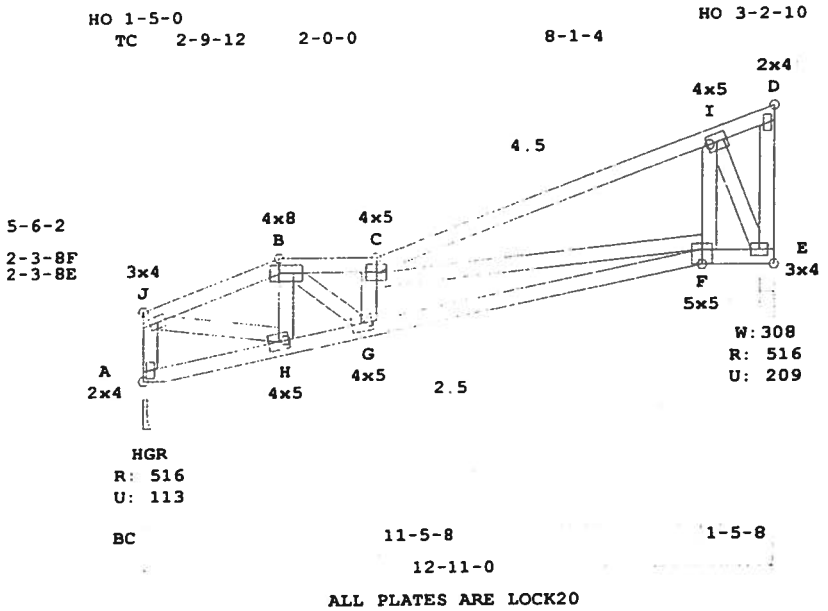
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0	-0.5	0.2	0.63
M	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
N	LOCK	4.0x 5.0	Ctr	0.5	0.61
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
O	LOCK	4.0x 5.0	Ctr	0.5	0.57
P	LOCK	4.0x 5.0	Ctr	0.5	0.58
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 6.0	0.3	0.5	0.89
C	LOCK	10.0x10.0	Ctr	Ctr	0.73
R	LOCK	6.0x 6.0	-0.6	0.2	0.95
K	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
J	LOCK	4.0x 8.0	Ctr	Ctr	0.66
I	LOCK	4.0x 5.0	Ctr	Ctr	0.55
S3	RHS	5.0x10.0	Ctr	Ctr	0.89
H	LOCK	4.0x 5.0	Ctr	Ctr	0.48
S4	RHS	5.0x10.0	Ctr	Ctr	0.94
G	LOCK	4.0x 5.0	Ctr	Ctr	0.58
F	LOCK	4.0x 8.0	0.5	Ctr	0.98
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10168 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R36A*	1	SP	121100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 92.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.57	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.67	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
J -B	0.16	708 C	0.05	0.11
B -C	0.33	1236 C	0.01	0.32
C -I	0.57	341 C	0.00	0.57
I -D	0.57	95 C	0.00	0.57
--------Bottom Chords---------				
A -H	0.04	329 C	0.00	0.04
H -G	0.19	682 T	0.11	0.08
G -F	0.44	1301 T	0.22	0.22
F -E	0.14	329 T	0.05	0.09
----------Webs----------				
A -J	0.09	492 C	WindLd	
J -H	0.21	678 T		
H -B	0.03	204 C		
B -G	0.22	685 T		
G -C	0.08	497 C		
C -F	0.67	935 C		
F -I	0.08	311 T		
I -E	0.15	759 C		
E -D	0.07	226 T		

LL Defl -0.07" in G -F L/999
TL Defl -0.16" in G -F L/917
Shear // Grain in I -D 0.37

Hz Disp	LL	DL	TL
Jt E	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.74
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr	-1.0	0.68
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

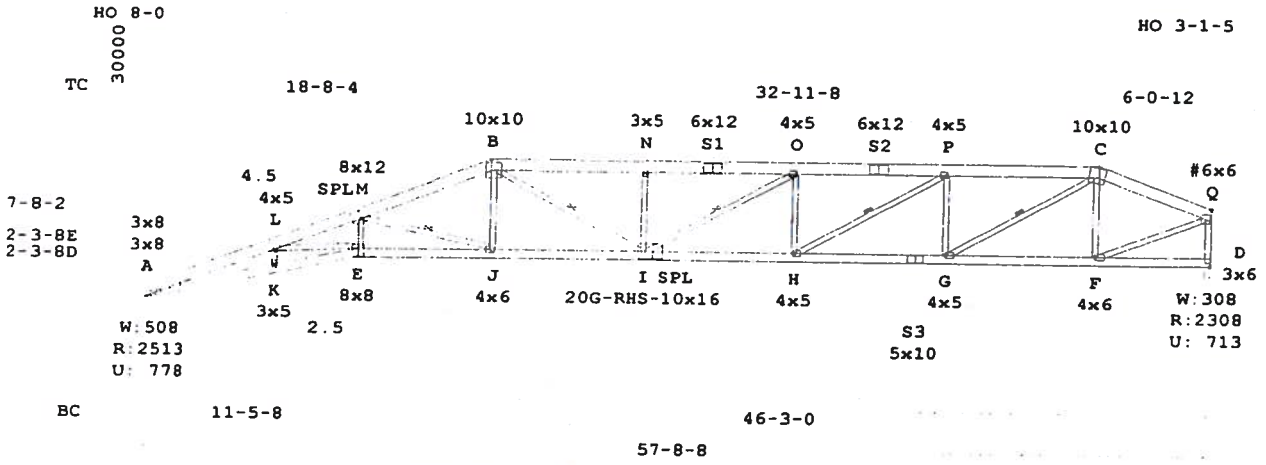
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1236 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R37	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 553.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX I -S3		2x 6	SP-#2
EX S3-D		2x 6	SP-#2
WB	0.92	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8

WB 1 rows CLB on M -J
WB 1 rows CLB on B -I
WB 1 rows CLB on I -O
WB 1 rows CLB on H -P
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	2514	779	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	130

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.34	9849 C	0.11	0.23
L -M	0.37	10186 C	0.12	0.25
M -B	0.23	6361 C	0.05	0.18
B -N	0.14	6823 C	0.05	0.09
N -S1	0.13	6823 C	0.06	0.07
S1-O	0.13	6823 C	0.06	0.07
O -S2	0.15	6526 C	0.05	0.10
S2-P	0.15	6526 C	0.05	0.10
P -C	0.12	5085 C	0.02	0.10
C -Q	0.10	2724 C	0.00	0.10
Bottom Chords				
A -K	0.63	9295 T	0.42	0.21
K -E	0.63	9335 T	0.42	0.21
E -J	0.47	9480 T	0.43	0.04
J -I	0.31	5958 T	0.27	0.04
I -H	0.88	6525 T	0.79	0.09
H -S3	0.72	5084 T	0.61	0.11
S3-G	0.67	5084 T	0.61	0.06
G -F	0.44	2558 T	0.31	0.13
F -D	0.18	129 T	0.00	0.18
Webs				
K -L	0.03	142 T		
L -E	0.13	433 T		
E -M	0.66	2069 T		
M -J	0.92	3647 C		1 Br
J -B	0.38	1206 T		
B -I	0.31	990 T		1 Br
I -N	0.16	503 C		
I -O	0.11	345 T		1 Br
H -O	0.22	685 C		
H -P	0.53	1675 T		1 Br
G -P	0.42	1277 C		
G -C	0.92	2898 T		1 Br
F -C	0.30	911 C		
F -Q	0.88	2762 T		
D -Q	0.39	2269 C		

LL Defl -0.74" in J -I L/924
TL Defl -1.50" in J -I L/457
Shear // Grain in A -L 0.23

Hz Disp	LL	DL	TL
Jt D	0.29"	0.30"	0.59"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

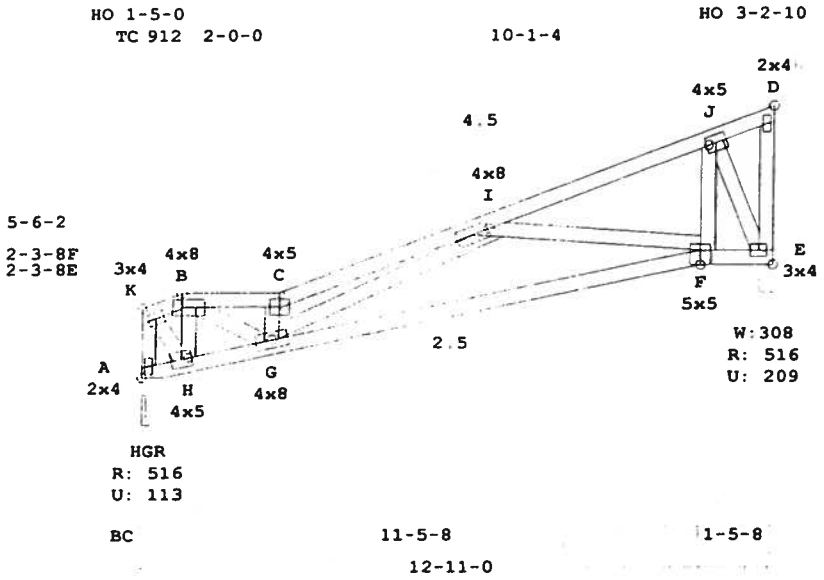
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0	-0.5	0.2	0.63
M	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	-0.5	Ctr	0.73
N	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	4.0x 5.0	Ctr	0.5	0.79
C	LOCK	10.0x10.0	-1.6	-5.8	0.96
Q	LOCK	6.0x 6.0	0.1	0.1	0.89
K	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
J	LOCK	4.0x 6.0	Ctr	Ctr	0.89
I	RHS	10.0x16.0	-1.0	Ctr	0.87
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	1.0	Ctr	1.00
F	LOCK	4.0x 6.0	1.0	Ctr	0.99
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10186 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R37A*	1	SP	121100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.43	2x 4	SP-#2
WB	0.32	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11- 0
BC	Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	113	3- 8	1- 0
		Hz =	214	
E	517	210	3- 8	1- 0
		Hz =	124	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
K -B	0.07	286	C	0.00	0.07
B -C	0.12	1187	C	0.00	0.12
C -I	0.22	1254	C	0.01	0.21
I -J	0.21	271	C	0.00	0.21
J -D	0.14	20	C	0.00	0.14
--------Bottom Chords---------					
A -H	0.02	306	C	0.00	0.02
H -G	0.38	433	C	0.03	0.35
G -F	0.43	989	T	0.08	0.35
F -E	0.14	237	T	0.00	0.14
----------Webs----------					
A -K	0.08	509	C	WindLd	
K -H	0.12	401	T		
H -B	0.06	389	C		
B -G	0.32	1012	T		
G -C	0.09	583	C		
G -I	0.12	475	T		
I -F	0.27	722	C		
F -J	0.11	443	T		
J -E	0.10	548	C		
E -D	0.00	26	C		

LL Defl -0.14" in G -F L/999
TL Defl -0.30" in G -F L/493
Shear // Grain in C -I 0.20

Hz Disp	LL	DL	TL
Jt E	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
K	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 8.0	Ctr	Ctr	0.91
J	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.61
G	LOCK	4.0x 8.0	Ctr	Ctr	0.81
F	LOCK	5.0x 5.0	Ctr	-1.0	0.81
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

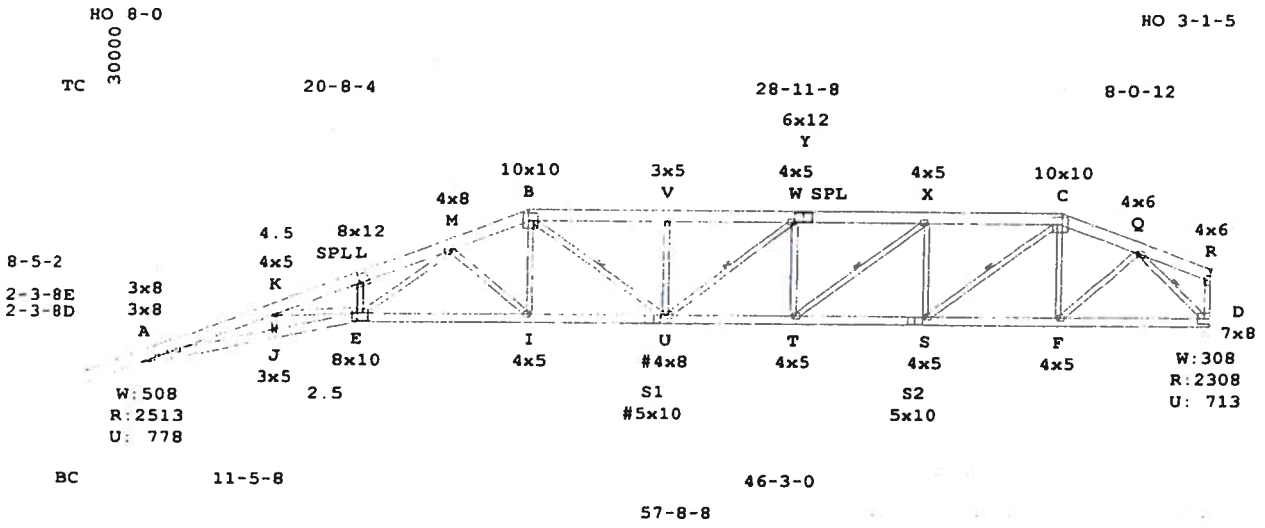
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1254 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R38	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 574.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.41	2x 8	SP-2400
BC	1.00	2x 6	SP-#2
EX A -E		2x 6	SP-2400
WB	0.82	2x 4	SP-#3
EX E -M		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8

WB 1 rows CLB on B -U
WB 1 rows CLB on U -W
WB 1 rows CLB on T -X
WB 1 rows CLB on S -C
WB 1 rows CLB on Q -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2514	779	5- 8	2- 1
			Hz =	210
D	2309	714	3- 8	2-12
			Hz =	135

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.37	9591	C	0.10	0.27
K -L	0.41	10422	C	0.12	0.29
L -M	0.26	10415	C	0.12	0.14
M -B	0.07	5820	C	0.03	0.04
B -V	0.12	5924	C	0.04	0.08
V -W	0.09	5924	C	0.04	0.05
W -Y	0.08	5637	C	0.06	0.02
Y -X	0.11	5637	C	0.04	0.07
X -C	0.08	4620	C	0.02	0.06
C -Q	0.06	3038	C	0.03	0.03
Q -R	0.02	57	T	0.00	0.02
Bottom Chords					
A -J	0.71	9041	T	0.41	0.30
J -E	0.71	9112	T	0.41	0.30
E -I	1.00	6711	T	0.81	0.19
I -S1	0.80	5431	T	0.66	0.14
S1-U	0.69	5431	T	0.66	0.03
U -T	0.75	5637	T	0.68	0.07
T -S2	0.64	4620	T	0.56	0.08
S2-S	0.57	4621	T	0.56	0.01
S -F	0.46	2841	T	0.34	0.12
F -D	0.36	2017	T	0.24	0.12
Webs					
J -K	0.05	256	C		
K -E	0.29	911	T		
E -L	0.05	269	C		
E -M	0.75	3891	T		
M -I	0.82	1682	C		
I -B	0.42	1331	T		
B -U	0.19	614	T	1 Br	
U -V	0.20	468	C		
U -W	0.11	366	T	1 Br	
T -W	0.28	654	C		
T -X	0.41	1295	T	1 Br	
S -X	0.51	1193	C		
S -C	0.70	2213	T	1 Br	
F -C	0.25	590	C		
F -Q	0.36	1124	T		
Q -D	0.55	2934	C	1 Br	
D -R	0.03	148	T		

LL Defl -0.69" in E -I L/995
TL Defl -1.43" in E -I L/477
Shear // Grain in A -K 0.23

Hz Disp	LL	DL	TL
Jt D	0.28"	0.28"	0.56"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.88
A	LOCK	3.0x 8.0	Ctr	Ctr	0.76
K	LOCK	4.0x 5.0-0.5	0.2		0.62
L	LOCK	8.0x12.0-0.6	1.5		0.97
M	LOCK	4.0x 8.0-1.2-0.1			0.89
B	LOCK	10.0x10.0-0.5	Ctr		0.73
V	LOCK	3.0x 5.0	Ctr	Ctr	0.39
W	LOCK	4.0x 5.0	Ctr	0.5	0.60
Y	LOCK	6.0x12.0	Ctr	Ctr	0.91
X	LOCK	4.0x 5.0	Ctr	0.5	0.63
C	LOCK	10.0x10.0	0.5	Ctr	0.74
Q	LOCK	4.0x 6.0	0.5	0.2	0.79
R	LOCK	4.0x 6.0-0.2	Ctr		1.00
J	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x10.0	Ctr-1.5		0.92
I	LOCK	4.0x 5.0	Ctr	Ctr	0.62
S1#	LOCK	5.0x10.0-0.9		0.1	0.82
U#	LOCK	4.0x 8.0	0.9	0.1	0.61
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
S	LOCK	4.0x 5.0	Ctr	Ctr	0.81
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	7.0x 8.0-0.5	Ctr		0.85

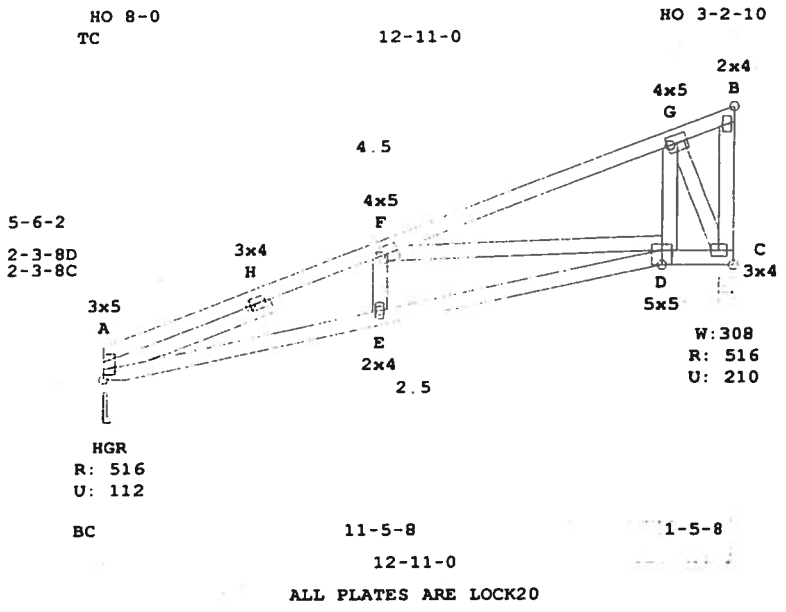
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10422 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R38A*	1	SP	121100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.46	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	112	3- 8	1- 0
			Hz =	218
C	517	211	3- 8	1- 0
			Hz =	117

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -H	0.10	746 C	0.04 0.06
H -F	0.24	1242 C	0.07 0.17
F -G	0.33	285 C	0.00 0.33
G -B	0.33	54 C	0.00 0.33
Bottom Chords			
A -E	0.29	1192 T	0.19 0.10
E -D	0.36	1198 T	0.20 0.16
D -C	0.17	264 T	0.04 0.13
Webs			
E -F	0.05	202 T	
F -D	0.46	897 C	
D -G	0.08	304 T	
G -C	0.11	609 C	
C -B	0.03	112 T	
Sliders			
A -H	0.10	558 C	

LL Defl -0.04" in E -D L/999
TL Defl -0.09" in E -D L/999
Shear // Grain in F -G 0.23

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

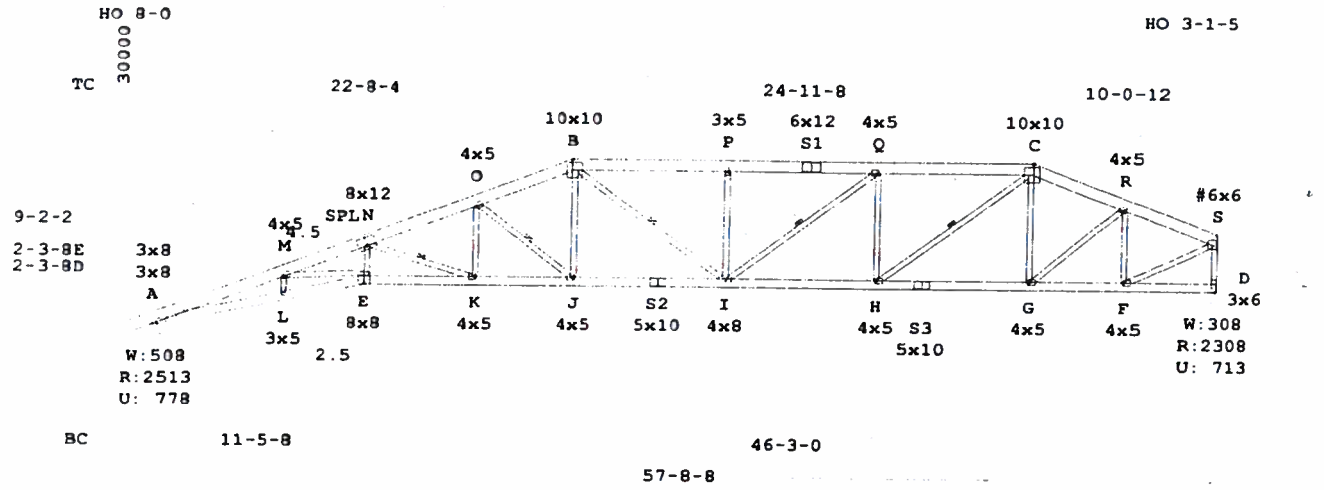
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.64
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.76
C	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1242 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R39	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 580.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S2-S3	2x 6	SP-#2
EX	S3-D	2x 6	SP-#2
WB	0.84	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on B -I
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2514	779	5- 8	2- 1
			Hz =	210
D	2308	714	3- 8	2-12
			Hz =	141

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -M	0.34	9843 C	0.11	0.23
M -N	0.37	10200 C	0.12	0.25
N -O	0.20	6757 C	0.05	0.15
O -B	0.09	5315 C	0.06	0.03
B -P	0.13	5228 C	0.03	0.10
P -S1	0.11	5228 C	0.02	0.09
S1-Q	0.09	5228 C	0.03	0.06
Q -C	0.09	4547 C	0.02	0.07
C -R	0.06	3210 C	0.03	0.03
R -S	0.05	2484 C	0.02	0.03
Bottom Chords				
A -L	0.63	9291 T	0.42	0.21
L -E	0.63	9332 T	0.42	0.21
E -K	0.48	9496 T	0.43	0.05
K -J	0.32	6362 T	0.29	0.03
J -S2	0.26	4959 T	0.22	0.04
S2-I	0.68	4959 T	0.60	0.08
I -H	0.63	4547 T	0.55	0.08
H -S3	0.42	2999 T	0.36	0.06
S3-G	0.46	2999 T	0.36	0.10
G -F	0.38	2333 T	0.28	0.10
F -D	0.07	140 T	0.00	0.07
Webs				
L -M	0.03	150 T		
M -E	0.14	453 T		
E -N	0.66	2081 T		
N -K	0.69	3340 C		1 Br
K -O	0.39	1223 T		
O -J	0.39	1784 C		1 Br
J -B	0.41	1283 T		
B -I	0.10	335 T		1 Br
I -P	0.30	554 C		
I -Q	0.27	860 T		1 Br
H -Q	0.54	993 C		
H -C	0.61	1928 T		1 Br
G -C	0.22	404 C		
G -R	0.27	868 T		
F -R	0.33	1170 C		
F -S	0.84	2646 T		
D -S	0.39	2255 C		

LL Defl -0.65" in E -K L/999
TL Defl -1.32" in E -K L/518
Shear // Grain in A -M 0.23

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

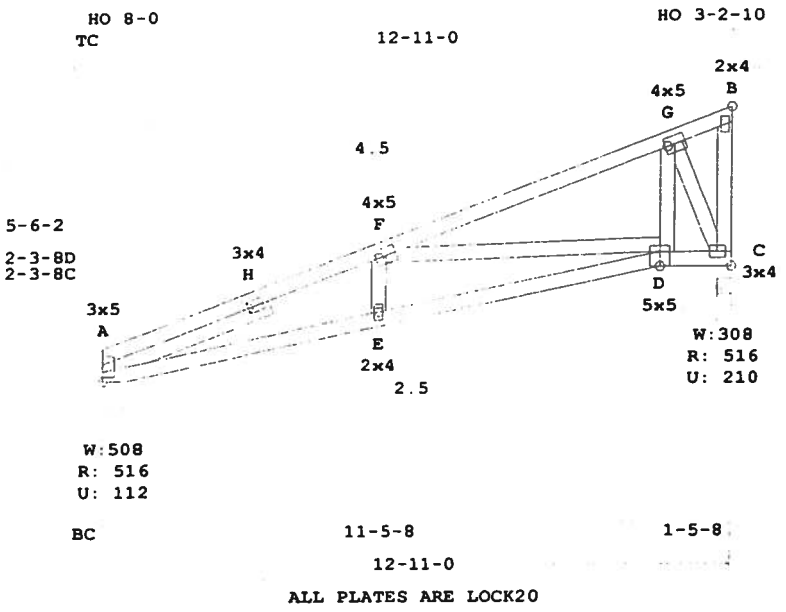
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
M	LOCK	4.0x 5.0-0.5	0.2		0.62
N	LOCK	8.0x12.0-0.6	1.5		0.97
O	LOCK	4.0x 5.0-0.5	0.2		0.60
B	LOCK	10.0x10.0-0.5	Ctr		0.73
P	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S	LOCK	6.0x 6.0	0.2	0.3	0.85
L	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr	-1.5	0.90
K	LOCK	4.0x 5.0	Ctr	Ctr	0.95
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.70
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 5.0	0.5	Ctr	0.99
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10200 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R39A*	1	SP	121100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.46	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	112	5- 8	1- 0
			Hz =	218
C	517	211	3- 8	1- 0
			Hz =	117

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -H	0.10	746 C	0.04 0.06
H -F	0.24	1242 C	0.07 0.17
F -G	0.33	285 C	0.00 0.33
G -B	0.33	54 C	0.00 0.33
--------Bottom Chords---------			
A -E	0.29	1192 T	0.19 0.10
E -D	0.36	1198 T	0.20 0.16
D -C	0.17	264 T	0.04 0.13
------------Webs------------			
E -F	0.05	202 T	
F -D	0.46	897 C	
D -G	0.08	304 T	
G -C	0.11	609 C	
C -B	0.03	112 T	
----------Sliders----------			
A -H	0.10	558 C	

LL Defl -0.04" in E -D L/999
TL Defl -0.09" in E -D L/999
Shear // Grain in F -G 0.23

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

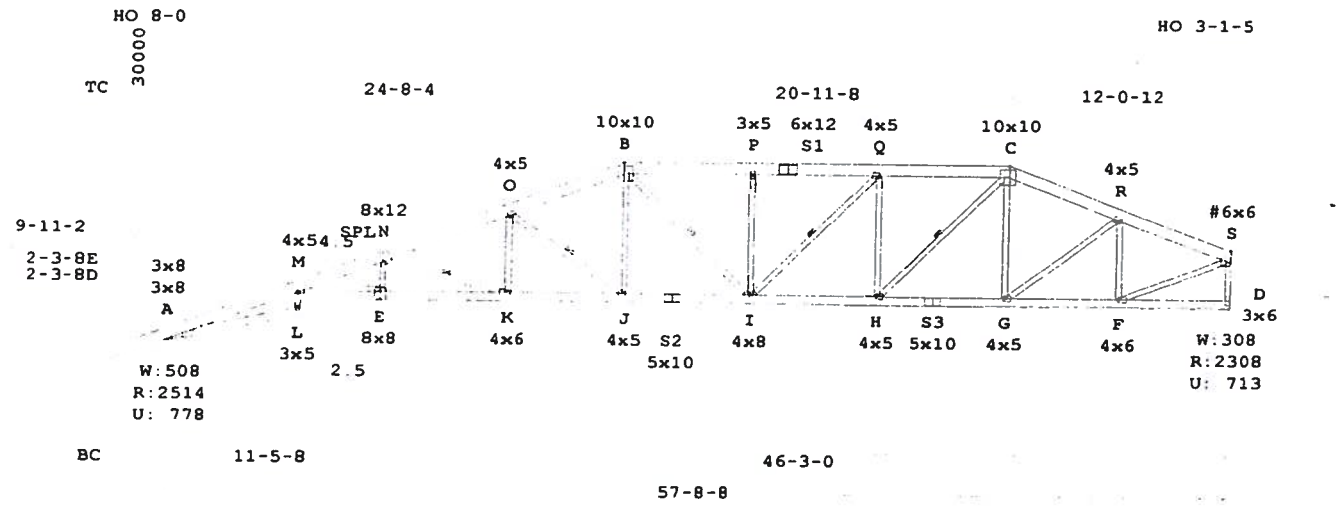
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.64
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr-1.0		0.76
C	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1242 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R40	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 593.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S2-S3	2x 6	SP-#2
EX	S3-D	2x 6	SP-#2
WB	0.89	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on B -I
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2514	779	5- 8	2- 1
			Hz =	210
D	2309	714	3- 8	2-12
			Hz =	146

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.34	9854	C	0.11	0.23
M -N	0.37	10196	C	0.12	0.25
N -O	0.22	6450	C	0.05	0.17
O -B	0.10	4884	C	0.05	0.05
B -P	0.09	4639	C	0.02	0.07
P -S1	0.08	4639	C	0.02	0.06
S1-Q	0.07	4638	C	0.05	0.02
Q -C	0.06	4130	C	0.01	0.05
C -R	0.07	3300	C	0.03	0.04
R -S	0.07	2718	C	0.03	0.04
Bottom Chords					
A -L	0.63	9302	T	0.42	0.21
L -E	0.63	9343	T	0.42	0.21
E -K	0.48	9492	T	0.43	0.05
K -J	0.30	6076	T	0.27	0.03
J -S2	0.22	4551	T	0.20	0.02
S2-I	0.61	4551	T	0.55	0.06
I -H	0.56	4130	T	0.50	0.06
H -S3	0.42	3085	T	0.37	0.05
S3-G	0.44	3084	T	0.37	0.07
G -F	0.38	2553	T	0.31	0.07
F -D	0.09	146	T	0.00	0.09
Webs					
L -M	0.03	147	T		
M -E	0.14	438	T		
E -N	0.66	2079	T		
N -K	0.85	3581	C		1 Br
K -O	0.38	1185	T		
O -J	0.49	1886	C		1 Br
J -B	0.40	1273	T		
B -I	0.04	142	T		1 Br
I -P	0.32	470	C		
I -Q	0.23	730	T		1 Br
H -Q	0.62	914	C		
H -C	0.47	1477	T		1 Br
G -C	0.18	265	C		
G -R	0.21	668	T		
F -R	0.33	1018	C		
F -S	0.89	2794	T		
D -S	0.39	2251	C		

LL Defl -0.65" in E -K L/999
TL Defl -1.32" in E -K L/516
Shear // Grain in A -M 0.23

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

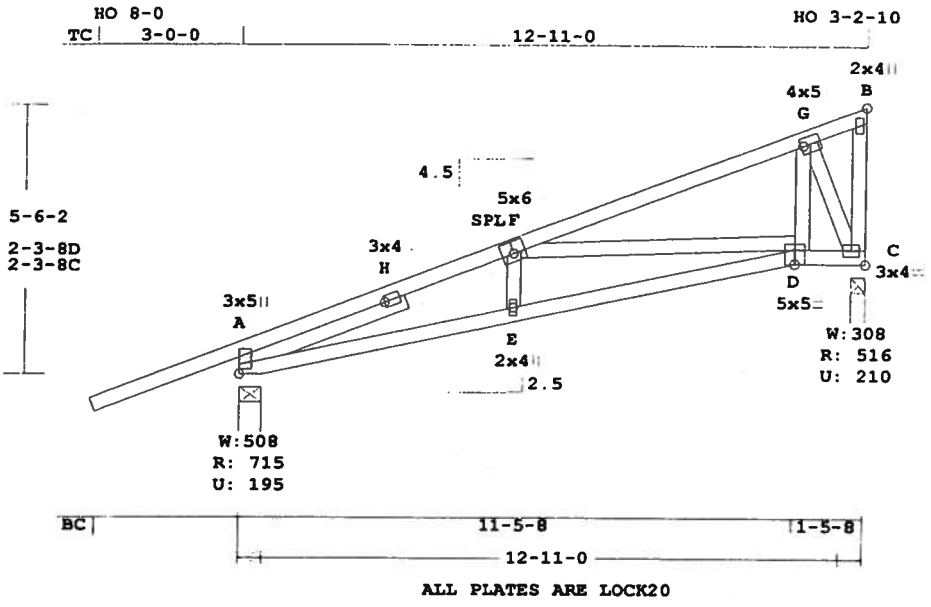
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.91
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
M	LOCK	4.0x 5.0	-0.5	0.2	0.62
N	LOCK	8.0x12.0	-0.6	1.5	0.97
O	LOCK	4.0x 5.0	-0.5	0.2	0.58
B	LOCK	10.0x10.0	-0.5	Ctr	0.73
P	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S	LOCK	6.0x 6.0	-0.2	-0.2	0.98
L	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
K	LOCK	4.0x 6.0	Ctr	Ctr	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 6.0	1.0	Ctr	0.99
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10196 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R40A	9	SP	121100	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 90.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.46	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	715	195	5- 8	1- 0
			Hz =	218
C	517	211	3- 8	1- 0
			Hz =	117

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -H	0.10	746 C	0.04 0.06
H -F	0.24	1242 C	0.07 0.17
F -G	0.33	285 C	0.00 0.33
G -B	0.33	54 C	0.00 0.33
Bottom Chords			
A -E	0.29	1192 T	0.19 0.10
E -D	0.36	1198 T	0.20 0.16
D -C	0.17	264 T	0.04 0.13
Webs			
E -F	0.05	202 T	
F -D	0.46	897 C	
D -G	0.08	304 T	
G -C	0.11	609 C	
C -B	0.03	112 T	
Sliders			
A -H	0.10	558 C	

LL Defl -0.04" in E -D L/999
TL Defl -0.09" in E -D L/999
Shear // Grain in F -G 0.23

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

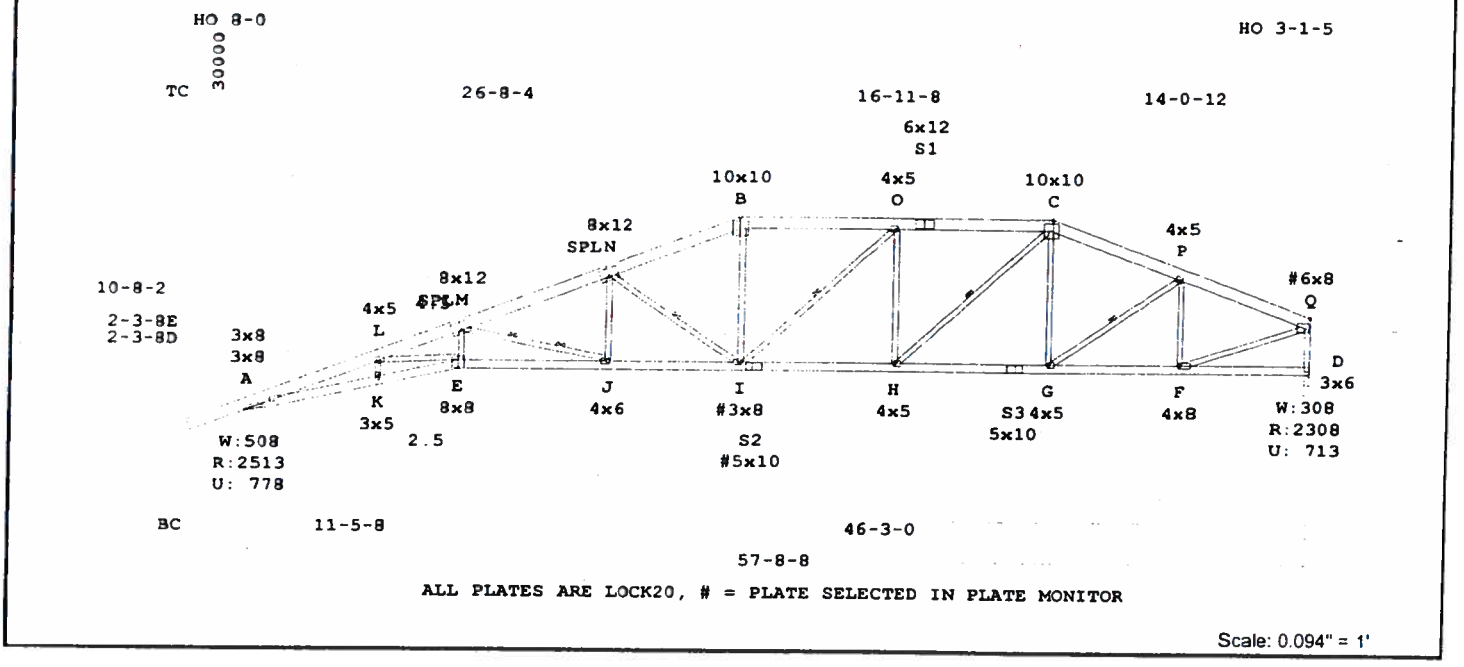
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	5.0x 6.0	-0.2	0.5	0.87
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.76
C	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1242 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R41*	1	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 563.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX S2-S3		2x 6	SP-#2
EX S3-D		2x 6	SP-#2
WB	0.93	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8
WB 2 rows CLB on M -J			
WB 1 rows CLB on N -I			
WB 1 rows CLB on I -O			
WB 1 rows CLB on H -C			
WB 1 rows CLB on G -P			

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	2514	779	5- 8	2- 1
			Hz =	210
D	2309	714	3- 8	2-12
			Hz =	160

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.34	9859	C	0.11	0.23
L -M	0.37	10190	C	0.12	0.25
M -N	0.23	6172	C	0.05	0.18
N -B	0.12	4494	C	0.02	0.10
B -O	0.11	4195	C	0.02	0.09
O -S1	0.09	4048	C	0.04	0.05
S1-C	0.08	4048	C	0.01	0.07
C -P	0.08	3335	C	0.03	0.05
P -Q	0.08	2906	C	0.03	0.05
Bottom Chords					
A -K	0.63	9307	T	0.42	0.21
K -E	0.63	9348	T	0.42	0.21
E -J	0.47	9487	T	0.43	0.04
J -I	0.31	5819	T	0.26	0.05
I -S2	0.23	4048	T	0.18	0.05
S2-H	0.56	4048	T	0.49	0.07
H -S3	0.44	3114	T	0.37	0.07
S3-G	0.47	3114	T	0.37	0.10
G -F	0.43	2733	T	0.33	0.10
F -D	0.12	159	T	0.00	0.12
Webs					
K -L	0.03	143	T		
L -E	0.13	428	T		
E -M	0.66	2081	T		
M -J	0.72	3804	C		2 Br
J -N	0.36	1142	T		
N -I	0.63	1984	C		1 Br
I -B	0.37	1172	T		
I -O	0.07	202	T		1 Br
H -O	0.58	694	C		
H -C	0.40	1266	T		1 Br
G -C	0.09	130	T		
G -P	0.15	468	T		1 Br
F -P	0.33	897	C		
F -Q	0.93	2923	T		
D -Q	0.39	2243	C		

LL Defl -0.65" in E -J L/999
TL Defl -1.34" in E -J L/511
Shear // Grain in A -L 0.23

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.91
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0-0.5	0.2		0.62
M	LOCK	8.0x12.0-0.6	1.5		0.97
N	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
C	LOCK	10.0x10.0	0.5	Ctr	0.73
P	LOCK	4.0x 5.0	0.5	0.2	0.56
Q	LOCK	6.0x 8.0-1.0	0.5		0.94
K	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr-1.5		0.89
J	LOCK	4.0x 6.0	Ctr	Ctr	0.93
I#	LOCK	3.0x 8.0-0.8	0.1		0.69
S2#	LOCK	5.0x10.0	0.9	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 8.0	1.0	Ctr	1.00
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

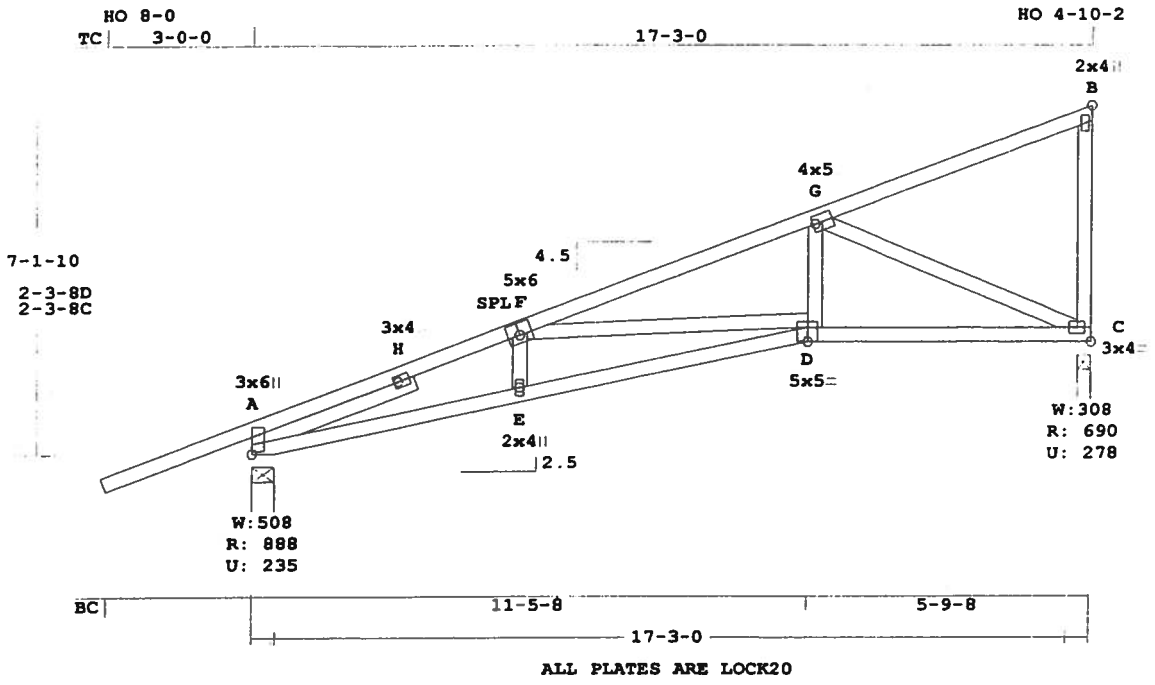
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10190 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R41A	8	SP	170300	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 116.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.42	2x 4	SP-#2
BC	0.50	2x 4	SP-#2
WB	0.64	2x 4	SP-#3
SL	0.18	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17- 3- 0
BC Cont.	0- 0- 0	17- 3- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	889	236	5- 8	1- 1
			Hz =	291
C	690	278	3- 8	1- 0
			Hz =	156

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -H	0.20	1013 C	0.00 0.20
H -F	0.22	1925 C	0.02 0.20
F -G	0.42	1214 C	0.00 0.42
G -B	0.42	72 C	0.00 0.42
Bottom Chords			
A -E	0.50	1823 T	0.30 0.20
E -D	0.47	1834 T	0.30 0.17
D -C	0.34	1132 T	0.19 0.15
Webs			
E -F	0.04	177 T	
F -D	0.34	642 C	
D -G	0.16	531 T	
G -C	0.64	1237 C	
C -B	0.04	182 T	
Sliders			
A -H	0.18	984 C	

LL Defl -0.08" in E -D L/999
TL Defl -0.18" in E -D L/999
Shear // Grain in F -G 0.26

Hz Disp	LL	DL	TL
Jt C	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

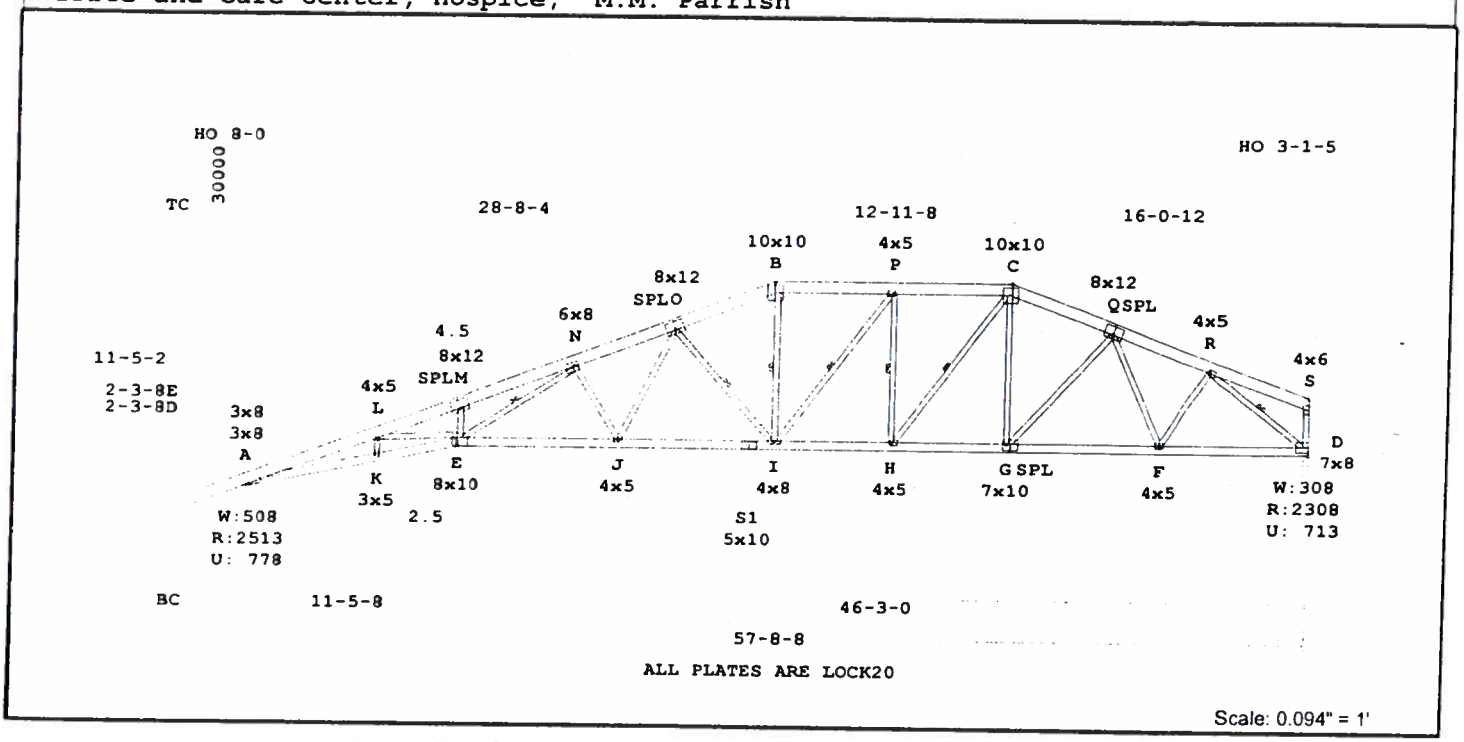
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	1.5	0.3	0.81
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	5.0x 6.0-0.2	0.5		0.87
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr-1.0		0.79
C	LOCK	3.0x 4.0	Ctr	Ctr	0.64

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1925 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R42	4	SP	570808	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 607.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.41	2x 8	SP-2400
BC	0.96	2x 6	SP-#2
EX	A -E	2x 6	SP-2400
WB	0.69	2x 4	SP-#3
EX	E -N	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on E -N
WB 1 rows CLB on O -I
WB 1 rows CLB on I -B
WB 1 rows CLB on I -P
WB 1 rows CLB on H -P
WB 1 rows CLB on H -C
WB 1 rows CLB on R -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2514	779	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	175

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.38	9578	C	0.10	0.28
L -M	0.41	10404	C	0.12	0.29
M -N	0.30	10382	C	0.13	0.17
N -O	0.13	5975	C	0.04	0.09
O -B	0.08	4096	C	0.05	0.03
B -P	0.07	3843	C	0.01	0.06
P -C	0.06	3667	C	0.01	0.05
C -Q	0.07	3310	C	0.04	0.03
Q -R	0.05	2976	C	0.01	0.04
R -S	0.05	75	T	0.00	0.05
Bottom Chords					
A -K	0.72	9027	T	0.41	0.31
K -E	0.72	9103	T	0.41	0.31
E -J	0.96	6300	T	0.76	0.20
J -S1	0.74	4843	T	0.58	0.16
S1-I	0.64	4843	T	0.58	0.06
I -H	0.50	3665	T	0.44	0.06
H -G	0.45	3094	T	0.37	0.08
G -F	0.44	3014	T	0.36	0.08
F -D	0.38	2366	T	0.28	0.10
Webs					
K -L	0.05	276	C		
L -E	0.28	901	T		
E -M	0.06	265	C		
E -N	0.77	4196	T		1 Br
N -J	0.57	1545	C		
J -O	0.56	1745	T		
O -I	0.46	1536	C		1 Br
I -B	0.34	1068	T		1 Br
I -P	0.09	297	T		1 Br
H -P	0.19	645	C		1 Br
H -C	0.30	949	T		1 Br
G -C	0.05	190	T		
G -Q	0.04	151	T		
Q -F	0.41	595	C		
F -R	0.26	838	T		
R -D	0.69	3150	C		1 Br
D -S	0.04	192	T		

LL Defl -0.66" in E -J L/999
TL Defl -1.36" in E -J L/502
Shear // Grain in A -L 0.23

Hz Disp	LL	DL	TL
Jt D	0.26"	0.27"	0.53"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

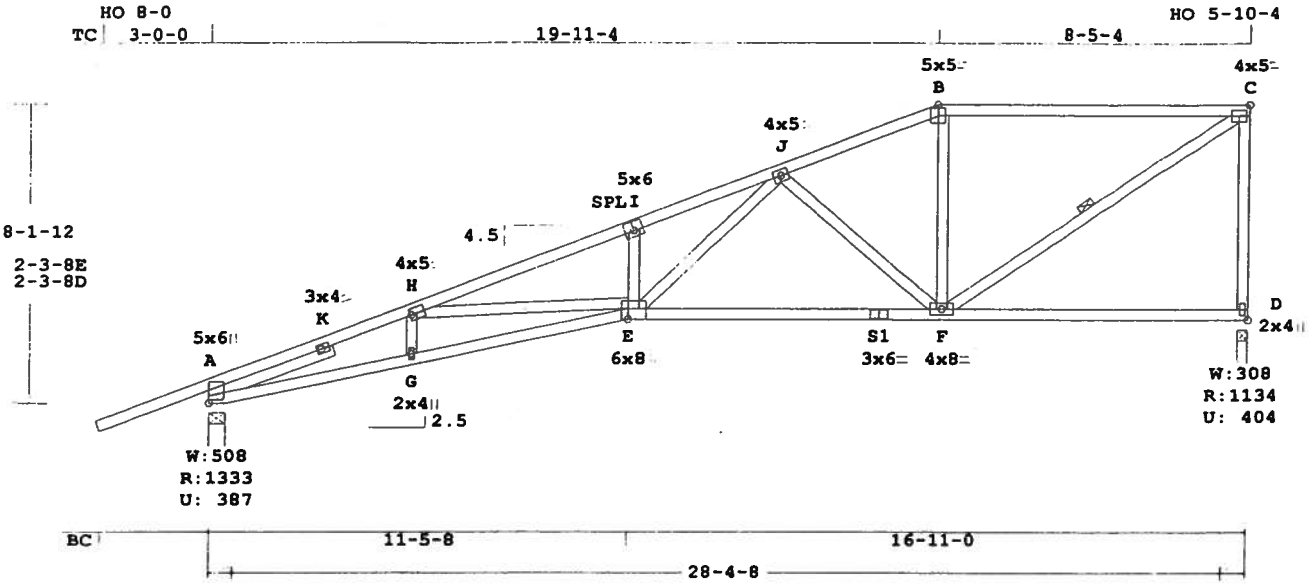
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.88
A	LOCK	3.0x 8.0	Ctr	Ctr	0.76
L	LOCK	4.0x 5.0-0.5	0.2	0.62	
M	LOCK	8.0x12.0-0.6	1.5	0.97	
N	LOCK	6.0x 8.0	Ctr	Ctr	0.89
O	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.94
S	LOCK	4.0x 6.0-0.2	Ctr		1.00
K	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x10.0	Ctr-1.5		0.94
J	LOCK	4.0x 5.0	Ctr	Ctr	0.76
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	7.0x10.0	Ctr-0.8		0.97
F	LOCK	4.0x 5.0	Ctr	Ctr	0.58
D	LOCK	7.0x 8.0-0.5	Ctr		0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10404 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineerin
060039	R42A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 199.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.91	2x 4	SP-#2
BC	0.71	2x 4	SP-#2
EX A -E		2x 4	SP-2400
WB	0.59	2x 4	SP-#3
SL	0.38	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 4- 8
BC	Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1333	387	5- 8	1- 2
			Hz =	337
D	1135	405	3- 8	1- 5
			Hz =	182

Membr	CSI	P Lbs	Ax	CSI	Bnd
----------Top Chords----------					
A -K	0.58	1753	C	0.01	0.57
K -H	0.66	3658	C	0.09	0.57
H -I	0.86	3535	C	0.20	0.66
I -J	0.42	3561	C	0.09	0.33
J -B	0.39	1303	C	0.01	0.38
B -C	0.91	1235	C	0.04	0.87
--------Bottom Chords---------					
A -G	0.55	3426	T	0.24	0.31
G -E	0.41	3453	T	0.24	0.17
E -S1	0.53	1963	T	0.32	0.21
S1-F	0.71	1963	T	0.20	0.51
F -D	0.51	181	T	0.00	0.51
----------Webs----------					
G -H	0.03	130	T		
H -E	0.03	129	T		
E -I	0.07	387	C		
E -J	0.59	1854	T		
J -F	0.50	970	C		
F -B	0.05	198	T		
F -C	0.47	1495	T		1 Br
D -C	0.49	1071	C		
----------Sliders----------					
A -K	0.38	2002	C		

LL Defl -0.26" in G -E L/999
TL Defl -0.55" in E -S1 L/597
Shear // Grain in B -C 0.34

Hz Disp	LL	DL	TL
Jt D	0.09"	0.09"	0.18"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

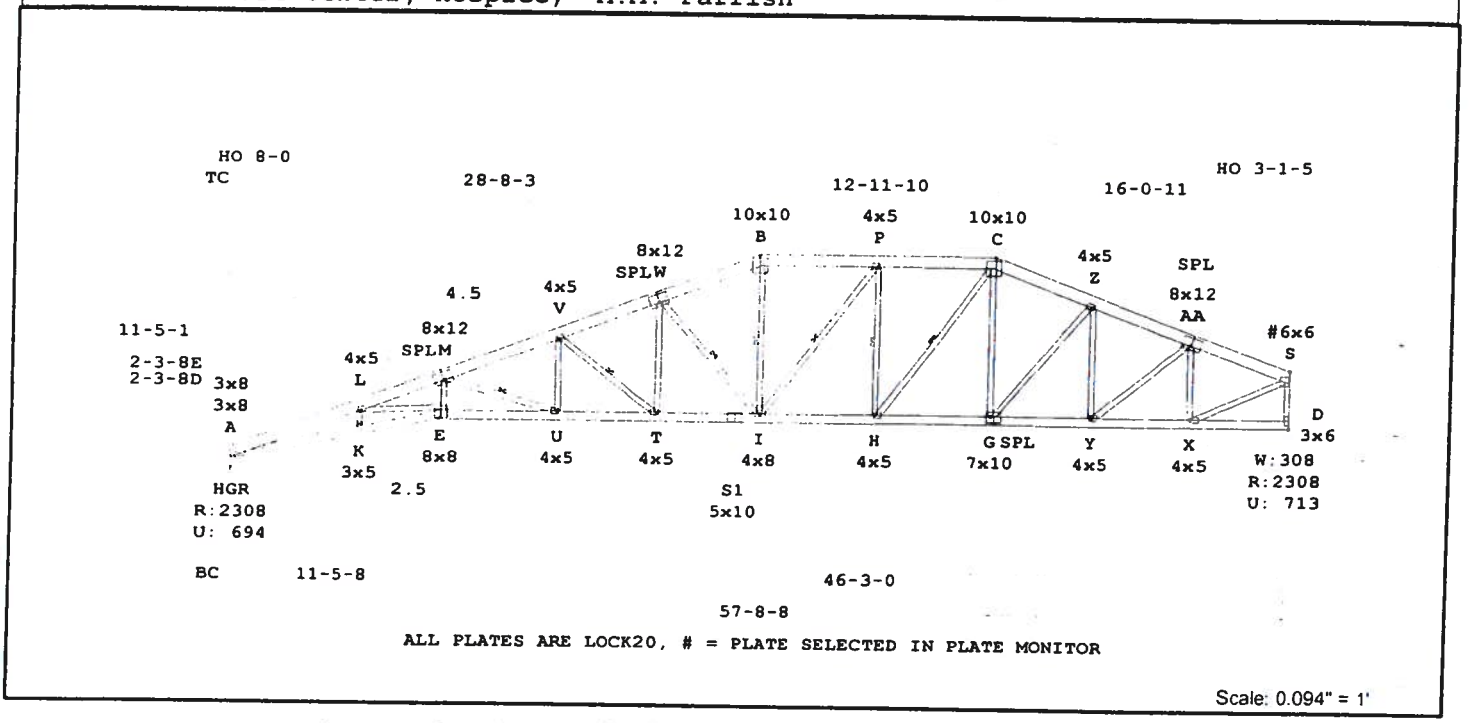
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 6.0	2.5	0.7	0.94
K	LOCK	3.0x 4.0	Ctr	Ctr	0.74
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
I	LOCK	5.0x 6.0	-0.2	0.5	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.83
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
C	LOCK	4.0x 5.0	Ctr	Ctr	0.61
G	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	6.0x 8.0	Ctr	-0.5	0.76
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 8.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.46

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 336 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3658 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R43	4	SP	570808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 611.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S1-G	2x 6	SP-#2
EX	G -D	2x 6	SP-#2
WB	0.86	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	57- 8- 8
BC Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on M -U
WB 1 rows CLB on V -T
WB 1 rows CLB on W -I
WB 1 rows CLB on I -B
WB 1 rows CLB on I -P
WB 1 rows CLB on H -P
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	2308	695	3- 8	1-15
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	175

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.34	9851 C	0.11		0.23
L -M	0.37	10188 C	0.12		0.25
M -V	0.21	6670 C	0.05		0.16
V -W	0.09	5195 C	0.06		0.03
W -B	0.08	4099 C	0.05		0.03
B -P	0.07	3843 C	0.01		0.06
P -C	0.06	3669 C	0.01		0.05
C -Z	0.07	3306 C	0.04		0.03
Z -AA	0.05	3232 C	0.03		0.02
AA-S	0.05	2561 C	0.02		0.03
Bottom Chords					
A -K	0.63	9299 T	0.42		0.21
K -E	0.63	9341 T	0.42		0.21
E -U	0.48	9483 T	0.43		0.05
U -T	0.31	6287 T	0.28		0.03
T -S1	0.23	4851 T	0.22		0.01
S1-I	0.64	4851 T	0.58		0.06
I -H	0.50	3669 T	0.44		0.06
H -G	0.42	3092 T	0.37		0.05
G -Y	0.41	3027 T	0.36		0.05
Y -X	0.33	2402 T	0.29		0.04
X -D	0.07	175 T	0.00		0.07
Webs					
K -L	0.03	149 T			
L -E	0.13	432 T			
E -M	0.66	2079 T			
M -U	0.73	3387 C			1 Br
U -V	0.38	1210 T			
V -T	0.42	1864 C			1 Br
T -W	0.41	1294 T			
W -I	0.46	1552 C			1 Br
I -B	0.34	1077 T			1 Br
I -P	0.09	294 T			1 Br
H -P	0.19	643 C			1 Br
H -C	0.30	957 T			1 Br
G -C	0.04	174 T			
G -Z	0.05	137 T			
Y -Z	0.25	431 C			
Y -AA	0.26	820 T			
X -AA	0.33	1096 C			
X -S	0.86	2688 T			
D -S	0.39	2257 C			

LL Defl -0.63" in E -U L/999
TL Defl -1.27" in E -U L/539
Shear // Grain in A -L 0.23

Hz Disp	LL	DL	TL
Jt D	0.26"	0.26"	0.52"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.91
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0	-0.5	0.2	0.62
M	LOCK	8.0x12.0	-0.6	1.5	0.97
V	LOCK	4.0x 5.0	-0.5	0.2	0.60
W	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
Z	LOCK	4.0x 5.0	0.5	0.2	0.56
AA	LOCK	8.0x12.0	0.6	1.5	0.97
S#	LOCK	6.0x 6.0	0.1	0.1	0.88
K	LOCK	3.0x 5.0	Ctr	Ctr	0.39
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
U	LOCK	4.0x 5.0	Ctr	Ctr	0.97
T	LOCK	4.0x 5.0	Ctr	Ctr	0.61
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	7.0x10.0	Ctr	-0.8	0.97
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.60
X	LOCK	4.0x 5.0	1.0	Ctr	1.00
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

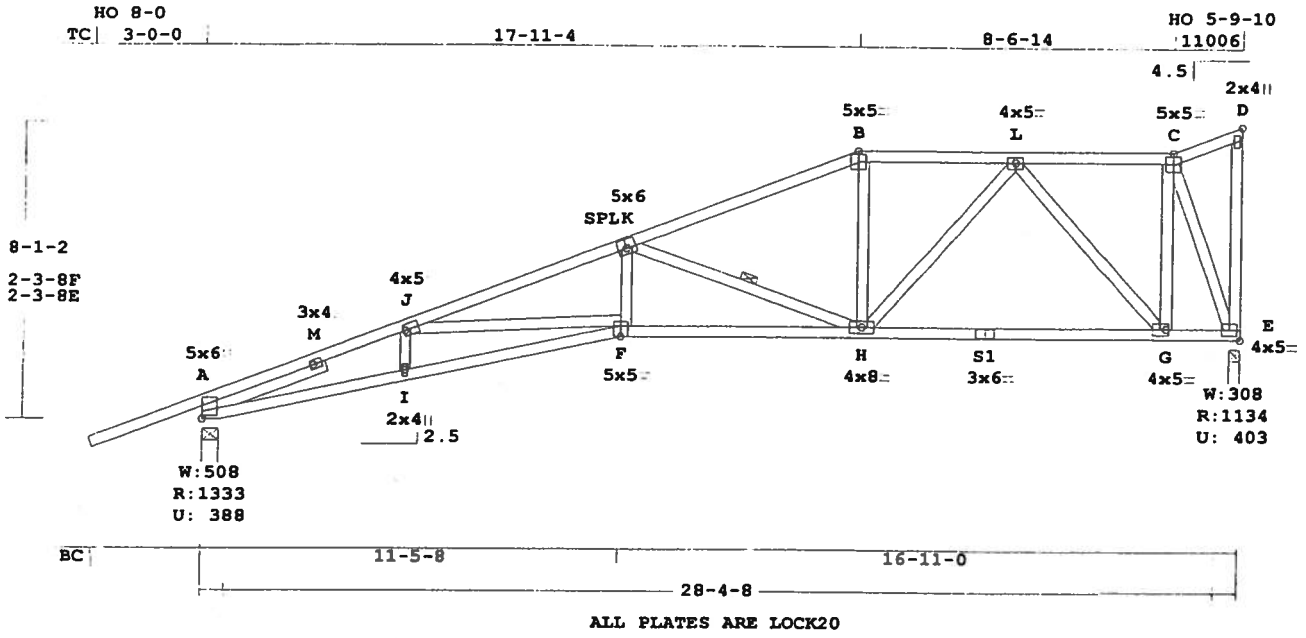
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10188 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R43A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 211.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.80	2x 4	SP-#2
BC	0.73	2x 4	SP-#2
EX A -F		2x 4	SP-2400
WB	0.59	2x 4	SP-#3
SL	0.39	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on K -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1333	388	5- 8	1- 2
			Hz =	334
E	1135	404	3- 8	1- 5
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.60	1725	C	0.01	0.59
M -J	0.68	3697	C	0.09	0.59
J -K	0.80	3489	C	0.19	0.61
K -B	0.44	1662	C	0.01	0.43
B -L	0.22	1549	C	0.10	0.12
L -C	0.14	437	C	0.00	0.14
C -D	0.05	31	C	0.00	0.05
Bottom Chords					
A -I	0.53	3463	T	0.24	0.29
I -F	0.40	3482	T	0.25	0.15
F -H	0.73	3257	T	0.54	0.19
H -S1	0.44	1103	T	0.11	0.33
S1-G	0.39	1103	T	0.11	0.28
G -E	0.32	419	T	0.04	0.28
Webs					
I -J	0.04	152	T		
J -F	0.06	134	T		
F -K	0.27	844	T		
K -H	0.42	1824	C		1 Br
H -B	0.09	341	T		
H -L	0.21	683	T		
L -G	0.59	1020	C		
G -C	0.29	918	T		
C -E	0.45	1166	C		
E -D	0.03	82	T		
Sliders					
A -M	0.39	2070	C		

LL Defl -0.26" in I -F L/999
TL Defl -0.55" in I -F L/599
Shear // Grain in M -J 0.30

Hz Disp	LL	DL	TL
Jt E	0.10"	0.10"	0.20"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

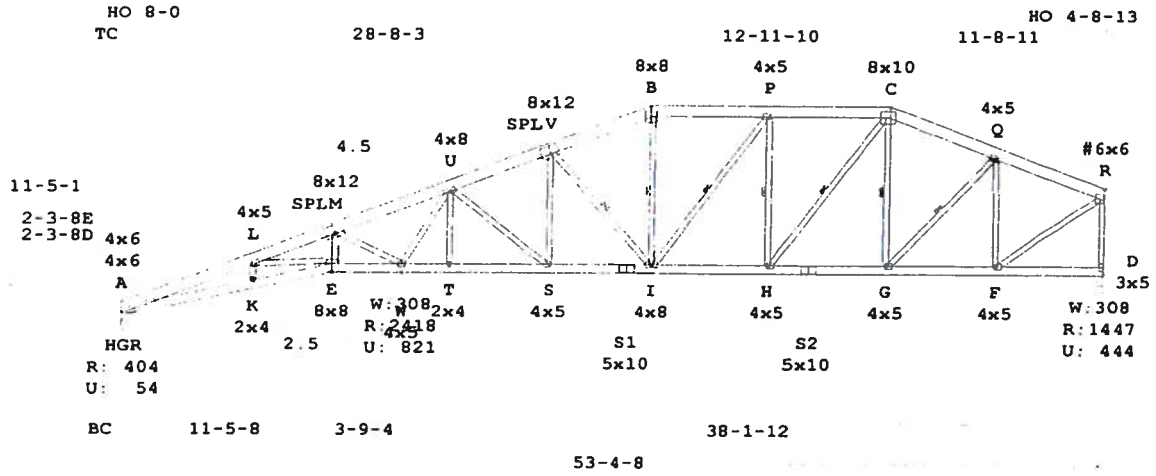
Jt Type	Plt	Size	X	Y	JSI
A	LOCK	5.0x 6.0	2.5	0.7	0.95
M	LOCK	3.0x 4.0	Ctr	Ctr	0.77
J	LOCK	4.0x 5.0	Ctr	Ctr	0.64
K	LOCK	5.0x 6.0-0.2	0.5		0.87
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
L	LOCK	4.0x 5.0	Ctr	Ctr	0.49
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
I	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	5.0x 5.0	Ctr-1.0		0.98
H	LOCK	4.0x 8.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3697 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R44*	2	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 571.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 8	SP-2400
BC	0.23	2x 6	SP-#2
WB	0.77	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 4- 8
BC Cont.		0- 0- 0	53- 4- 8

WB 1 rows CLB on V -I
WB 1 rows CLB on I -B
WB 1 rows CLB on I -P
WB 1 rows CLB on H -P
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
WB 1 rows CLB on G -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	404	54	3- 8	1- 0
		Hz =	284	
W	2419	821	3- 8	2- 9
D	1447	445	3- 8	1-11
		Hz =	183	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.08	583 C	0.00		0.08
L -M	0.09	286 T	0.01		0.08
M -U	0.10	916 T	0.02		0.08
U -V	0.05	1187 C	0.00		0.05
V -B	0.04	1406 C	0.01		0.03
B -P	0.05	1313 C	0.00		0.05
P -C	0.05	1527 C	0.00		0.05
C -Q	0.05	1425 C	0.01		0.04
Q -R	0.05	1181 C	0.01		0.04
Bottom Chords					
A -K	0.17	574 T	0.07		0.10
K -E	0.17	577 T	0.07		0.10
E -W	0.15	252 C	0.00		0.15
W -T	0.17	188 T	0.02		0.15
T -S	0.09	188 T	0.01		0.08
S -S1	0.18	1105 T	0.13		0.05
S1-I	0.17	1105 T	0.13		0.04
I -H	0.23	1527 T	0.18		0.05
H -S2	0.21	1327 T	0.16		0.05
S2-G	0.21	1327 T	0.16		0.05
G -F	0.18	1116 T	0.13		0.05
F -D	0.09	182 T	0.00		0.09
Webs					
K -L	0.05	187 T			
L -E	0.25	805 C			
E -M	0.06	232 T			
M -W	0.20	712 C			
W -U	0.77	2125 C			
T -U	0.02	101 T			
U -S	0.38	1190 T			
S -V	0.37	635 C			
V -I	0.10	320 T			1 Br
I -B	0.06	251 T			1 Br
I -P	0.15	361 C			1 Br
H -P	0.04	197 T			1 Br
H -C	0.10	332 T			1 Br
G -C	0.02	107 T			1 Br
G -Q	0.09	305 T			1 Br
F -Q	0.39	702 C			
F -R	0.44	1389 T			
D -R	0.37	1395 C			

LL Defl -0.03" in A -K L/999
TL Defl -0.06" in A -K L/999
Shear // Grain in B -P 0.12

Hz Disp	LL	DL	TL
Jt D	0.02"	0.02"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

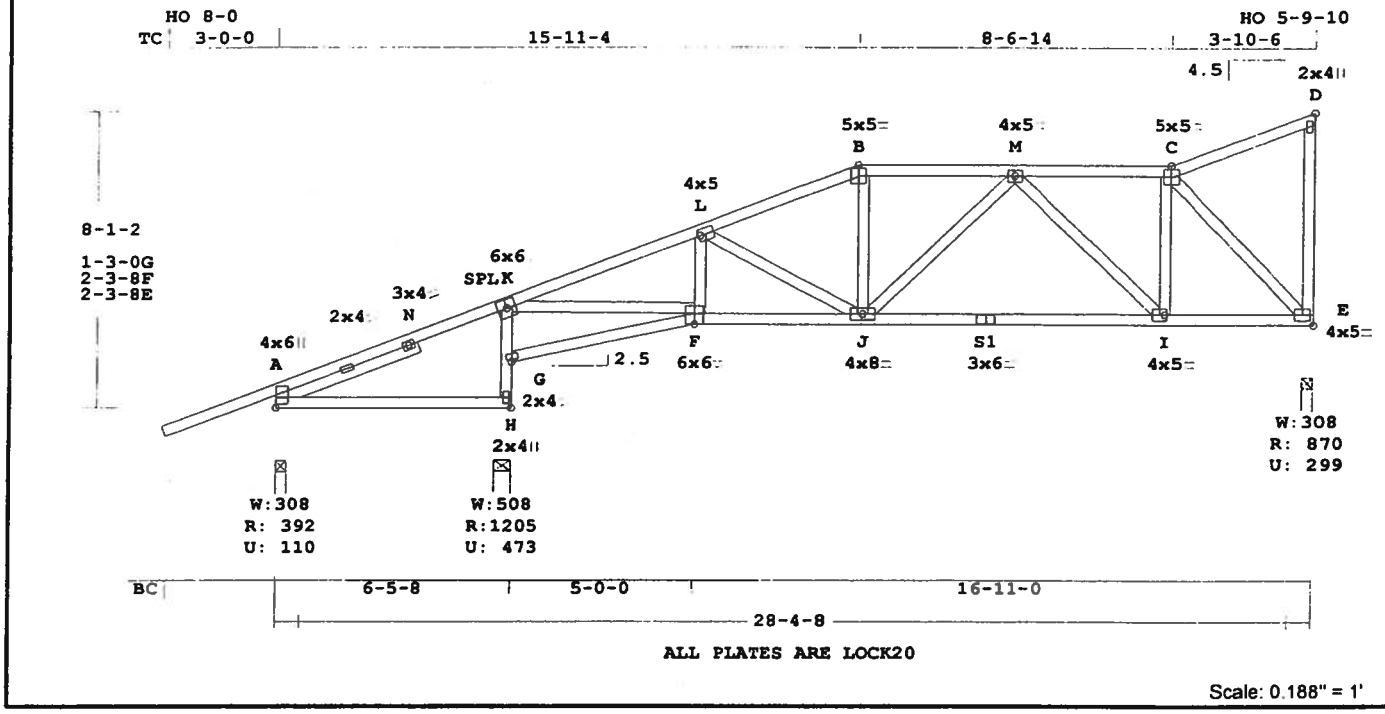
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.71
A	LOCK	4.0x 6.0	Ctr	Ctr	0.18
L	LOCK	4.0x 5.0-0.5	0.2		0.62
M	LOCK	8.0x12.0-0.6	1.5		0.97
U	LOCK	4.0x 8.0-0.5	0.2		0.69
V	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	8.0x 8.0	Ctr	Ctr	0.87
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	8.0x10.0	Ctr	Ctr	0.87
Q	LOCK	4.0x 5.0	0.5	0.2	0.56
R	LOCK	6.0x 6.0-0.1	0.2		0.82
K	LOCK	2.0x 4.0	Ctr	Ctr	0.56
E	LOCK	8.0x 8.0	Ctr-1.5		0.83
W	LOCK	4.0x 5.0	Ctr	Ctr	0.53
T	LOCK	2.0x 4.0	Ctr	Ctr	0.56
S	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2125 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R44A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 206.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 4	SP-#2
BC	0.49	2x 4	SP-#2
CW	0.26	2x 4	SP-#3
WB	0.45	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	393	110	3- 8	1- 0
			Hz =	334
H	1206	474	5- 8	1- 5
E	870	299	3- 8	1- 0
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.28	322	C	0.00	0.28
N -K	0.47	403	C	0.02	0.45
K -L	0.46	1518	C	0.01	0.45
L -B	0.21	1219	C	0.06	0.15
B -M	0.14	1142	C	0.00	0.14
M -C	0.18	720	C	0.00	0.18
C -D	0.19	62	C	0.00	0.19
Bottom Chords					
A -H	0.19	66	C	0.00	0.19
G -F	0.31	48	C	0.00	0.31
F -J	0.49	1410	T	0.23	0.26
J -S1	0.36	1058	T	0.10	0.26
S1-I	0.40	1058	T	0.10	0.30
I -E	0.37	708	T	0.07	0.30
Chord-Webs					
H -G	0.26	1146	C	0.01	0.25
G -K	0.17	1034	C	0.17	0.00
Webs					
K -F	0.44	1397	T		
F -L	0.02	89	T		
L -J	0.11	304	C		
J -B	0.06	228	T		
J -M	0.04	168	T		
M -I	0.23	473	C		
I -C	0.15	485	T		
C -E	0.45	1017	C		
E -D	0.05	161	T		
Sliders					
A -N	0.11	408	T		

LL Defl -0.03" in A -H L/999
TL Defl -0.08" in A -H L/847
Shear // Grain in N -K 0.28

Hz Disp	LL	DL	TL
Jt E	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

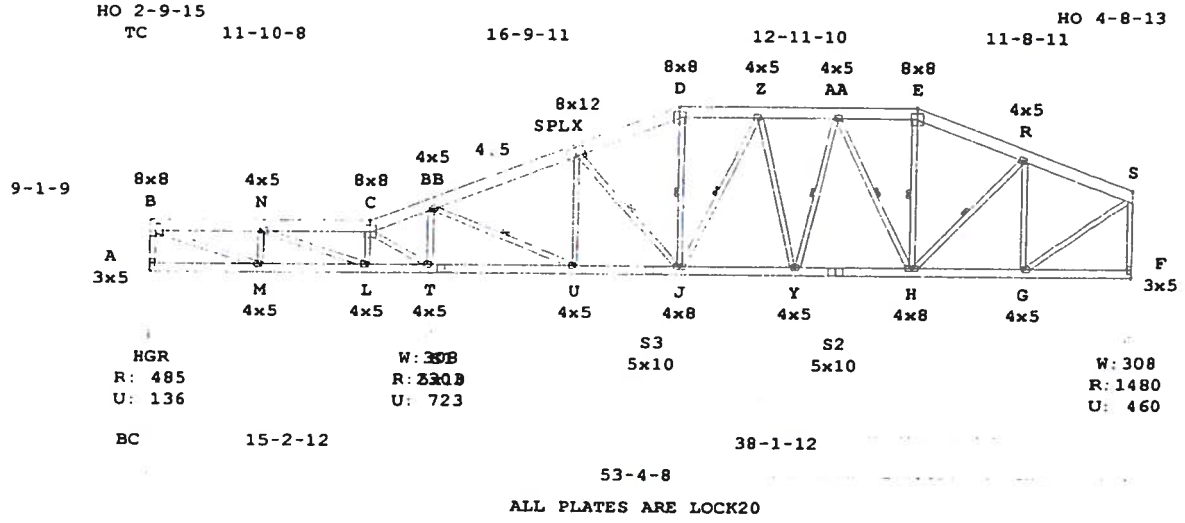
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	6.0x 6.0	Ctr	Ctr	0.87
L	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	4.0x 5.0	Ctr	Ctr	0.47
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	2.0x 4.0	Ctr	Ctr	0.58
G	LOCK	2.0x 4.0	Ctr	Ctr	0.63
F	LOCK	6.0x 6.0	Ctr-0.5		0.65
J	LOCK	4.0x 8.0	Ctr	Ctr	0.54
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1518 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*R45*	1	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 590.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.11	2x 8	SP-2400
BC	0.23	2x 6	SP-#2
WB	0.57	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 4- 8
BC Cont.		0- 0- 0	53- 4- 8

WB 1 rows CLB on BB-U
WB 1 rows CLB on X -J
WB 1 rows CLB on J -D
WB 1 rows CLB on J -Z
WB 1 rows CLB on Y -AA
WB 1 rows CLB on AA-H
WB 1 rows CLB on H -E
WB 1 rows CLB on H -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	486	136	3- 8	1- 0
			Hz =	-158
T	2304	723	3- 8	2- 7
F	1480	461	3- 8	1-12
			Hz =	111

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
B -N	0.05	672	C	0.00	0.05
N -C	0.04	93	C	0.00	0.04
C -BB	0.11	465	T	0.01	0.10
BB-X	0.10	1350	C	0.00	0.10
X -D	0.06	1511	C	0.02	0.04
D -Z	0.03	1407	C	0.02	0.01
Z -AA	0.02	1563	C	0.00	0.02
AA-E	0.03	1372	C	0.02	0.01
E -R	0.05	1474	C	0.01	0.04
R -S	0.05	1212	C	0.01	0.04
Bottom Chords					
A -M	0.09	127	C	0.00	0.09
M -L	0.13	672	T	0.08	0.05
L -T	0.17	80	T	0.00	0.17
T -S1	0.17	389	C	0.00	0.17
S1-U	0.10	389	C	0.00	0.10
U -S3	0.19	1254	T	0.09	0.10
S3-J	0.20	1254	T	0.15	0.05
J -Y	0.23	1554	T	0.18	0.05
Y -S2	0.22	1542	T	0.18	0.04
S2-H	0.23	1542	T	0.18	0.05
H -G	0.18	1144	T	0.13	0.05
G -F	0.09	110	T	0.00	0.09
Webs					
A -B	0.07	430	C	WindLd	
B -M	0.23	723	T		
M -N	0.04	161	T		
N -L	0.33	622	C		
L -C	0.08	294	T		
C -T	0.14	542	C		
T -BB	0.39	1860	C		
BB-U	0.57	1803	T		1 Br
U -X	0.35	622	C		
X -J	0.07	229	T		1 Br
J -D	0.07	285	T		1 Br
J -Z	0.11	332	C		1 Br
Z -Y	0.03	119	T		
Y -AA	0.03	145	T		1 Br
AA-H	0.13	383	C		1 Br
H -E	0.07	279	T		1 Br
H -R	0.10	333	T		1 Br
G -R	0.40	723	C		
G -S	0.45	1424	T		
F -S	0.38	1429	C		

LL Defl -0.05" in Y -S2 L/999
TL Defl -0.11" in J -Y L/999
Shear // Grain in BB-X 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

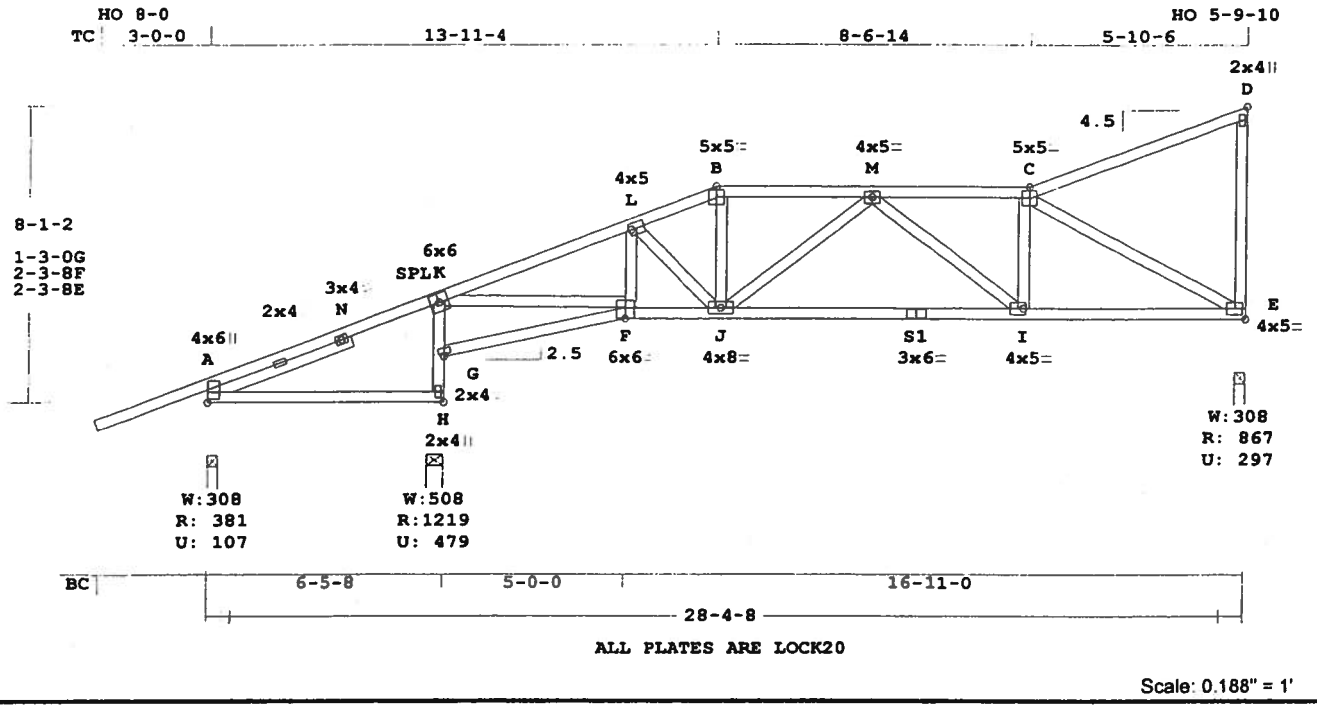
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	8.0x 8.0	0.5	Ctr	0.82
N	LOCK	4.0x 5.0	Ctr	0.5	0.68
C	LOCK	8.0x 8.0	Ctr	Ctr	0.87
BB	LOCK	4.0x 5.0	-0.5	0.2	0.83
X	LOCK	8.0x12.0	-0.6	1.5	0.97
D	LOCK	8.0x 8.0	Ctr	Ctr	0.87
Z	LOCK	4.0x 5.0	Ctr	0.5	0.62
AA	LOCK	4.0x 5.0	Ctr	0.5	0.62
E	LOCK	8.0x 8.0	Ctr	Ctr	0.87
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S					3.11
A	LOCK	3.0x 5.0	Ctr	Ctr	0.94
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
U	LOCK	4.0x 5.0	Ctr	Ctr	0.75
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 8.0	Ctr	Ctr	0.57
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.62
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 8.0	Ctr	Ctr	0.57
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1860 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R45A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 200.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.50	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
CW	0.35	2x 4	SP-#3
WB	0.86	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	382	108	3- 8	1- 0
			Hz =	334
H	1219	479	5- 8	1- 6
E	867	298	3- 8	1- 0
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -N	0.27	317	C	0.00	0.27
N -K	0.50	442	C	0.02	0.48
K -L	0.49	1487	C	0.01	0.48
L -B	0.13	1346	C	0.07	0.06
B -M	0.17	1267	C	0.01	0.16
M -C	0.37	1191	C	0.01	0.36
C -D	0.43	91	C	0.00	0.43
--------Bottom Chords---------					
A -H	0.19	79	C	0.00	0.19
G -F	0.31	34	C	0.00	0.31
F -J	0.52	1377	T	0.23	0.29
J -S1	0.39	1365	T	0.13	0.26
S1-I	0.46	1365	T	0.13	0.33
I -E	0.45	1182	T	0.12	0.33
----------Chord-Webs----------					
H -G	0.35	1160	C	0.03	0.32
G -K	0.17	1044	C	0.17	0.00
------------Webs-------------					
K -F	0.44	1399	T		
F -L	0.01	40	T		
L -J	0.04	190	T		
J -B	0.08	263	T		
J -M	0.05	125	C		
M -I	0.09	223	C		
I -C	0.09	373	T		
C -E	0.86	1346	C		
E -D	0.08	236	T		
----------Sliders------------					
A -N	0.11	418	T		

LL Defl -0.03" in A -H L/999
TL Defl -0.08" in A -H L/853
Shear // Grain in N -K 0.28

Hz Disp	LL	DL	TL
Jt E	0.03"	0.03"	0.07"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

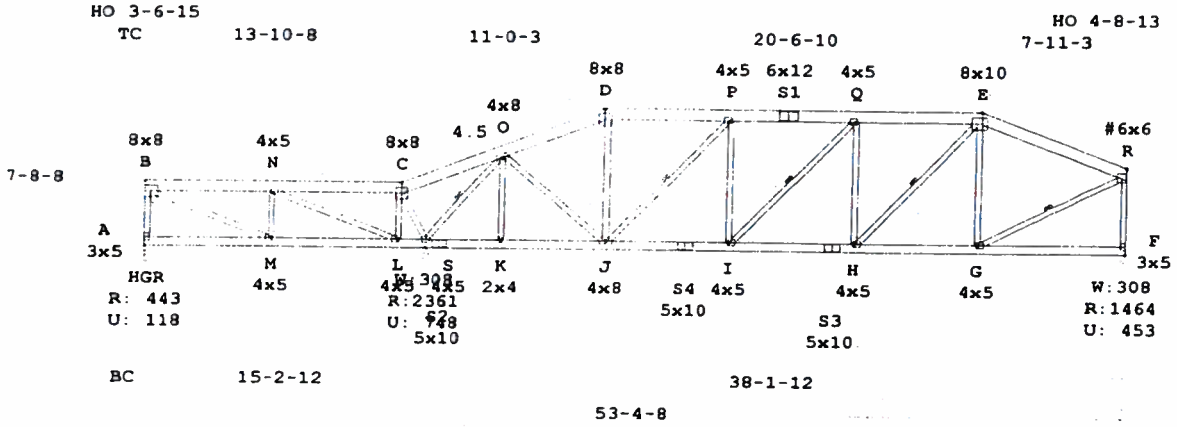
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	6.0x 6.0	Ctr	Ctr	0.87
L	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	4.0x 5.0	Ctr	Ctr	0.48
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	2.0x 4.0	Ctr	Ctr	0.58
G	LOCK	2.0x 4.0	Ctr	Ctr	0.64
F	LOCK	6.0x 6.0	Ctr	-0.5	0.65
J	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1487 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
060039	R46	1	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 564.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 8	SP-2400
BC	0.29	2x 6	SP-#2
WB	0.75	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	53- 4- 8
BC Cont.	0- 0- 0	53- 4- 8

WB 1 rows CLB on S -O
WB 1 rows CLB on J -P
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -E
WB 1 rows CLB on G -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	444	119	3- 8	1- 0
			Hz =	-147
S	2361	749	3- 8	2- 8
F	1465	453	3- 8	1-12
			Hz =	88

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
B -N	0.06	452 C	0.00	0.06
N -C	0.07	450 T	0.01	0.06
C -O	0.06	770 T	0.02	0.04
O -D	0.04	1390 C	0.00	0.04
D -P	0.06	1294 C	0.00	0.06
P -S1	0.06	1819 C	0.00	0.06
S1-Q	0.04	1819 C	0.00	0.04
Q -E	0.04	1798 C	0.00	0.04
E -R	0.12	1358 C	0.00	0.12
Bottom Chords				
A -M	0.13	102 T	0.00	0.13
M -L	0.16	452 T	0.03	0.13
L -S	0.15	464 C	0.00	0.15
S -S2	0.23	686 T	0.08	0.15
S2-K	0.16	686 T	0.08	0.08
K -J	0.16	686 T	0.05	0.11
J -S4	0.29	1819 T	0.22	0.07
S4-I	0.27	1819 T	0.22	0.05
I -S3	0.26	1798 T	0.21	0.05
S3-H	0.24	1798 T	0.21	0.03
H -G	0.27	1278 T	0.09	0.18
G -F	0.18	88 T	0.00	0.18
Webs				
A -B	0.08	382 C	WindLd	
B -M	0.15	495 T		
M -N	0.04	153 T		
N -L	0.75	989 C		
L -C	0.13	420 T		
C -S	0.10	541 C		
S -O	0.46	2207 C		1 Br
K -O	0.03	147 T		
O -J	0.26	818 T		
J -D	0.06	250 T		
J -P	0.28	765 C		1 Br
I -P	0.06	256 T		
I -Q	0.01	40 T		1 Br
H -Q	0.29	413 C		
H -E	0.23	746 T		1 Br
G -E	0.35	505 C		
G -R	0.46	1441 T		1 Br
F -R	0.37	1397 C		

LL Defl -0.06" in I -S3 L/999
TL Defl -0.14" in I -S3 L/999
Shear // Grain in B -N 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

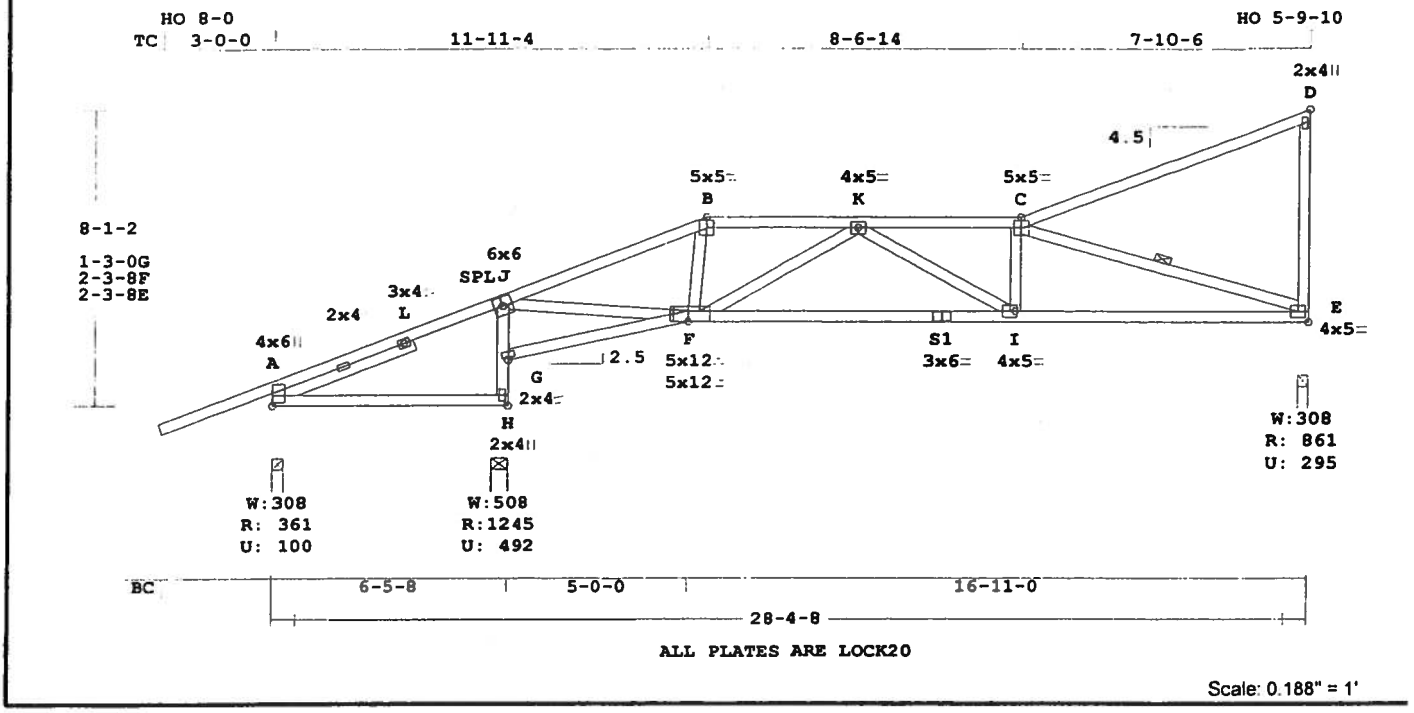
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	8.0x 8.0	0.5	Ctr	0.82
N	LOCK	4.0x 5.0	Ctr	0.5	0.65
C	LOCK	8.0x 8.0	Ctr	Ctr	0.87
O	LOCK	4.0x 8.0	-0.5	0.2	0.64
D	LOCK	8.0x 8.0	Ctr	Ctr	0.87
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
E	LOCK	8.0x10.0	Ctr	Ctr	0.87
R	LOCK	6.0x 6.0	Ctr	0.5	0.76
A	LOCK	3.0x 5.0	Ctr	Ctr	0.94
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S	LOCK	4.0x 5.0	Ctr	Ctr	0.55
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	2.0x 4.0	Ctr	Ctr	0.56
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S4	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2207 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R46A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 190.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.74	2x 4	SP-#2
BC	0.65	2x 4	SP-#2
CW	0.54	2x 4	SP-#3
WB	0.55	2x 4	SP-#3
SL	0.12	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on C -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	362	101	3- 8	1- 0
			Hz =	334
H	1245	493	5- 8	1- 6
E	862	295	3- 8	1- 0
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.27	309	C	0.00	0.27
L -J	0.53	434	C	0.03	0.50
J -B	0.51	1371	C	0.01	0.50
B -K	0.17	1272	C	0.01	0.16
K -C	0.66	1834	C	0.02	0.64
C -D	0.74	104	C	0.00	0.74
Bottom Chords					
A -H	0.19	113	C	0.00	0.19
G -F	0.19	149	C	0.00	0.19
F -S1	0.48	1686	T	0.28	0.20
S1-I	0.63	1686	T	0.17	0.46
I -E	0.65	1829	T	0.19	0.46
Chord-Webs					
H -G	0.54	1186	C	0.03	0.51
G -J	0.18	1090	C	0.18	0.00
Webs					
J -F	0.45	1415	T		
F -B	0.07	276	T		
F -K	0.19	476	C		
K -I	0.06	239	T		
I -C	0.07	268	T		
C -E	0.55	1915	C		1 Br
E -D	0.10	265	T		
Sliders					
A -L	0.12	431	T		

LL Defl -0.15" in F -S1 L/999
TL Defl -0.37" in F -S1 L/710
Shear // Grain in F -F 0.30

Hz Disp	LL	DL	TL
Jt E	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

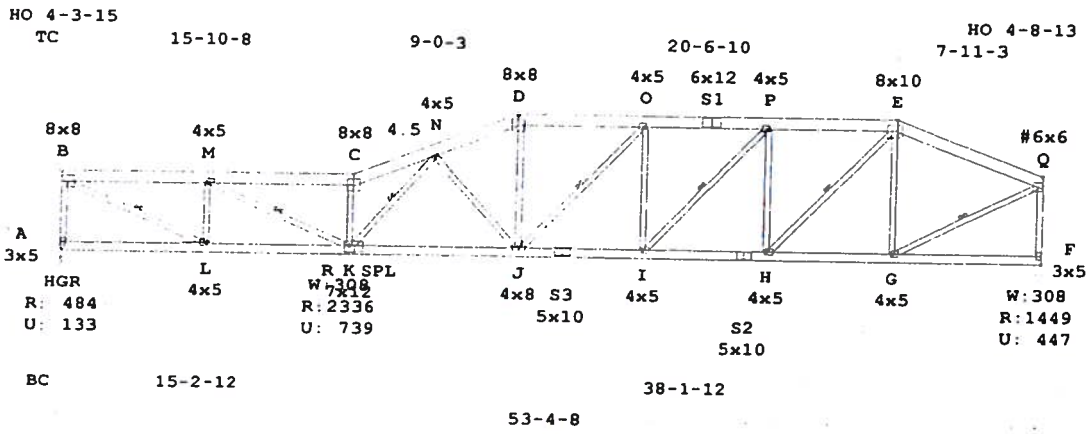
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	3.0x 4.0	Ctr	Ctr	0.54
J	LOCK	6.0x 6.0	Ctr	Ctr	0.87
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
K	LOCK	4.0x 5.0	Ctr	Ctr	0.51
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	2.0x 4.0	Ctr	Ctr	0.58
G	LOCK	2.0x 4.0	Ctr	Ctr	0.65
F	LOCK	5.0x12.0	Ctr	-1.0	0.97
F	LOCK	5.0x12.0	1.0	-1.0	0.88
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.63

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1915 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R47	1	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 559.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 8	SP-2400
BC	0.30	2x 6	SP-#2
WB	0.46	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 4- 8
BC Cont.		0- 0- 0	53- 4- 8

WB 1 rows CLB on B -L
WB 1 rows CLB on M -K
WB 1 rows CLB on K -N
WB 1 rows CLB on J -O
WB 1 rows CLB on I -P
WB 1 rows CLB on H -E
WB 1 rows CLB on G -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	485	134	3- 8	1- 0
			Hz =	-164
K	2336	739	3- 8	2- 8
F	1449	448	3- 8	1-11
			Hz =	82

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
B -M	0.09	444	C	0.00	0.09
M -C	0.10	556	T	0.01	0.09
C -N	0.04	591	T	0.02	0.02
N -D	0.03	1316	C	0.00	0.03
D -O	0.06	1234	C	0.00	0.06
O -S1	0.06	1771	C	0.00	0.06
S1-P	0.04	1771	C	0.00	0.04
P -E	0.04	1766	C	0.00	0.04
E -Q	0.12	1340	C	0.00	0.12
--------Bottom Chords---------					
A -L	0.16	107	T	0.00	0.16
L -K	0.25	444	T	0.03	0.22
K -J	0.27	725	T	0.05	0.22
J -S3	0.30	1771	T	0.13	0.17
S3-I	0.25	1771	T	0.21	0.04
I -S2	0.27	1766	T	0.21	0.06
S2-H	0.25	1766	T	0.21	0.04
H -G	0.27	1260	T	0.09	0.18
G -F	0.18	81	T	0.00	0.18
----------Webs----------					
A -B	0.12	413	C	WindLd	
B -L	0.15	494	T		1 Br
L -M	0.04	167	T		
M -K	0.33	1112	C		1 Br
K -C	0.04	195	T		
K -N	0.46	1982	C		1 Br
N -J	0.25	781	T		
J -D	0.06	230	T		
J -O	0.29	783	C		1 Br
I -O	0.06	254	T		
I -P	0.01	35	C		1 Br
H -P	0.27	398	C		
H -E	0.23	725	T		1 Br
G -E	0.34	496	C		
G -Q	0.45	1421	T		1 Br
F -Q	0.37	1381	C		

LL Defl -0.06" in I -S2 L/999
TL Defl -0.13" in I -S2 L/999
Shear // Grain in B -M 0.16

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	8.0x 8.0	0.5	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	0.5	0.63
C	LOCK	8.0x 8.0	Ctr	Ctr	0.87
N	LOCK	4.0x 5.0	-0.5	0.2	0.61
D	LOCK	8.0x 8.0	Ctr	Ctr	0.87
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
E	LOCK	8.0x10.0	Ctr	Ctr	0.87
Q#	LOCK	6.0x 6.0	0.1	-0.1	0.86
A	LOCK	3.0x 5.0	Ctr	Ctr	0.94
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	7.0x12.0	-0.5	-0.8	0.97
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

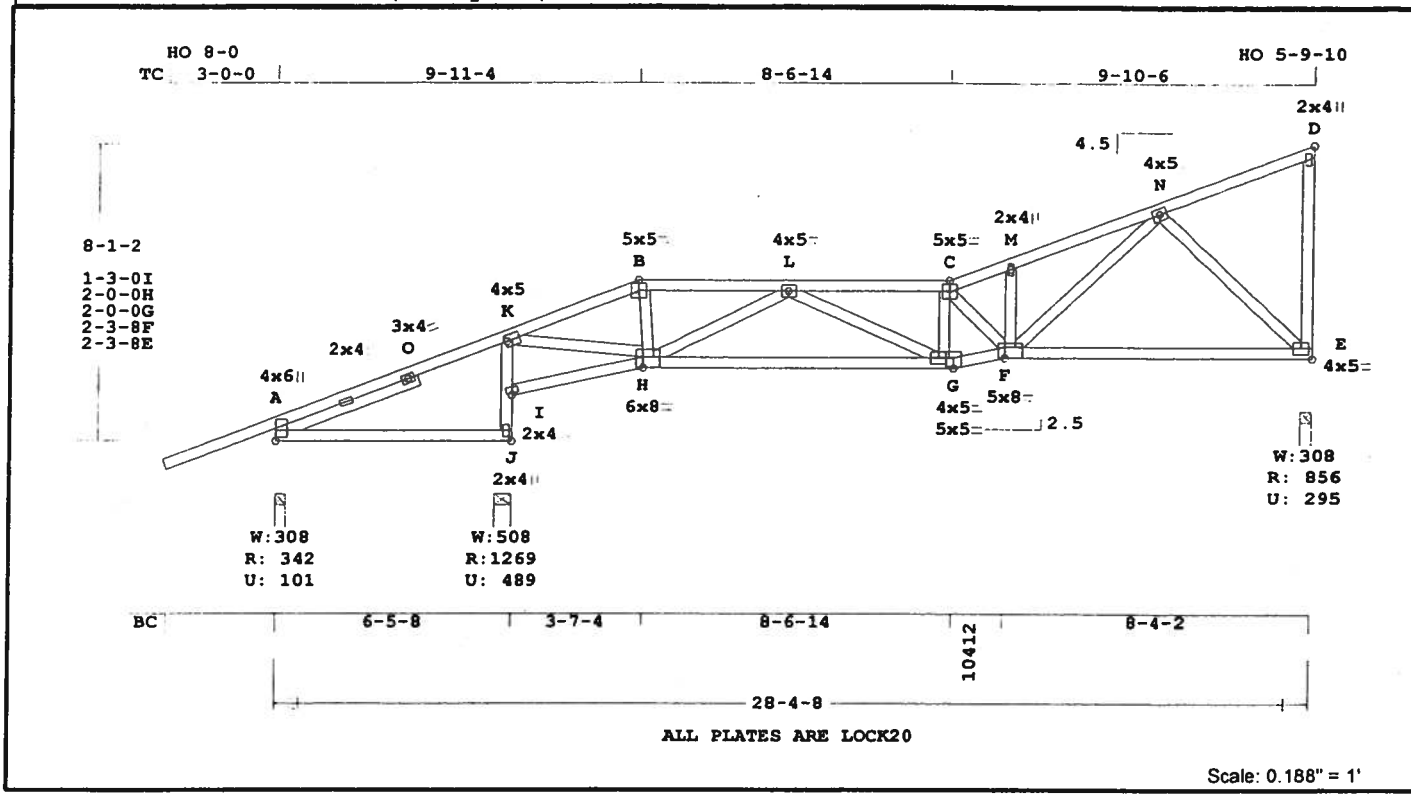
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1982 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R47A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 197.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.49	2x 4	SP-#2
BC	0.71	2x 4	SP-#2
CW	0.78	2x 4	SP-#3
WB	0.49	2x 4	SP-#3
SL	0.13	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 4- 8
BC	Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size	Req'd In-Sx	In-Sx
A	343	102	3- 8	1- 0	
			Hz =	334	
J	1269	490	5- 8	1- 6	
E	856	296	3- 8	1- 0	
			Hz =	180	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -O	0.31	288	C	0.00	0.31
O -K	0.49	433	C	0.04	0.45
K -B	0.46	1166	C	0.01	0.45
B -L	0.16	1092	C	0.01	0.15
L -C	0.18	2116	C	0.04	0.14
C -M	0.10	1934	C	0.06	0.04
M -N	0.24	1938	C	0.02	0.22
N -D	0.22	57	C	0.00	0.22
Bottom Chords					
A -J	0.19	167	C	0.00	0.19
I -H	0.38	193	C	0.00	0.38
H -G	0.71	1876	T	0.31	0.40
G -F	0.67	2202	T	0.37	0.30
F -E	0.51	685	T	0.06	0.45
Chord-Webs					
J -I	0.78	1208	C	0.03	0.75
I -K	0.16	1032	C	0.16	0.00
Webs					
K -H	0.40	1252	T		
B -H	0.07	273	T		
H -L	0.27	873	C		
L -G	0.08	321	T		
G -C	0.09	564	C		
C -F	0.07	417	C		
F -M	0.03	179	C		
F -N	0.49	1554	T		
N -E	0.44	961	C		
E -D	0.04	145	T		
Sliders					
A -O	0.13	466	T		

LL Defl -0.21" in F -E L/999
TL Defl -0.46" in F -E L/563
Shear // Grain in O -K 0.29

Hz Disp	LL	DL	TL
Jt E	0.03"	0.02"	0.05"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

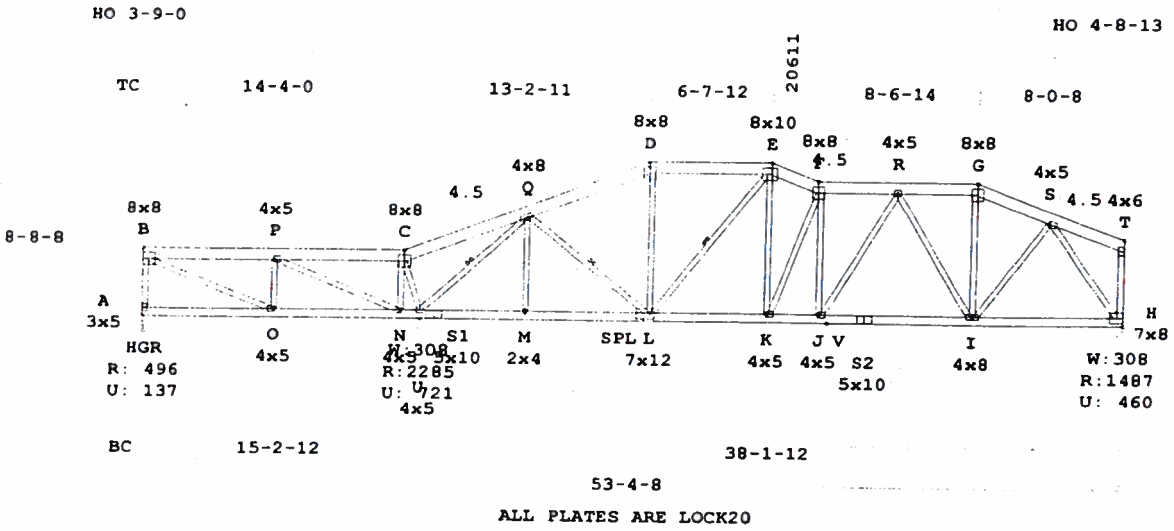
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
O	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	4.0x 5.0	Ctr	Ctr	0.53
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
L	LOCK	4.0x 5.0	Ctr	Ctr	0.55
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	2.0x 4.0	Ctr	Ctr	0.40
N	LOCK	4.0x 5.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	Ctr	0.58
I	LOCK	2.0x 4.0	Ctr	Ctr	0.67
H	LOCK	6.0x 8.0	Ctr	-0.5	0.63
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	5.0x 5.0	Ctr	1.2	0.68
F	LOCK	5.0x 8.0	Ctr	-1.0	0.86
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2116 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R48*	1	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 595.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 8	SP-2400
BC	0.34	2x 6	SP-#2
WB	0.97	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 4- 8
BC Cont.		0- 0- 0	53- 4- 8

WB 1 rows CLB on U -Q
WB 1 rows CLB on Q -L
WB 1 rows CLB on L -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	497	138	3- 8	1- 0
			Hz =	-169
U	2286	722	3- 8	2- 7
H	1487	461	3- 8	1-12
			Hz =	94

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -P	0.06	544 C	0.00	0.06
P -C	0.07	288 T	0.01	0.06
C -Q	0.07	511 T	0.02	0.05
Q -D	0.07	1544 C	0.02	0.05
D -E	0.07	1436 C	0.00	0.07
E -F	0.03	1759 C	0.02	0.01
F -R	0.02	1873 C	0.00	0.02
R -G	0.02	1307 C	0.00	0.02
G -S	0.03	1394 C	0.01	0.02
S -T	0.02	59 T	0.00	0.02
----------Bottom Chords---------				
A -O	0.14	122 T	0.00	0.14
O -N	0.18	544 T	0.04	0.14
N -U	0.17	299 C	0.00	0.17
U -S1	0.30	1084 T	0.13	0.17
S1-M	0.23	1084 T	0.13	0.10
M -L	0.22	1084 T	0.13	0.09
L -K	0.29	1666 T	0.20	0.09
K -J	0.26	1876 T	0.22	0.04
J -S2	0.26	1663 T	0.20	0.06
S2-I	0.34	1663 T	0.12	0.22
I -H	0.28	880 T	0.06	0.22
----------Webs----------				
A -B	0.10	433 C	WindLd	
B -O	0.19	599 T		
O -P	0.04	164 T		
P -N	0.75	916 C		
N -C	0.10	356 T		
C -U	0.11	554 C		
U -Q	0.59	2181 C		1 Br
M -Q	0.05	214 T		
Q -L	0.14	460 T		1 Br
L -D	0.07	266 T		
L -E	0.14	367 C		1 Br
K -E	0.20	569 T		
K -F	0.43	544 C		
J -F	0.18	253 C		
J -R	0.17	422 T		
R -I	0.67	716 C		
I -G	0.07	272 T		
I -S	0.23	719 T		
S -H	0.97	1569 C		
H -T	0.03	153 T		

LL Defl -0.07" in K -J L/999
TL Defl -0.14" in J -S2 L/999
Shear // Grain in B -P 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

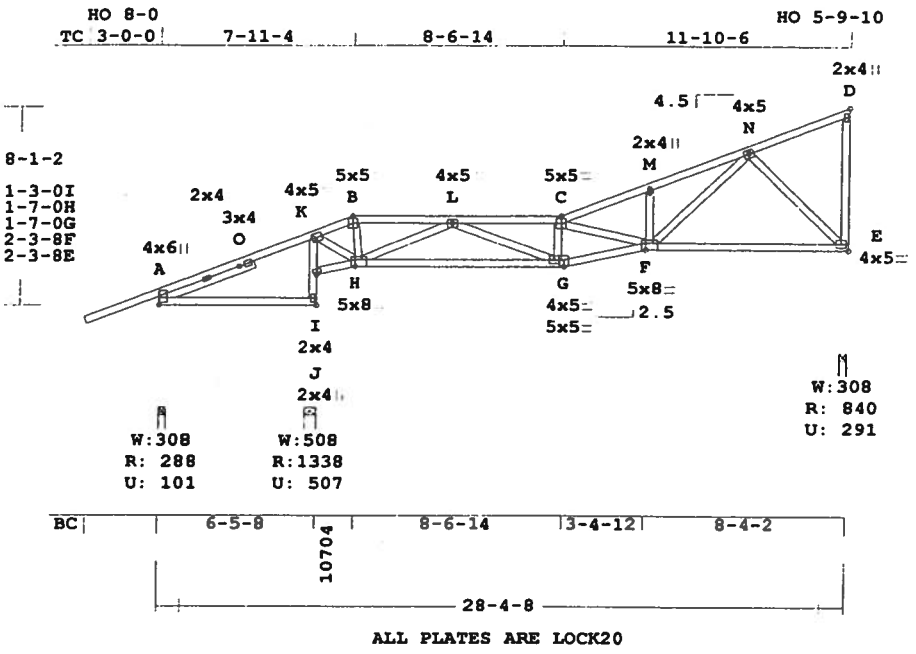
Jt Type	Plt Size	X	Y	JSI
B LOCK	8.0x 8.0	0.5	Ctr	0.82
P LOCK	4.0x 5.0	Ctr	0.5	0.65
C LOCK	8.0x 8.0	Ctr	Ctr	0.87
Q LOCK	4.0x 8.0-0.5	0.2		0.67
D LOCK	8.0x 8.0	Ctr	Ctr	0.87
E LOCK	8.0x10.0	Ctr	Ctr	0.87
F LOCK	8.0x 8.0	Ctr	Ctr	0.87
R LOCK	4.0x 5.0	Ctr	0.5	0.60
G LOCK	8.0x 8.0	Ctr	Ctr	0.87
S LOCK	4.0x 5.0	0.5	0.2	0.74
T LOCK	4.0x 6.0-0.2	Ctr		0.93
A LOCK	3.0x 5.0	Ctr	Ctr	0.94
O LOCK	4.0x 5.0	Ctr	Ctr	0.60
N LOCK	4.0x 5.0	Ctr	Ctr	0.60
U LOCK	4.0x 5.0	Ctr	Ctr	0.63
S1 LOCK	5.0x10.0	Ctr	Ctr	0.82
M LOCK	2.0x 4.0	Ctr	Ctr	0.56
L LOCK	7.0x12.0	1.0-0.8		0.97
K LOCK	4.0x 5.0	Ctr	Ctr	0.61
J LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2 LOCK	5.0x10.0	Ctr	Ctr	0.82
I LOCK	4.0x 8.0	Ctr	Ctr	0.54
H LOCK	7.0x 8.0-0.5	Ctr		0.79

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2181 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R48A	1	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 195.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.57	2x 4	SP-#2
BC	0.75	2x 4	SP-#2
CW	0.71	2x 4	SP-#2
WB	0.48	2x 4	SP-#3
SL	0.16	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	289	102	3- 8	1- 0
			Hz =	334
J	1339	507	5- 8	1- 8
E	841	292	3- 8	1- 0
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
--------Top Chords----------					
A -O	0.31	259	C	0.00	0.31
O -K	0.57	457	C	0.06	0.51
K -B	0.51	361	C	0.00	0.51
B -L	0.14	296	C	0.00	0.14
L -C	0.18	2389	C	0.04	0.14
C -M	0.21	1888	C	0.02	0.19
M -N	0.22	1898	C	0.02	0.20
N -D	0.20	57	C	0.00	0.20
--------Bottom Chords---------					
A -J	0.19	275	C	0.00	0.19
I -H	0.42	397	C	0.00	0.42
H -G	0.58	1656	T	0.27	0.31
G -F	0.75	2498	T	0.42	0.33
F -E	0.53	663	T	0.06	0.47
----------Chord-Webs----------					
J -I	0.71	1278	C	0.01	0.70
I -K	0.09	989	C	0.09	0.00
------------Webs------------					
K -H	0.22	714	T		
B -H	0.03	141	T		
H -L	0.43	1473	C		
L -G	0.25	792	T		
G -C	0.12	755	C		
C -F	0.17	671	C		
F -M	0.04	253	C		
F -N	0.48	1526	T		
N -E	0.43	930	C		
E -D	0.04	147	T		
----------Sliders------------					
A -O	0.16	520	T		

LL Defl -0.22" in F -E L/999
TL Defl -0.49" in F -E L/528
Shear // Grain in J -I 0.36

Hz Disp	LL	DL	TL
Jt E	0.03"	0.00"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

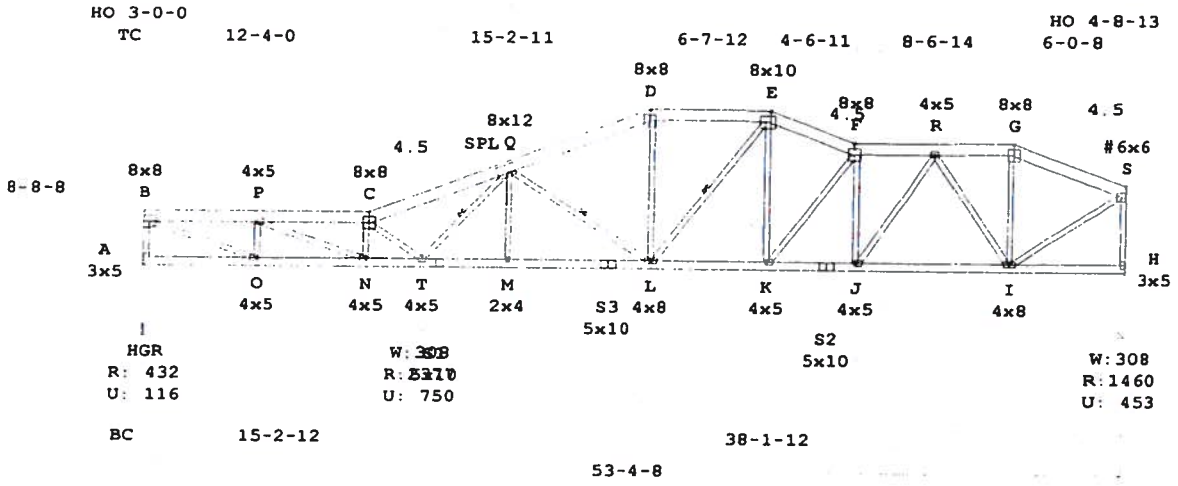
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
O	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	4.0x 5.0	Ctr	Ctr	0.49
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
L	LOCK	4.0x 5.0	Ctr	Ctr	0.64
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	2.0x 4.0	Ctr	Ctr	0.40
N	LOCK	4.0x 5.0	Ctr	Ctr	0.68
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	Ctr	0.58
I	LOCK	2.0x 4.0	Ctr	Ctr	0.71
H	LOCK	5.0x 8.0	Ctr-1.0		0.67
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	5.0x 5.0	Ctr 1.2		0.68
F	LOCK	5.0x 8.0	Ctr-1.0		0.85
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2389 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R49*	1	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 571.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.13	2x 8	SP-2400
BC	0.29	2x 6	SP-#2
WB	0.72	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 4- 8
BC Cont.		0- 0- 0	53- 4- 8

WB 1 rows CLB on T -Q
WB 1 rows CLB on Q -L
WB 1 rows CLB on L -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	433	117	3- 8	1- 0
			Hz =	-153
T	2377	750	3- 8	2- 9
H	1460	453	3- 8	1-12
			Hz =	100

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
B -P	0.05	509	C	0.00	0.05
P -C	0.03	238	T	0.00	0.03
C -Q	0.13	883	T	0.03	0.10
Q -D	0.10	1462	C	0.00	0.10
D -E	0.07	1354	C	0.00	0.07
E -F	0.06	1704	C	0.02	0.04
F -R	0.03	1961	C	0.02	0.01
R -G	0.02	1147	C	0.00	0.02
G -S	0.06	1224	C	0.01	0.05
Bottom Chords					
A -O	0.10	116	T	0.00	0.10
O -N	0.13	509	T	0.03	0.10
N -T	0.14	253	C	0.00	0.14
T -S1	0.23	812	T	0.09	0.14
S1-M	0.17	812	T	0.09	0.08
M -S3	0.17	812	T	0.09	0.08
S3-L	0.21	812	T	0.05	0.16
L -K	0.29	1604	T	0.19	0.10
K -S2	0.25	1967	T	0.23	0.02
S2-J	0.28	1967	T	0.23	0.05
J -I	0.28	1631	T	0.19	0.09
I -H	0.16	100	T	0.00	0.16
Webs					
A -B	0.06	377	C	WindLd	
B -O	0.17	550	T		
O -P	0.03	133	T		
P -N	0.47	807	C		
N -C	0.11	371	T		
C -T	0.16	685	C		
T -Q	0.56	2434	C	1 Br	
M -Q	0.06	231	T		
Q -L	0.21	656	T	1 Br	
L -D	0.06	245	T		
L -E	0.16	401	C	1 Br	
K -E	0.18	545	T		
K -F	0.50	579	C		
J -F	0.21	381	C		
J -R	0.23	612	T		
R -I	0.72	896	C		
I -G	0.05	216	T		
I -S	0.44	1402	T		
H -S	0.38	1413	C		

LL Defl -0.07" in S2-J L/999
TL Defl -0.14" in S2-J L/999
Shear // Grain in C -Q 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	8.0x 8.0	0.5	Ctr	0.82
P	LOCK	4.0x 5.0	Ctr	0.5	0.68
C	LOCK	8.0x 8.0	Ctr	Ctr	0.87
Q	LOCK	8.0x12.0	-0.6	1.5	0.97
D	LOCK	8.0x 8.0	Ctr	Ctr	0.87
E	LOCK	8.0x10.0	Ctr	Ctr	0.87
F	LOCK	8.0x 8.0	Ctr	Ctr	0.87
R	LOCK	4.0x 5.0	Ctr	0.5	0.58
G	LOCK	8.0x 8.0	Ctr	Ctr	0.87
S#	LOCK	6.0x 6.0	0.1	-0.3	0.91
A	LOCK	3.0x 5.0	Ctr	Ctr	0.94
O	LOCK	4.0x 5.0	Ctr	Ctr	0.60
N	LOCK	4.0x 5.0	Ctr	Ctr	0.60
T	LOCK	4.0x 5.0	Ctr	Ctr	0.49
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	2.0x 4.0	Ctr	Ctr	0.56
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 8.0	Ctr	Ctr	0.52
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 8.0	Ctr	Ctr	0.56
H	LOCK	3.0x 5.0	Ctr	Ctr	0.94

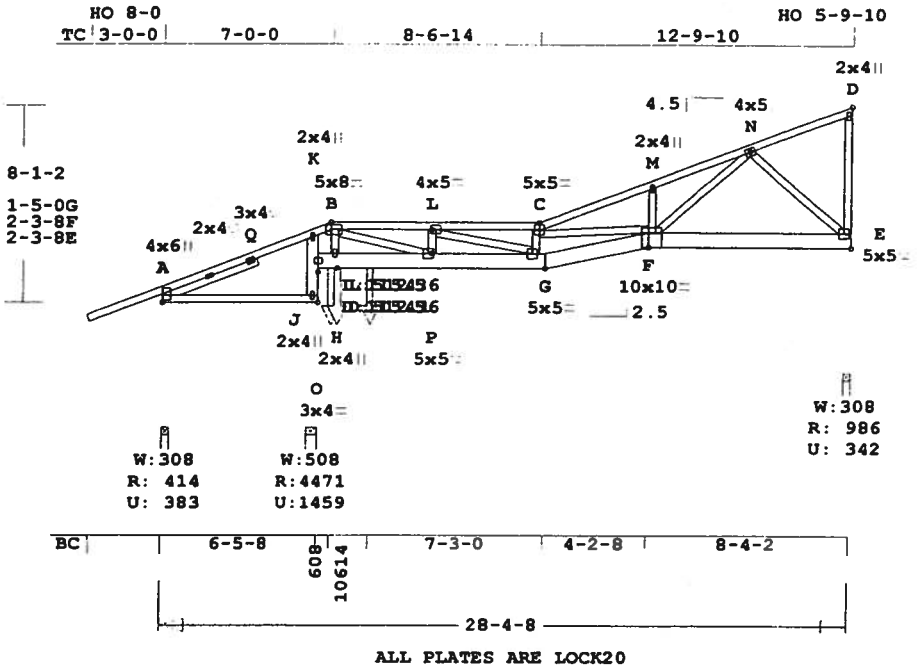
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2434 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R49A	1*2P	SP	280408	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 234.4 LBS
G = Gravity Uplift

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.77	2x 4	SP-#2
BC	0.41	2x 8	SP-2400
EX A -J		2x 4	SP-#2
CW	0.97	2x 6	SP-#2
WB	0.61	2x 4	SP-#3
SL	0.18	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	28.4'
BC V	0	20	0.0'	28.4'
TC V	50	25	7.0'	14.6'
TC V	-40	-20	14.6'	15.6'
BC V	0	25	7.2'	14.8'
BC V	0	-20	14.8'	15.8'
BC V	436	436	8.6'	CL-LB
BC V	116	116	7.0'	CL-LB
BC V	512	512	7.2'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	414	384G	3- 8	1- 0
			Hz =	336
J	4471	1459	5- 8	2- 8
E	987	342	3- 8	1- 0
			Hz =	181

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -Q	0.17	228 C	0.01 0.16
Q -K	0.77	1397 T	0.13 0.64
K -B	0.64	142 C	0.00 0.64
B -L	0.24	3157 C	0.01 0.23
L -C	0.19	3400 C	0.02 0.17
C -M	0.21	2624 C	0.01 0.20
M -N	0.15	2639 C	0.06 0.09
N -D	0.12	57 C	0.00 0.12
Bottom Chords			
A -J	0.11	1211 C	0.02 0.09
O -H	0.41	508 C	0.00 0.41
H -P	0.10	491 C	0.00 0.10
P -G	0.25	3156 T	0.06 0.19
G -F	0.25	3608 T	0.06 0.19
F -E	0.06	805 T	0.01 0.05
Chord-Webs			
J -O	0.97	4416 C	0.01 0.96
O -K	0.05	1909 C	0.05 0.00
Webs			
H -B	0.03	417 C	
B -P	0.61	3814 T	
P -L	0.04	555 C	
L -G	0.04	256 T	
G -C	0.09	1237 C	
C -F	0.08	1001 C	
F -M	0.02	306 C	
F -N	0.35	2184 T	
N -E	0.10	1108 C	
E -D	0.01	146 T	
Sliders			
A -Q	0.18	1175 T	

LL Defl -0.14" in P -G L/999
TL Defl -0.28" in P -G L/927
Shear // Grain in K -B 0.77

Hz Disp	LL	DL	TL
Jt E	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
Q	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	2.0x 4.0	Ctr	Ctr	0.38
B	LOCK	5.0x 8.0	0.5-0.3		0.90
L	LOCK	4.0x 5.0	Ctr	Ctr	0.76
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	2.0x 4.0	Ctr	Ctr	0.40
N	LOCK	4.0x 5.0	Ctr	Ctr	0.61
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	Ctr	0.91
O	LOCK	3.0x 4.0	Ctr	Ctr	0.91
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
P	LOCK	5.0x 5.0	Ctr	Ctr	0.93
G	LOCK	5.0x 5.0	Ctr	Ctr	0.61
F	LOCK	10.0x10.0	Ctr-2.2		0.55
E	LOCK	5.0x 5.0	Ctr	Ctr	0.74

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	1	12	24	0
BC	1	12	24	0
WB	1	4	4	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Provide connection to bearing for 335 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 4416 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R49A*	1*2P	SP	280408	4.5	3- 0- 0	0	

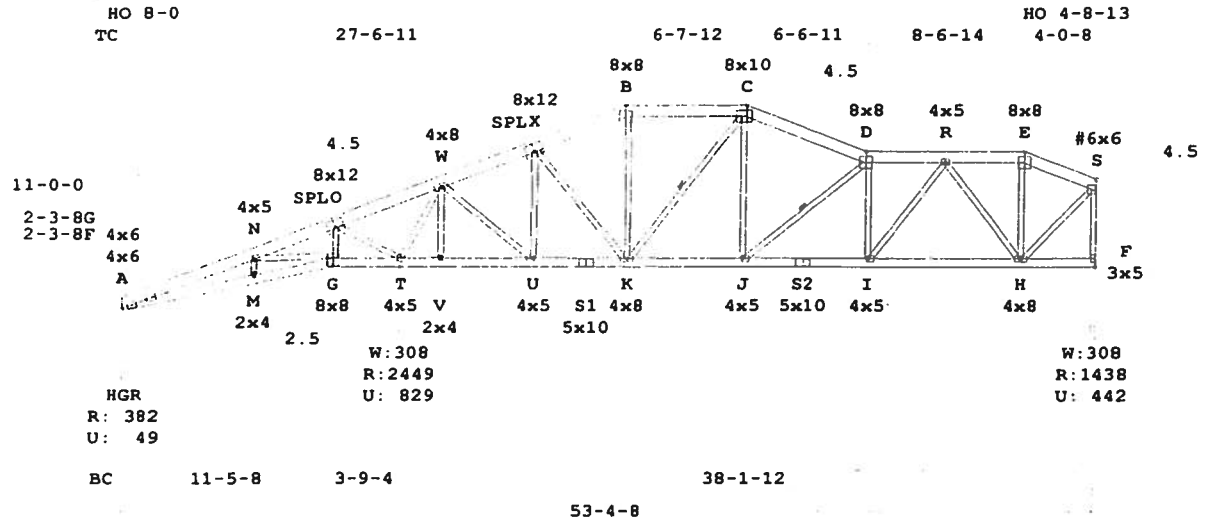
Office and Care Center, Hospice, M.M. Parrish

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R50*	1	SP	530408	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 563.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 8	SP-2400
BC	0.30	2x 6	SP-#2
WB	0.75	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	53- 4- 8
BC Cont.	0- 0- 0	53- 4- 8

WB 1 rows CLB on K -C
WB 1 rows CLB on J -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	383	49	3- 8	1- 0
		Hz =	284	
T	2449	829	3- 8	2-10
F	1438	442	3- 8	1-11
		Hz =	179	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -N	0.08	480	C	0.00	0.08
N -O	0.09	412	T	0.01	0.08
O -W	0.11	1003	T	0.03	0.08
W -X	0.05	1061	C	0.00	0.05
X -B	0.05	1365	C	0.02	0.03
B -C	0.07	1277	C	0.00	0.07
C -D	0.09	1662	C	0.02	0.07
D -R	0.03	2045	C	0.00	0.03
R -E	0.02	912	C	0.00	0.02
E -S	0.04	972	C	0.01	0.03
--------Bottom Chords---------					
A -M	0.16	477	T	0.05	0.11
M -G	0.16	480	T	0.05	0.11
G -T	0.16	371	C	0.00	0.16
T -V	0.16	70	T	0.00	0.16
V -U	0.06	70	T	0.00	0.06
U -S1	0.15	985	T	0.11	0.04
S1-K	0.15	985	T	0.11	0.04
K -J	0.23	1561	T	0.18	0.05
J -S2	0.29	2054	T	0.24	0.05
S2-I	0.30	2054	T	0.24	0.06
I -H	0.26	1562	T	0.18	0.08
H -F	0.13	179	T	0.00	0.13
----------Webs----------					
M -N	0.05	188	T		
N -G	0.26	829	C		
G -O	0.05	213	T		
O -T	0.18	668	C		
T -W	0.68	2104	C		
V -W	0.02	90	T		
W -U	0.41	1307	T		
U -X	0.39	738	C		
X -K	0.14	455	T		
K -B	0.06	226	T		
K -C	0.18	455	C	1 Br	
J -C	0.16	558	T		
J -D	0.19	622	C	1 Br	
I -D	0.23	517	C		
I -R	0.26	823	T		
R -H	0.75	1108	C		
H -E	0.04	169	T		
H -S	0.42	1321	T		
F -S	0.38	1415	C		

LL Defl -0.05" in I -H L/999
TL Defl -0.06" in A -M L/999
Shear // Grain in C -D 0.12

Hz Disp	LL	DL	TL
Jt F	0.02"	0.02"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

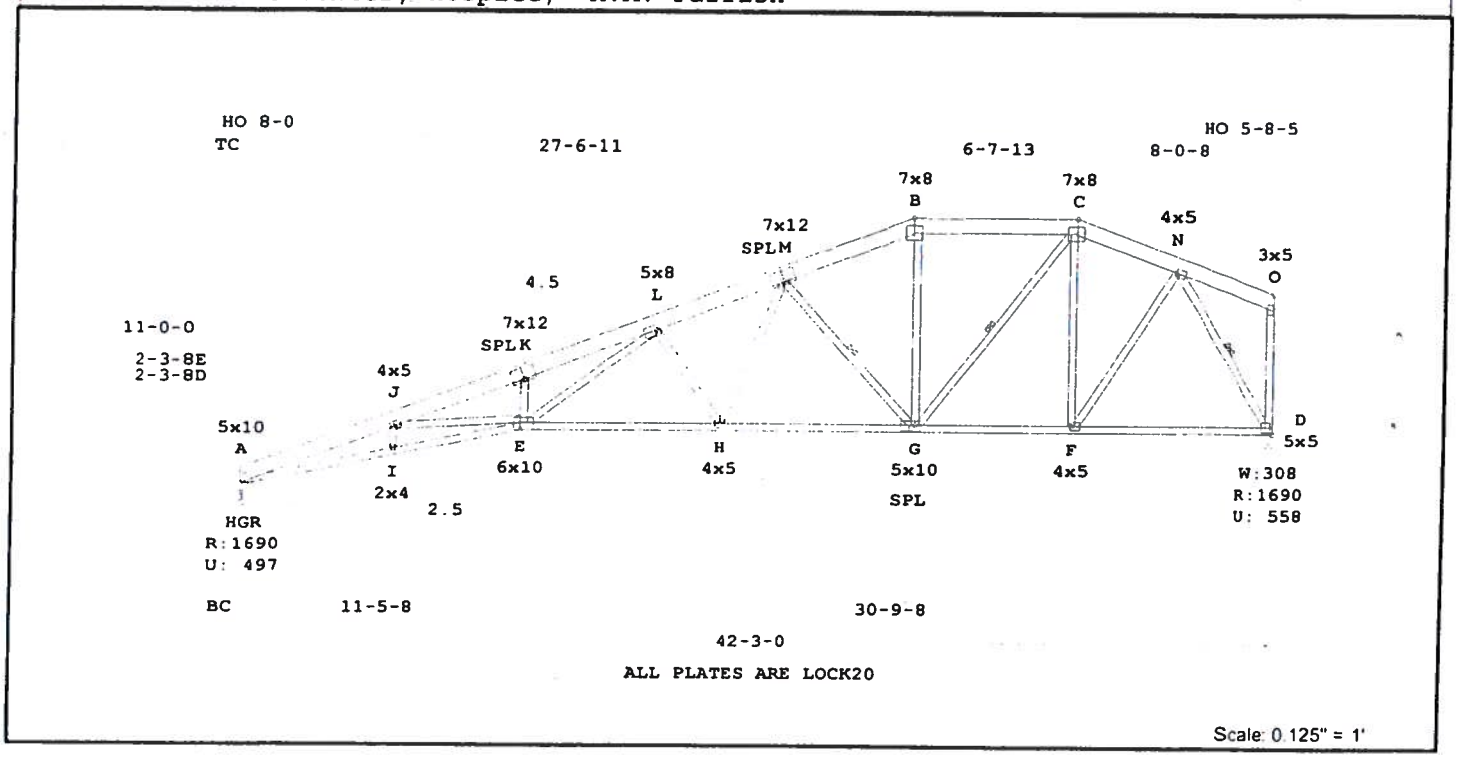
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.71
A	LOCK	4.0x 6.0	Ctr	Ctr	0.18
N	LOCK	4.0x 5.0-0.5	0.2	0.62	
O	LOCK	8.0x12.0-0.6	1.5	0.97	
W	LOCK	4.0x 8.0-0.5	0.2	0.76	
X	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	8.0x 8.0	Ctr	Ctr	0.87
C	LOCK	8.0x10.0	Ctr	Ctr	0.87
D	LOCK	8.0x 8.0	Ctr	Ctr	0.87
R	LOCK	4.0x 5.0	Ctr	0.5	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.87
S	LOCK	6.0x 6.0	0.2-0.1		0.86
M	LOCK	2.0x 4.0	Ctr	Ctr	0.56
G	LOCK	8.0x 8.0	Ctr-1.5		0.82
T	LOCK	4.0x 5.0	Ctr	Ctr	0.55
V	LOCK	2.0x 4.0	Ctr	Ctr	0.56
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.50
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	4.0x 8.0	Ctr	Ctr	0.55
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2104 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R51*	5	SP	420300	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 398.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 8	SP-2400
BC	0.92	2x 4	SP-#2
EX A -E		2x 4	SP-2400
WB	0.93	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	42- 3- 0
BC Cont.	0- 0- 0	42- 3- 0

WB 1 rows CLB on M -G
WB 1 rows CLB on G -C
WB 1 rows CLB on N -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1690	497	3- 8	1- 6
			Hz =	329
D	1690	559	3- 8	2- 0
			Hz =	191

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -J	0.32	6172 C	0.04 0.28
J -K	0.34	6679 C	0.05 0.29
K -L	0.12	6710 C	0.05 0.07
L -M	0.06	3557 C	0.04 0.02
M -B	0.05	2064 C	0.02 0.03
B -C	0.06	1932 C	0.00 0.06
C -N	0.03	1428 C	0.01 0.02
N -O	0.02	56 T	0.00 0.02
--------Bottom Chords---------			
A -I	0.59	5759 T	0.41 0.18
I -E	0.58	5783 T	0.41 0.17
E -H	0.92	3979 T	0.66 0.26
H -G	0.73	2809 T	0.47 0.26
G -F	0.51	1335 T	0.14 0.37
F -D	0.45	867 T	0.08 0.37
------------Webs------------			
I -J	0.03	121 T	
J -E	0.21	656 T	
E -K	0.07	421 C	
E -L	0.93	2916 T	
L -H	0.46	1239 C	
H -M	0.43	1342 T	
M -G	0.38	1335 C	1 Br
G -B	0.12	422 T	
G -C	0.30	964 T	1 Br
F -C	0.54	570 C	
F -N	0.27	870 T	
N -D	0.44	1746 C	1 Br
D -O	0.04	145 T	

LL Defl -0.42" in E -H L/999
TL Defl -0.92" in E -H L/546
Shear // Grain in A -J 0.29

Hz Disp	LL	DL	TL
Jt D	0.18"	0.18"	0.37"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

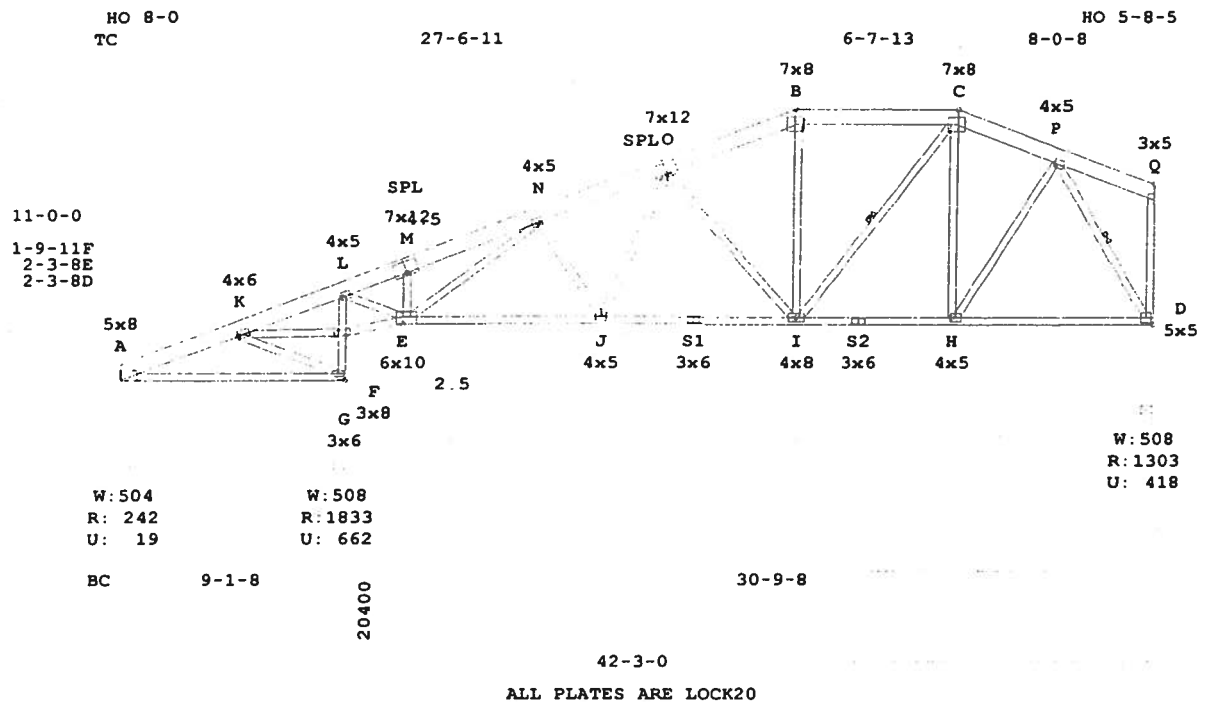
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0 Ctr	0.8		0.96
J	LOCK	4.0x 5.0-0.5	0.2		0.63
K	LOCK	7.0x12.0-0.7	2.0		0.97
L	LOCK	5.0x 8.0-1.9-0.7			0.97
M	LOCK	7.0x12.0-0.7	2.0		0.97
B	LOCK	7.0x 8.0 Ctr	Ctr		0.87
C	LOCK	7.0x 8.0 1.0	Ctr		0.87
N	LOCK	4.0x 5.0 0.5	0.2		0.72
O	LOCK	3.0x 5.0 Ctr	Ctr		0.89
I	LOCK	2.0x 4.0 Ctr	Ctr		0.49
E	LOCK	6.0x10.0 Ctr-0.5			0.94
H	LOCK	4.0x 5.0 Ctr	Ctr		0.60
G	LOCK	5.0x10.0 1.0-0.5			0.88
F	LOCK	4.0x 5.0 Ctr	Ctr		0.60
D	LOCK	5.0x 5.0 Ctr	Ctr		0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Provide connection to bearing for 329 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6710 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
060039	R56	1	SP	420300	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 413.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.08	2x 8	SP-2400
BC	0.58	2x 4	SP-#2
CW	0.35	2x 4	SP-#3
WB	0.54	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	42- 3- 0
BC Cont.		0- 0- 0	42- 3- 0

WB 1 rows CLB on I -C
WB 1 rows CLB on P -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	242	20	5- 4	1- 0
			Hz =	329
G	1834	662	5- 8	2- 0
D	1304	419	5- 8	1- 9
			Hz =	191

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.05	169	C	0.00	0.05
K -L	0.08	457	T	0.01	0.07
L -M	0.07	1136	C	0.00	0.07
M -N	0.04	1117	C	0.00	0.04
N -O	0.04	1774	C	0.02	0.02
O -B	0.04	1353	C	0.01	0.03
B -C	0.06	1262	C	0.00	0.06
C -P	0.03	1028	C	0.01	0.02
P -Q	0.02	57	T	0.00	0.02
Bottom Chords					
A -G	0.36	204	C	0.00	0.36
F -E	0.28	419	C	0.00	0.28
E -J	0.58	1687	T	0.28	0.30
J -S1	0.52	1590	T	0.16	0.36
S1-I	0.41	1590	T	0.26	0.15
I -S2	0.33	961	T	0.10	0.23
S2-H	0.46	961	T	0.10	0.36
H -D	0.42	644	T	0.06	0.36
Chord-Webs					
G -F	0.35	1686	C	0.06	0.29
F -L	0.24	1459	C	0.24	0.00
Webs					
K -G	0.07	244	T		
K -F	0.10	373	C		
L -E	0.49	1552	T		
E -M	0.04	185	C		
E -N	0.54	810	C		
N -J	0.03	138	T		
J -O	0.07	288	T		
O -I	0.47	500	C		
I -B	0.06	240	T		
I -C	0.15	485	T		1 Br
H -C	0.32	337	C		
H -P	0.18	591	T		
P -D	0.33	1297	C		1 Br
D -Q	0.04	146	T		

LL Defl -0.12" in A -G L/801
TL Defl -0.25" in A -G L/395
Shear // Grain in A -G 0.22

Hz Disp	LL	DL	TL
Jt D	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

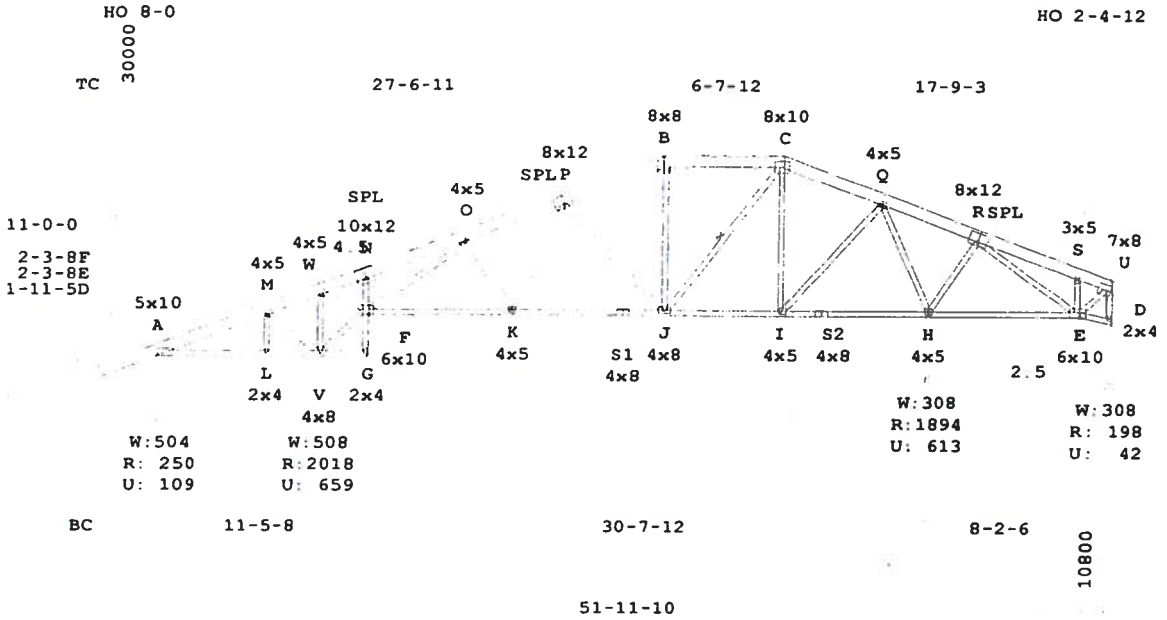
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0 Ctr	0.6		0.72
K	LOCK	4.0x 6.0-0.5	0.2		0.83
L	LOCK	4.0x 5.0-0.5	0.2		0.72
M	LOCK	7.0x12.0-0.7	2.0		0.97
N	LOCK	4.0x 5.0-0.7	0.1		0.66
O	LOCK	7.0x12.0-0.7	2.0		0.97
B	LOCK	7.0x 8.0 Ctr	Ctr		0.87
C	LOCK	7.0x 8.0	1.0	Ctr	0.87
P	LOCK	4.0x 5.0	0.5	0.2	0.72
Q	LOCK	3.0x 5.0 Ctr	Ctr		0.89
G	LOCK	3.0x 6.0 Ctr	Ctr		0.88
F	LOCK	3.0x 8.0 Ctr	Ctr		0.80
E	LOCK	6.0x10.0 Ctr	-0.5		0.70
J	LOCK	4.0x 5.0 Ctr	Ctr		0.58
S1	LOCK	3.0x 6.0 Ctr	Ctr		0.88
I	LOCK	4.0x 8.0 Ctr	Ctr		0.50
S2	LOCK	3.0x 6.0 Ctr	Ctr		0.88
H	LOCK	4.0x 5.0 Ctr	Ctr		0.60
D	LOCK	5.0x 5.0 Ctr	Ctr		0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Provide connection to bearing for 329 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1774 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R57*	1	SP	511110	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 507.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.08	2x 8	SP-2400
BC	0.50	2x 4	SP-#2
CW	0.27	2x 4	SP-#3
WB	0.93	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51-11-10
BC Cont.	0- 0- 0	51-11-10

WB 1 rows CLB on J -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	251	110	5- 4	1- 0
			Hz = 165	
V	2018	660	5- 8	2- 4
H	1895	613	3- 8	2- 0
D	199	43	3- 8	1- 0
			Hz = 168	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.07	400 T	0.01	0.06	
M -W	0.08	789 T	0.02	0.06	
W -N	0.05	235 C	0.00	0.05	
N -O	0.05	296 C	0.00	0.05	
O -P	0.04	1435 C	0.01	0.03	
P -B	0.04	1121 C	0.01	0.03	
B -C	0.06	1044 C	0.00	0.06	
C -Q	0.04	786 C	0.00	0.04	
Q -R	0.07	366 T	0.01	0.06	
R -S	0.06	185 T	0.00	0.06	
S -U	0.00	141 C			
Bottom Chords					
A -L	0.12	348 C	0.00	0.12	
L -V	0.12	348 C	0.00	0.12	
V -G	0.04	38 C	0.00	0.04	
F -K	0.50	1288 T	0.13	0.37	
K -S1	0.50	1319 T	0.13	0.37	
S1-J	0.40	1319 T	0.13	0.27	
J -I	0.35	730 T	0.08	0.27	
I -S2	0.26	206 T	0.02	0.24	
S2-H	0.44	206 T	0.02	0.42	
H -E	0.42	112 C	0.00	0.42	
E -D	0.11	175 T	0.00	0.11	
Chord-Webs					
G -F	0.27	36 T	0.00	0.27	
F -N	0.06	344 C	0.06	0.00	
Webs					
L -M	0.04	185 T			
M -V	0.12	534 C			
V -W	0.20	920 C			
V -F	0.22	1021 C			
W -F	0.36	1128 T			
F -O	0.84	1284 C			
O -K	0.05	204 T			
K -P	0.04	183 T			
P -J	0.40	413 C			
J -B	0.05	188 T			
J -C	0.16	506 T			1 Br
I -C	0.44	462 C			
I -Q	0.25	789 T			
Q -H	0.93	1446 C			
H -R	0.16	443 C			
R -E	0.13	309 T			
E -S	0.06	246 T			
E -U	0.05	178 T			
D -U	0.03	212 C			

LL Defl -0.08" in H -E L/999
TL Defl -0.16" in H -E L/723
Shear // Grain in S2-H 0.19

Hz Disp	LL	DL	TL
Jt H	0.02"	0.02"	0.05"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

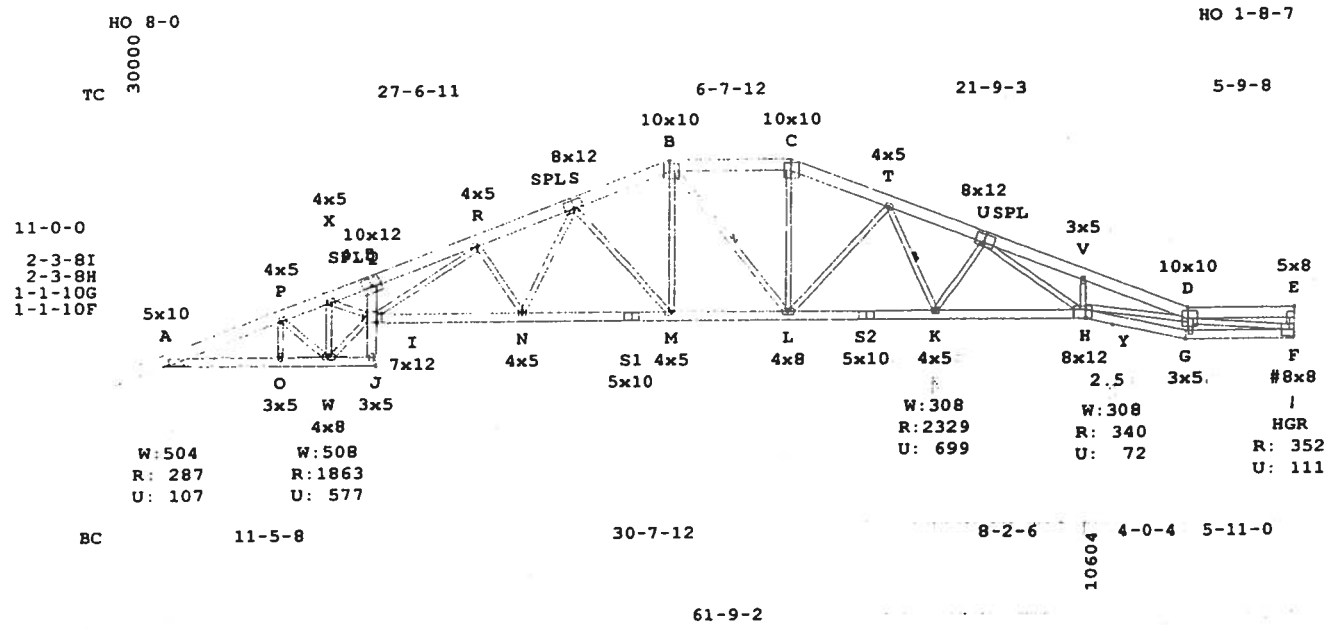
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	0.7	0.6	0.77
M	LOCK	4.0x 5.0-0.5	0.2		0.56
W	LOCK	4.0x 5.0-0.5	0.2		0.56
N	LOCK	10.0x12.0-0.6	1.7		0.80
O	LOCK	4.0x 5.0-0.5	0.2		0.87
P	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	8.0x 8.0 Ctr	Ctr		0.85
C	LOCK	8.0x10.0 Ctr	Ctr		0.85
Q	LOCK	4.0x 5.0	0.5	0.2	0.60
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	3.0x 5.0 Ctr	Ctr		0.37
U	LOCK	7.0x 8.0-1.5	0.6		0.92
L	LOCK	2.0x 4.0 Ctr	Ctr		0.55
V	LOCK	4.0x 8.0 Ctr	Ctr		0.51
G	LOCK	2.0x 4.0 Ctr	Ctr		0.69
F	LOCK	6.0x10.0 Ctr	1.3		0.88
K	LOCK	4.0x 5.0 Ctr	Ctr		0.57
S1	LOCK	4.0x 8.0 Ctr	0.2		0.75
J	LOCK	4.0x 8.0 Ctr	Ctr		0.50
I	LOCK	4.0x 5.0 Ctr	Ctr		0.60
S2	LOCK	4.0x 8.0 Ctr	0.2		0.75
H	LOCK	4.0x 5.0 Ctr	Ctr		0.58
E	LOCK	6.0x10.0 Ctr	-0.5		0.75
D	LOCK	2.0x 4.0 Ctr	Ctr		0.70

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1446 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R58*	1	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 638.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 8	SP-2400
BC	0.62	2x 6	SP-#2
CW	0.07	2x 6	SP-#2
WB	0.75	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on B -L
WB 1 rows CLB on T -K
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	288	107	5- 4	1- 0
			Hz =	-186
W	1864	577	5- 8	2- 1
K	2330	700	3- 8	2- 8
Y	340	73	3- 8	1- 0
F	353	111	3- 8	1- 0
			Hz =	193

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -P	0.07	360	T	0.01	0.06
P -X	0.08	697	T	0.02	0.06
X -Q	0.05	226	C	0.00	0.05
Q -R	0.05	304	C	0.00	0.05
R -S	0.03	1268	C	0.00	0.03
S -B	0.04	888	C	0.01	0.03
B -C	0.07	426	T	0.00	0.07
C -T	0.04	449	C	0.01	0.03
T -U	0.10	1075	T	0.03	0.07
U -V	0.08	287	T	0.01	0.07
V -D	0.08	303	T	0.01	0.07
D -E	0.05	0	T	0.00	0.05
Bottom Chords					
A -O	0.07	314	C	0.00	0.07
O -W	0.07	314	C	0.00	0.07
W -J	0.05	41	C	0.00	0.05
I -N	0.26	1180	T	0.14	0.12
N -S1	0.25	1121	T	0.08	0.17
S1-M	0.21	1121	T	0.08	0.13
M -L	0.19	826	T	0.06	0.13
L -S2	0.10	314	C	0.00	0.10
S2-K	0.27	314	C	0.00	0.27
K -H	0.27	748	C	0.00	0.27
H -Y	0.42	66	C	0.00	0.42
H -G	0.62	472	T	0.05	0.57
G -F	0.36	418	T	0.05	0.31
Chord-Webs					
J -I	0.07	21	T	0.00	0.07
I -Q	0.02	355	C	0.02	0.00
Webs					
O -P	0.05	190	T		
P -W	0.10	500	C		
W -X	0.16	801	C		
W -I	0.19	917	C		
X -I	0.32	1018	T		
I -R	0.75	1117	C		
R -N	0.03	136	T		
N -S	0.06	243	T		
S -M	0.40	436	C		
M -B	0.15	497	T		
B -L	0.26	663	C	1 Br	
L -C	0.17	193	C		
L -T	0.35	1101	T		
T -K	0.39	1814	C	1 Br	
K -U	0.17	495	C		
U -H	0.23	593	T		
H -V	0.09	390	C		
H -D	0.37	699	C		
G -D	0.06	248	T		
D -F	0.19	423	C		
F -E	0.02	172	C		

LL Defl -0.06" in G -F L/999
TL Defl -0.13" in G -F L/854
Shear // Grain in H -Y 0.27

Hz Disp	LL	DL	TL
Jt Y	0.02"	0.02"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0 Ctr	Ctr		0.93
P	LOCK	4.0x 5.0-0.5	0.2		0.56
X	LOCK	4.0x 5.0-0.5	0.2		0.56
Q	LOCK	10.0x12.0-0.4	1.0		0.89
R	LOCK	4.0x 5.0-0.5	0.2		0.87
S	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0-0.5	Ctr		0.76
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
T	LOCK	4.0x 5.0	0.5	0.2	0.60
U	LOCK	8.0x12.0	0.6	1.5	0.97
V	LOCK	3.0x 5.0	Ctr	Ctr	0.41
D	LOCK	10.0x10.0	1.0	Ctr	0.84
E	LOCK	5.0x 8.0-0.8	Ctr		0.91
O	LOCK	3.0x 5.0	Ctr	Ctr	0.41
W	LOCK	4.0x 8.0	Ctr	Ctr	0.50
J	LOCK	3.0x 5.0	Ctr	Ctr	0.73
I	LOCK	7.0x12.0	Ctr	0.7	0.87
N	LOCK	4.0x 5.0	Ctr	Ctr	0.57
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
H	LOCK	8.0x12.0	Ctr	-1.5	0.79
G	LOCK	3.0x 5.0	Ctr	Ctr	0.41
F#	LOCK	8.0x 8.0-0.7	-0.2		0.75

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf

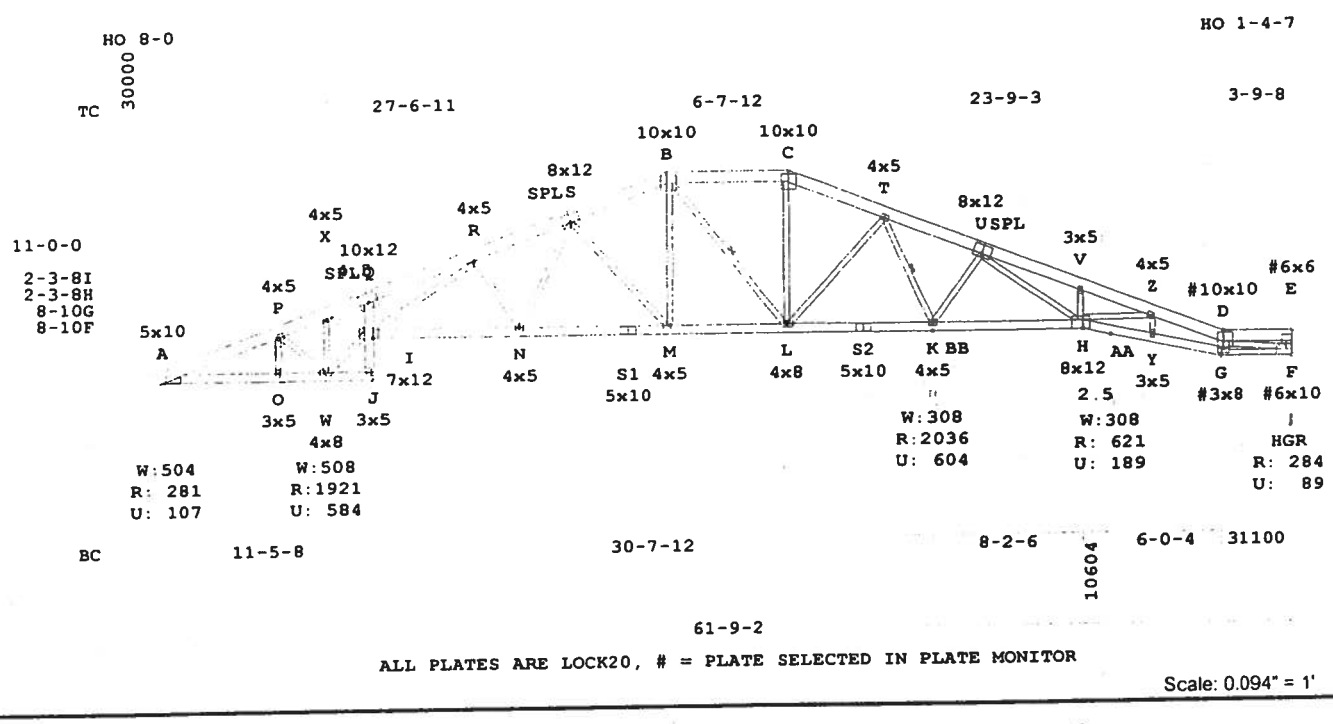
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R58*	1	SP	610902	4.5	3- 0- 0	0	
Office and Care Center, Hospice, M.M. Parrish								

BC Dead Load: 6.0 psf
Max comp. force 1814 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R59*	1	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 629.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.14	2x 8	SP-2400
BC	0.61	2x 6	SP-#2
EX G -F		2x 4	SP-#2
CW	0.07	2x 6	SP-#2
WB	0.79	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on B -L
WB 1 rows CLB on T -K
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	282	107	5- 4	1- 0
			Hz =	-195
W	1921	584	5- 8	2- 2
K	2037	605	3- 8	2- 3
AA	622	189	3- 8	1- 0
F	285	90	3- 8	1- 0
			Hz =	200

Membr	CSI	P Lbs	Ax	CSI	Bnd
Top Chords					
A -P	0.07	375	T	0.01	0.06
P -X	0.08	713	T	0.02	0.06
X -Q	0.05	263	C	0.00	0.05
Q -R	0.05	344	C	0.00	0.05
R -S	0.03	1369	C	0.00	0.03
S -B	0.04	1007	C	0.01	0.03
B -C	0.07	566	C	0.00	0.07
C -T	0.04	612	C	0.01	0.03
T -U	0.09	738	T	0.02	0.07
U -V	0.09	588	T	0.02	0.07
V -Z	0.14	536	T	0.01	0.13
Z -D	0.13	222	C	0.00	0.13
D -E	0.04	0	T	0.00	0.04
Bottom Chords					
A -O	0.07	328	C	0.00	0.07
O -W	0.07	328	C	0.00	0.07
W -J	0.06	40	C	0.00	0.06
I -N	0.27	1266	T	0.15	0.12
N -S1	0.26	1224	T	0.09	0.17
S1-M	0.22	1224	T	0.09	0.13
M -L	0.20	938	T	0.07	0.13
L -S2	0.10	227	T	0.00	0.10
S2-K	0.25	227	T	0.00	0.25
K -H	0.25	509	C	0.00	0.25
H -AA	0.60	194	T	0.02	0.58
AA-Y	0.61	313	T	0.03	0.58
Y -G	0.37	278	T	0.04	0.33
G -F	0.37	271	T	0.04	0.33
Chord-Webs					
J -I	0.07	21	T	0.00	0.07
I -Q	0.02	359	C	0.02	0.00
Webs					
O -P	0.05	189	T		
P -W	0.10	502	C		
W -X	0.17	841	C		
W -I	0.19	938	C		
X -I	0.34	1074	T		
I -R	0.79	1178	C		
R -N	0.03	147	T		
N -S	0.06	229	T		
S -M	0.39	423	C		
M -B	0.14	489	T		
B -L	0.24	597	C		1 Br
L -C	0.12	138	C		
L -T	0.30	963	T		
T -K	0.35	1631	C		1 Br
K -U	0.12	357	C		
U -H	0.02	60	T		
H -V	0.04	179	T		
H -Z	0.21	800	C		
Y -Z	0.03	140	T		
G -D	0.03	150	T		
D -F	0.06	274	C		
F -E	0.02	153	C		

LL Defl -0.09" in Y -G L/999
TL Defl -0.20" in Y -G L/582
Shear // Grain in H -AA 0.29

Hz Disp	LL	DL	TL
Jt AA	0.02"	0.02"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
P	LOCK	4.0x 5.0-0.5	0.2		0.56
X	LOCK	4.0x 5.0-0.5	0.2		0.56
Q	LOCK	10.0x12.0-0.4	1.0		0.89
R	LOCK	4.0x 5.0-0.5	0.2		0.87
S	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0-0.5	Ctr		0.76
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
T	LOCK	4.0x 5.0	0.5	0.2	0.60
U	LOCK	8.0x12.0	0.6	1.5	0.97
V	LOCK	3.0x 5.0	Ctr	Ctr	0.41
Z	LOCK	4.0x 5.0	0.5	0.2	0.60
D#	LOCK	10.0x10.0	1.0	0.9	0.65
E	LOCK	6.0x 6.0-1.4	0.9		0.92
O	LOCK	3.0x 5.0	Ctr	Ctr	0.41
W	LOCK	4.0x 8.0	Ctr	Ctr	0.50
J	LOCK	3.0x 5.0	Ctr	Ctr	0.73
I	LOCK	7.0x12.0	Ctr	0.7	0.87
N	LOCK	4.0x 5.0	Ctr	Ctr	0.57
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
H	LOCK	8.0x12.0	Ctr	-1.5	0.96
Y	LOCK	3.0x 5.0	Ctr	Ctr	0.41
G	LOCK	3.0x 8.0-1.8-0.3			0.35
F	LOCK	6.0x10.0-1.8-0.4			0.94

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R59*	1	SP	610902	4.5	3- 0- 0	0	

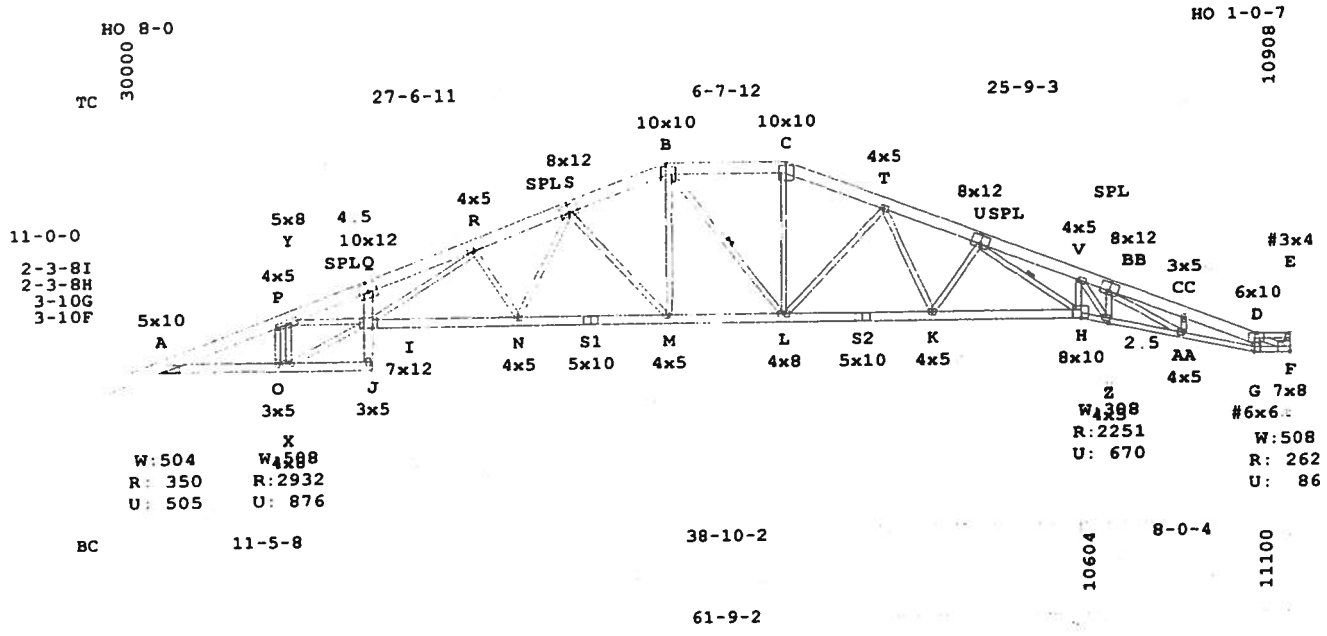
Office and Care Center, Hospice, M.M. Parrish

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1631 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R60*	1	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 626.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.20	2x 8	SP-2400
EX D -E		2x 4	SP-#2
BC	0.39	2x 6	SP-#2
EX H -G		2x 4	SP-#2
EX G -F		2x 4	SP-#2
CW	0.04	2x 6	SP-#2
WB	0.92	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on B -L
WB 1 rows CLB on U -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	351	506G	5- 4	1- 0
			Hz =	-203
X	2933	876	5- 8	3- 4
Z	2251	670	3- 8	2- 6
F	262	86	5- 8	1- 0
			Hz =	205

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -P	0.20	1796 T	0.06	0.14
P -Y	0.19	1695 T	0.05	0.14
Y -Q	0.09	1348 C	0.00	0.09
Q -R	0.05	1383 C	0.00	0.05
R -S	0.05	2565 C	0.03	0.02
S -B	0.05	2210 C	0.02	0.03
B -C	0.07	2037 C	0.00	0.07
C -T	0.05	2178 C	0.02	0.03
T -U	0.04	2337 C	0.00	0.04
U -V	0.07	809 C	0.00	0.07
V -BB	0.10	503 T	0.01	0.09
BB-CC	0.09	426 T	0.00	0.09
CC-D	0.06	336 C	0.00	0.06
D -E	0.06	0 T	0.00	0.06
Bottom Chords				
A -O	0.14	1635 C	0.00	0.14
O -X	0.21	1633 C	0.00	0.21
X -J	0.21	23 C	0.00	0.21
I -N	0.38	2380 T	0.28	0.10
N -S1	0.36	2344 T	0.28	0.08
S1-M	0.36	2344 T	0.28	0.08
M -L	0.34	2066 T	0.25	0.09
L -S2	0.36	2228 T	0.27	0.09
S2-K	0.35	2228 T	0.27	0.08
K -H	0.39	2065 T	0.25	0.14
H -Z	0.31	662 T	0.11	0.20
Z -AA	0.18	491 C	0.00	0.18
AA-G	0.16	355 T	0.04	0.12
G -F	0.13	362 T	0.06	0.07
Chord-Webs				
J -I	0.04	38 T	0.00	0.04
I -Q	0.02	243 C	0.02	0.00
Webs				
O -P	0.01	66 T		
P -X	0.10	606 C		
X -Y	0.22	1238 C		
X -I	0.65	1842 C		
Y -I	0.92	2890 T		
I -R	0.90	1338 C		
R -N	0.04	150 T		
N -S	0.06	227 T		
S -M	0.38	411 C		
M -B	0.14	477 T		
B -L	0.04	125 C		1 Br
L -C	0.13	442 T		
L -T	0.26	288 C		
T -K	0.04	102 T		
K -U	0.08	304 T		
U -H	0.36	1655 C		1 Br
H -V	0.35	1103 T		
V -Z	0.39	2137 C		
Z -BB	0.08	486 C		
BB-AA	0.32	1003 T		
AA-CC	0.07	305 T		
G -D	0.03	123 T		
D -F	0.07	388 C		
F -E	0.01	76 C		

LL Defl -0.11" in S1-M L/999
TL Defl -0.24" in S1-M L/999
Shear // Grain in P -Y 0.20

Hz Disp	LL	DL	TL
Jt Z	0.06"	0.06"	0.11"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
P	LOCK	4.0x 5.0-0.5	0.2	0.58	
Y	LOCK	5.0x 8.0	0.5	0.2	0.97
Q	LOCK	10.0x12.0-0.4	1.0	0.89	
R	LOCK	4.0x 5.0-0.5	0.2	0.87	
S	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	10.0x10.0-0.5	Ctr	0.76	
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
T	LOCK	4.0x 5.0	0.5	0.2	0.60
U	LOCK	8.0x12.0	0.6	1.5	0.97
V	LOCK	4.0x 5.0	0.5	0.2	0.58
BB	LOCK	8.0x12.0	0.6	1.5	0.97
CC	LOCK	3.0x 5.0	Ctr	Ctr	0.41
D	LOCK	6.0x10.0	1.0-3.0	0.90	
E#	LOCK	3.0x 4.0	0.1	0.6	0.64
O	LOCK	3.0x 5.0	Ctr	Ctr	0.41
X	LOCK	4.0x 8.0	1.5	Ctr	0.60
J	LOCK	3.0x 5.0	Ctr	Ctr	0.73
I	LOCK	7.0x12.0	Ctr	0.8	0.94
N	LOCK	4.0x 5.0	Ctr	Ctr	0.57
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
H	LOCK	8.0x10.0-0.5	0.4	0.80	
Z	LOCK	4.0x 5.0	Ctr	Ctr	0.53
AA	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G#	LOCK	6.0x 6.0	1.1	3.1	0.75
F	LOCK	7.0x 8.0-0.5	Ctr	0.77	

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES

Robbins Engineering Inc /Online Plus™ © 1996-2005 Version 18 0 020 Engineering - Portrait 01/26/06 9 45 24 AM Page 1 of 2

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R60*	1	SP	610902	4.5	3- 0- 0	0	

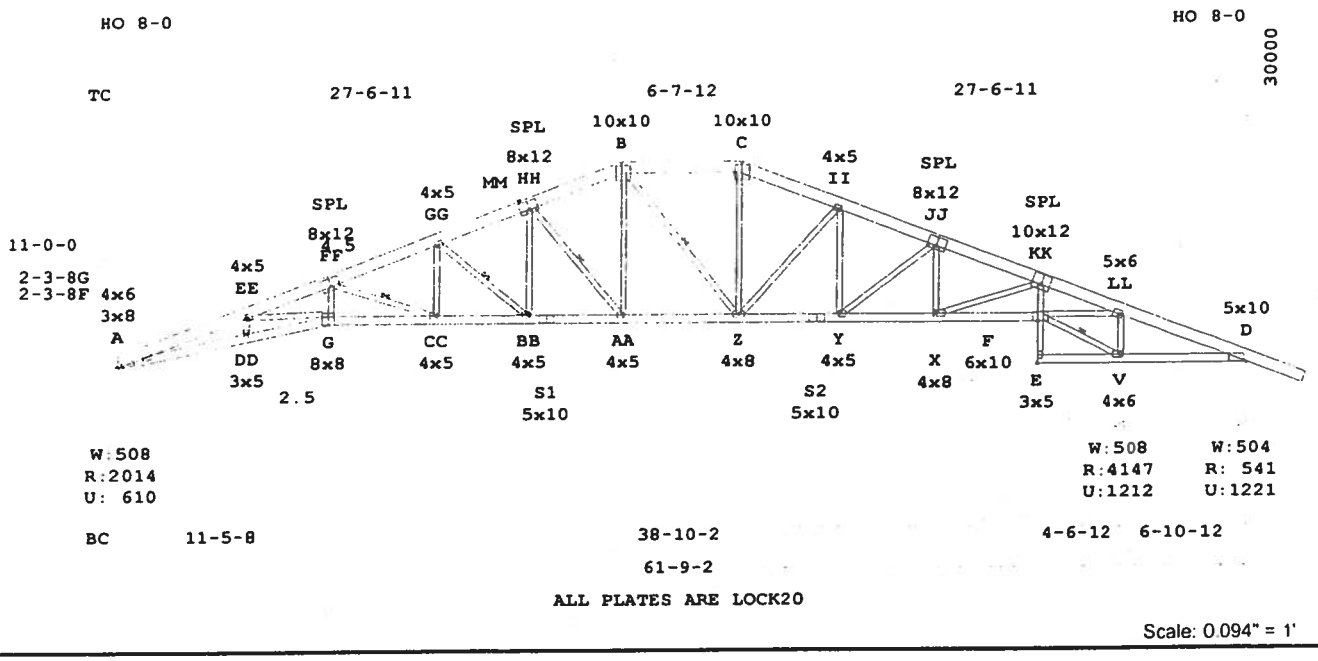
Office and Care Center, Hospice, M.M. Parrish

This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2565 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R61*	1	SP	610902	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 635.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.52	2x 6	SP-#2
EX A -G		2x 6	SP-2400
EX G -S1		2x 6	SP-2400
CW	0.34	2x 4	SP-#3
WB	0.83	2x 4	SP-#3
EX F -LL		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on FF-CC
WB 1 rows CLB on GG-BB
WB 1 rows CLB on HH-AA
WB 1 rows CLB on B -Z
WB 1 rows CLB on F -V
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2015	611	5- 8	1-11
			Hz =	-212
V	4147	1213	5- 8	4- 9
D	541	1221G	5- 4	1- 0
			Hz =	213

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -EE	0.28	8419 C	0.08		0.20
EE-FF	0.29	8571 C	0.08		0.21
FF-GG	0.15	5563 C	0.03		0.12
GG-HH	0.07	4242 C	0.04		0.03
HH-B	0.07	3247 C	0.04		0.03
B -C	0.08	2696 C	0.00		0.08
C -II	0.06	2878 C	0.03		0.03
II-JJ	0.05	2936 C	0.01		0.04
JJ-KK	0.05	2497 C	0.03		0.02
KK-LL	0.19	275 T	0.00		0.19
LL-D	0.31	3660 T	0.12		0.19
Bottom Chords					
A -DD	0.54	7951 T	0.36		0.18
DD-G	0.54	7987 T	0.36		0.18
G -CC	0.40	7979 T	0.36		0.04
CC-BB	0.27	5245 T	0.24		0.03
BB-S1	0.18	3955 T	0.18		0.00
S1-AA	0.52	3956 T	0.48		0.04
AA-Z	0.43	3040 T	0.36		0.07
Z -S2	0.40	2749 T	0.33		0.07
S2-Y	0.35	2749 T	0.33		0.02
Y -X	0.33	2336 T	0.28		0.05
X -F	0.12	258 T	0.00		0.12
E -V	0.30	32 C	0.00		0.30
V -D	0.33	3366 C	0.03		0.30
Chord-Webs					
E -F	0.27	46 T	0.00		0.27
F -KK	0.34	1978 C	0.34		0.00
Webs					
DD-EE	0.03	136 T			
EE-G	0.07	242 T			
G -FF	0.57	1800 T			
FF-CC	0.59	2918 C			1 Br
CC-GG	0.35	1107 T			
GG-BB	0.36	1701 C			1 Br
BB-HH	0.38	1213 T			
HH-AA	0.38	1407 C			1 Br
AA-B	0.40	1193 T			
B -Z	0.22	551 C			1 Br
Z -C	0.21	628 T			
Z -II	0.12	153 T			
Y -II	0.11	226 C			
Y -JJ	0.16	527 T			
X -JJ	0.21	807 C			
X -KK	0.83	2597 T			
F -LL	0.59	3248 T			
F -V	0.68	3756 C			1 Br
V -LL	0.38	2146 C			

LL Defl -0.52" in G -CC L/999
TL Defl -1.06" in G -CC L/613
Shear // Grain in A -EE 0.20

Hz Disp	LL	DL	TL
Jt V	0.23"	0.23"	0.47"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	3.0x 8.0	Ctr	Ctr	0.91
A LOCK	4.0x 6.0	Ctr	Ctr	0.73
EE LOCK	4.0x 5.0-0.5	0.2		0.62
FF LOCK	8.0x12.0-0.6	1.5		0.97
GG LOCK	4.0x 5.0-0.5	0.2		0.56
HH LOCK	8.0x12.0-0.6	1.5		0.97
B LOCK	10.0x10.0-0.5	Ctr		0.76
C LOCK	10.0x10.0	Ctr	Ctr	0.76
II LOCK	4.0x 5.0	0.5	0.2	0.56
JJ LOCK	8.0x12.0	0.6	1.5	0.97
KK LOCK	10.0x12.0	0.6	1.7	0.83
LL LOCK	5.0x 6.0	Ctr	Ctr	0.93
D LOCK	5.0x10.0	Ctr	Ctr	0.93
DD LOCK	3.0x 5.0	Ctr	Ctr	0.41
G LOCK	8.0x 8.0	Ctr-1.5		0.90
CC LOCK	4.0x 5.0	Ctr	Ctr	0.83
BB LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1 LOCK	5.0x10.0	Ctr	Ctr	0.82
AA LOCK	4.0x 5.0	Ctr	Ctr	0.60
Z LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2 LOCK	5.0x10.0	Ctr	Ctr	0.82
Y LOCK	4.0x 5.0	Ctr	Ctr	0.60
X LOCK	4.0x 8.0	0.5	Ctr	0.97
F LOCK	6.0x10.0	Ctr	0.2	0.90
E LOCK	3.0x 5.0	Ctr	Ctr	0.73
V LOCK	4.0x 6.0	Ctr	Ctr	0.84

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8571 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R61*	1	SP	610902	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

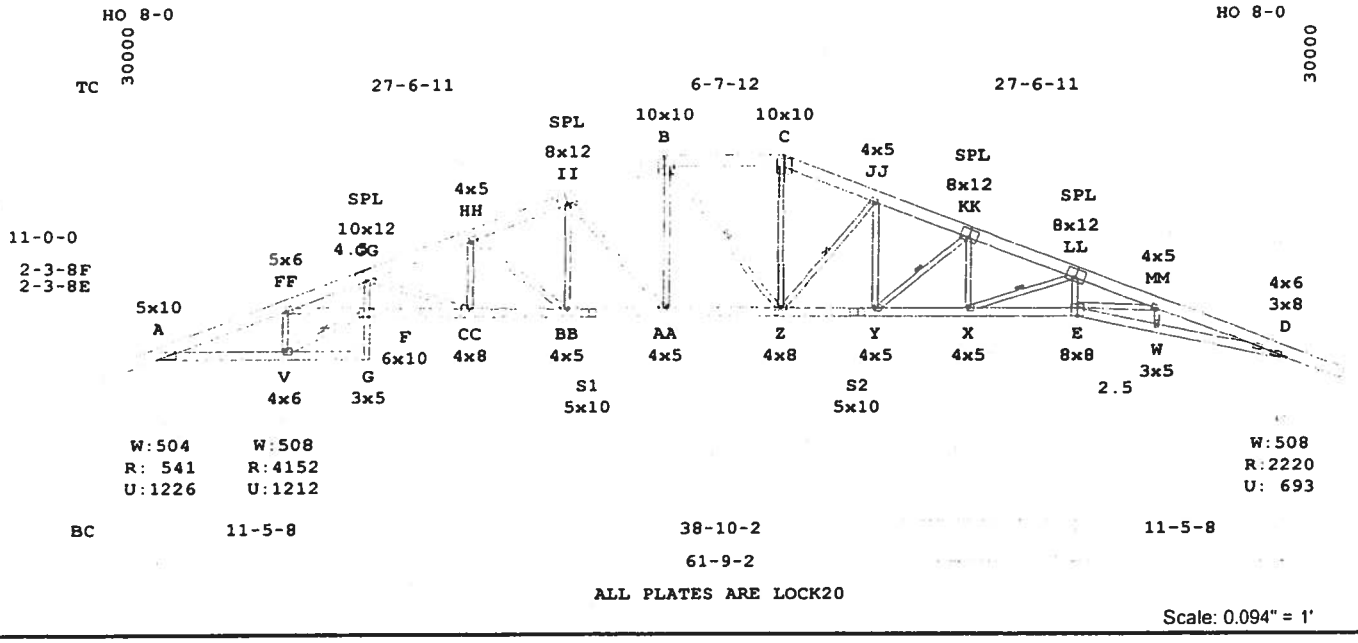
Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R62*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 647.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.54	2x 6	SP-#2
EX	S2-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.34	2x 4	SP-#3
WB	0.83	2x 4	SP-#3
EX	FF-F	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on V -F
WB 1 rows CLB on B -Z
WB 1 rows CLB on Z -JJ
WB 1 rows CLB on Y -KK
WB 1 rows CLB on X -LL
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	541	1226G	5- 4	1- 0
			Hz =	-212
V	4152	1213	5- 8	4- 9
D	2220	694	5- 8	1-13
			Hz =	213

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -FF	0.31	3671 T	0.12	0.19
FF-GG	0.19	272 T	0.00	0.19
GG-HH	0.05	2491 C	0.03	0.02
HH-II	0.05	2936 C	0.03	0.02
II-B	0.06	2871 C	0.03	0.03
B -C	0.07	3046 C	0.01	0.06
C -JJ	0.07	3249 C	0.04	0.03
JJ-KK	0.07	4245 C	0.05	0.02
KK-LL	0.15	5561 C	0.03	0.12
LL-MM	0.29	8571 C	0.08	0.21
MM-D	0.28	8419 C	0.08	0.20
--------Bottom Chords---------				
A -V	0.33	3376 C	0.03	0.30
V -G	0.30	32 C	0.00	0.30
F -CC	0.12	247 T	0.00	0.12
CC-BB	0.33	2330 T	0.28	0.05
BB-S1	0.35	2751 T	0.33	0.02
S1-AA	0.37	2751 T	0.33	0.04
AA-Z	0.38	2685 T	0.32	0.06
Z -S2	0.54	3964 T	0.48	0.06
S2-Y	0.18	3964 T	0.18	0.00
Y -X	0.27	5242 T	0.24	0.03
X -E	0.40	7980 T	0.36	0.04
E -W	0.54	7987 T	0.36	0.18
W -D	0.54	7951 T	0.36	0.18
----------Chord-Webs----------				
G -F	0.27	46 T	0.00	0.27
F -GG	0.34	1983 C	0.34	0.00
------------Webs------------				
V -FF	0.38	2146 C		
V -F	0.68	3766 C		1 Br
FF-F	0.59	3247 T		
GG-CC	0.83	2601 T		
CC-HH	0.21	811 C		
HH-BB	0.17	537 T		
BB-II	0.11	223 C		
II-AA	0.11	158 T		
AA-B	0.07	280 T		
B -Z	0.18	580 T		1 Br
Z -C	0.26	780 T		
Z -JJ	0.39	1433 C		1 Br
Y -JJ	0.38	1192 T		
Y -KK	0.36	1685 C		1 Br
X -KK	0.35	1112 T		
X -LL	0.60	2922 C		1 Br
E -LL	0.57	1800 T		
E -MM	0.07	242 T		
W -MM	0.03	136 T		

LL Defl -0.52" in X -E L/999
TL Defl -1.05" in X -E L/621
Shear // Grain in MM-D 0.20

Hz Disp	LL	DL	TL
Jt D	0.22"	0.22"	0.44"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	5.0x10.0	Ctr	Ctr	0.93
FF LOCK	5.0x 6.0	Ctr	Ctr	0.93
GG LOCK	10.0x12.0	-0.6	1.7	0.83
HH LOCK	4.0x 5.0	-0.5	0.2	0.56
II LOCK	8.0x12.0	-0.6	1.5	0.97
B LOCK	10.0x10.0	-0.5	Ctr	0.76
C LOCK	10.0x10.0	Ctr	Ctr	0.76
JJ LOCK	4.0x 5.0	0.5	0.2	0.59
KK LOCK	8.0x12.0	0.6	1.5	0.97
LL LOCK	8.0x12.0	0.6	1.5	0.97
MM LOCK	4.0x 5.0	0.5	0.2	0.62
D LOCK	3.0x 8.0	Ctr	Ctr	0.91
D LOCK	4.0x 6.0	Ctr	Ctr	0.73
V LOCK	4.0x 6.0	Ctr	Ctr	0.85
G LOCK	3.0x 5.0	Ctr	Ctr	0.73
F LOCK	6.0x10.0	Ctr	0.2	0.90
CC LOCK	4.0x 8.0	-0.5	Ctr	0.97
BB LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1 LOCK	5.0x10.0	Ctr	Ctr	0.82
AA LOCK	4.0x 5.0	Ctr	Ctr	0.60
Z LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2 LOCK	5.0x10.0	Ctr	Ctr	0.82
Y LOCK	4.0x 5.0	Ctr	Ctr	0.60
X LOCK	4.0x 5.0	Ctr	Ctr	0.83
E LOCK	8.0x 8.0	Ctr	-1.5	0.90
W LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8571 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R62*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

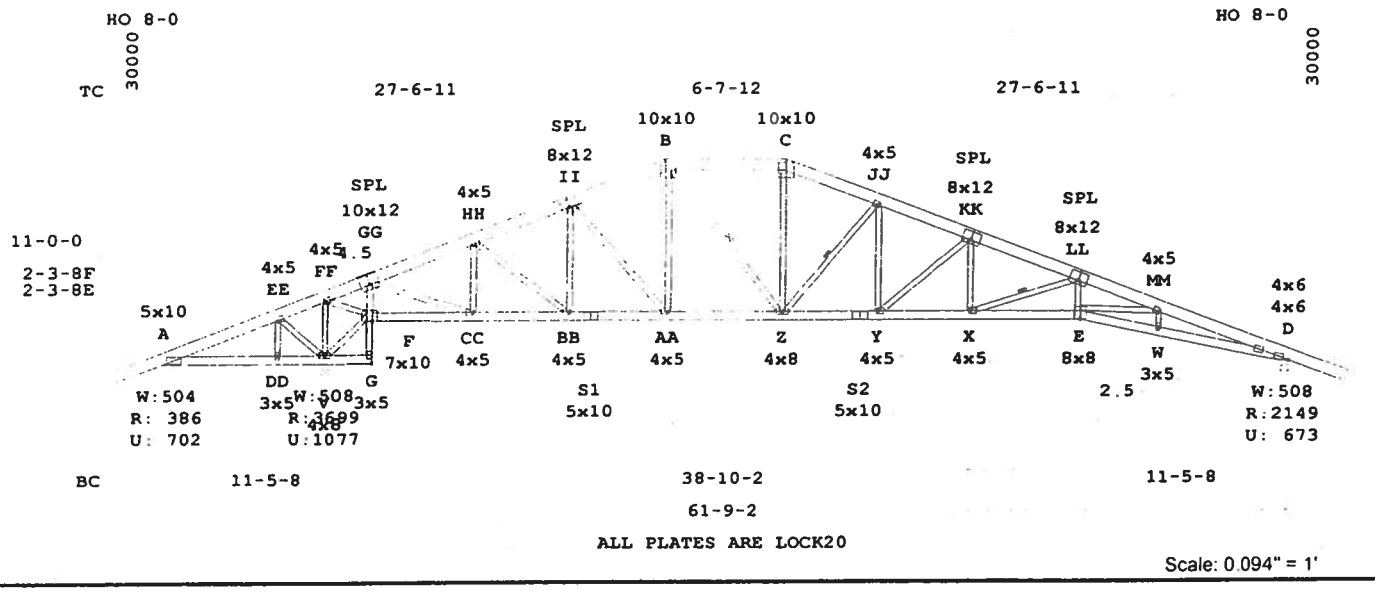
Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R63*	3	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 651.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.27	2x 8	SP-2400
BC	0.98	2x 6	SP-#2
EX E -D		2x 6	SP-2400
CW	0.48	2x 4	SP-#3
WB	0.99	2x 4	SP-#3
EX GG-CC		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -Z
WB 1 rows CLB on Z -JJ
WB 1 rows CLB on X -LL
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	386	703G	5- 4	1- 0
			Hz =	-212
V	3700	1078	5- 8	4- 1
D	2149	673	5- 8	1-12
			Hz =	213

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -EE	0.18	2352 T	0.08	0.10
EE-FF	0.19	2747 T	0.09	0.10
FF-GG	0.16	1833 T	0.06	0.10
GG-HH	0.10	1698 C	0.00	0.10
HH-II	0.05	2456 C	0.03	0.02
II-B	0.06	2559 C	0.03	0.03
B -C	0.06	2807 C	0.00	0.06
C -JJ	0.06	2995 C	0.03	0.03
JJ-KK	0.06	3977 C	0.04	0.02
KK-LL	0.15	5262 C	0.03	0.12
LL-MM	0.27	8168 C	0.07	0.20
MM-D	0.26	8075 C	0.07	0.19
Bottom Chords				
A -DD	0.15	2158 C	0.01	0.14
DD-V	0.15	2158 C	0.01	0.14
V -G	0.11	50 C	0.00	0.11
F -CC	0.16	1662 C	0.00	0.16
CC-BB	0.24	1567 T	0.19	0.05
BB-S1	0.31	2306 T	0.28	0.03
S1-AA	0.32	2306 T	0.28	0.04
AA-Z	0.35	2392 T	0.29	0.06
Z -S2	0.51	3714 T	0.45	0.06
S2-Y	0.46	3714 T	0.45	0.01
Y -X	0.65	4962 T	0.60	0.05
X -E	0.98	7603 T	0.92	0.06
E -W	0.52	7662 T	0.35	0.17
W -D	0.51	7628 T	0.34	0.17
Chord-Webs				
G -F	0.42	29 T	0.00	0.42
F -GG	0.48	2776 C	0.48	0.00
Webs				
DD-EE	0.05	219 T		
EE-V	0.12	599 C		
V -FF	0.16	788 C		
V -F	0.76	3503 C		
FF-F	0.31	996 T		
GG-CC	0.63	3436 T		
CC-HH	0.29	1105 C		
HH-BB	0.30	943 T		
BB-II	0.25	475 C		
II-AA	0.06	181 T		
AA-B	0.05	152 T		
B -Z	0.21	666 T		1 Br
Z -C	0.23	696 T		
Z -JJ	0.38	1415 C		1 Br
Y -JJ	0.37	1167 T		
Y -KK	0.99	1647 C		
X -KK	0.34	1080 T		
X -LL	0.57	2818 C		1 Br
E -LL	0.55	1731 T		
E -MM	0.05	180 T		
W -MM	0.03	133 T		

LL Defl -0.49" in X -E L/999
TL Defl -0.99" in X -E L/630
Shear // Grain in MM-D 0.20

Hz Disp	LL	DL	TL
Jt D	0.19"	0.20"	0.39"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
EE	LOCK	4.0x 5.0-0.5	0.2		0.56
FF	LOCK	4.0x 5.0-0.5	0.2		0.56
GG	LOCK	10.0x12.0-0.6	1.7		0.80
HH	LOCK	4.0x 5.0-0.5	0.2		0.56
II	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0-0.5	Ctr		0.76
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
JJ	LOCK	4.0x 5.0	0.5	0.2	0.58
KK	LOCK	8.0x12.0	0.6	1.5	0.97
LL	LOCK	8.0x12.0	0.6	1.5	0.97
MM	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	4.0x 6.0	Ctr	Ctr	0.99
D	LOCK	4.0x 6.0	Ctr	Ctr	0.98
DD	LOCK	3.0x 5.0	Ctr	Ctr	0.41
V	LOCK	4.0x 8.0	Ctr	Ctr	0.73
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x10.0	Ctr	0.8	0.76
CC	LOCK	4.0x 5.0-0.5	Ctr		0.98
BB	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
AA	LOCK	4.0x 5.0	Ctr	Ctr	0.60
Z	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.60
X	LOCK	4.0x 5.0	Ctr	Ctr	0.80
E	LOCK	8.0x 8.0	Ctr	-1.5	0.92
W	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8168 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for

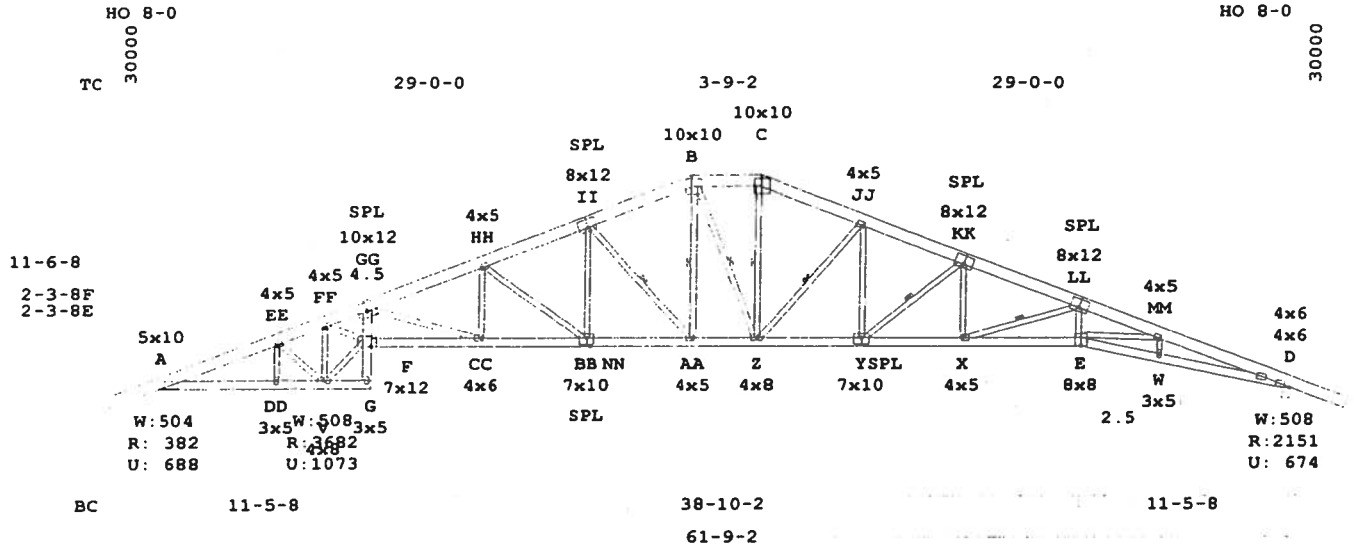
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R63*	3	SP	610902	4.5	3- 0- 0	3- 0- 0	
Office and Care Center, Hospice, M.M. Parrish								

further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R64*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 663.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.27	2x 8	SP-2400
BC	0.98	2x 6	SP-#2
EX	E -D	2x 6	SP-2400
CW	0.24	2x 6	SP-#2
WB	0.72	2x 4	SP-#3
EX	GG-CC	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on II-AA
WB 1 rows CLB on AA-B
WB 1 rows CLB on B -Z
WB 1 rows CLB on Z -C
WB 1 rows CLB on Z -JJ
WB 1 rows CLB on Y -KK
WB 1 rows CLB on X -LL
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	382	688G	5- 4	1- 0
			Hz =	-223
V	3683	1073	5- 8	4- 1
D	2152	674	5- 8	1-13
			Hz =	224

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
		Top Chords		
A -EE	0.18	2316 T	0.08	0.10
EE-FF	0.19	2714 T	0.09	0.10
FF-GG	0.18	1803 T	0.06	0.12
GG-HH	0.12	1846 C	0.00	0.12
HH-II	0.06	2528 C	0.03	0.03
II-B	0.06	2539 C	0.03	0.03
B -C	0.03	2584 C	0.03	0.00
C -JJ	0.06	2760 C	0.03	0.03
JJ-KK	0.07	3793 C	0.01	0.06
KK-LL	0.16	5133 C	0.03	0.13
LL-MM	0.27	8180 C	0.07	0.20
MM-D	0.26	8090 C	0.07	0.19
		Bottom Chords		
A -DD	0.15	2124 C	0.01	0.14
DD-V	0.15	2124 C	0.01	0.14
V -G	0.11	106 C	0.00	0.11
F -CC	0.26	1626 C	0.00	0.26
CC-BB	0.27	1706 T	0.20	0.07
BB-AA	0.32	2375 T	0.28	0.04
AA-Z	0.32	2372 T	0.28	0.04
Z -Y	0.47	3543 T	0.43	0.04
Y -X	0.63	4841 T	0.58	0.05
X -E	0.98	7614 T	0.92	0.06
E -W	0.52	7677 T	0.35	0.17
W -D	0.52	7643 T	0.35	0.17
		Chord-Webs		
G -F	0.24	30 T	0.00	0.24
F -GG	0.20	2786 C	0.20	0.00
		Webs		
DD-EE	0.05	219 T		
EE-V	0.13	611 C		
V -FF	0.15	754 C		
V -F	0.72	3448 C		
FF-F	0.32	1023 T		
GG-CC	0.66	3503 T		
CC-HH	0.29	1028 C		
HH-BB	0.26	841 T		
BB-II	0.22	375 C		
II-AA	0.03	158 T		1 Br
AA-B	0.04	168 T		1 Br
B -Z	0.18	568 T		1 Br
Z -C	0.22	690 T		1 Br
Z -JJ	0.44	1471 C		1 Br
Y -JJ	0.37	1173 T		
Y -KK	0.39	1679 C		1 Br
X -KK	0.34	1062 T		
X -LL	0.64	2932 C		1 Br
E -LL	0.55	1734 T		
E -MM	0.05	177 T		
W -MM	0.03	132 T		

LL Defl -0.50" in X -E L/999
TL Defl -1.01" in X -E L/620
Shear // Grain in MM-D 0.20
Hz Disp LL DL TL
Jt D 0.20" 0.20" 0.40"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
EE	LOCK	4.0x 5.0-0.5	0.2	0.56	
FF	LOCK	4.0x 5.0-0.5	0.2	0.56	
GG	LOCK	10.0x12.0-0.4	1.0	0.89	
HH	LOCK	4.0x 5.0-0.5	0.2	0.56	
II	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	10.0x10.0-0.5	Ctr	0.76	
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
JJ	LOCK	4.0x 5.0	0.5	0.2	0.58
KK	LOCK	8.0x12.0	0.6	1.5	0.97
LL	LOCK	8.0x12.0	0.6	1.5	0.97
MM	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	4.0x 6.0	Ctr	Ctr	0.99
D	LOCK	4.0x 6.0	Ctr	Ctr	0.98
DD	LOCK	3.0x 5.0	Ctr	Ctr	0.41
V	LOCK	4.0x 8.0	Ctr	Ctr	0.73
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.78
CC	LOCK	4.0x 6.0	Ctr	Ctr	0.99
BB	LOCK	7.0x10.0	Ctr-0.8		0.97
AA	LOCK	4.0x 5.0	Ctr	Ctr	0.60
Z	LOCK	4.0x 8.0	Ctr	Ctr	0.61
Y	LOCK	7.0x10.0	Ctr-0.8		0.97
X	LOCK	4.0x 5.0	Ctr	Ctr	0.84
E	LOCK	8.0x 8.0	Ctr-1.5		0.93
W	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8180 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering Cont.*
060039	*R64*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

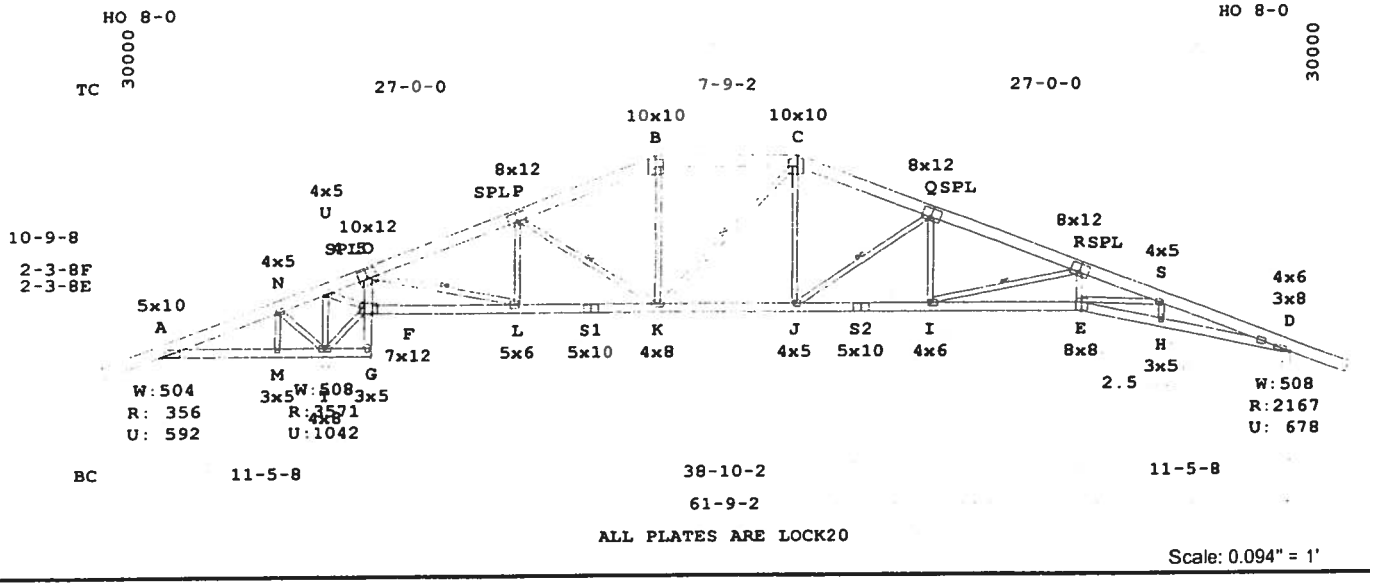
Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R65	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 625.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.27	2x 8	SP-2400
BC	0.60	2x 6	SP-#2
EX	S2-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.23	2x 6	SP-#2
WB	0.98	2x 4	SP-#3
EX	O -L	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on O -L
WB 1 rows CLB on P -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -Q
WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	356	592G	5- 4	1- 0
			Hz =	-208
T	3571	1043	5- 8	3-15
D	2167	678	5- 8	1-13
			Hz =	209

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.17	2070	T	0.07	0.10
N -U	0.18	2463	T	0.08	0.10
U -O	0.19	1356	T	0.04	0.15
O -P	0.15	2363	C	0.00	0.15
P -B	0.09	2670	C	0.03	0.06
B -C	0.11	2497	C	0.00	0.11
C -Q	0.10	3186	C	0.01	0.09
Q -R	0.17	4747	C	0.02	0.15
R -S	0.27	8290	C	0.07	0.20
S -D	0.26	8170	C	0.07	0.19
Bottom Chords					
A -M	0.14	1897	C	0.01	0.13
M -T	0.14	1897	C	0.01	0.13
T -G	0.10	100	C	0.00	0.10
F -L	0.29	1171	C	0.00	0.29
L -S1	0.34	2197	T	0.26	0.08
S1-K	0.38	2197	T	0.26	0.12
K -J	0.47	2960	T	0.35	0.12
J -S2	0.60	4476	T	0.54	0.06
S2-I	0.23	4476	T	0.20	0.03
I -E	0.39	7720	T	0.35	0.04
E -H	0.52	7752	T	0.35	0.17
H -D	0.52	7718	T	0.35	0.17
Chord-Webs					
G -F	0.23	29	T	0.00	0.23
F -O	0.19	2656	C	0.19	0.00
Webs					
M -N	0.05	213	T		
N -T	0.12	602	C		
T -U	0.18	871	C		
T -F	0.66	3129	C		
U -F	0.40	1249	T		
O -L	0.64	3472	T		1 Br
L -P	0.26	737	C		
P -K	0.11	359	T		1 Br
K -B	0.14	520	T		
K -C	0.29	665	C		1 Br
J -C	0.37	1182	T		
J -Q	0.59	1829	C		1 Br
I -Q	0.33	1033	T		
I -R	0.98	3359	C		1 Br
E -R	0.56	1752	T		
E -S	0.06	210	T		
H -S	0.03	127	T		

LL Defl -0.51" in I -E L/999
TL Defl -1.06" in I -E L/590
Shear // Grain in S -D 0.20
Hz Disp LL DL TL
Jt D 0.20" 0.21" 0.41"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
N	LOCK	4.0x 5.0-0.5	0.2		0.56
U	LOCK	4.0x 5.0-0.5	0.2		0.57
O	LOCK	10.0x12.0-0.4	1.0		0.89
P	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
C	LOCK	10.0x10.0	0.5	Ctr	0.76
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.91
D	LOCK	4.0x 6.0	Ctr	Ctr	0.67
M	LOCK	3.0x 5.0	Ctr	Ctr	0.41
T	LOCK	4.0x 8.0	Ctr	Ctr	0.66
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.78
L	LOCK	5.0x 6.0	Ctr	Ctr	0.91
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.52
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 6.0	Ctr	Ctr	0.82
E	LOCK	8.0x 8.0	Ctr	-1.5	0.90
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8290 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D

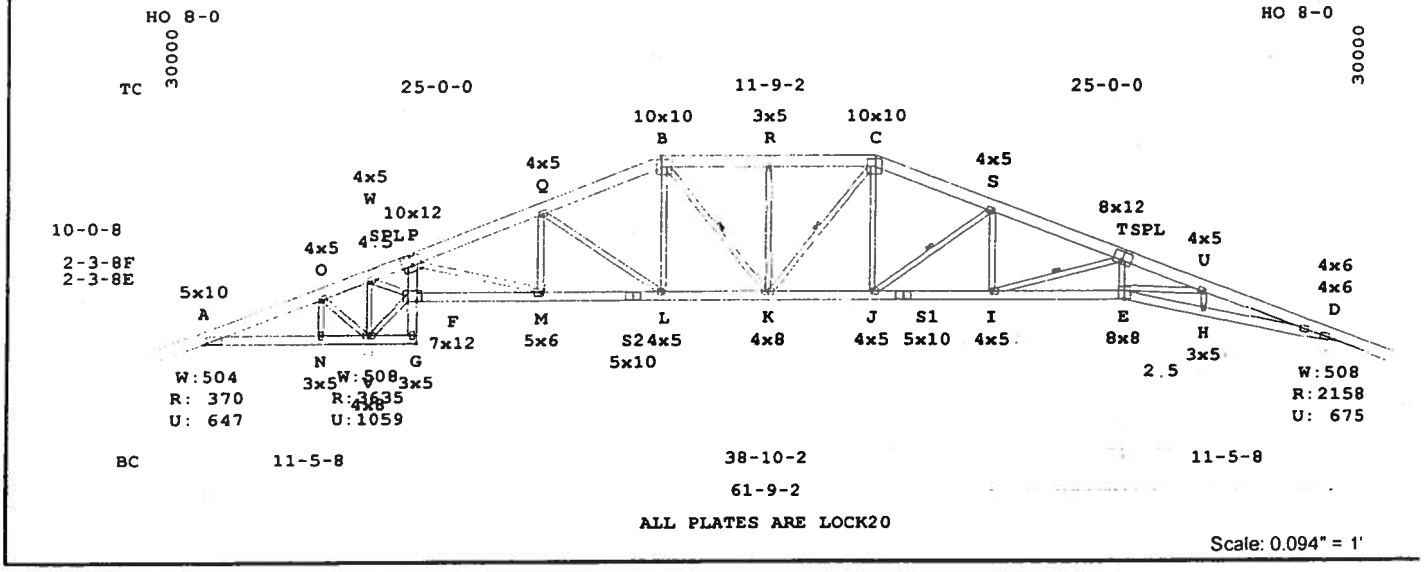
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R65*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R66	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 638.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.27	2x 8	SP-2400
BC	0.99	2x 6	SP-#2
EX E -D		2x 6	SP-2400
CW	0.24	2x 6	SP-#2
WB	0.76	2x 4	SP-#3
EX P -M		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -S
WB 1 rows CLB on I -T
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	370	648G	5- 4	1- 0
			Hz =	-192
V	3636	1059	5- 8	4- 0
D	2159	676	5- 8	1-13
			Hz =	193

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -O	0.17	2211 T		0.07	0.10
O -W	0.18	2609 T		0.08	0.10
W -P	0.18	1618 T		0.05	0.13
P -Q	0.13	2110 C		0.00	0.13
Q -B	0.08	2616 C		0.03	0.05
B -R	0.04	3055 C		0.01	0.03
R -C	0.06	3055 C		0.01	0.05
C -S	0.09	3507 C		0.04	0.05
S -T	0.17	4957 C		0.03	0.14
T -U	0.27	8221 C		0.07	0.20
U -D	0.26	8123 C		0.07	0.19
Bottom Chords					
A -N	0.14	2028 C		0.01	0.13
N -V	0.14	2028 C		0.01	0.13
V -G	0.10	103 C		0.00	0.10
F -M	0.27	1438 C		0.00	0.27
M -S2	0.30	1957 T		0.23	0.07
S2-L	0.30	1957 T		0.23	0.07
L -K	0.36	2447 T		0.29	0.07
K -J	0.43	3262 T		0.39	0.04
J -S1	0.60	4673 T		0.56	0.04
S1-I	0.61	4673 T		0.56	0.05
I -E	0.99	7653 T		0.93	0.06
E -H	0.52	7707 T		0.35	0.17
H -D	0.52	7672 T		0.35	0.17
Chord-Webs					
G -F	0.24	29 T		0.00	0.24
F -P	0.19	2731 C		0.19	0.00
Webs					
N -O	0.05	216 T			
O -V	0.13	607 C			
V -W	0.16	804 C			
V -F	0.70	3315 C			
W -F	0.35	1115 T			
P -M	0.84	3531 T			
M -Q	0.28	889 C			
Q -L	0.18	592 T			
L -B	0.13	188 C			
B -K	0.30	959 T			1 Br
K -R	0.27	384 C			
K -C	0.10	327 C			1 Br
J -C	0.37	1161 T			
J -S	0.47	1737 C			1 Br
I -S	0.33	1049 T			
I -T	0.76	3118 C			1 Br
E -T	0.55	1741 T			
E -U	0.06	187 T			
H -U	0.03	130 T			

LL Defl -0.51" in I -E L/999
TL Defl -1.04" in I -E L/599
Shear // Grain in U -D 0.20

Hz Disp	LL	DL	TL
Jt D	0.20"	0.20"	0.41"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
O	LOCK	4.0x 5.0	-0.5	0.2	0.56
W	LOCK	4.0x 5.0	-0.5	0.2	0.56
P	LOCK	10.0x12.0	-0.4	1.0	0.89
Q	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	10.0x10.0	-0.5	Ctr	0.76
R	LOCK	3.0x 5.0	Ctr	Ctr	0.41
C	LOCK	10.0x10.0	0.5	Ctr	0.76
S	LOCK	4.0x 5.0	0.5	0.2	0.56
T	LOCK	8.0x12.0	0.6	1.5	0.97
U	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	4.0x 6.0	Ctr	Ctr	1.00
D	LOCK	4.0x 6.0	Ctr	Ctr	0.99
N	LOCK	3.0x 5.0	Ctr	Ctr	0.41
V	LOCK	4.0x 8.0	Ctr	Ctr	0.70
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.78
M	LOCK	5.0x 6.0	Ctr	Ctr	0.93
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	4.0x 8.0	Ctr	Ctr	0.50
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.91
E	LOCK	8.0x 8.0	Ctr	-1.5	0.93
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8221 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

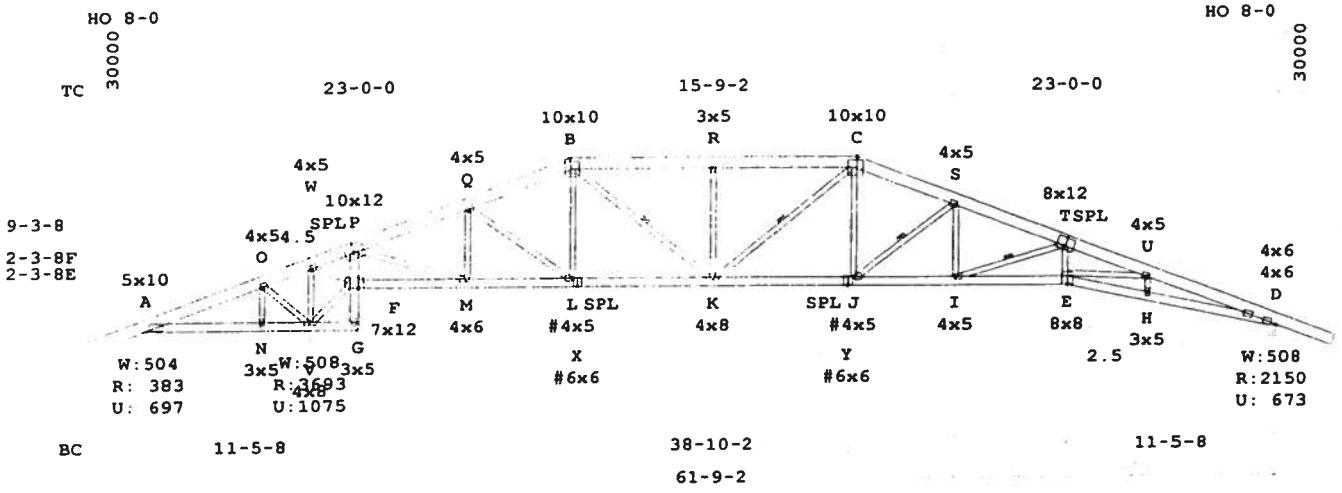
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R66*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R67	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 627.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.27	2x 8	SP-2400
BC	0.98	2x 6	SP-#2
EX E -D		2x 6	SP-2400
CW	0.25	2x 6	SP-#2
WB	0.73	2x 4	SP-#3
EX P -M		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on B -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -S
WB 1 rows CLB on I -T
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	384	697G	5- 4	1- 0
			Hz = -177	
V	3693	1075	5- 8	4- 1
D	2150	674	5- 8	1-12
			Hz = 177	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -O	0.18	2336 T	0.08		0.10
O -W	0.19	2739 T	0.09		0.10
W -P	0.18	1849 T	0.06		0.12
P -Q	0.12	1800 C	0.00		0.12
Q -B	0.07	2512 C	0.03		0.04
B -R	0.09	3402 C	0.01		0.08
R -C	0.10	3402 C	0.01		0.09
C -S	0.07	3869 C	0.04		0.03
S -T	0.16	5188 C	0.03		0.13
T -U	0.27	8180 C	0.07		0.20
U -D	0.26	8076 C	0.07		0.19
--------Bottom Chords--------					
A -N	0.15	2144 C	0.01		0.14
N -V	0.15	2144 C	0.01		0.14
V -G	0.11	107 C	0.00		0.11
F -M	0.27	1673 C	0.00		0.27
M -L	0.29	1664 T	0.20		0.09
L -X	0.37	2350 T	0.28		0.09
X -K	0.35	2350 T	0.28		0.07
K -Y	0.50	3607 T	0.43		0.07
Y -J	0.48	3607 T	0.43		0.05
J -I	0.64	4886 T	0.59		0.05
I -E	0.98	7615 T	0.92		0.06
E -H	0.52	7663 T	0.35		0.17
H -D	0.51	7628 T	0.34		0.17
----------Chord-Webs----------					
G -F	0.25	29 T	0.00		0.25
F -P	0.20	2792 C	0.20		0.00
------------Webs------------					
N -O	0.05	219 T			
O -V	0.13	612 C			
V -W	0.15	746 C			
V -F	0.73	3479 C			
W -F	0.31	996 T			
P -M	0.64	3518 T			
M -Q	0.29	1071 C			
Q -L	0.27	848 T			
L -B	0.19	344 C			
B -K	0.43	1349 T			1 Br
K -R	0.29	525 C			
K -C	0.10	264 C			1 Br
J -C	0.37	1156 T			
J -S	0.37	1618 C			1 Br
I -S	0.34	1074 T			
I -T	0.61	2899 C			1 Br
E -T	0.55	1732 T			
E -U	0.06	192 T			
H -U	0.03	133 T			

LL Defl -0.50" in I -E L/999
TL Defl -1.02" in I -E L/611
Shear // Grain in U -D 0.20

Hz Disp	LL	DL	TL
Jt D	0.20"	0.20"	0.40"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
O	LOCK	4.0x 5.0	-0.5	0.2	0.56
W	LOCK	4.0x 5.0	-0.5	0.2	0.56
P	LOCK	10.0x12.0	-0.4	1.0	0.89
Q	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	10.0x10.0	-0.5	Ctr	0.76
R	LOCK	3.0x 5.0	Ctr	Ctr	0.41
C	LOCK	10.0x10.0	0.5	Ctr	0.76
S	LOCK	4.0x 5.0	0.5	0.2	0.56
T	LOCK	8.0x12.0	0.6	1.5	0.97
U	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	4.0x 6.0	Ctr	Ctr	0.99
D	LOCK	4.0x 6.0	Ctr	Ctr	0.98
N	LOCK	3.0x 5.0	Ctr	Ctr	0.41
V	LOCK	4.0x 8.0	Ctr	Ctr	0.73
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.78
M	LOCK	4.0x 6.0	Ctr	Ctr	0.99
L#	LOCK	4.0x 5.0	-0.6	Ctr	0.77
X	LOCK	6.0x 6.0	0.2	0.3	0.76
K	LOCK	4.0x 8.0	Ctr	Ctr	0.55
Y	LOCK	6.0x 6.0	0.1	0.4	0.77
J#	LOCK	4.0x 5.0	0.9	0.4	0.92
I	LOCK	4.0x 5.0	Ctr	Ctr	0.83
E	LOCK	8.0x 8.0	Ctr	-1.5	0.93
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8180 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

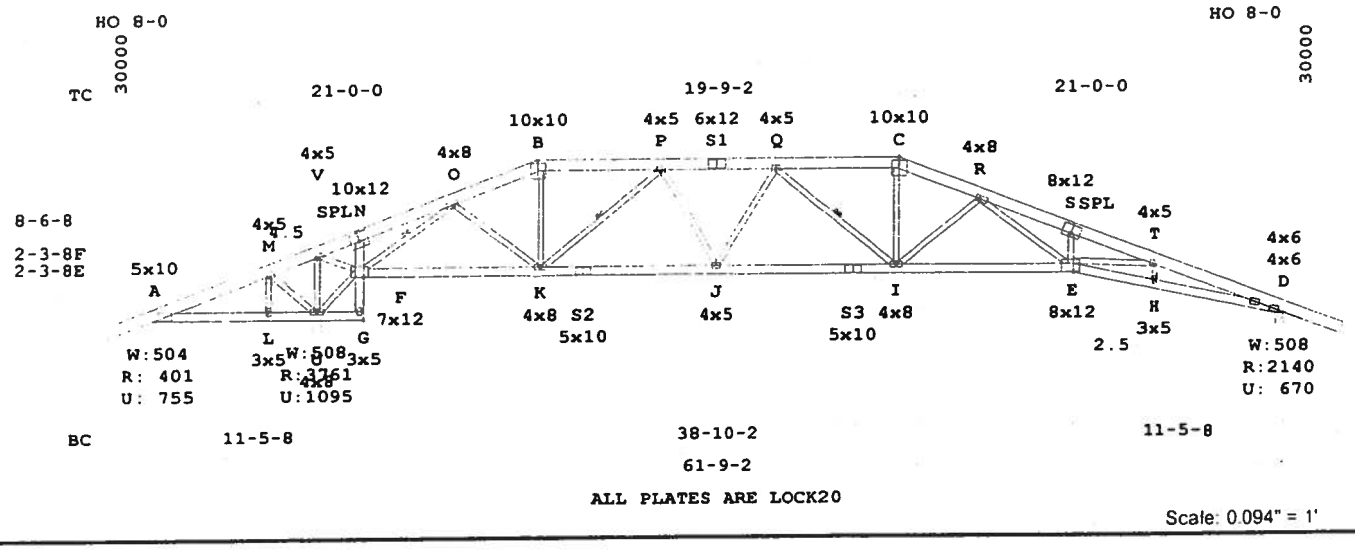
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R67*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R68*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 612.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.81	2x 6	SP-#2
EX E -D		2x 6	SP-2400
CW	0.26	2x 6	SP-#2
WB	0.80	2x 4	SP-#3
EX R -E		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on F -O
WB 1 rows CLB on K -P
WB 1 rows CLB on Q -I
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	401	756G	5- 4	1- 0
			Hz =	-161
U	3762	1095	5- 8	4- 2
D	2141	671	5- 8	1-12
			Hz =	162

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.19	2484 T	0.08		0.11
M -V	0.20	2901 T	0.09		0.11
V -N	0.23	2046 T	0.07		0.16
N -O	0.22	1900 T	0.06		0.16
O -B	0.06	2310 C	0.00		0.06
B -P	0.06	2179 C	0.00		0.06
P -S1	0.06	3706 C	0.04		0.02
S1-Q	0.06	3706 C	0.04		0.02
Q -C	0.07	4016 C	0.01		0.06
C -R	0.06	4293 C	0.04		0.02
R -S	0.19	8304 C	0.08		0.11
S -T	0.31	8314 C	0.08		0.23
T -D	0.29	7804 C	0.07		0.22
Bottom Chords					
A -L	0.15	2280 C	0.01		0.14
L -U	0.15	2280 C	0.01		0.14
U -G	0.11	121 C	0.00		0.11
F -K	0.42	1212 T	0.14		0.28
K -S2	0.49	3369 T	0.40		0.09
S2-J	0.51	3369 T	0.40		0.11
J -S3	0.58	3970 T	0.48		0.10
S3-I	0.67	3970 T	0.48		0.19
I -E	0.81	5152 T	0.62		0.19
E -H	0.58	7421 T	0.33		0.25
H -D	0.58	7360 T	0.33		0.25
Chord-Webs					
G -F	0.26	30 T	0.00		0.26
F -N	0.04	571 C	0.04		0.00
Webs					
L -M	0.05	221 T			
M -U	0.13	639 C			
U -V	0.13	647 C			
U -F	0.78	3691 C			
V -F	0.31	995 T			
F -O	0.80	3854 C		1 Br	
O -K	0.39	1242 T			
K -B	0.13	457 T			
K -P	0.48	1580 C		1 Br	
P -J	0.23	718 T			
J -Q	0.31	519 C			
Q -I	0.04	171 T		1 Br	
I -C	0.38	1196 T			
I -R	0.77	1511 C			
R -E	0.63	3339 T			
E -S	0.05	264 C			
E -T	0.18	583 T			
H -T	0.04	206 C			

LL Defl -0.50" in I -E L/999
TL Defl -1.05" in I -E L/593
Shear // Grain in T -D 0.20

Hz Disp	LL	DL	TL
Jt D	0.20"	0.20"	0.40"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
M	LOCK	4.0x 5.0-0.5	0.2		0.56
V	LOCK	4.0x 5.0-0.5	0.2		0.56
N	LOCK	10.0x12.0-0.4	1.0		0.89
O	LOCK	4.0x 8.0-1.0	Ctr		0.82
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.59
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
R	LOCK	4.0x 8.0	1.2-0.1		0.77
S	LOCK	8.0x12.0	0.6	1.5	0.97
T	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	4.0x 6.0	Ctr	Ctr	0.96
D	LOCK	4.0x 6.0	Ctr	Ctr	0.91
L	LOCK	3.0x 5.0	Ctr	Ctr	0.41
U	LOCK	4.0x 8.0	Ctr	Ctr	0.78
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.88
K	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
E	LOCK	8.0x12.0	Ctr	-1.5	0.89
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8314 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

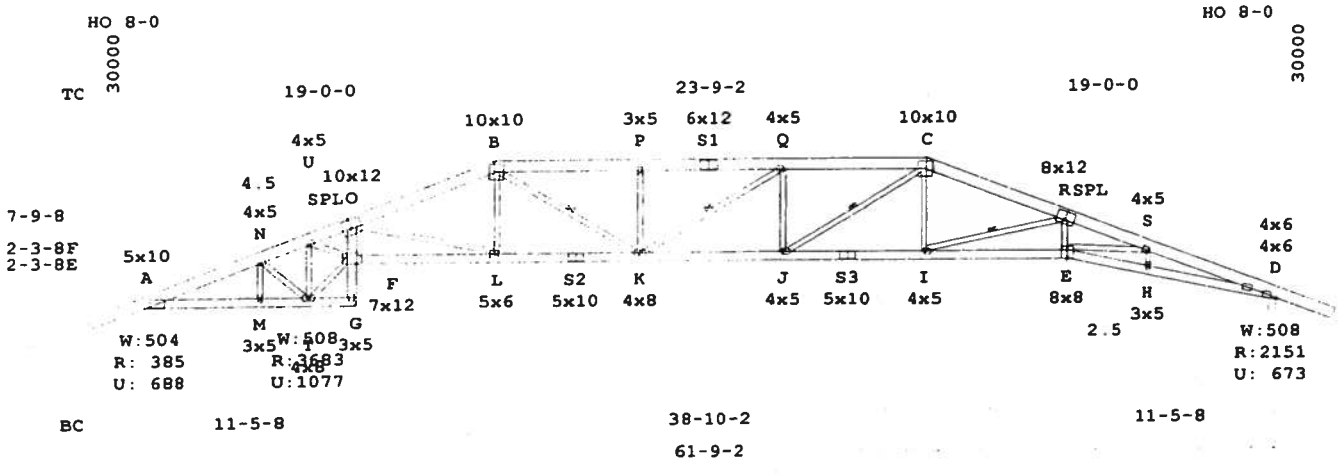
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R68*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	
Office and Care Center, Hospice, M.M. Parrish								

(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R69*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 605.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.27	2x 8	SP-2400
BC	0.65	2x 6	SP-#2
EX S3-E		2x 6	SP-2400
EX E -D		2x 6	SP-2400
CW	0.24	2x 6	SP-#2
WB	0.86	2x 4	SP-#3
EX O -L		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on B -K
WB 1 rows CLB on K -Q
WB 1 rows CLB on J -C
WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	386	688G	5- 4	1- 0
			Hz =	-146
T	3683	1077	5- 8	4- 1
D	2152	673	5- 8	1-13
			Hz =	146

G = Gravity Uplift

Membr	CSI	P Lbs	Ax	CSI	Bnd
Top Chords					
A -N	0.18	2311	T	0.08	0.10
N -U	0.19	2718	T	0.09	0.10
U -O	0.25	1714	T	0.06	0.19
O -B	0.19	2161	C	0.00	0.19
B -P	0.08	3860	C	0.01	0.07
P -S1	0.08	3860	C	0.01	0.07
S1-Q	0.08	3860	C	0.01	0.07
Q -C	0.10	4719	C	0.02	0.08
C -R	0.17	4808	C	0.02	0.15
R -S	0.27	8194	C	0.07	0.20
S -D	0.26	8086	C	0.07	0.19
Bottom Chords					
A -M	0.14	2120	C	0.01	0.13
M -T	0.14	2120	C	0.01	0.13
T -G	0.11	106	C	0.00	0.11
F -L	0.27	1502	C	0.00	0.27
L -S2	0.32	2004	T	0.24	0.08
S2-K	0.32	2004	T	0.24	0.08
K -J	0.65	4720	T	0.57	0.08
J -S3	0.60	4503	T	0.54	0.06
S3-I	0.24	4503	T	0.20	0.04
I -E	0.38	7630	T	0.34	0.04
E -H	0.52	7671	T	0.35	0.17
H -D	0.52	7637	T	0.35	0.17
Chord-Webs					
G -F	0.24	30	T	0.00	0.24
F -O	0.20	2857	C	0.20	0.00
Webs					
M -N	0.05	217	T		
N -T	0.13	617	C		
T -U	0.15	750	C		
T -F	0.73	3454	C		
U -F	0.37	1161	T		
O -L	0.95	3602	T		
L -B	0.26	766	C		
B -K	0.69	2167	T		1 Br
K -P	0.14	419	C		
K -Q	0.34	1017	C		1 Br
J -Q	0.06	224	T		
J -C	0.08	252	T		1 Br
I -C	0.33	1054	T		
I -R	0.86	3227	C		1 Br
E -R	0.55	1731	T		
E -S	0.06	197	T		
H -S	0.03	124	T		

LL Defl -0.53" in I -E L/999
TL Defl -1.09" in I -E L/572
Shear // Grain in S -D 0.20

Hz Disp	LL	DL	TL
Jt D	0.21"	0.21"	0.42"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
N	LOCK	4.0x 5.0	-0.5	0.2	0.56
U	LOCK	4.0x 5.0	-0.5	0.2	0.56
O	LOCK	10.0x12.0	-0.4	1.0	0.89
B	LOCK	10.0x10.0	-0.5	Ctr	0.77
P	LOCK	3.0x 5.0	Ctr	Ctr	0.41
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.76
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	4.0x 6.0	Ctr	Ctr	0.99
D	LOCK	4.0x 6.0	Ctr	Ctr	0.98
M	LOCK	3.0x 5.0	Ctr	Ctr	0.41
T	LOCK	4.0x 8.0	Ctr	Ctr	0.73
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.78
L	LOCK	5.0x 6.0	Ctr	Ctr	0.95
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.86
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.95
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8194 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593

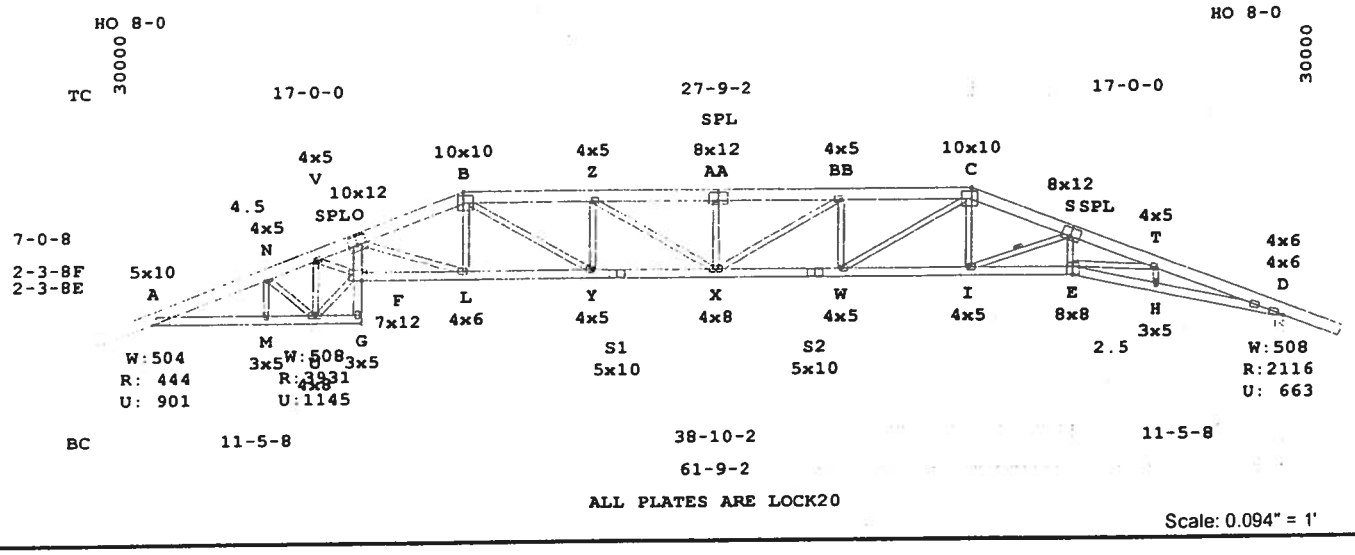
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R69*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R70	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 605.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.27	2x 8	SP-2400
BC	0.96	2x 6	SP-#2
EX E -D		2x 6	SP-2400
CW	0.29	2x 6	SP-#2
WB	0.88	2x 4	SP-#3
EX O -L		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on I -S
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	444	901G	5- 4	1- 0
			Hz =	-130
U	3932	1146	5- 8	4- 5
D	2116	664	5- 8	1-12
			Hz =	131

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.21	2852 T		0.10	0.11
N -V	0.22	3279 T		0.11	0.11
V -O	0.27	2633 T		0.09	0.18
O -B	0.18	1356 C		0.00	0.18
B -Z	0.10	3530 C		0.01	0.09
Z -AA	0.10	4926 C		0.02	0.08
AA-BB	0.07	4926 C		0.02	0.05
BB-C	0.10	5379 C		0.03	0.07
C -S	0.15	5227 C		0.03	0.12
S -T	0.27	7969 C		0.07	0.20
T -D	0.25	7906 C		0.07	0.18
Bottom Chords					
A -M	0.16	2622 C		0.01	0.15
M -U	0.16	2622 C		0.01	0.15
U -G	0.11	126 C		0.00	0.11
F -L	0.27	2404 C		0.01	0.26
L -Y	0.22	1246 T		0.15	0.07
Y -S1	0.47	3530 T		0.42	0.05
S1-X	0.50	3530 T		0.42	0.08
X -S2	0.72	5379 T		0.65	0.07
S2-W	0.68	5379 T		0.65	0.03
W -I	0.66	4899 T		0.59	0.07
I -E	0.96	7413 T		0.90	0.06
E -H	0.51	7500 T		0.34	0.17
H -D	0.51	7466 T		0.34	0.17
Chord-Webs					
G -F	0.29	31 C		0.00	0.29
F -O	0.22	3194 C			
Webs					
M -N	0.06	228 T			
N -U	0.13	643 C			
U -V	0.10	491 C			
U -F	0.88	4161 C			
V -F	0.24	770 T			
O -L	0.70	3827 T			
L -B	0.30	1141 C			
B -Y	0.87	2628 T			
Y -Z	0.31	1181 C			
Z -X	0.54	1643 T			
X -AA	0.11	442 C			
X -BB	0.46	532 C			
W -BB	0.04	188 T			
W -C	0.18	552 T			
I -C	0.35	1094 T			
I -S	0.52	2667 C			1 Br
E -S	0.54	1691 T			
E -T	0.04	147 T			
H -T	0.03	132 T			

LL Defl -0.55" in W -I L/999
TL Defl -1.12" in W -I L/560
Shear // Grain in T -D 0.19

Hz Disp	LL	DL	TL
Jt D	0.21"	0.21"	0.42"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
N	LOCK	4.0x 5.0-0.5	0.2	0.56	
V	LOCK	4.0x 5.0-0.5	0.2	0.56	
O	LOCK	10.0x12.0-0.4	1.0	0.89	
B	LOCK	10.0x10.0-0.5	Ctr	0.96	
Z	LOCK	4.0x 5.0	Ctr	0.5	0.78
AA	LOCK	8.0x12.0	Ctr	1.5	0.99
BB	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.76
S	LOCK	8.0x12.0	0.6	1.5	0.97
T	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	4.0x 6.0	Ctr	Ctr	0.97
D	LOCK	4.0x 6.0	Ctr	Ctr	0.94
M	LOCK	3.0x 5.0	Ctr	Ctr	0.41
U	LOCK	4.0x 8.0	Ctr	Ctr	0.88
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	7.0x12.0	Ctr	0.8	0.78
L	LOCK	4.0x 6.0-0.5	Ctr	0.92	
Y	LOCK	4.0x 5.0	Ctr	Ctr	1.00
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
X	LOCK	4.0x 8.0	Ctr	Ctr	0.65
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
W	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.76
E	LOCK	8.0x 8.0	Ctr-1.5	0.90	
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7969 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593

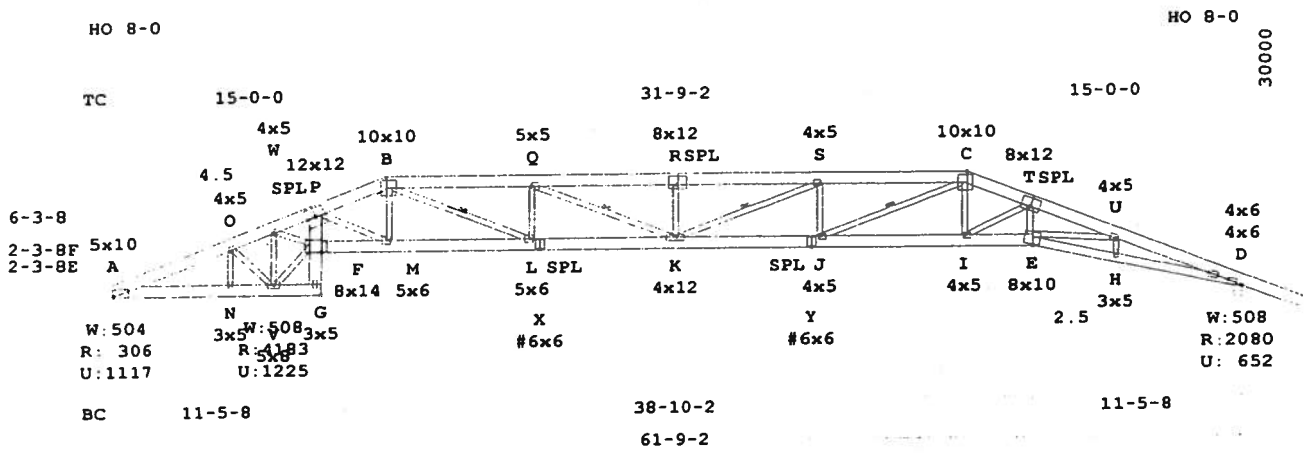
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R70*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R71*	1	SP	610902	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 629.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.33	2x 8	SP-2400
BC	0.28	2x 8	SP-2400
EX E -D		2x 6	SP-2400
CW	0.22	2x 8	SP-2400
WB	0.96	2x 4	SP-#3
EX P -M		2x 4	SP-#2
EX B -L		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -L
WB 1 rows CLB on Q -K
WB 1 rows CLB on K -S
WB 1 rows CLB on J -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	307	1117G	5- 4	1- 0
			Hz =	-115
V	4183	1225	5- 8	3- 4
D	2081	652	5- 8	1-12
			Hz =	115

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -O	0.21	3529	T	0.12	0.09
O -W	0.22	3879	T	0.13	0.09
W -P	0.33	3544	T	0.12	0.21
P -B	0.21	191	T	0.00	0.21
B -Q	0.16	3545	C	0.01	0.15
Q -R	0.15	5792	C	0.04	0.11
R -S	0.12	5792	C	0.04	0.08
S -C	0.14	6413	C	0.05	0.09
C -T	0.12	5931	C	0.04	0.08
T -U	0.28	7991	C	0.07	0.21
U -D	0.26	7584	C	0.06	0.20
Bottom Chords					
A -N	0.09	3268	C	0.01	0.08
N -V	0.10	3268	C	0.01	0.09
V -G	0.09	318	C	0.00	0.09
F -M	0.17	3305	C	0.01	0.16
M -L	0.11	198	T	0.00	0.11
L -X	0.18	3547	T	0.12	0.06
X -K	0.20	3547	T	0.12	0.08
K -Y	0.27	6415	T	0.22	0.05
Y -J	0.24	6415	T	0.22	0.02
J -I	0.23	5549	T	0.19	0.04
I -E	0.28	7450	T	0.25	0.03
E -H	0.53	7207	T	0.32	0.21
H -D	0.53	7156	T	0.32	0.21
Chord-Webs					
G -F	0.22	129	C	0.00	0.22
F -P	0.14	3607	C	0.14	0.00
Webs					
N -O	0.05	222	T		
O -V	0.10	532	C		
V -W	0.04	217	C		
V -F	0.96	4732	C		
W -F	0.14	445	T		
P -M	0.68	3723	T		
M -B	0.29	1468	C		
B -L	0.68	3704	T	1 Br	
L -Q	0.26	1319	C		
Q -K	0.78	2447	T	1 Br	
K -R	0.10	509	C		
K -S	0.20	676	C	1 Br	
J -S	0.05	233	T		
J -C	0.29	931	T	1 Br	
I -C	0.40	1277	T		
I -T	0.54	2149	C		
E -T	0.52	1623	T		
E -U	0.15	484	T		
H -U	0.04	168	T		

LL Defl -0.60" in Y -J L/999
TL Defl -1.20" in Y -J L/519
Shear // Grain in P -B 0.24

Hz Disp	LL	DL	TL
Jt D	0.19"	0.19"	0.38"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	5.0	2.2	0.95
O	LOCK	4.0x 5.0-0.5	0.2	0.5	0.56
W	LOCK	4.0x 5.0-0.5	0.2	0.5	0.56
P	LOCK	12.0x12.0	Ctr	Ctr	0.91
B	LOCK	10.0x10.0-0.5	Ctr		0.97
Q	LOCK	5.0x 5.0	0.5	Ctr	0.99
R	LOCK	8.0x12.0	Ctr	1.5	0.99
S	LOCK	4.0x 5.0	Ctr	0.5	0.66
C	LOCK	10.0x10.0	0.5	Ctr	0.85
T	LOCK	8.0x12.0	0.6	1.5	0.97
U	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	4.0x 6.0	Ctr	Ctr	0.93
D	LOCK	4.0x 6.0	Ctr	Ctr	0.86
N	LOCK	3.0x 5.0	Ctr	Ctr	0.41
V	LOCK	5.0x 8.0	Ctr	Ctr	0.80
G	LOCK	3.0x 5.0	Ctr	Ctr	0.96
F	LOCK	8.0x14.0	Ctr	0.4	0.76
M	LOCK	5.0x 6.0	Ctr	Ctr	0.91
L	LOCK	5.0x 6.0	Ctr	Ctr	0.91
X#	LOCK	6.0x 6.0	Ctr	Ctr	0.91
K	LOCK	4.0x12.0	Ctr	-0.5	0.96
Y#	LOCK	6.0x 6.0	Ctr	Ctr	0.91
J	LOCK	4.0x 5.0	Ctr	-0.5	0.66
I	LOCK	4.0x 5.0	Ctr	-0.5	0.80
E	LOCK	8.0x10.0	-0.2	-0.8	0.75
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

\# = Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7991 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

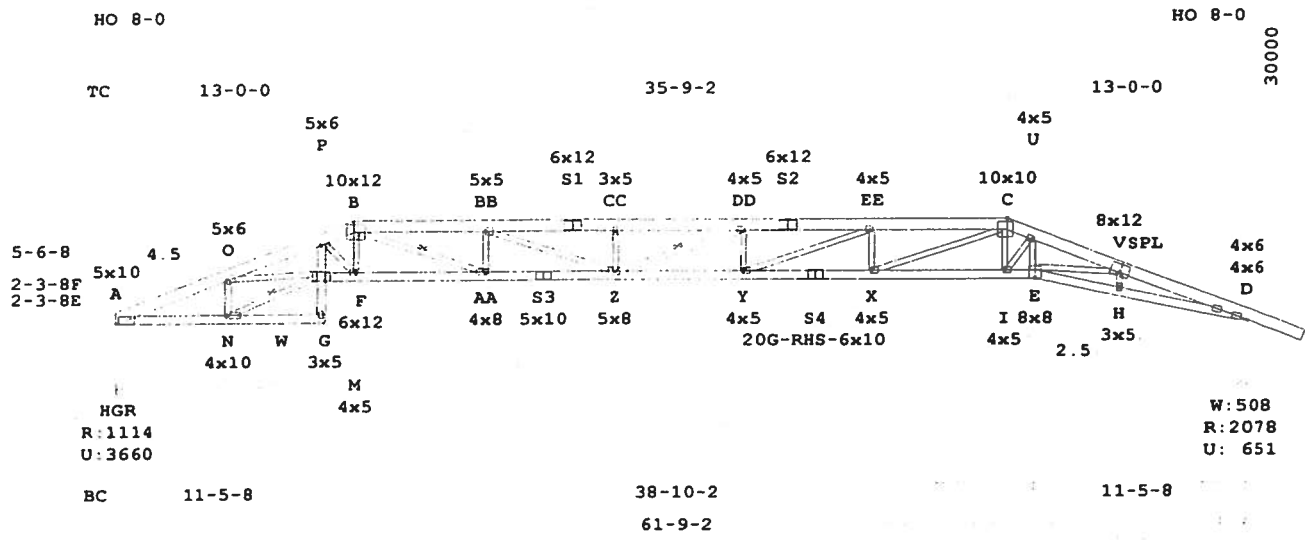
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R71*	1	SP	610902	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R72	1	SP	610902	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 576.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	Size	Lumber
TC	0.41	2x 8	SP-2400
BC	0.62	2x 6	SP-2400
EX F -S3		2x 6	SP-#2
CW	0.39	2x 6	SP-#2
WB	0.83	2x 4	SP-#3
EX N -F		2x 4	SP-#2
EX O -F		2x 4	SP-#2
EX B -AA		2x 4	SP-#2
EX BB-Z		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on N -F
WB 1 rows CLB on B -AA
WB 1 rows CLB on Z -DD
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1115	3660G	3- 8	1- 0
Cont. Brg	6728	2034	0- 0- 0 to Hz =	9- 1- 8 100
D	2079	652	5- 8	1-12

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -O	0.41	6979	T	0.24	0.17
O -P	0.33	3471	T	0.12	0.21
P -B	0.25	1305	T	0.04	0.21
B -BB	0.18	3004	C	0.01	0.17
BB-S1	0.17	6256	C	0.04	0.13
S1-CC	0.13	6256	C	0.04	0.09
CC-DD	0.12	6256	C	0.04	0.08
DD-S2	0.17	7711	C	0.07	0.10
S2-EE	0.18	7711	C	0.07	0.11
EE-C	0.17	7771	C	0.07	0.10
C -U	0.15	6724	C	0.04	0.11
U -V	0.27	7782	C	0.07	0.20
V -D	0.24	7695	C	0.06	0.18
Bottom Chords					
A -N	0.06	52	C	0.00	0.06
N -W	0.62	0	T	0.00	0.62
W -G	0.62	168	C	0.00	0.62
F -M	0.40	3266	C	0.02	0.38
M -AA	0.38	1397	C	0.00	0.38
AA-S3	0.54	3005	T	0.36	0.18
S3-Z	0.21	3005	T	0.13	0.08
Z -Y	0.40	7711	T	0.35	0.05
Y -S4	0.43	7772	T	0.35	0.08
S4-X	0.43	7772	T	0.35	0.08
X -I	0.35	6352	T	0.29	0.06
I -E	0.37	7243	T	0.33	0.04
E -H	0.50	7301	T	0.33	0.17
H -D	0.50	7265	T	0.33	0.17
Chord-Webs					
G -F	0.39	594	C	0.00	0.39
F -P	0.25	3583	C	0.25	0.00
Webs					
N -O	0.30	1763	C		
N -F	0.90	6930	C		1 Br
O -F	0.59	3235	T		
P -M	0.83	2590	T		
M -B	0.29	1677	C		
B -AA	0.85	4650	T		1 Br
AA-BB	0.25	1463	C		
BB-Z	0.84	3486	T		
Z -CC	0.09	513	C		
Z -DD	0.39	1559	C		1 Br
Y -DD	0.07	289	T		
Y -EE	0.13	169	C		
X -EE	0.06	360	C		
X -C	0.48	1500	T		
I -C	0.39	1244	T		
I -U	0.24	1421	C		
E -U	0.52	1632	T		
E -V	0.05	171	T		
H -V	0.03	143	T		

LL Defl -0.88" in Y -S4 L/710
TL Defl -1.78" in Y -S4 L/352
Shear // Grain in W -G 0.48

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.94
O	LOCK	5.0x 6.0	Ctr	Ctr	0.92
P	LOCK	5.0x 6.0-0.5	-0.2		1.00
B	LOCK	10.0x12.0	Ctr	Ctr	0.99
BB	LOCK	5.0x 5.0	0.5	Ctr	0.99
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
CC	LOCK	3.0x 5.0	Ctr	Ctr	0.41
DD	LOCK	4.0x 5.0	Ctr	0.5	0.68
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
EE	LOCK	4.0x 5.0	Ctr	0.5	0.68
C	LOCK	10.0x10.0	0.5	Ctr	0.89
U	LOCK	4.0x 5.0	0.5	0.2	0.81
V	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	4.0x 6.0	Ctr	Ctr	0.94
D	LOCK	4.0x 6.0	Ctr	Ctr	0.89
N	LOCK	4.0x10.0	1.5	Ctr	0.96
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	6.0x12.0	Ctr	0.3	0.95
M	LOCK	4.0x 5.0	Ctr	Ctr	0.98
AA	LOCK	4.0x 8.0	-0.5	Ctr	0.97
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
Z	LOCK	5.0x 8.0	-0.5	Ctr	0.95
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S4	RHS	6.0x10.0	Ctr	0.2	0.75
X	LOCK	4.0x 5.0	Ctr	Ctr	0.67
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	8.0x 8.0	Ctr	-1.5	0.88
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7782 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on informa-

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering Cont.*
060039	*R72*	1	SP	610902	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish

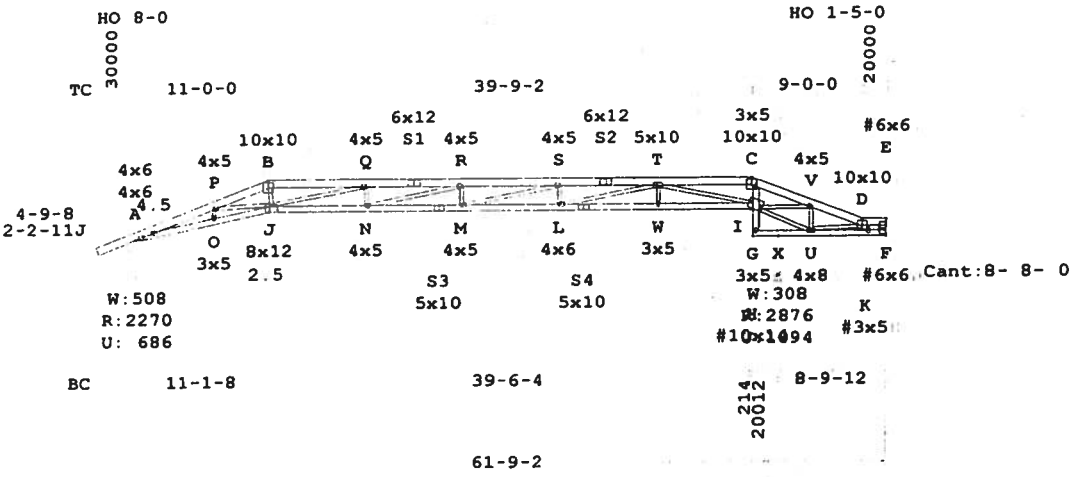
tion furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R73*	1*2P	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.063" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 566.6 LBS

```
Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
***************
* 2-Ply Truss *
***************

     CSI  -Size-  ----Lumber----
TC  0.16  2x 8  SP-2400
BC  0.88  2x 6  SP-#2
EX J -S3  2x 6  SP-2400
EX G -F   2x 6  SP-2400
CW  0.08  2x 6  SP-#2
WB  0.92  2x 4  SP-#3
EX K -D   2x 8  SP-2400
EX F -E   2x 8  SP-2400

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 61- 9- 2
 BC Cont.      0- 0- 0 61- 9- 2

Loading   Live   Dead  (psf)
TC         20.0   10.0
BC          0.0   10.0
Total      20.0   20.0   40.0
Spacing                  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A     2271   687  5- 8  1- 5
                Hz =   -75
X     2877  1094  3- 8  1- 1
                Hz =    78

Membr  CSI   P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -P  0.11   8973 C  0.02  0.09
P -B  0.11   8963 C  0.02  0.09
B -Q  0.12   8667 C  0.02  0.10
Q -S1 0.16  11517 C  0.04  0.12
S1-R  0.15  11517 C  0.04  0.11
R -S  0.14  12772 C  0.05  0.09
S -S2 0.13  11141 C  0.04  0.09
S2-T  0.13  11141 C  0.04  0.09
T -C  0.06    817 C  0.00  0.06
C -V  0.04    781 C  0.00  0.04
V -D  0.05    946 T  0.01  0.04
D -E  0.01      0 T  0.00  0.01
--------Bottom Chords---------
A -O  0.58   8476 T  0.51  0.07
O -J  0.78   8461 T  0.51  0.27
J -N  0.42  11516 T  0.26  0.16
N -S3 0.40  12771 T  0.29  0.11
S3-M  0.88  12771 T  0.77  0.11
M -L  0.77  11140 T  0.67  0.10
L -S4 0.51   6954 T  0.42  0.09
S4-W  0.55   6954 T  0.42  0.13
W -H  0.63   6954 T  0.42  0.21
G -X  0.64     82 C  0.00  0.64
X -U  0.64     85 C  0.00  0.64
U -K  0.21    104 C  0.00  0.21
K -F  0.01      0 T  0.00  0.01
----------Chord-Webs----------
G -H  0.08   1453 C  0.00  0.08
H -C  0.02    454 T  0.02  0.00
------------Webs-------------
O -P  0.03    243 T
P -J  0.02    149 T
B -J  0.40   2524 T
J -Q  0.39   2918 C
N -Q  0.05    420 T
N -R  0.18   1295 C
M -R  0.03    271 T
M -S  0.26   1683 T
L -S  0.06    884 C
L -T  0.69   4316 T
W -T  0.03    265 T
T -H  0.92   6460 C
H -V  0.22   1387 T
H -U  0.08    856 C
U -V  0.08   1022 C
U -D  0.07    747 C
K -D  0.00    317 T
F -E  0.00     16 T

LL Defl -0.93" in S3-M  L/677
TL Defl -1.86" in S3-M  L/336
LL Cant  0.29" in F -F  L/358
Shear // Grain in G -X   0.57
Hz Disp    LL      DL      TL
  Jt X   0.31"   0.32"   0.63"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  4.0x 6.0 Ctr Ctr 0.78
A  LOCK  4.0x 6.0 Ctr Ctr 0.27
P  LOCK  4.0x 5.0-0.5 0.2 0.64
B  LOCK 10.0x10.0 Ctr Ctr 0.76
Q  LOCK  4.0x 5.0 Ctr 0.5 0.78
S1 LOCK  6.0x12.0 Ctr Ctr 0.91
R  LOCK  4.0x 5.0 Ctr 0.5 0.77
S  LOCK  4.0x 5.0 Ctr 0.5 0.77
S2 LOCK  6.0x12.0 Ctr Ctr 0.91
T  LOCK  5.0x10.0-0.5 Ctr 0.99
C  LOCK 10.0x10.0 Ctr-3.2 0.71
C  LOCK  3.0x 5.0 Ctr Ctr 0.09
V  LOCK  4.0x 5.0 0.5 0.2 0.59
D  LOCK 10.0x10.0 Ctr Ctr 0.76
E  LOCK  6.0x 6.0 Ctr 0.6 1.00
O  LOCK  3.0x 5.0 Ctr Ctr 0.41
J  LOCK  8.0x12.0 Ctr-1.5 0.92
N  LOCK  4.0x 5.0 Ctr Ctr 0.67
S3 LOCK  5.0x10.0 Ctr Ctr 0.95
M  LOCK  4.0x 5.0 Ctr Ctr 0.67
L  LOCK  4.0x 6.0 1.0 Ctr 0.97
S4 LOCK  5.0x10.0 Ctr Ctr 0.82
W  LOCK  3.0x 5.0 Ctr Ctr 0.41
H  LOCK 10.0x14.0-0.4 3.3 0.99
G  LOCK  3.0x 5.0 Ctr Ctr 0.73
U  LOCK  4.0x 8.0 Ctr Ctr 0.75
K# LOCK  3.0x 5.0 2.9-0.6 0.96
F  LOCK  6.0x 6.0-0.2-1.3 0.63

# = Plate Monitor used

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3  screws -OR- 10d nails
  as each layer is applied.)
          ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  2     12      24      0
  BC  2     12      24      0
  WB  1      4       4
OH Loading
  Soffit psf 2.0
CAUTION: Use extra care during
  fabrication, shipping,
  handling and erection of
  this truss.  Temporary and
  permanent bracing are req'd
  and are designed by others.
  Seek advice of registered
  engineer.
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
  bearing locations.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R73*	1*2P	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish

for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 12772 Lbs
Quality Control Factor 1.25

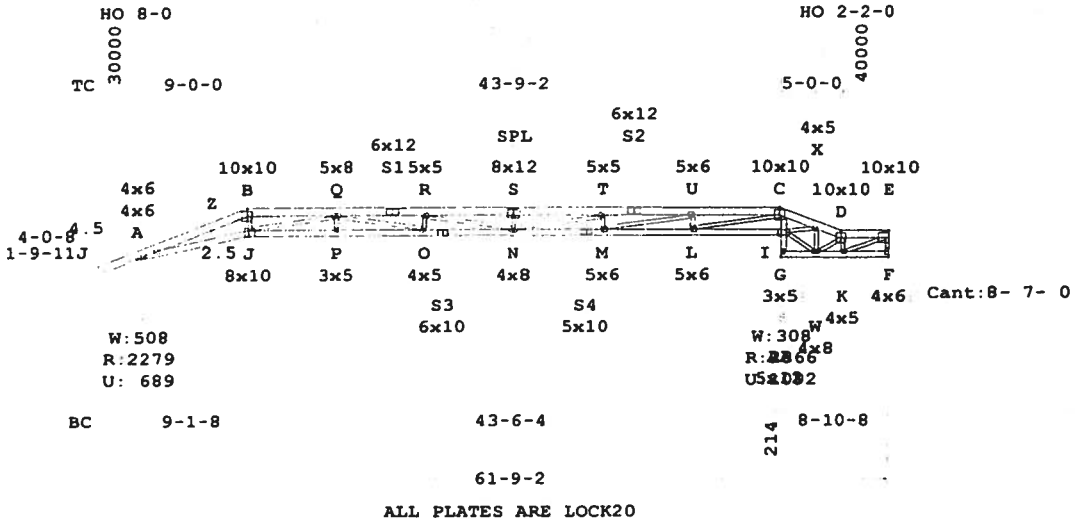
3. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R74*	1*2P	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.063" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 556.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	Size	Lumber
TC	0.26	2x 8	SP-2400
BC	0.84	2x 6	SP-#2
EX	J -S3	2x 6	SP-2400
EX	S3-S4	2x 6	SP-2400
CW	0.91	2x 6	SP-#2
WB	0.44	2x 4	SP-#3
EX	M -U	2x 4	SP-#2
EX	L -C	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2280	689	5- 8	1- 6
			Hz =	67
G	2867	1093	3- 8	1- 8
			Hz =	55

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -B	0.06	9157	C	0.04 0.02
B -Q	0.15	9026	C	0.02 0.13
Q -S1	0.23	15118	C	0.07 0.16
S1-R	0.20	15118	C	0.07 0.13
R -S	0.17	14629	C	0.06 0.11
S -T	0.15	14629	C	0.06 0.09
T -S2	0.17	11931	C	0.04 0.13
S2-U	0.26	11930	C	0.04 0.22
U -C	0.21	5172	C	0.00 0.21
C -X	0.17	1391	C	0.02 0.15
X -D	0.07	520	T	0.00 0.07
D -E	0.01	331	T	0.00 0.01
Bottom Chords				
A -J	0.84	8712	T	0.53 0.31
J -P	0.48	12470	T	0.28 0.20
P -O	0.44	12470	T	0.28 0.16
O -S3	0.40	15118	T	0.34 0.06
S3-N	0.41	15118	T	0.34 0.07
N -S4	0.33	11931	T	0.27 0.06
S4-M	0.79	11931	T	0.72 0.07
M -L	0.63	5172	T	0.31 0.32
L -AA	0.53	1700	C	0.00 0.53
G -W	0.02	999	C	0.02 0.00
W -K	0.02	350	C	0.00 0.02
K -F	0.02	0	T	0.00 0.02
Chord-Webs				
AA-C	0.05	1801	C	0.05 0.00
G -AA	0.91	2821	C	0.00 0.91
Webs				
B -J	0.43	2712	T	
J -Q	0.42	3512	C	
P -Q	0.02	191	T	
Q -O	0.43	2719	T	
O -R	0.03	387	C	
R -N	0.06	502	C	
N -S	0.03	405	C	
N -T	0.44	2770	T	
M -T	0.10	1310	C	
M -U	0.64	6939	T	
L -U	0.10	1385	C	
L -C	0.64	6975	T	
AA-X	0.14	1139	T	
AA-W	0.11	729	T	
W -X	0.03	402	C	
W -D	0.01	142	T	
K -D	0.04	280	T	
K -E	0.03	362	C	
F -E	0.00	62	T	

LL Defl -1.27" in S3-N L/495
TL Defl -2.56" in S3-N L/245
LL Cant 0.40" in K -F L/256
Shear // Grain in G -AA 0.41

Hz Disp	LL	DL	TL
Jt G	0.32"	0.33"	0.65"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.78
A	LOCK	4.0x 6.0	Ctr	Ctr	0.28
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
Q	LOCK	5.0x 8.0	Ctr	Ctr	0.62
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
R	LOCK	5.0x 5.0	Ctr	Ctr	0.60
S	LOCK	8.0x12.0	Ctr	1.5	0.99
T	LOCK	5.0x 5.0	Ctr	Ctr	0.63
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
U	LOCK	5.0x 6.0	Ctr	Ctr	0.90
C	LOCK	10.0x10.0	Ctr	Ctr	0.99
X	LOCK	4.0x 5.0	0.5	0.2	0.56
D	LOCK	10.0x10.0	Ctr	Ctr	0.76
E	LOCK	10.0x10.0	-1.5	Ctr	0.75
J	LOCK	8.0x10.0	1.0	-1.5	0.91
P	LOCK	3.0x 5.0	Ctr	Ctr	0.41
O	LOCK	4.0x 5.0	Ctr	Ctr	0.77
S3	LOCK	6.0x10.0	Ctr	0.2	0.94
N	LOCK	4.0x 8.0	Ctr	Ctr	0.78
S4	LOCK	5.0x10.0	Ctr	Ctr	0.89
M	LOCK	5.0x 6.0	Ctr	Ctr	0.90
L	LOCK	5.0x 6.0	Ctr	Ctr	0.91
AA	LOCK	5.0x12.0	Ctr	Ctr	0.96
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
W	LOCK	4.0x 8.0	Ctr	Ctr	0.53
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 6.0	-0.2	Ctr	0.90

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R74*	1*2P	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish

Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 15118 Lbs
Quality Control Factor 1.25

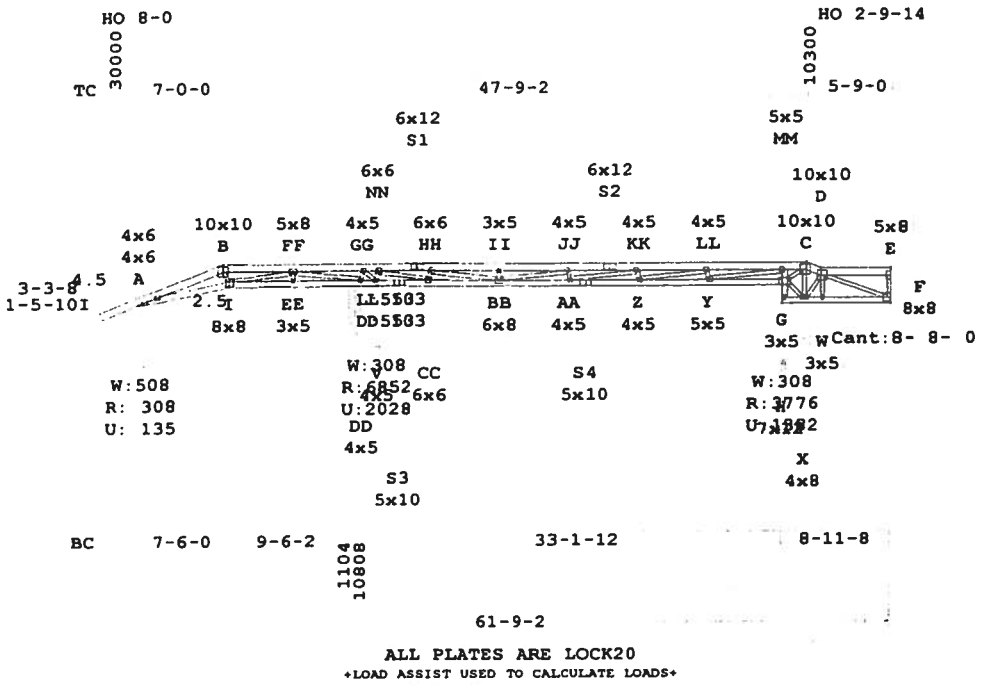
3. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R75	1*4P	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.063" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 558.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 4-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.22	2x 8	SP-2400
BC	0.55	2x 6	SP-#2
CW	0.14	2x 6	SP-#2
WB	0.80	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	61.8'
BC V 0	20	0.0'	61.8'
BC V 70	70	18.9'	38.9'
BC V 70	70	40.9'	52.9'
BC V 553	553	17.0'	CL-LB
BC V 103	103	17.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	309	135	5- 8	1- 0
		Hz =		96
V	6853	2029	3- 8	1-13
G	3777	1383	3- 8	1- 0
		Hz =		55

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.01	1015 T	0.00 0.01
B -FF	0.03	1347 T	0.01 0.02
FF-GG	0.16	8290 T	0.07 0.09
GG-NN	0.22	10324 T	0.08 0.14
NN-S1	0.14	635 T	0.00 0.14
S1-HH	0.03	635 T	0.00 0.03
HH-II	0.04	8963 C	0.00 0.04
II-JJ	0.07	8963 C	0.00 0.07
JJ-S2	0.09	13488 C	0.01 0.08
S2-KK	0.08	13488 C	0.01 0.07
KK-LL	0.09	13274 C	0.01 0.08
LL-MM	0.10	8387 C	0.00 0.10
MM-C	0.10	925 C	0.00 0.10
C -D	0.00	780 T	
D -E	0.01	0 T	0.00 0.01
--------Bottom Chords--------			
A -I	0.07	935 C	0.00 0.07
I -EE	0.10	3367 C	0.00 0.10
EE-DD	0.26	3367 C	0.00 0.26
DD-V	0.28	8291 C	0.01 0.27
V -S3	0.28	10325 C	0.01 0.27
S3-CC	0.10	10325 C	0.01 0.09
CC-BB	0.09	635 C	0.00 0.09
BB-AA	0.55	13488 T	0.41 0.14
AA-S4	0.50	13274 T	0.40 0.10
S4-Z	0.53	13274 T	0.40 0.13
Z -Y	0.41	8387 T	0.25 0.16
Y -H	0.28	843 T	0.00 0.28
G -X	0.01	377 C	0.00 0.01
X -W	0.02	534 C	0.00 0.02
W -F	0.02	536 C	0.00 0.02
----------Chord-Webs----------			
G -H	0.14	3799 C	0.00 0.14
H -MM	0.05	3468 C	0.05 0.00
------------Webs------------			
B -I	0.02	732 C	
I -FF	0.18	2253 T	
EE-FF	0.03	557 T	
FF-DD	0.21	5049 C	
DD-GG	0.18	2333 T	
GG-V	0.11	2896 C	
V -NN	0.16	4132 C	
NN-CC	0.80	10096 T	
CC-HH	0.08	2179 C	
HH-BB	0.78	9844 T	
BB-II	0.01	241 C	
BB-JJ	0.19	4640 C	
AA-JJ	0.06	890 T	
AA-KK	0.01	261 C	
Z -KK	0.01	427 C	
Z -LL	0.40	5012 T	
Y -LL	0.04	1041 C	
Y -MM	0.70	8819 T	
H -C	0.03	506 T	
H -X	0.02	518 C	
X -C	0.05	653 T	
X -D	0.01	309 C	
W -D	0.01	194 T	
D -F	0.04	569 T	
F -E	0.00	148 C	

LL Defl -0.31" in AA-S4 L/999
TL Defl -0.62" in AA-S4 L/633
LL Cant 0.17" in W -F L/598
Shear // Grain in EE-DD 0.25

Hz Disp	LL	DL	TL
Jt G	0.04"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.78
A	LOCK	4.0x 6.0	Ctr	Ctr	0.19
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
FF	LOCK	5.0x 8.0	Ctr	Ctr	0.60
GG	LOCK	4.0x 5.0	Ctr	0.5	0.60
NN	LOCK	6.0x 6.0	Ctr	Ctr	0.95
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
HH	LOCK	6.0x 6.0	Ctr	Ctr	0.92
II	LOCK	3.0x 5.0	Ctr	Ctr	0.41
JJ	LOCK	4.0x 5.0	Ctr	0.5	1.00
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
KK	LOCK	4.0x 5.0	Ctr	0.5	1.00
LL	LOCK	4.0x 5.0	Ctr	0.5	1.00
MM	LOCK	5.0x 5.0	Ctr	Ctr	0.99
C	LOCK	10.0x10.0	0.5	Ctr	0.76
D	LOCK	10.0x10.0	1.0	Ctr	0.90
E	LOCK	5.0x 8.0	-0.8	Ctr	0.91
I	LOCK	8.0x 8.0	0.5	-1.5	1.00
EE	LOCK	3.0x 5.0	Ctr	Ctr	0.41
DD	LOCK	4.0x 5.0	Ctr	Ctr	0.75
V	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
CC	LOCK	6.0x 6.0	Ctr	Ctr	0.95
BB	LOCK	6.0x 8.0	Ctr	Ctr	0.92
AA	LOCK	4.0x 5.0	Ctr	Ctr	0.75
S4	LOCK	5.0x10.0	Ctr	Ctr	0.82
Z	LOCK	4.0x 5.0	Ctr	Ctr	0.75
Y	LOCK	5.0x 5.0	Ctr	Ctr	0.99
H	LOCK	7.0x12.0	Ctr	0.8	0.82
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
X	LOCK	4.0x 8.0	Ctr	Ctr	0.51
W	LOCK	3.0x 5.0	Ctr	Ctr	0.41
F	LOCK	8.0x 8.0	-0.5	Ctr	0.79

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R75*	1*4P	SP	610902	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish

FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

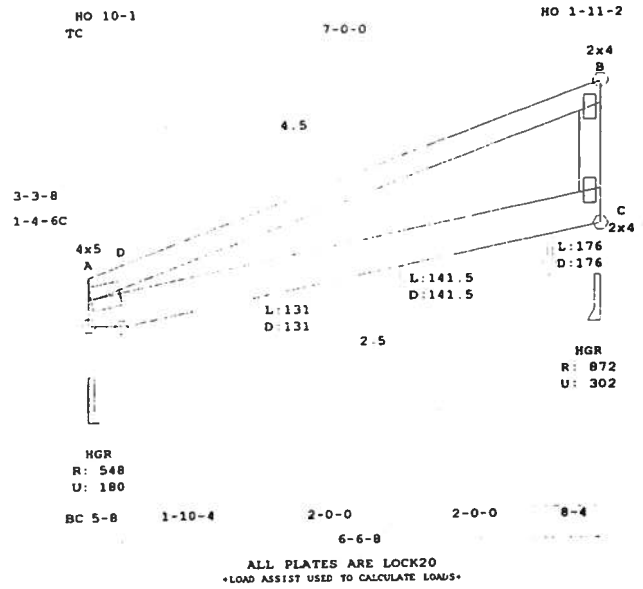
	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	2	12	24	24
BC	2	12	24	24
WB	1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 13488 Lbs
Quality Control Factor 1.25
5. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R76*	1	SP	60608	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.63	2x 4	SP-#2
BC	0.95	2x 6	SP-#2
WB	0.07	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 6- 8
BC Cont.	0- 0- 0	6- 6- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00		Ft=1.00
BC Fb=1.00	Fc=1.00		Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.5'	7.0'
BC V	0	20	0.5'	7.0'
BC V	131	131	2.3'	CL-LB
BC V	142	142	4.3'	CL-LB
BC V	176	176	6.3'	CL-LB

Plus 7 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	548	180	3- 8	1- 0
			Hz =	118
C	872	303	3- 8	1- 0
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.63	117 C	0.00 0.63
--------Bottom Chords---------			
A -C	0.95	60 T	0.00 0.95
------------Webs------------			
C -B	0.07	315 T	

LL Defl -0.04" in A -C L/999
TL Defl -0.09" in A -C L/887
Shear // Grain in A -C 0.54

Hz Disp	LL	DL	TL
Jt A	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.9	0.7	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

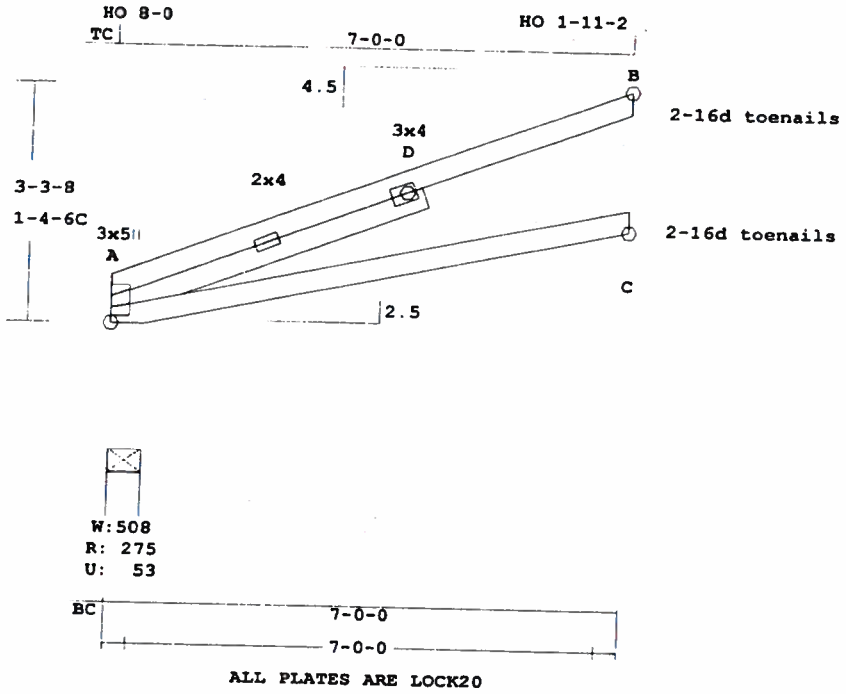
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 197 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R77	9	VJACK	70000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 36.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.13	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	275	54	5- 8	1- 0
			Hz =	118
B	207	135	3- 8	1- 0
C	116	0	3- 8	1- 0
			Hz =	63

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

A -D	0.53	443 C	0.00	0.53
D -B	0.53	116 C	0.00	0.53

--------Bottom Chords---------

A -C	0.35	62 T	0.00	0.35

----------Sliders----------

A -D	0.13	451 T

LL Defl -0.06" in A -C L/999
TL Defl -0.13" in A -C L/567
Shear // Grain in A -D 0.28

Hz Disp	LL	DL	TL
Jt B	0.04"	-0.01"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

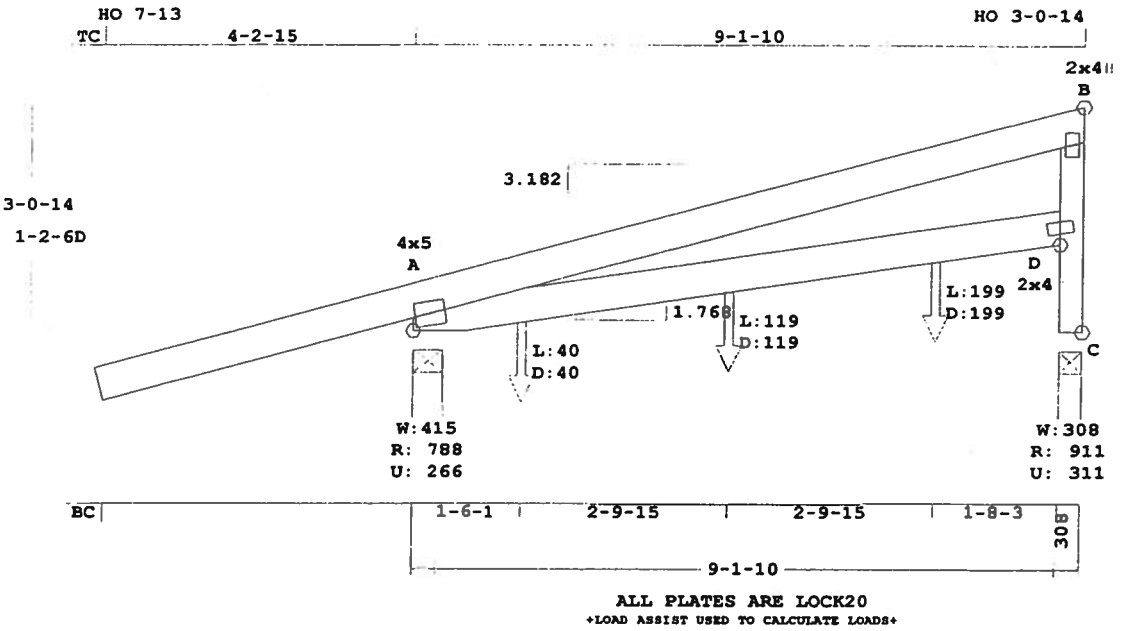
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 443 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R78	1	HFSC	90110	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.73	2x 6	SP-#2
BC	0.97	2x 6	SP-#2
WB	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 1-10
BC Cont.	0- 0- 0	9- 1-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.1'
BC V	0	20	0.0'	9.1'
TC V	-40	-20	0.0'	
	37	19		9.1'
BC V	0	-20	0.0'	
	0	19		8.8'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.2'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	789	267	4-15	1- 0
			Hz =	106
D	911	312	3- 8	1- 0
			Hz =	57

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.73	124 C	0.00 0.73
--------Bottom Chords---------			
A -D	0.97	56 T	0.00 0.97
-------------Webs-------------			
D -B	0.11	448 T	

LL Defl -0.09" in A -D L/999
TL Defl -0.21" in A -D L/486
Shear // Grain in A -D 0.42

Hz Disp	LL	DL	TL
Jt A	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.8	0.7	0.81
B	LOCK	2.0x 4.0	Ctr	Ctr	0.39
D	LOCK	2.0x 4.0	Ctr	Ctr	0.43

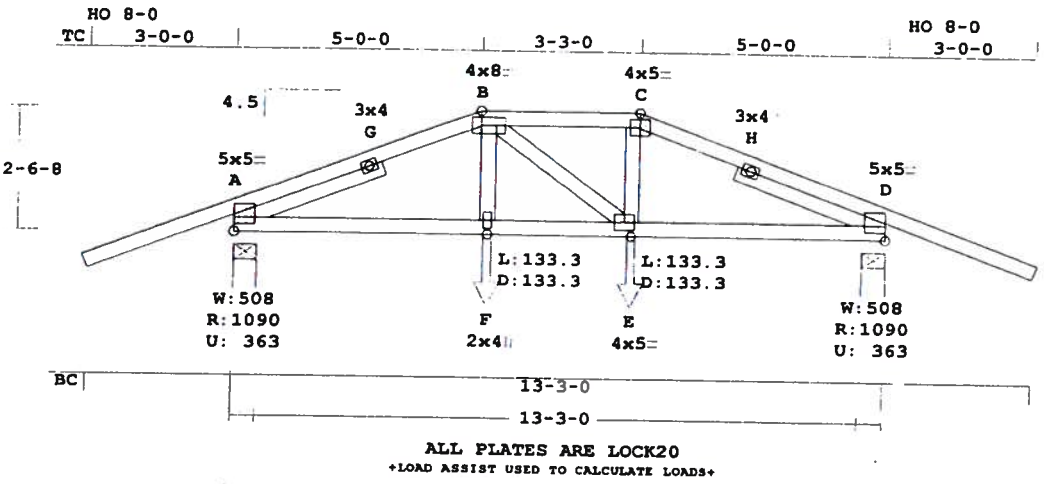
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 6-10- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 391 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R79	1	HIPP	130300	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 89.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 4	SP-#2
BC	0.48	2x 4	SP-#2
WB	0.10	2x 4	SP-#3
SL	0.15	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	13- 3- 0
BC Cont.	0- 0- 0	13- 3- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	13.2'
BC V 0	20	0.0'	13.2'
TC V 30	15	5.0'	8.2'
BC V 0	15	5.1'	8.1'
BC V 133	133	5.1'	CL-LB
BC V 133	133	8.1'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1090	364	5- 8	1- 5
D	1090	364	5- 8	1- 5

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -G	0.18	883 C	0.00	0.18
G -B	0.27	1612 C	0.18	0.09
B -C	0.31	1508 C	0.17	0.14
C -H	0.26	1618 C	0.18	0.08
H -D	0.19	860 C	0.00	0.19
Bottom Chords				
A -F	0.48	1505 T	0.27	0.21
F -E	0.38	1489 T	0.27	0.11
E -D	0.45	1508 T	0.27	0.18
Webs				
F -B	0.10	360 T		
B -E	0.00	25 T		
E -C	0.10	366 T		
Sliders				
A -G	0.15	792 C		
H -D	0.15	822 C		

LL Defl -0.04" in F -E L/999
TL Defl -0.09" in F -E L/999
Shear // Grain in B -C 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

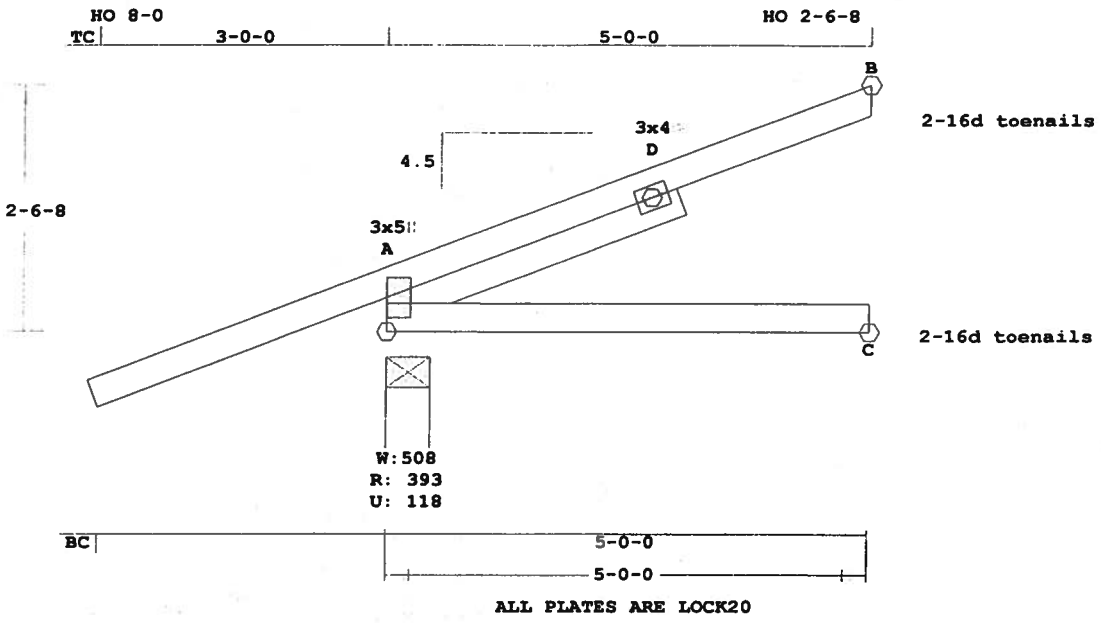
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.90
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	5.0x 5.0-2.5	0.5	0.90	
F	LOCK	2.0x 4.0	Ctr	Ctr	0.39
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 5- 0- 0
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
3- 1- 0. Support end or
provide level return.
Excessive Right OH condition.
Max allowable length is
3- 1- 0. Support end or
provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1618 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R80*	3	JACK	50000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
SL	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 0
BC Cont.	0- 0- 0	5- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	394	119	5- 8	1- 0
			Hz =	84
B	149	99	3- 8	1- 0
C	84	0	3- 8	1- 0
			Hz =	45

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

A -D	0.37	204	C	0.01	0.36
D -B	0.36	105	C	0.00	0.36

--------Bottom Chords---------

A -C	0.25	45	T	0.00	0.25

----------Sliders-----------

A -D 0.05 255 C

LL Defl -0.01" in A -C L/999
TL Defl -0.03" in A -C L/999
Shear // Grain in A -D 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	3.0x 5.0	1.5	0.3	0.65
D LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

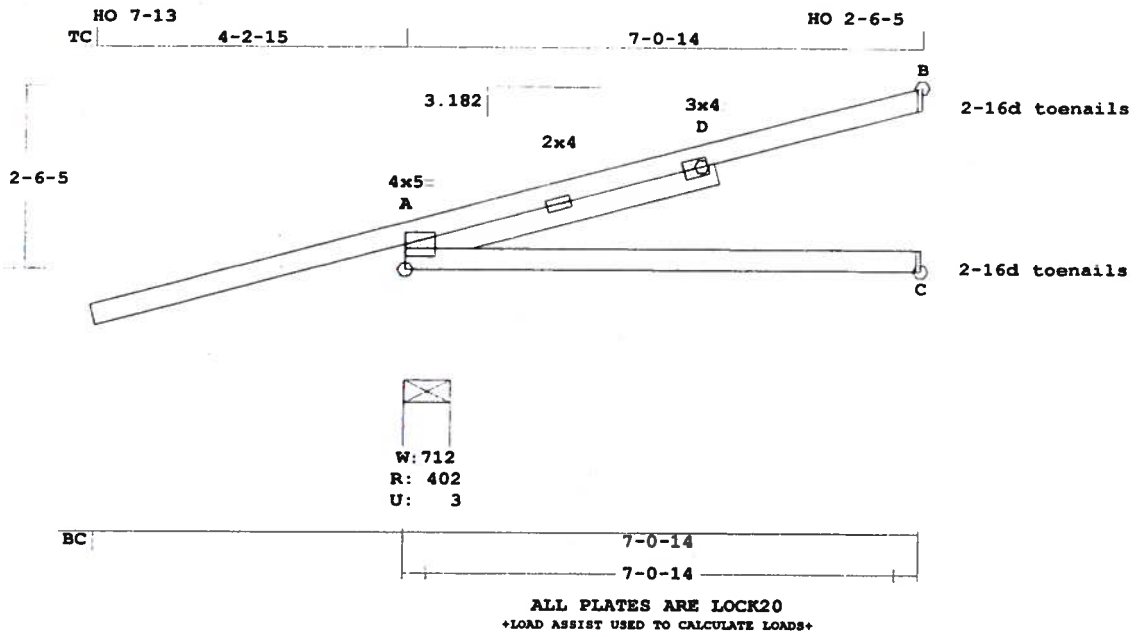
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 255 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R81*	2	KING	70014	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 43.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
SL	0.07	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0-14
BC Cont.	0- 0- 0	7- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	7.1'
BC V	0	20	0.0'	7.1'
TC V	-40	-20	0.0'	
	17	8		7.1'
BC V	0	-20	0.0'	
	0	8		7.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	403	4	7-12	1- 0
			Hz =	107
B	201	292	3- 8	1- 0
C	119	0	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.45	282 C	0.00	0.45
D -B	0.45	78 C	0.00	0.45
--------Bottom Chords---------				
A -C	0.25	32 T	0.00	0.25
----------Sliders----------				
A -D	0.07	286 T		

LL Defl -0.04" in A -C L/999
TL Defl -0.10" in A -C L/745
Shear // Grain in D -B 0.24

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.5	0.4	0.95
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

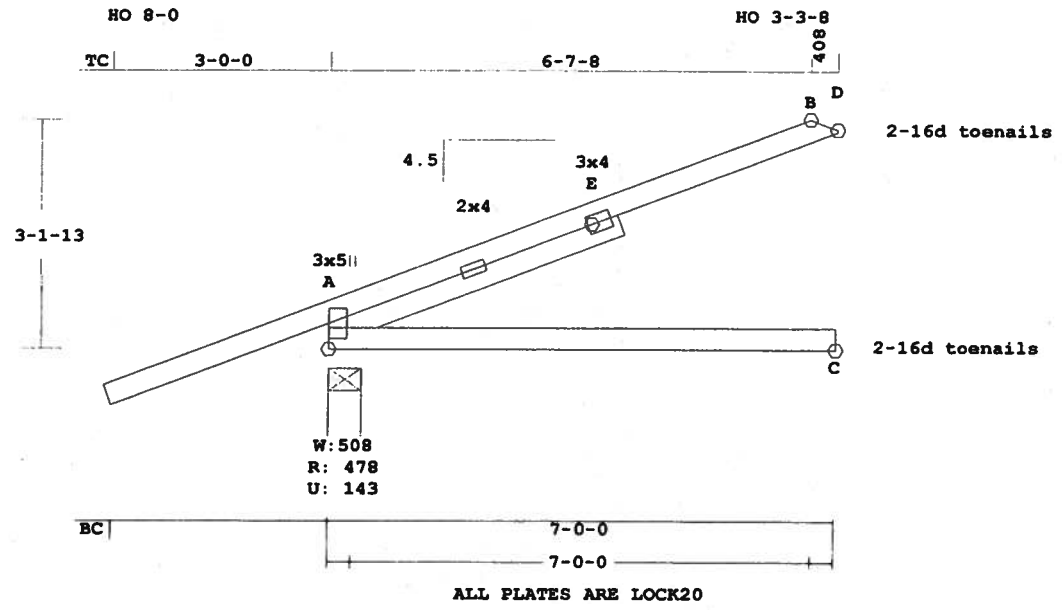
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 5- 0- 0
OH Loading
Soffit psf 2.0

Excessive Left OH condition.
Max allowable length is 3- 5- 9. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 282 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R83	3	SP	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 41.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.56	2x 4	SP-#2
BC	0.42	2x 4	SP-#2
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	479	143	5- 8	1- 0
			Hz =	112
D	199	97	3- 8	1- 0
			Hz =	60
C	120	0	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -E	0.54	399	C 0.00 0.54
E -D	0.56	98	C 0.00 0.56
Bottom Chords			
A -C	0.42	0	T 0.00 0.42
Sliders			
A -E	0.10	402	T

LL Defl -0.06" in A -C L/999
TL Defl -0.15" in A -C L/523
Shear // Grain in A -E 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50

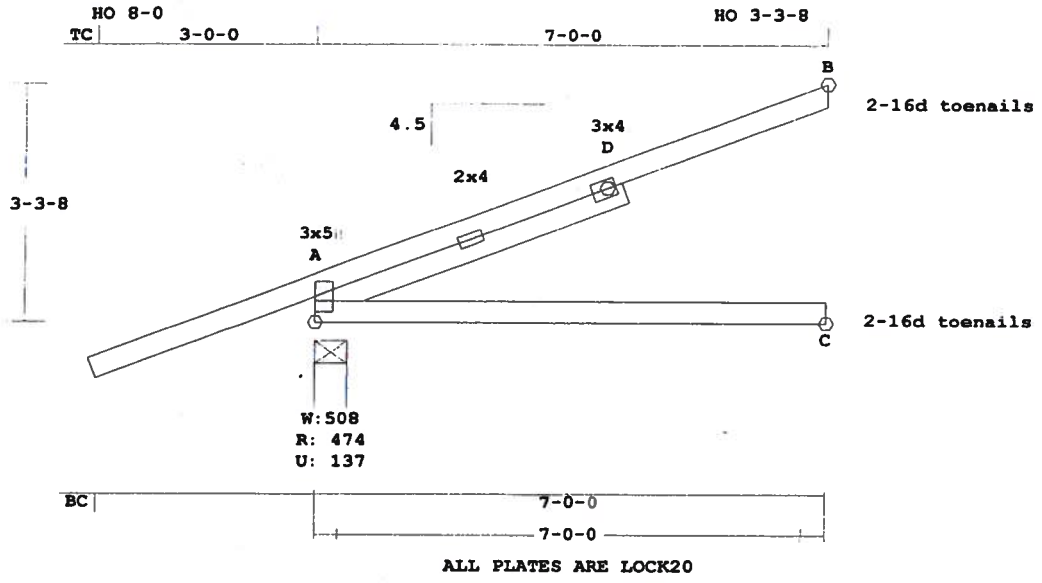
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 399 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R84*	1	MONO	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	474	138	5- 8	1- 0
			Hz = 118	
B	207	136	3- 8	1- 0
			Hz = 63	
C	118	0	3- 8	1- 0

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords

A -D	0.53	420	C	0.00	0.53
D -B	0.53	117	C	0.00	0.53

Bottom Chords

A -C	0.35	0	T	0.00	0.35

Sliders

A -D	0.11	424	T

LL Defl -0.05" in A -C L/999
TL Defl -0.13" in A -C L/565
Shear // Grain in A -D 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

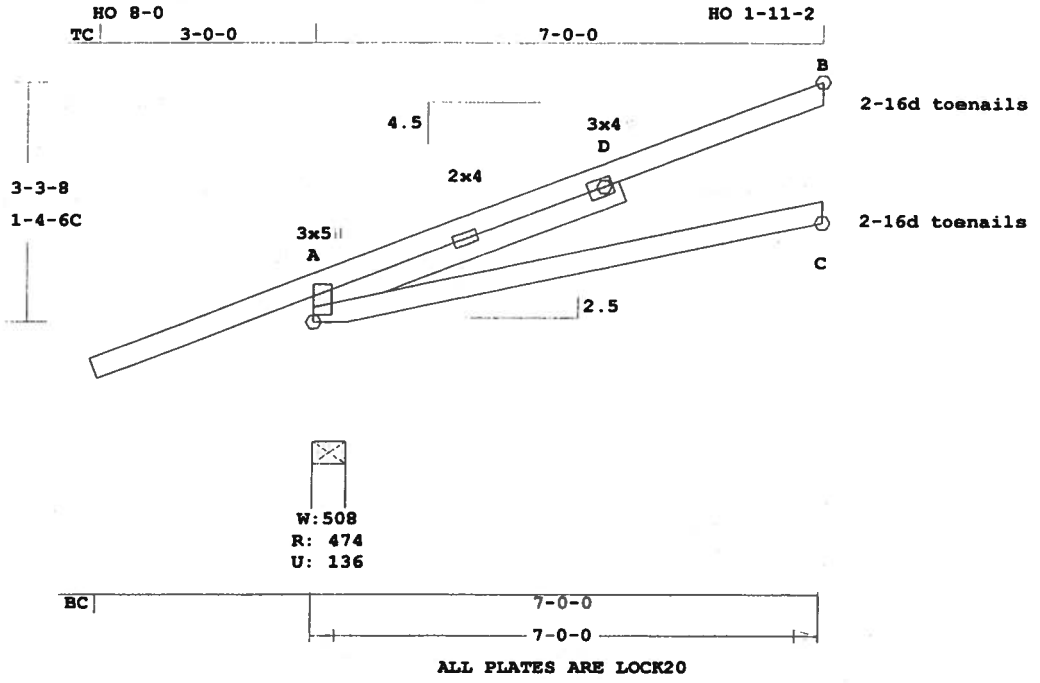
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 420 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R86	13	VJACK	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.13	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	474	137	5- 8	1- 0
			Hz =	118
B	207	135	3- 8	1- 0
C	116	0	3- 8	1- 0
			Hz =	63

Membr CSI P Lbs Axl-CSI-Bnd OH Loading

----------Top Chords----------

A	-D	0.53	443	C	0.00 0.53
D	-B	0.53	116	C	0.00 0.53

--------Bottom Chords---------

A	-C	0.35	62	T	0.00 0.35

-----------Sliders-----------

A	-D	0.13	451 T

LL Defl -0.06" in A -C L/999
TL Defl -0.13" in A -C L/568
Shear // Grain in A -D 0.28

Hz Disp	LL	DL	TL
Jt B	0.04"	-0.01"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

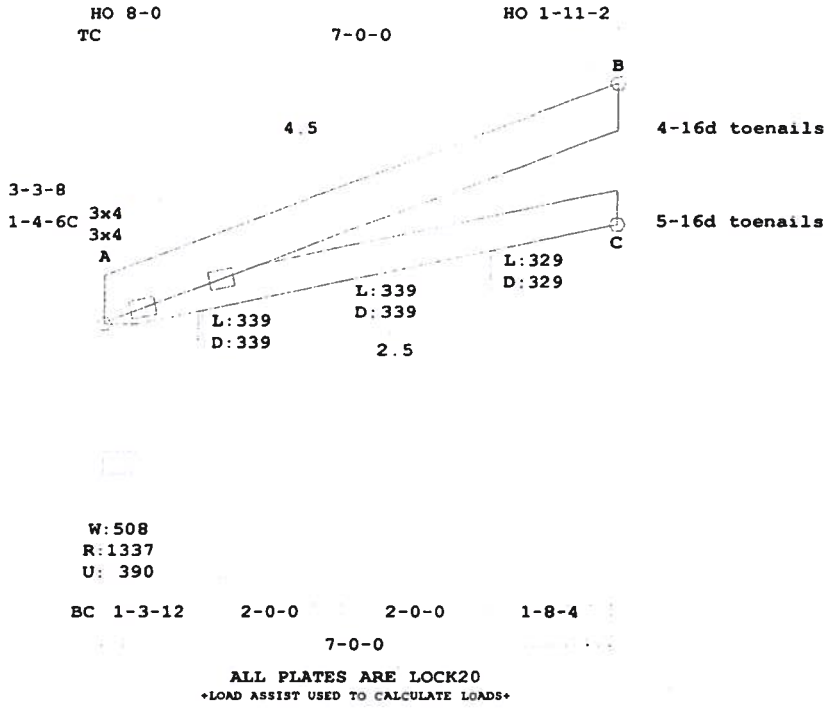
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 443 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R87*	1	VJACK	70000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 8	SP-2400
BC	0.55	2x 6	SP-2400

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 0
BC	Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	7.0'
BC V	0	20	0.0'	7.0'
BC V	339	339	1.3'	CL-LB
BC V	339	339	3.3'	CL-LB
BC V	329	329	5.3'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1338	390	5- 8	1- 2
			Hz =	118
B	534	230	3- 8	1- 0
C	737	184	3- 8	1- 0
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.53	203 C	0.00 0.53
--------Bottom Chords---------			
A -C	0.55	63 T	0.00 0.55

LL Defl -0.05" in A -C L/999
TL Defl -0.11" in A -C L/713
Shear // Grain in A -C 0.54

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.67
A	LOCK	3.0x 4.0	Ctr	Ctr	0.17

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

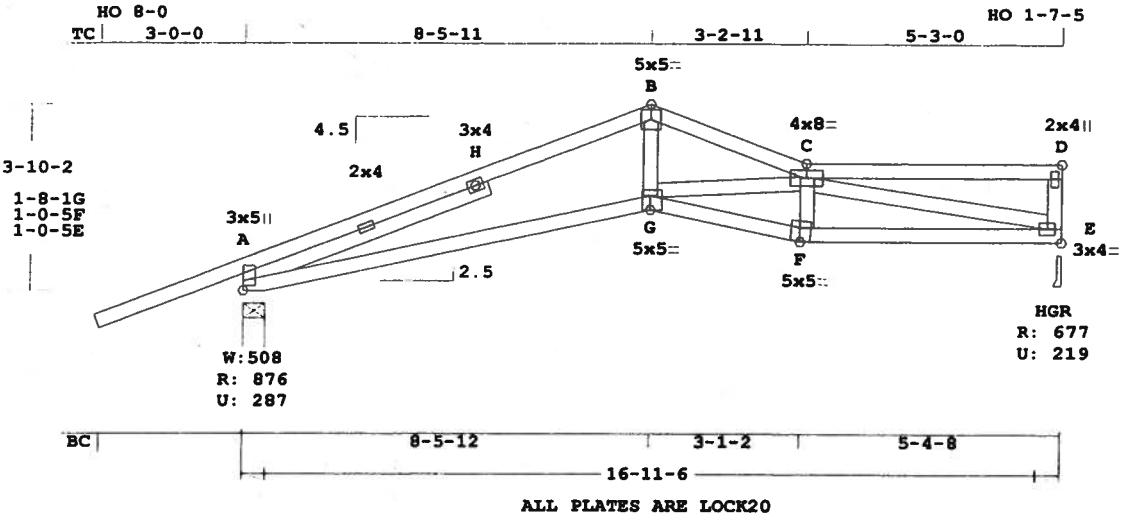
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 203 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R88	1	SP	161106	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 104.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.48	2x 4	SP-#2
BC	0.59	2x 4	SP-#2
WB	0.83	2x 4	SP-#3
SL	0.18	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	877	287	5- 8	1- 1
			Hz =	89
E	678	219	3- 8	1- 0
			Hz =	45

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -H	0.34	1164	C	0.01 0.33
H -B	0.39	1619	C	0.13 0.26
B -C	0.35	1702	C	0.01 0.34
C -D	0.48	0	T	0.00 0.48
Bottom Chords				
A -G	0.51	1572	T	0.26 0.25
G -F	0.59	2057	T	0.34 0.25
F -E	0.49	2005	T	0.33 0.16
Webs				
G -B	0.29	916	T	
G -C	0.08	421	C	
F -C	0.08	385	C	
C -E	0.83	2050	C	
E -D	0.03	188	C	
Sliders				
A -H	0.18	537	C	

LL Defl -0.12" in A -G L/999
TL Defl -0.30" in A -G L/656
Shear // Grain in A -H 0.24

Hz Disp	LL	DL	TL
Jt E	0.04"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.89
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x 8.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
G	LOCK	5.0x 5.0	0.5-1.1		0.67
F	LOCK	5.0x 5.0	0.3	2.7	0.46
E	LOCK	3.0x 4.0	Ctr	Ctr	0.79

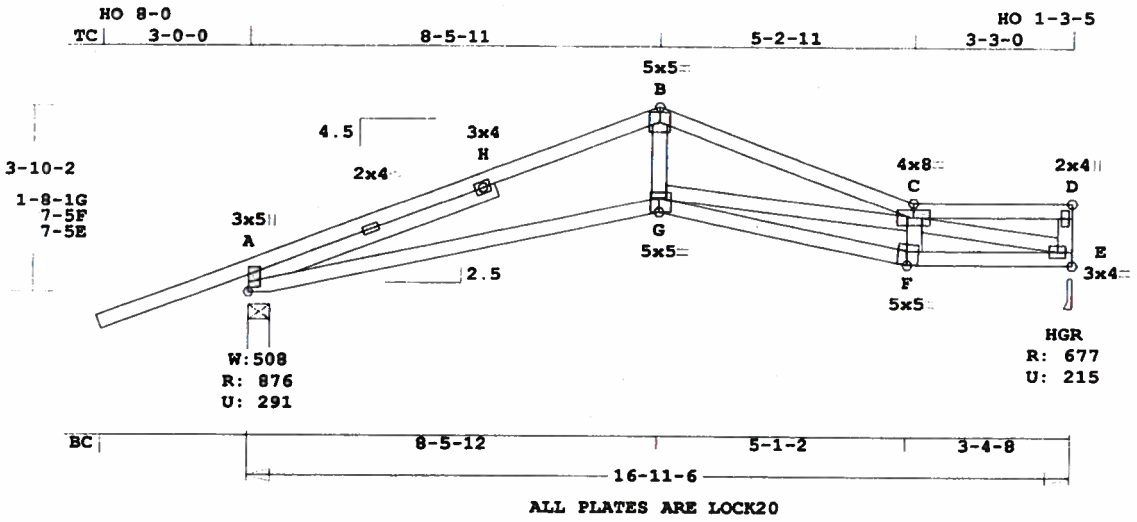
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2050 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R89*	1	SP	161106	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 103.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 4	SP-#2
BC	0.54	2x 4	SP-#2
WB	0.39	2x 4	SP-#3
SL	0.19	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	876	291	5- 8	1- 1
			Hz =	55
E	678	215	3- 8	1- 0
			Hz =	25

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -H	0.32	1161 C	0.01	0.31
H -B	0.36	1642 C	0.14	0.22
B -C	0.37	1712 C	0.15	0.22
C -D	0.37	0 T	0.00	0.37
--------Bottom Chords---------				
A -G	0.49	1594 T	0.26	0.23
G -F	0.54	1935 T	0.32	0.22
F -E	0.42	1889 T	0.31	0.11
------------Webs------------				
G -B	0.27	843 T		
G -C	0.12	292 T		
F -C	0.08	356 C		
C -E	0.39	1944 C		
E -D	0.02	165 C		
----------Sliders----------				
A -H	0.19	564 C		

LL Defl -0.10" in A -G L/999
TL Defl -0.26" in A -G L/751
Shear // Grain in A -H 0.23

Hz Disp	LL	DL	TL
Jt E	0.03"	0.04"	0.07"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.90
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x 8.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
G	LOCK	5.0x 5.0	0.5-1.1		0.62
F	LOCK	5.0x 5.0	0.3	2.7	0.46
E	LOCK	3.0x 4.0	Ctr	Ctr	0.75

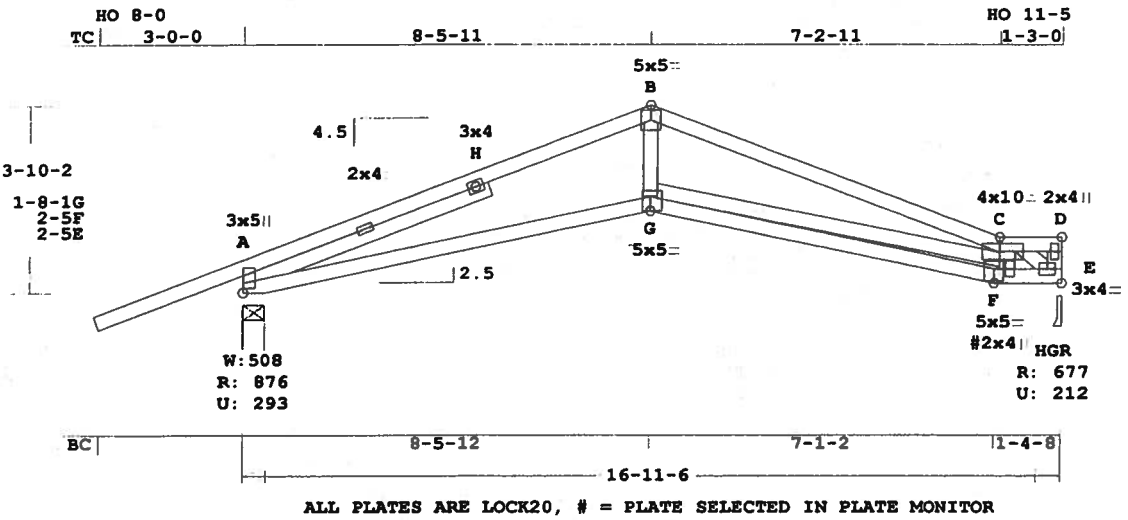
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1944 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R90	1	SP	161106	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 103.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.57	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.26	2x 4	SP-#3
SL	0.23	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	876	294	5- 8	1- 1
			Hz =	22
E	678	213	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -H	0.29	1123 C	0.07	0.22
H -B	0.40	1711 C	0.12	0.28
B -C	0.57	1778 C	0.06	0.51
C -D	0.09	0 T	0.00	0.09
--------Bottom Chords---------				
A -G	0.44	1663 T	0.27	0.17
G -F	0.36	1251 T	0.13	0.23
F -E	0.23	1253 T	0.13	0.10
------------Webs------------				
G -B	0.26	823 T		
G -C	0.16	449 T		
F -C	0.07	283 T		
C -E	0.22	1406 C		
E -D	0.00	74 C		
-----------Sliders-----------				
A -H	0.23	672 C		

LL Defl -0.09" in G -F L/999
TL Defl -0.21" in G -F L/939
Shear // Grain in B -C 0.26

Hz Disp	LL	DL	TL
Jt E	0.04"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.94
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x10.0	0.5	Ctr	0.94
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
G	LOCK	5.0x 5.0	0.5-1.1		0.60
F	LOCK	5.0x 5.0	Ctr	1.3	0.54
F#	LOCK	2.0x 4.0	0.7	0.1	0.40
E	LOCK	3.0x 4.0	Ctr	Ctr	0.64

= Plate Monitor used

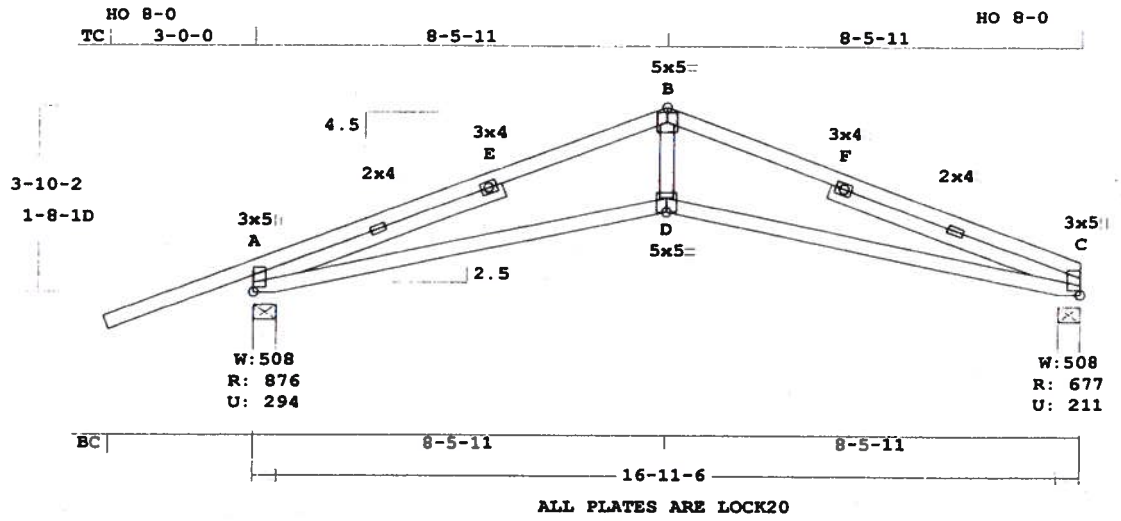
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1778 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R91*	2	SCISSOR	161106	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 97.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.38	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.27	2x 4	SP-#3
SL	0.23	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	876	295	5- 8	1- 1
C	678	212	5- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.29	1115 C	0.07	0.22
E -B	0.38	1705 C	0.12	0.26
B -F	0.38	1705 C	0.11	0.27
F -C	0.29	1115 C	0.07	0.22
--------Bottom Chords---------				
A -D	0.44	1657 T	0.27	0.17
D -C	0.44	1657 T	0.27	0.17
----------Webs----------				
D -B	0.27	844 T		
----------Sliders----------				
A -E	0.23	674 C		
F -C	0.23	673 C		

LL Defl -0.08" in D -C L/999
TL Defl -0.19" in D -C L/999
Shear // Grain in E -B 0.23

Hz Disp	LL	DL	TL
Jt C	0.03"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.94
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0-1.5	0.4		0.94
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	5.0x 5.0	Ctr-1.1		0.46

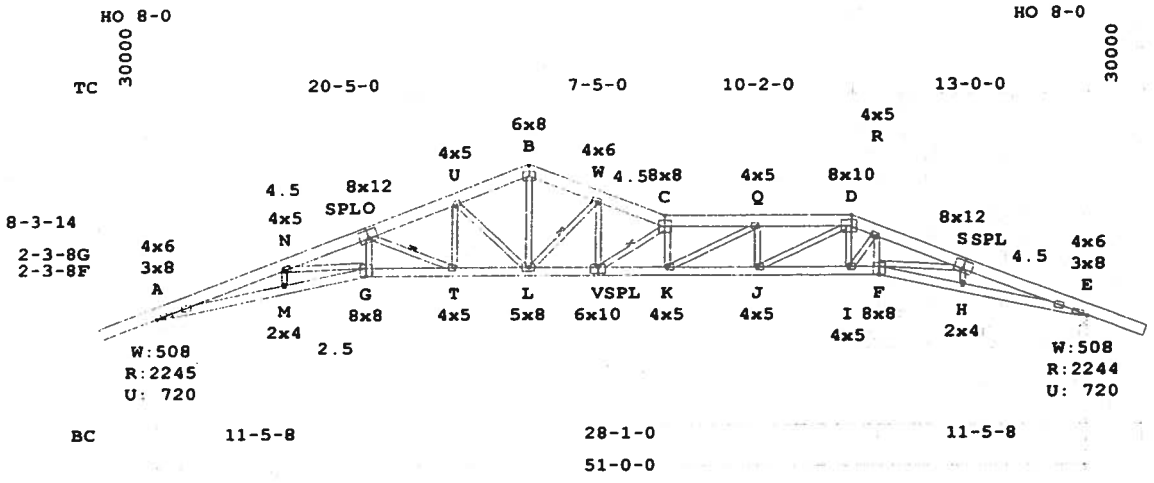
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1705 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R147*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 496.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.62	2x 6	SP-2400
WB	0.69	2x 4	SP-#3
EX L -B		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on O -T
WB 1 rows CLB on L -W
WB 1 rows CLB on V -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2245	721	5- 8	1-14
			Hz =	-140
E	2245	721	5- 8	1-14
			Hz =	143

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.28	8525	C	0.08	0.20
N -O	0.31	8722	C	0.09	0.22
O -U	0.15	6002	C	0.04	0.11
U -B	0.08	4834	C	0.06	0.02
B -W	0.09	4860	C	0.02	0.07
W -C	0.21	6742	C	0.04	0.17
C -Q	0.23	9419	C	0.09	0.14
Q -D	0.19	8487	C	0.08	0.11
D -R	0.18	7520	C	0.06	0.12
R -S	0.31	8703	C	0.09	0.22
S -E	0.28	8494	C	0.08	0.20
Bottom Chords					
A -M	0.54	8046	T	0.36	0.18
M -G	0.54	8083	T	0.36	0.18
G -T	0.41	8118	T	0.37	0.04
T -L	0.29	5647	T	0.25	0.04
L -V	0.33	6399	T	0.29	0.04
V -K	0.62	9441	T	0.43	0.19
K -J	0.57	8484	T	0.38	0.19
J -I	0.35	7122	T	0.32	0.03
I -F	0.42	8093	T	0.37	0.05
F -H	0.55	8053	T	0.36	0.19
H -E	0.55	8014	T	0.36	0.19
Webs					
M -N	0.03	144	T		
N -G	0.09	288	T		
G -O	0.58	1829	T		
O -T	0.49	2721	C		1 Br
T -U	0.37	1154	T		
U -L	0.68	1553	C		
L -B	0.59	3210	T		
L -W	0.51	2748	C		1 Br
V -W	0.69	2160	T		
V -C	0.63	3622	C		1 Br
K -C	0.11	628	C		
K -Q	0.34	1064	T		
J -Q	0.08	499	C		
J -D	0.48	1521	T		
I -D	0.40	1262	T		
I -R	0.27	1574	C		
F -R	0.59	1853	T		
F -S	0.09	294	T		
H -S	0.03	154	T		

LL Defl -1.01" in K -J L/597
TL Defl -2.03" in K -J L/296
Shear // Grain in D -R 0.25

Hz Disp	LL	DL	TL
Jt E	0.48"	0.48"	0.96"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
N	LOCK	4.0x 5.0	-0.5	0.2	0.62
O	LOCK	8.0x12.0	-0.6	1.5	0.97
U	LOCK	4.0x 5.0	-0.5	0.2	0.57
B	LOCK	6.0x 8.0	Ctr	Ctr	0.81
W	LOCK	4.0x 6.0	0.5	0.2	0.89
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	4.0x 5.0	Ctr	0.5	0.62
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	0.5	0.2	0.91
S	LOCK	8.0x12.0	0.6	1.5	0.97
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.75
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x 8.0	Ctr	-1.5	0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.75
L	LOCK	5.0x 8.0	Ctr	Ctr	0.86
V	LOCK	6.0x10.0	Ctr	-1.2	0.97
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
J	LOCK	4.0x 5.0	Ctr	Ctr	0.61
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	8.0x 8.0	Ctr	-1.5	0.78
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9419 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

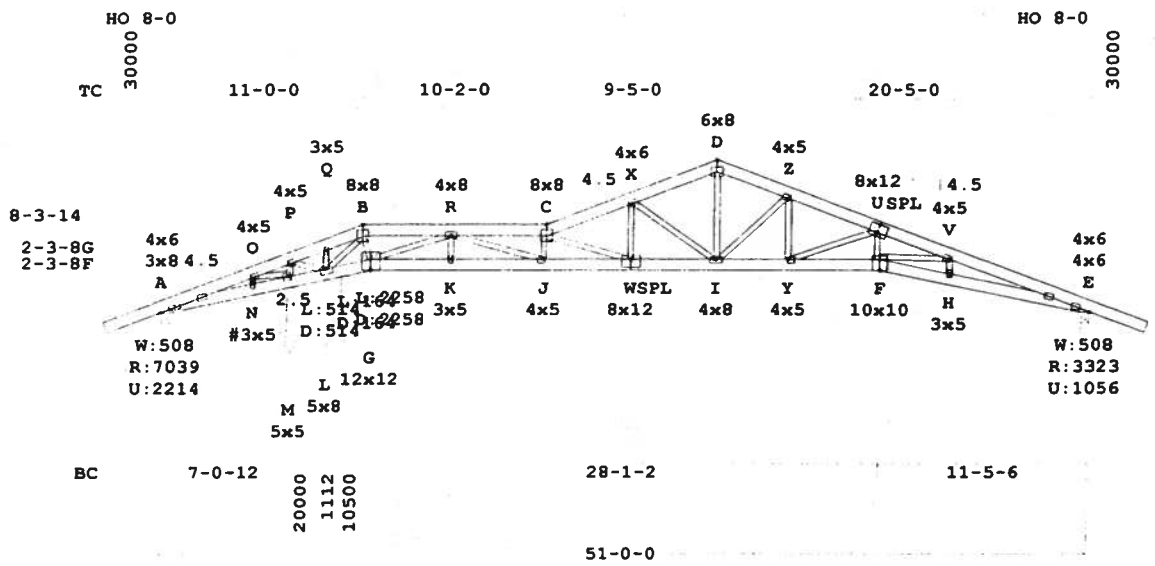
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R146*	1*3P	SP	510000	4.5	3- 0- 0	3- 0- 0	
Office and Care Center, Hospice, M.M. Parrish								

monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 35020 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*R146*	1*3P	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 536.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.42	2x 8	SP-2400
BC	0.73	2x 8	SP-2400
WB	0.86	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	514	514	7.1'	CL-LB
BC V	164	164	9.1'	CL-LB
BC V	2258	2258	10.0'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	7040	2215	5- 8	1-15
			Hz =	-143
E	3324	1057	5- 8	1- 0
			Hz =	143

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -O	0.38	28816	C 0.10	0.28
O -P	0.42	33651	C 0.14	0.28
P -Q	0.31	35020	C 0.15	0.16
Q -B	0.28	34849	C 0.15	0.13
B -R	0.32	32726	C 0.12	0.20
R -C	0.30	28959	C 0.10	0.20
C -X	0.27	15953	C 0.03	0.24
X -D	0.14	9319	C 0.01	0.13
D -Z	0.03	9203	C 0.03	0.00
Z -U	0.09	10729	C 0.01	0.08
U -V	0.11	15179	C 0.02	0.09
V -E	0.09	14620	C 0.02	0.07
--------Bottom Chords---------				
A -N	0.61	27009	T 0.31	0.30
N -M	0.61	27363	T 0.31	0.30
M -L	0.54	32608	T 0.37	0.17
L -G	0.59	29770	T 0.33	0.26
G -K	0.40	29917	T 0.34	0.06
K -J	0.73	29917	T 0.34	0.39
J -W	0.72	29061	T 0.33	0.39
W -I	0.29	15251	T 0.17	0.12
I -Y	0.13	10117	T 0.11	0.02
Y -F	0.18	14081	T 0.16	0.02
F -H	0.25	13964	T 0.16	0.09
H -E	0.25	13879	T 0.16	0.09
------------Webs------------				
N -O	0.08	1696	C	
O -M	0.56	5284	T	
M -P	0.08	1578	C	
P -L	0.12	1196	T	
L -Q	0.04	449	T	
L -B	0.54	5115	T	
B -G	0.86	8068	T	
G -R	0.31	3344	T	
K -R	0.05	656	T	
R -J	0.06	1467	C	
J -C	0.07	1422	C	
C -W	0.80	14539	C	
W -X	0.65	6091	T	
X -I	0.47	8364	C	
I -D	0.69	6484	T	
I -Z	0.11	2107	C	
Y -Z	0.17	1628	T	
Y -U	0.23	4312	C	
F -U	0.33	3177	T	
F -V	0.06	585	T	
H -V	0.01	331	C	

LL Defl -1.12" in K -J L/534
TL Defl -2.26" in K -J L/266
Shear // Grain in L -G 0.51

Hz Disp	LL	DL	TL
Jt E	0.43"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.88
A	LOCK	4.0x 6.0	Ctr	Ctr	0.82
O	LOCK	4.0x 5.0-0.5	0.2		0.85
P	LOCK	4.0x 5.0-0.5	0.2		0.56
Q	LOCK	3.0x 5.0	Ctr	Ctr	0.36
B	LOCK	8.0x 8.0	Ctr	Ctr	0.97
R	LOCK	4.0x 8.0	Ctr	0.5	0.74
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
X	LOCK	4.0x 6.0-0.5	0.2		0.84
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
Z	LOCK	4.0x 5.0	0.5	0.2	0.56
U	LOCK	8.0x12.0	0.6	1.5	0.97
V	LOCK	4.0x 5.0	0.5	0.2	0.61
E	LOCK	4.0x 6.0	Ctr	Ctr	0.69
E	LOCK	4.0x 6.0	Ctr	Ctr	0.30
N#	LOCK	3.0x 5.0-0.1-0.6			0.36
M	LOCK	5.0x 5.0	Ctr	Ctr	0.78
L	LOCK	5.0x 8.0	0.2-1.0		0.97
G	LOCK	12.0x12.0	Ctr-1.2		0.92
K	LOCK	3.0x 5.0	Ctr	Ctr	0.36
J	LOCK	4.0x 5.0	Ctr-0.5		0.71
W	LOCK	8.0x12.0	Ctr-1.5		0.99
I	LOCK	4.0x 8.0	Ctr-0.5		0.97
Y	LOCK	4.0x 5.0	Ctr-0.5		0.68
F	LOCK	10.0x10.0	Ctr-2.2		0.75
H	LOCK	3.0x 5.0	Ctr	Ctr	0.36

= Plate Monitor used

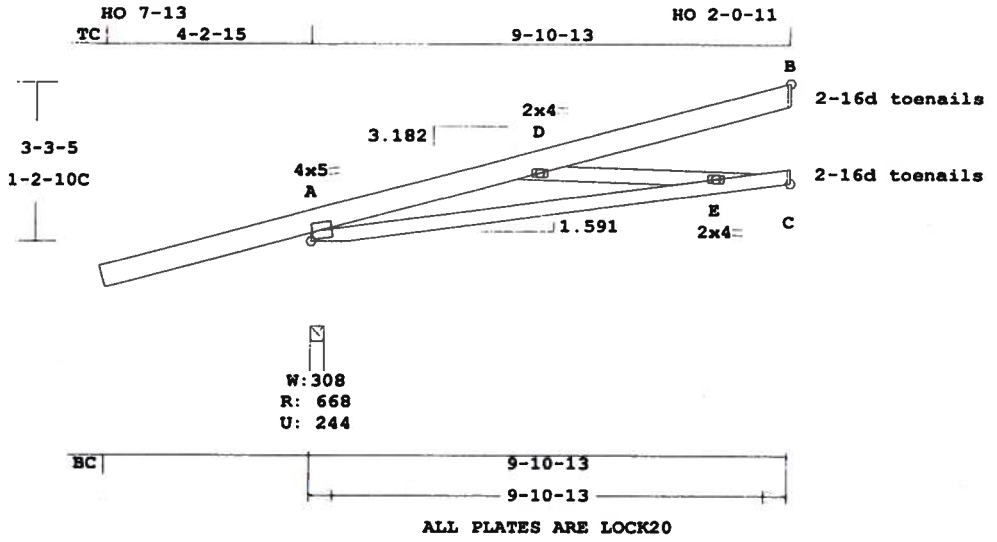
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R145*	1	VKING	91013	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 70.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 6	SP-#2
BC	0.56	2x 4	SP-#2
WB	0.35	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9-10-13
BC Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	669	245	3- 8	1- 0
			Hz =	118
B	194	127	3- 8	1- 0
C	233	20	3- 8	1- 0
			Hz =	64

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.20	883 C	0.05 0.15
D -B	0.22	74 C	0.00 0.22
Bottom Chords			
A -E	0.56	850 T	0.06 0.50
E -C	0.19	66 T	0.00 0.19
Webs			
D -E	0.35	848 C	

LL Defl -0.23" in A -C L/487
TL Defl -0.48" in A -C L/233
Shear // Grain in C -C 0.26

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.7	0.3	0.83
D	LOCK	2.0x 4.0	Ctr	Ctr	0.47
E	LOCK	2.0x 4.0	Ctr	Ctr	0.56

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

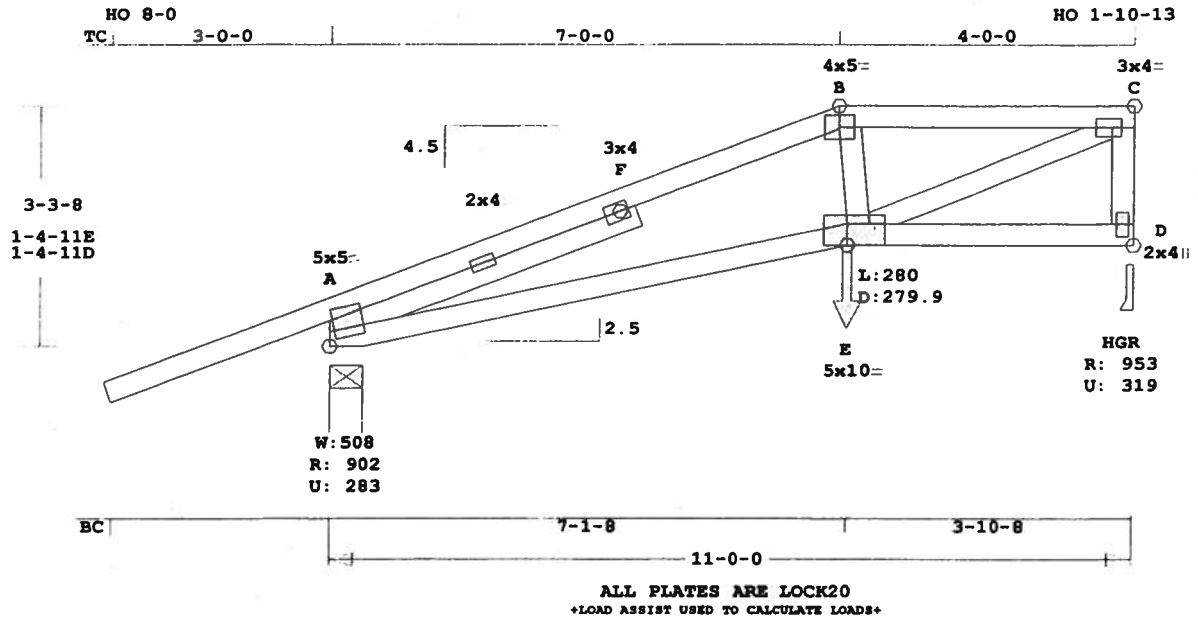
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 883 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R144*	1	SP	110000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
WB	0.64	2x 4	SP-#3
SL	0.21	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 0- 0
BC	Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	11.0'
BC V	0	20	0.0'	11.0'
TC V	50	25	7.0'	10.0'
TC V	-40	-20	10.0'	11.0'
BC V	0	25	7.1'	10.0'
BC V	0	-20	10.0'	11.0'
BC V	280	280	7.1'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	902	284	5- 8	1- 1
			Hz =	121
D	953	320	3- 8	1- 2
			Hz =	66

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.21	1114 C	0.09	0.12
F -B	0.36	1913 C	0.19	0.17
B -C	0.47	1853 C	0.19	0.28
--------Bottom Chords---------				
A -E	0.52	1833 T	0.33	0.19
E -D	0.15	66 T	0.00	0.15
------------Webs------------				
B -E	0.10	400 T		
E -C	0.64	2009 T		
D -C	0.17	913 C		
----------Sliders----------				
A -F	0.21	873 C		

LL Defl -0.05" in A -E L/999
TL Defl -0.11" in A -E L/999
Shear // Grain in B -C 0.29

Hz Disp	LL	DL	TL
Jt D	0.02"	0.02"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	0.97
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.69
C	LOCK	3.0x 4.0-0.5	Ctr		0.97
E	LOCK	5.0x10.0	1.0-1.0		0.95
D	LOCK	2.0x 4.0	Ctr	Ctr	0.56

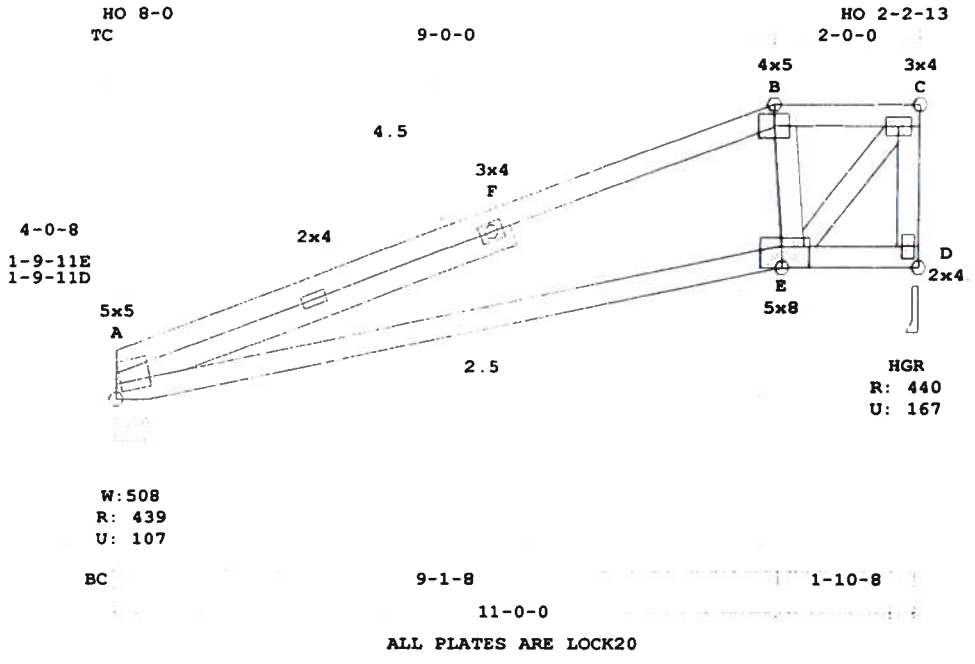
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1913 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R143*	1	SP	110000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.63	2x 4	SP-#2
BC	0.43	2x 4	SP-#2
WB	0.22	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 0
BC Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	440	107	5- 8	1- 0
			Hz =	152
D	440	168	3- 8	1- 0
			Hz =	83

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -F	0.63	801 C	0.00 0.63
F -B	0.61	480 C	0.00 0.61
B -C	0.36	459 C	0.00 0.36
Bottom Chords			
A -E	0.43	510 T	0.00 0.43
E -D	0.16	82 T	0.00 0.16
Webs			
B -E	0.09	381 T	
E -C	0.22	714 T	
D -C	0.09	476 C	
Sliders			
A -F	0.11	416 T	

LL Defl -0.12" in A -E L/999
TL Defl -0.28" in A -E L/443
Shear // Grain in A -F 0.31

Hz Disp	LL	DL	TL
Jt D	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	0.97
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.69
C	LOCK	3.0x 4.0	Ctr	Ctr	0.55
E	LOCK	5.0x 8.0	0.5-1.0		0.89
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

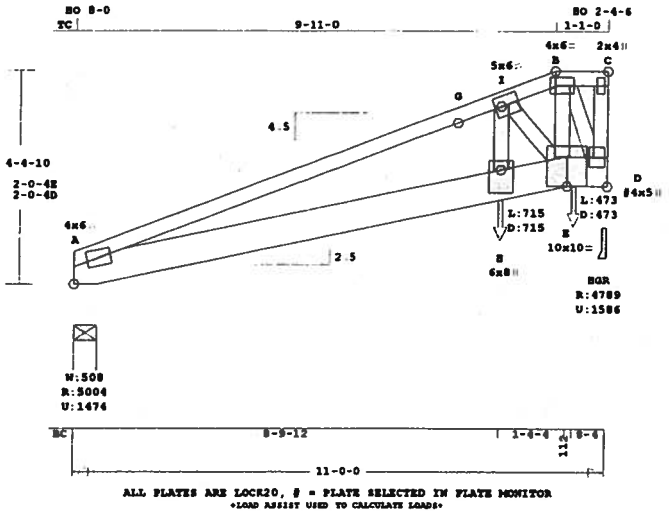
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 801 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R142*	1*2P	SP	110000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 78.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.66	2x 4	SP-2400
EX B -C		2x 4	SP-#2
BC	0.90	2x 8	SP-2400
WB	0.59	2x 4	SP-#3
EX H -I		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 0
BC Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	11.0'
BC V	0	20	0.0'	11.0'
BC V	418	418	0.0'	7.8'
BC V	715	715	8.8'	CL-LB
BC V	473	473	10.3'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	5004	1474	5- 8	2- 1
			Hz =	934
D	4789	1586	3- 8	2- 0
			Hz =	535

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.66	7092 C	0.20	0.46
I -B	0.21	1717 C	0.00	0.21
B -C	0.16	0 T	0.00	0.16
--------Bottom Chords---------				
A -H	0.90	6825 T	0.13	0.77
H -E	0.61	6367 T	0.12	0.49
E -D	0.16	1189 T	0.02	0.14
------------Webs------------				
H -I	0.61	6784 T		
I -E	0.58	7227 C		
E -B	0.59	3681 T		
B -D	0.24	3055 C		
D -C	0.02	284 C		

LL Defl -0.18" in A -H L/681
TL Defl -0.36" in A -H L/337
Shear // Grain in A -H 0.86

Hz Disp	LL	DL	TL
Jt D	0.02"	0.02"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.97
I	LOCK	5.0x 6.0	-0.5	-0.2	0.93
B	LOCK	4.0x 6.0	1.0	Ctr	0.85
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	6.0x 8.0	Ctr	-1.6	0.94
E	LOCK	10.0x10.0	Ctr	-2.3	0.63
D	LOCK	4.0x 5.0	0.9	Ctr	0.57

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

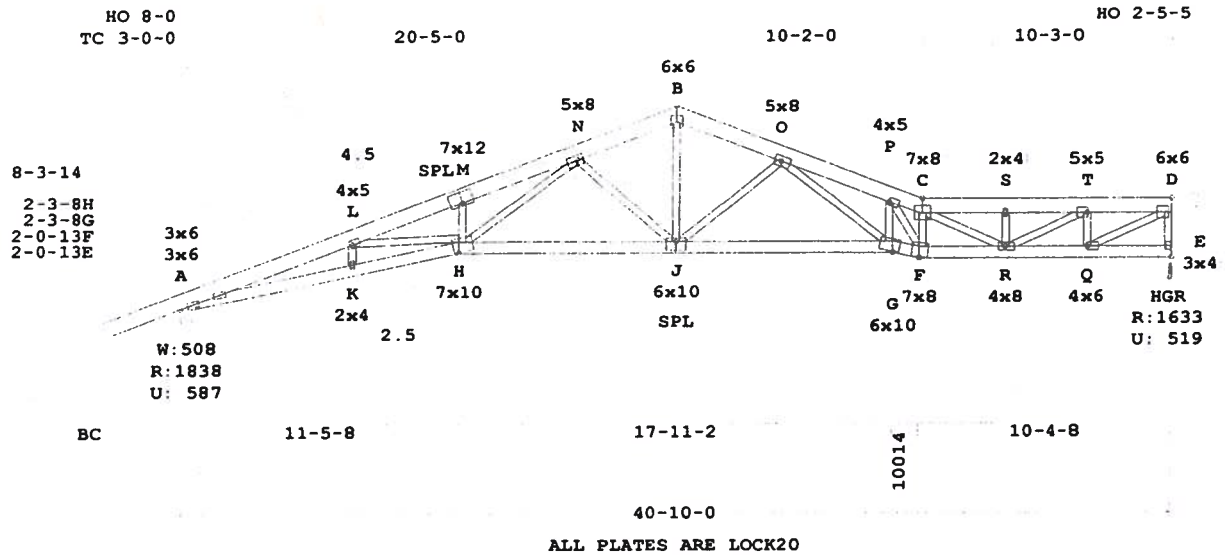
	Rows	----Spacing (In)---- Nails	Screws	Bolts
TC	1	12	24	0
BC	2	7.5	20	0
WB	1	4	4	

Plus clusters of nails where shown.
Provide connection to bearing for 933 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7227 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R141*	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 392.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 8	SP-2400
BC	0.85	2x 6	SP-#2
EX A -H		2x 6	SP-2400
WB	0.93	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	1839	587	5- 8	1- 8
			Hz =	174
E	1633	519	3- 8	1-15
			Hz =	114

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.22	6381 C	0.04		0.18
L -M	0.24	6592 C	0.05		0.19
M -N	0.14	6578 C	0.05		0.09
N -B	0.06	3268 C	0.04		0.02
B -O	0.06	3262 C	0.04		0.02
O -P	0.14	6466 C	0.04		0.10
P -C	0.15	6964 C	0.05		0.10
C -S	0.06	4740 C	0.05		0.01
S -T	0.06	4740 C	0.05		0.01
T -D	0.04	2546 C	0.03		0.01
Bottom Chords					
A -K	0.48	6023 T	0.27		0.21
K -H	0.48	6073 T	0.27		0.21
H -J	0.69	4022 T	0.48		0.21
J -G	0.70	4087 T	0.49		0.21
G -F	0.82	6260 T	0.76		0.06
F -R	0.85	6682 T	0.81		0.04
R -Q	0.36	2546 T	0.30		0.06
Q -E	0.04	113 T	0.00		0.04
Webs					
K -L	0.04	174 T			
L -H	0.08	278 T			
H -M	0.05	244 C			
H -N	0.88	2757 T			
N -J	0.60	1336 C			
J -B	0.66	2067 T			
J -O	0.64	1375 C			
O -G	0.83	2602 T			
G -P	0.05	200 T			
P -F	0.20	626 T			
F -C	0.30	1900 C			
C -R	0.50	2159 C			
R -S	0.04	261 C			
R -T	0.80	2523 T			
Q -T	0.22	1381 C			
Q -D	0.93	2928 T			
E -D	0.25	1587 C			

LL Defl -0.45" in H -J L/999
TL Defl -0.94" in H -J L/510
Shear // Grain in P -C 0.18

Hz Disp	LL	DL	TL
Jt E	0.18"	0.18"	0.36"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

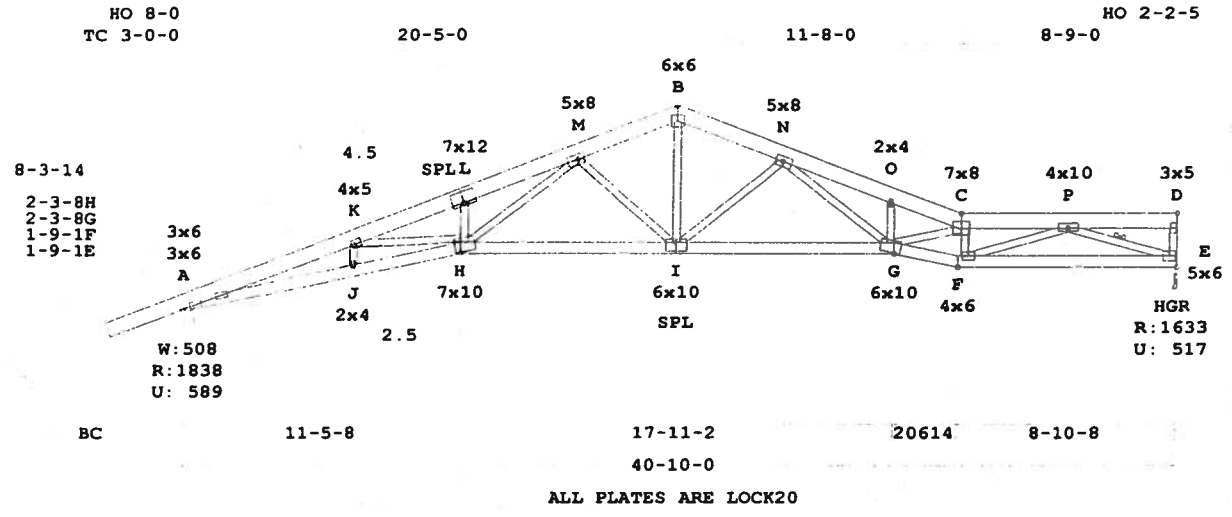
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	Ctr	Ctr	0.78
A	LOCK	3.0x 6.0	Ctr	Ctr	0.56
L	LOCK	4.0x 5.0-0.5	0.2		0.62
M	LOCK	7.0x12.0-0.7	2.0		0.97
N	LOCK	5.0x 8.0-1.1-0.4			0.98
B	LOCK	6.0x 6.0	Ctr	Ctr	0.77
O	LOCK	5.0x 8.0	0.4-0.2		0.96
P	LOCK	4.0x 5.0	0.5	0.2	0.58
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
S	LOCK	2.0x 4.0	Ctr	Ctr	0.48
T	LOCK	5.0x 5.0-0.5	Ctr		0.95
D	LOCK	6.0x 6.0-1.0	Ctr		0.97
K	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	7.0x10.0	0.2-1.2		0.95
J	LOCK	6.0x10.0	Ctr-1.2		0.97
G	LOCK	6.0x10.0-1.4-1.6			0.94
F	LOCK	7.0x 8.0	0.4	3.9	0.81
R	LOCK	4.0x 8.0	0.5	Ctr	0.99
Q	LOCK	4.0x 6.0	1.0	Ctr	0.97
E	LOCK	3.0x 4.0	Ctr	Ctr	0.92

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6964 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	***R140***	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 385.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.24	2x 8	SP-2400
BC	0.74	2x 6	SP-#2
EX A -H		2x 6	SP-2400
EX F -E		2x 6	SP-2400
WB	0.88	2x 4	SP-#3
EX F -P		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

WB 1 rows CLB on P -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	1839	589	5- 8	1- 8
			Hz =	149
E	1633	517	3- 8	1- 6
			Hz =	117

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.22	6383	C	0.04	0.18
K -L	0.24	6589	C	0.05	0.19
L -M	0.14	6575	C	0.05	0.09
M -B	0.06	3275	C	0.04	0.02
B -N	0.06	3273	C	0.04	0.02
N -O	0.11	6554	C	0.04	0.07
O -C	0.12	6595	C	0.04	0.08
C -P	0.09	6413	C	0.04	0.05
P -D	0.06	0	T	0.00	0.06
Bottom Chords					
A -J	0.47	6025	T	0.27	0.20
J -H	0.47	6075	T	0.27	0.20
H -I	0.73	4026	T	0.48	0.25
I -G	0.74	4084	T	0.49	0.25
G -F	0.43	6765	T	0.30	0.13
F -E	0.31	3402	T	0.15	0.16
Webs					
J -K	0.04	172	T		
K -H	0.08	273	T		
H -L	0.05	245	C		
H -M	0.88	2748	T		
M -I	0.60	1331	C		
I -B	0.66	2083	T		
I -N	0.64	1363	C		
N -G	0.85	2678	T		
G -O	0.04	169	T		
G -C	0.09	453	C		
F -C	0.40	2545	C		
F -P	0.59	3234	T		
P -E	0.64	3655	C		1 Br
E -D	0.02	175	C		

LL Defl -0.46" in H -I L/999
TL Defl -0.95" in I -G L/506
Shear // Grain in A -K 0.17

Hz Disp	LL	DL	TL
Jt E	0.18"	0.18"	0.37"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

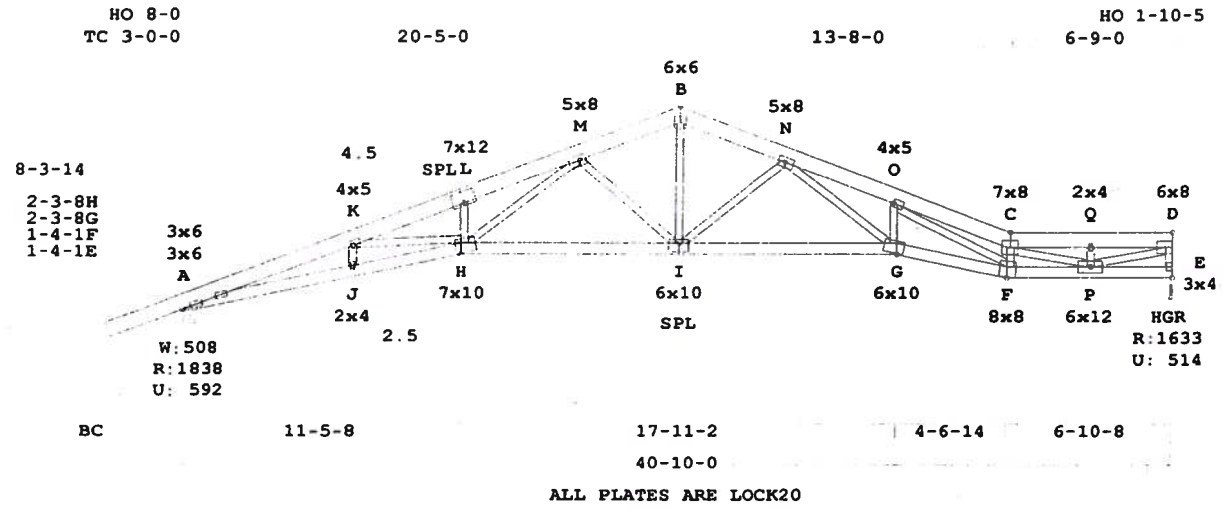
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	Ctr	Ctr	0.78
A	LOCK	3.0x 6.0	Ctr	Ctr	0.56
K	LOCK	4.0x 5.0	-0.5	0.2	0.62
L	LOCK	7.0x12.0	-0.7	2.0	0.97
M	LOCK	5.0x 8.0	-1.1	-0.4	0.97
B	LOCK	6.0x 6.0	Ctr	Ctr	0.77
N	LOCK	5.0x 8.0	0.4	-0.2	0.99
O	LOCK	2.0x 4.0	Ctr	Ctr	0.48
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
P	LOCK	4.0x10.0	Ctr	0.5	0.97
D	LOCK	3.0x 5.0	Ctr	Ctr	0.99
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	7.0x10.0	0.2	-1.2	0.95
I	LOCK	6.0x10.0	Ctr	-1.2	0.97
G	LOCK	6.0x10.0	0.1	-1.6	0.94
F	LOCK	4.0x 6.0	Ctr	Ctr	0.91
E	LOCK	5.0x 6.0	Ctr	Ctr	0.86

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6595 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R139*	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 386.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 8	SP-2400
BC	1.00	2x 6	SP-#2
EX	A -H	2x 6	SP-2400
WB	0.88	2x 4	SP-#3
EX	P -D	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1839	592	5- 8	1- 8
			Hz =	115
E	1633	515	3- 8	1-15
			Hz =	123

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.22	6381 C	0.04		0.18
K -L	0.24	6592 C	0.05		0.19
L -M	0.14	6578 C	0.05		0.09
M -B	0.06	3268 C	0.04		0.02
B -N	0.06	3261 C	0.04		0.02
N -O	0.11	6551 C	0.04		0.07
O -C	0.13	7253 C	0.06		0.07
C -Q	0.16	4251 C	0.01		0.15
Q -D	0.16	4251 C	0.01		0.15
Bottom Chords					
A -J	0.48	6023 T	0.27		0.21
J -H	0.48	6074 T	0.27		0.21
H -I	0.69	4023 T	0.48		0.21
I -G	0.70	4082 T	0.49		0.21
G -F	1.00	6320 T	0.76		0.24
F -P	1.00	7215 T	0.87		0.13
P -E	0.13	122 T	0.00		0.13
Webs					
J -K	0.04	171 T			
K -H	0.08	278 T			
H -L	0.05	244 C			
H -M	0.88	2757 T			
M -I	0.60	1335 C			
I -B	0.66	2063 T			
I -N	0.64	1367 C			
N -G	0.86	2697 T			
G -O	0.07	285 T			
O -F	0.22	691 T			
F -C	0.27	1728 C			
C -P	0.66	3091 C			
P -Q	0.10	686 C			
P -D	0.84	4582 T			
E -D	0.25	1640 C			

LL Defl -0.50" in I -G L/964
TL Defl -1.03" in H -I L/465
Shear // Grain in C -Q 0.23

Hz Disp	LL	DL	TL
Jt E	0.20"	0.20"	0.40"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

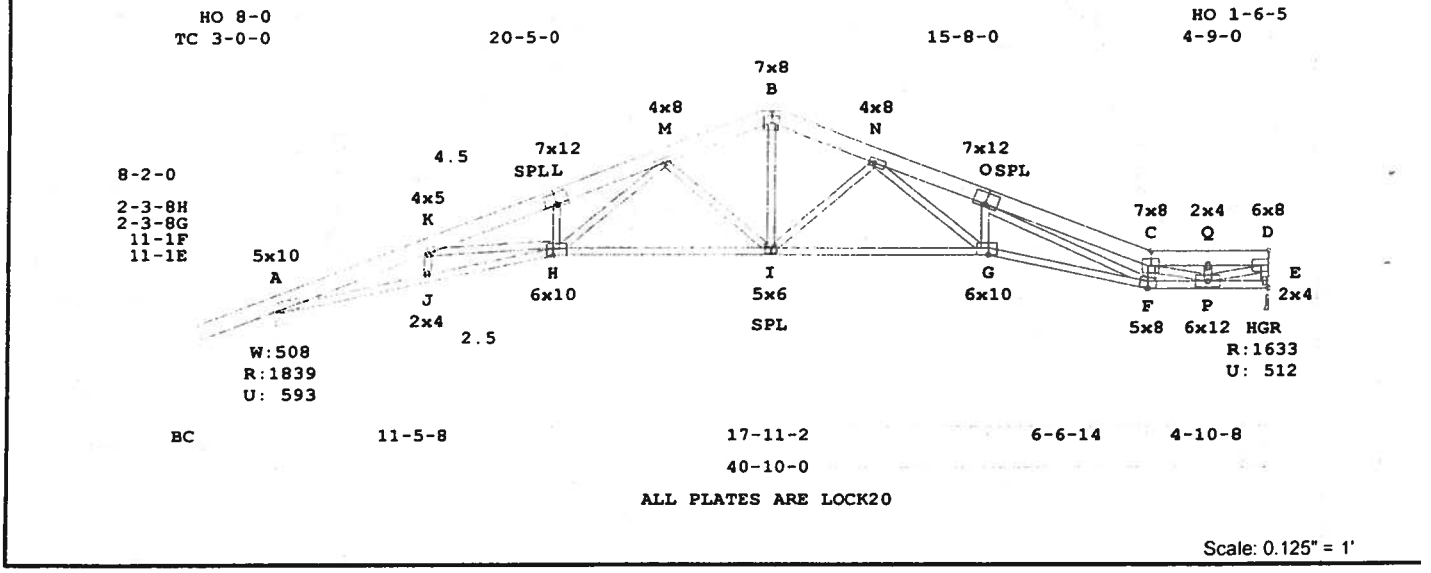
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	Ctr	Ctr	0.78
A	LOCK	3.0x 6.0	Ctr	Ctr	0.56
K	LOCK	4.0x 5.0	-0.5	0.2	0.62
L	LOCK	7.0x12.0	-0.7	2.0	0.97
M	LOCK	5.0x 8.0	-1.1	-0.4	0.98
B	LOCK	6.0x 6.0	Ctr	Ctr	0.77
N	LOCK	5.0x 8.0	0.4	-0.2	1.00
O	LOCK	4.0x 5.0	0.5	0.2	0.93
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.48
D	LOCK	6.0x 8.0	-0.5	Ctr	1.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	7.0x10.0	0.2	-1.2	0.95
I	LOCK	6.0x10.0	Ctr	-1.2	0.97
G	LOCK	6.0x10.0	-1.4	-1.6	0.98
F	LOCK	8.0x 8.0	0.5	4.4	0.88
P	LOCK	6.0x12.0	Ctr	Ctr	1.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.92

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7253 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R138*	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 349.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.32	2x 8	SP-2400
BC	0.61	2x 4	SP-2400
EX H -I		2x 4	SP-#2
EX I -G		2x 4	SP-#2
WB	0.86	2x 4	SP-#3
EX P -D		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	1839	594	5- 8	1- 8
			Hz =	-118
E	1633	513	3- 8	1- 6
			Hz =	128

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.30	5932	C	0.03	0.27
K -L	0.32	6384	C	0.04	0.28
L -M	0.09	6427	C	0.04	0.05
M -B	0.06	3195	C	0.04	0.02
B -N	0.06	3193	C	0.04	0.02
N -O	0.11	6298	C	0.04	0.07
O -C	0.17	6557	C	0.05	0.12
C -Q	0.19	3965	C	0.01	0.18
Q -D	0.19	3965	C	0.01	0.18
Bottom Chords					
A -J	0.57	5536	T	0.39	0.18
J -H	0.55	5558	T	0.39	0.16
H -I	0.96	3976	T	0.66	0.30
I -G	0.96	3975	T	0.66	0.30
G -F	0.61	6057	T	0.43	0.18
F -P	0.54	6689	T	0.48	0.06
P -E	0.05	127	T	0.00	0.05
Webs					
J -K	0.03	123	T		
K -H	0.19	601	T		
H -L	0.07	426	C		
H -M	0.86	2683	T		
M -I	0.64	1315	C		
I -B	0.62	1953	T		
I -N	0.64	1315	C		
N -G	0.82	2563	T		
G -O	0.08	328	T		
O -F	0.19	235	C		
F -C	0.21	1385	C		
C -P	0.49	2858	C		
P -Q	0.14	929	C		
P -D	0.80	4378	T		
E -D	0.25	1627	C		

LL Defl -0.55" in H -I L/875
TL Defl -1.21" in H -I L/398
Shear // Grain in C -Q 0.32

Hz Disp	LL	DL	TL
Jt E	0.27"	0.27"	0.55"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

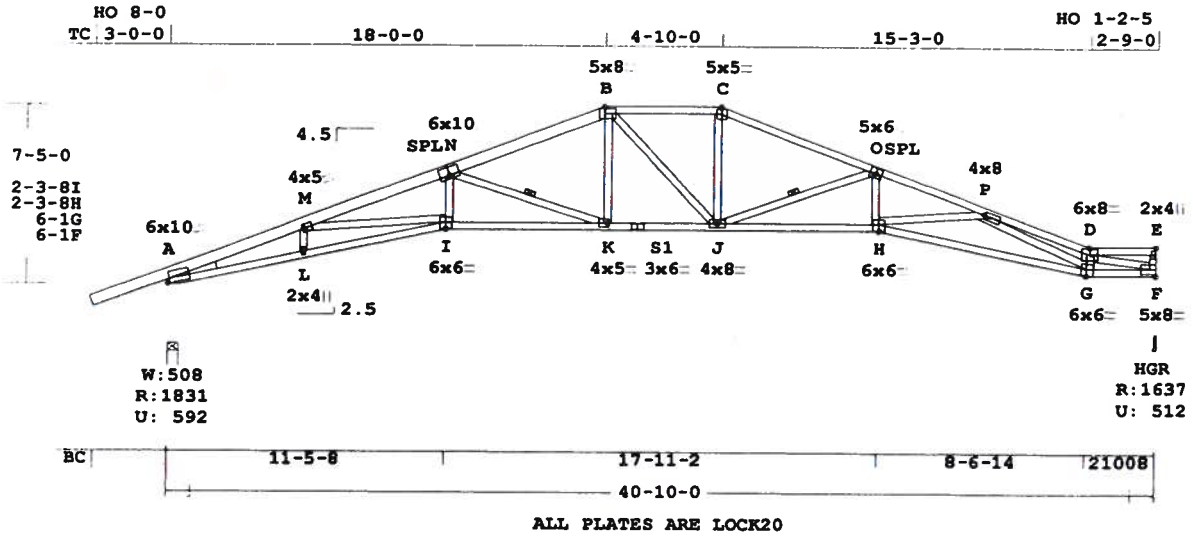
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	0.8	0.94
K	LOCK	4.0x 5.0-0.5	0.2	0.63	
L	LOCK	7.0x12.0-0.7	2.0	0.97	
M	LOCK	4.0x 8.0-1.3-0.1	1.00		
B	LOCK	7.0x 8.0	Ctr	Ctr	0.64
N	LOCK	4.0x 8.0	1.3-0.1	0.95	
O	LOCK	7.0x12.0	0.7	2.0	0.97
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.48
D	LOCK	6.0x 8.0-0.5	Ctr	Ctr	0.96
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	6.0x10.0	Ctr-0.5		0.87
I	LOCK	5.0x 6.0	Ctr-0.5		0.88
G	LOCK	6.0x10.0-0.5-0.5			0.96
F	LOCK	5.0x 8.0	0.3	2.9	0.95
P	LOCK	6.0x12.0	Ctr	Ctr	0.96
E	LOCK	2.0x 4.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6557 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R137	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 290.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.97	2x 4	SP-#2
EX A -N		2x 6	SP-2400
EX N -B		2x 6	SP-#2
BC	0.93	2x 4	SP-2400
WB	0.88	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on J -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1832	593	5- 8	1- 8
			Hz =	-110
F	1637	513	3- 8	1- 6
			Hz =	116

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -M	0.76	5876 C	0.06 0.70
M -N	0.43	6269 C	0.09 0.34
N -B	0.31	3672 C	0.15 0.16
B -C	0.32	3394 C	0.20 0.12
C -O	0.65	3622 C	0.22 0.43
O -P	0.63	6021 C	0.36 0.27
P -D	0.64	4810 C	0.16 0.48
D -E	0.97	0 T	0.00 0.97
Bottom Chords			
A -L	0.93	5464 T	0.39 0.54
L -I	0.63	5489 T	0.39 0.24
I -K	0.57	5867 T	0.42 0.15
K -S1	0.27	3413 T	0.24 0.03
S1-J	0.28	3413 T	0.24 0.04
J -H	0.50	5603 T	0.40 0.10
H -G	0.59	5734 T	0.41 0.18
G -F	0.53	4659 T	0.33 0.20
Webs			
L -M	0.03	138 T	
M -I	0.17	540 T	
I -N	0.41	1289 T	
N -K	0.59	2583 C	1 Br
K -B	0.30	960 T	
B -J	0.07	130 C	
J -C	0.30	944 T	
J -O	0.54	2356 C	1 Br
H -O	0.42	1319 T	
H -P	0.05	238 T	
P -G	0.53	1287 C	
G -D	0.09	533 C	
D -F	0.88	4821 C	
F -E	0.04	305 C	

LL Defl -0.69" in S1-J L/700
TL Defl -1.38" in S1-J L/349
Shear // Grain in A -M 0.40

Hz Disp	LL	DL	TL
Jt F	0.36"	0.36"	0.72"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

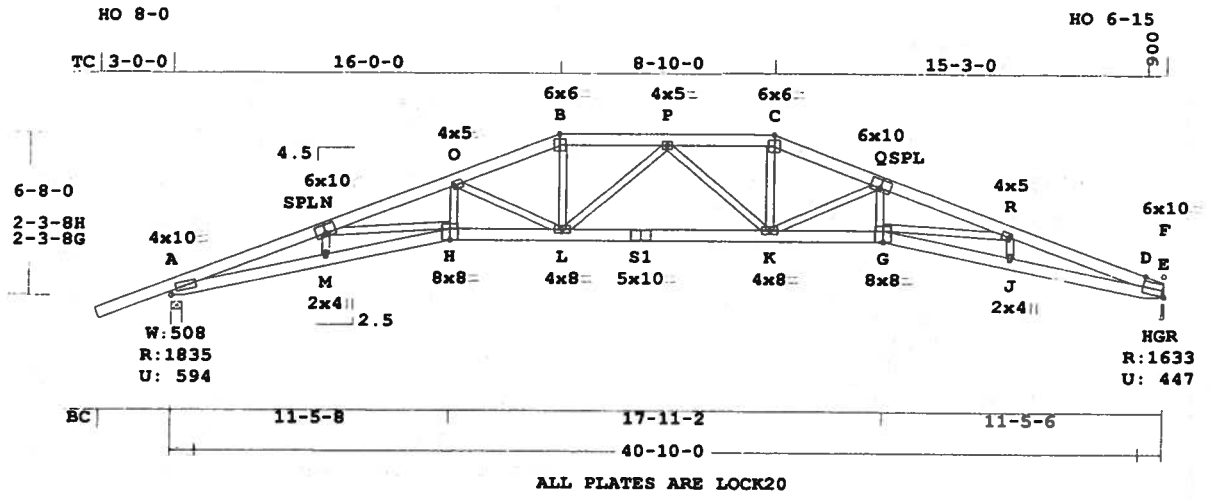
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
M	LOCK	4.0x 5.0	Ctr	Ctr	0.64
N	LOCK	6.0x10.0	-0.5	1.2	0.95
B	LOCK	5.0x 8.0	-0.5	Ctr	0.97
C	LOCK	5.0x 5.0	Ctr	Ctr	0.96
O	LOCK	5.0x 6.0	0.2	0.5	0.87
P	LOCK	4.0x 8.0	Ctr	Ctr	0.91
D	LOCK	6.0x 8.0	Ctr	Ctr	0.83
E	LOCK	2.0x 4.0	Ctr	Ctr	0.56
L	LOCK	2.0x 4.0	Ctr	Ctr	0.48
I	LOCK	6.0x 6.0	Ctr	-0.5	0.93
K	LOCK	4.0x 5.0	Ctr	Ctr	0.75
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	4.0x 8.0	Ctr	Ctr	0.60
H	LOCK	6.0x 6.0	Ctr	-0.5	0.96
G	LOCK	6.0x 6.0	0.3	3.3	0.98
F	LOCK	5.0x 8.0	-0.5	Ctr	0.89

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6269 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R136	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 329.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 6	SP-#2
BC	0.49	2x 6	SP-2400
EX	H -S1	2x 6	SP-#2
EX	S1-G	2x 6	SP-#2
WB	0.78	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1836	594	5- 8	1- 8
			Hz =	-106
F	1633	447	3- 8	1- 6
			Hz =	107

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.50	6250	C	0.11	0.39
N -O	0.52	6374	C	0.14	0.38
O -B	0.25	4288	C	0.17	0.08
B -P	0.20	4028	C	0.16	0.04
P -C	0.20	4028	C	0.16	0.04
C -Q	0.24	4288	C	0.17	0.07
Q -R	0.53	6377	C	0.14	0.39
R -F	0.49	6256	C	0.11	0.38
Bottom Chords					
A -M	0.48	5882	T	0.26	0.22
M -H	0.49	5939	T	0.27	0.22
H -L	0.78	5932	T	0.72	0.06
L -S1	0.59	4175	T	0.50	0.09
S1-K	0.61	4175	T	0.50	0.11
K -G	0.79	5935	T	0.72	0.07
G -J	0.48	5943	T	0.27	0.21
J -F	0.48	5887	T	0.27	0.21
Webs					
M -N	0.04	187	T		
N -H	0.05	183	T		
H -O	0.44	1386	T		
O -L	0.78	2123	C		
L -B	0.38	1204	T		
L -P	0.09	198	C		
P -K	0.09	198	C		
K -C	0.38	1205	T		
K -Q	0.78	2127	C		
G -Q	0.44	1387	T		
G -R	0.05	182	T		
J -R	0.04	188	T		

LL Defl -0.55" in S1-K L/878
TL Defl -1.12" in S1-K L/427
Shear // Grain in A -N 0.24

Hz Disp	LL	DL	TL
Jt F	0.31"	0.32"	0.63"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

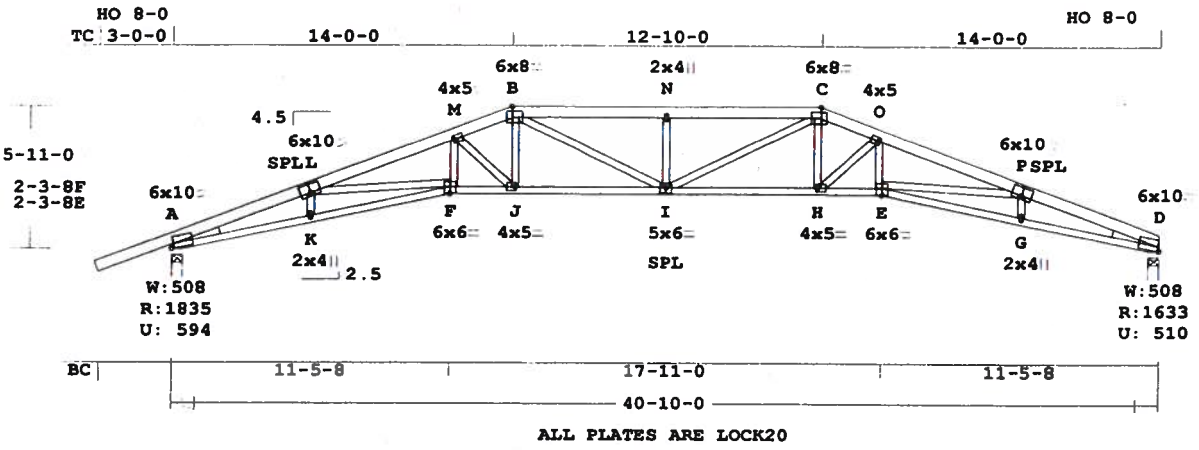
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0	Ctr	Ctr	0.99
N	LOCK	6.0x10.0	-0.5	1.2	0.95
O	LOCK	4.0x 5.0	Ctr	Ctr	0.69
B	LOCK	6.0x 6.0	Ctr	Ctr	0.90
P	LOCK	4.0x 5.0	Ctr	Ctr	0.48
C	LOCK	6.0x 6.0	Ctr	Ctr	0.90
Q	LOCK	6.0x10.0	0.5	1.2	0.95
R	LOCK	4.0x 5.0	Ctr	Ctr	0.64
F	LOCK	6.0x10.0	-5.5	1.2	0.94
M	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	8.0x 8.0	Ctr	-1.5	0.72
L	LOCK	4.0x 8.0	Ctr	Ctr	0.57
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.57
G	LOCK	8.0x 8.0	Ctr	-1.5	0.72
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6377 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R135	2	HPSC4	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 301.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.80	2x 6	SP-#2
EX A -L		2x 6	SP-2400
EX P -D		2x 6	SP-2400
BC	0.95	2x 4	SP-2400
WB	0.41	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1835	595	5- 8	1- 8
			Hz =	-94
D	1633	511	5- 8	1- 6
			Hz =	97

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.76	5928 C	0.06	0.70
L -M	0.80	6261 C	0.14	0.66
M -B	0.24	4923 C	0.19	0.05
B -N	0.29	5188 C	0.09	0.20
N -C	0.29	5187 C	0.09	0.20
C -O	0.24	4924 C	0.19	0.05
O -P	0.80	6262 C	0.14	0.66
P -D	0.76	5929 C	0.06	0.70
Bottom Chords				
A -K	0.95	5516 T	0.39	0.56
K -F	0.63	5544 T	0.39	0.24
F -J	0.59	5852 T	0.42	0.17
J -I	0.40	4583 T	0.32	0.08
I -H	0.40	4583 T	0.32	0.08
H -E	0.59	5852 T	0.42	0.17
E -G	0.63	5544 T	0.39	0.24
G -D	0.95	5517 T	0.39	0.56
Webs				
K -L	0.03	139 T		
L -F	0.15	473 T		
F -M	0.41	1305 T		
M -J	0.35	1664 C		
J -B	0.36	1143 T		
B -I	0.21	670 T		
I -N	0.08	437 C		
I -C	0.21	669 T		
H -C	0.36	1143 T		
H -O	0.35	1664 C		
E -O	0.41	1305 T		
E -P	0.15	473 T		
G -P	0.03	139 T		

LL Defl -0.73" in I -H L/652
TL Defl -1.50" in I -H L/319
Shear // Grain in A -L 0.39

Hz Disp	LL	DL	TL
Jt D	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

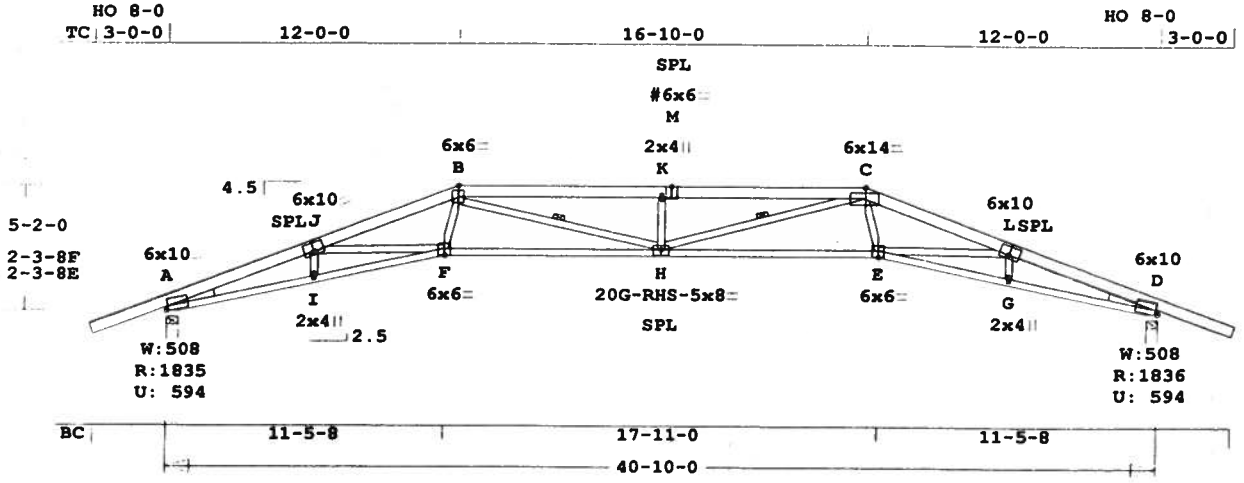
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
L	LOCK	6.0x10.0	-0.5	1.2	0.95
M	LOCK	4.0x 5.0	Ctr	Ctr	0.65
B	LOCK	6.0x 8.0	-0.5	Ctr	0.90
N	LOCK	2.0x 4.0	Ctr	Ctr	0.48
C	LOCK	6.0x 8.0	0.5	Ctr	0.90
O	LOCK	4.0x 5.0	Ctr	Ctr	0.65
P	LOCK	6.0x10.0	0.5	1.2	0.95
D	LOCK	6.0x10.0	-4.6	1.5	0.91
K	LOCK	2.0x 4.0	Ctr	Ctr	0.48
F	LOCK	6.0x 6.0	Ctr	-0.5	0.94
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	5.0x 6.0	Ctr	-0.5	0.88
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x 6.0	Ctr	-0.5	0.94
G	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6262 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R134	2	HPSC4	401000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 293.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.87	2x 6	SP-#2
EX A -J		2x 6	SP-2400
EX L -D		2x 6	SP-2400
BC	0.99	2x 4	SP-2400
WB	0.46	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

WB 1 rows CLB on B -H
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1835	595	5- 8	1- 8
			Hz =	-83
D	1837	595	5- 8	1- 8
			Hz =	83

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.75	5939	C	0.06	0.69
J -B	0.83	6256	C	0.14	0.69
B -K	0.53	6757	C	0.22	0.31
K -M	0.37	6757	C	0.24	0.13
M -C	0.67	6757	C	0.21	0.46
C -L	0.87	6378	C	0.15	0.72
L -D	0.75	5886	C	0.06	0.69
Bottom Chords					
A -I	0.99	5540	T	0.39	0.60
I -F	0.69	5581	T	0.40	0.29
F -H	0.53	5464	T	0.24	0.29
H -E	0.53	5426	T	0.39	0.14
E -G	0.66	5524	T	0.39	0.27
G -D	0.96	5486	T	0.39	0.57
Webs					
I -J	0.03	150	T		
J -F	0.14	440	T		
F -B	0.45	1409	T		
B -H	0.42	1337	T		1 Br
H -K	0.08	488	C		
H -C	0.44	1382	T		1 Br
C -E	0.46	1451	T		
E -L	0.20	632	T		
G -L	0.03	146	T		

LL Defl -0.86" in H -E L/554
TL Defl -1.83" in H -E L/261
Shear // Grain in L -D 0.39

Hz Disp	LL	DL	TL
Jt D	0.46"	0.46"	0.92"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

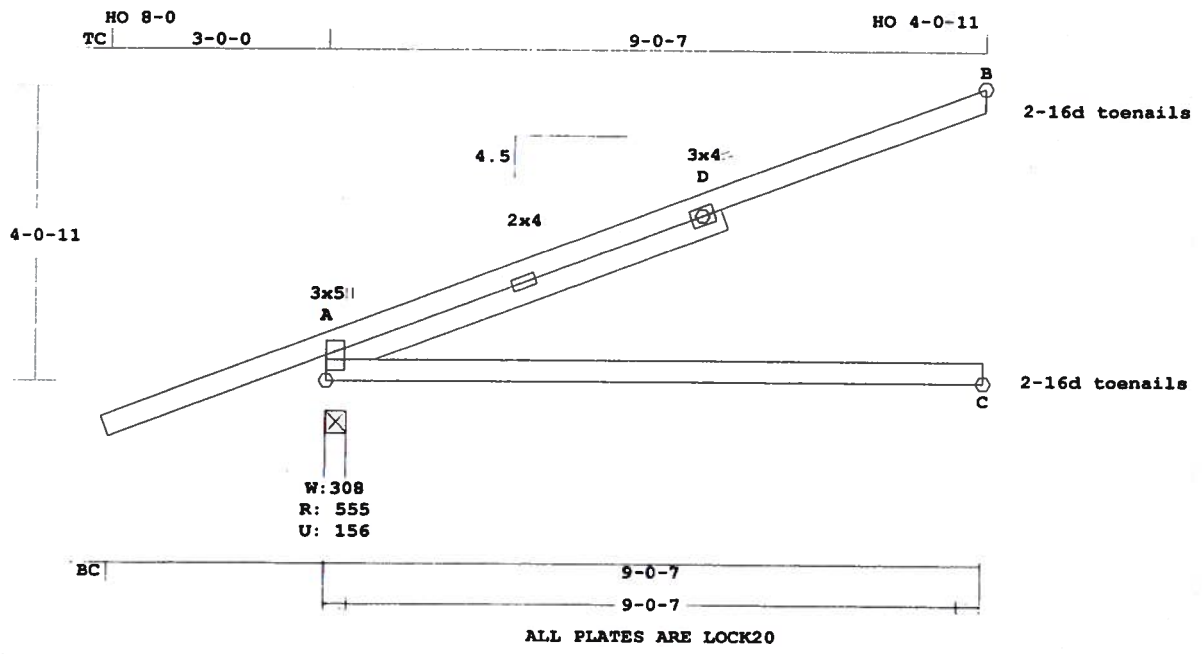
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
J	LOCK	6.0x10.0-0.5	1.2		0.95
B	LOCK	6.0x 6.0 Ctr	Ctr		0.90
K	LOCK	2.0x 4.0 Ctr	Ctr		0.48
M	LOCK	6.0x 6.0 Ctr-0.4			0.95
C	LOCK	6.0x14.0-0.5-0.1			0.96
L	LOCK	6.0x10.0	0.5	1.2	0.95
D	LOCK	6.0x10.0-4.6	1.5		0.91
I	LOCK	2.0x 4.0 Ctr	Ctr		0.48
F	LOCK	6.0x 6.0 Ctr-0.5			0.94
H	RHS	5.0x 8.0 Ctr-0.5			0.88
E	LOCK	6.0x 6.0 Ctr-0.5			0.93
G	LOCK	2.0x 4.0 Ctr	Ctr		0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6757 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R133*	1	JACK	90007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 52.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.91	2x 4	SP-#2
BC	0.53	2x 4	SP-#2
SL	0.20	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0- 7
BC	Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	556	157	3- 8	1- 0
			Hz =	152
B	266	173	3- 8	1- 0
C	153	0	3- 8	1- 0
			Hz =	82

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.91	713	C 0.00 0.91
D -B	0.91	130	C 0.00 0.91
--------Bottom Chords---------			
A -C	0.53	81	T 0.00 0.53
----------Sliders------------			
A -D	0.20	716	T

LL Defl -0.15" in A -C L/656
TL Defl -0.38" in A -C L/265
Shear // Grain in A -D 0.38

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.73
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

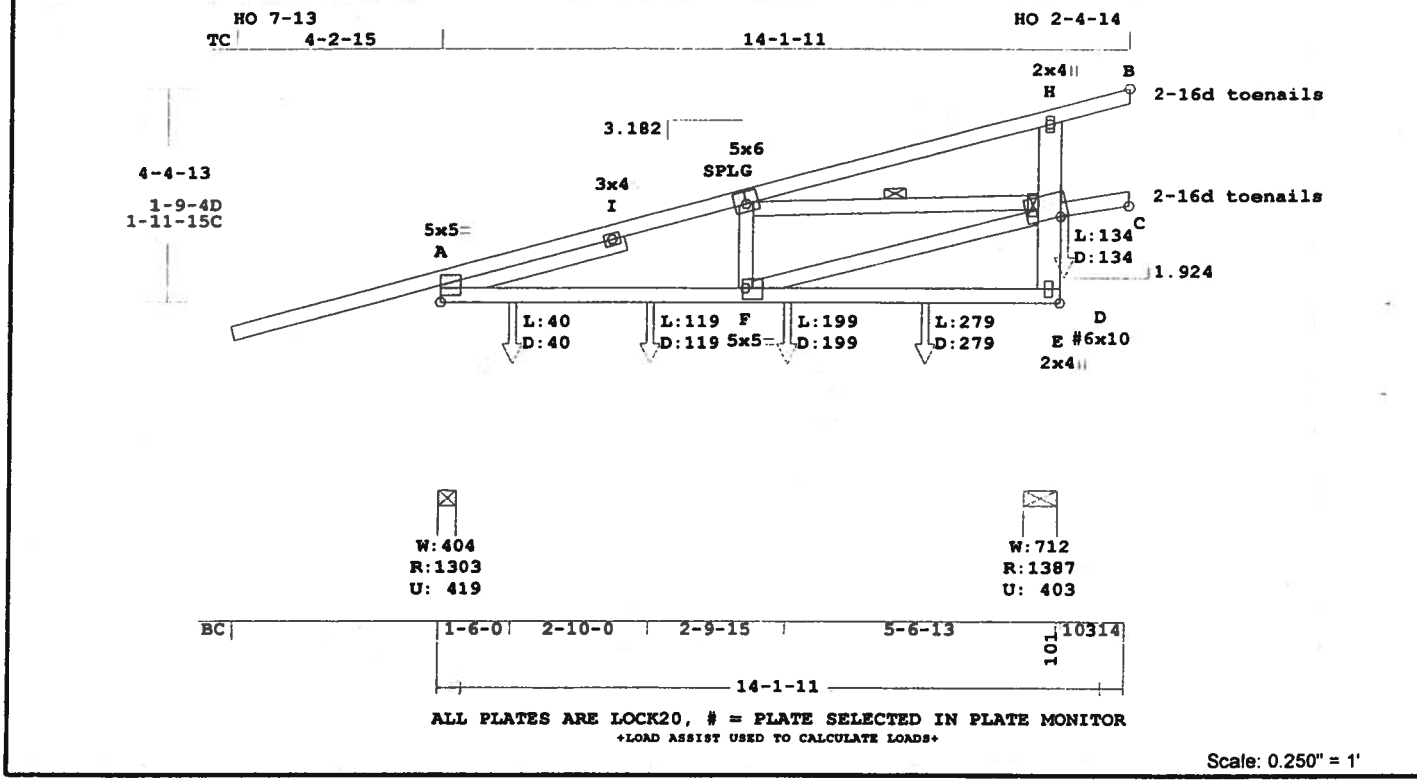
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 713 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R128	1	SP	140111	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 105.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.66	2x 4	SP-#2
BC	0.98	2x 4	SP-2400
EX D -D		2x 4	SP-#2
CW	0.03	2x 6	SP-#2
WB	0.61	2x 4	SP-#3
SL	0.17	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 1-11
BC Cont.	0- 0- 0	14- 1-11

WB 1 rows CLB on G -D
WB 1 rows CLB on E -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.1'
BC V	0	20	0.0'	14.1'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.2'	CL-LB
BC V	279	279	10.0'	CL-LB
BC V	134	134	12.8'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd	Lbs	Lbs	In-Sx	In-Sx
A	1303	420	4- 4	1- 1			
			Hz =	115			
D	1388	403	7-12	1- 2			
B	214	139	3- 8	1- 0			
			Hz =	18			
C	77	1	3- 8	1- 0			

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -I	0.23	1501 C	0.01	0.22	
I -G	0.56	2281 C	0.15	0.41	
G -H	0.66	75 C	0.00	0.66	
H -B	0.43	67 C	0.00	0.43	
Bottom Chords					
A -F	0.65	2101 T	0.16	0.49	
F -E	0.98	278 T	0.01	0.97	
E -D	0.27	242 T	0.04	0.23	
D -C	0.16	15 C	0.00	0.16	
Chord-Webs					
D -F	0.03	115 T	0.03	0.00	
E -D	0.00	115 T		1 Br	
Webs					
F -G	0.13	518 T			
G -D	0.45	2218 C		1 Br	
F -D	0.61	1929 T			
Sliders					
A -I	0.17	836 C			

LL Defl 0.12" in F -E L/999
TL Defl 0.22" in F -E L/640
Shear // Grain in F -E 0.68

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.3	0.78
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	5.0x 6.0	-0.1	0.5	0.87
H	LOCK	2.0x 4.0	Ctr	Ctr	0.47
F	LOCK	5.0x 5.0	Ctr	-0.2	0.92
E	LOCK	2.0x 4.0	Ctr	Ctr	0.91
D	LOCK	6.0x10.0	-0.8	1.4	0.73

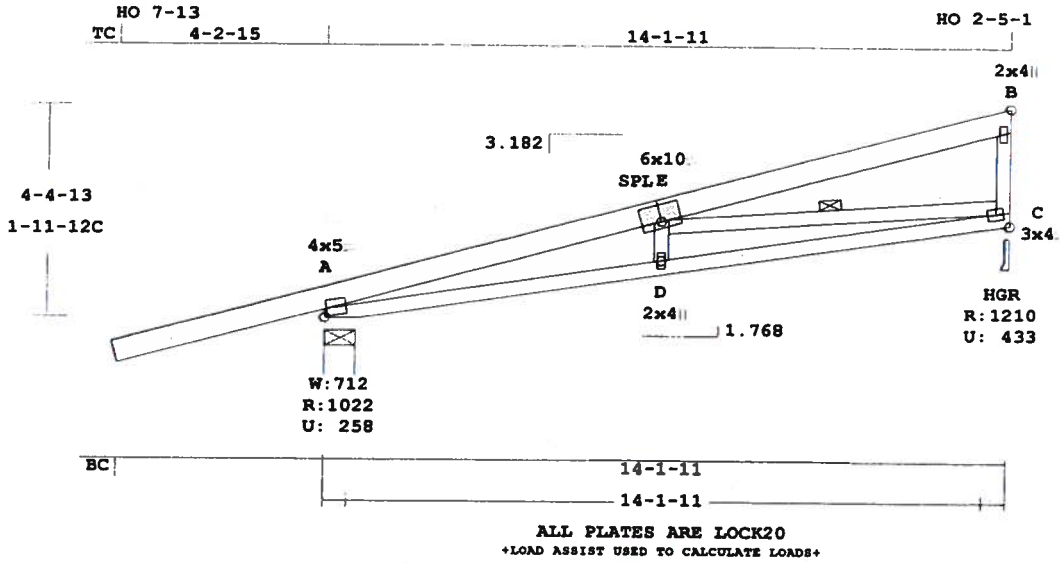
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 5- 9. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2281 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R127*	2	HFSC	140111	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 98.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.88	2x 6	SP-#2
BC	0.63	2x 4	SP-2400
WB	0.61	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 1-11
BC Cont.	0- 0- 0	14- 1-11

WB 1 rows CLB on E -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.1'
BC V	0	20	0.0'	14.1'
TC V	-40	-20	0.0'	
	87	44		14.1'
BC V	0	-20	0.0'	
	0	44		14.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd	Lbs	Lbs	In-Sx	In-Sx
A	1023	258	7-12	1- 0			
			Hz =	268			
C	1210	433	3- 8	1- 0			
			Hz =	145			

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.75	2683 C	0.02	0.73
E -B	0.88	163 C	0.00	0.88
--------Bottom Chords---------				
A -D	0.50	2607 T	0.20	0.30
D -C	0.63	2611 T	0.12	0.51
------------Webs------------				
D -E	0.13	486 T		
E -C	0.61	2581 C		1 Br
C -B	0.14	579 C		

LL Defl -0.23" in D -C L/687
TL Defl -0.53" in D -C L/297
Shear // Grain in E -B 0.44

Hz Disp	LL	DL	TL
Jt C	0.02"	0.02"	0.05"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

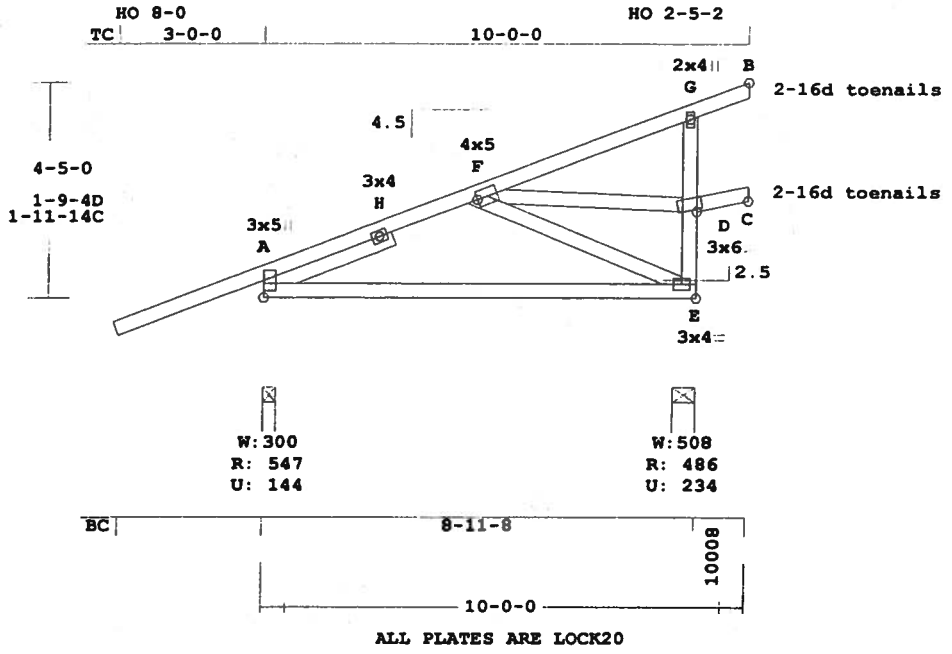
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.1	0.4	0.86
E	LOCK	6.0x10.0	-0.3	1.2	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.45
D	LOCK	2.0x 4.0	Ctr	Ctr	0.44
C	LOCK	3.0x 4.0	Ctr	Ctr	0.99

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 10- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2683 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R126	3	SP	100000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 74.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 4	SP-#2
BC	0.40	2x 4	SP-#2
CW	0.07	2x 4	SP-#3
WB	0.12	2x 4	SP-#3
SL	0.07	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 0- 0
BC	Cont.	0- 0- 0	10- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	547	144	3- 0	1- 0
			Hz =	169
G	486	234	5- 8	1- 0
B	57	50G	3- 8	1- 0
			Hz =	91
C	28	0	3- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd		
----------Top Chords----------					
A -H	0.11	536 C	0.00	0.11	
H -F	0.18	377 C	0.00	0.18	
F -G	0.18	127 C	0.00	0.18	
G -B	0.17	102 T	0.00	0.17	
--------Bottom Chords---------					
A -E	0.40	380 T	0.03	0.37	
E -D	0.10	129 T	0.01	0.09	
D -C	0.00	0 T			
----------Chord-Webs----------					
E -D	0.00	290 T			
D -G	0.07	245 T	0.07	0.00	
------------Webs------------					
F -E	0.12	359 T			
F -D	0.03	134 C			
----------Sliders----------					
A -H	0.07	290 T			

LL Defl -0.06" in A -E L/999
TL Defl -0.13" in A -E L/783
Shear // Grain in F -G 0.19

Hz Disp	LL	DL	TL
Jt C	0.01"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.74
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.76
G	LOCK	2.0x 4.0	Ctr	Ctr	0.16
E	LOCK	3.0x 4.0	Ctr	Ctr	0.77
D	LOCK	3.0x 6.0	Ctr	Ctr	0.43

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

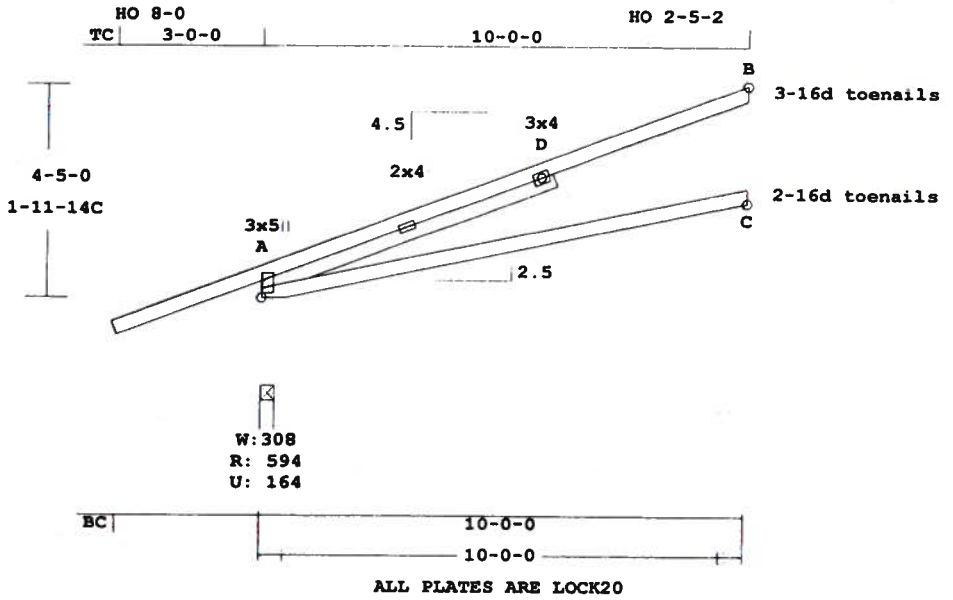
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 536 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R125	19	VJACK	100000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 58.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.72	2x 4	SP-2400
BC	0.58	2x 4	SP-#2
SL	0.28	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 0- 0
BC Cont.	0- 0- 0	10- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	594	165	3- 8	1- 0
			Hz =	169
B	298	192	3- 8	1- 0
C	164	0	3- 8	1- 0
			Hz =	91

Membr CSI P Lbs Axl-CSI-Bnd OH Loading

Top Chords					
A -D	0.72	935	C	0.00	0.72
D -B	0.72	137	C	0.00	0.72
Bottom Chords					
A -C	0.58	88	T	0.00	0.58
Sliders					
A -D	0.28	942	T		

LL Defl -0.22" in A -C L/511
TL Defl -0.55" in A -C L/207
Shear // Grain in A -D 0.39

Hz Disp	LL	DL	TL
Jt B	0.05"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.75
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.67

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

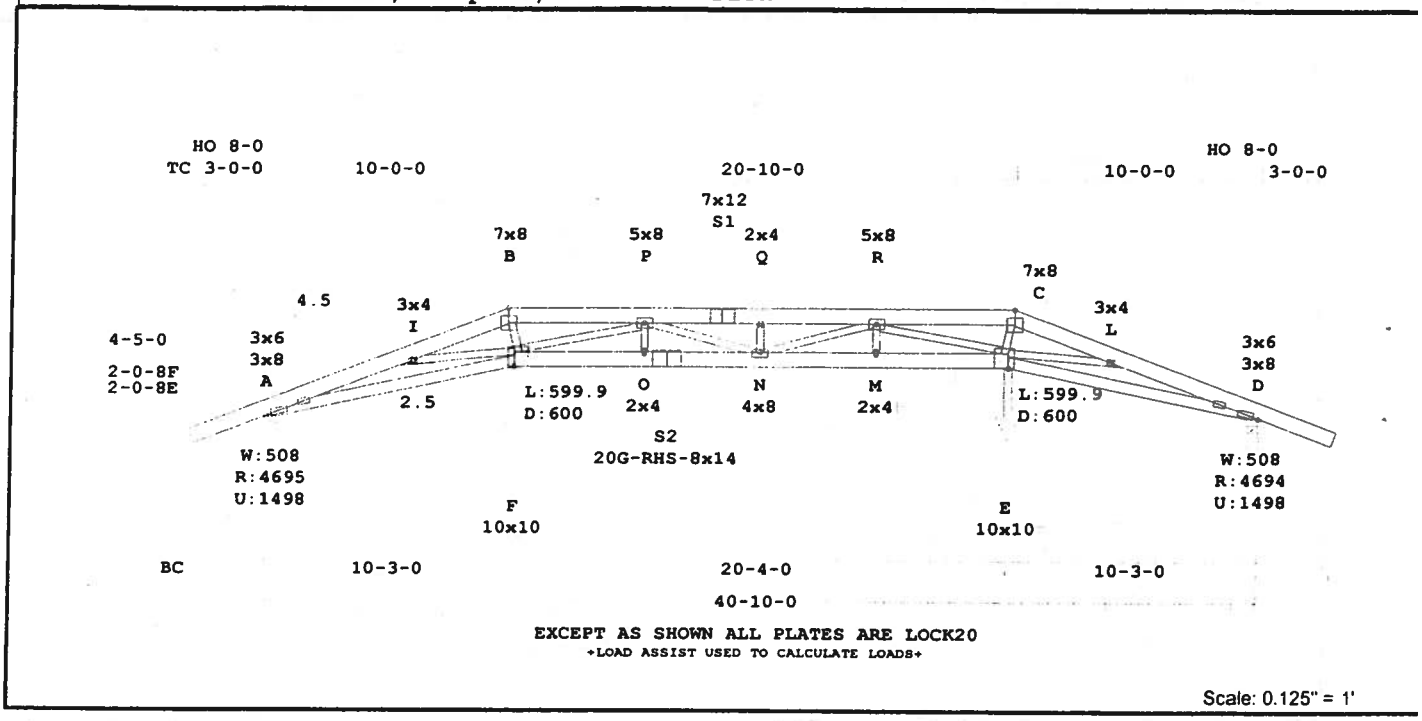
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 935 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R124	2*2P	HPSC3	401000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 387.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.58	2x 8	SP-2400
BC	0.73	2x 6	SP-2400
EX F -S2		2x 8	SP-2400
EX S2-E		2x 8	SP-2400
WB	0.49	2x 4	SP-#3
EX B -F		2x 4	SP-#2
EX E -C		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	40.8'
BC V	0	20	0.0'	40.8'
TC V	80	40	10.0'	30.8'
BC V	0	40	10.2'	30.6'
BC V	600	600	10.2'	CL-LB
BC V	600	600	30.6'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	4696	1498	5- 8	1-15
			Hz =	-68
D	4695	1498	5- 8	1-15
			Hz =	68

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.29	20652 C	0.14	0.15
I -B	0.32	23224 C	0.19	0.13
B -P	0.46	23313 C	0.19	0.27
P -S1	0.58	29269 C	0.30	0.28
S1-Q	0.52	29269 C	0.30	0.22
Q -R	0.58	29269 C	0.30	0.28
R -C	0.46	23315 C	0.19	0.27
C -L	0.32	23225 C	0.19	0.13
L -D	0.29	20652 C	0.14	0.15
--------Bottom Chords---------				
A -F	0.73	19453 T	0.49	0.24
F -O	0.85	26385 T	0.50	0.35
O -S2	0.85	26385 T	0.50	0.35
S2-N	0.82	26384 T	0.50	0.32
N -M	0.85	26385 T	0.50	0.35
M -E	0.85	26385 T	0.50	0.35
E -D	0.73	19454 T	0.49	0.24
------------Webs------------				
I -F	0.44	2752 T		
B -F	0.65	7128 T		
F -P	0.30	3177 C		
O -P	0.07	688 C		
P -N	0.49	3080 T		
N -Q	0.09	1218 C		
N -R	0.49	3079 T		
M -R	0.07	687 C		
R -E	0.30	3176 C		
E -C	0.65	7129 T		
E -L	0.44	2753 T		

LL Defl -1.33" in S2-N L/359
TL Defl -2.67" in S2-N L/179
Shear // Grain in A -I 0.23

Hz Disp	LL	DL	TL
Jt D	0.57"	0.57"	1.14"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.95
A	LOCK	3.0x 6.0	Ctr	Ctr	1.00
I	LOCK	3.0x 4.0	Ctr	Ctr	0.75
B	LOCK	7.0x 8.0	Ctr	Ctr	0.85
P	LOCK	5.0x 8.0	Ctr	Ctr	0.75
S1	LOCK	7.0x12.0	Ctr	Ctr	0.88
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.48
R	LOCK	5.0x 8.0	Ctr	Ctr	0.75
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
L	LOCK	3.0x 4.0	Ctr	Ctr	0.75
D	LOCK	3.0x 8.0	Ctr	Ctr	0.95
D	LOCK	3.0x 6.0	Ctr	Ctr	1.00
F	LOCK	10.0x10.0	Ctr	-2.2	0.95
O	LOCK	2.0x 4.0	Ctr	Ctr	0.48
S2	RHS	8.0x14.0	Ctr	0.4	0.92
N	LOCK	4.0x 8.0	Ctr	-0.5	0.86
M	LOCK	2.0x 4.0	Ctr	Ctr	0.48
E	LOCK	10.0x10.0	Ctr	-2.2	0.95

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 10- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

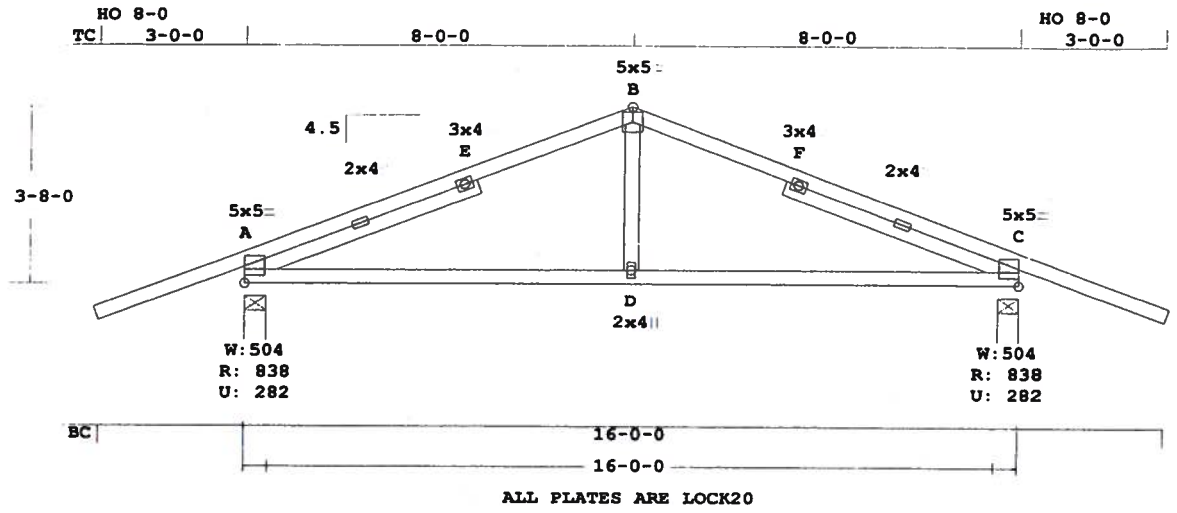
	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	4	4	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 29269 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R123*	3	TR	160000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 100.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.36	2x 4	SP-#2
BC	0.45	2x 4	SP-#2
WB	0.08	2x 4	SP-#3
SL	0.07	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16- 0- 0
BC Cont.	0- 0- 0	16- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	839	283	5- 4	1- 0
C	839	283	5- 4	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -E	0.33	724 C	0.00	0.33
E -B	0.36	866 C	0.07	0.29
B -F	0.36	866 C	0.07	0.29
F -C	0.33	724 C	0.00	0.33
Bottom Chords				
A -D	0.45	846 T	0.14	0.31
D -C	0.45	846 T	0.14	0.31
Webs				
D -B	0.08	325 T		
Sliders				
A -E	0.07	220 C		
F -C	0.07	220 C		

LL Defl -0.04" in D -C L/999
TL Defl -0.10" in D -C L/999
Shear // Grain in E -B 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.96
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.49
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	5.0x 5.0-2.5	0.5		0.96
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

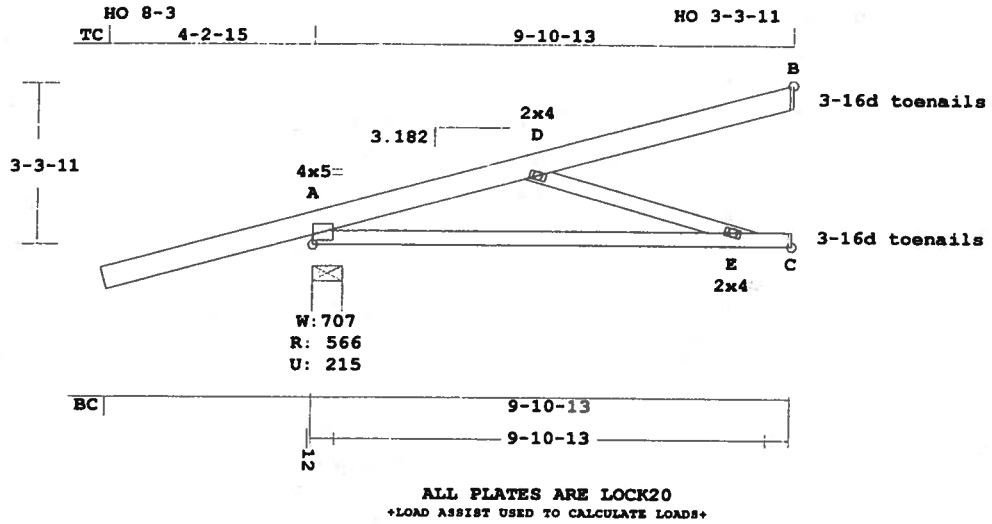
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as

Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 866 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R122	4	KING	91013	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 6	SP-#2
BC	0.69	2x 4	SP-#2
WB	0.25	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9-10-13
BC Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
TC V	-40	-20	0.0'	
	45	22		9.9'
BC V	0	-20	0.0'	
	0	22		9.9'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	566	215	7- 7	1- 0
			Hz =	125
B	297	193	3- 8	1- 0
C	320	13	3- 8	1- 0
			Hz =	68

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -D	0.20	596	C	0.00	0.20
D -B	0.29	106	C	0.00	0.29
Bottom Chords					
A -E	0.69	587	T	0.04	0.65
E -C	0.28	67	T	0.00	0.28
Webs					
D -E	0.25	610	C		

LL Defl -0.25" in A -E L/433
TL Defl -0.52" in A -E L/207
Shear // Grain in E -C 0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.6	0.1	0.71
D	LOCK	2.0x 4.0	Ctr	Ctr	0.37
E	LOCK	2.0x 4.0	Ctr	Ctr	0.48

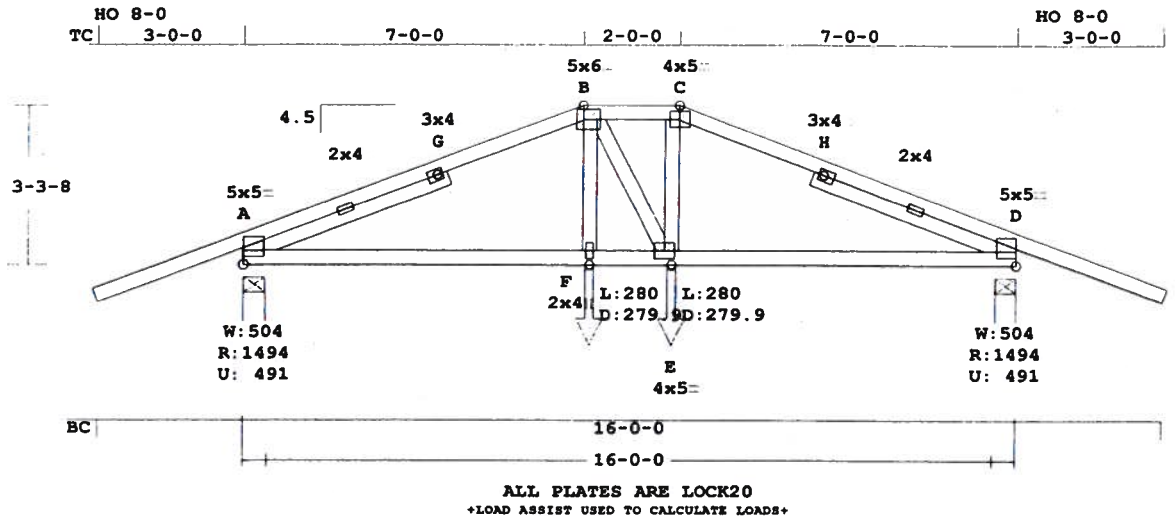
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 610 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R121	2	HIPP	160000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 107.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 4	SP-#2
BC	0.68	2x 4	SP-#2
WB	0.20	2x 4	SP-#3
SL	0.34	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16- 0- 0
BC Cont.	0- 0- 0	16- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	16.0'
BC V	0	20	0.0'	16.0'
TC V	50	25	7.0'	9.0'
BC V	0	25	7.1'	8.9'
BC V	280	280	7.1'	CL-LB
BC V	280	280	8.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1495	491	5- 4	1-12
D	1495	491	5- 4	1-12

Membr CSI P Lbs Axl-CSI-Bnd Girder Step Down Hip

Top Chords				
A -G	0.28	1341 C	0.10	0.18
G -B	0.35	2501 C	0.20	0.15
B -C	0.27	2344 C	0.20	0.07
C -H	0.35	2504 C	0.20	0.15
H -D	0.28	1322 C	0.10	0.18
Bottom Chords				
A -F	0.68	2343 T	0.43	0.25
F -E	0.45	2322 T	0.42	0.03
E -D	0.65	2344 T	0.43	0.22
Webs				
F -B	0.20	633 T		
B -E	0.01	59 C		
E -C	0.19	622 T		
Sliders				
A -G	0.34	1239 C		
H -D	0.34	1260 C		

LL Defl -0.06" in F -E L/999
TL Defl -0.14" in F -E L/999
Shear // Grain in A -G 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.96
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 6.0	-0.5	Ctr	0.64
C	LOCK	4.0x 5.0	Ctr	Ctr	0.78
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	5.0x 5.0	-2.5	0.5	0.96
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.68
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60

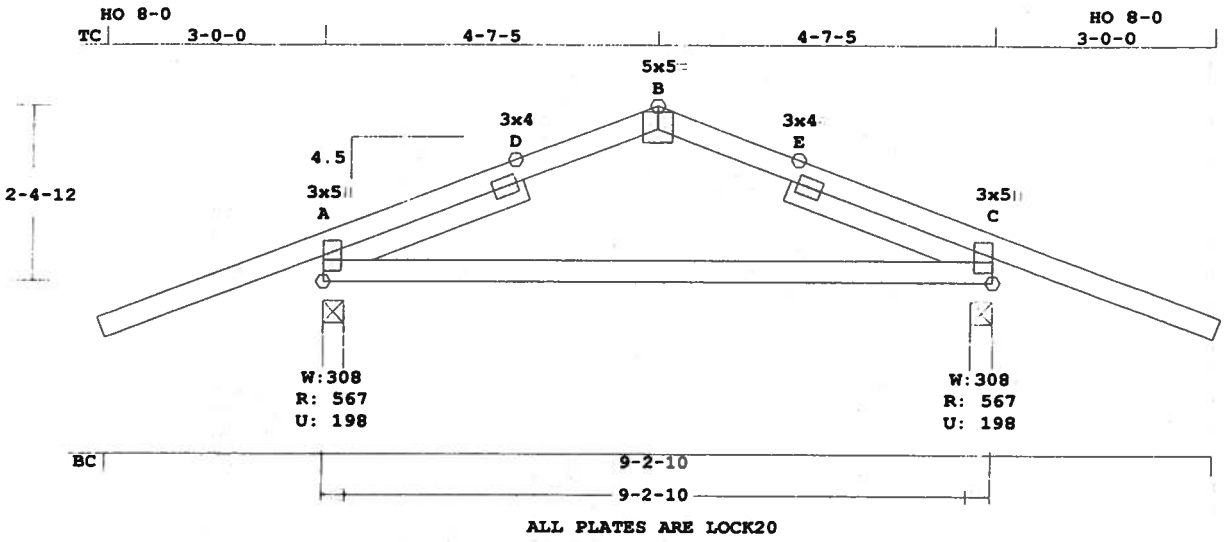
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2504 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R120	5	TR	90210	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 59.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
SL	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 2-10
BC Cont.	0- 0- 0	9- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	567	198	3- 8	1- 0
C	567	198	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.16	564 C	0.04	0.12
D -B	0.18	481 T	0.06	0.12
B -E	0.18	481 T	0.06	0.12
E -C	0.16	564 C	0.04	0.12
--------Bottom Chords---------				
A -C	0.32	382 T	0.03	0.29
----------Sliders----------				
A -D	0.08	315 T		
E -C	0.08	315 T		

LL Defl -0.07" in A -C L/999
TL Defl -0.14" in A -C L/747
Shear // Grain in D -B 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.73
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr-1.6		0.38
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0-1.5	0.3		0.73

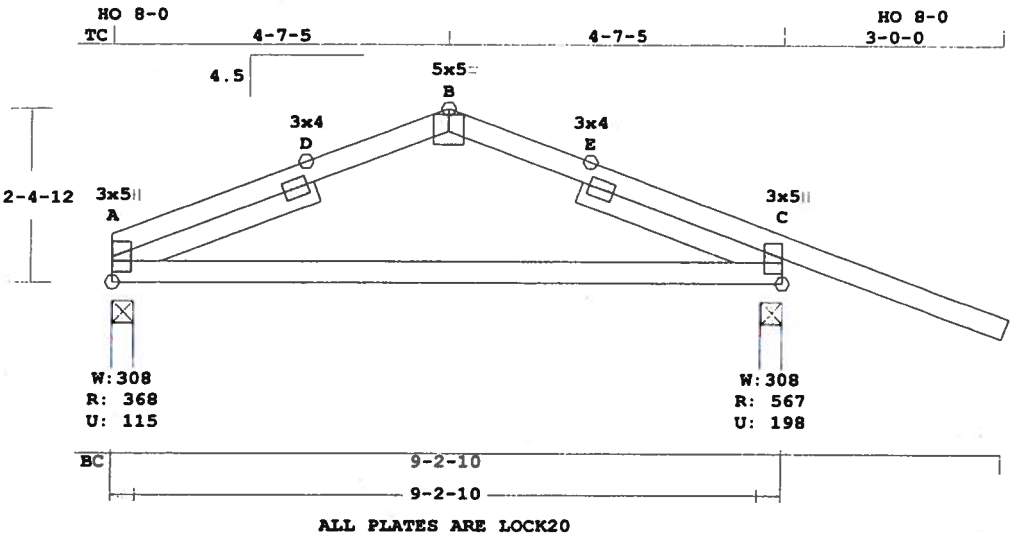
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
2-11- 5. Support end or
provide level return.
Excessive Right OH condition.
Max allowable length is
2-11- 5. Support end or
provide level return.
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 564 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R119*	1	TR	90210	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 53.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
SL	0.08	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 2-10
BC	Cont.	0- 0- 0	9- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	369	115	3- 8	1- 0
C	567	198	3- 8	1- 0

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -D	0.16	564	C	0.04	0.12
D -B	0.18	481	T	0.06	0.12
B -E	0.18	481	T	0.06	0.12
E -C	0.16	564	C	0.04	0.12
Bottom Chords					
A -C	0.32	382	T	0.03	0.29
Sliders					
A -D	0.08	315	T		
E -C	0.08	315	T		

LL Defl -0.07" in A -C L/999
TL Defl -0.14" in A -C L/747
Shear // Grain in D -B 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.73
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr-1.6		0.38
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0	-1.5	0.3	0.73

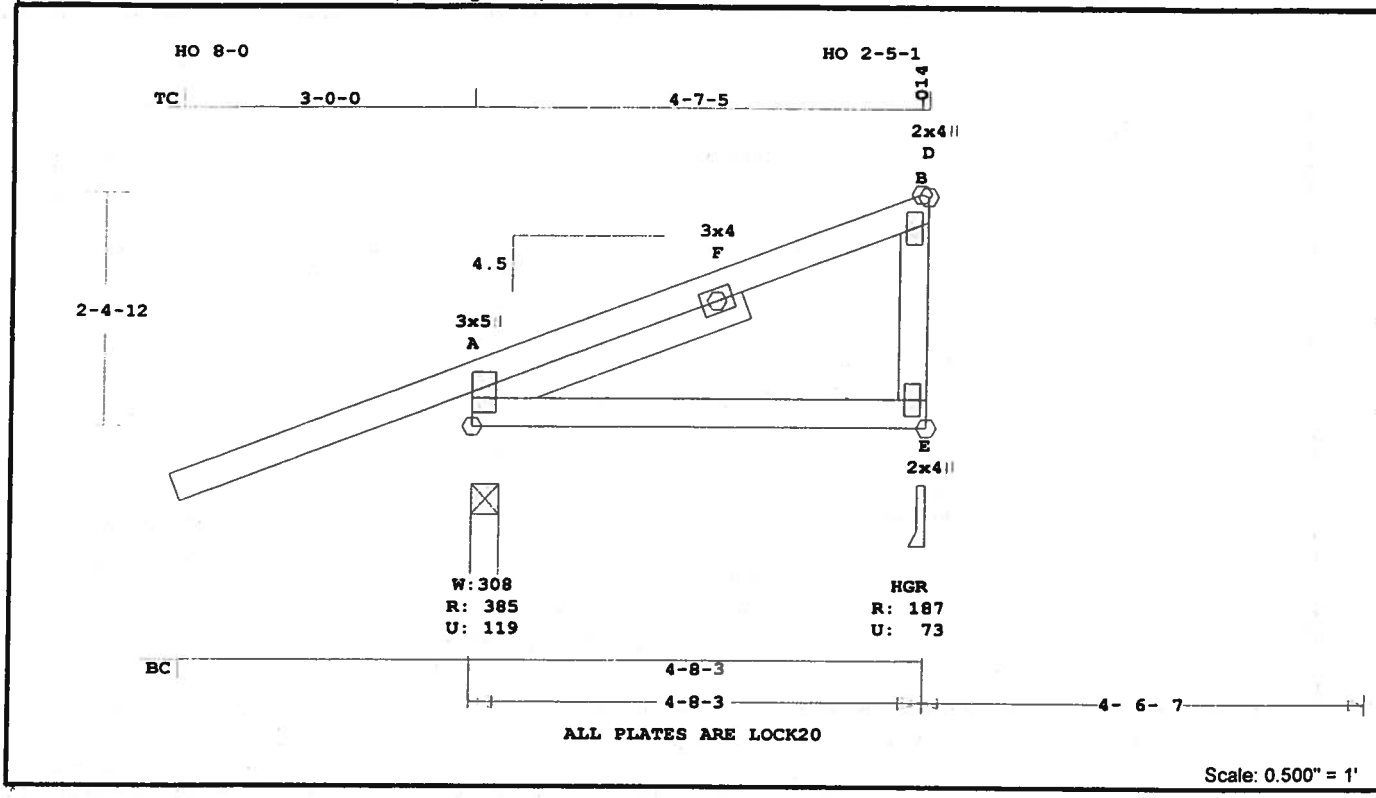
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Right OH condition.
Max allowable length is
2-11- 5. Support end or
provide level return.
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 564 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R118	1	TR	40803	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
WB	0.06	2x 4	SP-#3
SL	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 8- 3
BC Cont.	0- 0- 0	4- 8- 3

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	386	120	3- 8	1- 0
			Hz =	78
E	187	74	3- 8	1- 0
			Hz =	42

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -F	0.34	170 C	0.01 0.33
F -D	0.33	90 C	0.00 0.33
--------Bottom Chords---------			
A -E	0.25	41 T	0.00 0.25
------------Webs------------			
E -D	0.06	245 T	
----------Sliders----------			
A -F	0.04	193 C	

LL Defl -0.01" in A -E L/999
TL Defl -0.03" in A -E L/999
Shear // Grain in A -F 0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

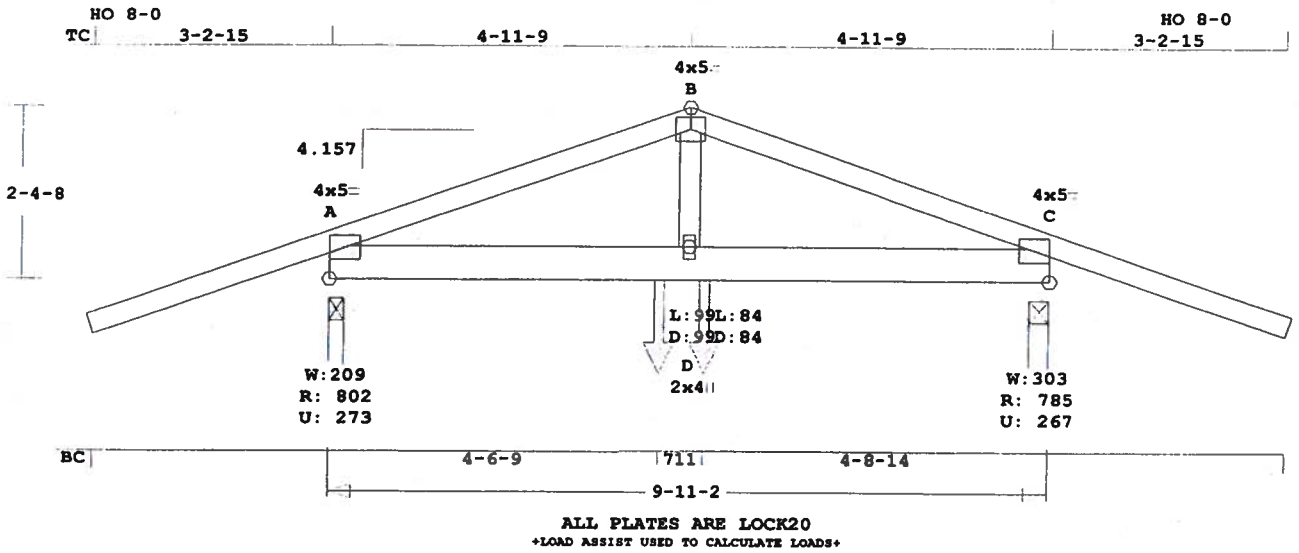
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 193 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R117*	1	TR	91102	4.157	3- 2-15	3- 2-15	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.39	2x 4	SP-#2
BC	0.23	2x 6	SP-#2
WB	0.12	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9-11- 2
BC	Cont.	0- 0- 0	9-11- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
BC V	99	99	4.5'	CL-LB
BC V	84	84	5.2'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	802	273	2- 9	1- 0
		Hz =	-27	
C	785	268	3- 3	1- 0
		Hz =	28	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.38	945 C	0.13	0.25
B -C	0.39	946 C	0.13	0.26
--------Bottom Chords---------				
A -D	0.23	887 T	0.11	0.12
D -C	0.21	887 T	0.11	0.10
------------Webs------------				
D -B	0.12	458 T		

LL Defl -0.01" in A -D L/999
TL Defl -0.02" in A -D L/999
Shear // Grain in A -B 0.24

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

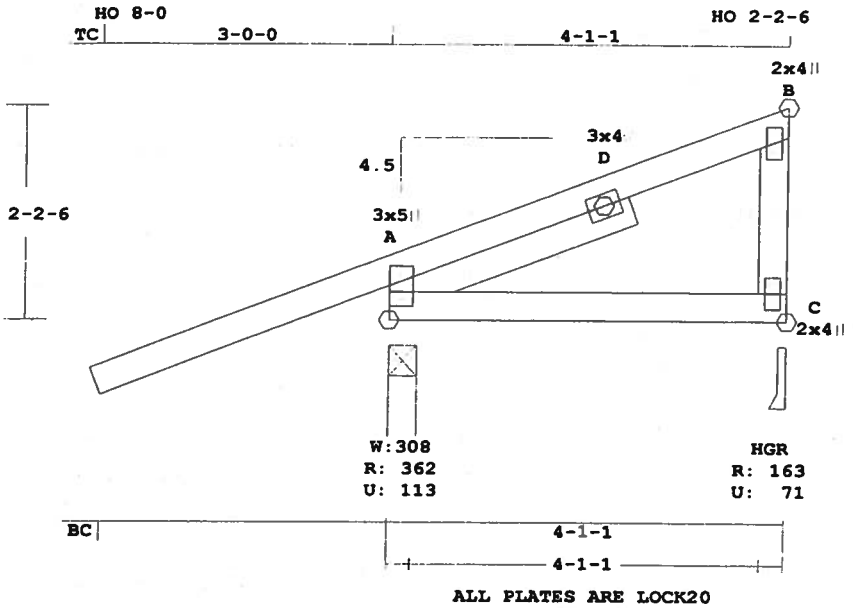
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.8	0.3	0.79
B	LOCK	4.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x 5.0	-0.8	0.3	0.79
D	LOCK	2.0x 4.0	Ctr	Ctr	0.49

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
2-11-10. Support end or
provide level return.
Excessive Right OH condition.
Max allowable length is
2-11-10. Support end or
provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 946 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R116	1	MONO	40101	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.25	2x 4	SP-#2
BC	0.19	2x 4	SP-#2
WB	0.05	2x 4	SP-#3
SL	0.03	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 1- 1
BC Cont.	0- 0- 0	4- 1- 1

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	362	113	3- 8	1- 0
			Hz =	69
C	164	72	3- 8	1- 0
			Hz =	37

Membr	CSI	P Lbs	Axl-CSI-Bnd

Top Chords

A -D	0.25	125 C	0.00	0.25
D -B	0.25	82 C	0.00	0.25

Bottom Chords

A -C	0.19	37 T	0.00	0.19

Webs

C -B	0.05	226 T		

Sliders

A -D	0.03	154 C		

LL Defl -0.01" in A -C L/999
TL Defl -0.02" in A -C L/999
Shear // Grain in A -D 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

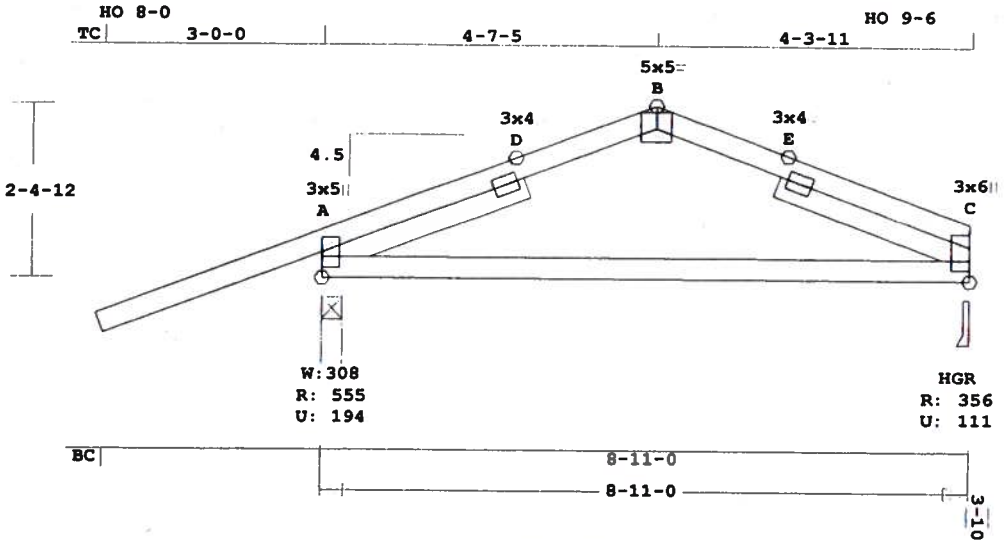
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 154 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R115*	1	TR	81100	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 52.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8-11- 0
BC Cont.	0- 0- 0	8-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	555	194	3- 8	1- 0
C	357	112	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.16	543 C	0.00	0.16
D -B	0.18	455 T	0.05	0.13
B -E	0.15	462 T	0.06	0.09
E -C	0.11	454 C	0.02	0.09
--------Bottom Chords---------				
A -C	0.35	354 T	0.03	0.32
----------Sliders----------				
A -D	0.08	315 T		
E -C	0.07	296 T		

LL Defl -0.06" in A -C L/999
TL Defl -0.13" in A -C L/767
Shear // Grain in D -B 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

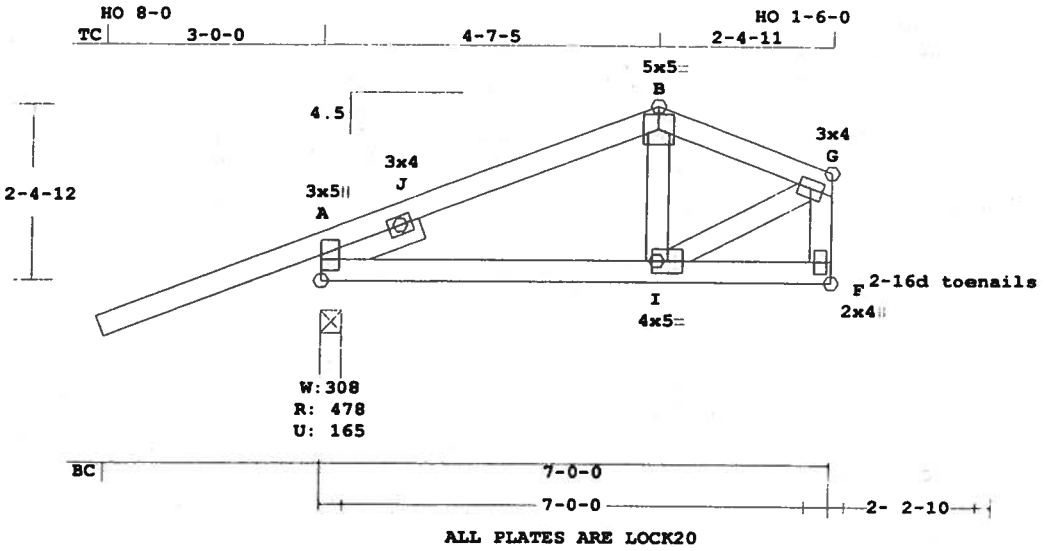
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.72
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr-1.6	0.3	0.37
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 6.0	-1.5	0.3	0.81

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
2-11- 3. Support end or
provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 543 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R114	2	TR	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.08	2x 4	SP-#3
SL	0.07	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	479	166	3- 8	1- 0
			Hz =	37
F	280	92	3- 8	1- 0
			Hz =	18

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -J	0.02	184 C	0.00 0.02
J -B	0.21	278 T	0.03 0.18
B -G	0.12	300 T	0.03 0.09
--------Bottom Chords---------			
A -I	0.17	244 C	0.00 0.17
I -F	0.08	18 T	0.00 0.08
-------------Webs-------------			
I -B	0.03	124 T	
I -G	0.08	279 C	
F -G	0.08	330 T	
----------Sliders------------			
A -J	0.07	287 T	

LL Defl -0.01" in A -I L/999
TL Defl -0.01" in A -I L/999
Shear // Grain in J -B 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.38
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

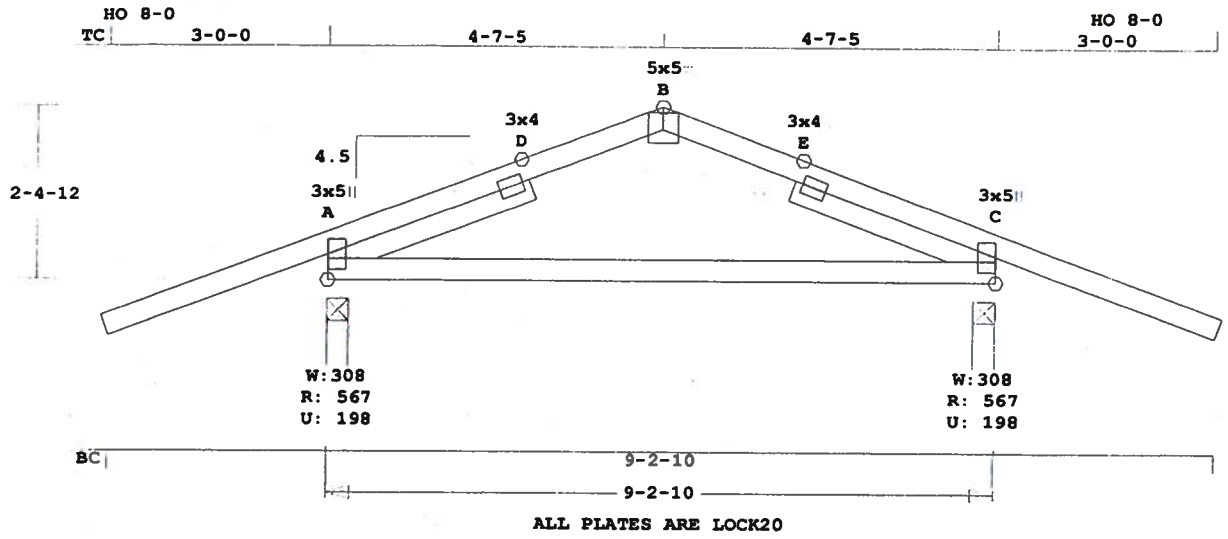
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 2-10- 3. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 279 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R113*	12	TR	90210	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 59.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
SL	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 2-10
BC Cont.	0- 0- 0	9- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	567	198	3- 8	1- 0
C	567	198	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.16	564 C	0.04	0.12
D -B	0.18	481 T	0.06	0.12
B -E	0.18	481 T	0.06	0.12
E -C	0.16	564 C	0.04	0.12
--------Bottom Chords---------				
A -C	0.32	382 T	0.03	0.29
----------Sliders----------				
A -D	0.08	315 T		
E -C	0.08	315 T		

LL Defl -0.07" in A -C L/999
TL Defl -0.14" in A -C L/747
Shear // Grain in D -B 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.73
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	-1.6	0.38
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0	-1.5	0.3	0.73

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
2-11- 5. Support end or
provide level return.
Excessive Right OH condition.
Max allowable length is
2-11- 5. Support end or
provide level return.
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 564 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R112	1*4P	SP	450404	4.5	0	0	Cont.

Office and Care Center, Hospice, M.M. Parrish

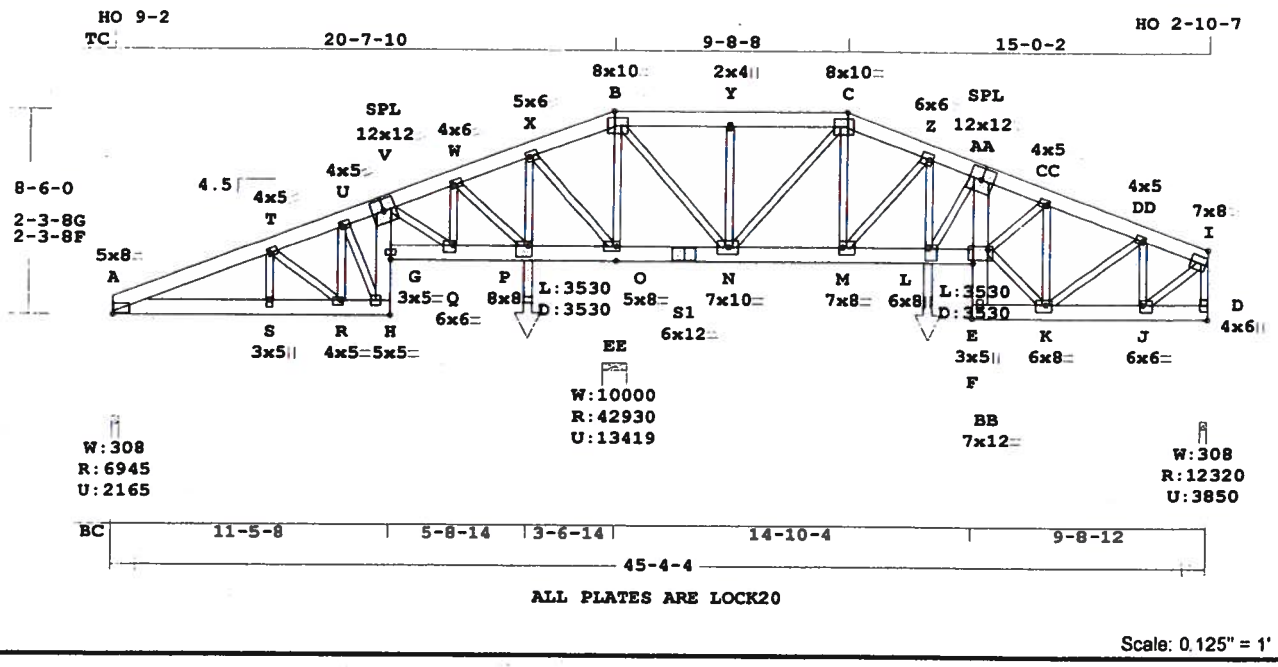
TC	2	12	24	24
BC	2	6	11	14
WB	1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 22708 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R112	1*4P	SP	450404	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 562.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 4-Ply Truss *
****

	CSI	Size	Lumber
TC	0.38	2x 8	SP-2400
BC	0.46	2x 8	SP-2400
CW	0.41	2x 8	SP-2400
WB	0.94	2x 4	SP-#3
EX P -X		2x 4	SP-#2
EX B -N		2x 4	SP-#2
EX M -C		2x 4	SP-#2
EX L -Z		2x 4	SP-#2
EX BB-K		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	45- 4- 4
BC Cont.		0- 0- 0	45- 4- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To	
TC V 40	20	0.0'	45.4'	
BC V 490	510	0.0'	45.4'	
BC V 3530	3530	17.2'	CL-LB	
BC V 3530	3530	33.8'	CL-LB	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	6945	2166	3- 8	1- 7
			Hz =	-117
O	42930	13420	12- 0	8-10
D	12320	3851	3- 8	2- 9
			Hz =	127

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -T	0.08	9597 C	0.00 0.08
T -U	0.02	4833 C	0.00 0.02
U -V	0.11	2449 C	0.00 0.11
V -W	0.13	2840 T	0.02 0.11
W -X	0.29	9356 T	0.08 0.21
X -B	0.38	19760 T	0.17 0.21
B -Y	0.21	4372 T	0.03 0.18
Y -C	0.15	4372 T	0.03 0.12
C -Z	0.15	7289 C	0.00 0.15
Z -AA	0.16	16732 C	0.01 0.15
AA-CC	0.14	19190 C	0.02 0.12
CC-DD	0.05	12923 C	0.01 0.04
DD-I	0.02	9261 C	0.02 0.00
--------Bottom Chords---------			
A -S	0.24	9071 T	0.07 0.17
S -R	0.17	9071 T	0.07 0.10
R -H	0.06	4498 T	0.03 0.03
G -Q	0.19	3101 T	0.02 0.17
Q -P	0.09	2530 C	0.00 0.09
P -O	0.44	8351 C	0.00 0.44
O -S1	0.46	18709 C	0.02 0.44
S1-N	0.06	18709 C	0.05 0.01
N -M	0.20	6972 T	0.06 0.14
M -L	0.23	15586 T	0.13 0.10
L -BB	0.31	18098 T	0.15 0.16
E -K	0.07	1415 T	0.01 0.06
K -J	0.15	8662 T	0.07 0.08
J -D	0.04	127 T	0.00 0.04
----------Chord-Webs----------			
H -G	0.41	6431 T	0.06 0.35
G -V	0.06	6749 T	0.06 0.00
E -BB	0.24	974 T	0.00 0.24
BB-AA	0.04	4969 T	0.04 0.00
------------Webs------------			
S -T	0.40	5025 T	
T -R	0.24	6009 C	
R -U	0.43	5411 T	
U -H	0.23	5930 C	
V -Q	0.27	6875 C	
Q -W	0.69	8614 T	
W -P	0.33	8190 C	
P -X	0.63	13668 T	
X -O	0.69	16757 C	
O -B	0.94	22708 C	
B -N	1.00	21743 T	
N -Y	0.01	207 T	
N -C	0.76	17234 C	
M -C	0.67	14510 T	
M -Z	0.54	13025 C	
L -Z	0.66	14472 T	
L -AA	0.19	4793 C	
BB-CC	0.62	7836 T	
BB-K	0.64	13957 T	
K -CC	0.32	7956 C	
K -DD	0.35	4377 T	
J -DD	0.17	4355 C	
J -I	0.93	11598 T	
D -I	0.43	11073 C	

LL Defl -0.17" in L -BB L/999
TL Defl -0.33" in L -BB L/857
Shear // Grain in P -O 0.55

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

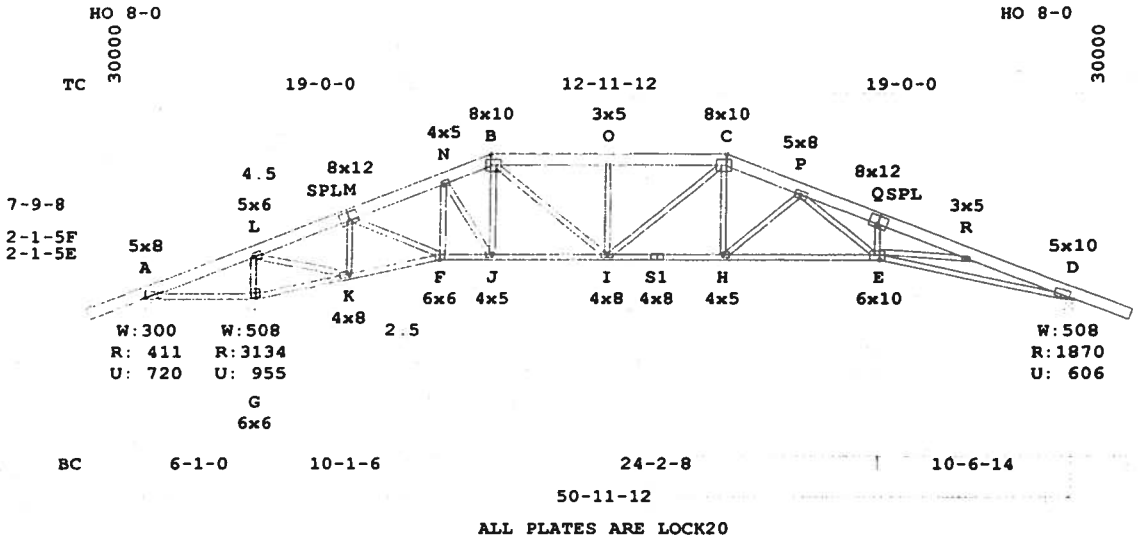
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	4.0	1.4	0.86
T	LOCK	4.0x 5.0	-0.5	0.2	0.62
U	LOCK	4.0x 5.0	-0.5	0.2	0.67
V	LOCK	12.0x12.0	Ctr	Ctr	0.91
W	LOCK	4.0x 6.0	-0.5	0.2	0.89
X	LOCK	5.0x 6.0	-0.5	-0.2	0.95
B	LOCK	8.0x10.0	0.5	Ctr	0.97
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.51
C	LOCK	8.0x10.0	Ctr	Ctr	0.79
Z	LOCK	6.0x 6.0	0.5	-0.2	0.87
AA	LOCK	12.0x12.0	Ctr	Ctr	0.91
CC	LOCK	4.0x 5.0	0.5	0.2	0.98
DD	LOCK	4.0x 5.0	0.5	0.2	0.56
I	LOCK	7.0x 8.0	-1.5	0.6	0.93
S	LOCK	3.0x 5.0	Ctr	-1.1	0.90
R	LOCK	4.0x 5.0	Ctr	-0.5	0.85
H	LOCK	5.0x 5.0	Ctr	Ctr	0.96
G	LOCK	3.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x 6.0	Ctr	-0.6	0.81
P	LOCK	8.0x 8.0	-0.5	-1.6	0.96
O	LOCK	5.0x 8.0	-0.5	Ctr	0.77
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
N	LOCK	7.0x10.0	-0.5	-0.1	0.98
M	LOCK	7.0x 8.0	0.5	-0.1	0.72
L	LOCK	6.0x 8.0	-0.5	-2.1	0.93
BB	LOCK	7.0x12.0	Ctr	Ctr	0.83
E	LOCK	3.0x 5.0	Ctr	Ctr	0.96
K	LOCK	6.0x 8.0	-1.0	-0.6	0.89
J	LOCK	6.0x 6.0	Ctr	-0.6	0.93
D	LOCK	4.0x 6.0	-0.2	Ctr	0.91

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 51- 0- 0
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R110*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 459.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.32	2x 8	SP-2400
BC	0.89	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.96	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50-11-12
BC Cont.	0- 0- 0	50-11-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	411	720G	3- 0	1- 0
		Hz =	-132	
G	3135	955	5- 8	3- 7
D	1870	607	5- 8	1- 9
		Hz =	133	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.27	2367 T	0.08		0.19
L -M	0.19	896 C	0.00		0.19
M -N	0.05	2410 C	0.00		0.05
N -B	0.03	2478 C	0.03		0.00
B -O	0.05	3137 C	0.01		0.04
O -C	0.07	3137 C	0.01		0.06
C -P	0.06	3476 C	0.04		0.02
P -Q	0.11	6730 C	0.05		0.06
Q -R	0.32	6690 C	0.05		0.27
R -D	0.29	5771 C	0.03		0.26
--------Bottom Chords---------					
A -G	0.71	2128 C	0.04		0.67
G -K	0.88	2193 C	0.05		0.83
K -F	0.36	806 T	0.13		0.23
F -J	0.68	2265 T	0.38		0.30
J -I	0.49	2320 T	0.38		0.11
I -S1	0.65	3253 T	0.54		0.11
S1-H	0.73	3253 T	0.54		0.19
H -E	0.89	4217 T	0.70		0.19
E -D	0.52	5352 T	0.38		0.14
----------Webs----------					
G -L	0.44	2573 C			
L -K	0.96	3013 T			
K -M	0.27	1340 C			
M -F	0.51	1592 T			
F -N	0.09	338 C			
N -J	0.03	127 T			
J -B	0.02	88 T			
B -I	0.40	1038 T			
I -O	0.16	423 C			
I -C	0.13	148 C			
H -C	0.31	970 T			
H -P	0.55	1244 C			
P -E	0.87	2722 T			
E -Q	0.08	427 C			
E -R	0.34	1070 T			

LL Defl -0.07" in A -G L/918
TL Defl -0.15" in A -G L/417
Shear // Grain in R -D 0.32

Hz Disp	LL	DL	TL
Jt D	0.21"	0.21"	0.42"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

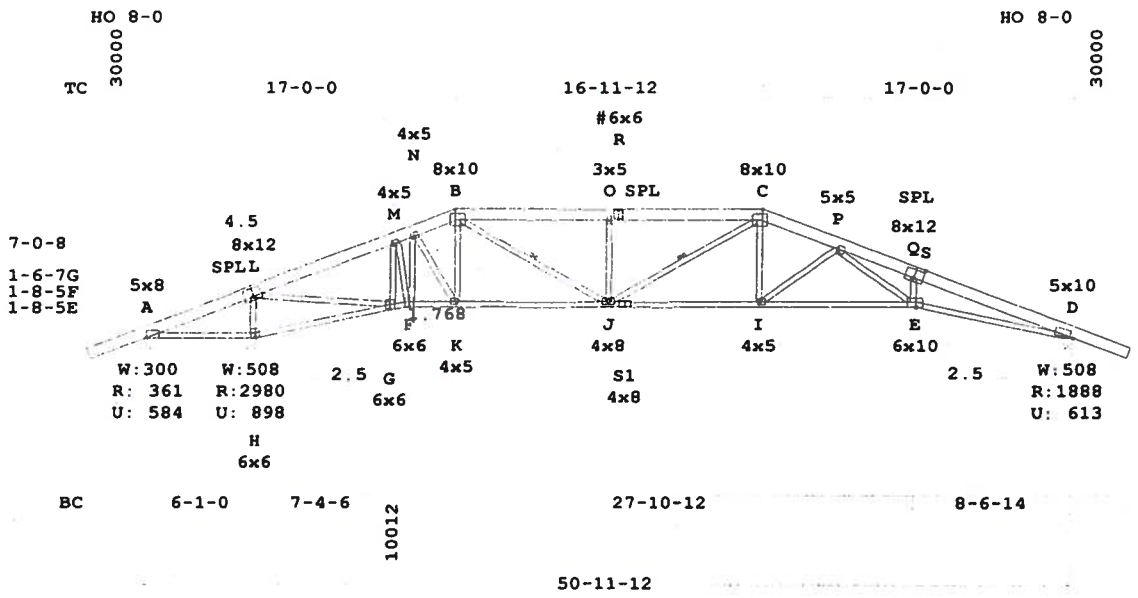
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
L	LOCK	5.0x 6.0	0.5	0.2	0.97
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	3.0x 5.0	Ctr	Ctr	0.36
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
P	LOCK	5.0x 8.0	0.4	-0.2	0.98
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	3.0x 5.0	Ctr	Ctr	0.54
D	LOCK	5.0x10.0	Ctr	1.1	1.00
G	LOCK	6.0x 6.0	-0.3	3.3	0.75
K	LOCK	4.0x 8.0	-0.5	-0.1	0.97
F	LOCK	6.0x 6.0	Ctr	-0.5	0.75
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S1	LOCK	4.0x 8.0	Ctr	0.2	0.75
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x10.0	Ctr	-0.5	0.87

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6730 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R109*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 451.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.33	2x 8	SP-2400
BC	0.95	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.63	2x 4	SP-#3
EX L -G		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50-11-12
BC Cont.	0- 0- 0	50-11-12

WB 1 rows CLB on B -J
WB 1 rows CLB on J -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	362	585G	3- 0	1- 0
		Hz =	-118	
H	2980	899	5- 8	3- 5
D	1889	613	5- 8	1- 9
		Hz =	119	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.28	1989 T	0.07	0.21	
L -M	0.21	1891 C	0.00	0.21	
M -N	0.03	2131 C	0.00	0.03	
N -B	0.03	2365 C	0.00	0.03	
B -O	0.09	3487 C	0.04	0.05	
O -R	0.09	3487 C	0.04	0.05	
R -C	0.11	3487 C	0.01	0.10	
C -P	0.06	3698 C	0.04	0.02	
P -Q	0.30	6262 C	0.04	0.26	
Q -D	0.33	6432 C	0.05	0.28	
Bottom Chords					
A -H	0.66	1774 C	0.03	0.63	
H -G	0.85	1825 C	0.04	0.81	
G -F	0.44	1801 T	0.30	0.14	
F -K	0.50	2008 T	0.33	0.17	
K -J	0.59	2218 T	0.23	0.36	
J -S1	0.78	3454 T	0.57	0.21	
S1-I	0.79	3454 T	0.57	0.22	
I -E	0.95	4389 T	0.73	0.22	
E -D	0.62	6053 T	0.43	0.19	
Webs					
H -L	0.43	2497 C			
L -G	0.91	3532 T			
G -M	0.25	1074 C			
M -F	0.26	835 T			
F -N	0.12	496 C			
N -K	0.12	390 T			
K -B	0.06	192 C			
B -J	0.46	1451 T			1 Br
J -O	0.19	556 C			
J -C	0.04	134 C			1 Br
I -C	0.28	903 T			
I -P	0.50	1167 C			
P -E	0.63	1967 T			
E -Q	0.07	271 T			

LL Defl -0.07" in A -H L/938
TL Defl -0.15" in A -H L/442
Shear // Grain in Q -D 0.27

Hz Disp	LL	DL	TL
Jt D	0.21"	0.21"	0.42"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

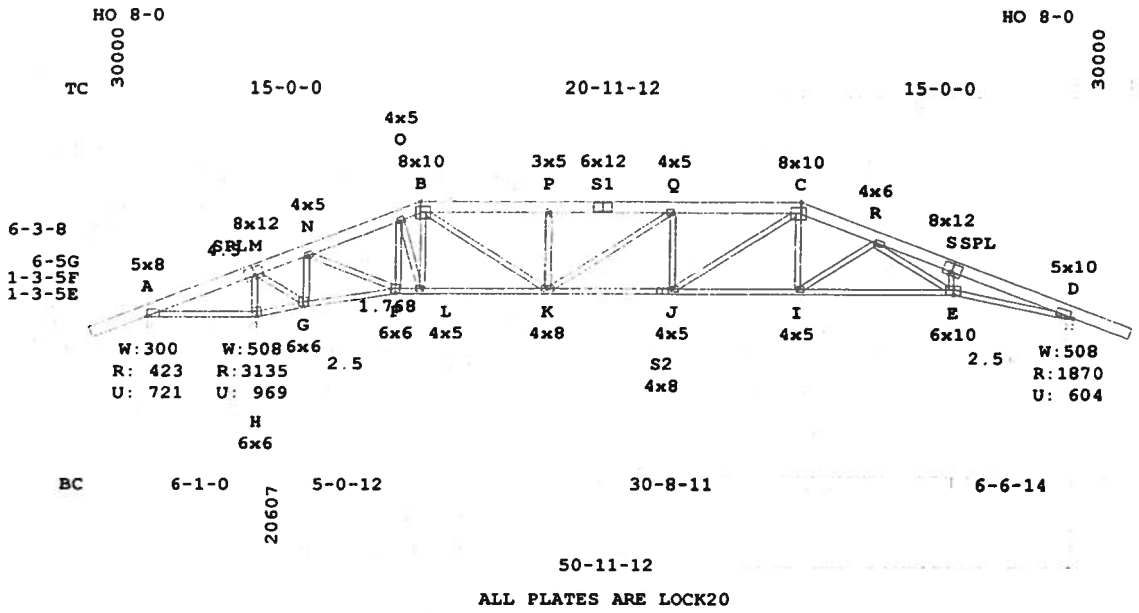
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0 Ctr	0.6		0.78
L	LOCK	8.0x12.0-0.6	1.5		0.97
M	LOCK	4.0x 5.0-0.5	0.2		0.62
N	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	8.0x10.0 Ctr	Ctr		0.84
O	LOCK	3.0x 5.0 Ctr	Ctr		0.36
R	LOCK	6.0x 6.0 Ctr	0.3		0.91
C	LOCK	8.0x10.0 Ctr	Ctr		0.84
P	LOCK	5.0x 5.0 Ctr	Ctr		0.78
Q	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	5.0x10.0 Ctr	1.1		1.00
H	LOCK	6.0x 6.0-0.3	3.3		0.75
G	LOCK	6.0x 6.0 Ctr	Ctr		0.89
F	LOCK	6.0x 6.0 Ctr-0.5			0.75
K	LOCK	4.0x 5.0 Ctr	Ctr		0.60
J	LOCK	4.0x 8.0 Ctr	Ctr		0.57
S1	LOCK	4.0x 8.0 Ctr	0.2		0.75
I	LOCK	4.0x 5.0 Ctr	Ctr		0.60
E	LOCK	6.0x10.0-0.5	-0.5		0.77

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6432 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R108*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 456.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 8	SP-2400
BC	0.91	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.69	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50-11-12
BC Cont.	0- 0- 0	50-11-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	424	721G	3- 0	1- 0
		Hz =	-104	
H	3135	969	5- 8	3- 7
D	1870	605	5- 8	1- 9
		Hz =	105	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -M	0.26	2366 T	0.08	0.18
M -N	0.19	441 T	0.01	0.18
N -O	0.04	1722 C	0.02	0.02
O -B	0.03	1911 C	0.02	0.01
B -P	0.06	3195 C	0.01	0.05
P -S1	0.06	3195 C	0.01	0.05
S1-Q	0.06	3195 C	0.01	0.05
Q -C	0.08	3772 C	0.01	0.07
C -R	0.06	3713 C	0.04	0.02
R -S	0.35	5896 C	0.04	0.31
S -D	0.35	6100 C	0.04	0.31
--------Bottom Chords---------				
A -H	0.61	2131 C	0.04	0.57
H -G	0.89	2216 C	0.03	0.86
G -F	0.23	404 C	0.00	0.23
F -L	0.67	1631 T	0.27	0.40
L -K	0.44	1732 T	0.18	0.26
K -S2	0.77	3772 T	0.63	0.14
S2-J	0.74	3772 T	0.63	0.11
J -I	0.77	3462 T	0.58	0.19
I -E	0.91	4335 T	0.72	0.19
E -D	0.55	5696 T	0.40	0.15
------------Webs------------				
H -M	0.43	2485 C		
M -G	0.64	2022 T		
G -N	0.31	1657 C		
N -F	0.69	2174 T		
F -O	0.18	711 C		
O -L	0.09	302 T		
L -B	0.10	338 C		
B -K	0.62	1725 T		
K -P	0.11	387 C		
K -Q	0.65	692 C		
J -Q	0.04	162 T		
J -C	0.15	364 T		
I -C	0.24	778 T		
I -R	0.42	1050 C		
R -E	0.50	1560 T		
E -S	0.11	418 T		

LL Defl -0.06" in A -H L/999
TL Defl -0.14" in A -H L/467
Shear // Grain in S -D 0.30

Hz Disp	LL	DL	TL
Jt D	0.18"	0.18"	0.35"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

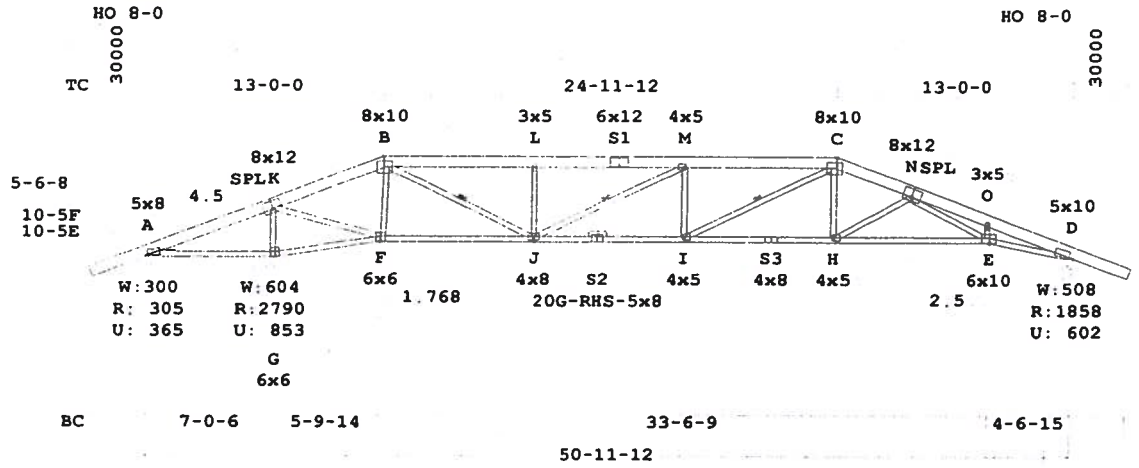
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	4.0x 5.0	-0.5	0.2	0.99
O	LOCK	4.0x 5.0	-0.5	0.2	0.67
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
P	LOCK	3.0x 5.0	Ctr	Ctr	0.36
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 6.0	0.5	0.2	0.94
S	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	5.0x10.0	Ctr	1.1	1.00
H	LOCK	6.0x 6.0	-0.3	3.3	0.75
G	LOCK	6.0x 6.0	Ctr	Ctr	0.88
F	LOCK	6.0x 6.0	Ctr	-0.5	0.76
L	LOCK	4.0x 5.0	Ctr	Ctr	0.66
K	LOCK	4.0x 8.0	Ctr	Ctr	0.69
S2	LOCK	4.0x 8.0	Ctr	0.3	0.75
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x10.0	-0.5	-0.5	0.75

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6100 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R107*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 437.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 8	SP-2400
BC	0.91	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.82	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50-11-12
BC Cont.	0- 0- 0	50-11-12

WB 1 rows CLB on B -J
WB 1 rows CLB on J -M
WB 1 rows CLB on I -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	306	365G	3- 0	1- 0
			Hz =	-90
G	2791	853	6- 4	3- 2
D	1859	603	5- 8	1- 9
			Hz =	91

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -K	0.26	1540 T	0.05	0.21
K -B	0.21	1245 C	0.00	0.21
B -L	0.09	3254 C	0.01	0.08
L -S1	0.09	3254 C	0.01	0.08
S1-M	0.08	3254 C	0.01	0.07
M -C	0.10	4055 C	0.01	0.09
C -N	0.06	3697 C	0.04	0.02
N -O	0.35	5296 C	0.03	0.32
O -D	0.35	5512 C	0.03	0.32
--------Bottom Chords---------				
A -G	0.56	1370 C	0.02	0.54
G -F	0.81	1401 C	0.01	0.80
F -J	0.54	1198 T	0.20	0.34
J -S2	0.83	4056 T	0.68	0.15
S2-I	0.87	4056 T	0.68	0.19
I -S3	0.76	3448 T	0.57	0.19
S3-H	0.78	3448 T	0.57	0.21
H -E	0.91	4186 T	0.70	0.21
E -D	0.49	5082 T	0.36	0.13
------------Webs------------				
G -K	0.44	2416 C		
K -F	0.82	2584 T		
B -F	0.24	891 C		
B -J	0.73	2288 T		1 Br
J -L	0.12	443 C		
J -M	0.30	899 C		1 Br
I -M	0.05	209 T		
I -C	0.21	674 T		1 Br
H -C	0.20	650 T		
H -N	0.33	853 C		
N -E	0.32	1020 T		
E -O	0.17	559 T		

LL Defl -0.07" in A -G L/999
TL Defl -0.18" in A -G L/426
Shear // Grain in O -D 0.34

Hz Disp	LL	DL	TL
Jt D	0.15"	0.15"	0.29"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

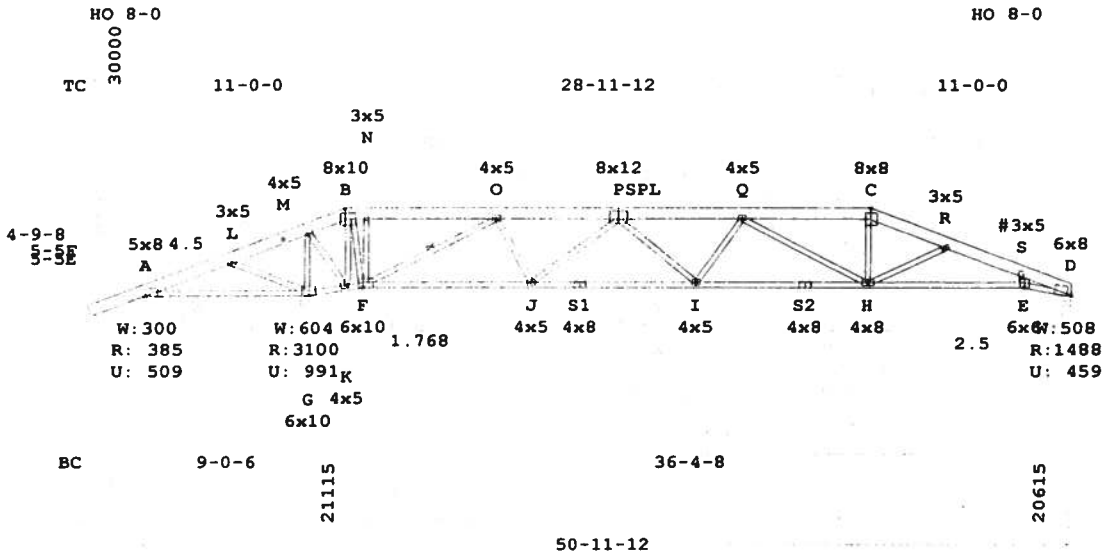
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
K	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	8.0x10.0	0.5	Ctr	0.98
L	LOCK	3.0x 5.0	Ctr	Ctr	0.36
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
M	LOCK	4.0x 5.0	Ctr	0.5	0.62
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	3.0x 5.0	Ctr	Ctr	0.36
D	LOCK	5.0x10.0	Ctr	1.1	1.00
G	LOCK	6.0x 6.0	-0.2	3.2	0.75
F	LOCK	6.0x 6.0	Ctr	-0.5	0.99
J	LOCK	4.0x 8.0	Ctr	Ctr	0.94
S2	RHS	5.0x 8.0	Ctr	0.8	0.75
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	4.0x 8.0	Ctr	0.2	0.75
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x10.0	-0.5	-0.5	0.77

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5512 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*R106*	1	SP	501112	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 431.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.25	2x 8	SP-2400
BC	0.84	2x 4	SP-#2
EX G -F		2x 4	SP-2400
WB	0.80	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50-11-12
BC	Cont.	0- 0- 0	50-11-12

WB 1 rows CLB on F -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	385	510G	3- 0	1- 0
		Hz =	-76	
G	3100	991	6- 4	3- 7
D	1488	459	5- 8	1-12
		Hz =	77	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.23	1471 T	0.05		0.18
L -M	0.25	2342 T	0.07		0.18
M -B	0.06	1203 T	0.04		0.02
B -N	0.05	713 T	0.02		0.03
N -O	0.08	711 T	0.02		0.06
O -P	0.06	2225 C	0.00		0.06
P -Q	0.07	3434 C	0.01		0.06
Q -C	0.08	2963 C	0.01		0.07
C -R	0.06	3152 C	0.03		0.03
R -S	0.24	3615 C	0.01		0.23
S -D	0.24	3922 C	0.01		0.23
Bottom Chords					
A -G	0.66	1265 C	0.04		0.62
G -K	0.70	2242 C	0.02		0.68
K -F	0.25	1101 C	0.00		0.25
F -J	0.63	1782 T	0.29		0.34
J -S1	0.76	3100 T	0.32		0.44
S1-I	0.77	3100 T	0.52		0.25
I -S2	0.83	3506 T	0.58		0.25
S2-H	0.84	3506 T	0.58		0.26
H -E	0.84	3454 T	0.58		0.26
E -D	0.72	3507 T	0.58		0.14
Webs					
L -G	0.35	1035 C			
G -M	0.43	1997 C			
M -K	0.56	1746 T			
K -B	0.48	1907 C			
B -F	0.49	1535 T			
F -N	0.06	278 C			
F -O	0.80	2810 C		1 Br	
O -J	0.31	986 T			
J -P	0.63	1127 C			
P -I	0.14	449 T			
I -Q	0.05	216 T			
Q -H	0.56	623 C			
H -C	0.23	733 T			
H -R	0.20	544 C			
E -S	0.22	710 T			

LL Defl -0.13" in A -G L/757
TL Defl -0.31" in A -G L/316
Shear // Grain in G -K 0.48

Hz Disp	LL	DL	TL
Jt D	0.10"	0.11"	0.21"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
L	LOCK	3.0x 5.0	Ctr Ctr		0.36
M	LOCK	4.0x 5.0	-0.5	0.2	0.98
B	LOCK	8.0x10.0	Ctr Ctr		0.84
N	LOCK	3.0x 5.0	Ctr Ctr		0.36
O	LOCK	4.0x 5.0	Ctr	0.5	0.89
P	LOCK	8.0x12.0	Ctr	1.5	0.99
Q	LOCK	4.0x 5.0	Ctr	0.5	0.61
C	LOCK	8.0x 8.0	Ctr Ctr		0.84
R	LOCK	3.0x 5.0	Ctr Ctr		0.36
S#	LOCK	3.0x 5.0	-0.6	1.4	0.41
D	LOCK	6.0x 8.0	Ctr	1.1	1.00
G	LOCK	6.0x10.0	-0.2	3.4	0.86
K	LOCK	4.0x 5.0	Ctr Ctr		0.72
F	LOCK	6.0x10.0	Ctr	-0.5	0.75
J	LOCK	4.0x 5.0	Ctr Ctr		0.55
S1	LOCK	4.0x 8.0	Ctr	0.2	0.75
I	LOCK	4.0x 5.0	Ctr Ctr		0.51
S2	LOCK	4.0x 8.0	Ctr	0.2	0.83
H	LOCK	4.0x 8.0	Ctr Ctr		0.56
E	LOCK	6.0x 6.0	Ctr	-0.5	0.75

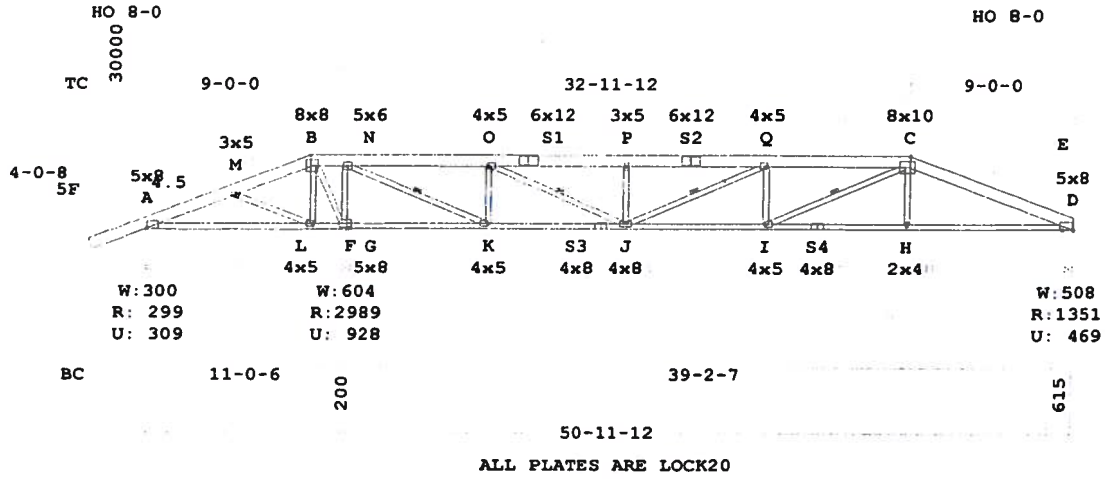
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3922 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R105*	1	SP	501112	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 419.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 8	SP-2400
BC	0.74	2x 4	SP-#2
WB	0.65	2x 4	SP-#3
EX N -K		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 4-13
BC	Cont.	0- 0- 0	50- 4-13

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on J -Q
WB 1 rows CLB on I -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	299	309G	3- 0	1- 0
			Hz =	-62
G	2990	928	6- 4	3- 5
D	1351	470	5- 8	1-10
			Hz =	63

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.12	1133 T	0.04		0.08
M -B	0.13	1745 T	0.05		0.08
B -N	0.21	2095 T	0.07		0.14
N -O	0.14	1029 C	0.00		0.14
O -S1	0.06	2883 C	0.03		0.03
S1-P	0.07	2883 C	0.01		0.06
P -S2	0.08	2883 C	0.01		0.07
S2-Q	0.08	2883 C	0.01		0.07
Q -C	0.09	3407 C	0.01		0.08
C -D	0.11	2788 C	0.01		0.10
--------Bottom Chords---------					
A -L	0.30	1000 C	0.00		0.30
L -G	0.30	1669 C	0.00		0.30
G -K	0.32	2181 C	0.01		0.31
K -S3	0.42	1029 T	0.11		0.31
S3-J	0.40	1029 T	0.11		0.29
J -I	0.74	3407 T	0.57		0.17
I -S4	0.57	2593 T	0.43		0.14
S4-H	0.62	2593 T	0.43		0.19
H -D	0.62	2599 T	0.43		0.19
------------Webs------------					
M -L	0.25	708 C			
L -B	0.11	377 T			
B -G	0.21	877 C			
G -N	0.46	2119 C			
N -K	0.64	3516 T			1 Br
K -O	0.28	1288 C			
O -J	0.65	2049 T			1 Br
J -P	0.10	477 C			
J -Q	0.17	578 C			1 Br
I -Q	0.05	210 T			
I -C	0.28	885 T			1 Br
H -C	0.08	324 T			

LL Defl -0.22" in J -I L/999
TL Defl -0.17" in A -L L/761
Shear // Grain in B -N 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

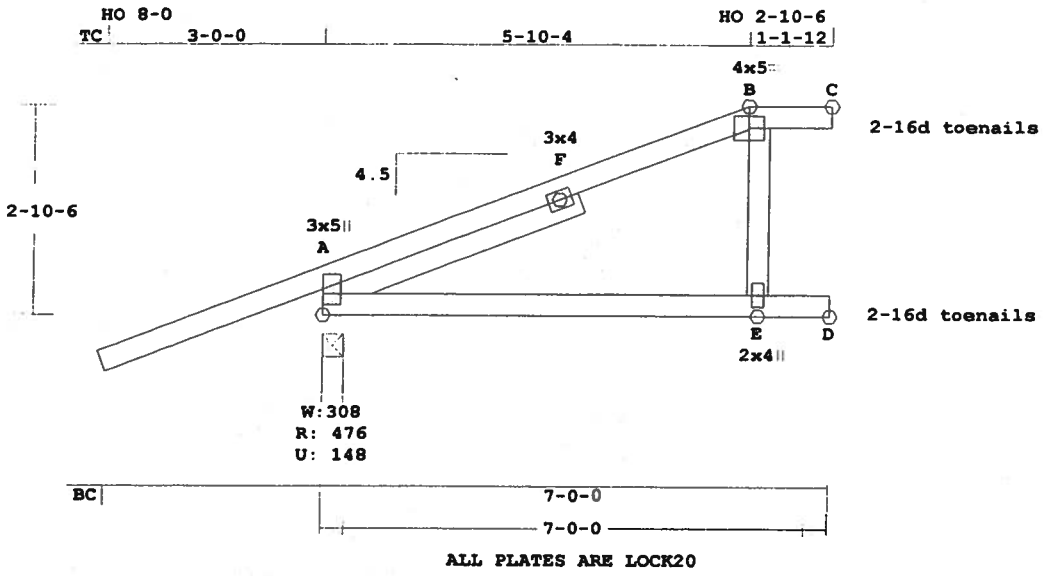
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
M	LOCK	3.0x 5.0	Ctr	Ctr	0.36
B	LOCK	8.0x 8.0-1.0	Ctr		0.84
N	LOCK	5.0x 6.0	Ctr	Ctr	0.86
O	LOCK	4.0x 5.0	Ctr	0.5	0.97
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	3.0x 5.0	Ctr	Ctr	0.36
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.64
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
D	LOCK	5.0x 8.0	Ctr	0.8	0.81
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	5.0x 8.0-1.0	Ctr		0.81
K	LOCK	4.0x 5.0-1.0	Ctr		1.00
S3	LOCK	4.0x 8.0	Ctr	0.2	0.75
J	LOCK	4.0x 8.0	Ctr	Ctr	0.87
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S4	LOCK	4.0x 8.0	Ctr	0.2	0.75
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3407 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R104	5	HHIP	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.62	2x 4	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.09	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	477	149	3- 8	1- 0
			Hz =	99
C	96	4	3- 8	1- 0
			Hz =	53
D	198	110	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.62	397 C	0.05	0.57
F -B	0.57	92 C	0.00	0.57
B -C	0.10	53 T	0.00	0.10
--------Bottom Chords---------				
A -E	0.40	21 C	0.00	0.40
E -D	0.40	0 T	0.00	0.40
------------Webs------------				
E -B	0.09	381 T		
----------Sliders----------				
A -F	0.11	493 C		

LL Defl -0.08" in A -E L/972
TL Defl -0.17" in A -E L/453
Shear // Grain in A -F 0.35

Hz Disp	LL	DL	TL
Jt C	0.10"	-0.01"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.62
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

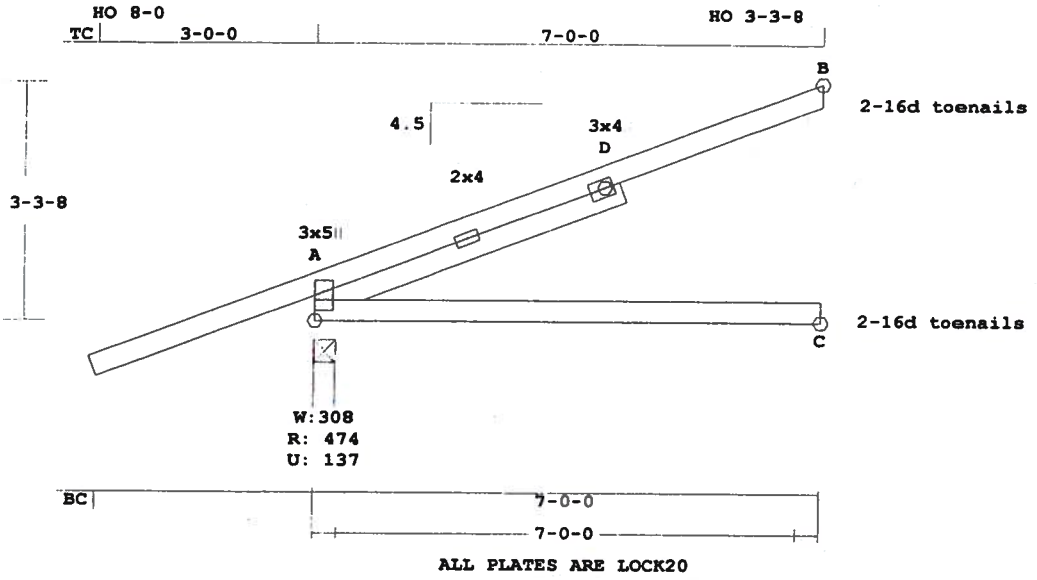
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 493 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R103*	3	JACK	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 0
BC	Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	474	138	3- 8	1- 0
			Hz =	118
B	207	136	3- 8	1- 0
C	118	0	3- 8	1- 0
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.53	420 C	0.00 0.53
D -B	0.53	117 C	0.00 0.53
Bottom Chords			
A -C	0.35	63 T	0.00 0.35
Sliders			
A -D	0.11	424 T	

LL Defl -0.05" in A -C L/999
TL Defl -0.13" in A -C L/580
Shear // Grain in A -D 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

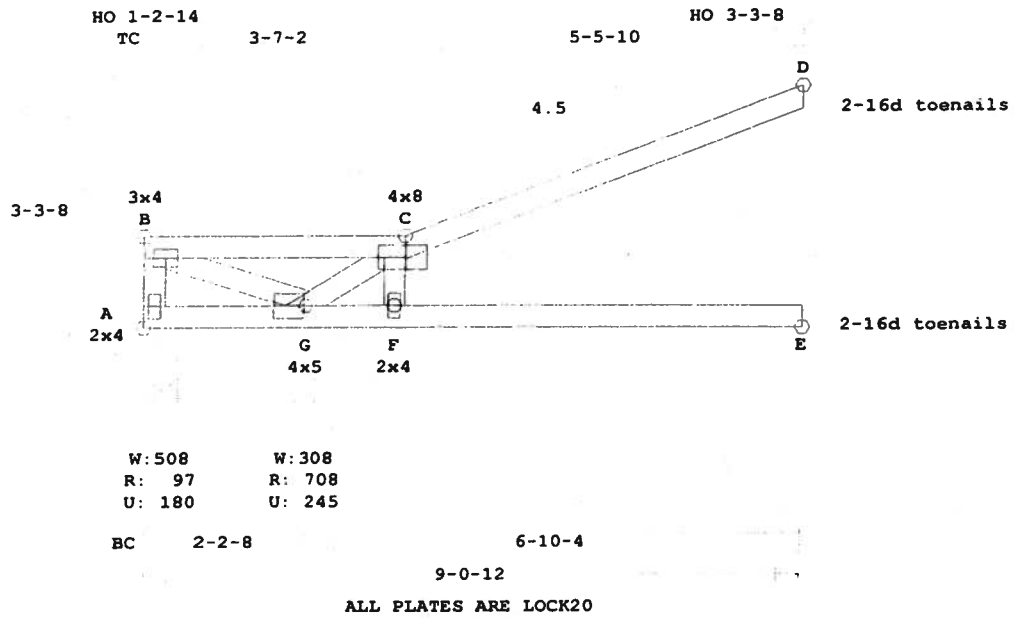
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 420 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R102*	1	SP	90012	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.19	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	97	180G	5- 8	1- 0
			Hz =	114
G	709	245	3- 8	1- 0
D	145	105	3- 8	1- 0
			Hz =	66
E	97	0	3- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
B -C	0.35	674 C	0.09	0.26
C -D	0.34	98 C	0.00	0.34
Bottom Chords				
A -G	0.07	220 C	0.00	0.07
G -F	0.15	31 T	0.00	0.15
F -E	0.16	0 T	0.00	0.16
Webs				
A -B	0.05	243 C	WindLd	
B -G	0.19	759 T		
G -C	0.13	700 C		
F -C	0.05	206 T		

LL Defl -0.03" in F -E L/999
TL Defl -0.06" in F -E L/999
Shear // Grain in C -D 0.30

Hz Disp	LL	DL	TL
Jt D	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.52
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	4.0x 5.0	0.5	Ctr	0.97
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

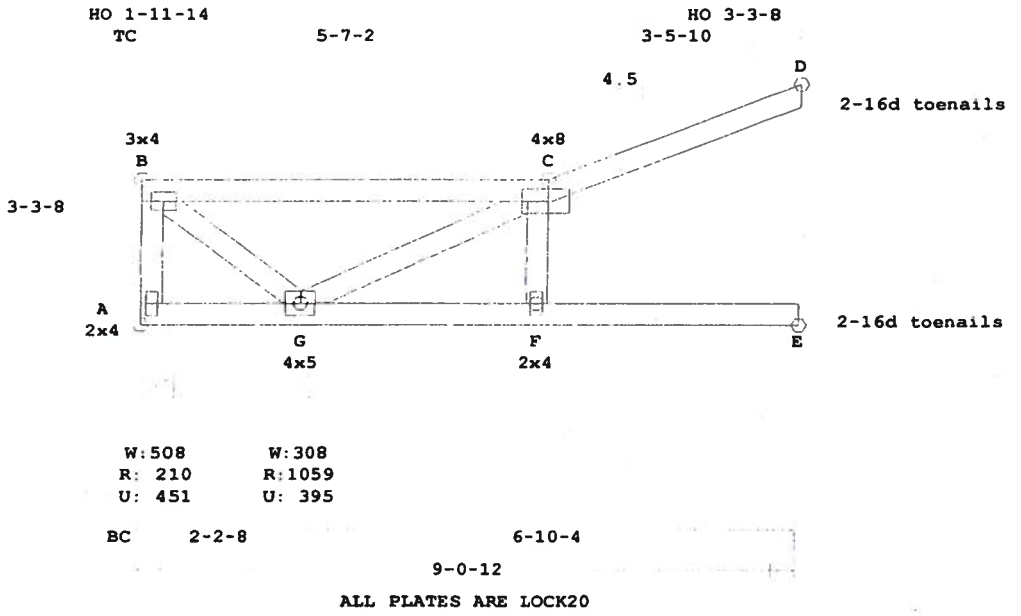
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 700 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R101*	1	SP	90012	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.41	2x 4	SP-#2
BC	0.10	2x 4	SP-#2
WB	0.26	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	210	452G	5- 8	1- 0
			Hz =	100
G	1059	395	3- 8	1- 2
D	72	54	3- 8	1- 0
			Hz =	64
E	71	0	3- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
B -C	0.41	779 C	0.12	0.29
C -D	0.26	59 T	0.00	0.26
Bottom Chords				
A -G	0.10	165 C	0.00	0.10
G -F	0.10	11 T	0.00	0.10
F -E	0.10	0 T	0.00	0.10
Webs				
A -B	0.14	498 C	WindLd	
B -G	0.26	1063 T		
G -C	0.19	804 C		
F -C	0.03	127 T		

LL Defl -0.01" in F -E L/999
TL Defl -0.03" in F -E L/999
Shear // Grain in B -C 0.24

Hz Disp	LL	DL	TL
Jt D	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.60
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.48
G	LOCK	4.0x 5.0	Ctr	Ctr	0.54
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

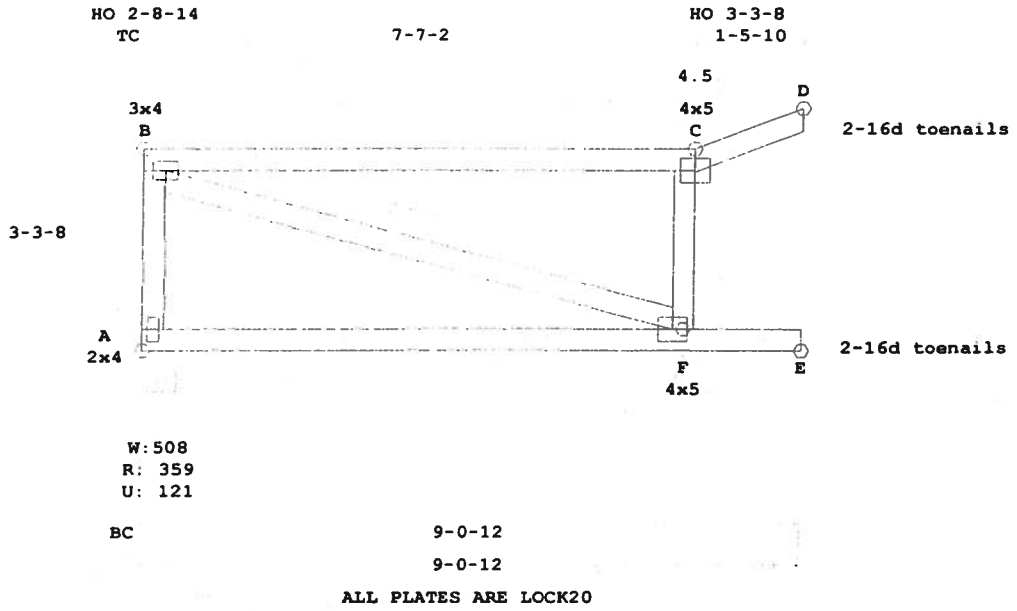
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 933 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R100*	1	SP	90012	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 56.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.87	2x 4	SP-#2
BC	0.57	2x 4	SP-#2
WB	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	360	122	5- 8	1- 0
			Hz =	87
D	203	3	3- 8	1- 0
			Hz =	62
E	203	126	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
B -C	0.87	71 C	0.00 0.87
C -D	0.32	71 T	0.00 0.32
A -F	0.57	110 C	0.00 0.57
F -E	0.51	0 T	0.00 0.51
A -B	0.10	246 C	WindLd
B -F	0.04	135 T	
F -C	0.07	294 T	

Top Chords
Bottom Chords
Webs

LL Defl -0.21" in A -F L/482
TL Defl -0.48" in A -F L/209
Shear // Grain in B -C 0.31

Hz Disp	LL	DL	TL
Jt D	0.01"	0.01"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.78

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

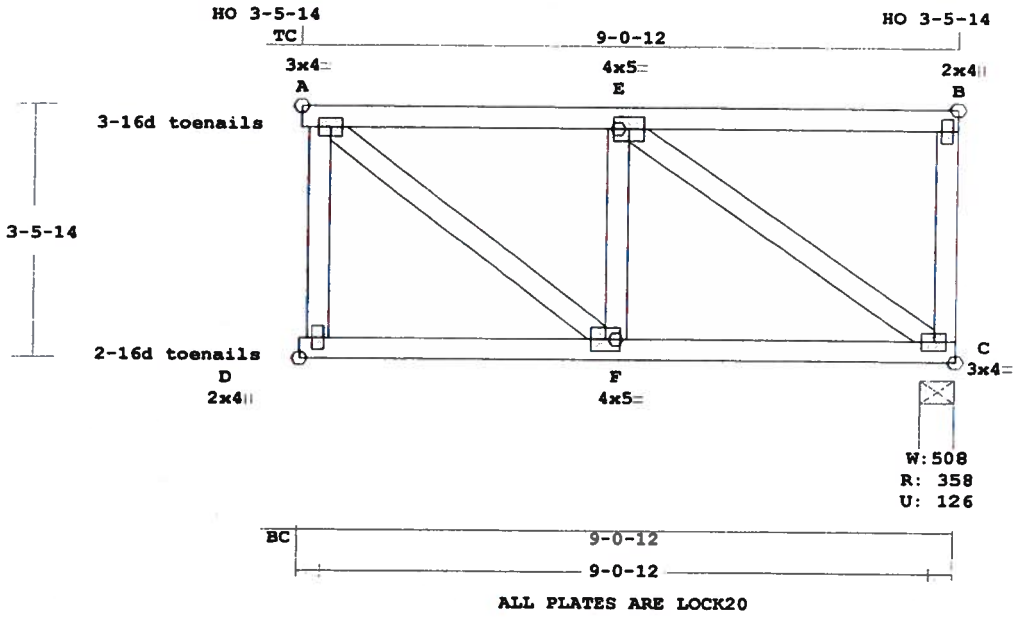
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 246 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R99	1	FLAT	90012	30514	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.15	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	324	141	3- 8	1- 0
D	67	0	3- 8	1- 0
			Hz =	-91
C	358	127	5- 8	1- 0
			Hz =	65

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -E	0.24	288 C	0.00 0.24
E -B	0.24	0 T	0.00 0.24
Bottom Chords			
D -F	0.13	76 C	0.00 0.13
F -C	0.16	288 T	0.03 0.13
Webs			
D -A	0.10	0 T	WindLd
A -F	0.11	364 T	
F -E	0.04	167 T	
E -C	0.15	352 C	
C -B	0.03	143 T	

LL Defl -0.01" in F -C L/999
TL Defl -0.02" in F -C L/999
Shear // Grain in E -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.52
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.52

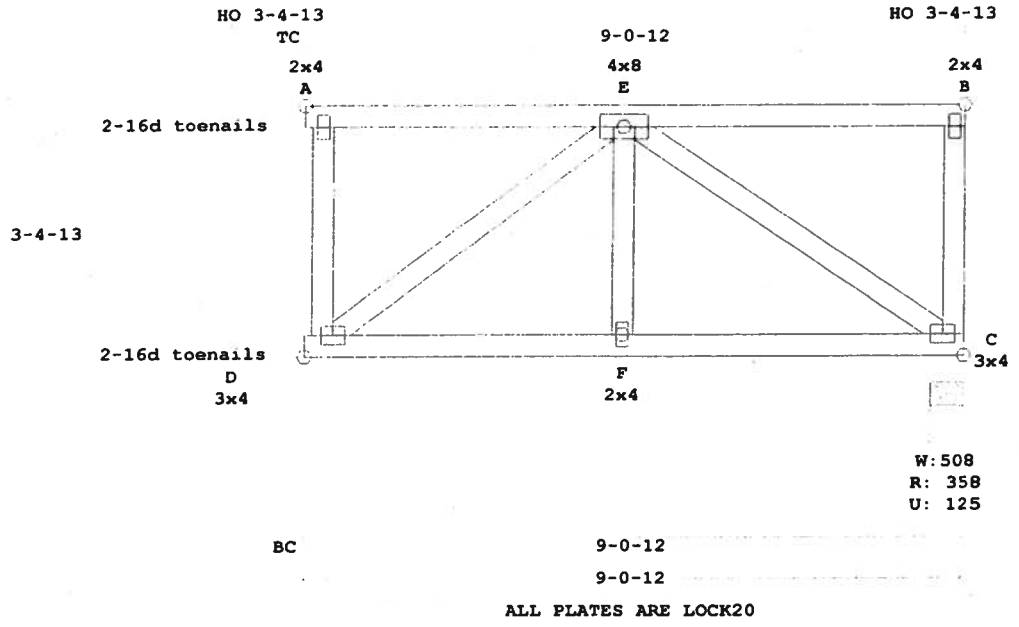
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 352 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R98*	1	FLAT	90012	30413	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.15	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	101	62	3- 8	1- 0
D	257	59	3- 8	1- 0
			Hz =	-88
C	358	126	5- 8	1- 0
			Hz =	63

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords					
A -E	0.24	74	T	0.00	0.24
E -B	0.24	0	T	0.00	0.24
Bottom Chords					
D -F	0.16	296	T	0.03	0.13
F -C	0.16	296	T	0.03	0.13
Webs					
D -A	0.09	0	T	WindLd	
D -E	0.14	370	C		
F -E	0.05	213	T		
E -C	0.15	359	C		
C -B	0.03	143	T		

LL Defl -0.01" in F -C L/999
TL Defl -0.03" in F -C L/999
Shear // Grain in E -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	4.0x 8.0	Ctr	Ctr	0.52
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	3.0x 4.0	Ctr	Ctr	0.52
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	3.0x 4.0	Ctr	Ctr	0.52

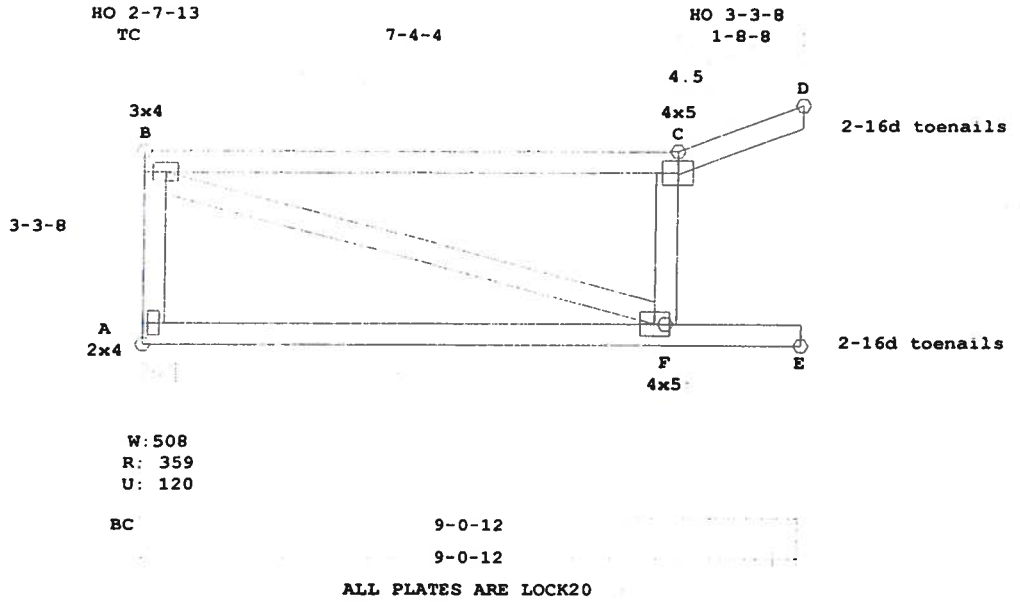
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 370 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*R97*	1	SP	90012	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.85	2x 4	SP-#2
BC	0.60	2x 4	SP-#2
WB	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	360	121	5- 8	1- 0
			Hz =	88
D	184	26	3- 8	1- 0
			Hz =	62
E	193	104	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.85	79 C	0.00 0.85
C -D	0.35	65 T	0.00 0.35
--------Bottom Chords---------			
A -F	0.60	117 C	0.00 0.60
F -E	0.56	0 T	0.00 0.56
-------------Webs-------------			
A -B	0.10	244 C	WindLd
B -F	0.04	141 T	
F -C	0.06	268 T	

LL Defl -0.21" in A -F L/467
TL Defl -0.48" in A -F L/206
Shear // Grain in B -C 0.31

Hz Disp	LL	DL	TL
Jt D	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.78

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

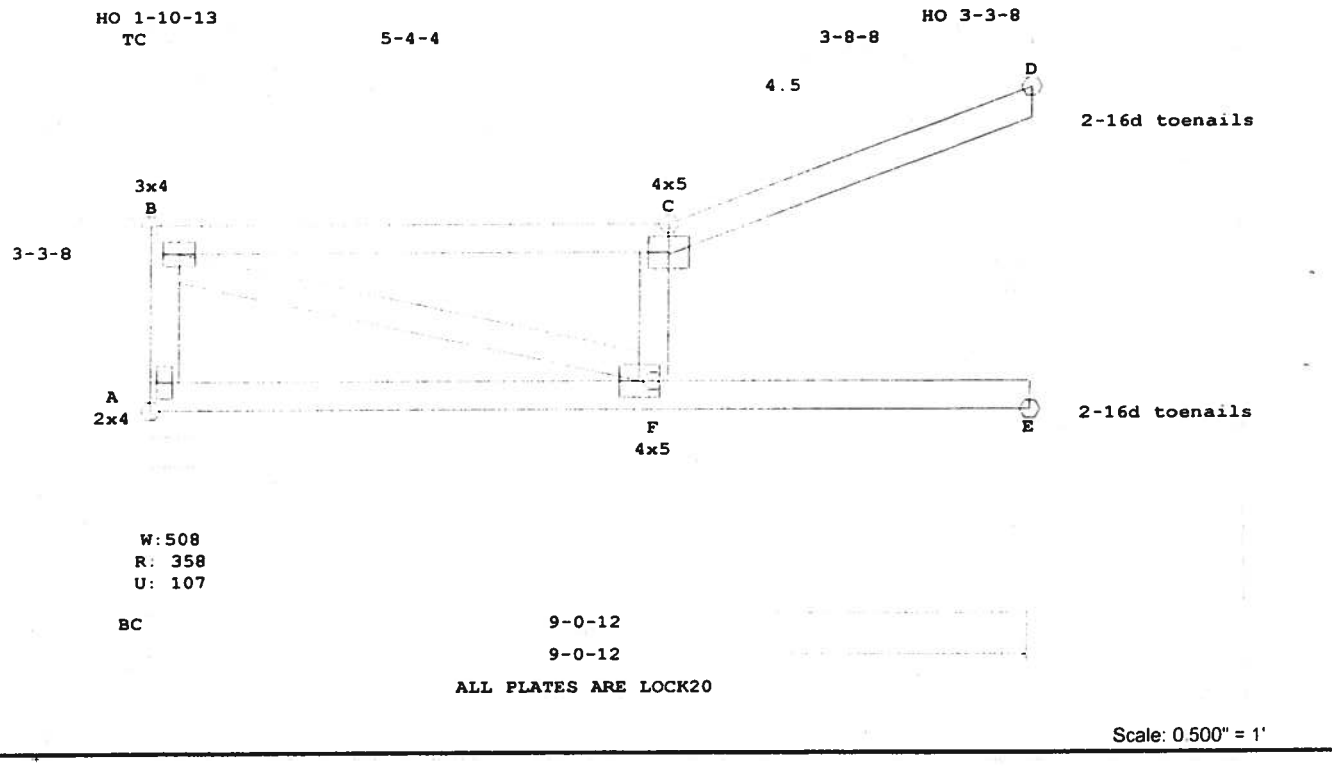
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 244 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R96*	1	SP	90012	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.72	2x 4	SP-#2
BC	0.76	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	359	108	5- 8	1- 0
			Hz =	102
D	217	108	3- 8	1- 0
			Hz =	64
E	149	27	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.72	156 C	0.00 0.72
C -D	0.59	104 C	0.00 0.59
--------Bottom Chords---------			
A -F	0.76	172 C	0.00 0.76
F -E	0.76	0 T	0.00 0.76
-------------Webs-------------			
A -B	0.06	225 C	WindLd
B -F	0.05	200 T	
F -C	0.05	204 T	

LL Defl -0.22" in A -F L/450
TL Defl -0.46" in A -F L/216
Shear // Grain in B -C 0.28

Hz Disp	LL	DL	TL
Jt D	0.02"	0.02"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.77

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

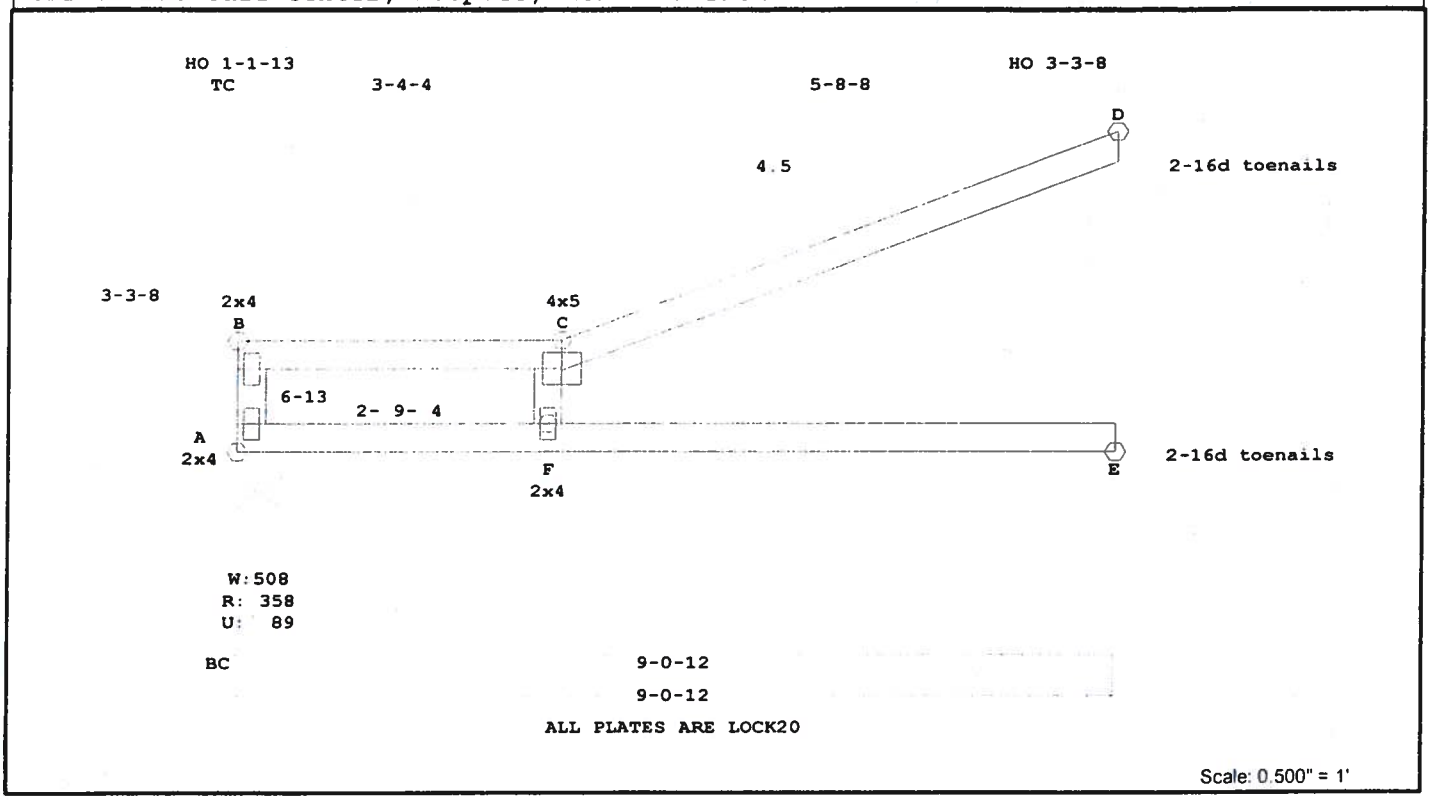
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 225 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R95*	1	SP	90012	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 4	SP-#2
BC	0.78	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	359	89	5- 8	1- 0
			Hz =	115
D	235	136	3- 8	1- 0
			Hz =	66
E	158	1	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.28	346 C	0.00 0.28
C -D	0.71	129 C	0.00 0.71
--------Bottom Chords---------			
A -F	0.49	432 C	0.05 0.44
F -E	0.78	0 T	0.00 0.78
------------Webs------------			
A -B	0.04	201 C	0.04 0.00
F -C	0.04	171 T	0.04 0.00

LL Defl -0.21" in F -E L/464
TL Defl -0.46" in F -E L/218
Shear // Grain in C -D 0.30

Hz Disp	LL	DL	TL
Jt D	0.03"	0.04"	0.07"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
B LOCK	2.0x 4.0	Ctr	Ctr	0.38
C LOCK	4.0x 5.0	Ctr	Ctr	0.66
A LOCK	2.0x 4.0	Ctr	Ctr	0.38
F LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

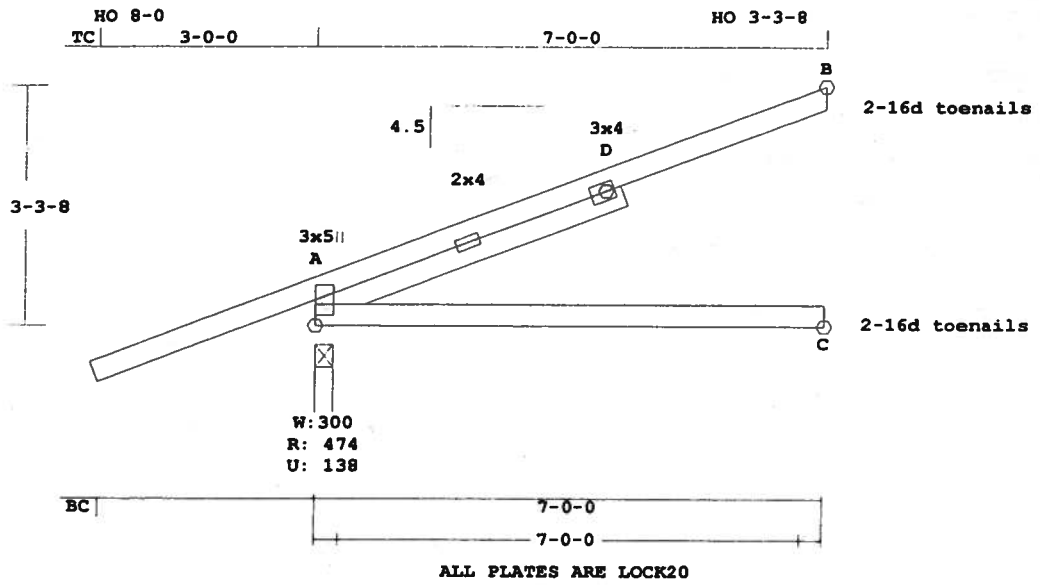
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 432 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R94	10	JACK	70000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	475	138	3- 0	1- 0
			Hz = 118	
B	206	135	3- 8	1- 0
C	118	0	3- 8	1- 0
			Hz = 63	

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.53	422	C 0.00 0.53
D -B	0.53	116	C 0.00 0.53
Bottom Chords			
A -C	0.36	63	T 0.00 0.36
Sliders			
A -D	0.11	426	T

LL Defl -0.06" in A -C L/999
TL Defl -0.13" in A -C L/575
Shear // Grain in A -D 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 422 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R93*	1*2P	HIPP	501112	4.5	3- 0- 0	0	

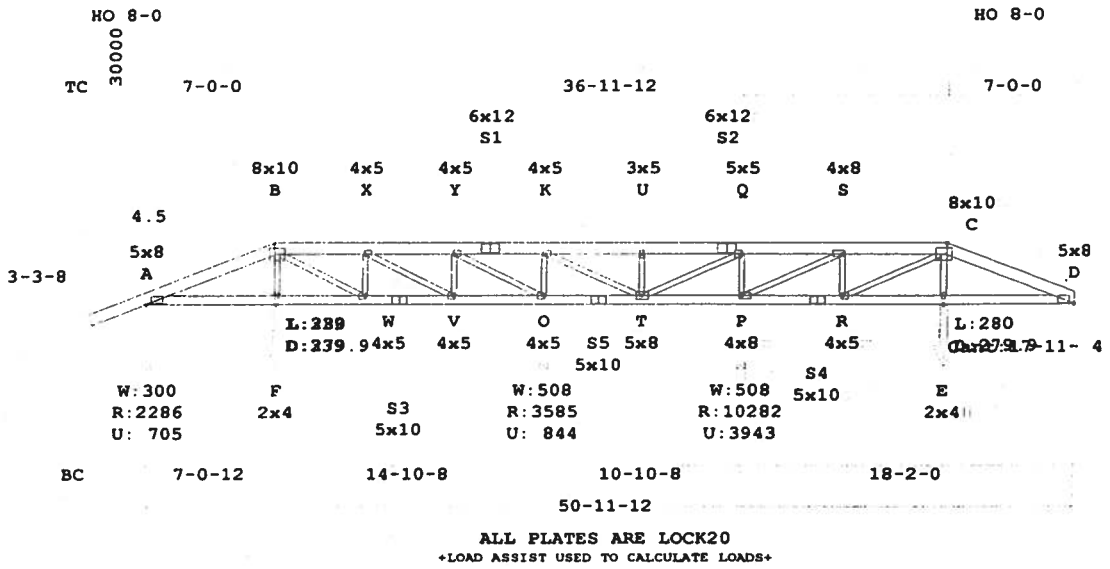
Office and Care Center, Hospice, M.M. Parrish

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 15793 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R93	1*2P	HIPP	501112	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 459.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	Size	Lumber
TC	0.55	2x 8	SP-2400
BC	0.80	2x 6	SP-#2
WB	1.00	2x 4	SP-#3
EX T -Q		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	70	70	8.1'	22.1'
BC V	91	91	22.1'	34.1'
BC V	70	70	34.1'	50.1'
TC V	50	25	7.0'	44.0'
BC V	0	25	7.1'	43.8'
BC V	339	339	7.1'	CL-LB
BC V	280	280	7.1'	CL-LB
BC V	280	280	43.8'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2286	706	3- 0	1- 6
			Hz =	-47
O	3586	844	5- 8	2- 0
P	10282	3943	5- 8	5-11 **
			Hz =	48

Membr	CSI	P Lbs	Axl-CSI-Bnd

Top Chords

A -B	0.04	4849	C	0.02	0.02
B -X	0.06	4023	C	0.00	0.06
X -Y	0.04	394	C	0.00	0.04
Y -S1	0.11	5572	T	0.10	0.01
S1-K	0.21	5572	T	0.10	0.11
K -U	0.28	9292	T	0.17	0.11
U -S2	0.33	9292	T	0.17	0.16
S2-Q	0.42	9292	T	0.17	0.25
Q -S	0.55	15793	T	0.30	0.25
S -C	0.28	6938	T	0.13	0.15
C -D	0.15	1505	T	0.02	0.13

Bottom Chords

A -F	0.38	4540	T	0.30	0.08
F -W	0.50	4471	T	0.29	0.21
W -S3	0.47	4023	T	0.26	0.21
S3-V	0.38	4023	T	0.26	0.12
V -O	0.36	372	T	0.02	0.34
O -S5	0.36	5572	C	0.02	0.34
S5-T	0.25	5572	C	0.02	0.23
T -P	0.80	15793	C	0.22	0.58
P -S4	0.62	6938	C	0.04	0.58
S4-R	0.32	6938	C	0.04	0.28
R -E	0.28	1395	C	0.00	0.28
E -D	0.24	1322	C	0.00	0.24

Webs

F -B	0.23	1519	T
B -W	0.04	502	C
W -X	0.20	1459	T
X -V	0.39	4184	C
V -Y	0.45	2828	T
Y -O	0.64	6813	C
O -K	0.15	1169	T
K -T	0.41	4171	C
T -U	0.04	345	T
T -Q	0.67	7290	T
P -Q	0.35	4441	C
P -S	1.00	9903	C
R -S	0.55	3478	T
R -C	0.61	6083	C
E -C	0.24	1565	T

LL Defl -0.08" in F -W L/999
TL Defl -0.16" in F -W L/999
LL Cant -0.57" in E -D L/378
Shear // Grain in T -P 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	5.0x 8.0	Ctr	Ctr	0.90
B LOCK	8.0x10.0	Ctr	Ctr	0.84
X LOCK	4.0x 5.0	Ctr	0.5	0.62
Y LOCK	4.0x 5.0	Ctr	0.5	0.90
S1 LOCK	6.0x12.0	Ctr	Ctr	0.91
K LOCK	4.0x 5.0	Ctr	0.5	0.63
U LOCK	3.0x 5.0	Ctr	Ctr	0.36
S2 LOCK	6.0x12.0	Ctr	Ctr	0.91
Q LOCK	5.0x 5.0-0.5	Ctr		0.99
S LOCK	4.0x 8.0-0.5	0.5		0.84
C LOCK	8.0x10.0	Ctr	Ctr	0.84
D LOCK	5.0x 8.0	Ctr	Ctr	0.90
F LOCK	2.0x 4.0	Ctr	Ctr	0.82
W LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3 LOCK	5.0x10.0	Ctr	Ctr	0.82
V LOCK	4.0x 5.0	Ctr	Ctr	0.66
O LOCK	4.0x 5.0	Ctr	Ctr	0.90
S5 LOCK	5.0x10.0	Ctr	Ctr	0.82
T LOCK	5.0x 8.0	0.5	Ctr	0.97
P LOCK	4.0x 8.0	0.5	Ctr	0.84
S4 LOCK	5.0x10.0	Ctr	Ctr	0.82
R LOCK	4.0x 5.0	Ctr	Ctr	0.84
E LOCK	2.0x 4.0	Ctr	Ctr	0.84

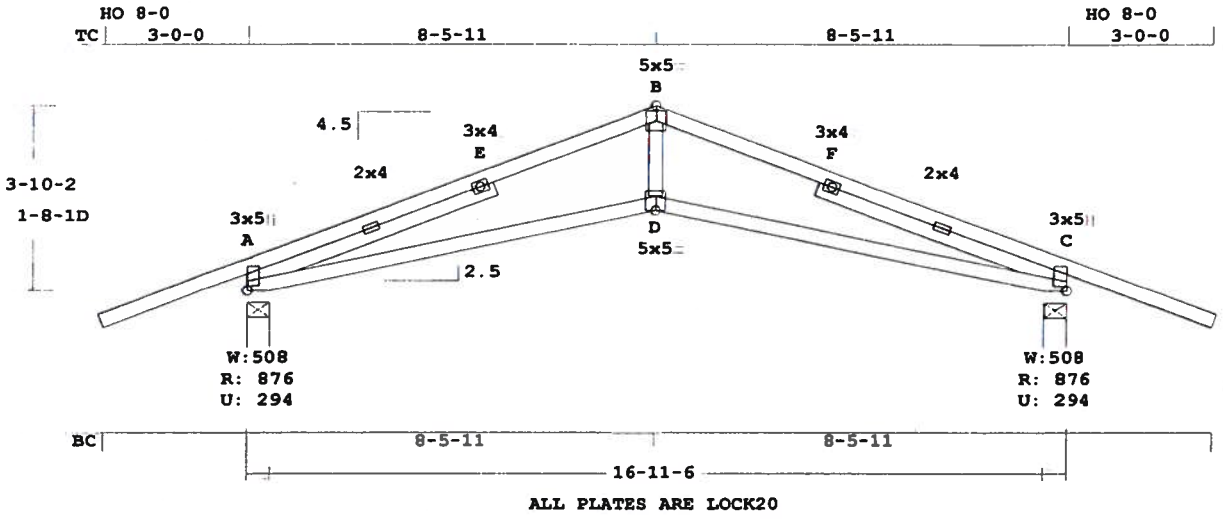
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R92	5	SCISSOR	161106	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 103.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.38	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.27	2x 4	SP-#3
SL	0.23	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16-11- 6
BC	Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	876	295	5- 8	1- 1
C	876	295	5- 8	1- 1

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -E	0.29	1115 C	0.07	0.22
E -B	0.38	1705 C	0.12	0.26
B -F	0.38	1705 C	0.11	0.27
F -C	0.29	1115 C	0.07	0.22
Bottom Chords				
A -D	0.44	1656 T	0.27	0.17
D -C	0.44	1656 T	0.27	0.17
Webs				
D -B	0.27	844 T		
Sliders				
A -E	0.23	674 C		
F -C	0.23	673 C		

LL Defl -0.08" in D -C L/999
TL Defl -0.19" in D -C L/999
Shear // Grain in E -B 0.23

Hz Disp	LL	DL	TL
Jt C	0.03"	0.04"	0.08"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

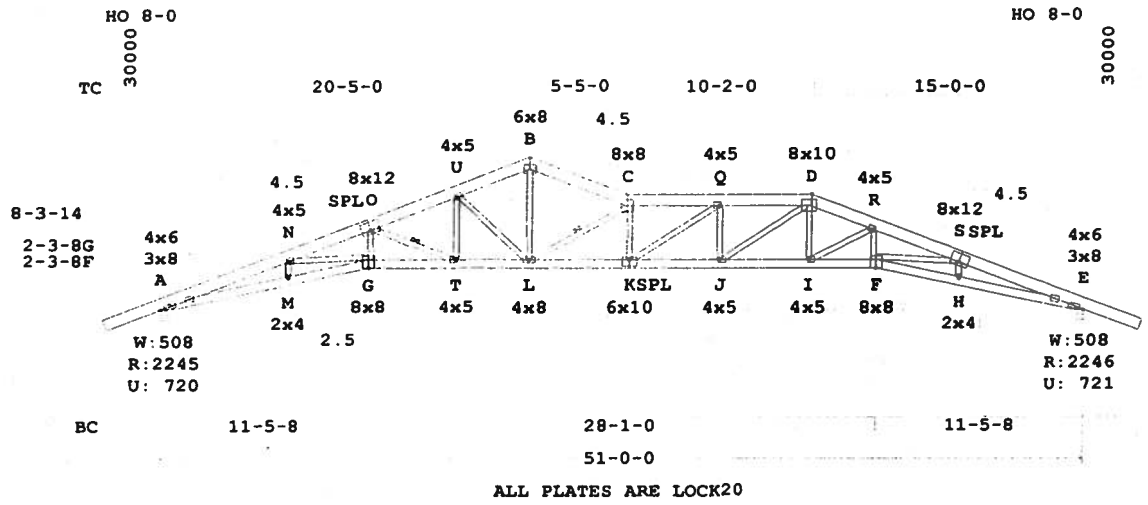
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.94
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0-1.5	0.4		0.94
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	5.0x 5.0	Ctr-1.1		0.46

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1705 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R148*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 492.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.95	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0
WB 1 rows CLB on O -T			
WB 1 rows CLB on L -C			

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2245	721	5- 8	1-14
		Hz =	-142	
E	2246	721	5- 8	1-14
		Hz =	143	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.28	8530	C	0.08	0.20
N -O	0.31	8732	C	0.09	0.22
O -U	0.15	5997	C	0.04	0.11
U -B	0.08	4868	C	0.06	0.02
B -C	0.18	4832	C	0.02	0.16
C -Q	0.20	7485	C	0.06	0.14
Q -D	0.13	7018	C	0.05	0.08
D -R	0.17	6515	C	0.04	0.13
R -S	0.31	8702	C	0.09	0.22
S -E	0.28	8519	C	0.08	0.20
Bottom Chords					
A -M	0.54	8058	T	0.36	0.18
M -G	0.55	8095	T	0.37	0.18
G -T	0.41	8134	T	0.37	0.04
T -L	0.35	5646	T	0.25	0.10
L -K	0.47	7500	T	0.34	0.13
K -J	0.45	7021	T	0.32	0.13
J -I	0.31	6123	T	0.28	0.03
I -F	0.41	8099	T	0.37	0.04
F -H	0.55	8083	T	0.36	0.19
H -E	0.55	8045	T	0.36	0.19
Webs					
M -N	0.03	148	T		
N -G	0.09	293	T		
G -O	0.58	1836	T		
O -T	0.50	2740	C		1 Br
T -U	0.35	1118	T		
U -L	0.65	1491	C		
L -B	0.95	2977	T		
L -C	0.71	3377	C		1 Br
K -C	0.09	379	C		
K -Q	0.19	571	T		
J -Q	0.08	405	C		
J -D	0.34	1071	T		
I -D	0.39	1216	T		
I -R	0.59	2270	C		
F -R	0.58	1829	T		
F -S	0.08	269	T		
H -S	0.03	150	T		

LL Defl -0.88" in K -J L/679
TL Defl -1.78" in K -J L/337
Shear // Grain in A -N 0.21

Hz Disp	LL	DL	TL
Jt E	0.45"	0.45"	0.90"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

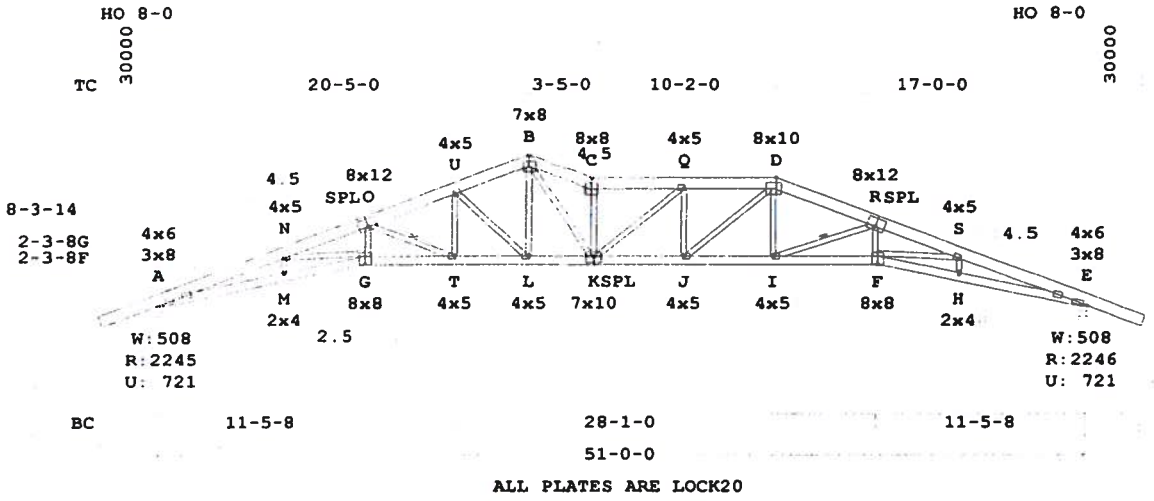
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
N	LOCK	4.0x 5.0	-0.5	0.2	0.62
O	LOCK	8.0x12.0	-0.6	1.5	0.97
U	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.96
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	0.5	0.2	0.90
S	LOCK	8.0x12.0	0.6	1.5	0.97
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.75
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x 8.0	Ctr	-1.5	0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.76
L	LOCK	4.0x 8.0	Ctr	Ctr	1.00
K	LOCK	6.0x10.0	Ctr	-1.2	0.97
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	8.0x 8.0	Ctr	-1.5	0.79
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8732 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R149*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 501.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.90	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

WB 1 rows CLB on O -T
WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2245	721	5- 8	1-14
			Hz = -140	
E	2246	721	5- 8	1-14
			Hz = 143	

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -N	0.28	8526	C	0.08 0.20
N -O	0.31	8720	C	0.09 0.22
O -U	0.15	6013	C	0.04 0.11
U -B	0.08	4843	C	0.06 0.02
B -C	0.10	6447	C	0.04 0.06
C -Q	0.09	6081	C	0.04 0.05
Q -D	0.08	5983	C	0.04 0.04
D -R	0.17	5766	C	0.04 0.13
R -S	0.31	8695	C	0.09 0.22
S -E	0.28	8529	C	0.08 0.20
Bottom Chords				
A -M	0.54	8049	T	0.36 0.18
M -G	0.55	8086	T	0.37 0.18
G -T	0.41	8117	T	0.37 0.04
T -L	0.32	5664	T	0.25 0.07
L -K	0.28	4497	T	0.20 0.08
K -J	0.35	5981	T	0.27 0.08
J -I	0.26	5405	T	0.24 0.02
I -F	0.41	8089	T	0.37 0.04
F -H	0.55	8088	T	0.37 0.18
H -E	0.54	8050	T	0.36 0.18
Webs				
M -N	0.03	146	T	
N -G	0.09	284	T	
G -O	0.58	1832	T	
O -T	0.49	2701	C	1 Br
T -U	0.36	1126	T	
U -L	0.69	1580	C	
L -B	0.42	1318	T	
B -K	0.90	2807	T	
K -C	0.64	2450	C	
K -Q	0.09	172	C	
J -Q	0.08	316	C	
J -D	0.23	738	T	
I -D	0.36	1125	T	
I -R	0.56	2851	C	1 Br
F -R	0.58	1819	T	
F -S	0.08	254	T	
H -S	0.03	141	T	

LL Defl -0.78" in K -J L/775
TL Defl -1.56" in K -J L/385
Shear // Grain in A -N 0.20

Hz Disp	LL	DL	TL
Jt E	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

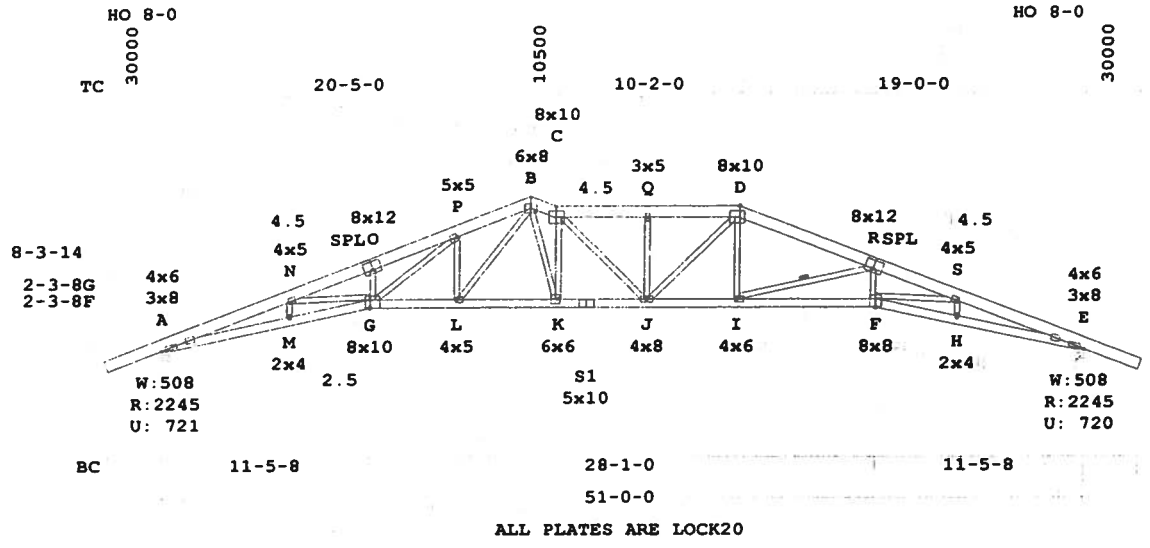
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
N	LOCK	4.0x 5.0	-0.5	0.2	0.62
O	LOCK	8.0x12.0	-0.6	1.5	0.97
U	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	7.0x 8.0	0.5	Ctr	0.96
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.63
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x 8.0	Ctr	-1.5	0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.75
L	LOCK	4.0x 5.0	Ctr	Ctr	0.62
K	LOCK	7.0x10.0	Ctr	-0.8	0.97
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.82
F	LOCK	8.0x 8.0	Ctr	-1.5	0.79
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8720 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R150*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 504.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 8	SP-2400
BC	0.61	2x 6	SP-2400
EX G -S1		2x 6	SP-#2
WB	0.89	2x 4	SP-#3
EX G -P		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2246	721	5- 8	1-14
			Hz =	-141
E	2245	721	5- 8	1-14
			Hz =	143

Membr	CSI	P Lbs	Axl-CSI-Bnd	
		--------Top Chords---------		
A -N	0.30	8340 C	0.07	0.23
N -O	0.33	8902 C	0.09	0.24
O -P	0.21	8909 C	0.09	0.12
P -B	0.09	6030 C	0.04	0.05
B -C	0.09	5374 C	0.03	0.06
C -Q	0.08	5128 C	0.02	0.06
Q -D	0.08	5128 C	0.02	0.06
D -R	0.19	5183 C	0.03	0.16
R -S	0.30	8713 C	0.09	0.21
S -E	0.28	8543 C	0.08	0.20
		--------Bottom Chords---------		
A -M	0.61	7865 T	0.36	0.25
M -G	0.61	7925 T	0.36	0.25
G -L	0.81	5667 T	0.68	0.13
L -K	0.59	4449 T	0.54	0.05
K -S1	0.67	5144 T	0.62	0.05
S1-J	0.26	5144 T	0.23	0.03
J -I	0.25	4856 T	0.22	0.03
I -F	0.40	8112 T	0.37	0.03
F -H	0.55	8102 T	0.37	0.18
H -E	0.54	8067 T	0.36	0.18
		------------Webs------------		
M -N	0.04	201 C		
N -G	0.20	641 T		
G -O	0.05	309 C		
G -P	0.63	3419 T		
L -P	0.32	1356 C		
L -B	0.62	1932 T		
B -K	0.70	2191 T		
K -C	0.67	1962 C		
C -J	0.07	109 C		
J -Q	0.06	196 C		
J -D	0.13	377 T		
I -D	0.34	1083 T		
I -R	0.89	3364 C	1 Br	
F -R	0.58	1821 T		
F -S	0.08	262 T		
H -S	0.03	129 T		

LL Defl -0.73" in S1-J L/818
TL Defl -1.48" in S1-J L/407
Shear // Grain in A -N 0.21

Hz Disp	LL	DL	TL
Jt E	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

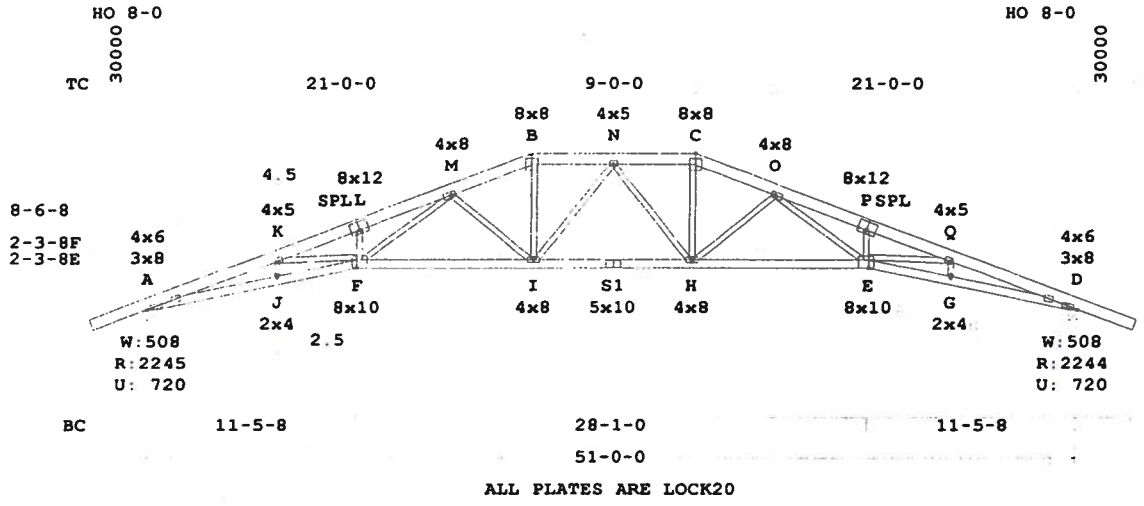
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.71
N	LOCK	4.0x 5.0-0.5	0.2		0.62
O	LOCK	8.0x12.0-0.6	1.5		0.97
P	LOCK	5.0x 5.0-0.5-0.2			0.95
B	LOCK	6.0x 8.0	Ctr	Ctr	0.95
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
Q	LOCK	3.0x 5.0	Ctr	Ctr	0.36
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.62
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x10.0	Ctr-1.5		0.81
L	LOCK	4.0x 5.0	Ctr	Ctr	0.76
K	LOCK	6.0x 6.0	Ctr	Ctr	0.95
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
I	LOCK	4.0x 6.0	Ctr	Ctr	0.82
F	LOCK	8.0x 8.0	Ctr-1.5		0.79
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8909 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R151*	2	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 490.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 8	SP-2400
BC	0.62	2x 6	SP-2400
EX	F -S1	2x 6	SP-#2
EX	S1-E	2x 6	SP-#2
WB	0.80	2x 4	SP-#3
EX	F -M	2x 4	SP-#2
EX	O -E	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2245	721	5- 8	1-14
			Hz =	-146
D	2245	721	5- 8	1-14
			Hz =	147

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -K	0.31	8299 C	0.07	0.24
K -L	0.34	8904 C	0.09	0.25
L -M	0.21	8893 C	0.09	0.12
M -B	0.07	4693 C	0.05	0.02
B -N	0.06	4392 C	0.05	0.01
N -C	0.06	4392 C	0.05	0.01
C -O	0.07	4693 C	0.05	0.02
O -P	0.21	8893 C	0.09	0.12
P -Q	0.34	8903 C	0.09	0.25
Q -D	0.31	8299 C	0.07	0.24
--------Bottom Chords---------				
A -J	0.61	7824 T	0.35	0.26
J -F	0.62	7889 T	0.36	0.26
F -I	0.87	5570 T	0.67	0.20
I -S1	0.70	4477 T	0.54	0.16
S1-H	0.70	4477 T	0.54	0.16
H -E	0.87	5570 T	0.67	0.20
E -G	0.62	7888 T	0.36	0.26
G -D	0.61	7824 T	0.35	0.26
------------Webs------------				
J -K	0.04	222 C		
K -F	0.21	679 T		
F -L	0.05	263 C		
F -M	0.66	3507 T		
M -I	0.80	1569 C		
I -B	0.43	1362 T		
I -N	0.10	148 T		
N -H	0.10	148 T		
H -C	0.43	1362 T		
H -O	0.80	1569 C		
O -E	0.66	3507 T		
E -P	0.05	263 C		
E -Q	0.21	678 T		
G -Q	0.04	222 C		

LL Defl -0.68" in F -I L/890
TL Defl -1.41" in F -I L/425
Shear // Grain in A -K 0.21

Hz Disp	LL	DL	TL
Jt D	0.40"	0.41"	0.81"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

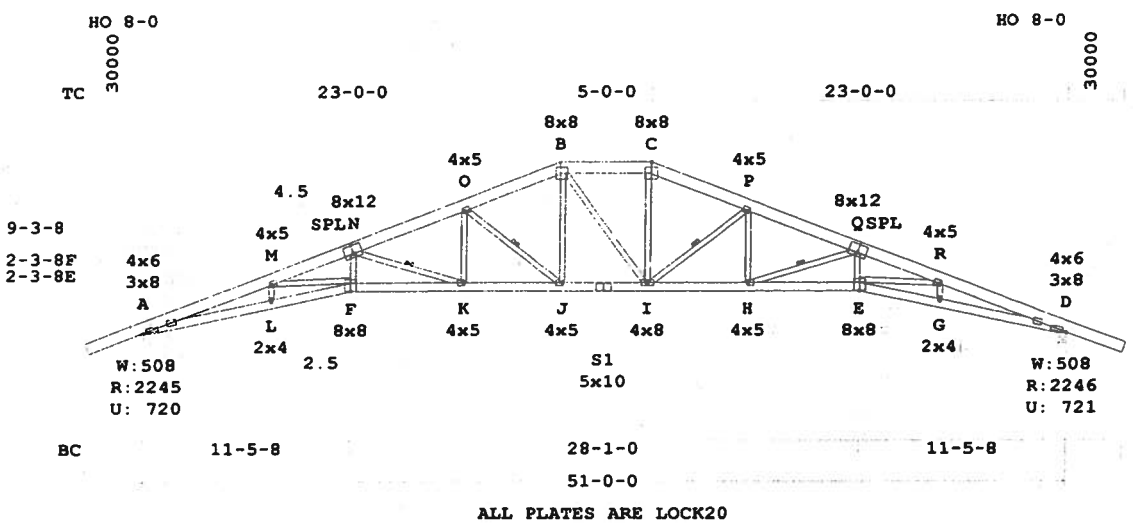
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.70
K	LOCK	4.0x 5.0	-0.5	0.2	0.62
L	LOCK	8.0x12.0	-0.6	1.5	0.97
M	LOCK	4.0x 8.0	-1.2	-0.1	0.81
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
N	LOCK	4.0x 5.0	Ctr	0.5	0.56
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
O	LOCK	4.0x 8.0	1.2	-0.1	0.81
P	LOCK	8.0x12.0	0.6	1.5	0.97
Q	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.70
J	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x10.0	Ctr	-1.5	0.81
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 8.0	Ctr	Ctr	0.51
E	LOCK	8.0x10.0	Ctr	-1.5	0.81
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8904 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R152*	2	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 501.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.63	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2245	721	5- 8	1-14
			Hz =	-160
D	2246	721	5- 8	1-14
			Hz =	161

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.28	8539	C	0.08	0.20
M -N	0.31	8716	C	0.09	0.22
N -O	0.16	5593	C	0.04	0.12
O -B	0.08	4220	C	0.05	0.03
B -C	0.06	3954	C	0.01	0.05
C -P	0.08	4228	C	0.05	0.03
P -Q	0.17	5595	C	0.04	0.13
Q -R	0.31	8716	C	0.09	0.22
R -D	0.28	8540	C	0.08	0.20
--------Bottom Chords---------					
A -L	0.54	8063	T	0.36	0.18
L -F	0.55	8100	T	0.37	0.18
F -K	0.41	8115	T	0.37	0.04
K -J	0.27	5268	T	0.24	0.03
J -S1	0.18	3933	T	0.18	0.00
S1-I	0.22	3933	T	0.18	0.04
I -H	0.28	5272	T	0.24	0.04
H -E	0.41	8115	T	0.37	0.04
E -G	0.55	8101	T	0.37	0.18
G -D	0.54	8065	T	0.36	0.18
------------Webs------------					
L -M	0.03	137	T		
M -F	0.08	268	T		
F -N	0.58	1822	T		
N -K	0.63	3026	C		1 Br
K -O	0.36	1122	T		
O -J	0.38	1691	C		1 Br
J -B	0.36	1152	T		
B -I	0.10	144	T		
I -C	0.37	1168	T		
I -P	0.38	1692	C		1 Br
H -P	0.35	1103	T		
H -Q	0.63	3021	C		1 Br
E -Q	0.58	1826	T		
E -R	0.08	267	T		
G -R	0.03	137	T		

LL Defl -0.68" in K -J L/887
TL Defl -1.37" in K -J L/440
Shear // Grain in A -M 0.20

Hz Disp	LL	DL	TL
Jt D	0.41"	0.42"	0.83"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

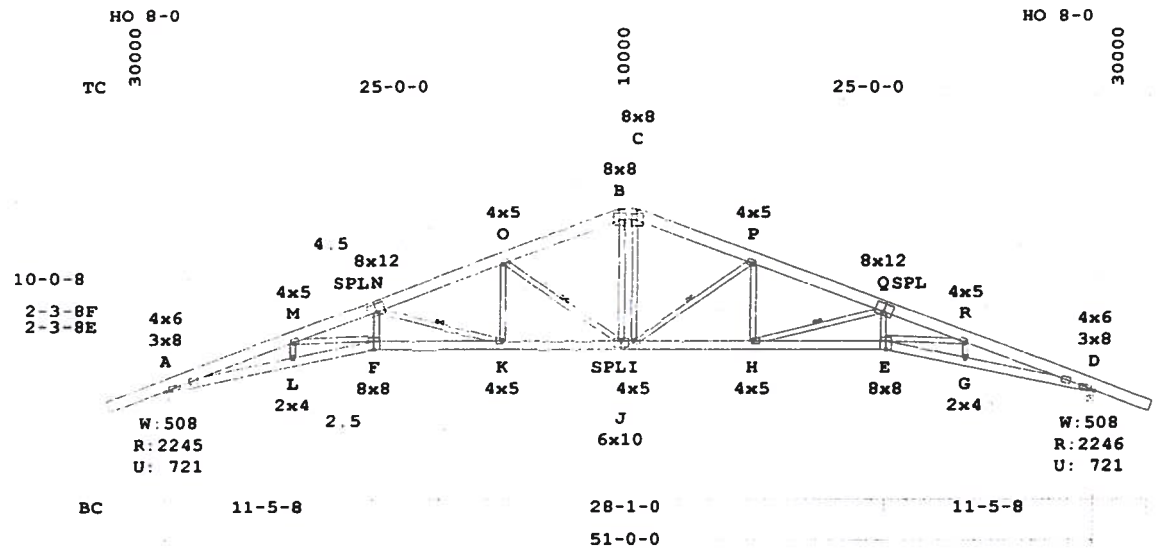
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	4.0x 5.0-0.5	0.2		0.62
N	LOCK	8.0x12.0-0.6	1.5		0.97
O	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	8.0x 8.0-1.0	Ctr	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.56
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.76
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr-1.5		0.80
K	LOCK	4.0x 5.0	Ctr	Ctr	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.86
E	LOCK	8.0x 8.0	Ctr-1.5		0.80
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8716 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R153*	2	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 500.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.79	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2246	721	5- 8	1-14
			Hz =	-175
D	2246	721	5- 8	1-14
			Hz =	175

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.28	8546	C	0.08	0.20
M -N	0.31	8712	C	0.09	0.22
N -O	0.17	5317	C	0.03	0.14
O -B	0.09	3829	C	0.04	0.05
B -C	0.04	3573	C	0.04	0.00
C -P	0.09	3829	C	0.04	0.05
P -Q	0.17	5317	C	0.03	0.14
Q -R	0.31	8712	C	0.09	0.22
R -D	0.28	8546	C	0.08	0.20
Bottom Chords					
A -L	0.55	8072	T	0.37	0.18
L -F	0.55	8108	T	0.37	0.18
F -K	0.40	8112	T	0.37	0.03
K -J	0.26	5014	T	0.22	0.04
J -I	0.20	3574	T	0.16	0.04
I -H	0.26	5014	T	0.22	0.04
H -E	0.40	8112	T	0.37	0.03
E -G	0.55	8108	T	0.37	0.18
G -D	0.55	8071	T	0.37	0.18
Webs					
L -M	0.03	135	T		
M -F	0.08	257	T		
F -N	0.58	1826	T		
N -K	0.79	3241	C		1 Br
K -O	0.34	1071	T		
O -J	0.48	1794	C		1 Br
J -B	0.36	1152	T		
I -C	0.36	1152	T		
I -P	0.48	1794	C		1 Br
H -P	0.34	1071	T		
H -Q	0.79	3241	C		1 Br
E -Q	0.58	1826	T		
E -R	0.08	257	T		
G -R	0.03	135	T		

LL Defl -0.69" in K -J L/875
TL Defl -1.38" in K -J L/434
Shear // Grain in A -M 0.20

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

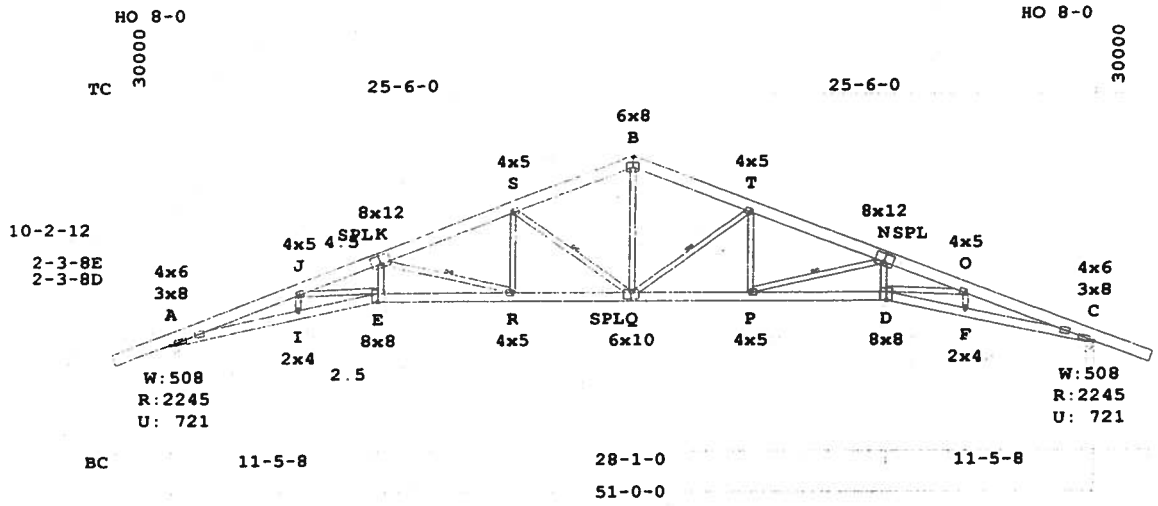
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	4.0x 5.0	-0.5	0.2	0.62
N	LOCK	8.0x12.0	-0.6	1.5	0.97
O	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.56
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.76
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr	-1.5	0.80
K	LOCK	4.0x 5.0	Ctr	Ctr	0.94
J	LOCK	6.0x10.0	Ctr	-1.3	0.97
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	4.0x 5.0	Ctr	Ctr	0.94
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8712 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R154*	43	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 489.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2246	721	5- 8	1-14
			Hz =	-177
C	2246	721	5- 8	1-14
			Hz =	179

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -J	0.28	8543 C	0.08	0.20
J -K	0.30	8709 C	0.09	0.21
K -S	0.18	5248 C	0.03	0.15
S -B	0.09	3757 C	0.04	0.05
B -T	0.09	3757 C	0.04	0.05
T -N	0.18	5248 C	0.03	0.15
N -O	0.30	8708 C	0.09	0.21
O -C	0.28	8542 C	0.08	0.20
Bottom Chords				
A -I	0.54	8065 T	0.36	0.18
I -E	0.55	8101 T	0.37	0.18
E -R	0.40	8105 T	0.37	0.03
R -Q	0.27	4945 T	0.22	0.05
Q -P	0.27	4945 T	0.22	0.05
P -D	0.40	8104 T	0.37	0.03
D -F	0.55	8100 T	0.37	0.18
F -C	0.54	8064 T	0.36	0.18
Webs				
I -J	0.03	134 T		
J -E	0.08	257 T		
E -K	0.58	1826 T		
K -R	0.84	3296 C		1 Br
R -S	0.34	1060 T		
S -Q	0.50	1802 C		1 Br
Q -B	0.74	2310 T		
Q -T	0.50	1801 C		1 Br
P -T	0.34	1060 T		
P -N	0.84	3295 C		1 Br
D -N	0.58	1825 T		
D -O	0.08	256 T		
F -O	0.03	134 T		

LL Defl -0.69" in R -Q L/868
TL Defl -1.39" in R -Q L/431
Shear // Grain in A -J 0.20

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

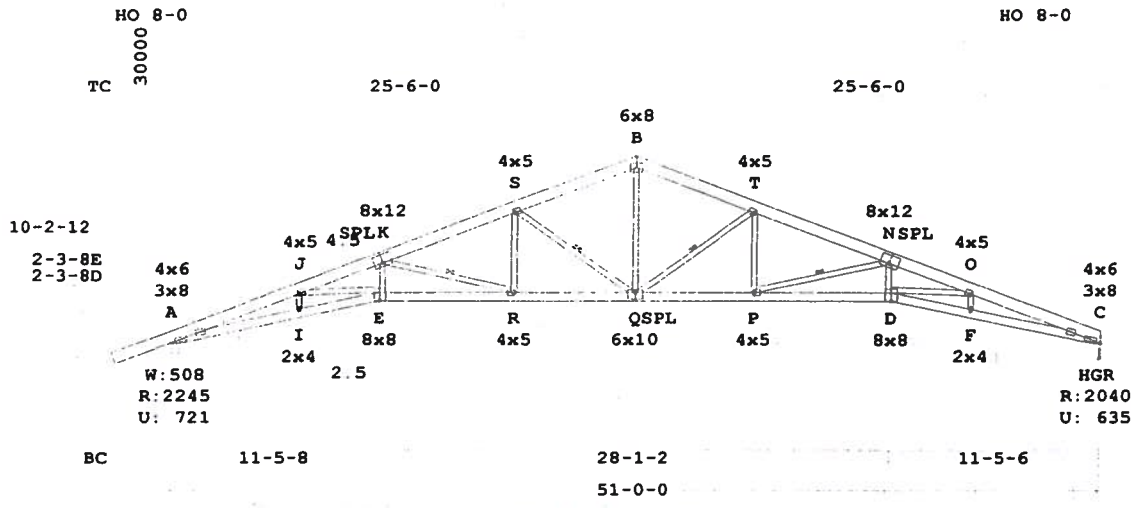
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.96
D	LOCK	8.0x 8.0	Ctr	-1.5	0.80
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8709 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R155*	6	SP	510000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 477.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2246	721	5- 8	1-14
			Hz =	-178
C	2040	636	3- 8	1-11
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.28	8548	C	0.08	0.20
J -K	0.30	8714	C	0.09	0.21
K -S	0.18	5251	C	0.03	0.15
S -B	0.09	3759	C	0.04	0.05
B -T	0.09	3759	C	0.04	0.05
T -N	0.18	5250	C	0.03	0.15
N -O	0.30	8714	C	0.09	0.21
O -C	0.28	8561	C	0.08	0.20
Bottom Chords					
A -I	0.55	8074	T	0.37	0.18
I -E	0.55	8110	T	0.37	0.18
E -R	0.40	8114	T	0.37	0.03
R -Q	0.27	4952	T	0.22	0.05
Q -P	0.27	4951	T	0.22	0.05
P -D	0.40	8114	T	0.37	0.03
D -F	0.55	8123	T	0.37	0.18
F -C	0.55	8087	T	0.37	0.18
Webs					
I -J	0.03	134	T		
J -E	0.08	257	T		
E -K	0.58	1828	T		
K -R	0.84	3299	C		1 Br
R -S	0.34	1061	T		
S -Q	0.50	1803	C		1 Br
Q -B	0.74	2311	T		
Q -T	0.50	1802	C		1 Br
P -T	0.34	1060	T		
P -N	0.84	3299	C		1 Br
D -N	0.58	1829	T		
D -O	0.07	244	T		
F -O	0.03	132	T		

LL Defl -0.69" in Q -P L/870
TL Defl -1.40" in Q -P L/432
Shear // Grain in A -J 0.20

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

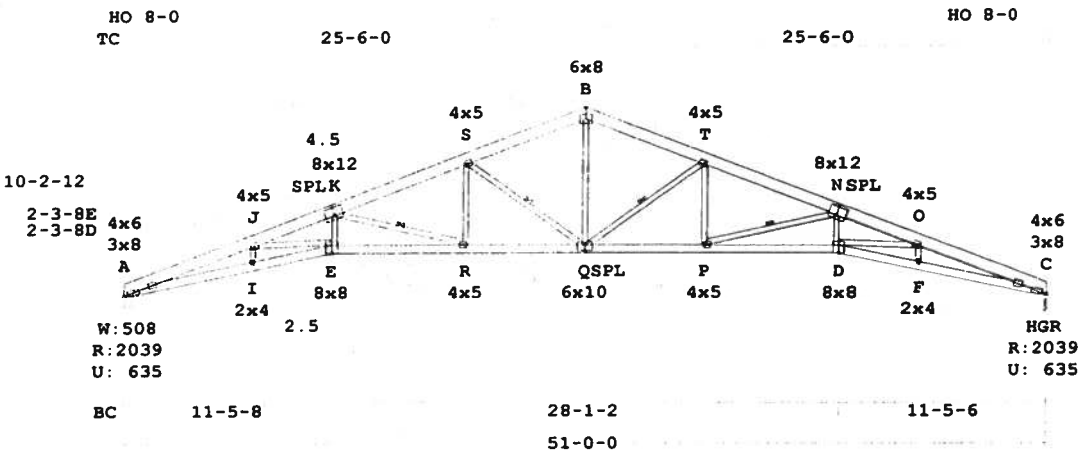
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.96
D	LOCK	8.0x 8.0	Ctr	-1.5	0.81
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8714 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R156*	5	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 465.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2040	636	5- 8	1-11
			Hz =	-178
C	2040	635	3- 8	1-11
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.28	8548	C	0.08	0.20
J -K	0.31	8715	C	0.09	0.22
K -S	0.18	5251	C	0.03	0.15
S -B	0.09	3759	C	0.04	0.05
B -T	0.09	3759	C	0.04	0.05
T -N	0.18	5250	C	0.03	0.15
N -O	0.30	8715	C	0.09	0.21
O -C	0.28	8562	C	0.08	0.20
Bottom Chords					
A -I	0.55	8076	T	0.37	0.18
I -E	0.55	8112	T	0.37	0.18
E -R	0.40	8116	T	0.37	0.03
R -Q	0.27	4953	T	0.22	0.05
Q -P	0.27	4953	T	0.22	0.05
P -D	0.40	8116	T	0.37	0.03
D -F	0.55	8125	T	0.37	0.18
F -C	0.55	8090	T	0.37	0.18
Webs					
I -J	0.03	134	T		
J -E	0.08	257	T		
E -K	0.58	1828	T		
K -R	0.84	3299	C		1 Br
R -S	0.34	1061	T		
S -Q	0.50	1803	C		1 Br
Q -B	0.74	2311	T		
Q -T	0.50	1802	C		1 Br
P -T	0.34	1060	T		
P -N	0.84	3300	C		1 Br
D -N	0.58	1830	T		
D -O	0.07	244	T		
F -O	0.03	132	T		

LL Defl -0.69" in Q -P L/870
TL Defl -1.40" in Q -P L/431
Shear // Grain in A -J 0.20

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

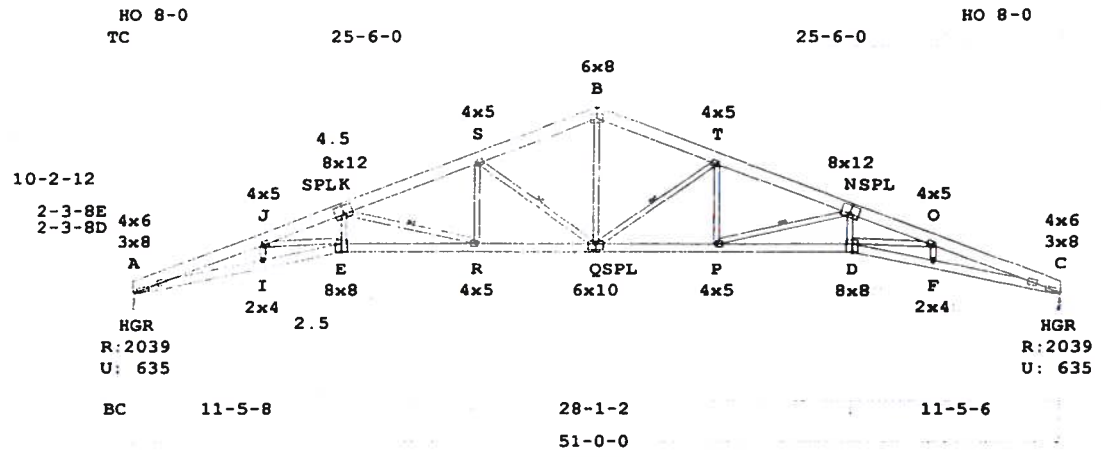
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.96
D	LOCK	8.0x 8.0	Ctr	-1.5	0.81
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8715 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R157*	9	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 465.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	2040	636	3- 8	1-11
			Hz =	-178
C	2040	635	3- 8	1-11
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -J	0.28	8548	C	0.08	0.20
J -K	0.31	8715	C	0.09	0.22
K -S	0.18	5251	C	0.03	0.15
S -B	0.09	3759	C	0.04	0.05
B -T	0.09	3759	C	0.04	0.05
T -N	0.18	5250	C	0.03	0.15
N -O	0.30	8715	C	0.09	0.21
O -C	0.28	8562	C	0.08	0.20
--------Bottom Chords---------					
A -I	0.55	8076	T	0.37	0.18
I -E	0.55	8112	T	0.37	0.18
E -R	0.40	8116	T	0.37	0.03
R -Q	0.27	4953	T	0.22	0.05
Q -P	0.27	4953	T	0.22	0.05
P -D	0.40	8116	T	0.37	0.03
D -F	0.55	8125	T	0.37	0.18
F -C	0.55	8090	T	0.37	0.18
------------Webs------------					
I -J	0.03	134	T		
J -E	0.08	257	T		
E -K	0.58	1828	T		
K -R	0.84	3299	C		1 Br
R -S	0.34	1061	T		
S -Q	0.50	1803	C		1 Br
Q -B	0.74	2311	T		
Q -T	0.50	1802	C		1 Br
P -T	0.34	1060	T		
P -N	0.84	3300	C		1 Br
D -N	0.58	1830	T		
D -O	0.07	244	T		
F -O	0.03	132	T		

LL Defl -0.69" in Q -P L/872
TL Defl -1.40" in Q -P L/433
Shear // Grain in A -J 0.20

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

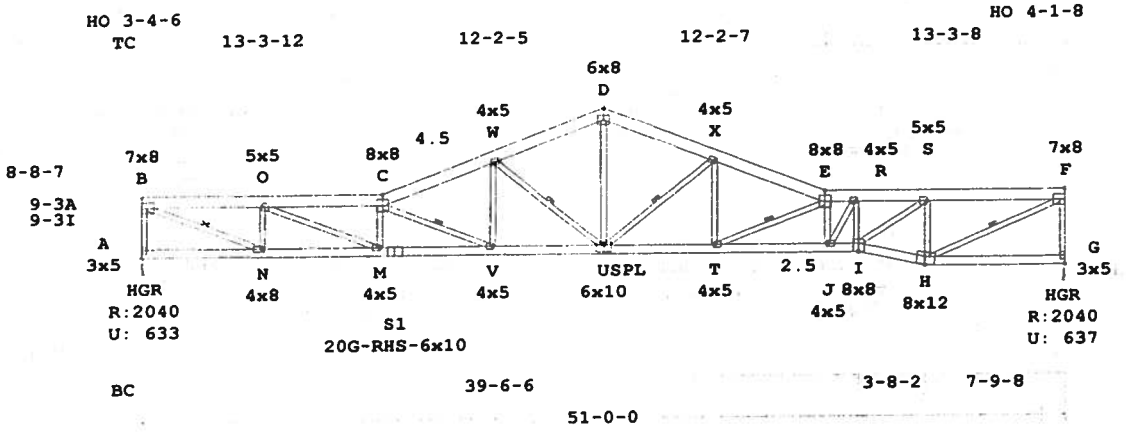
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr-1.5		0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x10.0	Ctr-1.2		0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.96
D	LOCK	8.0x 8.0	Ctr-1.5		0.81
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8715 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R159*	1	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 514.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 8	SP-2400
BC	0.50	2x 6	SP-2400
EX I -H		2x 6	SP-#2
EX H -G		2x 6	SP-#2
WB	0.98	2x 4	SP-#3
EX B -N		2x 4	SP-#2
EX O -M		2x 4	SP-#2
EX H -F		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on B -N
WB 1 rows CLB on C -V
WB 1 rows CLB on W -U
WB 1 rows CLB on U -X
WB 1 rows CLB on T -E
WB 1 rows CLB on H -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2040	634	3- 8	1-11
			Hz =	-159
G	2040	637	3- 8	2- 7
			Hz =	91

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
B -O	0.09	4084	C	0.01	0.08
O -C	0.15	6973	C	0.05	0.10
C -W	0.16	5124	C	0.03	0.13
W -D	0.10	3752	C	0.01	0.09
D -X	0.09	3751	C	0.04	0.05
X -E	0.15	5130	C	0.03	0.12
E -R	0.16	6939	C	0.05	0.11
R -S	0.15	6284	C	0.04	0.11
S -F	0.12	3666	C	0.01	0.11
--------Bottom Chords---------					
A -N	0.06	118	T	0.00	0.06
N -M	0.30	4084	T	0.18	0.12
M -S1	0.44	7022	T	0.32	0.12
S1-V	0.44	7022	T	0.32	0.12
V -U	0.27	4843	T	0.22	0.05
U -T	0.28	4845	T	0.22	0.06
T -J	0.50	7002	T	0.32	0.18
J -I	0.46	6310	T	0.28	0.18
I -H	0.92	3773	T	0.45	0.47
H -G	0.45	90	T	0.00	0.45
------------Webs------------					
A -B	0.35	1983	C	WindLd	
B -N	0.82	4454	T		1 Br
N -O	0.28	1584	C		
O -M	0.85	3151	T		
M -C	0.22	1244	C		
C -V	0.52	2353	C		1 Br
V -W	0.34	1085	T		
W -U	0.46	1755	C		1 Br
U -D	0.75	2345	T		
U -X	0.46	1756	C		1 Br
T -X	0.35	1098	T		
T -E	0.52	2328	C		1 Br
J -E	0.28	1585	C		
J -R	0.42	1314	T		
I -R	0.12	687	C		
I -S	0.98	3055	T		
H -S	0.46	2144	C		
H -F	0.74	4025	T		1 Br
G -F	0.40	1896	C		

LL Defl -0.56" in T -J L/999
TL Defl -1.13" in T -J L/535
Shear // Grain in E -R 0.33

Hz Disp	LL	DL	TL
Jt G	0.11"	0.12"	0.23"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

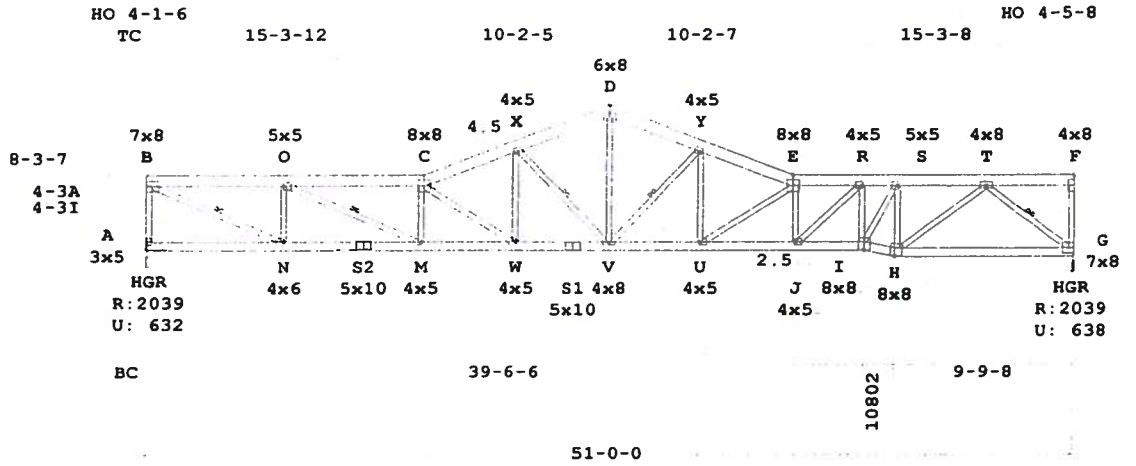
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
O	LOCK	5.0x 5.0	Ctr	Ctr	0.94
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
W	LOCK	4.0x 5.0	-0.5	0.2	0.56
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
X	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	Ctr	0.5	0.76
S	LOCK	5.0x 5.0	-1.0	Ctr	1.00
F	LOCK	7.0x 8.0	-0.5	Ctr	0.90
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
N	LOCK	4.0x 8.0	-0.5	Ctr	0.87
M	LOCK	4.0x 5.0	-0.5	Ctr	0.90
S1	RHS	6.0x10.0	Ctr	0.2	0.75
V	LOCK	4.0x 5.0	Ctr	Ctr	0.66
U	LOCK	6.0x10.0	Ctr	-1.2	0.97
T	LOCK	4.0x 5.0	Ctr	Ctr	0.66
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	8.0x 8.0	Ctr	-1.5	0.97
H	LOCK	8.0x12.0	0.5	4.6	0.92
G	LOCK	3.0x 5.0	Ctr	Ctr	0.90

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6973 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R160*	1	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 532.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.12	2x 8	SP-2400
BC	0.90	2x 6	SP-#2
WB	0.97	2x 4	SP-#3
EX B -N		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

WB 1 rows CLB on B -N
WB 1 rows CLB on O -M
WB 1 rows CLB on X -V
WB 1 rows CLB on V -Y
WB 1 rows CLB on T -G
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	633	3- 8	2- 7
		Hz =	-168	
G	2040	638	3- 8	2- 7
		Hz =	79	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
B -O	0.10	3697 C	0.01		0.09
O -C	0.11	6034 C	0.04		0.07
C -X	0.11	4821 C	0.02		0.09
X -D	0.07	3746 C	0.04		0.03
D -Y	0.07	3746 C	0.04		0.03
Y -E	0.11	4825 C	0.02		0.09
E -R	0.12	6020 C	0.04		0.08
R -S	0.09	4944 C	0.02		0.07
S -T	0.07	3971 C	0.01		0.06
T -F	0.06	0 T	0.00		0.06
Bottom Chords					
A -N	0.17	115 T	0.00		0.17
N -S2	0.58	3697 T	0.44		0.14
S2-M	0.59	3697 T	0.44		0.15
M -W	0.85	6065 T	0.73		0.12
W -S1	0.59	4549 T	0.55		0.04
S1-V	0.60	4549 T	0.55		0.05
V -U	0.60	4552 T	0.55		0.05
U -J	0.90	6052 T	0.73		0.17
J -I	0.77	4972 T	0.60		0.17
I -H	0.80	4037 T	0.49		0.31
H -G	0.55	2166 T	0.26		0.29
Webs					
A -B	0.42	1975 C	WindLd		
B -N	0.75	4097 T		1 Br	
N -O	0.33	1569 C			
O -M	0.83	2589 T		1 Br	
M -C	0.22	1049 C			
C -W	0.97	1791 C			
W -X	0.34	1087 T			
X -V	0.37	1543 C		1 Br	
V -D	0.75	2366 T			
V -Y	0.37	1545 C		1 Br	
U -Y	0.35	1093 T			
U -E	0.96	1772 C			
J -E	0.23	1098 C			
J -R	0.47	1490 T			
I -R	0.21	1012 C			
I -S	0.60	1877 T			
H -S	0.42	1794 C			
H -T	0.73	2284 T			
T -G	0.57	2832 C		1 Br	
G -F	0.02	99 C			

LL Defl -0.42" in M -W L/999
TL Defl -0.86" in M -W L/703
Shear // Grain in I -H 0.29

Hz Disp	LL	DL	TL
Jt G	0.10"	0.10"	0.20"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

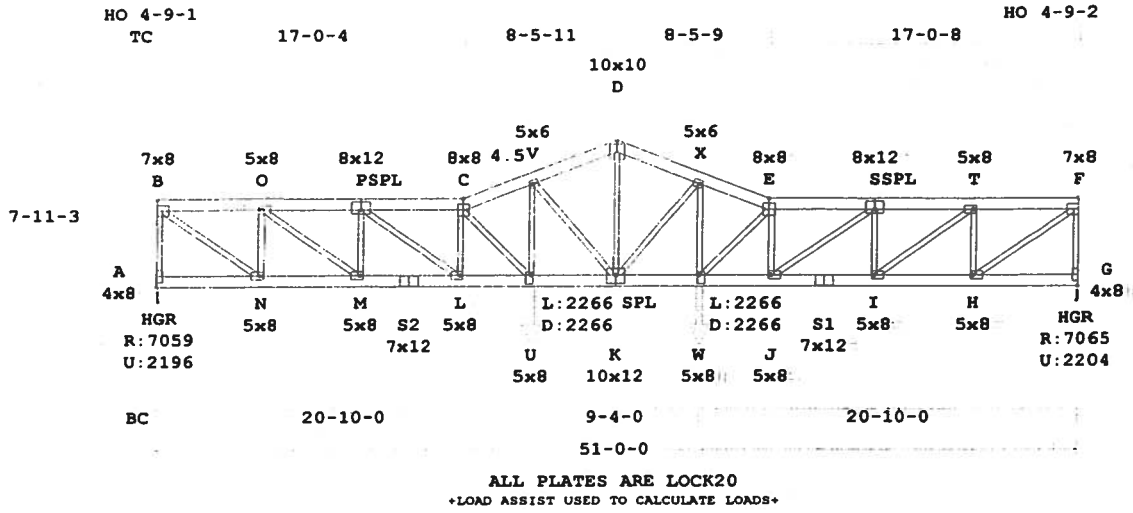
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
O	LOCK	5.0x 5.0	0.5	Ctr	1.00
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
X	LOCK	4.0x 5.0	-0.5	0.2	0.56
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
Y	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	Ctr	0.5	0.74
S	LOCK	5.0x 5.0	Ctr	Ctr	0.71
T	LOCK	4.0x 8.0	Ctr	0.5	0.87
F	LOCK	4.0x 8.0-0.2	Ctr	Ctr	0.76
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
N	LOCK	4.0x 6.0-1.0	Ctr	Ctr	0.94
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	4.0x 5.0-0.5	Ctr	Ctr	1.00
W	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
V	LOCK	4.0x 8.0	Ctr	Ctr	0.79
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	8.0x 8.0	Ctr	-1.5	0.73
H	LOCK	8.0x 8.0	1.0	4.4	1.00
G	LOCK	7.0x 8.0	-0.5	Ctr	0.75

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6034 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R161*	1*2P	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 592.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.35	2x 8	SP-2400
BC	0.68	2x 8	SP-2400
WB	0.90	2x 4	SP-#3
EX B -N		2x 4	SP-2400
EX O -M		2x 4	SP-#2
EX P -L		2x 4	SP-#2
EX U -V		2x 4	SP-#2
EX K -D		2x 4	SP-2400
EX W -X		2x 4	SP-#2
EX J -S		2x 4	SP-#2
EX I -T		2x 4	SP-#2
EX H -F		2x 4	SP-2400

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
TC V	50	50	20.7'	30.5'
BC V	2266	2266	20.8'	CL-LB
BC V	2266	2266	30.2'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	7060	2196	3- 8	2-15
			Hz =	-173
G	7065	2205	3- 8	2-15
			Hz =	75

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -O	0.06	9246	C 0.05 0.01
O -P	0.17	17561	C 0.10 0.07
P -C	0.33	25594	C 0.23 0.10
C -V	0.35	24468	C 0.21 0.14
V -D	0.17	19234	C 0.12 0.05
D -X	0.17	19232	C 0.12 0.05
X -E	0.35	24493	C 0.21 0.14
E -S	0.33	25611	C 0.23 0.10
S -T	0.17	17573	C 0.10 0.07
T -F	0.06	9252	C 0.05 0.01
--------Bottom Chords--------			
A -N	0.02	111	T 0.00 0.02
N -M	0.19	9245	T 0.17 0.02
M -S2	0.43	17560	T 0.33 0.10
S2-L	0.52	17560	T 0.33 0.19
L -U	0.68	25742	T 0.49 0.19
U -K	0.55	22989	T 0.43 0.12
K -W	0.55	23016	T 0.43 0.12
W -J	0.68	25759	T 0.49 0.19
J -S1	0.52	17572	T 0.33 0.19
S1-I	0.43	17572	T 0.33 0.10
I -H	0.19	9251	T 0.17 0.02
H -G	0.02	74	T 0.00 0.02
------------Webs------------			
A -B	0.58	6981	C WindLd
B -N	0.45	11490	T
N -O	0.56	6745	C
O -M	0.96	10431	T
M -P	0.50	5953	C
P -L	0.92	9983	T
L -C	0.52	6182	C
C -U	0.36	3865	C
U -V	0.65	7122	T
V -K	0.89	7713	C
K -D	0.49	12608	T
K -X	0.90	7746	C
W -X	0.65	7128	T
W -E	0.36	3862	C
J -E	0.52	6184	C
J -S	0.92	9989	T
I -S	0.50	5958	C
I -T	0.96	10439	T
H -T	0.56	6750	C
H -F	0.45	11499	T
G -F	0.58	6986	C

LL Defl -0.71" in K -W L/850
TL Defl -1.43" in K -W L/424
Shear // Grain in P -C 0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
O	LOCK	5.0x 8.0	0.5	Ctr	0.96
P	LOCK	8.0x12.0	Ctr	1.5	0.99
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
V	LOCK	5.0x 6.0-0.5-0.2			0.99
D	LOCK	10.0x10.0	Ctr	Ctr	0.90
X	LOCK	5.0x 6.0	0.5-0.2		0.99
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
S	LOCK	8.0x12.0	Ctr	1.5	0.99
T	LOCK	5.0x 8.0-0.5	Ctr		0.96
F	LOCK	7.0x 8.0-0.5	Ctr		0.90
A	LOCK	4.0x 8.0	0.2	Ctr	0.76
N	LOCK	5.0x 8.0-0.5	Ctr		0.98
M	LOCK	5.0x 8.0-0.5	Ctr		0.96
S2	LOCK	7.0x12.0	Ctr	Ctr	0.94
L	LOCK	5.0x 8.0-0.5	Ctr		0.92
U	LOCK	5.0x 8.0	0.5-1.9		0.96
K	LOCK	10.0x12.0	Ctr-0.5		0.99
W	LOCK	5.0x 8.0-0.5-1.9			0.96
J	LOCK	5.0x 8.0	0.5	Ctr	0.92
S1	LOCK	7.0x12.0	Ctr	Ctr	0.94
I	LOCK	5.0x 8.0	0.5	Ctr	0.96
H	LOCK	5.0x 8.0	0.5	Ctr	0.98
G	LOCK	4.0x 8.0	0.2	Ctr	0.76

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	4	4	

Plus clusters of nails where shown.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 25611 Lbs

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R161*	1*2P	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish

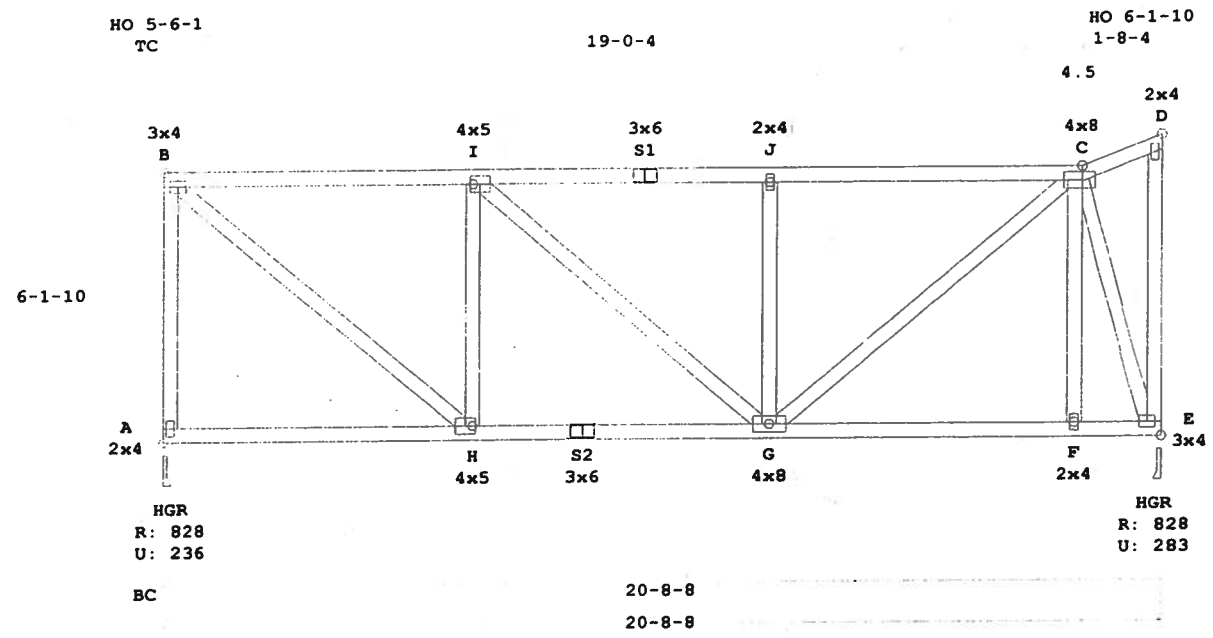
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R162*	1	SP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 179.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.42	2x 4	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.35	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	237	3- 8	1- 0
			Hz =	166
E	828	284	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -I	0.42	741 C	0.00	0.42
I -S1	0.42	808 C	0.00	0.42
S1-J	0.33	808 C	0.00	0.33
J -C	0.33	808 C	0.00	0.33
C -D	0.06	27 C	0.00	0.06
--------Bottom Chords---------				
A -H	0.24	155 C	0.00	0.24
H -S2	0.31	741 T	0.07	0.24
S2-G	0.26	741 T	0.07	0.19
G -F	0.21	259 T	0.02	0.19
F -E	0.17	259 T	0.02	0.15
------------Webs------------				
A -B	0.31	774 C	WindLd	
B -H	0.30	962 T		
H -I	0.19	475 C		
I -G	0.03	87 T		
G -J	0.15	370 C		
G -C	0.28	701 T		
F -C	0.05	215 T		
C -E	0.35	808 C		
E -D	0.03	72 T		

LL Defl -0.05" in A -H L/999
TL Defl -0.10" in A -H L/999
Shear // Grain in B -I 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

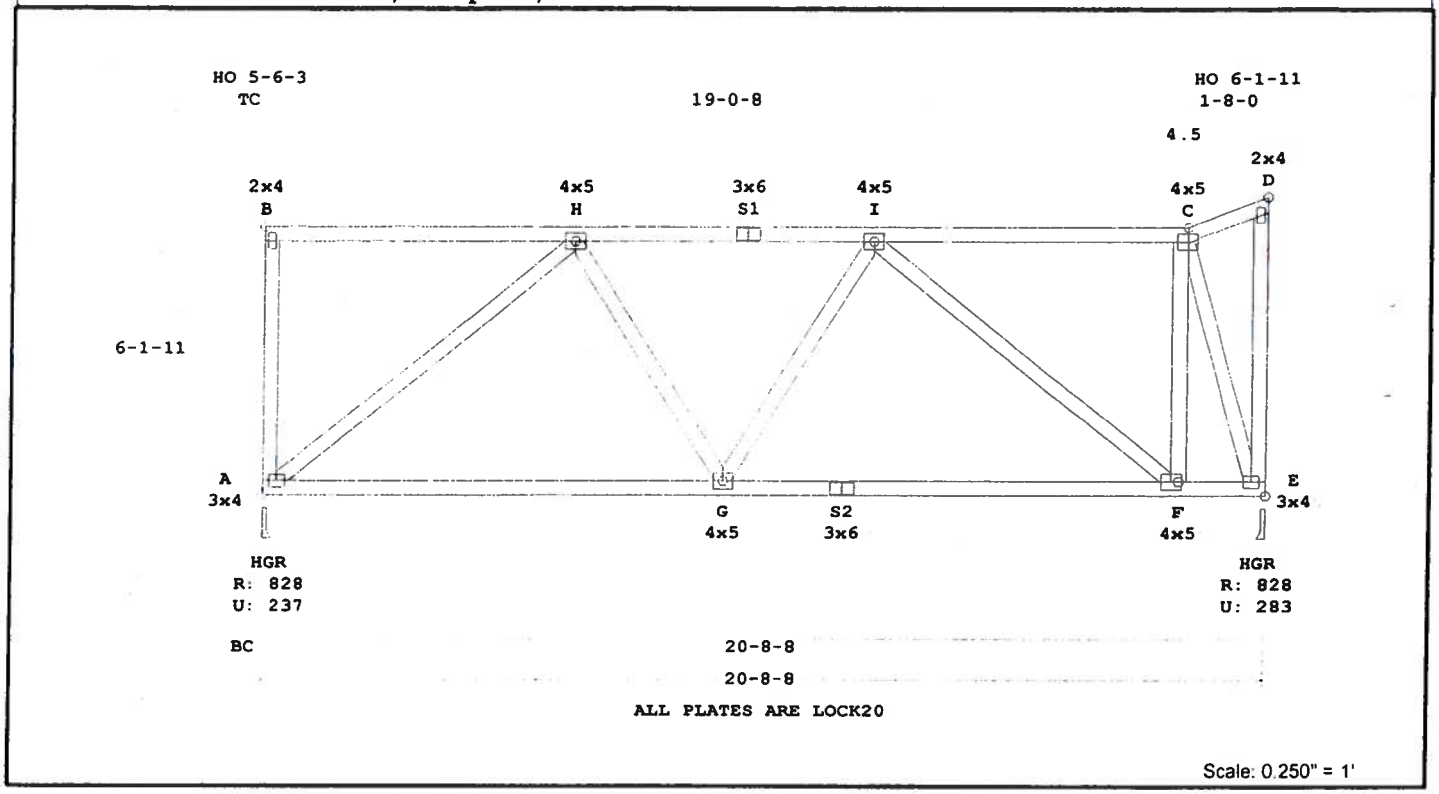
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	2.0x 4.0	Ctr	Ctr	0.40
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
G	LOCK	4.0x 8.0	Ctr	Ctr	0.50
F	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 808 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R163*	1	SP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 169.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.42	2x 4	SP-#2
BC	0.67	2x 4	SP-#2
WB	0.90	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	237	3- 8	1- 0
			Hz =	165
E	828	284	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -H	0.42	106 T	0.00 0.42
H -S1	0.42	817 C	0.00 0.42
S1-I	0.36	817 C	0.00 0.36
I -C	0.36	276 C	0.00 0.36
C -D	0.07	18 C	0.00 0.07
--------Bottom Chords---------			
A -G	0.67	715 T	0.07 0.60
G -S2	0.67	792 T	0.07 0.60
S2-F	0.38	792 T	0.07 0.31
F -E	0.34	266 T	0.03 0.31
------------Webs------------			
A -B	0.26	162 C	WindLd
A -H	0.90	926 C	
H -G	0.08	319 T	
G -I	0.04	184 T	
I -F	0.64	668 C	
F -C	0.18	605 T	
C -E	0.38	885 C	
E -D	0.01	46 T	

LL Defl -0.20" in A -G L/999
TL Defl -0.41" in A -G L/585
Shear // Grain in B -H 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

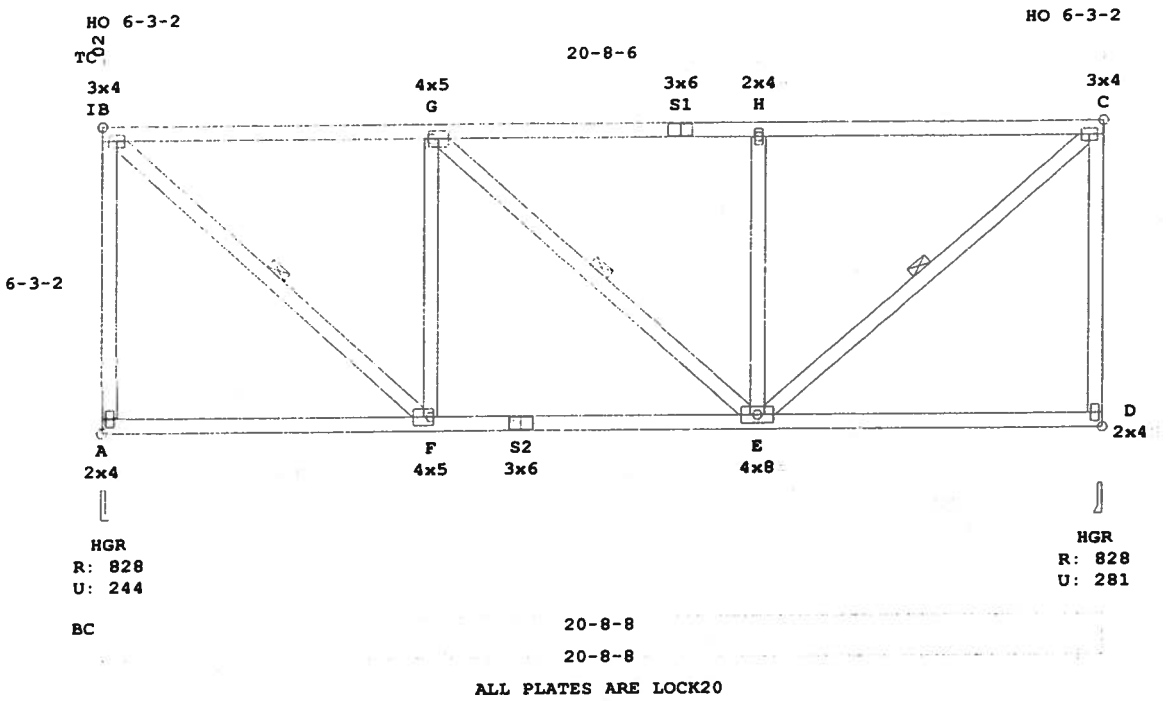
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.53
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.53
C	LOCK	4.0x 5.0	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.53
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 926 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R164	1	SP	200808	0	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.40	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on B -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	244	3- 8	1- 0
			Hz =	-174
D	828	281	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -G	0.47	672 C	0.00	0.47
G -S1	0.47	689 C	0.00	0.47
S1-H	0.51	689 C	0.00	0.51
H -C	0.51	689 C	0.00	0.51
--------Bottom Chords---------				
A -F	0.27	122 C	0.00	0.27
F -S2	0.34	672 T	0.07	0.27
S2-E	0.36	672 T	0.07	0.29
E -D	0.29	123 T	0.00	0.29
------------Webs------------				
A -B	0.40	771 C	WindLd	
B -F	0.28	902 T		1 Br
F -G	0.23	455 C		
G -E	0.01	47 T		1 Br
E -H	0.23	455 C		
E -C	0.29	908 T		1 Br
D -C	0.40	768 C		

LL Defl -0.07" in E -D L/999
TL Defl -0.15" in E -D L/999
Shear // Grain in H -C 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

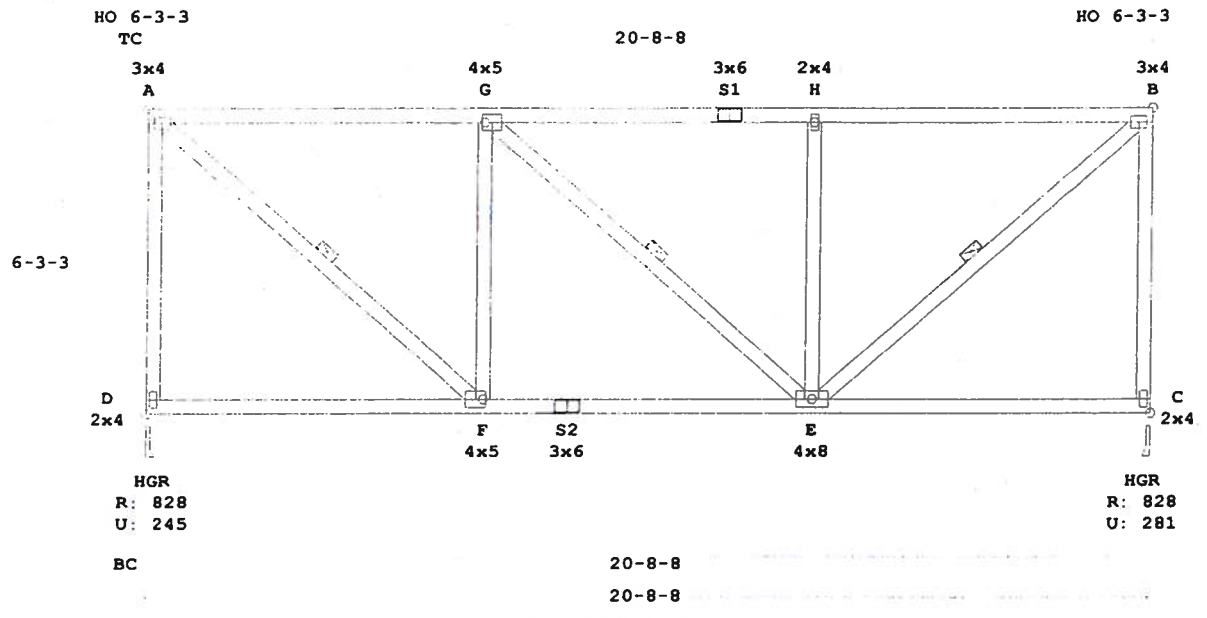
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 771 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R165*	1	FLAT	200808	60303	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.40	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	245	3- 8	1- 0
			Hz =	-175
C	828	281	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -G	0.49	680	C	0.00	0.49
G -S1	0.49	679	C	0.00	0.49
S1-H	0.49	679	C	0.00	0.49
H -B	0.49	679	C	0.00	0.49
--------Bottom Chords---------					
D -F	0.28	122	C	0.00	0.28
F -S2	0.35	680	T	0.07	0.28
S2-E	0.35	680	T	0.07	0.28
E -C	0.28	123	T	0.00	0.28
------------Webs------------					
D -A	0.40	770	C	WindLd	
A -F	0.29	905	T		1 Br
F -G	0.23	448	C		
G -E	0.01	54	T		1 Br
E -H	0.23	447	C		
E -B	0.29	904	T		1 Br
C -B	0.40	769	C		

LL Defl -0.07" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

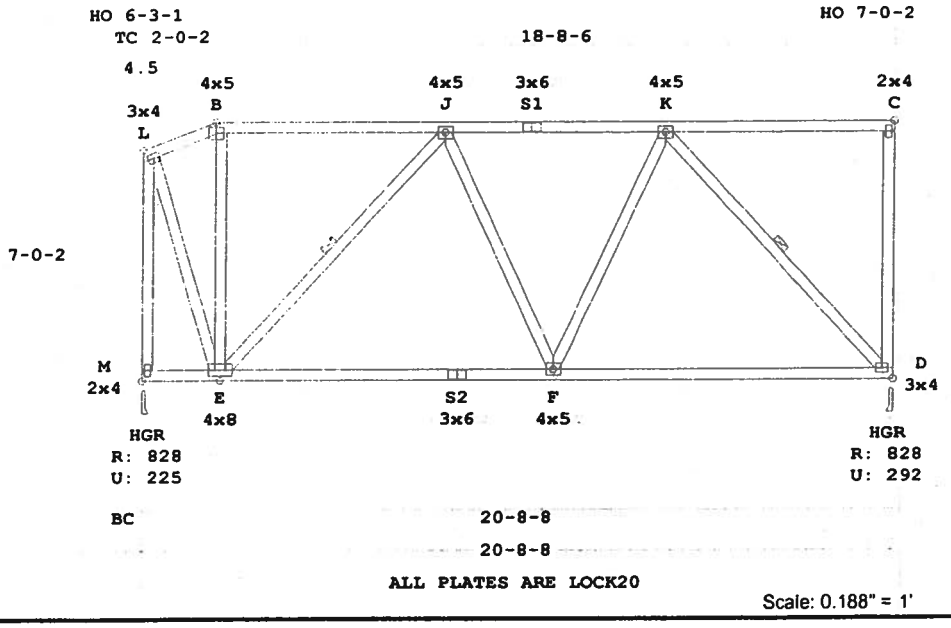
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R166*	1	HHIP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 185.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.40	2x 4	SP-#2
BC	0.64	2x 4	SP-#2
WB	0.45	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8
WB	1 rows	CLB on E -J	
WB	1 rows	CLB on K -D	

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	828	225	3- 8	1- 0
			Hz =	194
D	828	293	3- 8	1- 0
			Hz =	143

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
L -B	0.11	249 C	0.00 0.11
B -J	0.36	244 C	0.00 0.36
J -S1	0.36	633 C	0.00 0.36
S1-K	0.40	633 C	0.00 0.40
K -C	0.40	0 T	0.00 0.40
Bottom Chords			
M -E	0.29	184 C	0.00 0.29
E -S2	0.35	621 T	0.06 0.29
S2-F	0.64	621 T	0.06 0.58
F -D	0.63	549 T	0.05 0.58
Webs			
M -L	0.45	849 C	WindLd
L -E	0.24	771 T	
E -B	0.11	178 C	
E -J	0.18	555 C	1 Br
J -F	0.04	158 T	
F -K	0.08	310 T	
K -D	0.27	809 C	1 Br
D -C	0.10	159 C	

LL Defl -0.19" in F -D L/999
TL Defl -0.38" in F -D L/637
Shear // Grain in K -C 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

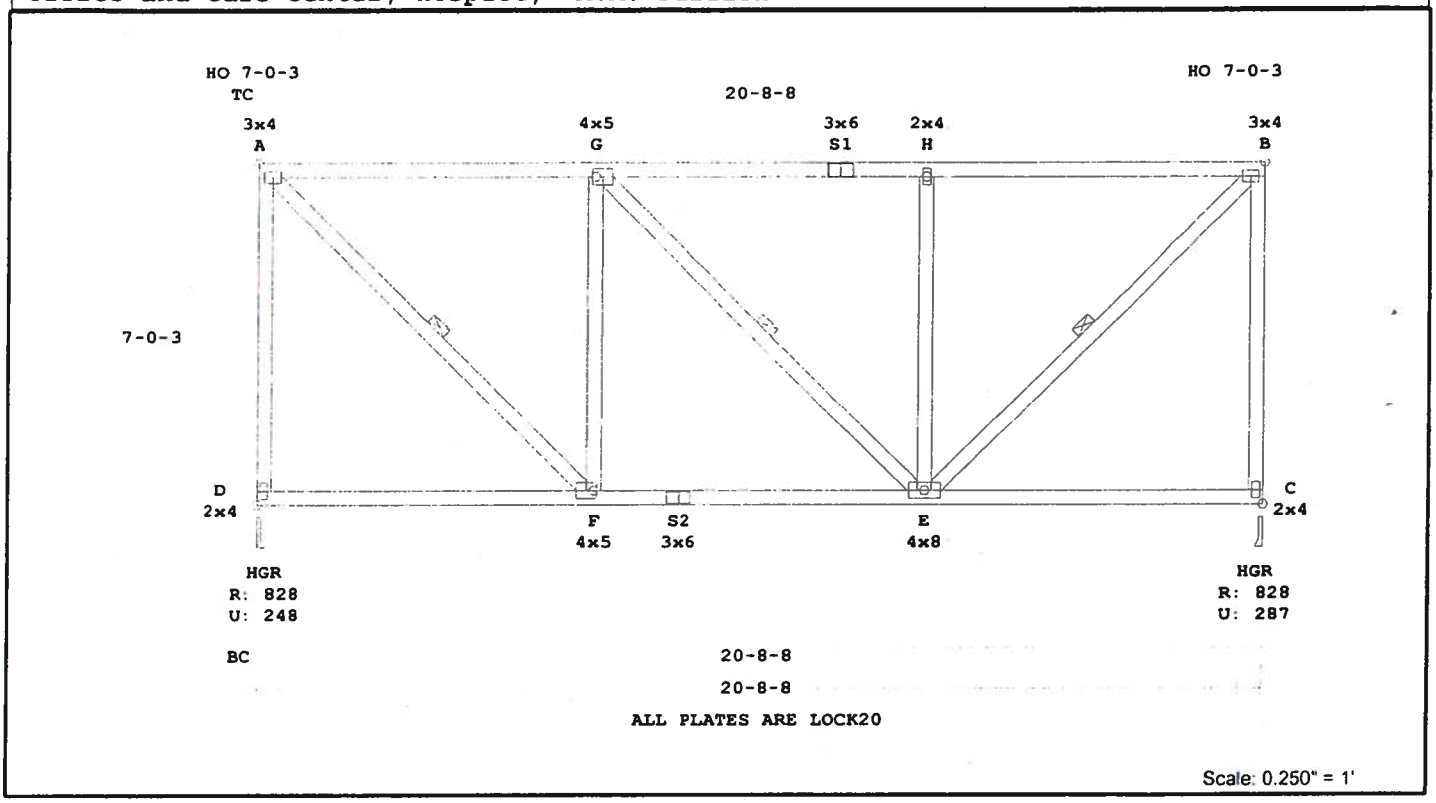
Jt	Type	Plt Size X	Y	JSI
L	LOCK	3.0x 4.0 Ctr	Ctr	0.69
B	LOCK	4.0x 5.0 Ctr	Ctr	0.87
J	LOCK	4.0x 5.0 Ctr	Ctr	0.58
S1	LOCK	3.0x 6.0 Ctr	Ctr	0.88
K	LOCK	4.0x 5.0 Ctr	Ctr	0.58
C	LOCK	2.0x 4.0 Ctr	Ctr	0.40
M	LOCK	2.0x 4.0 Ctr	Ctr	0.40
E	LOCK	4.0x 8.0 Ctr	Ctr	0.65
S2	LOCK	3.0x 6.0 Ctr	Ctr	0.88
F	LOCK	4.0x 5.0 Ctr	Ctr	0.58
D	LOCK	3.0x 4.0 Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 849 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R167*	1	FLAT	200808	70003	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 178.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.50	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	249	3- 8	1- 0
			Hz =	-197
C	828	288	3- 8	1- 0
			Hz =	139

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	604 C	0.00 0.49
G -S1	0.49	604 C	0.00 0.49
S1-H	0.49	604 C	0.00 0.49
H -B	0.49	604 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	139 C	0.00 0.28
F -S2	0.34	604 T	0.06 0.28
S2-E	0.34	604 T	0.06 0.28
E -C	0.28	139 T	0.00 0.28
------------Webs------------			
D -A	0.50	770 C	WindLd
A -F	0.27	850 T	1 Br
F -G	0.29	448 C	
G -E	0.01	65 T	1 Br
E -H	0.29	447 C	
E -B	0.27	849 T	1 Br
C -B	0.50	769 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

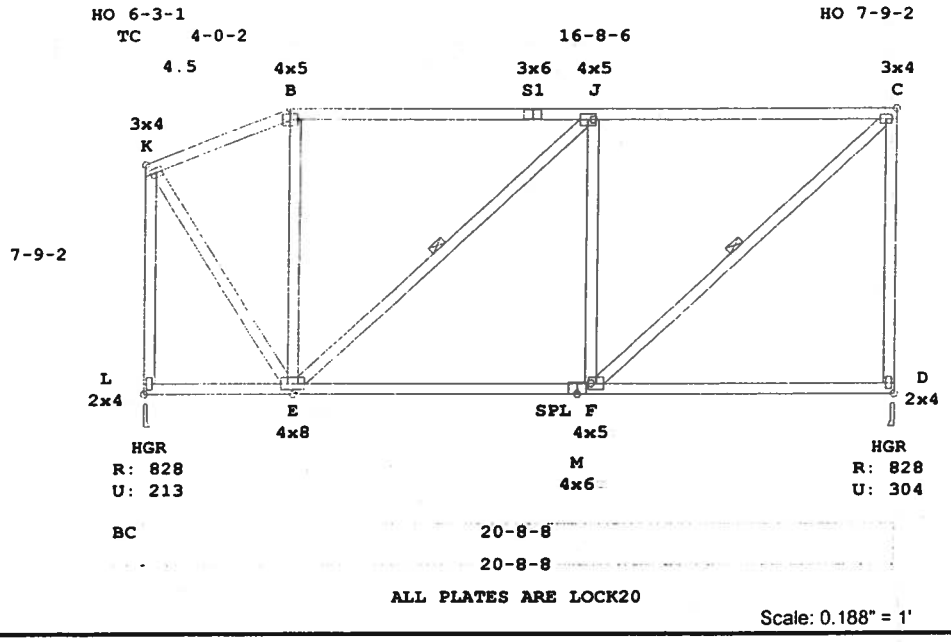
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R168*	1	HHIP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 183.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.84	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
WB	0.61	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
L	828	213	3- 8	1- 0
			Hz =	228
D	828	304	3- 8	1- 0
			Hz =	161

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
K -B	0.19	395 C	0.00 0.19
B -S1	0.42	379 C	0.00 0.42
S1-J	0.84	379 C	0.00 0.84
J -C	0.84	617 C	0.00 0.84
--------Bottom Chords---------			
L -E	0.23	244 C	0.00 0.23
E -M	0.34	617 T	0.06 0.28
M -F	0.52	617 T	0.06 0.46
F -D	0.46	161 T	0.00 0.46
------------Webs------------			
L -K	0.43	803 C	WindLd
K -E	0.21	683 T	
E -B	0.17	220 C	
E -J	0.15	322 C	1 Br
F -J	0.30	394 T	
F -C	0.26	835 T	1 Br
D -C	0.61	760 C	

LL Defl -0.12" in F -D L/999
TL Defl -0.25" in F -D L/981
Shear // Grain in J -C 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

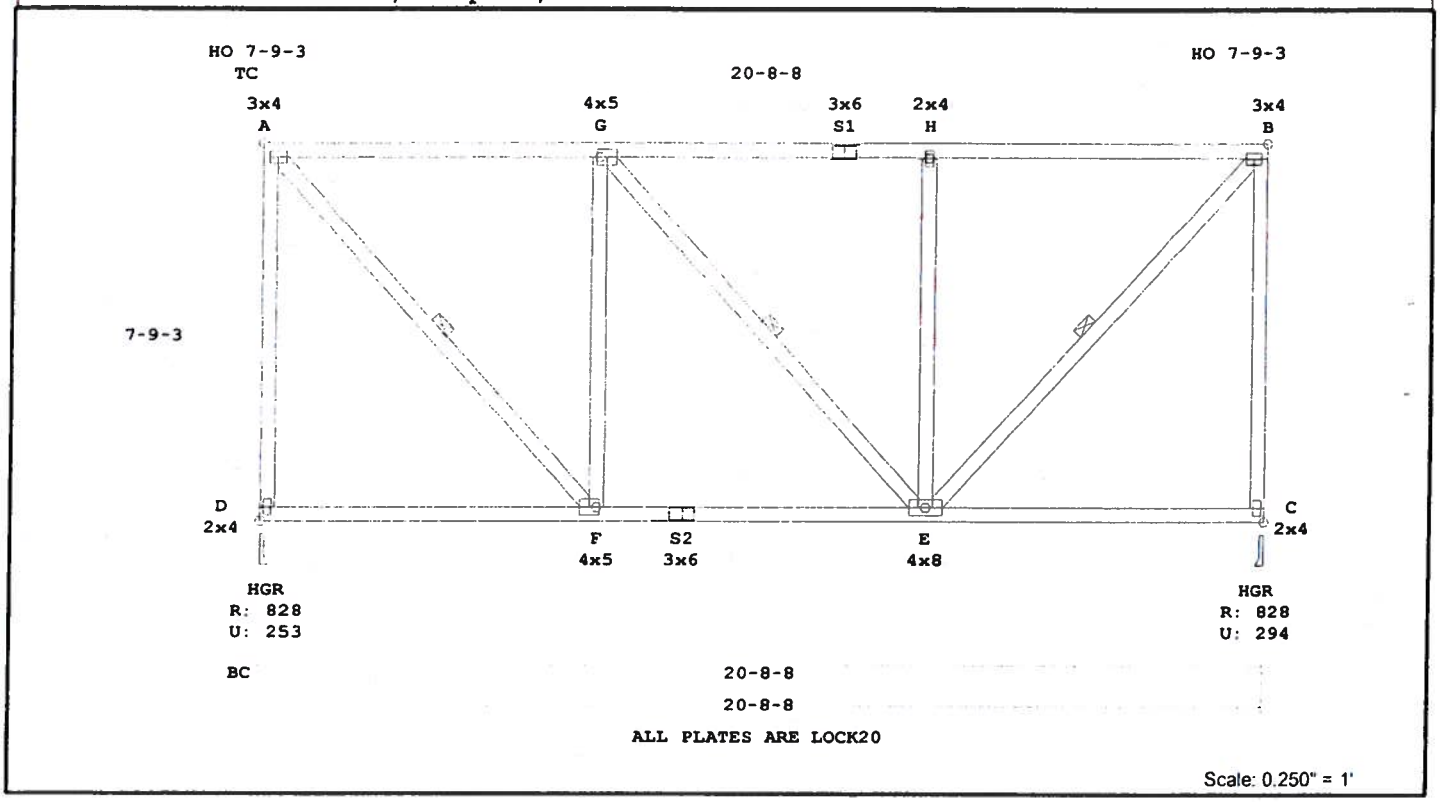
Jt	Type	Plt Size	X	Y	JSI
K	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
L	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.53
M	LOCK	4.0x 6.0	Ctr	0.2	0.91
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 803 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R169*	1	FLAT	200808	70903	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 187.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.62	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	253	3- 8	1- 0
			Hz =	-220
C	828	295	3- 8	1- 0
			Hz =	155

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -G	0.49	544 C	0.00	0.49
G -S1	0.49	543 C	0.00	0.49
S1-H	0.49	543 C	0.00	0.49
H -B	0.49	543 C	0.00	0.49
--------Bottom Chords---------				
D -F	0.28	155 C	0.00	0.28
F -S2	0.33	544 T	0.05	0.28
S2-E	0.33	544 T	0.05	0.28
E -C	0.28	155 T	0.00	0.28
------------Webs------------				
D -A	0.62	770 C	WindLd	
A -F	0.25	808 T		1 Br
F -G	0.36	448 C		
G -E	0.02	77 T		1 Br
E -H	0.36	447 C		
E -B	0.25	807 T		1 Br
C -B	0.62	770 C		

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

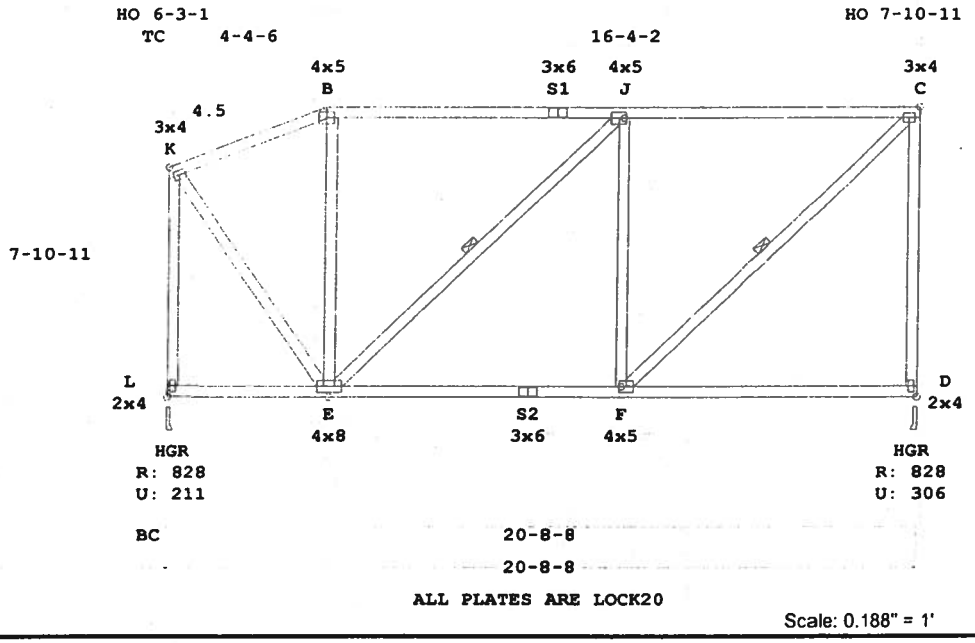
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R170*	1	HHIP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 184.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.80	2x 4	SP-#2
BC	0.50	2x 4	SP-#2
WB	0.64	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
L	828	211	3- 8	1- 0
			Hz =	234
D	828	306	3- 8	1- 0
			Hz =	164

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords					
K -B	0.22	413	C	0.03	0.19
B -S1	0.39	395	C	0.00	0.39
S1-J	0.80	395	C	0.00	0.80
J -C	0.80	599	C	0.00	0.80
Bottom Chords					
L -E	0.23	255	C	0.00	0.23
E -S2	0.29	599	T	0.06	0.23
S2-F	0.50	599	T	0.06	0.44
F -D	0.44	164	T	0.00	0.44
Webs					
L -K	0.43	798	C	WindLd	
K -E	0.21	672	T		
E -B	0.18	218	C		
E -J	0.13	281	C		1 Br
F -J	0.32	399	T		
F -C	0.26	825	T		1 Br
D -C	0.64	762	C		

LL Defl -0.11" in F -D L/999
TL Defl -0.23" in F -D L/999
Shear // Grain in J -C 0.38

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

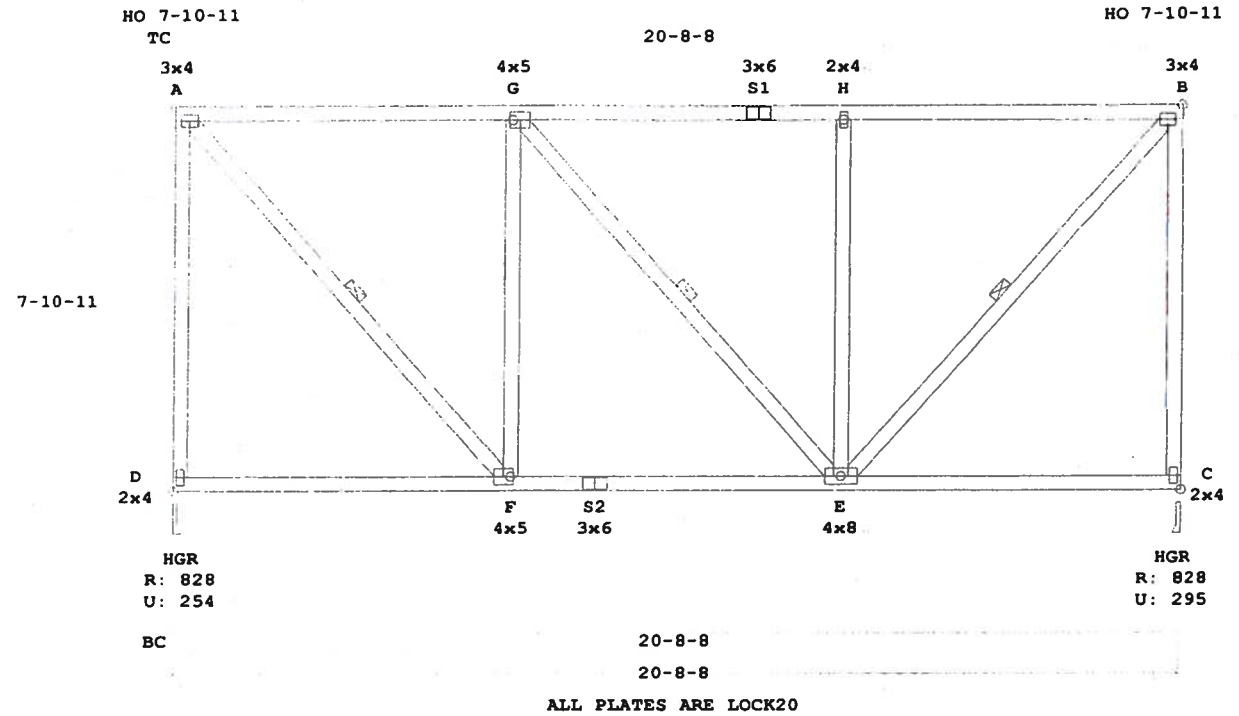
Jt	Type	Plt Size	X	Y	JSI
K	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
L	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.52
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 798 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R171	1	FLAT	200808	71011	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 188.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.64	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	254	3- 8	1- 0
			Hz =	-223
C	828	296	3- 8	1- 0
			Hz =	158

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	535 C	0.00 0.49
G -S1	0.49	534 C	0.00 0.49
S1-H	0.49	534 C	0.00 0.49
H -B	0.49	534 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	158 C	0.00 0.28
F -S2	0.33	535 T	0.05 0.28
S2-E	0.33	535 T	0.05 0.28
E -C	0.28	157 T	0.00 0.28
------------Webs------------			
D -A	0.64	770 C	WindLd
A -F	0.25	802 T	1 Br
F -G	0.37	448 C	
G -E	0.02	79 T	1 Br
E -H	0.37	447 C	
E -B	0.25	801 T	1 Br
C -B	0.64	770 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.13" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

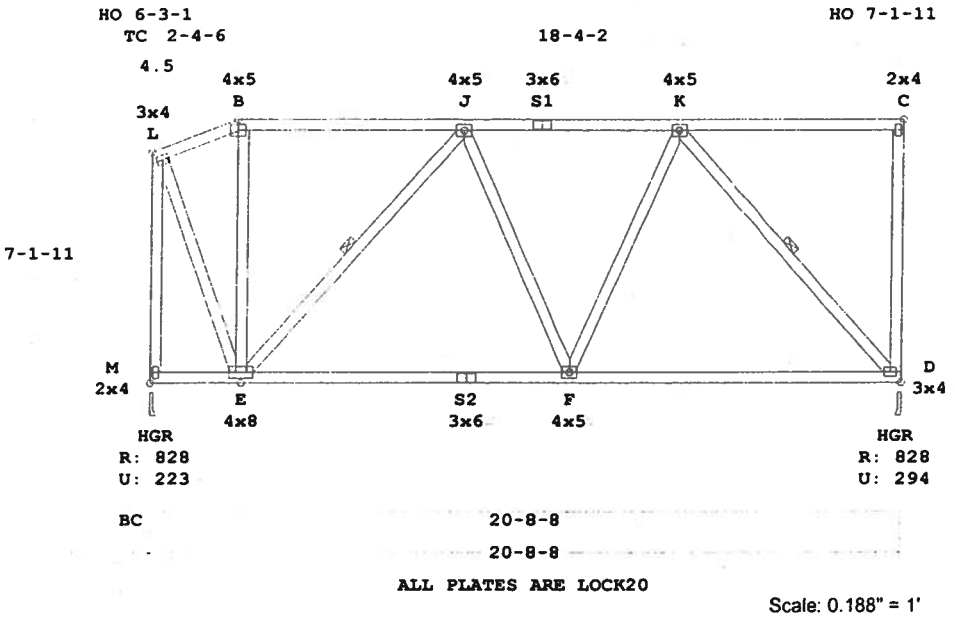
Jt	Type	Plt Size X	Y	JSI
A	LOCK	3.0x 4.0 Ctr	Ctr	0.69
G	LOCK	4.0x 5.0 Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0 Ctr	Ctr	0.88
H	LOCK	2.0x 4.0 Ctr	Ctr	0.40
B	LOCK	3.0x 4.0 Ctr	Ctr	0.69
D	LOCK	2.0x 4.0 Ctr	Ctr	0.40
F	LOCK	4.0x 5.0 Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0 Ctr	Ctr	0.88
E	LOCK	4.0x 8.0 Ctr	Ctr	0.50
C	LOCK	2.0x 4.0 Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R172*	1	HHIP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 186.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.39	2x 4	SP-#2
BC	0.62	2x 4	SP-#2
WB	0.45	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on K -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	828	223	3- 8	1- 0
			Hz =	200
D	828	294	3- 8	1- 0
			Hz =	146

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
L -B	0.10	281 C	0.00 0.10
B -J	0.34	272 C	0.00 0.34
J -S1	0.34	618 C	0.00 0.34
S1-K	0.39	618 C	0.00 0.39
K -C	0.39	0 T	0.00 0.39
Bottom Chords			
M -E	0.28	195 C	0.00 0.28
E -S2	0.34	614 T	0.06 0.28
S2-F	0.62	614 T	0.06 0.56
F -D	0.61	532 T	0.05 0.56
Webs			
M -L	0.45	837 C	WindLd
L -E	0.23	743 T	
E -B	0.11	168 C	
E -J	0.17	513 C	1 Br
J -F	0.04	164 T	
F -K	0.08	314 T	
K -D	0.27	801 C	1 Br
D -C	0.10	156 C	

LL Defl -0.17" in F -D L/999
TL Defl -0.35" in F -D L/689
Shear // Grain in K -C 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

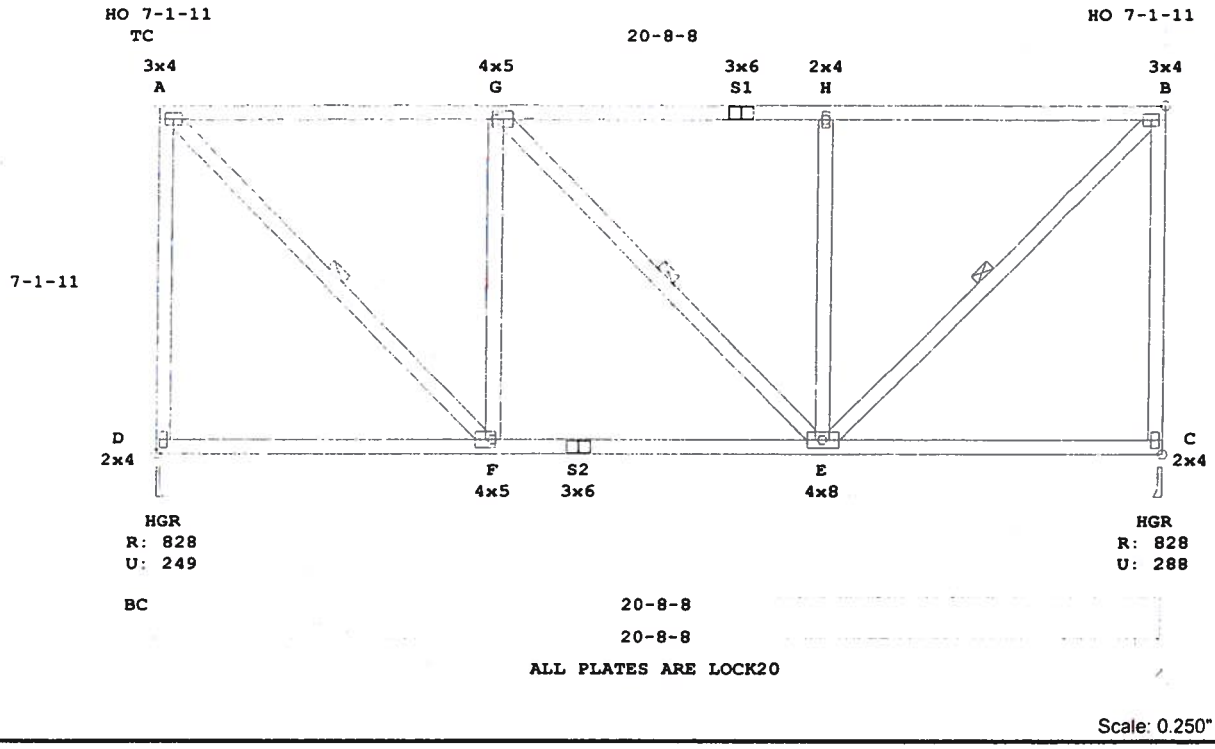
Jt	Type	Plt Size	X	Y	JSI
L	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.58
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40
M	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.62
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.58
D	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 837 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R173*	1	FLAT	200808	70111	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 180.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.52	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	250	3- 8	1- 0
			Hz =	-201
C	828	289	3- 8	1- 0
			Hz =	142

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	593 C	0.00 0.49
G -S1	0.49	593 C	0.00 0.49
S1-H	0.49	593 C	0.00 0.49
H -B	0.49	593 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	141 C	0.00 0.28
F -S2	0.34	593 T	0.06 0.28
S2-E	0.34	593 T	0.06 0.28
E -C	0.28	142 T	0.00 0.28
------------Webs------------			
D -A	0.52	770 C	WindLd
A -F	0.27	842 T	1 Br
F -G	0.30	448 C	
G -E	0.01	67 T	1 Br
E -H	0.30	447 C	
E -B	0.26	841 T	1 Br
C -B	0.52	770 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

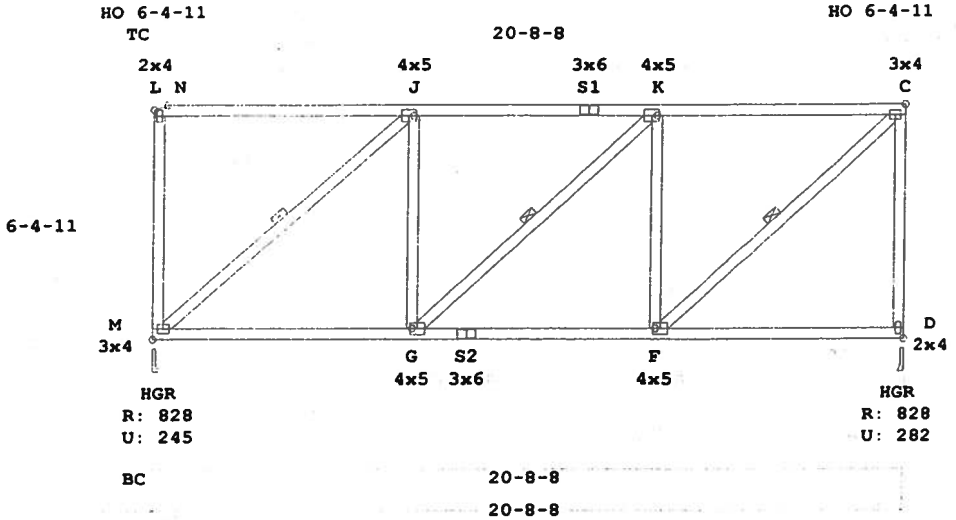
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R174*	1	HHIP	200808	0	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 171.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.42	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on M -J
WB 1 rows CLB on G -K
WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	828	246	3- 8	1- 0
			Hz =	-178
D	828	282	3- 8	1- 0
			Hz =	126

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords					
L -J	0.51	125	T	0.00	0.51
J -S1	0.51	679	C	0.00	0.51
S1-K	0.46	679	C	0.00	0.46
K -C	0.46	657	C	0.00	0.46
Bottom Chords					
M -G	0.36	679	T	0.07	0.29
G -S2	0.35	657	T	0.06	0.29
S2-F	0.33	657	T	0.06	0.27
F -D	0.27	126	T	0.00	0.27
Webs					
M -L	0.35	179	C	WindLd	
M -J	0.31	898	C		1 Br
G -J	0.07	288	T		
G -K	0.02	65	C		1 Br
F -K	0.24	455	C		
F -C	0.28	889	T		1 Br
D -C	0.42	770	C		

LL Defl -0.08" in M -G L/999
TL Defl -0.16" in M -G L/999
Shear // Grain in L -J 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

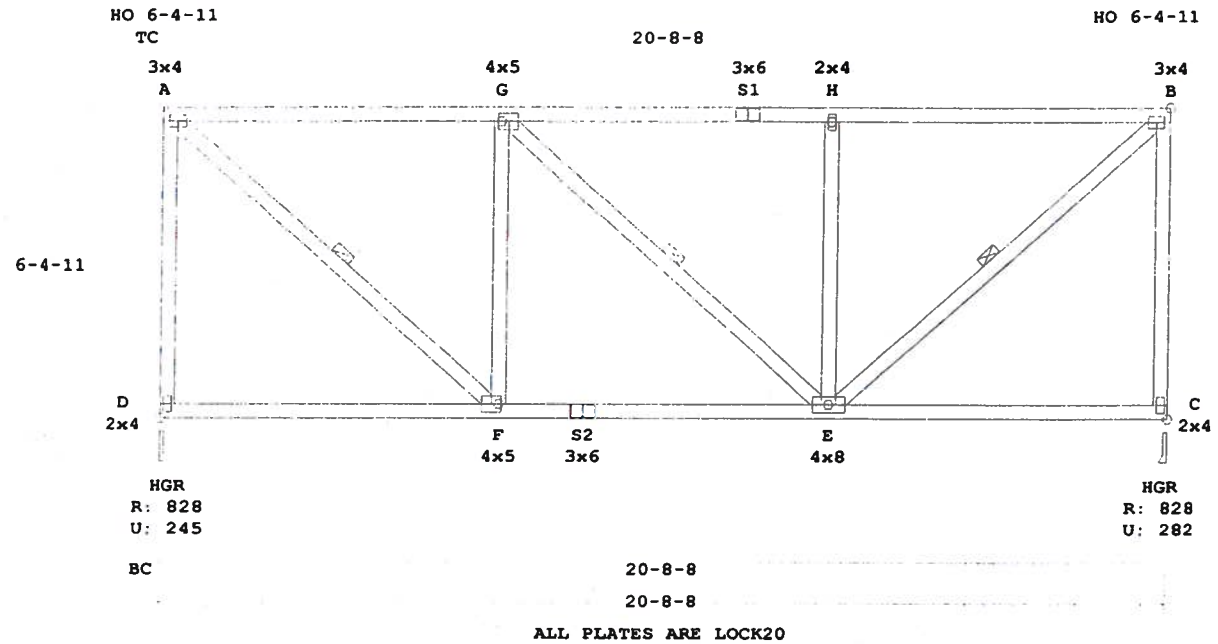
Jt	Type	Plt Size	X	Y	JSI
L	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
M	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 898 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R175*	1	FLAT	200808	60411	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 171.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.42	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	246	3- 8	1- 0
			Hz =	-178
C	828	282	3- 8	1- 0
			Hz =	126

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	666 C	0.00 0.49
G -S1	0.49	665 C	0.00 0.49
S1-H	0.49	665 C	0.00 0.49
H -B	0.49	665 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	125 C	0.00 0.28
F -S2	0.34	666 T	0.06 0.28
S2-E	0.34	666 T	0.06 0.28
E -C	0.28	126 T	0.00 0.28
------------Webs------------			
D -A	0.42	770 C	WindLd
A -F	0.28	894 T	1 Br
F -G	0.24	448 C	
G -E	0.01	56 T	1 Br
E -H	0.24	447 C	
E -B	0.28	894 T	1 Br
C -B	0.42	769 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

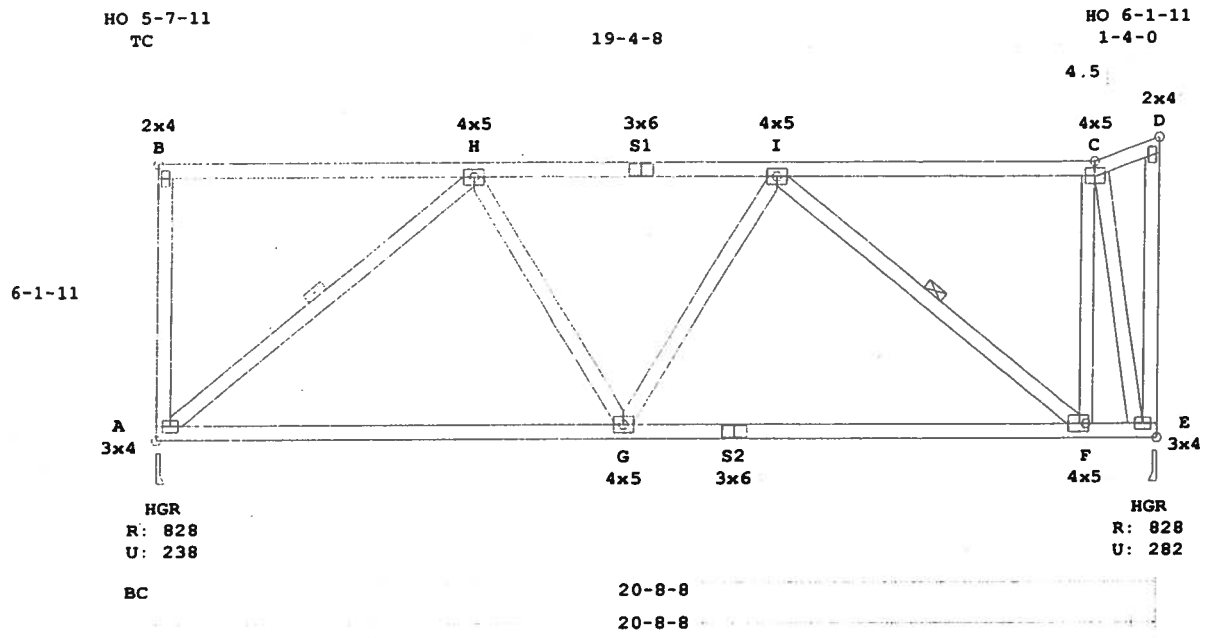
Jt	Type	Plt Size X	Y	JSI
A	LOCK	3.0x 4.0 Ctr	Ctr	0.69
G	LOCK	4.0x 5.0 Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0 Ctr	Ctr	0.88
H	LOCK	2.0x 4.0 Ctr	Ctr	0.40
B	LOCK	3.0x 4.0 Ctr	Ctr	0.69
D	LOCK	2.0x 4.0 Ctr	Ctr	0.40
F	LOCK	4.0x 5.0 Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0 Ctr	Ctr	0.88
E	LOCK	4.0x 8.0 Ctr	Ctr	0.50
C	LOCK	2.0x 4.0 Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R176*	1	SP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.69	2x 4	SP-#2
WB	0.41	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -H
WB 1 rows CLB on I -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	238	3- 8	1- 0
			Hz =	163
E	828	283	3- 8	1- 0
			Hz =	122

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -H	0.43	109 T	0.00	0.43
H -S1	0.43	801 C	0.00	0.43
S1-I	0.38	801 C	0.00	0.38
I -C	0.38	224 C	0.00	0.38
C -D	0.12	22 C	0.00	0.12
--------Bottom Chords---------				
A -G	0.68	705 T	0.06	0.62
G -S2	0.69	768 T	0.07	0.62
S2-F	0.40	768 T	0.07	0.33
F -E	0.35	207 T	0.02	0.33
------------Webs------------				
A -B	0.27	165 C	WindLd	
A -H	0.27	917 C		1 Br
H -G	0.08	312 T		
G -I	0.05	198 T		
I -F	0.21	706 C		1 Br
F -C	0.20	657 T		
C -E	0.41	933 C		
E -D	0.01	31 C		

LL Defl -0.22" in A -G L/999
TL Defl -0.44" in A -G L/546
Shear // Grain in B -H 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

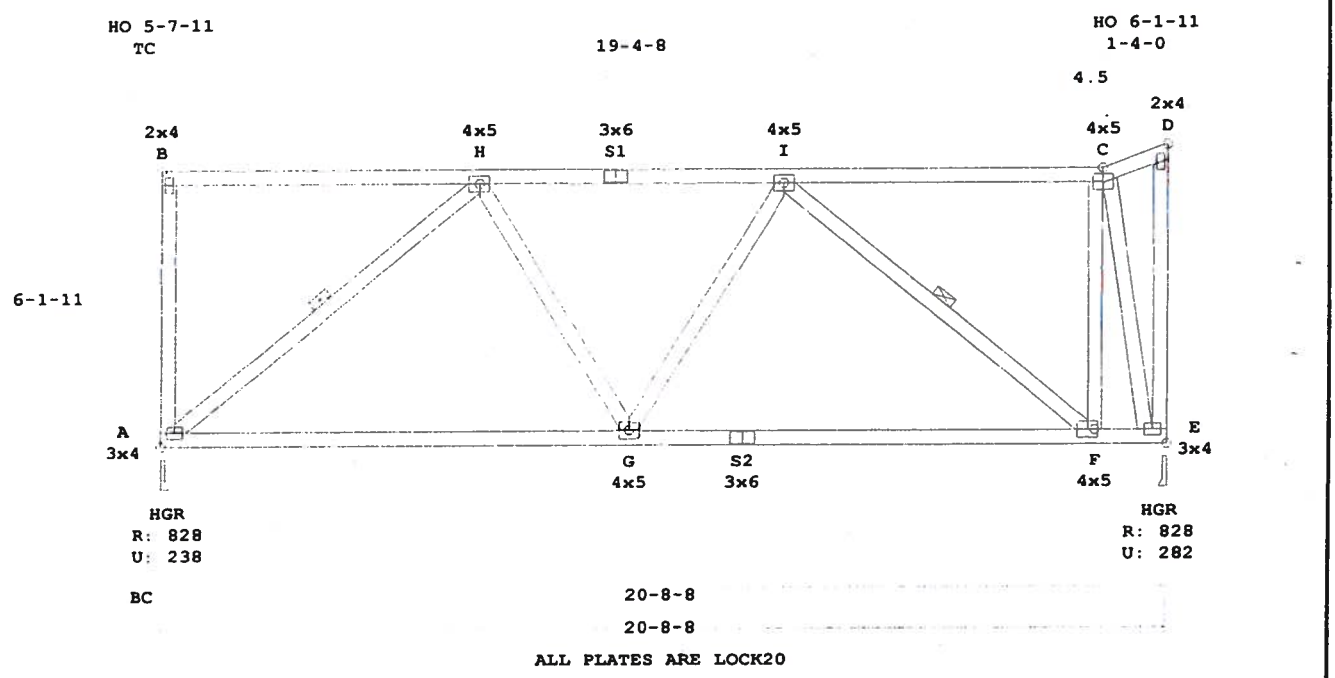
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 933 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R177*	1	SP	200808	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.69	2x 4	SP-#2
WB	0.41	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -H
WB 1 rows CLB on I -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	238	3- 8	1- 0
			Hz = 163	
E	828	283	3- 8	1- 0
			Hz = 122	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -H	0.43	109 T	0.00	0.43
H -S1	0.43	801 C	0.00	0.43
S1-I	0.38	801 C	0.00	0.38
I -C	0.38	224 C	0.00	0.38
C -D	0.12	22 C	0.00	0.12
--------Bottom Chords---------				
A -G	0.68	705 T	0.06	0.62
G -S2	0.69	768 T	0.07	0.62
S2-F	0.40	768 T	0.07	0.33
F -E	0.35	207 T	0.02	0.33
-------------Webs-------------				
A -B	0.27	165 C	WindLd	
A -H	0.27	917 C		1 Br
H -G	0.08	312 T		
G -I	0.05	198 T		
I -F	0.21	706 C		1 Br
F -C	0.20	657 T		
C -E	0.41	933 C		
E -D	0.01	31 C		

LL Defl -0.22" in A -G L/999
TL Defl -0.44" in A -G L/546
Shear // Grain in B -H 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

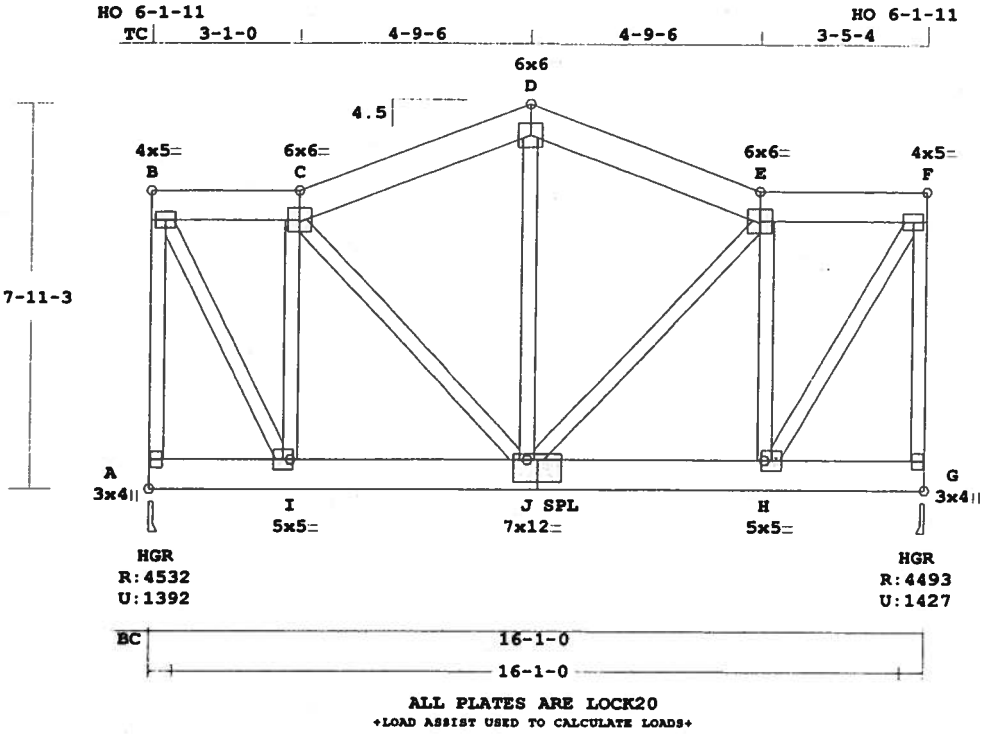
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 933 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R178	1*2P	SP	160100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 222.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.09	2x 8	SP-2400
BC	0.16	2x 8	SP-2400
WB	0.69	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16- 1- 0
BC	Cont.	0- 0- 0	16- 1- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	16.1'
BC V	187	207	0.0'	16.1'
TC V	90	90	3.1'	12.6'

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	4532	1393	3- 8	1-14
			Hz =	-153
G	4494	1428	3- 8	1-14
			Hz =	107

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

B -C	0.01	1954	C	0.00	0.01
C -D	0.09	2894	C	0.03	0.06
D -E	0.09	2892	C	0.00	0.09
E -F	0.02	2149	C	0.02	0.00

--------Bottom Chords---------

A -I	0.04	114	C	0.00	0.04
I -J	0.15	1997	T	0.03	0.12
J -H	0.16	2188	T	0.04	0.12
H -G	0.05	106	T	0.00	0.05

-------------Webs-------------

A -B	0.43	3984	C	WindLd
B -I	0.69	4341	T	
I -C	0.22	2424	C	
C -J	0.16	1049	T	
J -D	0.13	866	T	
J -E	0.12	769	T	
H -E	0.20	2236	C	
H -F	0.69	4351	T	
G -F	0.36	3888	C	

LL Defl -0.04" in J -H L/999
TL Defl -0.07" in J -H L/999
Shear // Grain in I -J 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	4.0x 5.0	Ctr	Ctr	0.94
C	LOCK	6.0x 6.0	Ctr	Ctr	0.80
D	LOCK	6.0x 6.0	Ctr	Ctr	0.62
E	LOCK	6.0x 6.0	Ctr	Ctr	0.80
F	LOCK	4.0x 5.0	Ctr	Ctr	0.91
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
I	LOCK	5.0x 5.0	Ctr	Ctr	0.85
J	LOCK	7.0x12.0	Ctr	-2.0	0.99
H	LOCK	5.0x 5.0	Ctr	Ctr	0.81
G	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 20- 8- 8
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

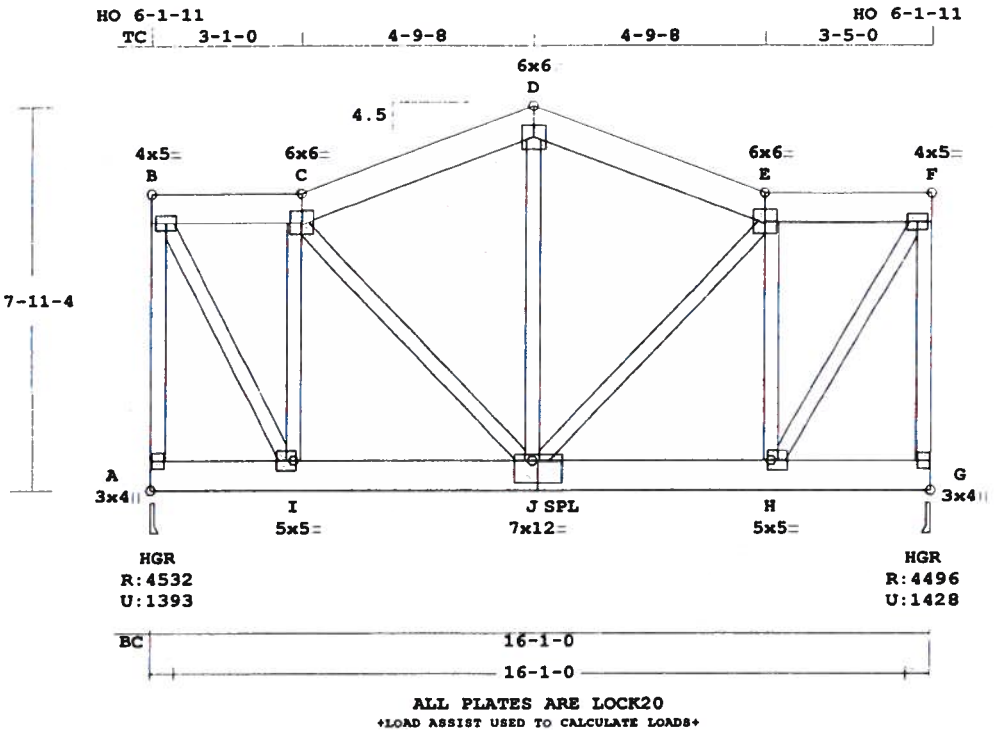
	Rows	Nails	Screws	Bolts
	----Spacing (In)----			
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3984 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R179	1*2P	SP	160100	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 222.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.09	2x 8	SP-2400
BC	0.16	2x 8	SP-2400
WB	0.69	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16- 1- 0
BC	Cont.	0- 0- 0	16- 1- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	16.1'
BC V	187	207	0.0'	16.1'
TC V	90	90	3.1'	12.7'

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	4533	1393	3- 8	1-14
			Hz =	-153
G	4497	1429	3- 8	1-14
			Hz =	107

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

B -C	0.01	1955	C	0.00	0.01
C -D	0.09	2894	C	0.03	0.06
D -E	0.09	2892	C	0.00	0.09
E -F	0.02	2139	C	0.02	0.00

--------Bottom Chords---------

A -I	0.04	114	C	0.00	0.04
I -J	0.15	1998	T	0.03	0.12
J -H	0.16	2178	T	0.04	0.12
H -G	0.05	106	T	0.00	0.05

------------Webs-------------

A -B	0.43	3986	C	WindLd
B -I	0.69	4343	T	
I -C	0.22	2424	C	
C -J	0.16	1047	T	
J -D	0.13	865	T	
J -E	0.12	783	T	
H -E	0.20	2247	C	
H -F	0.69	4352	T	
G -F	0.36	3895	C	

LL Defl -0.04" in J -H L/999
TL Defl -0.07" in J -H L/999
Shear // Grain in I -J 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	4.0x 5.0	Ctr	Ctr	0.94
C	LOCK	6.0x 6.0	Ctr	Ctr	0.80
D	LOCK	6.0x 6.0	Ctr	Ctr	0.62
E	LOCK	6.0x 6.0	Ctr	Ctr	0.80
F	LOCK	4.0x 5.0	Ctr	Ctr	0.92
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
I	LOCK	5.0x 5.0	Ctr	Ctr	0.85
J	LOCK	7.0x12.0	Ctr-2.0		0.99
H	LOCK	5.0x 5.0	Ctr	Ctr	0.82
G	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 20- 8- 8
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

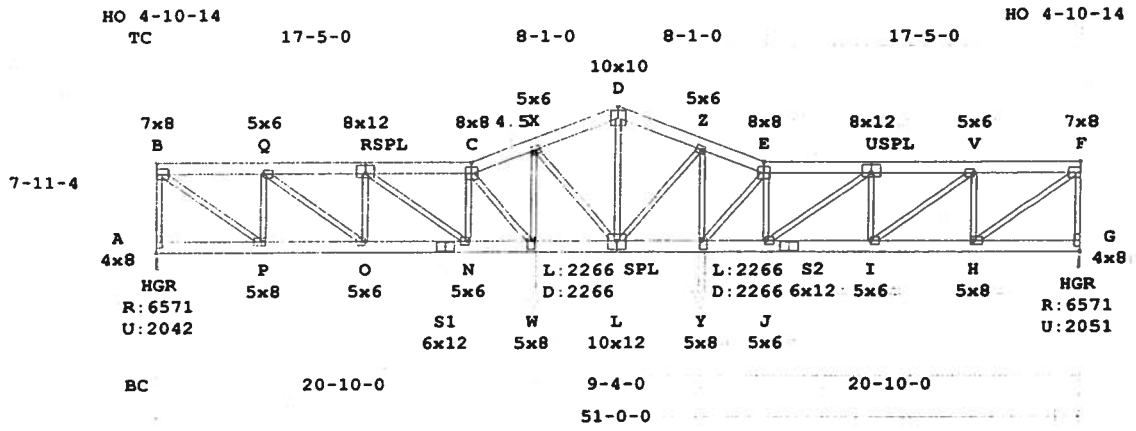
	Rows	----Spacing (In)---- Nails	Screws	Bolts
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3986 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R180*	1*2P	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 596.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.60	2x 8	SP-2400
WB	0.84	2x 4	SP-#3
EX	B -P	2x 4	SP-#2
EX	Q -O	2x 4	SP-#2
EX	R -N	2x 4	SP-#2
EX	W -X	2x 4	SP-#2
EX	L -D	2x 4	SP-2400
EX	Y -Z	2x 4	SP-#2
EX	J -U	2x 4	SP-#2
EX	I -V	2x 4	SP-#2
EX	H -F	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	2266	2266	20.8'	CL-LB
BC V	2266	2266	30.2'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	6571	2043	3- 8	2-12
			Hz =	-175
G	6571	2051	3- 8	2-12
			Hz =	80

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -Q	0.06	8458 C	0.05 0.01
Q -R	0.15	16052 C	0.09 0.06
R -C	0.28	23288 C	0.18 0.10
C -X	0.30	22499 C	0.17 0.13
X -D	0.13	17567 C	0.10 0.03
D -Z	0.13	17567 C	0.10 0.03
Z -E	0.30	22499 C	0.17 0.13
E -U	0.28	23288 C	0.18 0.10
U -V	0.15	16052 C	0.09 0.06
V -F	0.06	8457 C	0.05 0.01
--------Bottom Chords---------			
A -P	0.02	111 T	0.00 0.02
P -O	0.18	8457 T	0.16 0.02
O -S1	0.43	16052 T	0.30 0.13
S1-N	0.46	16052 T	0.30 0.16
N -W	0.60	23416 T	0.44 0.16
W -L	0.52	21130 T	0.40 0.12
L -Y	0.52	21130 T	0.40 0.12
Y -J	0.60	23416 T	0.44 0.16
J -S2	0.46	16051 T	0.30 0.16
S2-I	0.43	16051 T	0.30 0.13
I -H	0.18	8457 T	0.16 0.02
H -G	0.02	79 T	0.00 0.02
------------Webs------------			
A -B	0.55	6487 C	WindLd
B -P	0.97	10557 T	
P -Q	0.53	6249 C	
Q -O	0.88	9569 T	
O -R	0.46	5499 C	
R -N	0.83	9033 T	
N -C	0.47	5584 C	
C -W	0.32	3467 C	
W -X	0.64	6950 T	
X -L	0.84	7238 C	
L -D	0.47	11887 T	
L -Z	0.84	7238 C	
Y -Z	0.64	6950 T	
Y -E	0.32	3467 C	
J -E	0.47	5584 C	
J -U	0.83	9033 T	
I -U	0.46	5499 C	
I -V	0.88	9570 T	
H -V	0.53	6250 C	
H -F	0.97	10558 T	
G -F	0.55	6487 C	

LL Defl -0.65" in W -L L/931
TL Defl -1.30" in W -L L/465
Shear // Grain in W -L 0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.97
Q	LOCK	5.0x 6.0	1.0	Ctr	0.96
R	LOCK	8.0x12.0	Ctr	1.5	0.99
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
X	LOCK	5.0x 6.0	-0.5	-0.2	0.97
D	LOCK	10.0x10.0	Ctr	Ctr	0.85
Z	LOCK	5.0x 6.0	0.5	-0.2	0.97
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
U	LOCK	8.0x12.0	Ctr	1.5	0.99
V	LOCK	5.0x 6.0	-1.0	Ctr	0.96
F	LOCK	7.0x 8.0	-0.5	Ctr	0.97
A	LOCK	4.0x 8.0	0.2	Ctr	0.76
P	LOCK	5.0x 8.0	-0.5	Ctr	0.97
O	LOCK	5.0x 6.0	-1.0	Ctr	0.96
S1	LOCK	6.0x12.0	Ctr	Ctr	1.00
N	LOCK	5.0x 6.0	-0.5	Ctr	0.95
W	LOCK	5.0x 8.0	0.5	-1.8	0.95
L	LOCK	10.0x12.0	Ctr	-0.5	0.99
Y	LOCK	5.0x 8.0	-0.5	-1.8	0.95
J	LOCK	5.0x 6.0	0.5	Ctr	0.95
S2	LOCK	6.0x12.0	Ctr	Ctr	1.00
I	LOCK	5.0x 6.0	1.0	Ctr	0.96
H	LOCK	5.0x 8.0	0.5	Ctr	0.97
G	LOCK	4.0x 8.0	-0.2	Ctr	0.76

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	4	4	

Plus clusters of nails where shown.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 23288 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,

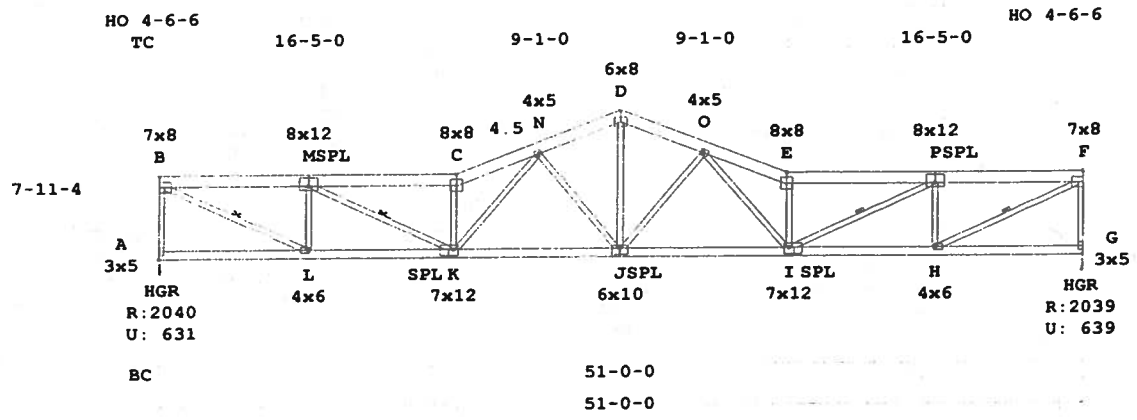
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R180*	1*2P	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish

Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R181*	1	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 511.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 8	SP-2400
BC	0.71	2x 6	SP-#2
WB	1.00	2x 4	SP-#3
EX B -L		2x 4	SP-#2
EX H -F		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on B -L
WB 1 rows CLB on M -K
WB 1 rows CLB on I -P
WB 1 rows CLB on H -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	2040	632	3- 8	2- 7
			Hz =	-172
G	2040	639	3- 8	2- 7
			Hz =	77

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
B -M	0.10	3535	C	0.01	0.09
M -C	0.10	5629	C	0.03	0.07
C -N	0.10	5985	C	0.07	0.03
N -D	0.06	3762	C	0.04	0.02
D -O	0.06	3762	C	0.04	0.02
O -E	0.10	5985	C	0.07	0.03
E -P	0.10	5629	C	0.03	0.07
P -F	0.10	3535	C	0.01	0.09
--------Bottom Chords---------					
A -L	0.18	113	T	0.00	0.18
L -K	0.53	3535	T	0.42	0.11
K -J	0.71	4387	T	0.53	0.18
J -I	0.71	4387	T	0.53	0.18
I -H	0.53	3535	T	0.42	0.11
H -G	0.18	77	T	0.00	0.18
------------Webs------------					
A -B	0.48	1969	C	WindLd	
B -L	0.72	3947	T		1 Br
L -M	0.38	1564	C		
M -K	0.74	2337	T		1 Br
K -C	0.59	2417	C		
K -N	0.62	1956	T		
N -J	1.00	1386	C		
J -D	0.76	2390	T		
J -O	1.00	1386	C		
O -I	0.62	1956	T		
I -E	0.59	2417	C		
I -P	0.74	2337	T		1 Br
H -P	0.38	1564	C		
H -F	0.72	3946	T		1 Br
G -F	0.48	1969	C		

LL Defl -0.40" in K -J L/999
TL Defl -0.83" in K -J L/725
Shear // Grain in B -M 0.16

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

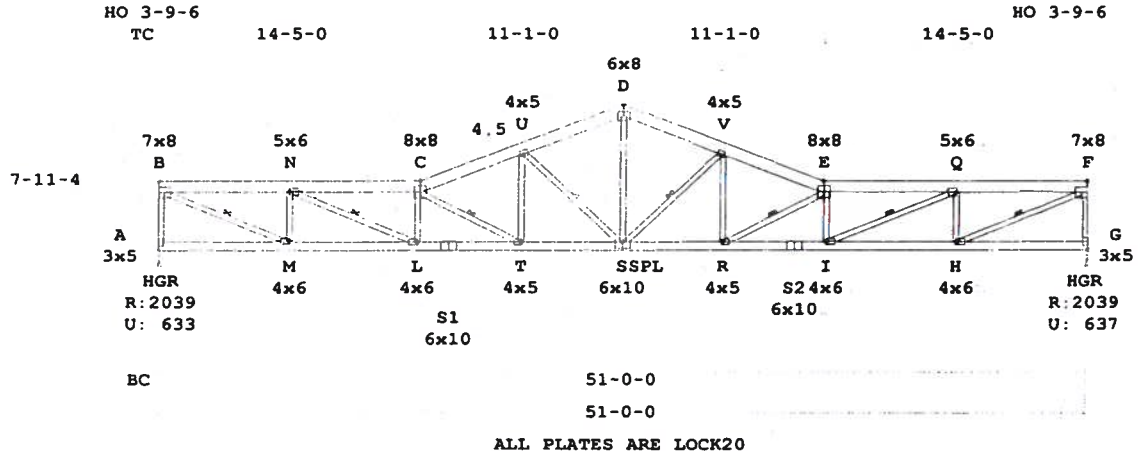
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
M	LOCK	8.0x12.0	Ctr	1.5	0.99
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
N	LOCK	4.0x 5.0	-0.5	0.2	0.87
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
O	LOCK	4.0x 5.0	0.5	0.2	0.87
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	8.0x12.0	Ctr	1.5	0.99
F	LOCK	7.0x 8.0	-0.5	Ctr	0.90
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
L	LOCK	4.0x 6.0	-0.5	Ctr	0.98
K	LOCK	7.0x12.0	0.5	-0.8	0.97
J	LOCK	6.0x10.0	Ctr	-1.2	0.97
I	LOCK	7.0x12.0	-0.5	-0.8	0.97
H	LOCK	4.0x 6.0	0.5	Ctr	0.98
G	LOCK	3.0x 5.0	Ctr	Ctr	0.90

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5985 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R182*	1	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 511.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.13	2x 8	SP-2400
BC	0.93	2x 6	SP-#2
WB	0.90	2x 4	SP-#3
EX B -M		2x 4	SP-#2
EX H -F		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on B -M
WB 1 rows CLB on N -L
WB 1 rows CLB on C -T
WB 1 rows CLB on U -S
WB 1 rows CLB on S -V
WB 1 rows CLB on R -E
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	633	3- 8	2- 7
			Hz =	-164
G	2040	638	3- 8	2- 7
			Hz =	84

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
B -N	0.09	3851 C	0.01 0.08
N -C	0.12	6411 C	0.04 0.08
C -U	0.13	4949 C	0.02 0.11
U -D	0.08	3749 C	0.04 0.04
D -V	0.08	3748 C	0.04 0.04
V -E	0.13	4949 C	0.02 0.11
E -Q	0.12	6412 C	0.04 0.08
Q -F	0.09	3851 C	0.01 0.08
Bottom Chords			
A -M	0.15	116 T	0.00 0.15
M -L	0.64	3851 T	0.46 0.18
L -S1	0.93	6449 T	0.78 0.15
S1-T	0.92	6449 T	0.78 0.14
T -S	0.62	4673 T	0.56 0.06
S -R	0.62	4674 T	0.56 0.06
R -S2	0.92	6449 T	0.78 0.14
S2-I	0.93	6449 T	0.78 0.15
I -H	0.64	3851 T	0.46 0.18
H -G	0.15	84 T	0.00 0.15
Webs			
A -B	0.38	1979 C	WindLd
B -M	0.78	4239 T	1 Br
M -N	0.31	1580 C	
N -L	0.90	2818 T	1 Br
L -C	0.22	1130 C	
C -T	0.42	2002 C	1 Br
T -U	0.34	1082 T	
U -S	0.41	1628 C	1 Br
S -D	0.75	2359 T	
S -V	0.41	1629 C	1 Br
R -V	0.34	1083 T	
R -E	0.42	2002 C	1 Br
I -E	0.22	1131 C	
I -Q	0.90	2818 T	1 Br
H -Q	0.31	1580 C	
H -F	0.78	4240 T	1 Br
G -F	0.38	1979 C	

LL Defl -0.47" in R -S2 L/999
TL Defl -0.94" in R -S2 L/640
Shear // Grain in C -U 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

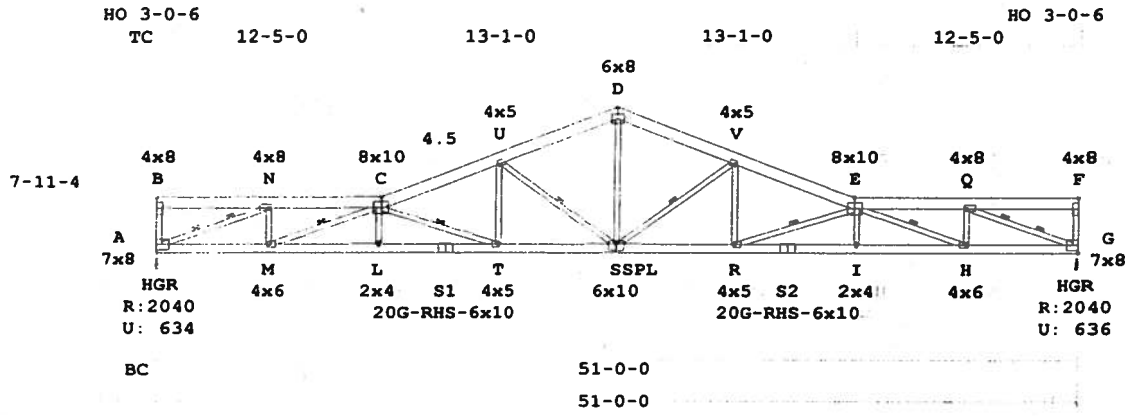
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
N	LOCK	5.0x 6.0	1.0	Ctr	0.98
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
U	LOCK	4.0x 5.0	-0.5	0.2	0.56
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
V	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	5.0x 6.0	-1.0	Ctr	0.98
F	LOCK	7.0x 8.0	-0.5	Ctr	0.90
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
M	LOCK	4.0x 6.0	-1.0	Ctr	0.98
L	LOCK	4.0x 6.0	-1.0	Ctr	0.98
S1	LOCK	6.0x10.0	Ctr	0.2	0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S	LOCK	6.0x10.0	Ctr	-1.2	0.97
R	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	6.0x10.0	Ctr	0.2	0.80
I	LOCK	4.0x 6.0	1.0	Ctr	0.98
H	LOCK	4.0x 6.0	1.0	Ctr	0.98
G	LOCK	3.0x 5.0	Ctr	Ctr	0.90

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6412 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*R183*	1	SP	510000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 497.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 8	SP-2400
BC	0.49	2x 6	SP-2400
WB	0.81	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 2 rows CLB on A -N
WB 1 rows CLB on M -C
WB 1 rows CLB on C -T
WB 1 rows CLB on U -S
WB 1 rows CLB on S -V
WB 1 rows CLB on R -E
WB 1 rows CLB on E -H
WB 2 rows CLB on Q -G
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	634	3- 8	1-11
			Hz =	-155
G	2040	637	3- 8	1-11
			Hz =	96

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
B -N	0.07	34	T	0.00	0.07
N -C	0.08	4352	C	0.02	0.06
C -U	0.11	5258	C	0.03	0.08
U -D	0.09	3749	C	0.04	0.05
D -V	0.09	3749	C	0.04	0.05
V -E	0.11	5258	C	0.03	0.08
E -Q	0.08	4353	C	0.02	0.06
Q -F	0.07	0	T	0.00	0.07
Bottom Chords					
A -M	0.22	4352	T	0.19	0.03
M -L	0.49	7857	T	0.35	0.14
L -S1	0.49	7858	T	0.35	0.14
S1-T	0.42	7858	T	0.35	0.07
T -S	0.28	4945	T	0.22	0.06
S -R	0.28	4945	T	0.22	0.06
R -S2	0.42	7858	T	0.35	0.07
S2-I	0.49	7858	T	0.35	0.14
I -H	0.49	7858	T	0.35	0.14
H -G	0.22	4352	T	0.19	0.03
Webs					
A -B	0.05	189	C	WindLd	
A -N	0.81	4708	C		2 Br
M -N	0.46	1443	T		
M -C	0.79	3723	C		1 Br
L -C	0.04	156	T		
C -T	0.71	3061	C		1 Br
T -U	0.36	1148	T		
U -S	0.50	1809	C		1 Br
S -D	0.73	2289	T		
S -V	0.50	1809	C		1 Br
R -V	0.36	1148	T		
R -E	0.71	3061	C		1 Br
I -E	0.04	156	T		
E -H	0.79	3723	C		1 Br
H -Q	0.46	1443	T		
Q -G	0.81	4708	C		2 Br
G -F	0.03	189	C		

LL Defl -0.57" in R -S2 L/999
TL Defl -1.16" in R -S2 L/522
Shear // Grain in C -U 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

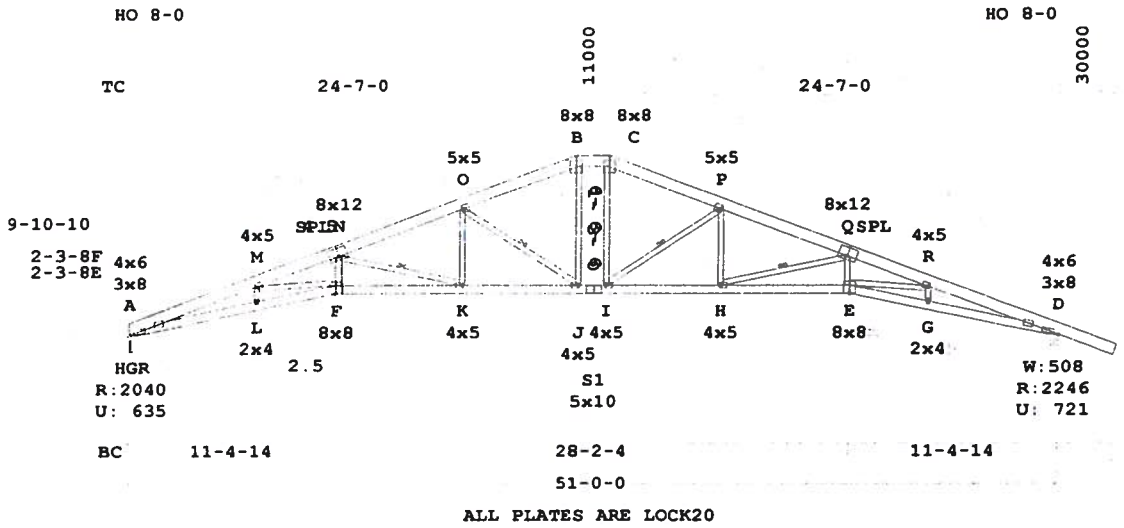
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	4.0x 8.0	0.2	Ctr	0.76
N	LOCK	4.0x 8.0-0.5	0.5		0.83
C	LOCK	8.0x10.0-0.5	Ctr		0.85
U	LOCK	4.0x 5.0-0.5	0.2		0.57
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
V	LOCK	4.0x 5.0	0.5	0.2	0.57
E	LOCK	8.0x10.0	0.5	Ctr	0.85
Q	LOCK	4.0x 8.0	0.5	0.5	0.83
F	LOCK	4.0x 8.0-0.2	Ctr		0.76
A	LOCK	7.0x 8.0	0.5	Ctr	0.83
M	LOCK	4.0x 6.0	Ctr	Ctr	0.89
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
S1	RHS	6.0x10.0	Ctr	0.2	0.75
T	LOCK	4.0x 5.0	Ctr	Ctr	0.89
S	LOCK	6.0x10.0	Ctr	-1.2	0.97
R	LOCK	4.0x 5.0	Ctr	Ctr	0.89
S2	RHS	6.0x10.0	Ctr	0.2	0.75
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
H	LOCK	4.0x 6.0	Ctr	Ctr	0.89
G	LOCK	7.0x 8.0	-0.5	Ctr	0.83

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5258 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R191*	1	HPSC4	510000	4.5	0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 485.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.76	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	636	3- 8	1-11
			Hz =	-172
D	2247	721	5- 8	1-14
			Hz =	171

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.28	8614	C	0.08	0.20
M -N	0.31	8759	C	0.09	0.22
N -O	0.17	5383	C	0.03	0.14
O -B	0.09	3908	C	0.04	0.05
B -C	0.04	3648	C	0.04	0.00
C -P	0.09	3908	C	0.04	0.05
P -Q	0.17	5383	C	0.03	0.14
Q -R	0.31	8758	C	0.09	0.22
R -D	0.28	8613	C	0.08	0.20
Bottom Chords					
A -L	0.55	8141	T	0.37	0.18
L -F	0.55	8177	T	0.37	0.18
F -K	0.40	8154	T	0.37	0.03
K -J	0.26	5077	T	0.23	0.03
J -S1	0.19	3650	T	0.16	0.03
S1-I	0.19	3650	T	0.16	0.03
I -H	0.26	5077	T	0.23	0.03
H -E	0.40	8153	T	0.37	0.03
E -G	0.55	8176	T	0.37	0.18
G -D	0.55	8140	T	0.37	0.18
Webs					
L -M	0.03	133	T		
M -F	0.07	234	T		
F -N	0.59	1840	T		
N -K	0.76	3224	C		1 Br
K -O	0.34	1079	T		
O -J	0.46	1785	C		1 Br
J -B	0.37	1158	T		
I -C	0.37	1158	T		
I -P	0.46	1784	C		1 Br
H -P	0.34	1079	T		
H -Q	0.76	3223	C		1 Br
E -Q	0.59	1840	T		
E -R	0.07	233	T		
G -R	0.03	133	T		

LL Defl -0.69" in K -J L/877
TL Defl -1.39" in K -J L/435
Shear // Grain in A -M 0.20

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES TO BE MANUFACTURED BY ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

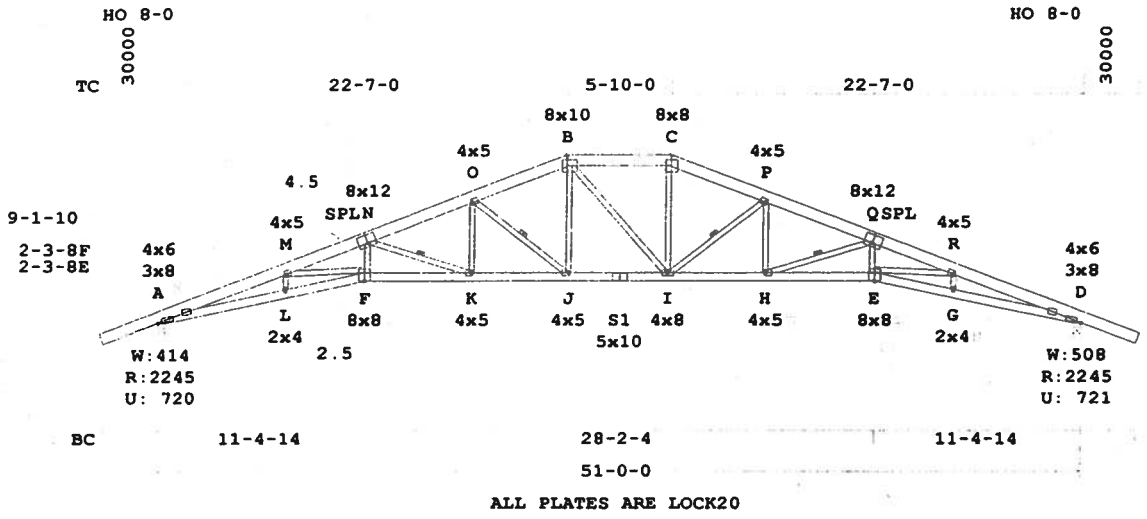
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.78
M	LOCK	4.0x 5.0-0.5	0.2		0.84
N	LOCK	8.0x12.0-0.6	1.5		0.97
O	LOCK	5.0x 5.0	Ctr	Ctr	0.67
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	5.0x 5.0	Ctr	Ctr	0.67
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.84
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.78
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr-1.5		0.77
K	LOCK	4.0x 5.0	Ctr	Ctr	0.94
J	LOCK	4.0x 5.0	Ctr	Ctr	0.89
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.89
H	LOCK	4.0x 5.0	Ctr	Ctr	0.94
E	LOCK	8.0x 8.0	Ctr-1.5		0.77
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8759 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R192*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 499.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.62	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2245	721	4-14	1-14
			Hz =	-158
D	2245	721	5- 8	1-14
			Hz =	158

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.28	8604	C	0.08	0.20
M -N	0.31	8762	C	0.09	0.22
N -O	0.17	5663	C	0.04	0.13
O -B	0.08	4310	C	0.05	0.03
B -C	0.07	4040	C	0.01	0.06
C -P	0.08	4318	C	0.05	0.03
P -Q	0.17	5666	C	0.04	0.13
Q -R	0.31	8761	C	0.09	0.22
R -D	0.28	8605	C	0.08	0.20
--------Bottom Chords---------					
A -L	0.55	8131	T	0.37	0.18
L -F	0.55	8166	T	0.37	0.18
F -K	0.41	8158	T	0.37	0.04
K -J	0.27	5335	T	0.24	0.03
J -S1	0.19	4020	T	0.18	0.01
S1-I	0.22	4020	T	0.18	0.04
I -H	0.28	5339	T	0.24	0.04
H -E	0.41	8156	T	0.37	0.04
E -G	0.55	8167	T	0.37	0.18
G -D	0.55	8131	T	0.37	0.18
------------Webs------------					
L -M	0.03	135	T		
M -F	0.07	247	T		
F -N	0.58	1836	T		
N -K	0.62	3008	C		1 Br
K -O	0.36	1129	T		
O -J	0.37	1673	C		1 Br
J -B	0.37	1161	T		
B -I	0.12	148	T		
I -C	0.37	1178	T		
I -P	0.37	1676	C		1 Br
H -P	0.35	1109	T		
H -Q	0.61	3001	C		1 Br
E -Q	0.59	1840	T		
E -R	0.07	245	T		
G -R	0.03	135	T		

LL Defl -0.68" in K -J L/886
TL Defl -1.37" in K -J L/439
Shear // Grain in A -M 0.20

Hz Disp	LL	DL	TL
Jt D	0.41"	0.42"	0.83"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

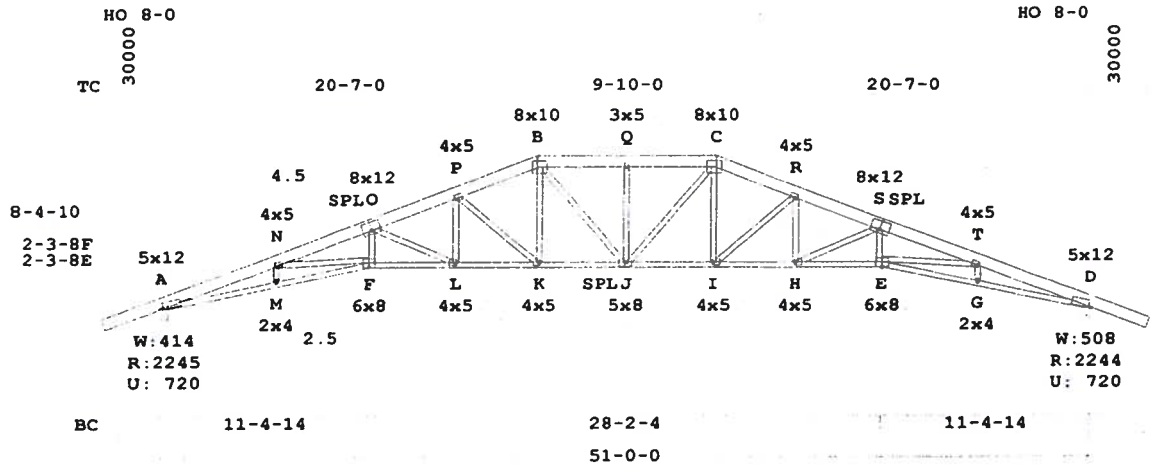
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.77
M	LOCK	4.0x 5.0-0.5	0.2		0.62
N	LOCK	8.0x12.0-0.6	1.5		0.97
O	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.56
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.77
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr-1.5		0.80
K	LOCK	4.0x 5.0	Ctr	Ctr	0.86
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.86
E	LOCK	8.0x 8.0	Ctr-1.5		0.80
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8762 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R193*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 463.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.41	2x 8	SP-2400
BC	0.75	2x 4	SP-2400
WB	0.97	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2245	721	4-14	1-14
			Hz =	-144
D	2245	721	5- 8	1-14
			Hz =	144

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -N	0.38	7898	C	0.06	0.32
N -O	0.41	8466	C	0.08	0.33
O -P	0.14	5945	C	0.04	0.10
P -B	0.07	4689	C	0.05	0.02
B -Q	0.06	4576	C	0.05	0.01
Q -C	0.06	4576	C	0.05	0.01
C -R	0.07	4689	C	0.05	0.02
R -S	0.14	5945	C	0.04	0.10
S -T	0.41	8464	C	0.08	0.33
T -D	0.38	7897	C	0.06	0.32
--------Bottom Chords---------					
A -M	0.73	7384	T	0.53	0.20
M -F	0.74	7397	T	0.53	0.21
F -L	0.75	7924	T	0.57	0.18
L -K	0.45	5591	T	0.40	0.05
K -J	0.35	4387	T	0.31	0.04
J -I	0.35	4387	T	0.31	0.04
I -H	0.45	5590	T	0.40	0.05
H -E	0.75	7922	T	0.57	0.18
E -G	0.74	7395	T	0.53	0.21
G -D	0.73	7382	T	0.53	0.20
----------Webs----------					
M -N	0.04	171	T		
N -F	0.23	741	T		
F -O	0.52	1641	T		
O -L	0.97	2596	C		
L -P	0.37	1177	T		
P -K	0.79	1561	C		
K -B	0.36	1127	T		
B -J	0.10	288	T		
J -Q	0.15	339	C		
J -C	0.10	289	T		
I -C	0.36	1127	T		
I -R	0.79	1561	C		
H -R	0.37	1177	T		
H -S	0.97	2595	C		
E -S	0.52	1640	T		
E -T	0.23	741	T		
G -T	0.04	171	T		

LL Defl -0.81" in K -J L/742
TL Defl -1.64" in K -J L/367
Shear // Grain in A -N 0.34

Hz Disp	LL	DL	TL
Jt D	0.53"	0.53"	1.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

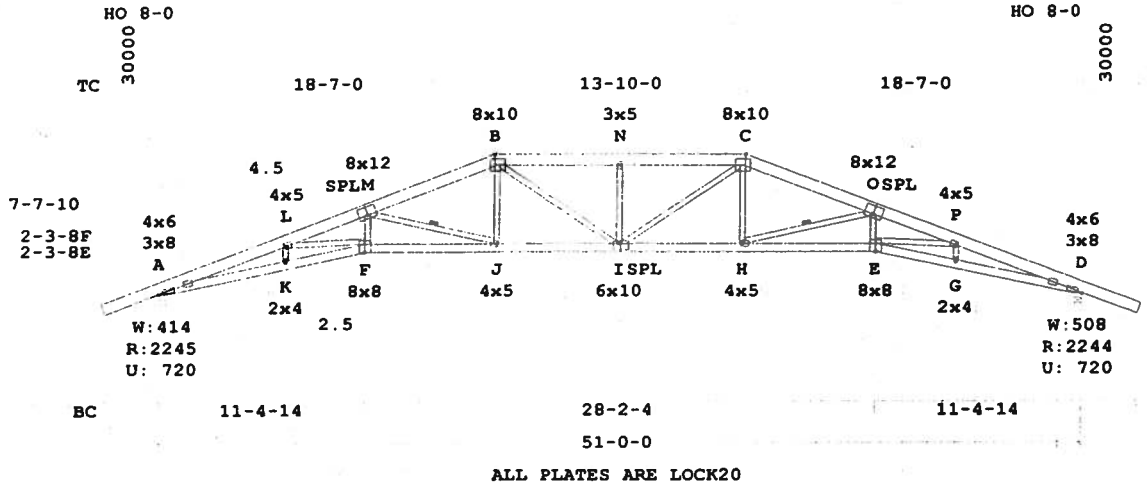
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x12.0 Ctr	0.8		0.99
N	LOCK	4.0x 5.0-0.5	0.2		0.63
O	LOCK	8.0x12.0-0.6	1.5		0.97
P	LOCK	4.0x 5.0-0.5	0.2		0.58
B	LOCK	8.0x10.0 Ctr	Ctr		0.84
Q	LOCK	3.0x 5.0 Ctr	Ctr		0.36
C	LOCK	8.0x10.0 Ctr	Ctr		0.84
R	LOCK	4.0x 5.0	0.5	0.2	0.58
S	LOCK	8.0x12.0	0.6	1.5	0.97
T	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	5.0x12.0 Ctr	0.8		0.99
M	LOCK	2.0x 4.0 Ctr	Ctr		0.54
F	LOCK	6.0x 8.0 Ctr-0.5			0.99
L	LOCK	4.0x 5.0 Ctr	Ctr		0.71
K	LOCK	4.0x 5.0 Ctr	Ctr		0.60
J	LOCK	5.0x 8.0 0.5-0.5			0.88
I	LOCK	4.0x 5.0 Ctr	Ctr		0.60
H	LOCK	4.0x 5.0 Ctr	Ctr		0.71
E	LOCK	6.0x 8.0 Ctr-0.5			0.99
G	LOCK	2.0x 4.0 Ctr	Ctr		0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8466 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R194*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 480.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.82	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on M -J
WB 1 rows CLB on H -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2245	721	4-14	1-14
			Hz =	-130
D	2245	721	5- 8	1-14
			Hz =	130

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.28	8609	C	0.08	0.20
L -M	0.31	8754	C	0.09	0.22
M -B	0.18	5298	C	0.03	0.15
B -N	0.09	5382	C	0.03	0.06
N -C	0.09	5382	C	0.03	0.06
C -O	0.18	5297	C	0.03	0.15
O -P	0.31	8752	C	0.09	0.22
P -D	0.28	8607	C	0.08	0.20
--------Bottom Chords---------					
A -K	0.55	8136	T	0.37	0.18
K -F	0.55	8171	T	0.37	0.18
F -J	0.40	8149	T	0.37	0.03
J -I	0.25	4965	T	0.22	0.03
I -H	0.25	4965	T	0.22	0.03
H -E	0.40	8148	T	0.37	0.03
E -G	0.55	8169	T	0.37	0.18
G -D	0.55	8134	T	0.37	0.18
----------Webs----------					
K -L	0.03	127	T		
L -F	0.07	233	T		
F -M	0.58	1834	T		
M -J	0.82	3297	C		1 Br
J -B	0.35	1093	T		
B -I	0.22	505	T		
I -N	0.14	441	C		
I -C	0.22	506	T		
H -C	0.35	1093	T		
H -O	0.82	3296	C		1 Br
E -O	0.58	1833	T		
E -P	0.07	233	T		
G -P	0.03	127	T		

LL Defl -0.71" in J -I L/842
TL Defl -1.44" in J -I L/418
Shear // Grain in A -L 0.20

Hz Disp	LL	DL	TL
Jt D	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

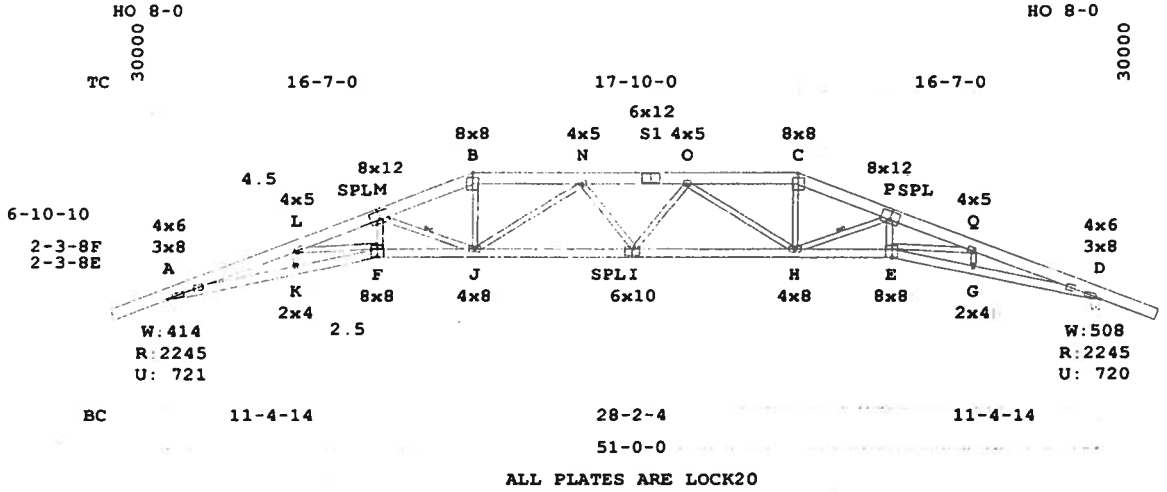
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.78
L	LOCK	4.0x 5.0	-0.5	0.2	0.63
M	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
N	LOCK	3.0x 5.0	Ctr	Ctr	0.36
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	8.0x12.0	0.6	1.5	0.97
P	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.78
K	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr	-1.5	0.79
J	LOCK	4.0x 5.0	Ctr	Ctr	0.97
I	LOCK	6.0x10.0	Ctr	-1.2	0.97
H	LOCK	4.0x 5.0	Ctr	Ctr	0.97
E	LOCK	8.0x 8.0	Ctr	-1.5	0.79
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8754 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R195*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 474.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.58	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on M -J
WB 1 rows CLB on H -P
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2246	721	4-14	1-14
		Hz =	-115	
D	2245	721	5- 8	1-14
		Hz =	116	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.28	8597	C	0.08	0.20
L -M	0.31	8736	C	0.09	0.22
M -B	0.17	5929	C	0.04	0.13
B -N	0.09	5594	C	0.03	0.06
N -S1	0.10	6286	C	0.04	0.06
S1-O	0.09	6286	C	0.04	0.05
O -C	0.09	5593	C	0.03	0.06
C -P	0.17	5928	C	0.04	0.13
P -Q	0.31	8735	C	0.09	0.22
Q -D	0.28	8595	C	0.08	0.20
--------Bottom Chords---------					
A -K	0.55	8120	T	0.37	0.18
K -F	0.55	8156	T	0.37	0.18
F -J	0.40	8126	T	0.37	0.03
J -I	0.33	6222	T	0.28	0.05
I -H	0.33	6221	T	0.28	0.05
H -E	0.40	8125	T	0.37	0.03
E -G	0.55	8155	T	0.37	0.18
G -D	0.55	8118	T	0.37	0.18
------------Webs------------					
K -L	0.03	136	T		
L -F	0.07	225	T		
F -M	0.58	1818	T		
M -J	0.52	2744	C		1 Br
J -B	0.52	1627	T		
J -N	0.55	765	C		
N -I	0.05	223	T		
I -O	0.05	223	T		
O -H	0.55	765	C		
H -C	0.52	1627	T		
H -P	0.52	2743	C		1 Br
E -P	0.58	1817	T		
E -Q	0.07	224	T		
G -Q	0.03	136	T		

LL Defl -0.74" in J -I L/816
TL Defl -1.50" in J -I L/400
Shear // Grain in A -L 0.20

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

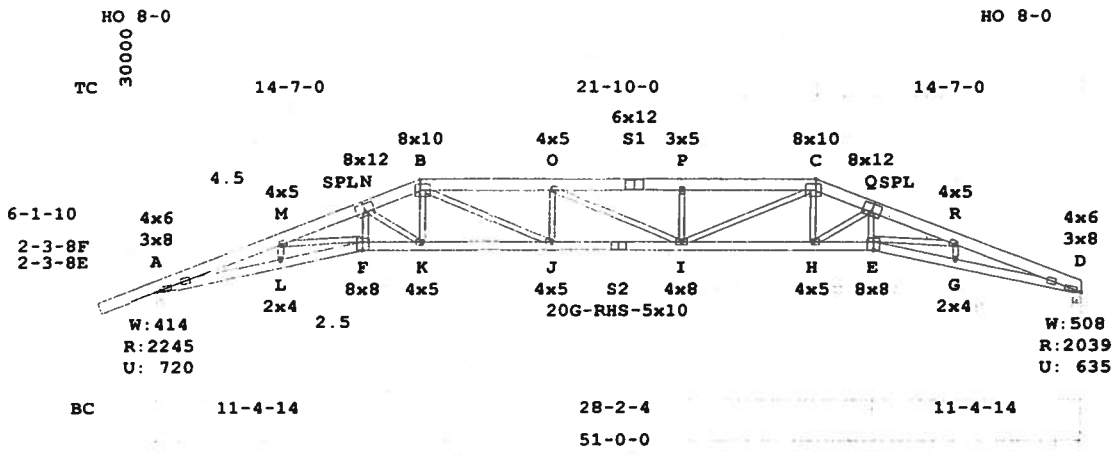
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.77
L	LOCK	4.0x 5.0	-0.5	0.2	0.63
M	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
N	LOCK	4.0x 5.0	Ctr	0.5	0.58
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
O	LOCK	4.0x 5.0	Ctr	0.5	0.58
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	8.0x12.0	0.6	1.5	0.97
Q	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.77
K	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr	-1.5	0.79
J	LOCK	4.0x 8.0	Ctr	Ctr	0.61
I	LOCK	6.0x10.0	Ctr	-1.2	0.97
H	LOCK	4.0x 8.0	Ctr	Ctr	0.61
E	LOCK	8.0x 8.0	Ctr	-1.5	0.79
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8736 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R196*	1	HPSC4	510000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 462.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.56	2x 6	SP-2400
WB	0.58	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2245	721	4-14	1-14
			Hz =	-102
D	2040	635	5- 8	1-11
			Hz =	101

Membr	CSI	P Lbs	Ax	CSI	Bnd
----------Top Chords----------					
A -M	0.28	8583	C	0.08	0.20
M -N	0.31	8749	C	0.09	0.22
N -B	0.17	6718	C	0.05	0.12
B -O	0.15	7607	C	0.07	0.08
O -S1	0.17	7620	C	0.07	0.10
S1-P	0.16	7620	C	0.07	0.09
P -C	0.15	7620	C	0.07	0.08
C -Q	0.17	6717	C	0.05	0.12
Q -R	0.31	8752	C	0.09	0.22
R -D	0.28	8583	C	0.08	0.20
--------Bottom Chords---------					
A -L	0.56	8109	T	0.37	0.19
L -F	0.56	8146	T	0.37	0.19
F -K	0.41	8143	T	0.37	0.04
K -J	0.33	6305	T	0.28	0.05
J -S2	0.40	7610	T	0.34	0.06
S2-I	0.40	7610	T	0.34	0.06
I -H	0.33	6304	T	0.28	0.05
H -E	0.41	8145	T	0.37	0.04
E -G	0.56	8147	T	0.37	0.19
G -D	0.56	8109	T	0.37	0.19
----------Webs----------					
L -M	0.03	141	T		
M -F	0.08	251	T		
F -N	0.58	1820	T		
N -K	0.50	2156	C		
K -B	0.41	1299	T		
B -J	0.47	1416	T		
J -O	0.08	417	C		
O -I	0.09	120	T		
I -P	0.08	423	C		
I -C	0.47	1431	T		
H -C	0.41	1298	T		
H -Q	0.50	2160	C		
E -Q	0.58	1821	T		
E -R	0.08	253	T		
G -R	0.03	142	T		

LL Defl -0.86" in J -S2 L/703
TL Defl -1.73" in J -S2 L/348
Shear // Grain in A -M 0.20

Hz Disp	LL	DL	TL
Jt D	0.44"	0.45"	0.88"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

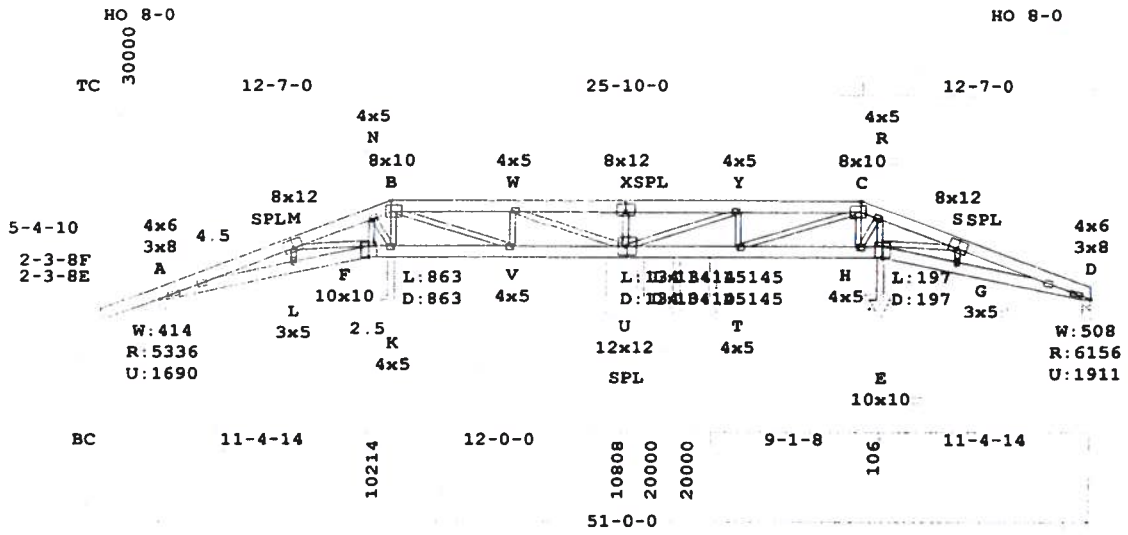
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.77
M	LOCK	4.0x 5.0-0.5	0.2		0.63
N	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	4.0x 5.0	Ctr	Ctr	0.64
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	3.0x 5.0	Ctr	Ctr	0.36
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.77
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr-1.5		0.79
K	LOCK	4.0x 5.0	Ctr	Ctr	0.61
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	RHS	5.0x10.0	Ctr	Ctr	0.92
I	LOCK	4.0x 8.0	Ctr	Ctr	0.62
H	LOCK	4.0x 5.0	Ctr	Ctr	0.61
E	LOCK	8.0x 8.0	Ctr-1.5		0.79
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8752 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R197*	1*3P	HPSC4	510000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 504.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 3-Ply Truss *
****

	CSI	Size	Lumber
TC	0.28	2x 8	SP-2400
BC	0.46	2x 8	SP-2400
WB	0.64	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	73	73	13.6'	23.6'
BC V	126	126	40.5'	50.5'
BC V	863	863	12.6'	CL-LB
BC V	134	134	24.6'	CL-LB
BC V	134	134	26.4'	CL-LB
BC V	145	145	28.4'	CL-LB
BC V	145	145	30.4'	CL-LB
BC V	197	197	39.5'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	5337	1691	4-14	1- 8
			Hz =	-296
D	6157	1911	5- 8	1-11
			Hz =	-145

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -M	0.20	25096 C	0.07 0.13
M -N	0.28	26689 C	0.09 0.19
N -B	0.26	24187 C	0.07 0.19
B -W	0.19	27348 C	0.10 0.09
W -X	0.20	28547 C	0.10 0.10
X -Y	0.19	28547 C	0.10 0.09
Y -C	0.17	25521 C	0.09 0.08
C -R	0.17	21956 C	0.05 0.12
R -S	0.24	25021 C	0.07 0.17
S -D	0.24	25974 C	0.08 0.16
--------Bottom Chords---------			
A -L	0.43	23778 T	0.27 0.16
L -F	0.43	23933 T	0.27 0.16
F -K	0.32	24702 T	0.28 0.04
K -V	0.35	23202 T	0.26 0.09
V -U	0.41	27340 T	0.31 0.10
U -T	0.41	25513 T	0.29 0.12
T -H	0.30	20860 T	0.24 0.06
H -E	0.29	23194 T	0.26 0.03
E -G	0.44	24823 T	0.28 0.16
G -D	0.46	24694 T	0.28 0.18
------------Webs------------			
L -M	0.03	676 C	
M -F	0.16	1561 T	
F -N	0.56	5275 T	
N -K	0.13	2489 C	
K -B	0.46	4391 T	
B -V	0.46	4368 T	
V -W	0.02	552 C	
W -U	0.13	1291 T	
U -X	0.02	514 C	
U -Y	0.34	3257 T	
T -Y	0.06	1312 C	
T -C	0.52	4913 T	
H -C	0.44	4163 T	
H -R	0.22	4214 C	
E -R	0.64	6029 T	
E -S	0.04	808 C	
G -S	0.05	565 T	

LL Defl -0.97" in V -U L/618
TL Defl -1.95" in V -U L/308
Shear // Grain in N -B 0.46

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.52
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	4.0x 5.0	-0.5	0.2	0.87
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
W	LOCK	4.0x 5.0	Ctr	0.5	0.69
X	LOCK	8.0x12.0	Ctr	1.5	0.99
Y	LOCK	4.0x 5.0	Ctr	0.5	0.69
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	0.5	0.2	0.99
S	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.59
L	LOCK	3.0x 5.0	Ctr	Ctr	0.36
F	LOCK	10.0x10.0	Ctr-2.2		0.71
K	LOCK	4.0x 5.0	Ctr-0.5		0.92
V	LOCK	4.0x 5.0	Ctr-0.5		0.76
U	LOCK	12.0x12.0	Ctr	Ctr	0.95
T	LOCK	4.0x 5.0	Ctr-0.5		0.85
H	LOCK	4.0x 5.0	Ctr-0.5		0.87
E	LOCK	10.0x10.0	Ctr-2.2		0.71
G	LOCK	3.0x 5.0	Ctr	Ctr	0.36

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 28547 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R197*	1*3P	HPSC4	510000	4.5	3- 0- 0	0	

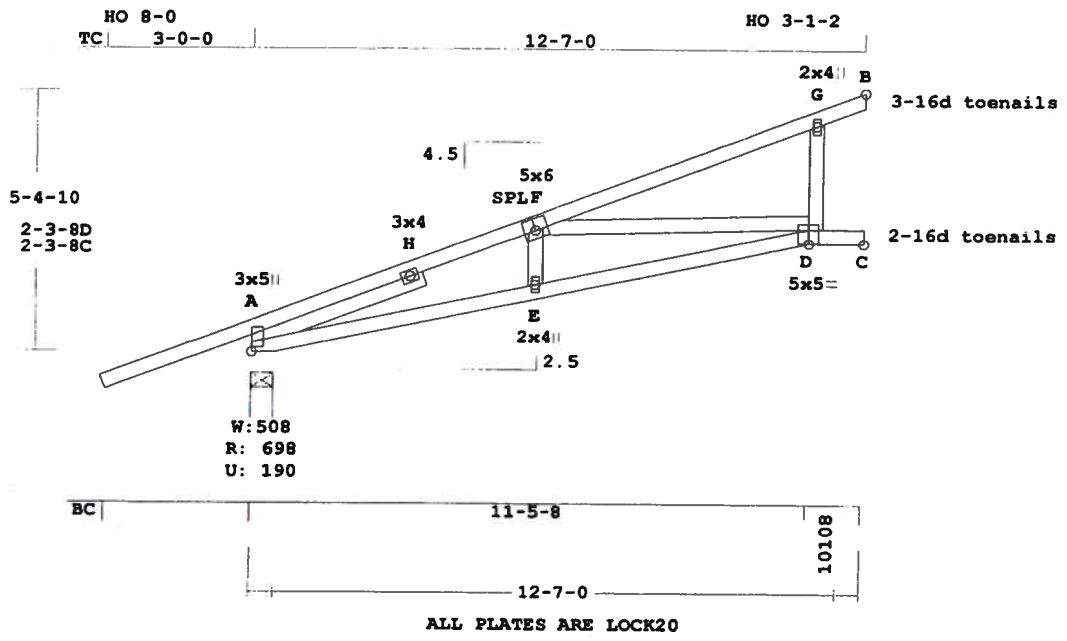
Office and Care Center, Hospice, M.M. Parrish

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	R198	4	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 79.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.56	2x 4	SP-#2
WB	0.59	2x 4	SP-#3
SL	0.12	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	698	191	5- 8	1- 0
			Hz =	212
B	299	84	3- 8	1- 0
			Hz =	114
C	208	122	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -H	0.11	712 C	0.00	0.11
H -F	0.33	1262 C	0.07	0.26
F -G	0.51	111 C	0.00	0.51
G -B	0.33	119 T	0.01	0.32
Bottom Chords				
A -E	0.31	1220 T	0.20	0.11
E -D	0.56	1211 T	0.20	0.36
D -C	0.36	0 T	0.00	0.36
Webs				
E -F	0.06	247 T		
F -D	0.59	1188 C		
D -G	0.06	242 T		
Sliders				
A -H	0.12	615 C		

LL Defl -0.11" in E -D L/999
TL Defl -0.24" in E -D L/595
Shear // Grain in G -B 0.34

Hz Disp	LL	DL	TL
Jt C	0.02"	0.02"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	5.0x 6.0	-0.2	0.5	0.87
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.78

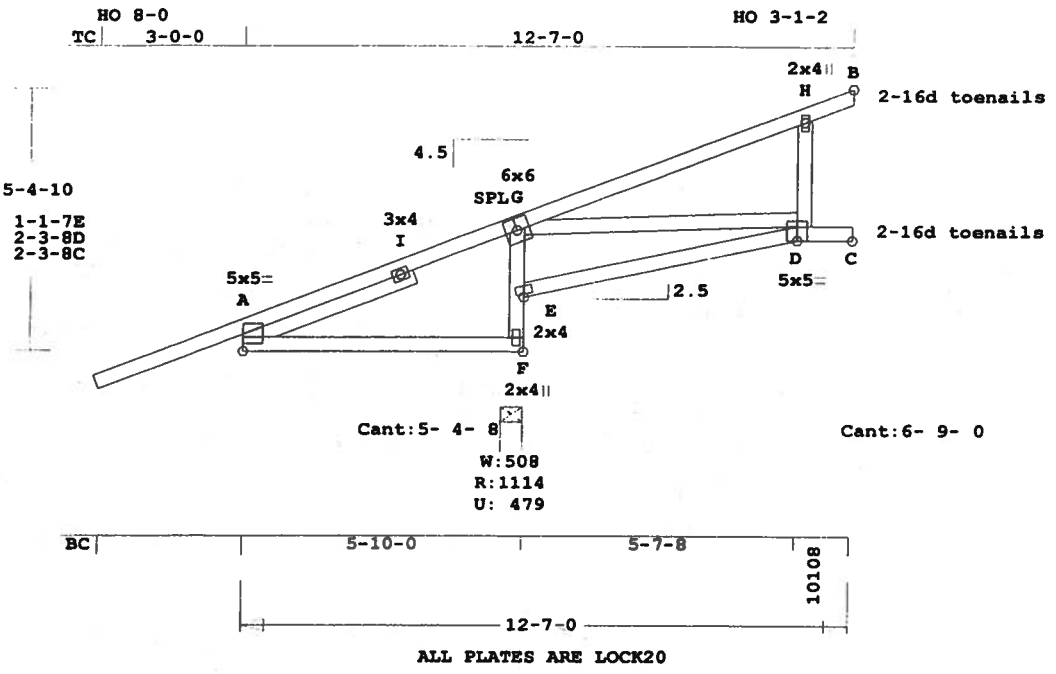
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1262 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R199	10	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.64	2x 4	SP-#2
BC	0.37	2x 4	SP-#2
CW	0.89	2x 4	SP-#2
WB	0.17	2x 4	SP-#3
SL	0.17	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
F	1115	480	5- 8	1- 4
			Hz =	213
B	72	0	3- 8	1- 0
			Hz =	114
C	77	72	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.26	194 T	0.02	0.24
I -G	0.64	487 C	0.08	0.56
G -H	0.56	125 C	0.00	0.56
H -B	0.09	130 T	0.00	0.09
--------Bottom Chords---------				
A -F	0.22	487 T	0.06	0.16
E -D	0.37	580 C	0.00	0.37
D -C	0.18	0 T	0.00	0.18
----------Chord-Webs----------				
F -E	0.89	863 C	0.00	0.89
E -G	0.07	644 C	0.07	0.00
------------Webs------------				
G -D	0.17	549 T		
D -H	0.06	269 T		
----------Sliders----------				
A -I	0.17	697 C		

LL Defl -0.01" in E -D L/999
TL Defl -0.03" in E -D L/999
LL Cant -0.11" in A -F L/612
Shear // Grain in F -E 0.51

Hz Disp	LL	DL	TL
Jt B	0.04"	0.04"	0.09"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.89
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	6.0x 6.0	Ctr	Ctr	0.87
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	2.0x 4.0	Ctr	Ctr	0.58
E	LOCK	2.0x 4.0	Ctr	Ctr	0.58
D	LOCK	5.0x 5.0	Ctr	-1.0	0.77

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

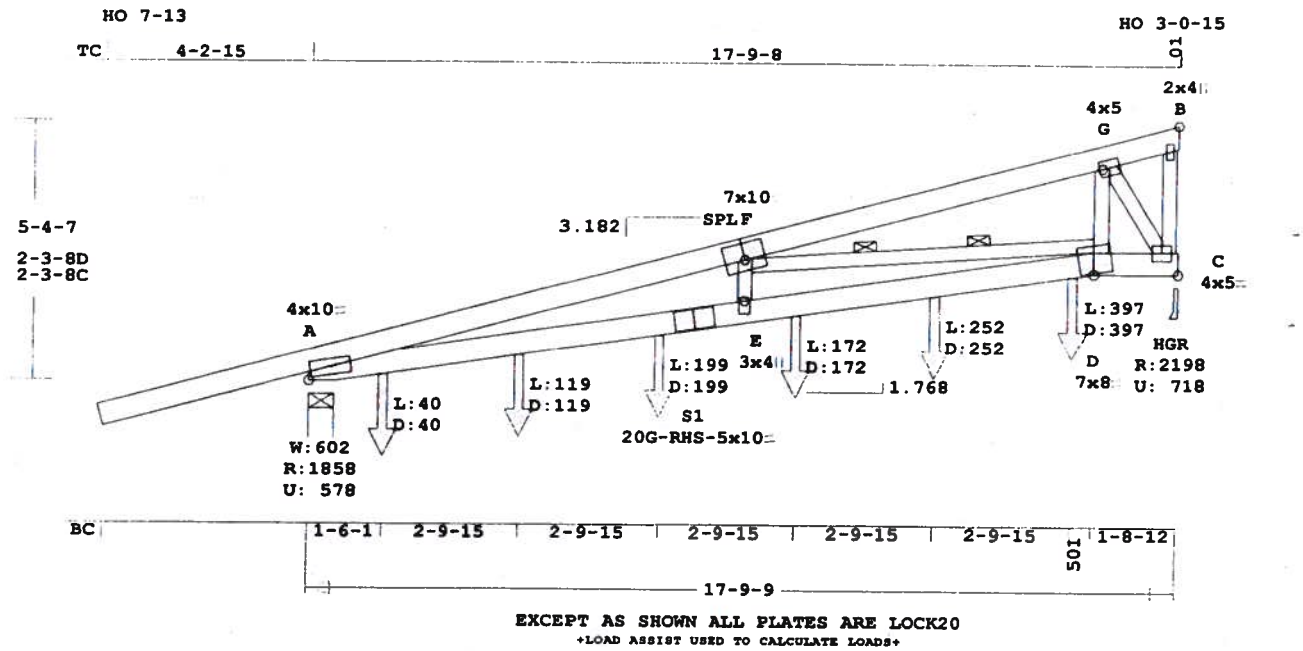
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 863 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R200*	1	SP	170909	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 142.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.89	2x 6	SP-#2
BC	0.76	2x 6	SP-2400
EX D -C		2x 6	SP-#2
WB	0.83	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17- 9- 9
BC Cont.	0- 0- 0	17- 9- 9

WB 2 rows CLB on F -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	17.8'
BC V	0	20	0.0'	17.8'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.2'	CL-LB
BC V	172	172	10.0'	CL-LB
BC V	252	252	12.8'	CL-LB
BC V	397	397	15.6'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1858	579	6- 2	1- 9
			Hz =	212
C	2199	719	3- 8	2-10
			Hz =	114

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.89	6335 C	0.19	0.70
F -G	0.62	1518 C	0.00	0.62
G -B	0.62	113 C	0.00	0.62
--------Bottom Chords---------				
A -S1	0.65	6175 T	0.31	0.34
S1-E	0.48	6231 T	0.31	0.17
E -D	0.76	6221 T	0.30	0.46
D -C	0.62	1431 T	0.19	0.43
------------Webs------------				
E -F	0.31	1159 T		
F -D	0.83	4603 C		2 Br
D -G	0.58	1841 T		
G -C	0.46	2671 C		
C -B	0.12	391 T		

LL Defl -0.35" in S1-E L/580
TL Defl -0.70" in S1-E L/289
Shear // Grain in E -D 0.94

Hz Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.09"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

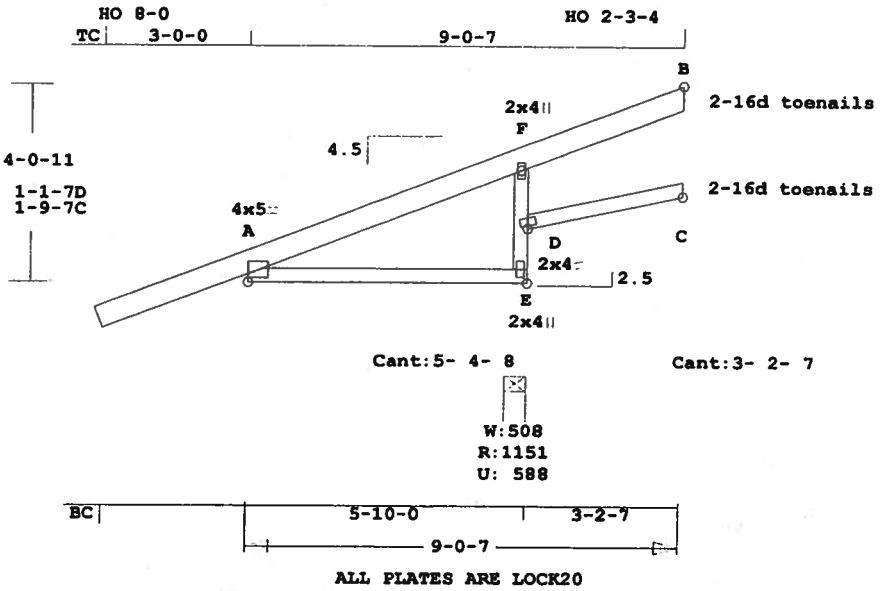
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0	5.2	1.3	0.94
F	LOCK	7.0x10.0	-0.2	0.8	0.96
G	LOCK	4.0x 5.0	Ctr	Ctr	0.88
B	LOCK	2.0x 4.0	Ctr	Ctr	0.49
S1	RHS	5.0x10.0	Ctr	Ctr	0.96
E	LOCK	3.0x 4.0	Ctr	-1.3	0.75
D	LOCK	7.0x 8.0	0.2	-1.6	0.95
C	LOCK	4.0x 5.0	Ctr	Ctr	0.62

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6335 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R202	1	SP	90007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 58.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.77	2x 6	SP-#2
BC	0.63	2x 4	SP-#2
CW	0.62	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	1152	588	5- 8	1- 6
			Hz =	153
B	73	165G	3- 8	1- 0
			Hz =	82
C	67	63G	3- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -F	0.77	356 C	0.03	0.74
F -B	0.74	96 C	0.00	0.74
Bottom Chords				
A -E	0.26	320 T	0.04	0.22
E -D	0.34	81 C	0.00	0.34
D -C	0.63	24 T	0.00	0.63
Chord-Webs				
E -D	0.62	70 C	0.00	0.62
D -F	0.22	917 T	0.22	0.00

LL Defl	0.02"	in D -C	L/999
TL Defl	0.04"	in D -C	L/819
LL Cant	-0.28"	in A -E	L/233
Shear // Grain		in F -B	0.29

Hz Disp	LL	DL	TL
Jt B	0.21"	-0.04"	0.17"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
F	LOCK	2.0x 4.0	Ctr	Ctr	0.45
E	LOCK	2.0x 4.0	Ctr	Ctr	0.58
D	LOCK	2.0x 4.0	Ctr	Ctr	0.58

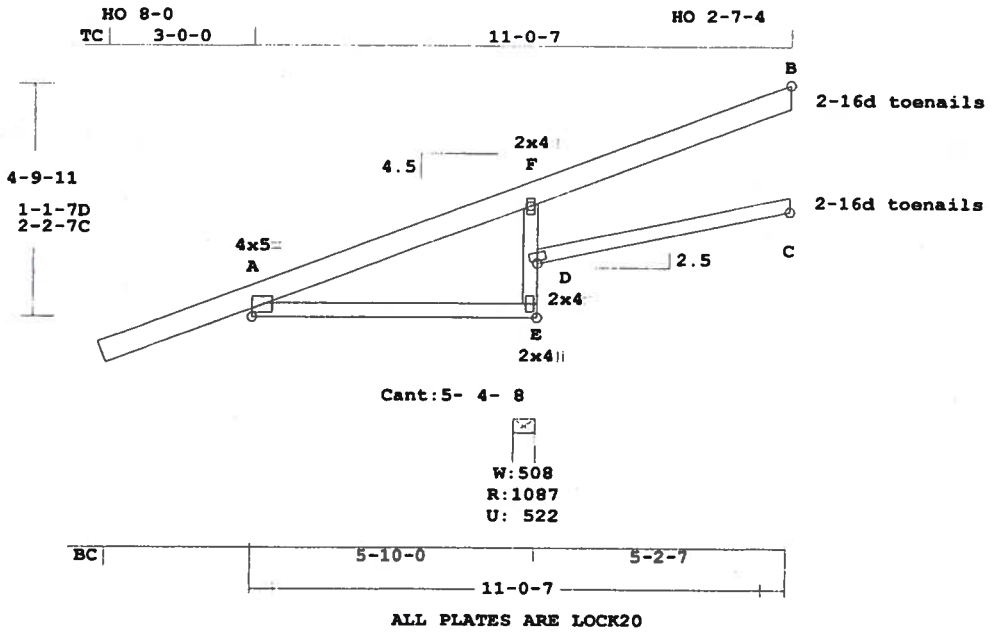
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 766 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R203*	1	SP	110007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.78	2x 6	SP-#2
BC	0.60	2x 4	SP-#2
CW	0.51	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 7
BC Cont.	0- 0- 0	11- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	1088	522	5- 8	1- 5
			Hz =	187
B	36	58G	3- 8	1- 0
C	54	3G	3- 8	1- 0
			Hz =	100

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.78	305 C	0.03	0.75
F -B	0.75	59 C	0.00	0.75
--------Bottom Chords---------				
A -E	0.21	280 T	0.03	0.18
E -D	0.28	62 C	0.00	0.28
D -C	0.60	100 T	0.00	0.60
----------Chord-Webs----------				
E -D	0.51	54 C	0.00	0.51
D -F	0.18	722 C	0.18	0.00

LL Defl 0.05" in D -C L/999
TL Defl 0.08" in D -C L/744
LL Cant -0.33" in A -E L/195
Shear // Grain in A -F 0.26

Hz Disp	LL	DL	TL
Jt B	0.22"	-0.04"	0.17"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
F	LOCK	2.0x 4.0	Ctr	Ctr	0.36
E	LOCK	2.0x 4.0	Ctr	Ctr	0.58
D	LOCK	2.0x 4.0	Ctr	Ctr	0.58

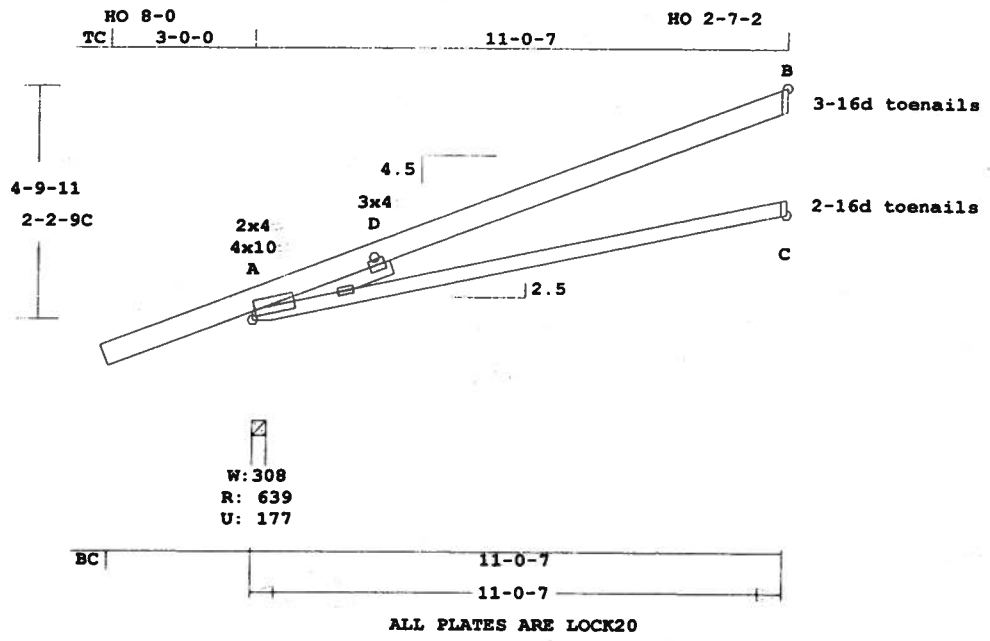
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 722 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R204	1	VJACK1	110007	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.77	2x 6	SP-#2
BC	0.50	2x 4	SP-#2
SL	0.22	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 0- 7
BC	Cont.	0- 0- 0	11- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	639	178	3- 8	1- 0
			Hz =	186
B	343	211	3- 8	1- 0
C	166	0G	3- 8	1- 0
			Hz =	100

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.44	658 C	0.00	0.44
D -B	0.77	150 C	0.00	0.77
--------Bottom Chords---------				
A -C	0.50	98 T	0.00	0.50
----------Sliders----------				
A -D	0.22	833 T		

LL Defl -0.26" in D -C L/491
TL Defl -0.62" in D -C L/201
Shear // Grain in D -D 0.47

Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0	4.0	1.3	0.97
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.52

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

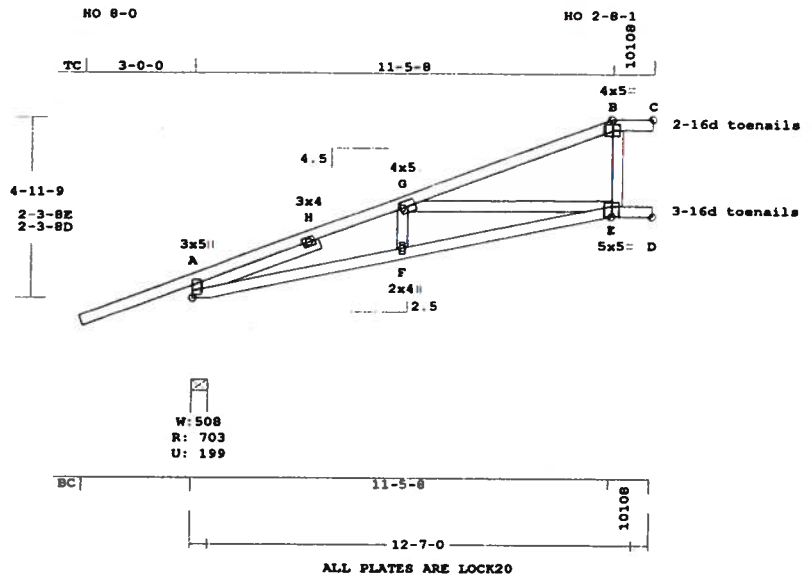
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 658 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R205*	1	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 79.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.48	2x 4	SP-#2
BC	0.81	2x 4	SP-#2
WB	0.63	2x 4	SP-#3
SL	0.13	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	704	200	5- 8	1- 0
			Hz = 194	
C	163	33	3- 8	1- 0
			Hz = 104	
D	338	165	3- 8	1- 0

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -H	0.14	695 C	0.00	0.14	
H -G	0.34	1311 C	0.09	0.25	
G -B	0.48	84 T	0.00	0.48	
B -C	0.26	104 T	0.00	0.26	
Bottom Chords					
A -F	0.39	1264 T	0.21	0.18	
F -E	0.81	1247 T	0.21	0.60	
E -D	0.60	0 T	0.00	0.60	
Webs					
F -G	0.07	278 T			
G -E	0.63	1237 C			
E -B	0.05	207 T			
Sliders					
A -H	0.13	681 C			

LL Defl -0.15" in F -E L/927
TL Defl -0.33" in F -E L/432
Shear // Grain in E -D 0.43

Hz Disp	LL	DL	TL
Jt C	0.03"	0.03"	0.06"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	4.0x 5.0	Ctr	Ctr	0.72
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	5.0x 5.0	Ctr	-1.0	0.59

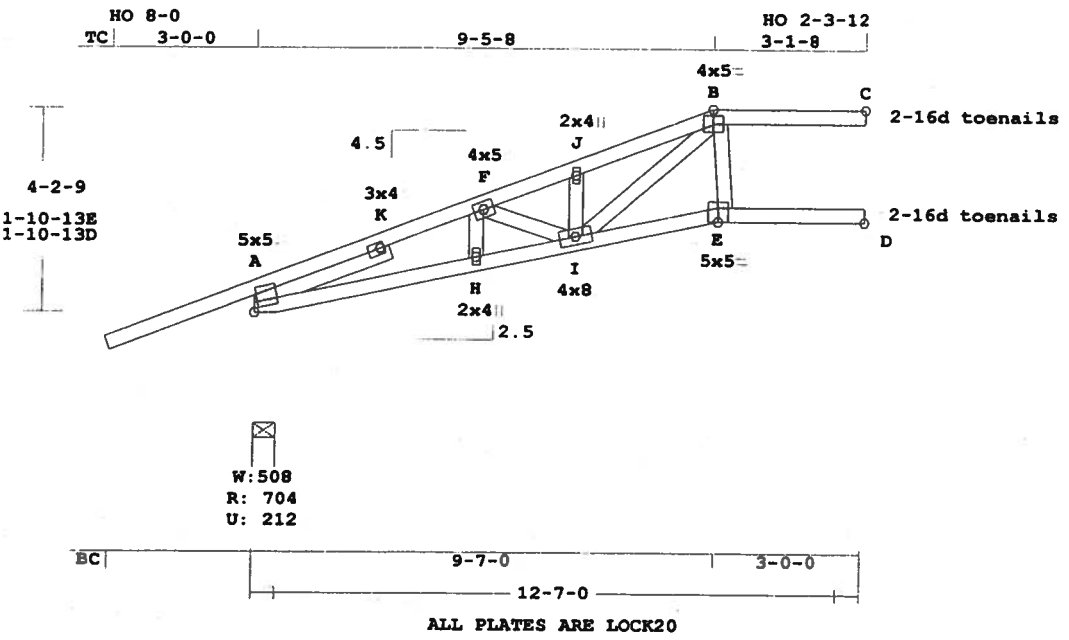
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1311 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R206	1	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 79.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.85	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.48	2x 4	SP-#3
SL	0.09	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	705	213	5- 8	1- 0
		Hz =	160	
C	242	110	3- 8	1- 0
		Hz =	86	
D	259	75	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -K	0.06	686 C	0.00	0.06
K -F	0.12	1173 C	0.08	0.04
F -J	0.26	1189 C	0.10	0.16
J -B	0.85	1292 C	0.01	0.84
B -C	0.84	85 T	0.00	0.84
--------Bottom Chords--------				
A -H	0.26	1105 T	0.18	0.08
H -I	0.44	1127 T	0.19	0.25
I -E	0.76	26 C	0.00	0.76
E -D	0.76	0 T	0.00	0.76
----------Webs----------				
H -F	0.02	104 T		
F -I	0.01	77 C		
I -J	0.10	438 C		
I -B	0.48	1520 T		
B -E	0.10	428 C		
----------Sliders----------				
A -K	0.09	547 C		

LL Defl -0.27" in I -E L/524
TL Defl -0.55" in I -E L/260
Shear // Grain in J -B 0.35

Hz Disp	LL	DL	TL
Jt C	0.11"	0.11"	0.22"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	1.00
K	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	2.0x 4.0	Ctr	Ctr	0.38
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
I	LOCK	4.0x 8.0	Ctr	Ctr	0.67
E	LOCK	5.0x 5.0	Ctr	-1.0	0.44

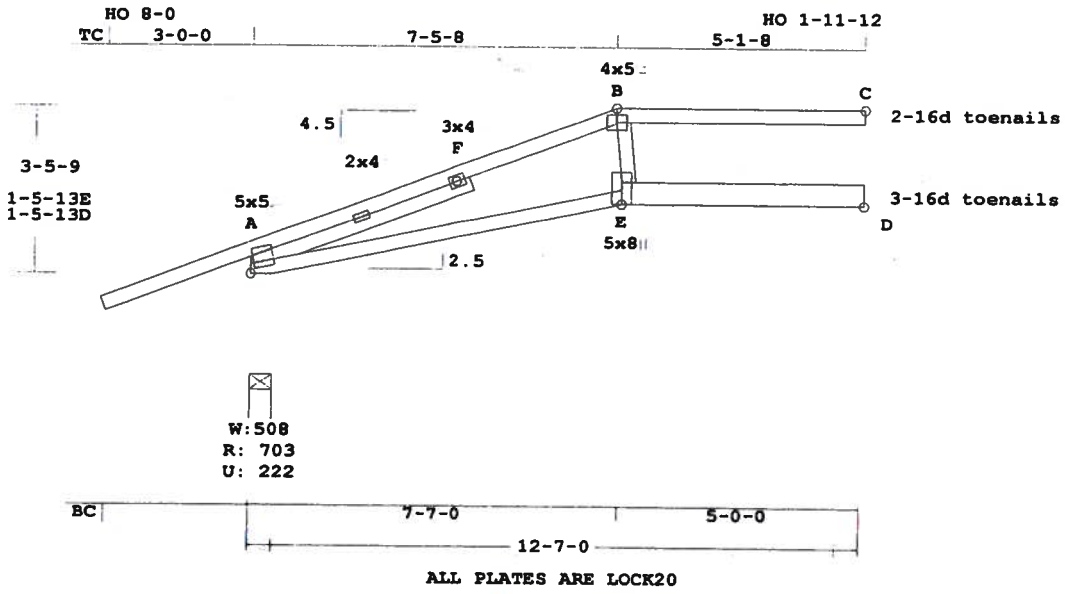
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1292 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R207	1	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 71.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 4	SP-#2
BC	0.00	2x 4	SP-#2
EX E -D		2x 6	SP-2400
WB	0.10	2x 4	SP-#3
SL	0.15	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	704	222	5- 8	1- 0
			Hz =	126
C	165	86	3- 8	1- 0
			Hz =	68
D	337	89	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.71	549 C	0.00	0.71
F -B	0.71	66 C	0.00	0.71
B -C	0.41	67 T	0.00	0.41
--------Bottom Chords---------				
A -E	0.66	20 C	0.00	0.66
E -D	0.66	0 T	0.00	0.66
------------Webs------------				
B -E	0.10	413 T		
----------Sliders----------				
A -F	0.15	506 T		

LL Defl -0.38" in A -E L/376
TL Defl -0.76" in A -E L/186
Shear // Grain in A -F 0.33

Hz Disp	LL	DL	TL
Jt C	0.21"	0.21"	0.43"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	1.00
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.71
E	LOCK	5.0x 8.0	Ctr	0.4	0.42

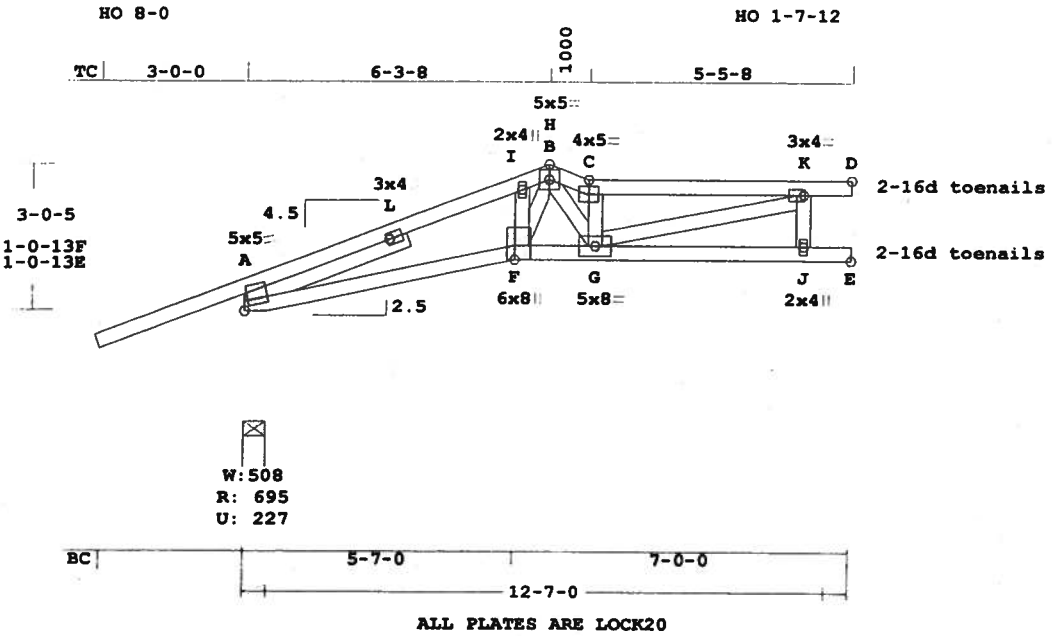
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 549 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R208	1	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.36	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.38	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	696	227	5- 8	1- 0
			Hz =	93
D	226	52	3- 8	1- 0
			Hz =	49
E	283	118	3- 8	1- 0

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.15	687	C	0.05	0.10
L -I	0.20	1130	C	0.11	0.09
I -H	0.21	1136	C	0.12	0.09
H -C	0.12	1223	C	0.11	0.01
C -K	0.36	1155	C	0.01	0.35
K -D	0.23	48	T	0.00	0.23
--------Bottom Chords---------					
A -F	0.25	1084	T	0.18	0.07
F -G	0.24	877	T	0.14	0.10
G -J	0.36	0	T	0.00	0.36
J -E	0.36	0	T	0.00	0.36
----------Webs----------					
F -I	0.06	252	T		
F -H	0.13	424	C		
H -G	0.15	488	T		
G -C	0.12	614	C		
G -K	0.38	1210	T		
J -K	0.07	290	T		
----------Sliders----------					
A -L	0.11	506	C		

LL Defl -0.06" in G -J L/999
TL Defl -0.13" in G -J L/999
Shear // Grain in J -E 0.36

Hz Disp	LL	DL	TL
Jt E	0.01"	0.02"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	0.99
L	LOCK	3.0x 4.0	Ctr	Ctr	0.50
I	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	5.0x 5.0	Ctr	Ctr	0.44
C	LOCK	4.0x 5.0	Ctr	Ctr	0.70
K	LOCK	3.0x 4.0	Ctr	Ctr	0.92
F	LOCK	6.0x 8.0	1.0	0.5	0.66
G	LOCK	5.0x 8.0	Ctr	Ctr	1.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.38

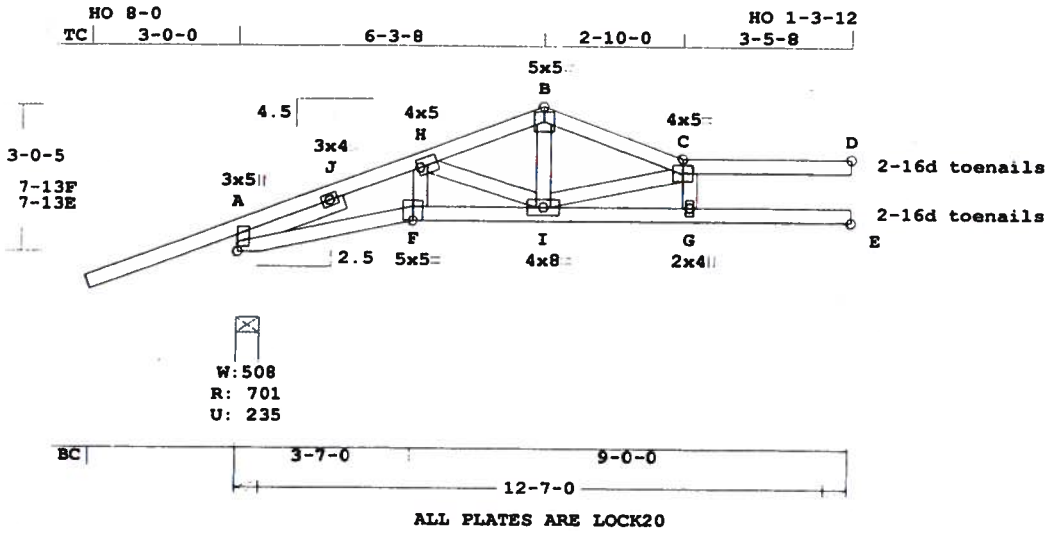
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 0-12. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1223 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R209	1	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 76.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.82	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
EX F -G		2x 4	SP-2400
WB	0.26	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	12- 7- 0
BC Cont.		0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	701	235	5- 8	1- 0
			Hz =	59
D	236	101	3- 8	1- 0
			Hz =	29
E	268	61	3- 8	1- 0

Membr	CSI	P Lbs	Axl	CSI-Bnd
**----------Top Chords----------				
A -J	0.14	696 C	0.06	0.08
J -H	0.19	1153 C	0.11	0.08
H -B	0.19	890 C	0.09	0.10
B -C	0.82	843 C	0.00	0.82
C -D	0.82	29 T	0.00	0.82
**--------Bottom Chords---------				
A -F	0.29	1070 T	0.17	0.12
F -I	0.26	1055 T	0.07	0.19
I -G	0.83	66 T	0.00	0.83
G -E	0.83	0 T	0.00	0.83
**----------Webs----------				
F -H	0.05	185 T		
H -I	0.05	220 C		
I -B	0.07	222 T		
I -C	0.26	836 T		
G -C	0.11	467 C		
**----------Sliders----------				
A -J	0.10	532 C		

LL Defl -0.29" in I -G L/490
TL Defl -0.58" in I -G L/244
Shear // Grain in I -G 0.38

Hz Disp	LL	DL	TL
Jt D	0.06"	0.06"	0.13"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.73
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.41
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
F	LOCK	5.0x 5.0	Ctr	-1.0	0.46
I	LOCK	4.0x 8.0	Ctr	Ctr	0.79
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

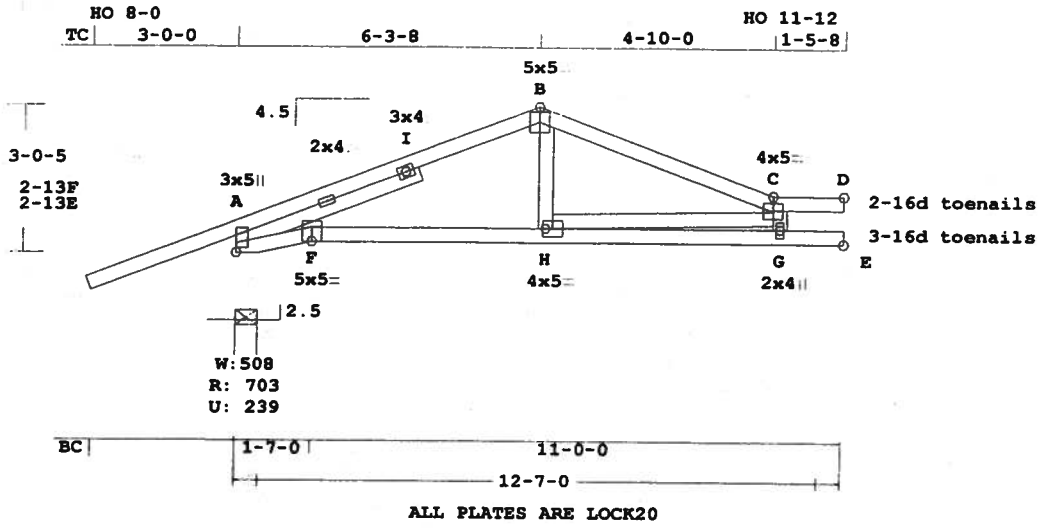
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 0-12. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1153 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R210	1	SP	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 76.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
BC	0.71	2x 4	SP-#2
WB	0.21	2x 4	SP-#3
SL	0.01	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	704	239	5- 8	1- 0
			Hz =	25
D	170	43	3- 8	1- 0
E	332	115	3- 8	1- 0

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -I	0.28	817	C	0.08 0.20
I -B	0.28	747	C	0.08 0.20
B -C	0.44	760	C	0.00 0.44
C -D	0.33	0	T	0.00 0.33
Bottom Chords				
A -F	0.23	713	T	0.07 0.16
F -H	0.42	731	T	0.12 0.30
H -G	0.71	88	C	0.01 0.70
G -E	0.70	9	T	0.00 0.70
Webs				
H -B	0.06	244	T	
H -C	0.21	660	T	
G -C	0.10	424	T	
Sliders				
A -I	0.01	75	T	

LL Defl -0.11" in H -G L/999
TL Defl -0.24" in H -G L/590
Shear // Grain in G -E 0.42

Hz Disp	LL	DL	TL
Jt D	0.02"	0.02"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.46
C	LOCK	4.0x 5.0	Ctr	Ctr	0.72
F	LOCK	5.0x 5.0	Ctr	0.8	0.41
H	LOCK	4.0x 5.0	Ctr	Ctr	0.75
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38

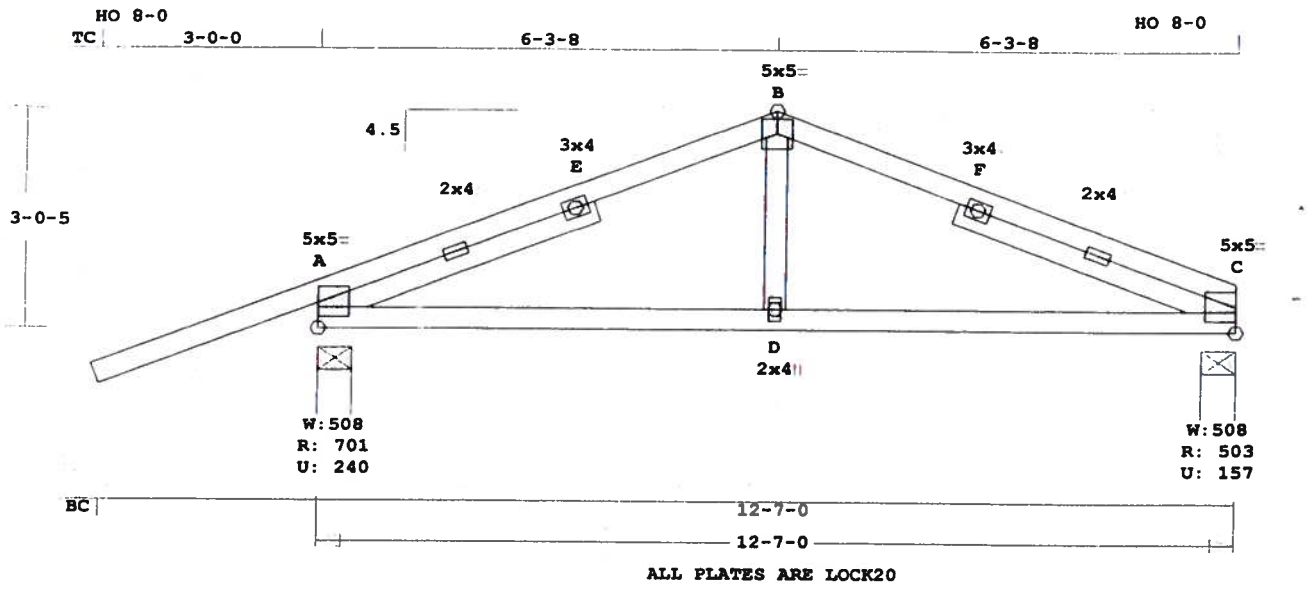
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 0-12. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 817 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R211*	1	TR	120700	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 75.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.25	2x 4	SP-#2
BC	0.28	2x 4	SP-#2
WB	0.06	2x 4	SP-#3
SL	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	702	240	5- 8	1- 0
C	503	157	5- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.22	500 C	0.04	0.18
E -B	0.25	649 C	0.06	0.19
B -F	0.25	649 C	0.06	0.19
F -C	0.22	500 C	0.04	0.18
--------Bottom Chords---------				
A -D	0.28	632 T	0.10	0.18
D -C	0.28	632 T	0.10	0.18
----------Webs----------				
D -B	0.06	249 T		
----------Sliders----------				
A -E	0.05	211 T		
F -C	0.05	211 T		

LL Defl -0.02" in D -C L/999
TL Defl -0.04" in D -C L/999
Shear // Grain in E -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.89
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.45
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	5.0x 5.0-2.5	0.5		0.89
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

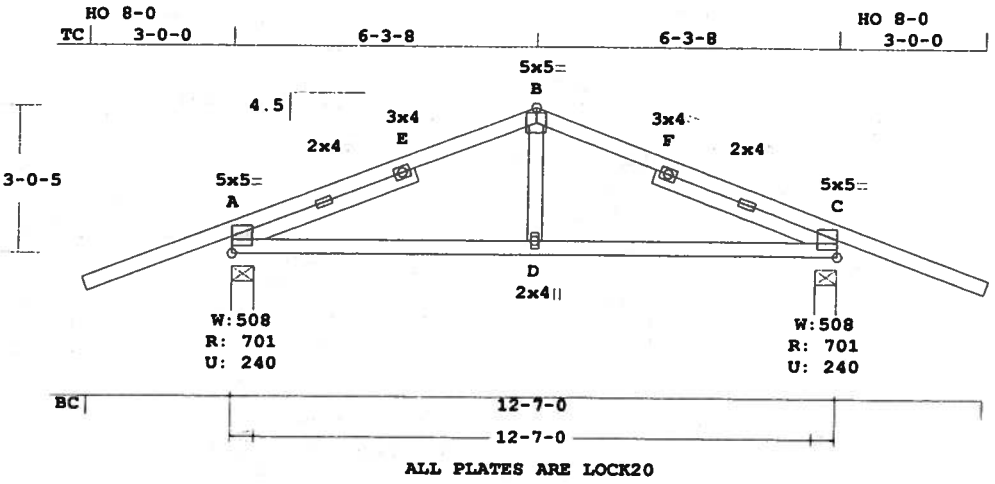
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
3- 0-12. Support end or
provide level return.

Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 649 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R212	2	TR	120700	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.25	2x 4	SP-#2
BC	0.28	2x 4	SP-#2
WB	0.06	2x 4	SP-#3
SL	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	702	240	5- 8	1- 0
C	702	240	5- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.22	500 C	0.04	0.18
E -B	0.25	649 C	0.06	0.19
B -F	0.25	649 C	0.06	0.19
F -C	0.22	500 C	0.04	0.18
--------Bottom Chords---------				
A -D	0.28	632 T	0.10	0.18
D -C	0.28	632 T	0.10	0.18
------------Webs------------				
D -B	0.06	249 T		
----------Sliders----------				
A -E	0.05	211 T		
F -C	0.05	211 T		

LL Defl -0.02" in D -C L/999
TL Defl -0.04" in D -C L/999
Shear // Grain in E -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

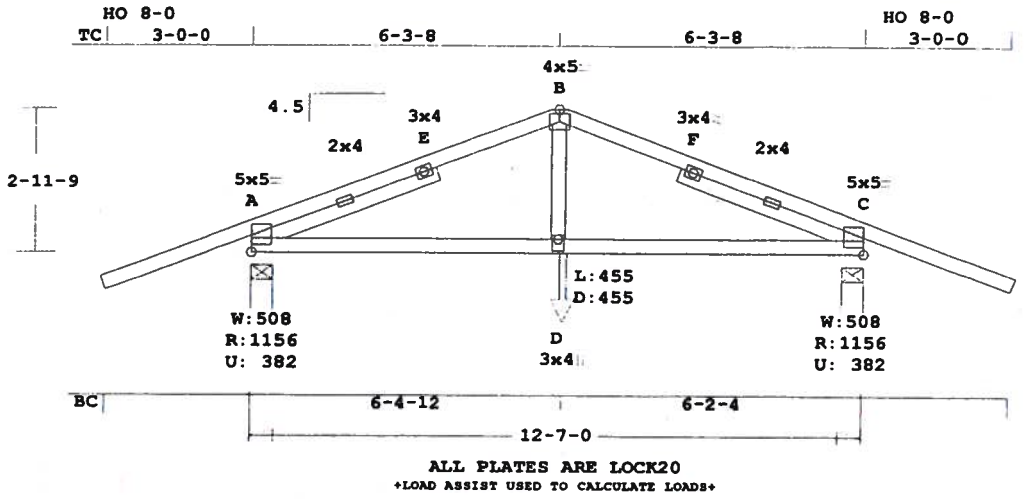
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.89
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	Ctr	0.45
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	5.0x 5.0-2.5	0.5	0.89	
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 0-12. Support end or provide level return.
Excessive Right OH condition.
Max allowable length is 3- 0-12. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 649 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R213*	1	TR	120700	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 4	SP-#2
BC	0.46	2x 4	SP-#2
WB	0.32	2x 4	SP-#3
SL	0.19	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	12.6'
BC V	0	20	0.0'	12.6'
BC V	455	455	6.4'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1157	382	5- 8	1- 6
C	1157	382	5- 8	1- 6

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -E	0.24	946 C	0.09	0.15
E -B	0.30	1713 C	0.16	0.14
B -F	0.30	1713 C	0.16	0.14
F -C	0.24	946 C	0.09	0.15
Bottom Chords				
A -D	0.46	1607 T	0.22	0.24
D -C	0.46	1607 T	0.22	0.24
Webs				
D -B	0.32	1110 T		
Sliders				
A -E	0.19	837 C		
F -C	0.19	837 C		

LL Defl -0.03" in A -D L/999
TL Defl -0.06" in A -D L/999
Shear // Grain in A -E 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.89
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.52
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	5.0x 5.0	-2.5	0.5	0.89
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	-0.9	0.80

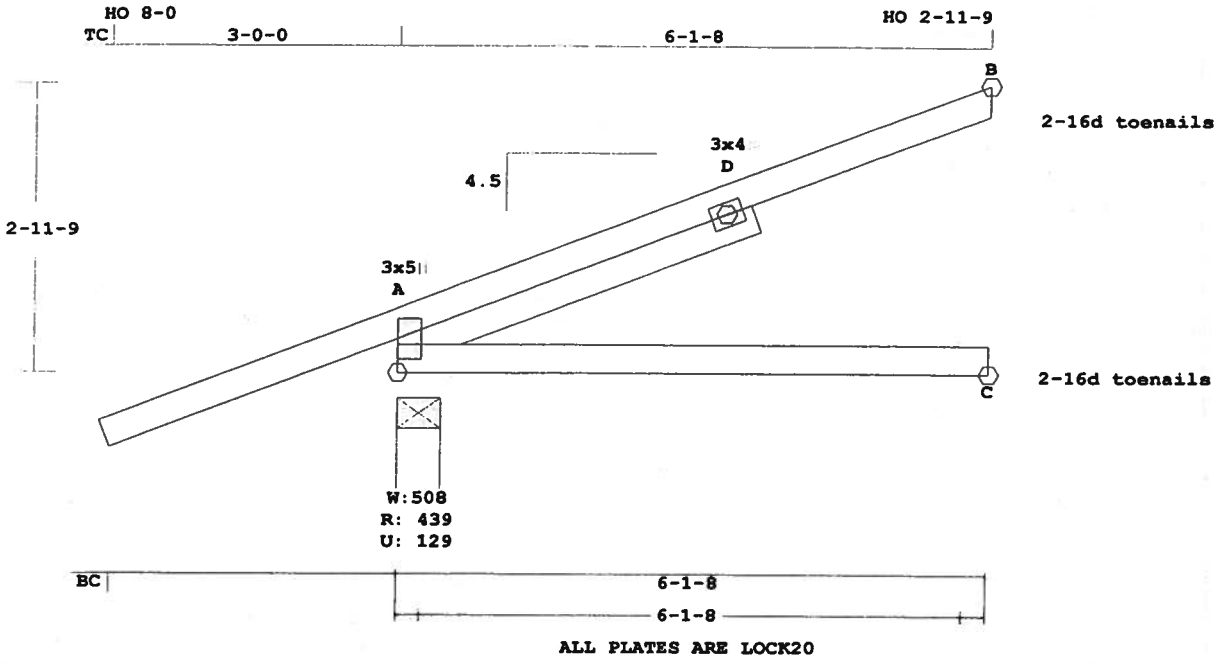
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is
3- 0-12. Support end or
provide level return.
Excessive Right OH condition.
Max allowable length is
3- 0-12. Support end or
provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1713 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R214	2	JACK	60108	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
SL	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 1- 8
BC Cont.	0- 0- 0	6- 1- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	439	130	5- 8	1- 0
			Hz =	103
B	182	120	3- 8	1- 0
C	103	0	3- 8	1- 0
			Hz =	56

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.43	316 C	0.01 0.42
D -B	0.42	110 C	0.00 0.42
--------Bottom Chords---------			
A -C	0.29	55 T	0.00 0.29
----------Sliders------------			
A -D	0.08	321 T	

LL Defl -0.03" in A -C L/999
TL Defl -0.08" in A -C L/851
Shear // Grain in D -B 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.67
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

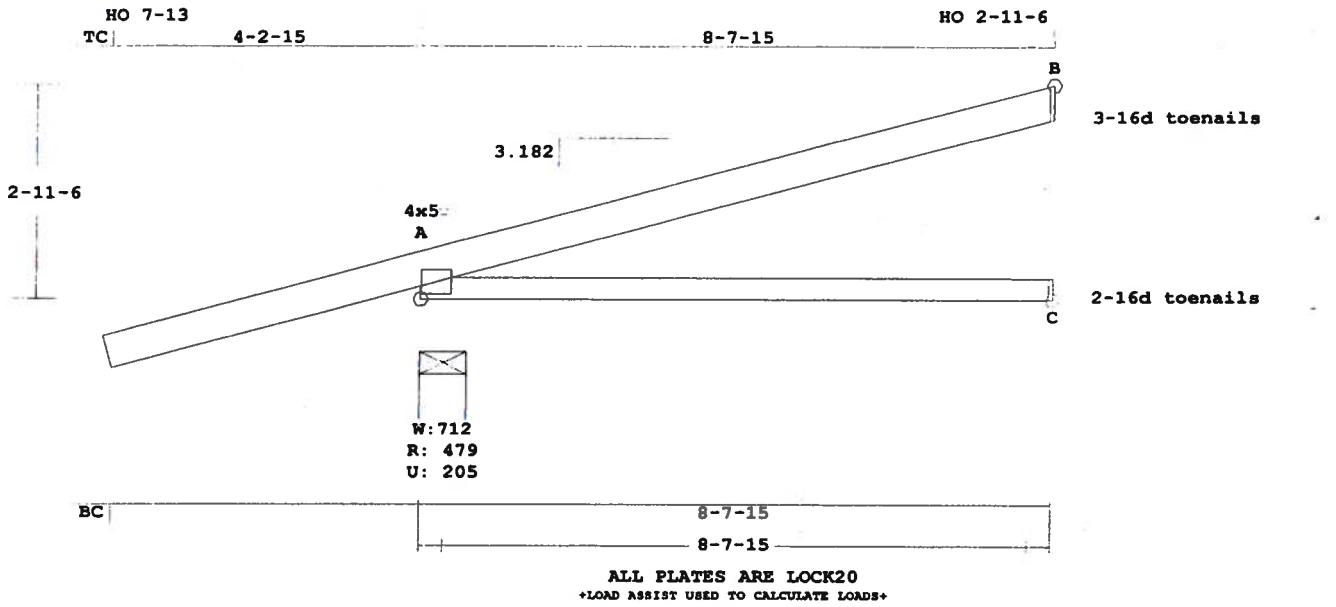
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 316 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R215*	2	KING	80715	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 54.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.48	2x 6	SP-#2
BC	0.42	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 7-15
BC Cont.	0- 0- 0	8- 7-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	8.7'
BC V	0	20	0.0'	8.7'
TC V	-40	-20	0.0'	
	32	16		8.7'
BC V	0	-20	0.0'	
	0	16		8.7'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd	Lbs	Lbs	In-Sx	In-Sx
A	479	206	7-12	1- 0			
			Hz =	94			
B	326	202	3- 8	1- 0			
C	186	0	3- 8	1- 0			
			Hz =	50			

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.48	113 C	0.00 0.48
--------Bottom Chords---------			
A -C	0.42	50 T	0.00 0.42

LL Defl -0.13" in A -C L/724
TL Defl -0.29" in A -C L/325
Shear // Grain in A -B 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71

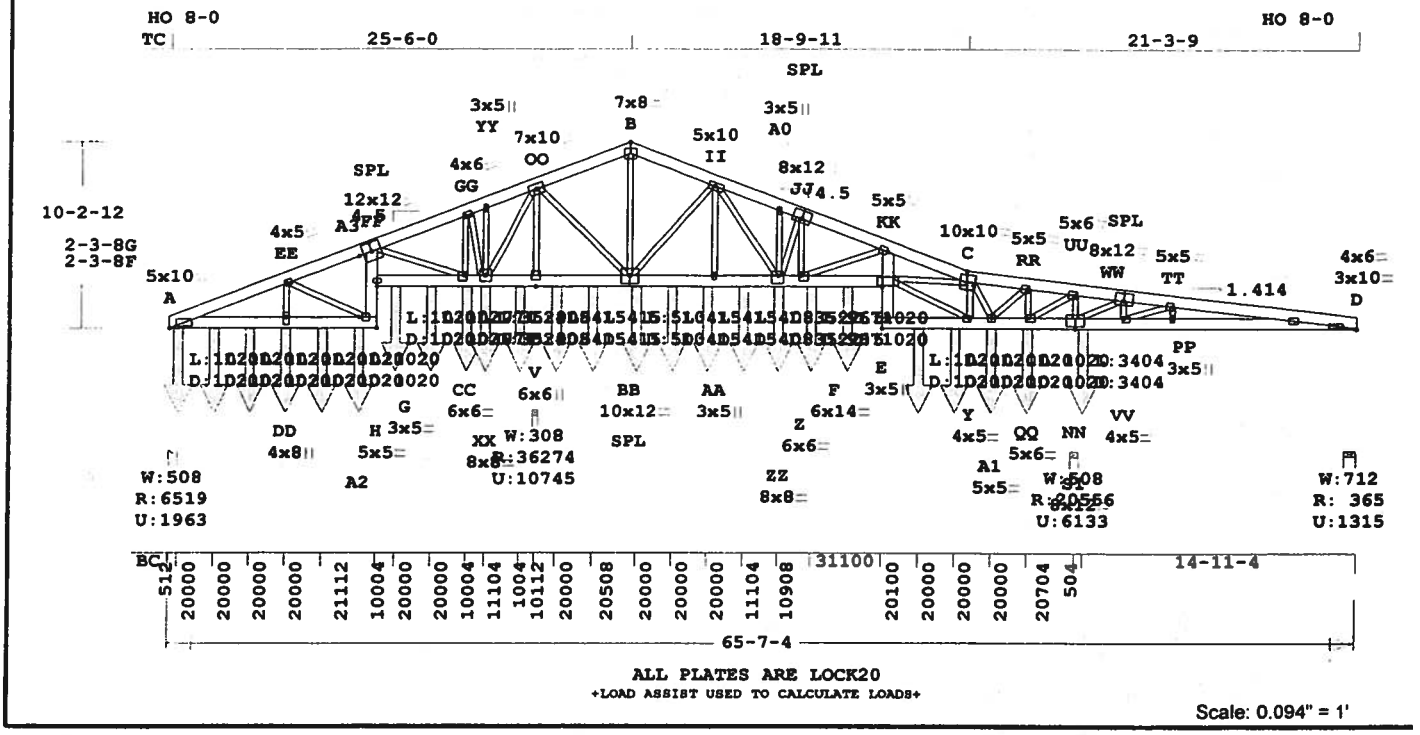
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 6- 1- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 113 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R216	1*4P	SP	650704	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 737.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 4-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.29	2x 8	SP-2400
BC	0.40	2x 8	SP-2400
CW	0.39	2x 8	SP-2400
WB	0.99	2x 4	SP-#3
EX	V -OO	2x 4	SP-#2
EX	OO-BB	2x 4	SP-#2
EX	II-ZZ	2x 4	SP-#2
EX	QQ-UU	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	65- 7- 4
BC Cont.	0- 0- 0	65- 7- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	65.6'
BC V 0	20	0.0'	65.6'
BC V 1020	1020	0.5'	CL-LB
BC V 1020	1020	12.5'	CL-LB
BC V 1020	1020	14.5'	CL-LB
BC V 771	771	16.5'	CL-LB
BC V 3530	3530	17.5'	CL-LB
BC V 408	408	19.4'	CL-LB
BC V 415	415	21.4'	CL-LB
BC V 415	415	23.4'	CL-LB
BC V 510	510	25.9'	CL-LB
BC V 415	415	27.9'	CL-LB
BC V 415	415	29.9'	CL-LB
BC V 408	408	31.9'	CL-LB
BC V 3523	3523	33.8'	CL-LB
BC V 967	967	35.6'	CL-LB
BC V 1020	1020	37.6'	CL-LB
BC V 1020	1020	41.6'	CL-LB
BC V 1020	1020	43.6'	CL-LB
BC V 3404	3404	50.7'	CL-LB
BC V 1020	1020	45.6'	CL-LB
BC V 1020	1020	47.6'	CL-LB
BC V 1020	1020	10.5'	CL-LB
BC V 1020	1020	8.5'	CL-LB
BC V 1020	1020	6.5'	CL-LB
BC V 1020	1020	4.5'	CL-LB
BC V 1020	1020	2.5'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	6519	1964	5- 8	1- 6
			Hz =	-168
V	36274	10745	3- 8	6-13 **
S1	20556	6134	5- 8	4- 0
D	365	1315G	7-12	1- 0
			Hz =	179

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -EE	0.10	7278 C	0.00 0.10
EE-FF	0.01	722 T	0.00 0.01
FF-GG	0.13	13622 T	0.11 0.02
GG-YY	0.14	16264 T	0.14 0.00
YY-OO	0.29	16637 T	0.14 0.15
OO-B	0.20	6741 T	0.05 0.15
B -II	0.08	6856 T	0.05 0.03
II-A0	0.04	11052 C	0.00 0.04
A0-JJ	0.04	11424 C	0.00 0.04
JJ-KK	0.22	13381 C	0.01 0.21
KK-C	0.23	19536 C	0.02 0.21
C -RR	0.08	6835 C	0.00 0.08
RR-UU	0.10	779 T	0.00 0.10
UU-WW	0.22	14132 T	0.12 0.10
WW-TT	0.20	13130 T	0.11 0.09
TT-D	0.18	10785 T	0.09 0.09
Bottom Chords			
A -DD	0.22	6947 T	0.06 0.16
DD-H	0.19	6947 T	0.06 0.13
G -CC	0.18	758 C	0.00 0.18
CC-XX	0.10	12737 C	0.01 0.09
XX-V	0.40	21046 C	0.02 0.38
V -BB	0.40	21046 C	0.02 0.38
BB-AA	0.09	1985 T	0.01 0.08
AA-ZZ	0.07	1985 T	0.01 0.06
ZZ-Z	0.16	12762 T	0.11 0.05
Z -F	0.40	19643 T	0.17 0.23
E -Y	0.17	2190 T	0.01 0.16
Y -A1	0.16	9786 T	0.08 0.08
A1-QQ	0.06	829 C	0.00 0.06
QQ-S1	0.23	13998 C	0.01 0.22
S1-VV	0.23	13070 C	0.01 0.22
VV-PP	0.09	10684 C	0.00 0.09
PP-D	0.09	10684 C	0.00 0.09
Chord-Webs			
H -G	0.18	6280 T	0.06 0.12
G -FF	0.08	8825 T	0.08 0.00
E -F	0.39	1447 T	0.01 0.38
F -KK	0.05	6265 T	0.05 0.00
Webs			
DD-EE	0.49	6242 T	
EE-H	0.35	8642 C	
FF-CC	0.53	12779 C	
CC-GG	0.73	9225 T	
GG-XX	0.36	9060 C	
XX-YY	0.07	950 T	
XX-OO	0.99	12463 T	
V -OO	0.78	30955 C	
OO-BB	0.96	20917 T	
BB-B	0.20	4537 C	
BB-II	0.57	12944 C	
AA-II	0.18	2368 T	
II-ZZ	0.72	15620 T	
ZZ-A0	0.07	955 T	
ZZ-JJ	0.30	7547 C	
Z -JJ	0.51	6438 T	
Z -KK	0.30	7416 C	
F -C	0.65	8156 T	
F -Y	0.68	8492 T	
Y -C	0.03	815 C	
C -A1	0.25	6457 C	
A1-RR	0.89	11158 T	
QQ-RR	0.36	9382 C	
QQ-UU	0.74	16217 T	
S1-UU	0.46	11976 C	
S1-WW	0.04	1074 C	
VV-WW	0.08	1073 T	
VV-TT	0.10	2675 C	
PP-TT	0.05	672 T	

LL Defl -0.23" in E -F L/999
TL Defl -0.47" in E -F L/762
Shear // Grain in S1-VV 0.96

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.96
EE	LOCK	4.0x 5.0-0.5	0.2	0.76	
FF	LOCK	12.0x12.0	Ctr	Ctr	0.91
GG	LOCK	4.0x 6.0-0.5	0.2	0.95	
YY	LOCK	3.0x 5.0	Ctr	Ctr	0.42
OO	LOCK	7.0x10.0	Ctr	Ctr	0.97
B	LOCK	7.0x 8.0	Ctr	Ctr	0.84
II	LOCK	5.0x10.0	0.5-0.2	0.91	
A0	LOCK	3.0x 5.0	Ctr	Ctr	0.42

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R216*	1*4P	SP	650704	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish

JJ	LOCK	8.0x12.0	0.6	1.5	0.97
KK	LOCK	5.0x 5.0	0.5-0.2		0.58
C	LOCK	10.0x10.0	Ctr	Ctr	0.80
RR	LOCK	5.0x 5.0	Ctr	Ctr	0.89
UU	LOCK	5.0x 6.0	Ctr	Ctr	0.96
WW	LOCK	8.0x12.0	0.2	1.5	0.99
TT	LOCK	5.0x 5.0	Ctr	Ctr	0.60
D	LOCK	3.0x10.0	Ctr	Ctr	0.95
D	LOCK	4.0x 6.0	Ctr	Ctr	0.21
DD	LOCK	4.0x 8.0	Ctr-0.9		0.84
H	LOCK	5.0x 5.0	Ctr	Ctr	0.96
G	LOCK	3.0x 5.0	Ctr	Ctr	0.96
CC	LOCK	6.0x 6.0	Ctr-0.6		0.86
XX	LOCK	8.0x 8.0	Ctr	Ctr	0.97
V	LOCK	6.0x 6.0	Ctr	Ctr	0.85
BB	LOCK	10.0x12.0	Ctr-0.5		0.99
AA	LOCK	3.0x 5.0	Ctr	Ctr	0.42
ZZ	LOCK	8.0x 8.0-1.0-1.2			0.97
Z	LOCK	6.0x 6.0	Ctr-0.6		0.60
F	LOCK	6.0x14.0	Ctr	Ctr	0.98
E	LOCK	3.0x 5.0	Ctr	Ctr	0.96
Y	LOCK	4.0x 5.0	Ctr-0.5		0.99
A1	LOCK	5.0x 5.0	Ctr	Ctr	0.93
QQ	LOCK	5.0x 6.0	Ctr	Ctr	0.98
S1	LOCK	8.0x12.0	Ctr-1.5		0.99
VV	LOCK	4.0x 5.0	Ctr-0.5		0.72
PP	LOCK	3.0x 5.0	Ctr	Ctr	0.42

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

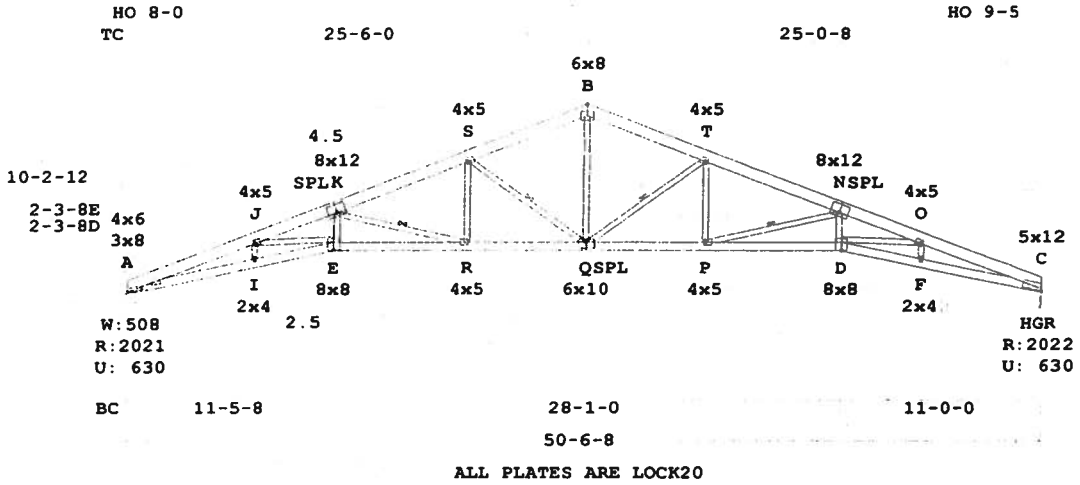
		----Spacing (In)----		
	Rows	Nails	Screws	Bolts
TC	2	12	24	24
BC	2	12	24	24
WB	1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 30955 Lbs
Quality Control Factor 1.25
5. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R217*	1	SP	500608	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 462.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 6- 8
BC Cont.	0- 0- 0	50- 6- 8

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2022	631	5- 8	1-11
			Hz =	-175
C	2022	631	3- 8	1-11
			Hz =	177

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -J	0.28	8458 C	0.08 0.20
J -K	0.30	8612 C	0.09 0.21
K -S	0.17	5176 C	0.03 0.14
S -B	0.09	3691 C	0.04 0.05
B -T	0.09	3691 C	0.04 0.05
T -N	0.18	5121 C	0.03 0.15
N -O	0.30	8370 C	0.08 0.22
O -C	0.27	7939 C	0.07 0.20
Bottom Chords			
A -I	0.54	7989 T	0.36 0.18
I -E	0.54	8025 T	0.36 0.18
E -R	0.39	8019 T	0.36 0.03
R -Q	0.27	4882 T	0.22 0.05
Q -P	0.27	4830 T	0.22 0.05
P -D	0.40	7799 T	0.35 0.05
D -F	0.52	7500 T	0.34 0.18
F -C	0.52	7457 T	0.34 0.18
Webs			
I -J	0.03	133 T	
J -E	0.07	245 T	
E -K	0.58	1810 T	
K -R	0.84	3273 C	1 Br
R -S	0.33	1054 T	
S -Q	0.50	1794 C	1 Br
Q -B	0.72	2264 T	
Q -T	0.48	1730 C	1 Br
P -T	0.31	995 T	
P -N	0.79	3097 C	1 Br
D -N	0.54	1691 T	
D -O	0.17	532 T	
F -O	0.03	147 T	

LL Defl -0.66" in R -Q L/903
TL Defl -1.33" in R -Q L/448
Shear // Grain in O -C 0.25

Hz Disp	LL	DL	TL
Jt C	0.41"	0.41"	0.82"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

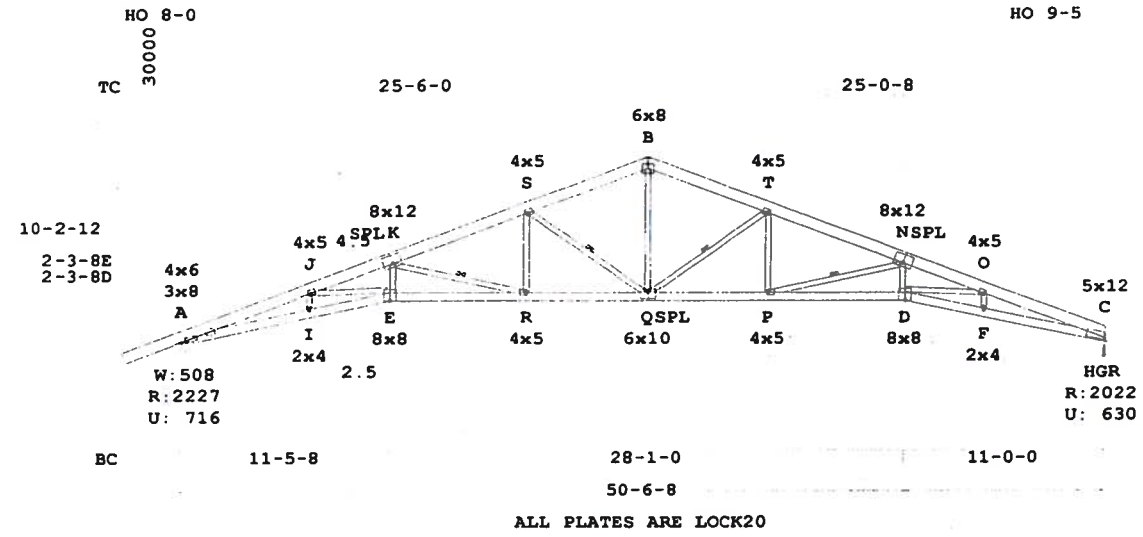
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.80
A	LOCK	4.0x 6.0	Ctr	Ctr	0.74
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	5.0x12.0	-6.4	2.4	0.99
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.95
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.90
D	LOCK	8.0x 8.0	Ctr	-1.5	0.80
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8612 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R218*	6	SP	500608	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 474.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 6- 8
BC Cont.	0- 0- 0	50- 6- 8

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2227	716	5- 8	1-14
			Hz =	-175
C	2022	631	3- 8	1-11
			Hz =	177

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -J	0.28	8459 C	0.08 0.20
J -K	0.30	8613 C	0.09 0.21
K -S	0.17	5177 C	0.03 0.14
S -B	0.09	3692 C	0.04 0.05
B -T	0.09	3692 C	0.04 0.05
T -N	0.18	5121 C	0.03 0.15
N -O	0.30	8371 C	0.08 0.22
O -C	0.27	7939 C	0.07 0.20
Bottom Chords			
A -I	0.54	7990 T	0.36 0.18
I -E	0.54	8025 T	0.36 0.18
E -R	0.39	8021 T	0.36 0.03
R -Q	0.27	4882 T	0.22 0.05
Q -P	0.27	4831 T	0.22 0.05
P -D	0.40	7799 T	0.35 0.05
D -F	0.52	7501 T	0.34 0.18
F -C	0.52	7458 T	0.34 0.18
Webs			
I -J	0.03	133 T	
J -E	0.07	245 T	
E -K	0.58	1810 T	
K -R	0.84	3274 C	1 Br
R -S	0.33	1054 T	
S -Q	0.50	1794 C	1 Br
Q -B	0.72	2264 T	
Q -T	0.48	1730 C	1 Br
P -T	0.31	995 T	
P -N	0.79	3097 C	1 Br
D -N	0.54	1691 T	
D -O	0.17	532 T	
F -O	0.03	147 T	

LL Defl -0.66" in R -Q L/902
TL Defl -1.33" in R -Q L/448
Shear // Grain in O -C 0.25

Hz Disp	LL	DL	TL
Jt C	0.41"	0.41"	0.82"

Plates for each ply each face.
ALL CONNECTOR PLATES TO BE MANUFACTURED BY ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

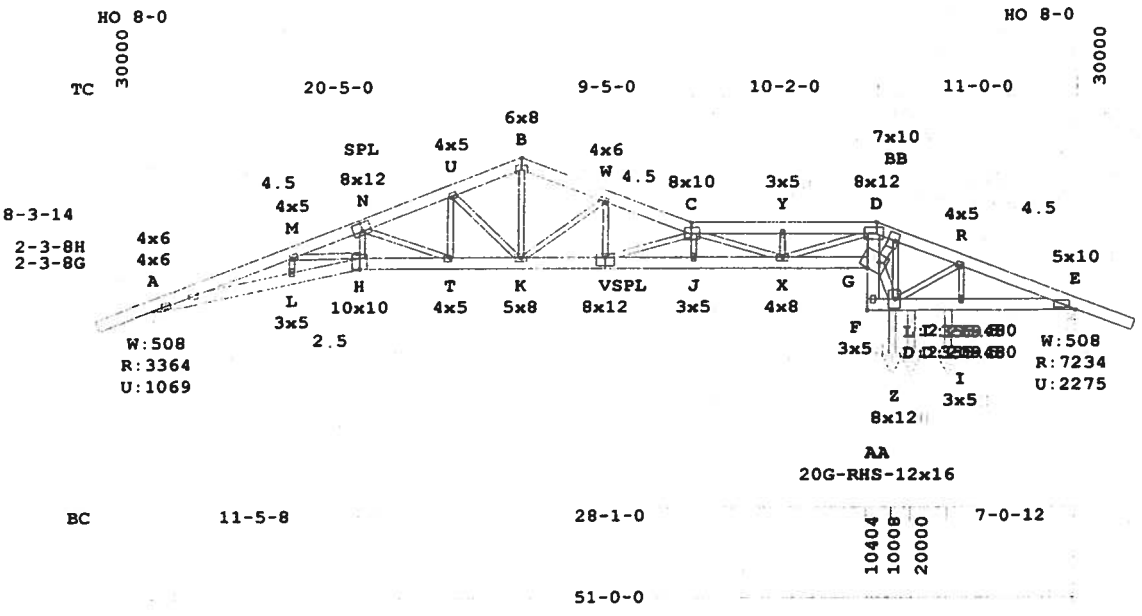
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.80
A	LOCK	4.0x 6.0	Ctr	Ctr	0.74
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	5.0x12.0	-6.4	2.4	0.99
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.95
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.90
D	LOCK	8.0x 8.0	Ctr	-1.5	0.80
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8613 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R223*	1*3P	SP	510000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 557.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.46	2x 8	SP-2400
BC	0.66	2x 8	SP-2400
CW	0.76	2x 8	SP-2400
WB	0.88	2x 4	SP-#3
EX	AA-BB	2x 6	SP-#2
EX	AA-Z	2x 6	SP-2400

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	2356	2356	40.9'	CL-LB
BC V	220	220	41.9'	CL-LB
BC V	480	480	43.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	3365	1070	5- 8	1- 0
			Hz =	-143
E	7235	2275	5- 8	2- 0
			Hz =	145

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -M	0.10	14821 C	0.02	0.08
M -N	0.11	15425 C	0.02	0.09
N -U	0.09	10914 C	0.01	0.08
U -B	0.03	9378 C	0.03	0.00
B -W	0.19	9540 C	0.01	0.18
W -C	0.27	16344 C	0.03	0.24
C -Y	0.41	29516 C	0.10	0.31
Y -D	0.41	29516 C	0.11	0.30
D -BB	0.46	29354 C	0.10	0.36
BB-R	0.39	16983 C	0.03	0.36
R -E	0.07	17997 C	0.07	0.00
--------Bottom Chords---------				
A -L	0.25	14073 T	0.16	0.09
L -H	0.25	14160 T	0.16	0.09
H -T	0.18	14315 T	0.16	0.02
T -K	0.13	10296 T	0.11	0.02
K -V	0.29	15666 T	0.18	0.11
V -J	0.62	31013 T	0.35	0.27
J -X	0.62	31030 T	0.35	0.27
X -AA	0.66	31174 T	0.36	0.30
F -Z	0.11	3304 T	0.03	0.08
Z -I	0.31	16844 T	0.19	0.12
I -E	0.29	16844 T	0.19	0.10
----------Chord-Webs----------				
AA-D	0.06	5148 T	0.06	0.00
F -AA	0.76	765 C	0.00	0.76
------------Webs-------------				
L -M	0.02	343 C		
M -H	0.06	629 T		
H -N	0.34	3223 T		
N -T	0.24	4371 C		
T -U	0.17	1638 T		
U -K	0.11	2123 C		
K -B	0.72	6743 T		
K -W	0.48	8677 C		
V -W	0.65	6105 T		
V -C	0.88	16114 C		
J -C	0.05	1041 C		
C -X	0.08	1577 C		
X -Y	0.18	1769 T		
X -D	0.10	2205 C		
AA-BB	0.96	21713 T		
AA-Z	0.44	26423 T		
Z -BB	0.84	15908 C		
Z -R	0.08	1565 C		
I -R	0.01	194 T		

LL Defl -1.12" in J -X L/534
TL Defl -2.25" in J -X L/266
Shear // Grain in D -BB 0.93

Hz Disp	LL	DL	TL
Jt E	0.42"	0.42"	0.84"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	RHS 20 Ga, Gross Area JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.69
A	LOCK	4.0x 6.0	Ctr	Ctr	0.30
M	LOCK	4.0x 5.0-0.5	0.2		0.61
N	LOCK	8.0x12.0-0.6	1.5		0.97
U	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
W	LOCK	4.0x 6.0	0.5	0.2	0.84
C	LOCK	8.0x10.0	0.5	Ctr	0.89
Y	LOCK	3.0x 5.0	Ctr	Ctr	0.42
D	LOCK	8.0x12.0-0.5	Ctr		0.86
BB	LOCK	7.0x10.0	Ctr	Ctr	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	3.0x 5.0	Ctr	Ctr	0.36
H	LOCK	10.0x10.0	Ctr-2.2		0.75
T	LOCK	4.0x 5.0	Ctr-0.5		0.68
K	LOCK	5.0x 8.0	Ctr	Ctr	0.72
V	LOCK	8.0x12.0	Ctr-1.5		0.99
J	LOCK	3.0x 5.0	Ctr	Ctr	0.36
X	LOCK	4.0x 8.0	Ctr-0.5		0.73
AA	RHS	12.0x16.0	0.5	0.9	0.97
F	LOCK	3.0x 5.0	Ctr	Ctr	0.96
Z	LOCK	8.0x12.0-0.5	Ctr		0.91
I	LOCK	3.0x 5.0	Ctr	Ctr	0.36

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	4	4	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R223*	1*3P	SP	510000	4.5	3- 0- 0	3- 0- 0	

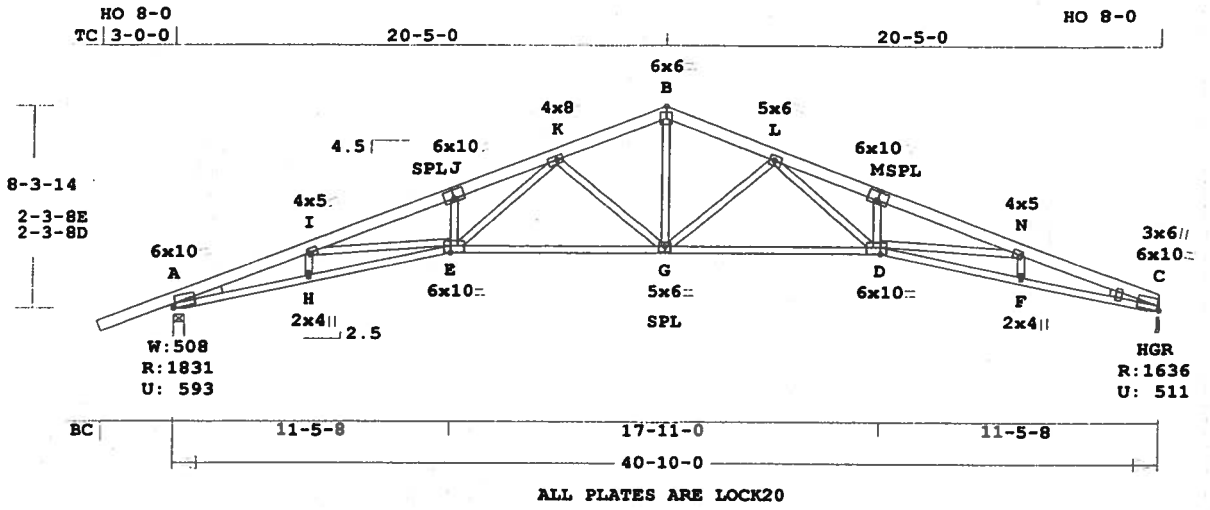
Office and Care Center, Hospice, M.M. Parrish

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 29516 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R224*	3	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 303.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.72	2x 6	SP-#2
EX A -J		2x 6	SP-2400
BC	0.94	2x 4	SP-2400
EX E -G		2x 4	SP-#2
EX G -D		2x 4	SP-#2
WB	0.85	2x 4	SP-#3
WG	---	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	1831	593	5- 8	1- 8
			Hz =	-139
C	1637	512	3- 8	1- 6
			Hz =	139

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -I	0.74	5786	C	0.06 0.68
I -J	0.45	6348	C	0.09 0.36
J -K	0.32	6396	C	0.26 0.06
K -B	0.21	3172	C	0.13 0.08
B -L	0.21	3172	C	0.13 0.08
L -M	0.32	6338	C	0.26 0.06
M -N	0.72	6307	C	0.15 0.57
N -C	0.56	5852	C	0.10 0.46
--------Bottom Chords---------				
A -H	0.94	5376	T	0.38 0.56
H -E	0.63	5412	T	0.38 0.25
E -G	0.98	3983	T	0.66 0.32
G -D	0.98	3974	T	0.66 0.32
D -F	0.61	5501	T	0.39 0.22
F -C	0.64	5465	T	0.39 0.25
------------Webs------------				
H -I	0.03	147	T	
I -E	0.22	699	T	
E -J	0.07	453	C	
E -K	0.85	2663	T	
K -G	0.69	1334	C	
G -B	0.62	1941	T	
G -L	0.68	1323	C	
L -D	0.83	2607	T	
D -M	0.07	405	C	
D -N	0.17	560	T	
F -N	0.03	140	T	

LL Defl -0.63" in E -G L/769
TL Defl -1.36" in E -G L/354
Shear // Grain in A -I 0.40

Hz Disp	LL	DL	TL
Jt C	0.38"	0.38"	0.76"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

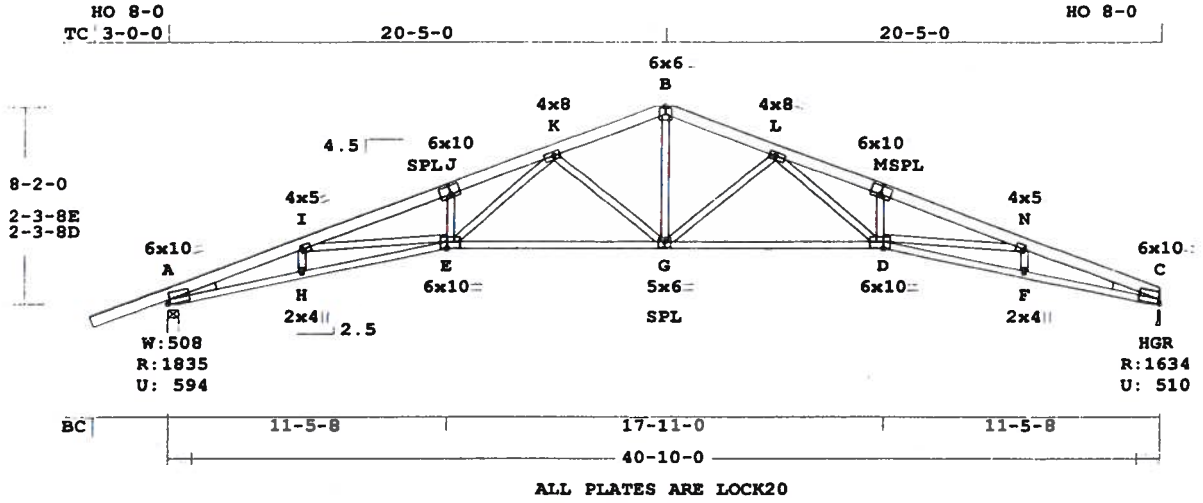
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	6.0x10.0	-0.5	1.2	0.95
K	LOCK	4.0x 8.0	-0.7	-0.3	0.92
B	LOCK	6.0x 6.0	Ctr	Ctr	0.70
L	LOCK	5.0x 6.0	0.5	-0.2	0.99
M	LOCK	6.0x10.0	0.5	1.2	0.95
N	LOCK	4.0x 5.0	Ctr	Ctr	0.64
C	LOCK	6.0x10.0	-4.6	1.5	0.91
C	LOCK	3.0x 6.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.48
E	LOCK	6.0x10.0	Ctr	-0.5	0.85
G	LOCK	5.0x 6.0	Ctr	-0.5	0.88
D	LOCK	6.0x10.0	Ctr	-0.5	0.86
F	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6396 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R225*	1	SP	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 303.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.74	2x 6	SP-2400
EX J -B		2x 6	SP-#2
EX B -M		2x 6	SP-#2
BC	0.94	2x 4	SP-2400
EX E -G		2x 4	SP-#2
EX G -D		2x 4	SP-#2
WB	0.85	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1835	595	5- 8	1- 8
			Hz =	-140
C	1634	511	3- 8	1- 6
			Hz =	139

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.74	5807 C	0.06	0.68
I -J	0.45	6377 C	0.09	0.36
J -K	0.32	6425 C	0.26	0.06
K -B	0.21	3189 C	0.13	0.08
B -L	0.21	3189 C	0.13	0.08
L -M	0.32	6424 C	0.26	0.06
M -N	0.45	6376 C	0.09	0.36
N -C	0.58	5805 C	0.06	0.52
--------Bottom Chords---------				
A -H	0.94	5402 T	0.38	0.56
H -E	0.64	5438 T	0.39	0.25
E -G	0.98	4009 T	0.67	0.31
G -D	0.98	4009 T	0.67	0.31
D -F	0.64	5437 T	0.39	0.25
F -C	0.81	5400 T	0.38	0.43
----------Webs----------				
H -I	0.03	147 T		
I -E	0.22	707 T		
E -J	0.07	454 C		
E -K	0.85	2673 T		
K -G	0.69	1338 C		
G -B	0.62	1950 T		
G -L	0.69	1338 C		
L -D	0.85	2673 T		
D -M	0.07	454 C		
D -N	0.22	708 T		
F -N	0.03	147 T		

LL Defl -0.66" in E -G L/729
TL Defl -1.42" in E -G L/337
Shear // Grain in A -I 0.40

Hz Disp	LL	DL	TL
Jt C	0.40"	0.40"	0.80"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

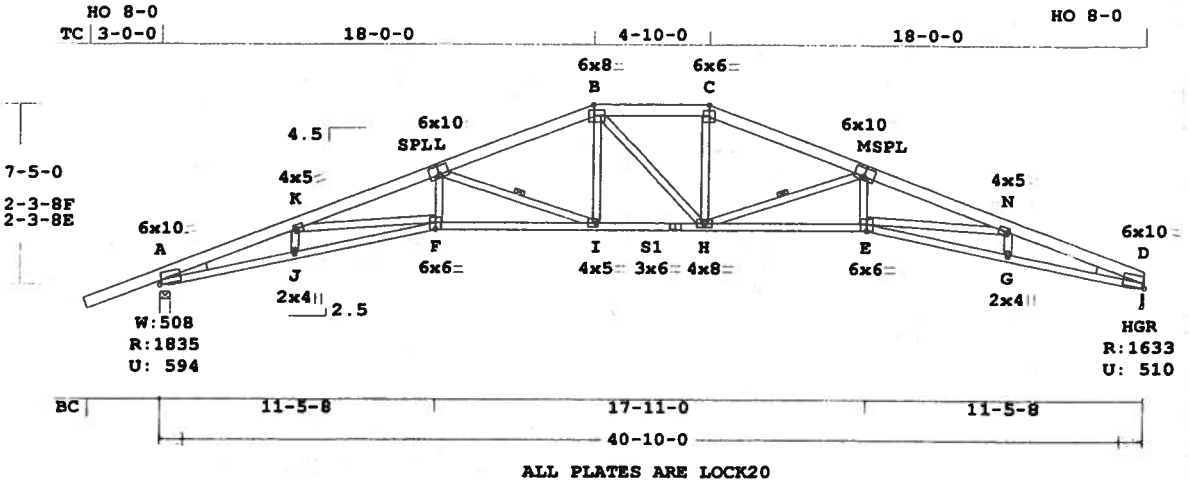
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	6.0x10.0	-0.5	1.2	0.95
K	LOCK	4.0x 8.0	-0.7	-0.3	0.92
B	LOCK	6.0x 6.0	Ctr	Ctr	0.70
L	LOCK	4.0x 8.0	0.7	-0.3	0.92
M	LOCK	6.0x10.0	0.5	1.2	0.95
N	LOCK	4.0x 5.0	Ctr	Ctr	0.64
C	LOCK	6.0x10.0	-4.6	1.5	0.91
H	LOCK	2.0x 4.0	Ctr	Ctr	0.48
E	LOCK	6.0x10.0	Ctr	-0.5	0.86
G	LOCK	5.0x 6.0	Ctr	-0.5	0.88
D	LOCK	6.0x10.0	Ctr	-0.5	0.86
F	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6425 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R226	1	HPSC4	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 303.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.29	2x 6	SP-#2
EX A -L		2x 6	SP-2400
EX M -D		2x 6	SP-2400
BC	0.94	2x 4	SP-2400
WB	0.58	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

WB 1 rows CLB on L -I
WB 1 rows CLB on H -M
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	1836	595	5- 8	1- 8
			Hz =	-123
D	1633	510	3- 8	1- 6
			Hz =	125

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.76	5897	C	0.06	0.70
K -L	0.43	6277	C	0.09	0.34
L -B	0.29	3705	C	0.15	0.14
B -C	0.19	3468	C	0.15	0.04
C -M	0.29	3703	C	0.15	0.14
M -N	0.43	6280	C	0.09	0.34
N -D	0.60	5895	C	0.06	0.54
Bottom Chords					
A -J	0.94	5486	T	0.39	0.55
J -F	0.63	5512	T	0.39	0.24
F -I	0.57	5874	T	0.42	0.15
I -S1	0.26	3453	T	0.24	0.02
S1-H	0.28	3453	T	0.24	0.04
H -E	0.57	5878	T	0.42	0.15
E -G	0.63	5510	T	0.39	0.24
G -D	0.81	5484	T	0.39	0.42
Webs					
J -K	0.03	137	T		
K -F	0.16	525	T		
F -L	0.41	1295	T		
L -I	0.58	2548	C		1 Br
I -B	0.30	947	T		
B -H	0.06	126	T		
H -C	0.30	942	T		
H -M	0.58	2557	C		1 Br
E -M	0.41	1291	T		
E -N	0.17	531	T		
G -N	0.03	138	T		

LL Defl -0.69" in I -S1 L/699
TL Defl -1.38" in I -S1 L/349
Shear // Grain in A -K 0.40

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES TO BE MANUFACTURED BY ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

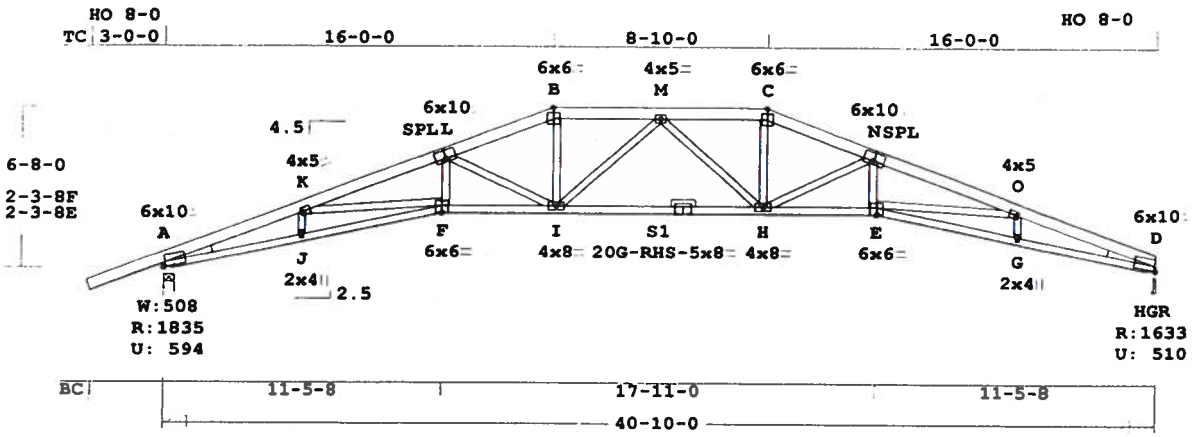
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
K	LOCK	4.0x 5.0 Ctr	Ctr		0.64
L	LOCK	6.0x10.0-0.5	1.2		0.95
B	LOCK	6.0x 8.0-0.5	Ctr		0.90
C	LOCK	6.0x 6.0 Ctr	Ctr		0.90
M	LOCK	6.0x10.0	0.5	1.2	0.95
N	LOCK	4.0x 5.0 Ctr	Ctr		0.64
D	LOCK	6.0x10.0-4.6	1.5		0.91
J	LOCK	2.0x 4.0 Ctr	Ctr		0.48
F	LOCK	6.0x 6.0 Ctr-0.5			0.93
I	LOCK	4.0x 5.0 Ctr	Ctr		0.74
S1	LOCK	3.0x 6.0 Ctr	Ctr		0.88
H	LOCK	4.0x 8.0 Ctr	Ctr		0.62
E	LOCK	6.0x 6.0 Ctr-0.5			0.93
G	LOCK	2.0x 4.0 Ctr	Ctr		0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6280 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R227*	1	HPSC4	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 304.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.25	2x 6	SP-#2
EX A -L		2x 6	SP-2400
EX N -D		2x 6	SP-2400
BC	0.94	2x 4	SP-2400
WB	0.79	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1836	595	5- 8	1- 8
			Hz =	-107
D	1634	510	3- 8	1- 6
			Hz =	111

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.76	5900	C	0.06	0.70
K -L	0.43	6248	C	0.09	0.34
L -B	0.25	4248	C	0.17	0.08
B -M	0.20	3983	C	0.16	0.04
M -C	0.20	3984	C	0.16	0.04
C -N	0.25	4248	C	0.17	0.08
N -O	0.43	6249	C	0.09	0.34
O -D	0.60	5901	C	0.06	0.54
Bottom Chords					
A -J	0.94	5482	T	0.39	0.55
J -F	0.63	5508	T	0.39	0.24
F -I	0.57	5838	T	0.42	0.15
I -S1	0.41	4117	T	0.29	0.12
S1-H	0.40	4117	T	0.29	0.11
H -E	0.57	5839	T	0.42	0.15
E -G	0.63	5509	T	0.39	0.24
G -D	0.81	5484	T	0.39	0.42
Webs					
J -K	0.03	146	T		
K -F	0.15	493	T		
F -L	0.40	1266	T		
L -I	0.79	2093	C		
I -B	0.38	1207	T		
I -M	0.09	192	C		
M -H	0.09	192	C		
H -C	0.38	1207	T		
H -N	0.79	2093	C		
E -N	0.40	1266	T		
E -O	0.15	492	T		
G -O	0.03	146	T		

LL Defl -0.69" in I -S1 L/699
TL Defl -1.46" in I -S1 L/330
Shear // Grain in A -K 0.39

Hz Disp	LL	DL	TL
Jt D	0.41"	0.42"	0.83"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

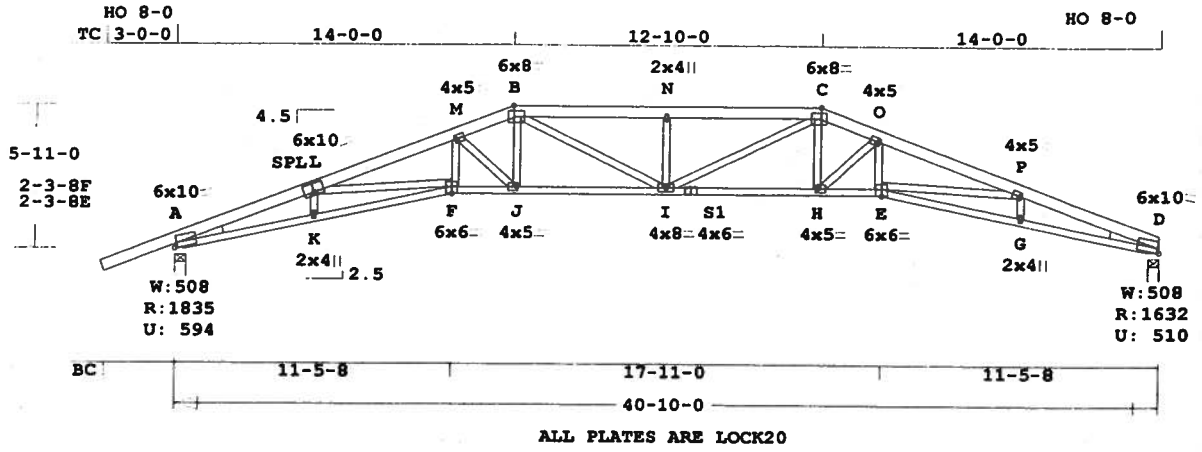
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
K	LOCK	4.0x 5.0	Ctr	Ctr	0.64
L	LOCK	6.0x10.0	-0.5	1.2	0.95
B	LOCK	6.0x 6.0	Ctr	Ctr	0.90
M	LOCK	4.0x 5.0	Ctr	Ctr	0.48
C	LOCK	6.0x 6.0	Ctr	Ctr	0.90
N	LOCK	6.0x10.0	0.5	1.2	0.95
O	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	6.0x10.0	-4.6	1.5	0.91
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48
F	LOCK	6.0x 6.0	Ctr	-0.5	0.93
I	LOCK	4.0x 8.0	Ctr	Ctr	0.55
S1	RHS	5.0x 8.0	Ctr	0.8	0.84
H	LOCK	4.0x 8.0	Ctr	Ctr	0.55
E	LOCK	6.0x 6.0	Ctr	-0.5	0.93
G	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6249 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R228*	1	HPSC4	401000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 301.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.76	2x 6	SP-2400
EX L -B		2x 6	SP-#2
EX B -C		2x 6	SP-#2
BC	0.95	2x 4	SP-2400
WB	0.41	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40-10- 0
BC Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1835	595	5- 8	1- 8
			Hz =	-94
D	1632	510	5- 8	1- 6
			Hz =	97

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.76	5927 C	0.06	0.70
L -M	0.80	6260 C	0.14	0.66
M -B	0.24	4923 C	0.19	0.05
B -N	0.29	5186 C	0.09	0.20
N -C	0.29	5186 C	0.09	0.20
C -O	0.11	4921 C	0.04	0.07
O -P	0.43	6254 C	0.09	0.34
P -D	0.76	5899 C	0.06	0.70
Bottom Chords				
A -K	0.95	5515 T	0.39	0.56
K -F	0.63	5543 T	0.39	0.24
F -J	0.59	5850 T	0.42	0.17
J -I	0.40	4582 T	0.32	0.08
I -S1	0.40	4582 T	0.32	0.08
S1-H	0.39	4582 T	0.32	0.07
H -E	0.59	5846 T	0.42	0.17
E -G	0.62	5513 T	0.39	0.23
G -D	0.94	5486 T	0.39	0.55
Webs				
K -L	0.03	139 T		
L -F	0.15	473 T		
F -M	0.41	1304 T		
M -J	0.35	1663 C		
J -B	0.36	1143 T		
B -I	0.21	669 T		
I -N	0.08	436 C		
I -C	0.21	668 T		
H -C	0.36	1137 T		
H -O	0.35	1656 C		
E -O	0.41	1299 T		
E -P	0.15	497 T		
G -P	0.03	141 T		

LL Defl -0.73" in I -S1 L/657
TL Defl -1.48" in J -I L/322
Shear // Grain in A -L 0.39

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

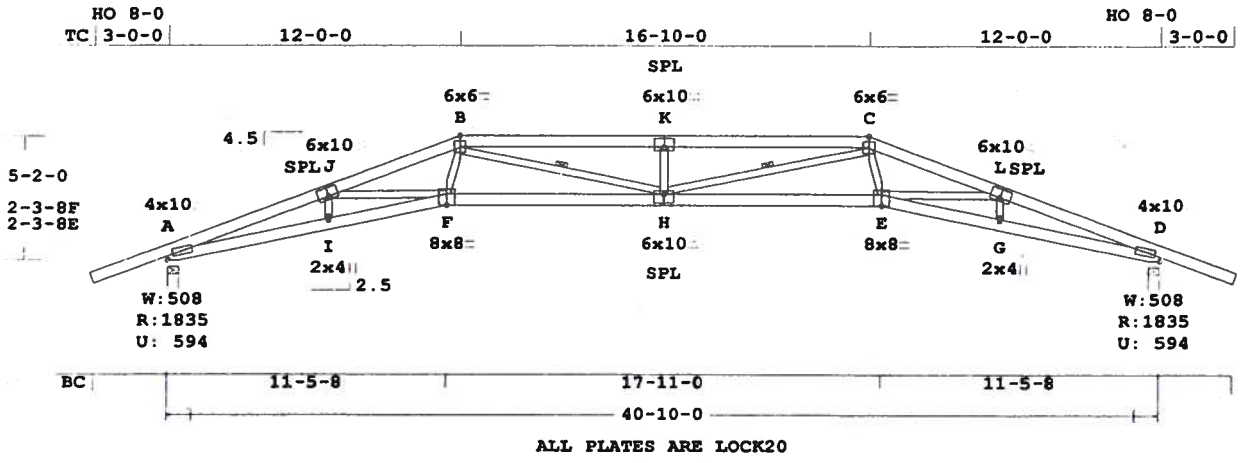
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
L	LOCK	6.0x10.0	-0.5	1.2	0.95
M	LOCK	4.0x 5.0	Ctr	Ctr	0.65
B	LOCK	6.0x 8.0	-0.5	Ctr	0.90
N	LOCK	2.0x 4.0	Ctr	Ctr	0.48
C	LOCK	6.0x 8.0	0.5	Ctr	0.90
O	LOCK	4.0x 5.0	Ctr	Ctr	0.65
P	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	6.0x10.0	-4.6	1.5	0.91
K	LOCK	2.0x 4.0	Ctr	Ctr	0.48
F	LOCK	6.0x 6.0	Ctr	-0.5	0.94
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 8.0	Ctr	Ctr	0.55
S1	LOCK	4.0x 6.0	Ctr	0.2	0.92
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x 6.0	Ctr	-0.5	0.93
G	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6260 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R229*	1	HPSC4	401000	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 321.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.61	2x 6	SP-#2
BC	0.54	2x 6	SP-2400
EX F -H		2x 6	SP-#2
EX H -E		2x 6	SP-#2
WB	0.50	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

WB 1 rows CLB on B -H
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1836	595	5- 8	1- 8
			Hz =	-81
D	1836	595	5- 8	1- 8
			Hz =	82

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -J	0.48	6164	C 0.11 0.37
J -B	0.56	6453	C 0.14 0.42
B -K	0.61	6814	C 0.22 0.39
K -C	0.61	6814	C 0.22 0.39
C -L	0.56	6455	C 0.14 0.42
L -D	0.48	6165	C 0.11 0.37
--------Bottom Chords---------			
A -I	0.54	5798	T 0.26 0.28
I -F	0.54	5873	T 0.26 0.28
F -H	0.87	5540	T 0.67 0.20
H -E	0.87	5541	T 0.67 0.20
E -G	0.54	5875	T 0.26 0.28
G -D	0.54	5800	T 0.26 0.28
------------Webs------------			
I -J	0.05	266	C
J -F	0.10	332	T
F -B	0.50	1570	T
B -H	0.42	1309	T 1 Br
H -K	0.08	445	C
H -C	0.41	1307	T 1 Br
C -E	0.50	1570	T
E -L	0.10	332	T
G -L	0.05	266	C

LL Defl -0.70" in H -E L/684
TL Defl -1.44" in H -E L/331
Shear // Grain in A -J 0.24

Hz Disp	LL	DL	TL
Jt D	0.34"	0.35"	0.69"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

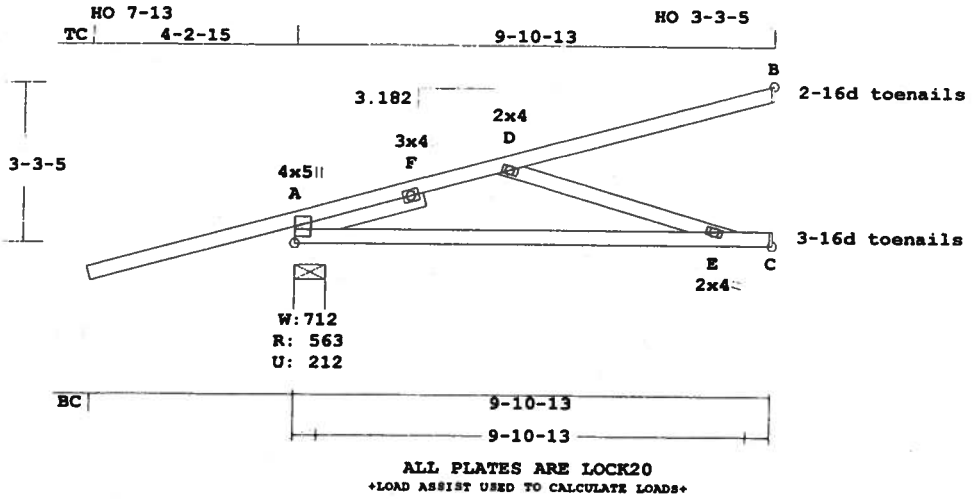
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0	Ctr	Ctr	0.99
J	LOCK	6.0x10.0	-0.5	1.2	0.95
B	LOCK	6.0x 6.0	Ctr	Ctr	0.90
K	LOCK	6.0x10.0	Ctr	1.2	0.97
C	LOCK	6.0x 6.0	Ctr	Ctr	0.90
L	LOCK	6.0x10.0	0.5	1.2	0.95
D	LOCK	4.0x10.0	Ctr	Ctr	0.99
I	LOCK	2.0x 4.0	Ctr	Ctr	0.48
F	LOCK	8.0x 8.0	Ctr	-1.5	0.74
H	LOCK	6.0x10.0	Ctr	-1.2	0.97
E	LOCK	8.0x 8.0	Ctr	-1.5	0.74
G	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6814 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R232	2	KING	91013	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.52	2x 4	SP-#2
BC	0.68	2x 4	SP-#2
WB	0.30	2x 4	SP-#3
SL	0.14	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9-10-13
BC Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
TC V	-40	-20	0.0'	
	45	22		9.9'
BC V	0	-20	0.0'	
	0	22		9.9'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	564	213	7-12	1- 0
			Hz =	125
B	278	188	3- 8	1- 0
C	336	18	3- 8	1- 0
			Hz =	68

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -F	0.25	895 C	0.00 0.25
F -D	0.35	657 C	0.00 0.35
D -B	0.52	104 C	0.00 0.52
--------Bottom Chords---------			
A -E	0.68	657 T	0.05 0.63
E -C	0.30	67 T	0.00 0.30
------------Webs------------			
D -E	0.30	685 C	
-----------Sliders-----------			
A -F	0.14	546 T	

LL Defl -0.22" in A -E L/491
TL Defl -0.46" in A -E L/235
Shear // Grain in E -C 0.37

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.0	0.3	0.72
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.37
E	LOCK	2.0x 4.0	Ctr	Ctr	0.47

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

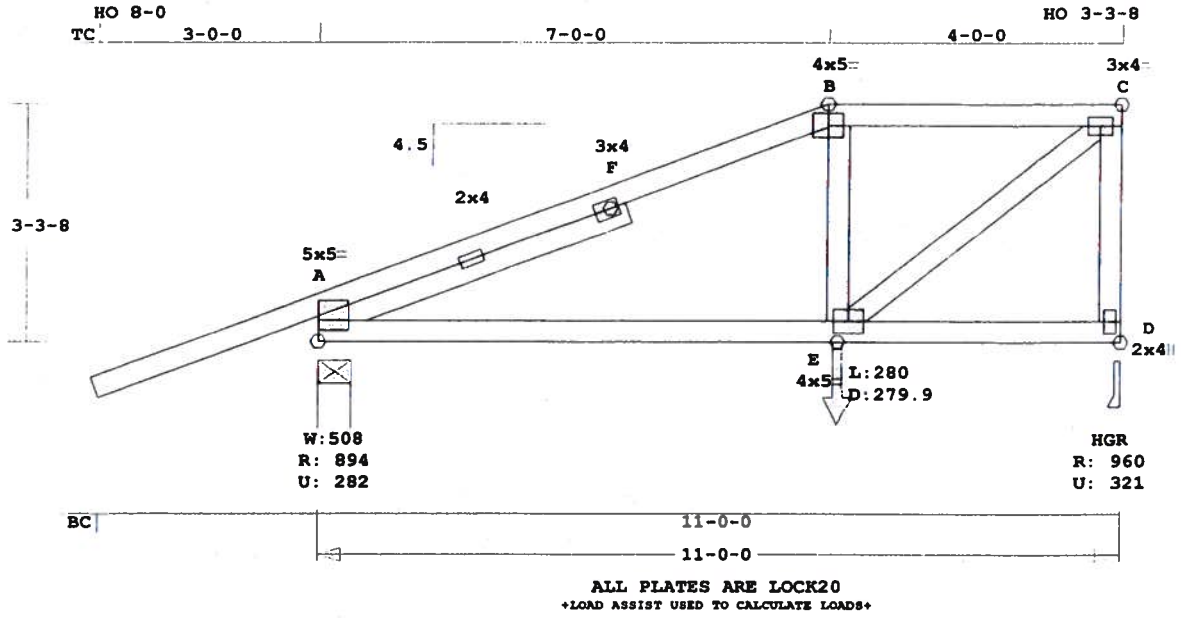
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 3- 4-13. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 895 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R233*	1	HHIP	110000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 75.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.38	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.41	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 0- 0
BC	Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	11.0'
BC V	0	20	0.0'	11.0'
TC V	50	25	7.0'	10.0'
TC V	-40	-20	10.0'	11.0'
BC V	0	25	7.1'	10.0'
BC V	0	-20	10.0'	11.0'
BC V	280	280	7.1'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	894	282	5- 8	1- 1
			Hz =	121
D	960	322	3- 8	1- 2
			Hz =	66

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -F	0.32	739 C	0.05 0.27
F -B	0.38	1048 C	0.09 0.29
B -C	0.34	1004 C	0.01 0.33
--------Bottom Chords---------			
A -E	0.35	1004 T	0.18 0.17
E -D	0.24	66 T	0.00 0.24
------------Webs------------			
E -B	0.07	295 T	
E -C	0.41	1291 T	
D -C	0.18	933 C	
----------Sliders----------			
A -F	0.10	378 C	

LL Defl -0.03" in A -E L/999
TL Defl -0.07" in A -E L/999
Shear // Grain in B -C 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

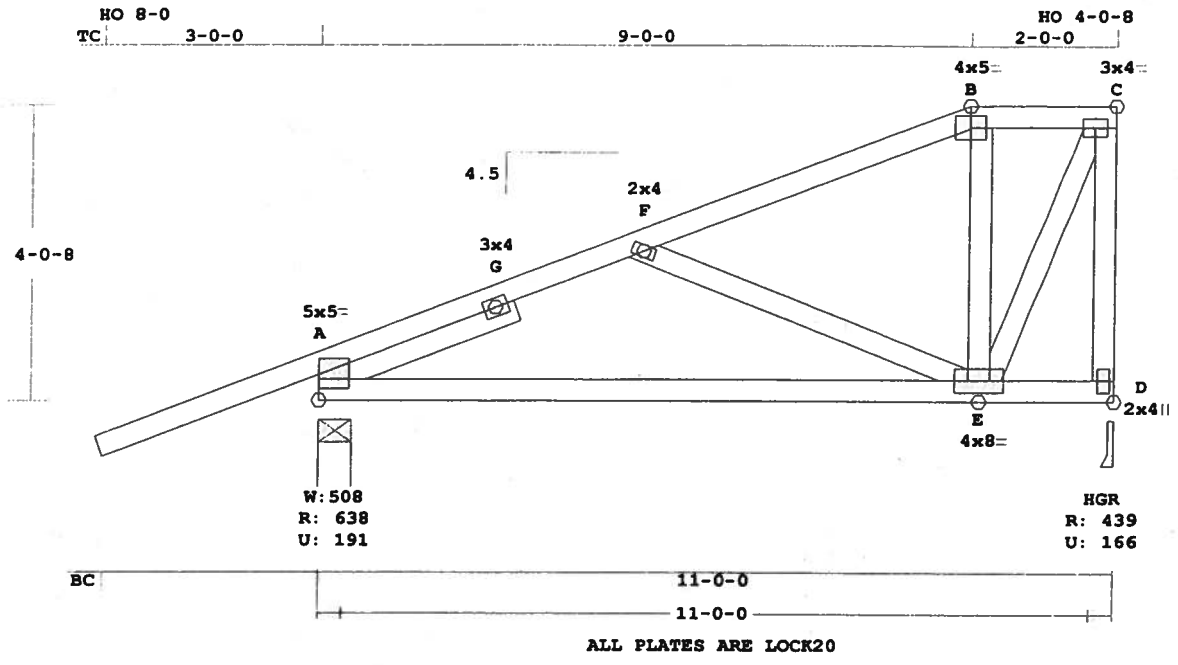
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.69
C	LOCK	3.0x 4.0	Ctr	Ctr	0.79
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1048 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R234*	1	HHIP	110000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.16	2x 4	SP-#3
SL	0.07	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 0
BC Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	639	191	5- 8	1- 0
			Hz =	152
D	440	167	3- 8	1- 0
			Hz =	83

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -G	0.10	654 C	0.00 0.10
G -F	0.20	581 C	0.00 0.20
F -B	0.22	222 C	0.00 0.22
B -C	0.10	202 C	0.01 0.09
Bottom Chords			
A -E	0.40	568 T	0.05 0.35
E -D	0.32	82 T	0.00 0.32
Webs			
F -E	0.16	395 C	
E -B	0.06	263 T	
E -C	0.15	488 T	
D -C	0.11	477 C	
Sliders			
A -G	0.07	278 T	

LL Defl -0.09" in A -E L/999
TL Defl -0.17" in A -E L/703
Shear // Grain in F -B 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.85
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
B	LOCK	4.0x 5.0	Ctr	Ctr	0.69
C	LOCK	3.0x 4.0	Ctr	Ctr	0.55
E	LOCK	4.0x 8.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

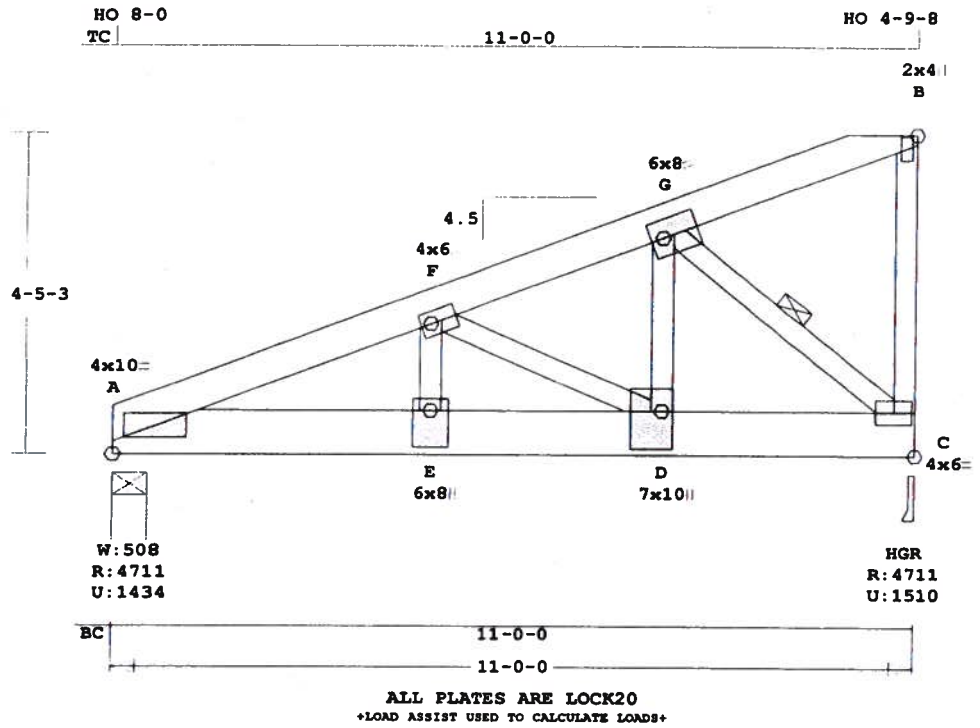
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 654 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R235*	1	MONO	110000	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 102.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.56	2x 6	SP-#2
BC	0.58	2x 8	SP-2400
WB	0.85	2x 4	SP-#3
EX D -G		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 0
BC Cont.	0- 0- 0	11- 0- 0

WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	11.0'
BC V	388	408	0.0'	11.0'

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	4712	1435	5- 8	3-14
			Hz =	187
C	4712	1510	3- 8	3-14 **
			Hz =	100

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -F	0.56	7065	C	0.41	0.15
F -G	0.31	3983	C	0.23	0.08
G -B	0.08	60	C	0.00	0.08
Bottom Chords					
A -E	0.58	6620	T	0.25	0.33
E -D	0.47	6620	T	0.25	0.22
D -C	0.40	3752	T	0.14	0.26
Webs					
E -F	0.80	2505	T		
F -D	0.72	3267	C		
D -G	0.81	4438	T		
G -C	0.85	5001	C		1 Br
C -B	0.03	158	T		

LL Defl -0.07" in E -D L/999
TL Defl -0.14" in E -D L/852
Shear // Grain in D -C 0.85

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0	Ctr	Ctr	0.96
F	LOCK	4.0x 6.0-0.5-0.2			0.86
G	LOCK	6.0x 8.0	Ctr	Ctr	0.92
B	LOCK	2.0x 4.0	Ctr	Ctr	0.41
E	LOCK	6.0x 8.0	Ctr-2.1		0.90
D	LOCK	7.0x10.0	Ctr-1.2		0.88
C	LOCK	4.0x 6.0	Ctr	Ctr	0.95

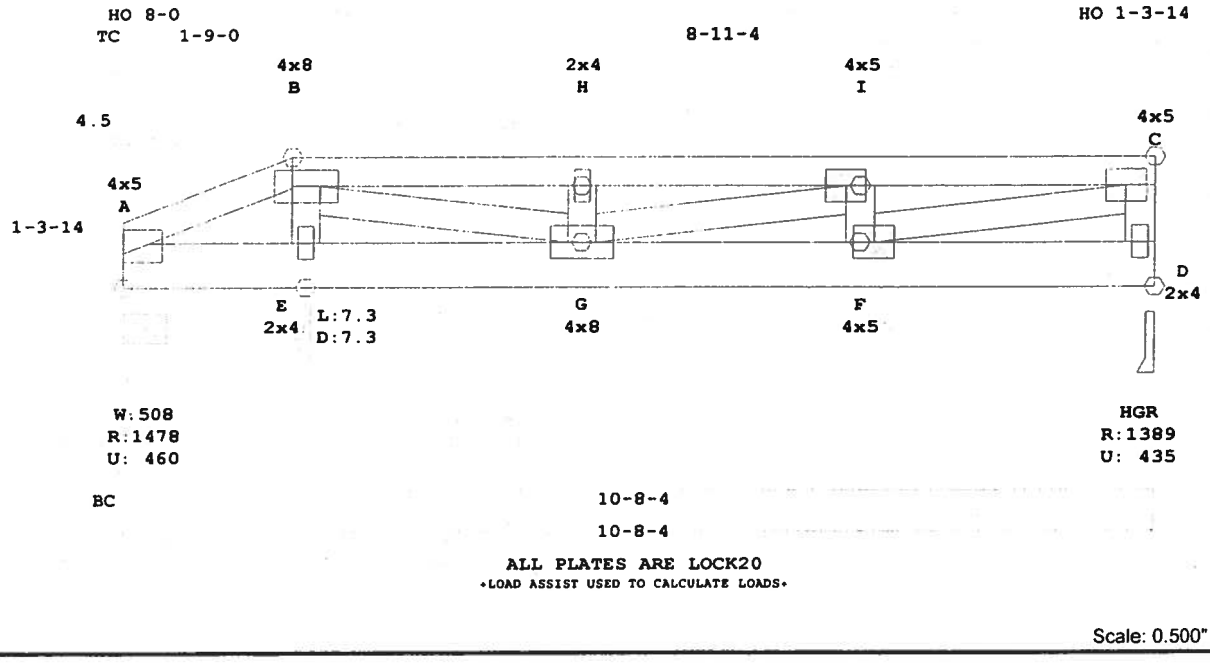
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder Common
Loading BC
Span 40-10- 0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7065 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R236*	1*2P	HHIP	100804	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 70.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.39	2x 6	SP-#2
WB	0.50	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	10- 8- 4
BC Cont.		0- 0- 0	10- 8- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	10.7'
BC V	0	20	0.0'	10.7'
BC V	100	100	0.0'	10.7'
TC V	-4	-2	1.8'	9.7'
TC V	-40	-20	9.7'	10.7'
BC V	0	-2	1.9'	9.7'
BC V	0	-20	9.7'	10.7'
BC V	7	7	1.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd	Lbs	Lbs	In-Sx	In-Sx
A	1478	460				5- 8	1- 0
						Hz =	30
D	1390	436				3- 8	1- 0
						Hz =	17

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -B	0.16	2240	C	0.09	0.07
B -H	0.19	3526	C	0.15	0.04
H -I	0.20	3526	C	0.15	0.05
I -C	0.16	2991	C	0.12	0.04
--------Bottom Chords---------					
A -E	0.31	2048	T	0.13	0.18
E -G	0.30	2016	T	0.13	0.17
G -F	0.39	2991	T	0.20	0.19
F -D	0.13	16	T	0.00	0.13
------------Webs------------					
E -B	0.05	373	T		
B -G	0.25	1560	T		
G -H	0.01	96	T		
G -I	0.09	563	T		
F -I	0.03	343	C		
F -C	0.50	3148	T		
D -C	0.08	1029	C		

LL Defl -0.04" in G -F L/999
TL Defl -0.09" in G -F L/999
Shear // Grain in A -E 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.8	0.3	0.79
B	LOCK	4.0x 8.0	Ctr	Ctr	0.69
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
I	LOCK	4.0x 5.0	Ctr	Ctr	0.75
C	LOCK	4.0x 5.0	Ctr	Ctr	0.89
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	4.0x 8.0	Ctr	Ctr	0.75
F	LOCK	4.0x 5.0	Ctr	Ctr	0.89
D	LOCK	2.0x 4.0	Ctr	Ctr	0.45

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 1- 7- 8
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

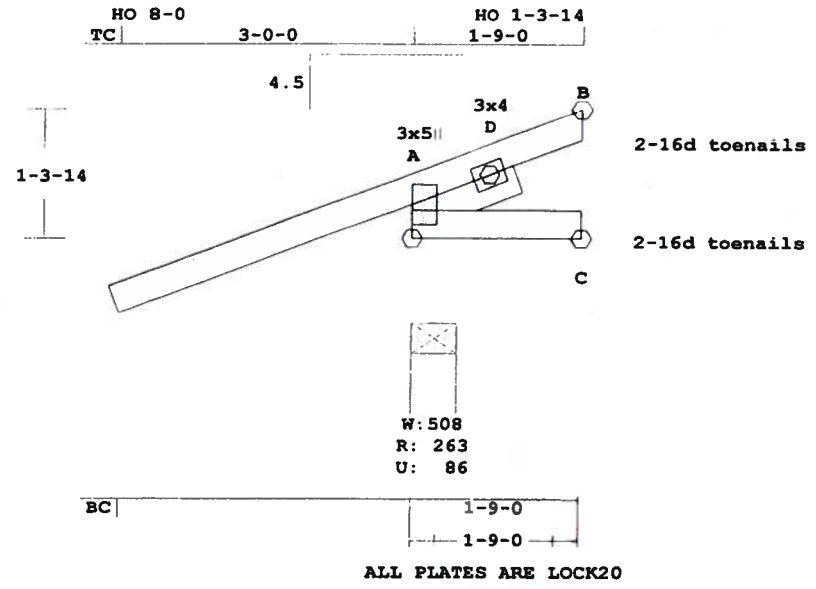
	Rows	----Spacing (In)---- Nails	Screws	Bolts
TC	1	12	24	0
BC	2	12	24	0
WB	1	4	4	

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3526 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R237*	5	JACK	10900	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.03	2x 4	SP-#2
SL	0.00	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 9- 0
BC Cont.	0- 0- 0	1- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	263	87	5- 8	1- 0
			Hz =	30
B	55	40	3- 8	1- 0
C	30	0	3- 8	1- 0
			Hz =	16

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.03	37 C	0.00 0.03
D -B	0.03	42 C	0.00 0.03
--------Bottom Chords---------			
A -C	0.03	15 T	0.00 0.03
----------Sliders------------			
A -D	0.00	14 T	

LL Defl 0.00" in D -C L/999
TL Defl 0.00" in D -C L/999
Shear // Grain in D -B 0.07

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

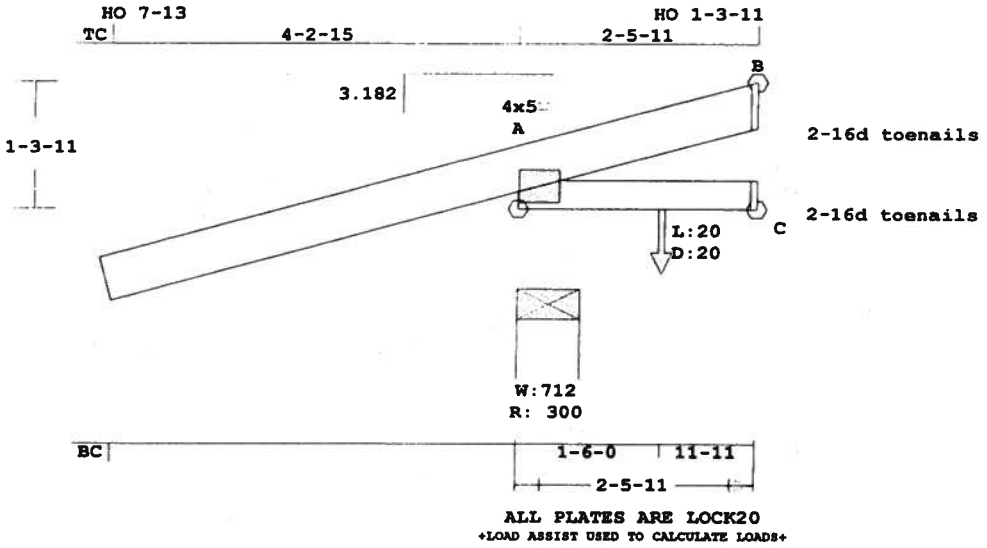
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 42 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R238*	1	KING	20511	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.01	2x 6	SP-#2
BC	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	2- 5-11
BC Cont.	0- 0- 0	2- 5-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	2.5'
BC V	0	20	0.0'	2.5'
TC V	-40	-20	0.0'	
	-31	-15		2.5'
BC V	0	-20	0.0'	
	0	-15		2.5'
BC V	20	20	1.5'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	300	0	7-12	1- 0
B	19	31	3- 8	1- 0
C	27	22	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.01	8 C	0.00	0.01
--------Bottom Chords---------				
A -C	0.03	1 T	0.00	0.03

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in A -C 0.02

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	4.0x 5.0 Ctr	Ctr	0.71

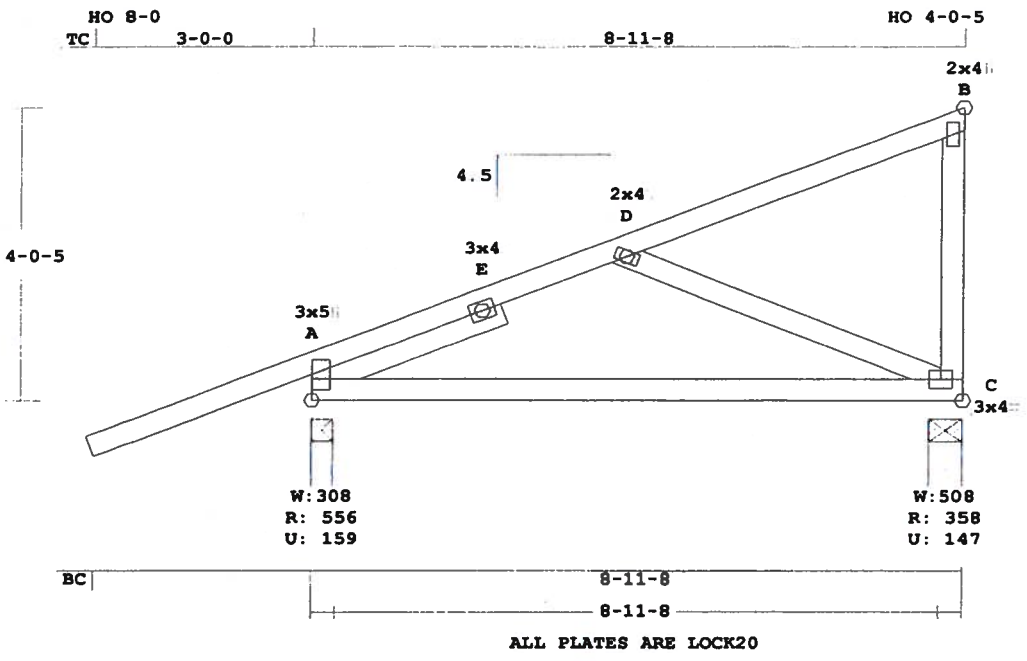
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 1- 7- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R239*	1	MONO	81108	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 62.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.27	2x 4	SP-#2
BC	0.47	2x 4	SP-#2
WB	0.17	2x 4	SP-#3
SL	0.13	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8-11- 8
BC Cont.	0- 0- 0	8-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	557	159	3- 8	1- 0
			Hz =	151
C	358	148	5- 8	1- 0
			Hz =	81

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -E	0.16	715 C	0.00 0.16
E -D	0.24	405 C	0.00 0.24
D -B	0.27	82 C	0.00 0.27
Bottom Chords			
A -C	0.47	413 T	0.03 0.44
Webs			
D -C	0.17	446 C	
C -B	0.05	216 T	
Sliders			
A -E	0.13	493 T	

LL Defl -0.12" in A -C L/800
TL Defl -0.25" in A -C L/396
Shear // Grain in A -C 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

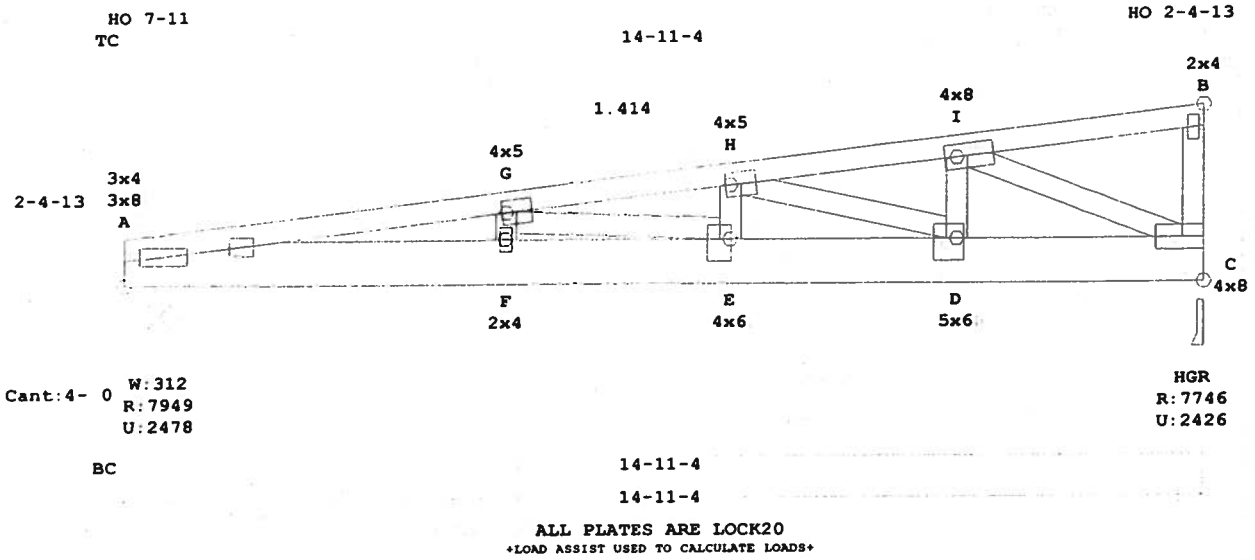
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.73
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.37
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	3.0x 4.0	Ctr	Ctr	0.52

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 715 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R240*	1*3P	MONO	141104	1.414	0	0	

Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 108.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.89	2x 4	SP-#2
BC	0.69	2x 8	SP-2400
WB	0.74	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	14-11- 4
BC Cont.		0- 0- 0	14-11- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.9'
BC V	485	505	0.0'	14.9'

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	7950	2478	3-12	2- 0
			Hz =	79
C	7746	2427	3- 8	2- 2
			Hz =	74

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.89	24677 C	0.53 0.36
G -H	0.75	21588 C	0.40 0.35
H -I	0.33	12274 C	0.29 0.04
I -B	0.05	14 C	0.00 0.05
--------Bottom Chords---------			
A -F	0.68	24492 T	0.28 0.40
F -E	0.69	24492 T	0.28 0.41
E -D	0.43	21453 T	0.24 0.19
D -C	0.27	12194 T	0.14 0.13
------------Webs------------			
F -G	0.12	1211 T	
G -E	0.16	3141 C	
E -H	0.46	4347 T	
H -D	0.52	9903 C	
D -I	0.74	7013 T	
I -C	0.71	13502 C	
C -B	0.00	117 C	

LL Defl -0.25" in F -E L/668
TL Defl -0.50" in F -E L/334
Shear // Grain in A -F 0.50

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.76
A	LOCK	3.0x 4.0	Ctr	Ctr	0.76
G	LOCK	4.0x 5.0	Ctr	Ctr	0.75
H	LOCK	4.0x 5.0	Ctr	Ctr	0.94
I	LOCK	4.0x 8.0	0.2	Ctr	0.78
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	2.0x 4.0	Ctr	Ctr	0.43
E	LOCK	4.0x 6.0	Ctr	-0.6	0.81
D	LOCK	5.0x 6.0	0.5	-0.6	0.87
C	LOCK	4.0x 8.0	-0.5	Ctr	0.77

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder Common
Loading BC
Span 50- 6- 8
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

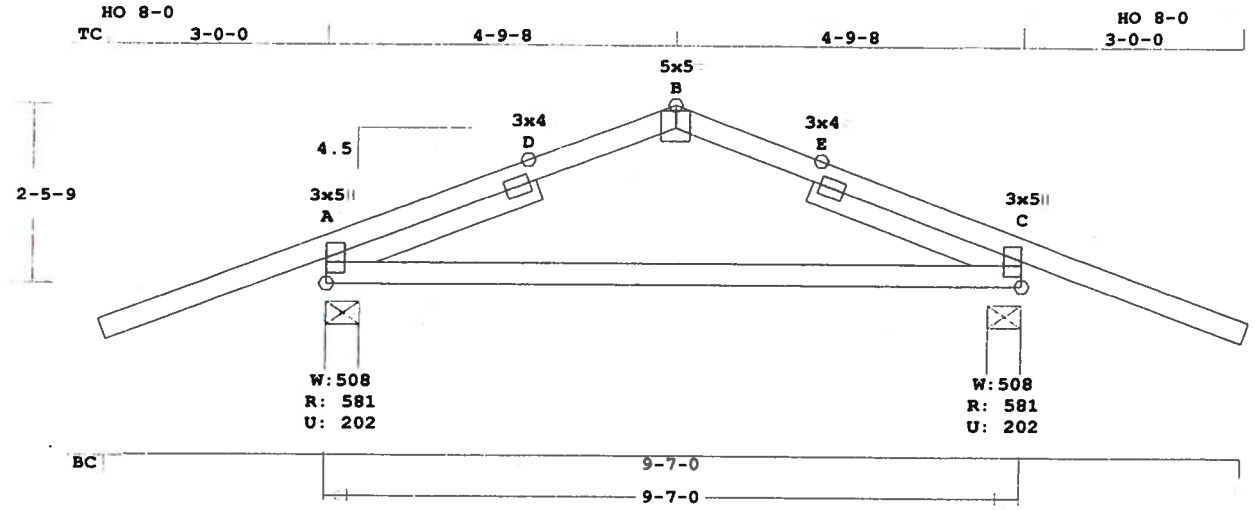
	Rows	----Spacing (In)---- Nails	Screws	Bolts
TC	1	12	24	0
BC	2	7	12.5	0
WB	1	4	4	

No bolts in 2x4s or smaller.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 24677 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R241*	5	TR	90700	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.19	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.09	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 7- 0
BC Cont.	0- 0- 0	9- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	582	203	5- 8	1- 0
C	582	203	5- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
**----------Top Chords----------				
A -D	0.17	609 C	0.00	0.17
D -B	0.19	498 T	0.06	0.13
B -E	0.19	498 T	0.06	0.13
E -C	0.17	609 C	0.00	0.17
--------Bottom Chords---------				
A -C	0.35	402 T	0.03	0.32
**----------Sliders----------				
A -D	0.09	347 T		
E -C	0.09	347 T		

LL Defl -0.08" in A -C L/999
TL Defl -0.16" in A -C L/636
Shear // Grain in A -C 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.74
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	5.0x 5.0	Ctr	-1.6	0.38
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0	-1.5	0.3	0.74

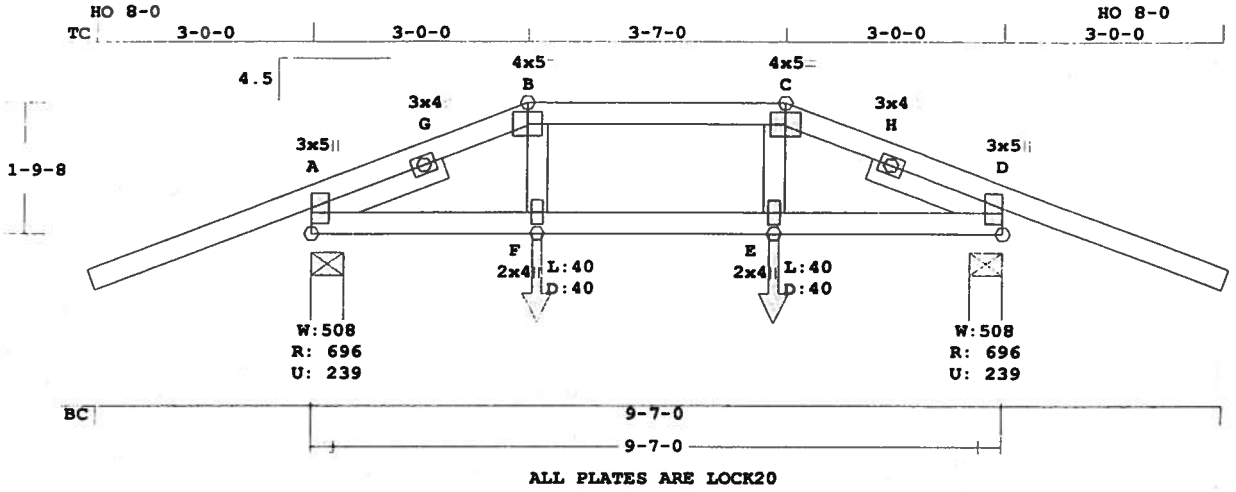
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 2-11- 8. Support end or provide level return.
Excessive Right OH condition.
Max allowable length is 2-11- 8. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 609 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R242	2	HIPP	90700	4.5	3- 0- 0	3- 0- 0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.04	2x 4	SP-#3
SL	0.07	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 7- 0
BC Cont.	0- 0- 0	9- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.6'
BC V	0	20	0.0'	9.6'
TC V	10	5	3.0'	6.6'
BC V	0	5	3.1'	6.4'
BC V	40	40	3.1'	CL-LB
BC V	40	40	6.4'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	697	239	5- 8	1- 0
D	697	239	5- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.13	484 C	0.00 0.13
G -B	0.15	788 C	0.10 0.05
B -C	0.24	730 C	0.09 0.15
C -H	0.15	788 C	0.10 0.05
H -D	0.13	484 C	0.00 0.13
--------Bottom Chords---------			
A -F	0.26	730 T	0.13 0.13
F -E	0.22	730 T	0.13 0.09
E -D	0.26	730 T	0.13 0.13
----------Webs----------			
F -B	0.04	174 T	
E -C	0.04	174 T	
----------Sliders----------			
A -G	0.07	360 C	
H -D	0.07	360 C	

LL Defl -0.02" in F -E L/999
TL Defl -0.04" in F -E L/999
Shear // Grain in B -C 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.74
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.67
C	LOCK	4.0x 5.0	Ctr	Ctr	0.67
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	3.0x 5.0-1.5	0.3		0.74
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

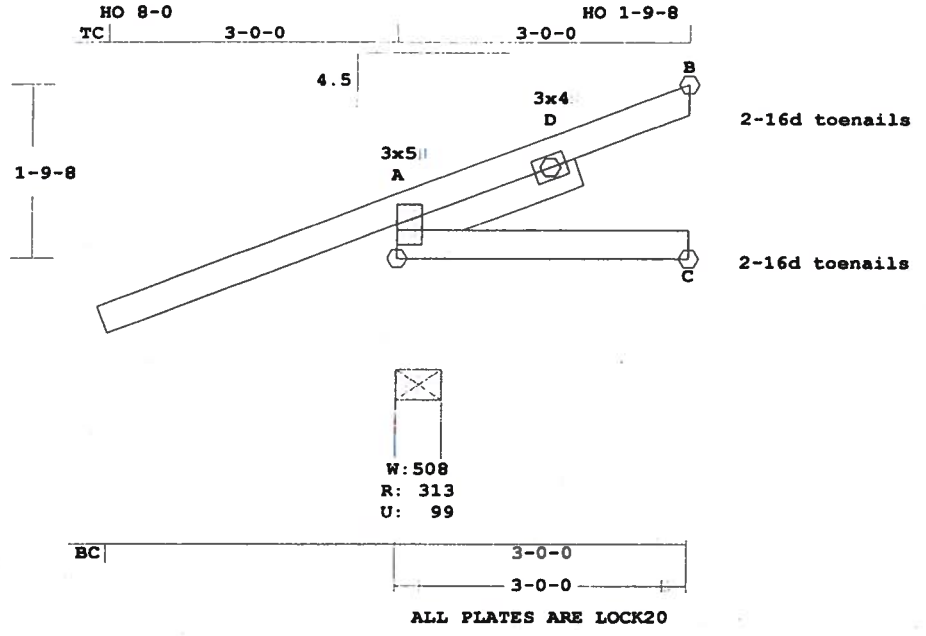
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 3- 0- 0
OH Loading
Soffit psf 2.0

Excessive Left OH condition.
Max allowable length is 2-11- 8. Support end or provide level return.
Excessive Right OH condition.
Max allowable length is 2-11- 8. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 788 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R243*	6	JACK	30000	4.5	3- 0- 0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
SL	0.01	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	314	100	5- 8	1- 0
			Hz =	51
B	91	63	3- 8	1- 0
C	51	0	3- 8	1- 0
			Hz =	27

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -D	0.11		58 C	0.00	0.11
D -B	0.11		66 C	0.00	0.11

--------Bottom Chords---------

A -C	0.09		27 T	0.00	0.09

----------Sliders-----------

A -D	0.01	85 C	

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in D -D 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.65
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

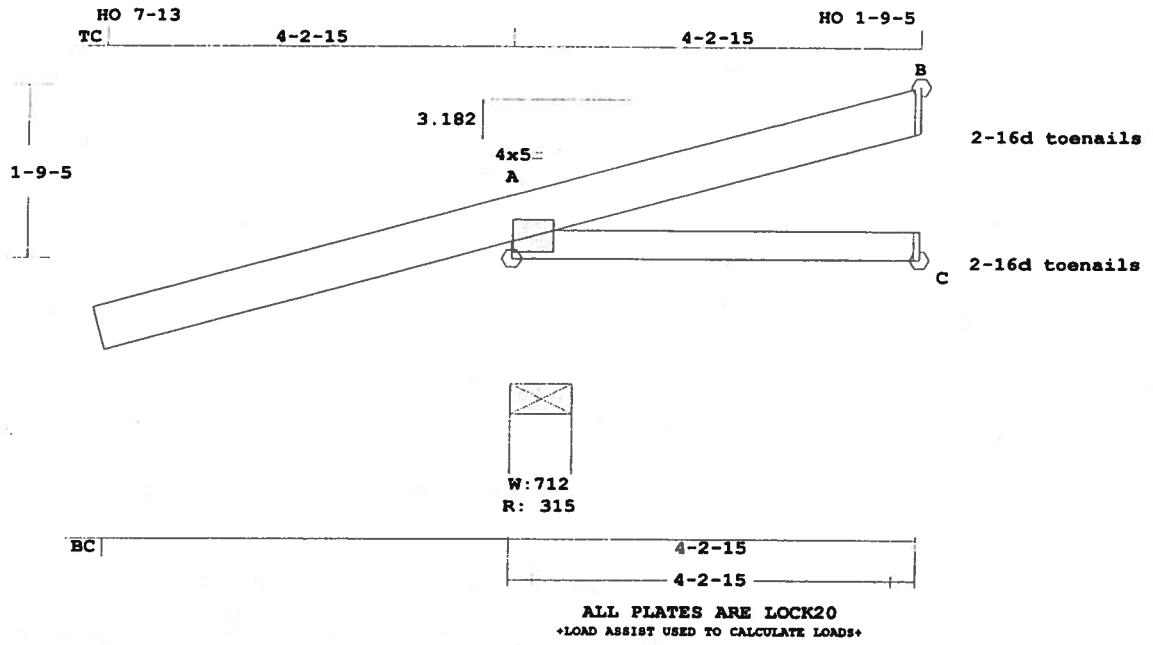
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 85 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R244	4	KING	40215	3.182	4- 2-15	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.05	2x 6	SP-#2
BC	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 2-15
BC Cont.	0- 0- 0	4- 2-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	4.2'
BC V	0	20	0.0'	4.2'
TC V	-40	-20	0.0'	
	-12	-6		4.2'
BC V	0	-20	0.0'	
	0	-6		4.2'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	315	0	7-12	1- 0
			Hz =	45
B	64	118	3- 8	1- 0
C	36	0	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.05	31 C	0.00 0.05
--------Bottom Chords---------			
A -C	0.03	9 T	0.00 0.03

LL Defl 0.00" in A -C L/999
TL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.05

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

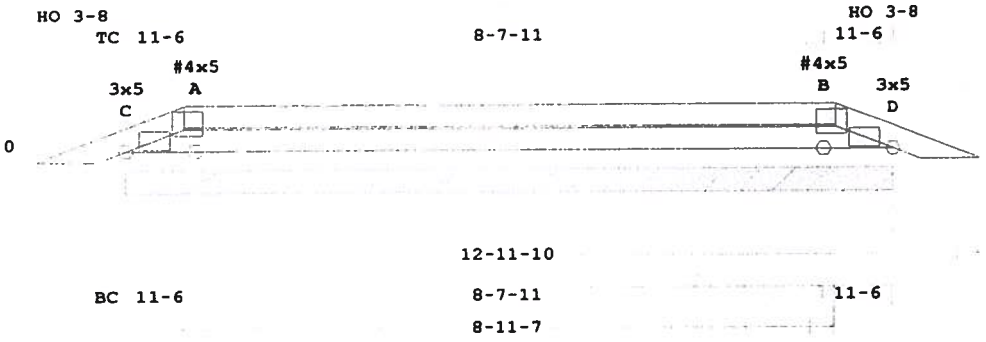
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder King Jack
Loading TC and BC
Setback 3- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 31 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T1*	1	SP	81107	0	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.94	2x 4	SP-#2
BC	0.37	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8-11- 7
BC	Cont.	0- 0- 0	8-11- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 10- 6- 7			
	872	320	Hz =	0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.94	1685	C	0.09 0.85
--------Bottom Chords---------				
C -D	0.37	106	C	0.00 0.37

LL Defl -0.11" in A -B L/999
TL Defl -0.24" in A -B L/530
Shear // Grain in A -B 0.33

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
C	LOCK	3.0x 5.0	Ctr	Ctr	0.60
A#	LOCK	4.0x 5.0	0.5	-0.9	0.51
B#	LOCK	4.0x 5.0	-0.6	-1.2	0.53
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60

= Plate Monitor used

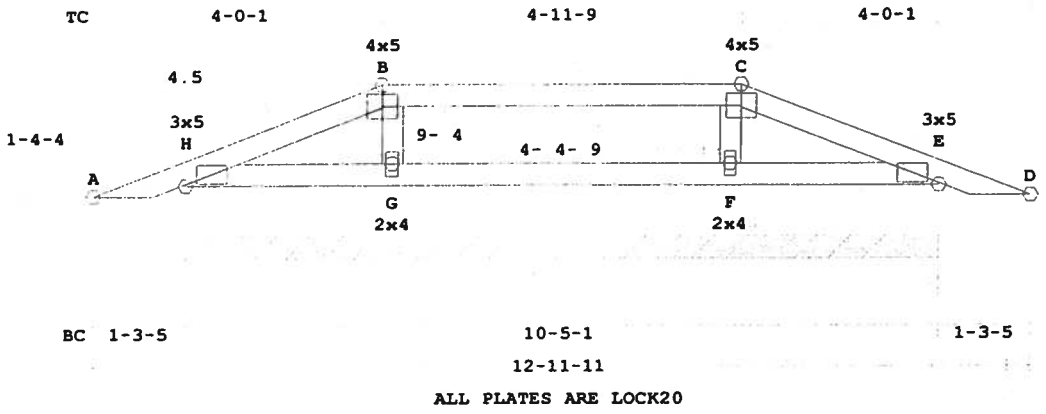
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1685 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T2*	1	PBH	121111	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.19	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11-11
BC Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 997	0- 0- 0 to 329	Hz =	10- 5- 1 2

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
H -B	0.05	88 T	0.00	0.05
B -C	0.19	124 T	0.00	0.19
C -E	0.05	88 T	0.00	0.05
--------Bottom Chords---------				
H -G	0.09	21 T	0.00	0.09
G -F	0.09	0 T	0.00	0.09
F -E	0.09	21 T	0.00	0.09
------------Webs------------				
G -B	0.05	213 C		
F -C	0.05	213 C		

LL Defl -0.01" in G -F L/999
TL Defl -0.02" in G -F L/999
Shear // Grain in B -C 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
H	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	Ctr	0.68
C	LOCK	4.0x 5.0	Ctr	Ctr	0.68
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

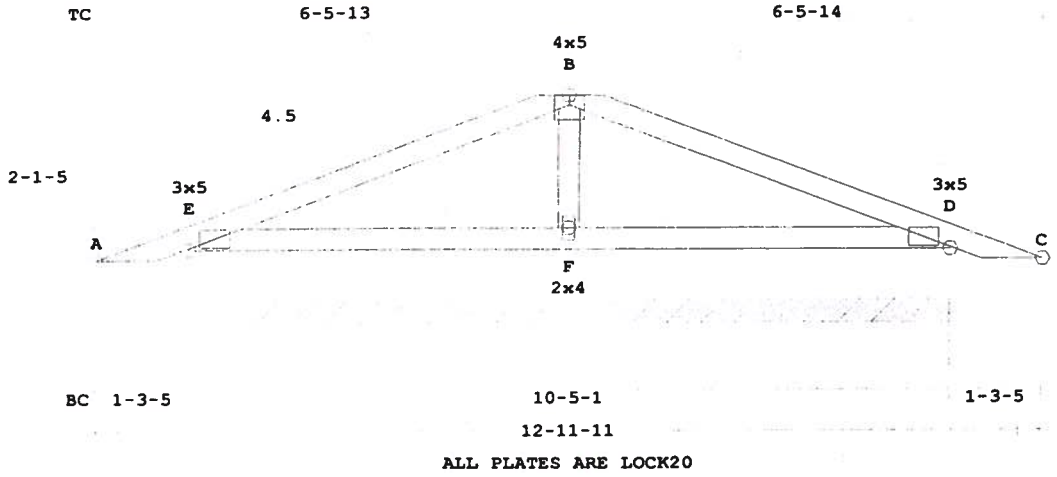
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 213 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T3*	1	PB	121111	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 10- 5- 1			
	997	329	Hz =	4

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
E -B	0.21	355	C	0.03	0.18
B -D	0.21	355	C	0.03	0.18
Bottom Chords					
E -F	0.17	61	C	0.00	0.17
F -D	0.17	61	C	0.00	0.17

Webs

F -B	0.02	84	T

LL Defl -0.01" in F -D L/999
TL Defl -0.02" in F -D L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

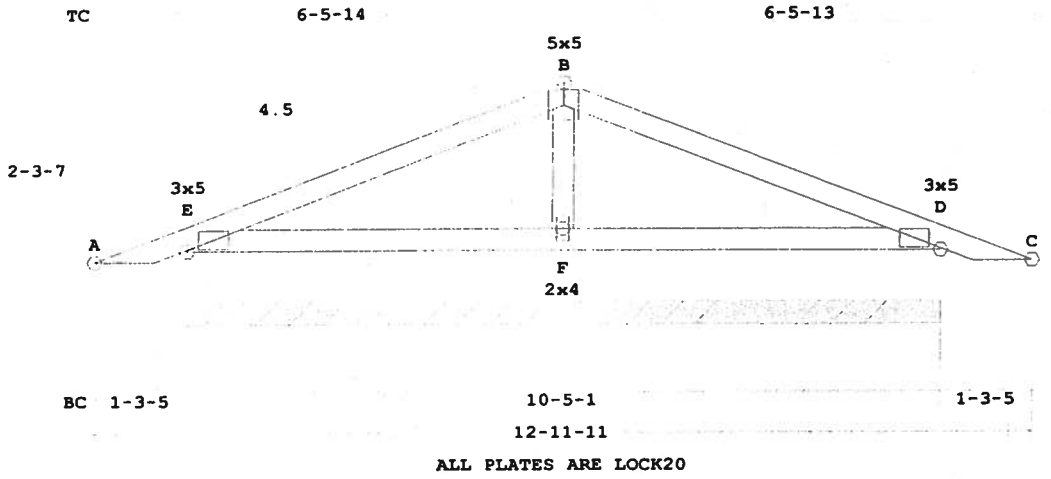
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	-0.4	0.58
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 355 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*T4*	4	PB	121111	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 997	0- 0- 0 to 329 Hz =	10- 5- 1	4

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
E -B	0.21	347 C	0.03	0.18
B -D	0.21	347 C	0.03	0.18
--------Bottom Chords---------				
E -F	0.17	61 C	0.00	0.17
F -D	0.17	61 C	0.00	0.17

----------Webs----------
F -B 0.02 91 T

LL Defl -0.01" in F -D L/999
TL Defl -0.02" in F -D L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

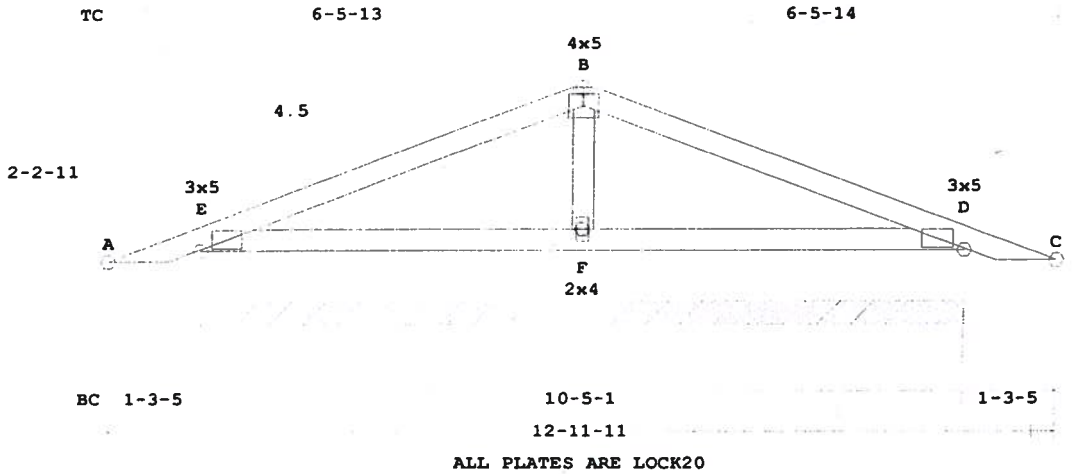
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	5.0x 5.0	Ctr	Ctr	0.42
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 347 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T5*	1	PB	121111	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11-11
BC Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 10- 5- 1			
	997	329	Hz =	4

Membr	CSI	P Lbs	Axl	CSI-Bnd	
----------Top Chords----------					
E -B	0.21	347	C	0.03	0.18
B -D	0.21	347	C	0.03	0.18
--------Bottom Chords---------					
E -F	0.17	61	C	0.00	0.17
F -D	0.17	61	C	0.00	0.17

----------Webs----------
F -B 0.02 90 T

LL Defl -0.01" in F -D L/999
TL Defl -0.02" in F -D L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
E LOCK	3.0x 5.0	Ctr	Ctr	0.60
B LOCK	4.0x 5.0	Ctr	Ctr	0.50
D LOCK	3.0x 5.0	Ctr	Ctr	0.60
F LOCK	2.0x 4.0	Ctr	Ctr	0.38

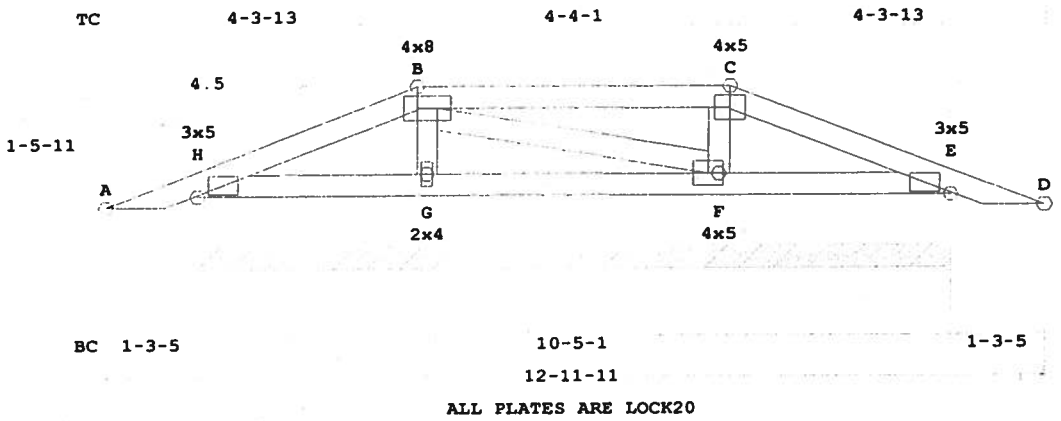
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 347 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc /Online Plus™ © 1996-2005 Version 18 0 020 Engineering - Portrait 01/26/06 9 45 56 AM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T6*	1	PBH	121111	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11-11
BC Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 10- 5- 1			
	997	329	Hz =	3

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
H -B	0.07	137 T	0.01	0.06
B -C	0.13	165 T	0.00	0.13
C -E	0.07	130 T	0.01	0.06
--------Bottom Chords---------				
H -G	0.07	30 T	0.00	0.07
G -F	0.07	0 T	0.00	0.07
F -E	0.07	30 T	0.00	0.07
------------Webs------------				
G -B	0.04	197 C		
B -F	0.01	47 C		
F -C	0.04	177 T		

LL Defl 0.00" in G -F L/999
TL Defl -0.01" in G -F L/999
Shear // Grain in B -C 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
H	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 8.0	Ctr	Ctr	0.68
C	LOCK	4.0x 5.0	Ctr	Ctr	0.68
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.76

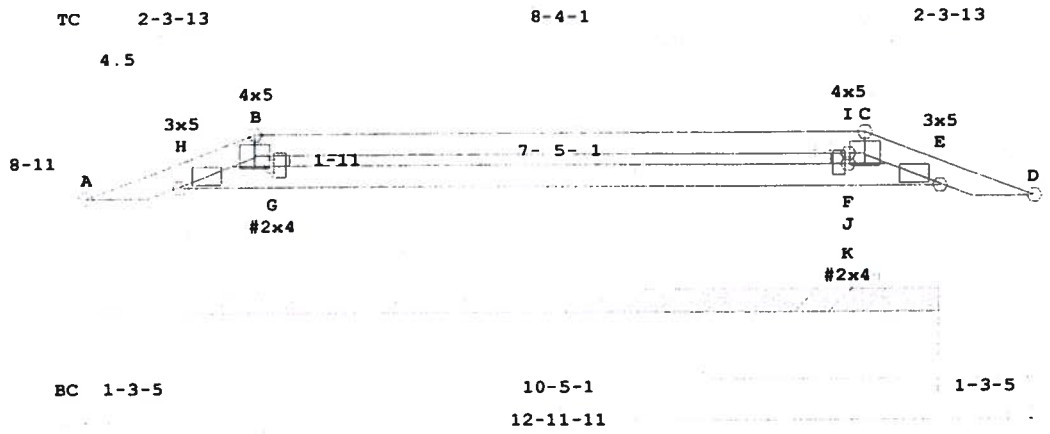
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 197 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T7*	1	PBH	121111	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
WB	0.05	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	12-11-11
BC Cont.		0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 10- 5- 1			
	997	329	Hz =	1

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
H -B	0.13	145 T	0.02	0.11
B -I	0.49	35 T	0.00	0.49
I -E	0.13	145 T	0.02	0.11
--------Bottom Chords---------				
H -G	0.29	126 T	0.00	0.29
G -K	0.29	0 T	0.00	0.29
K -E	0.29	126 T	0.00	0.29
------------Webs------------				
G -B	0.05	613 C		
K -I	0.05	613 C		

LL Defl -0.06" in G -F L/999
TL Defl -0.11" in G -F L/999
Shear // Grain in B -B 0.42

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
H	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.73
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
G#	LOCK	2.0x 4.0	1.2	0.1	0.45
K#	LOCK	2.0x 4.0-1.5	0.2		0.45

= Plate Monitor used

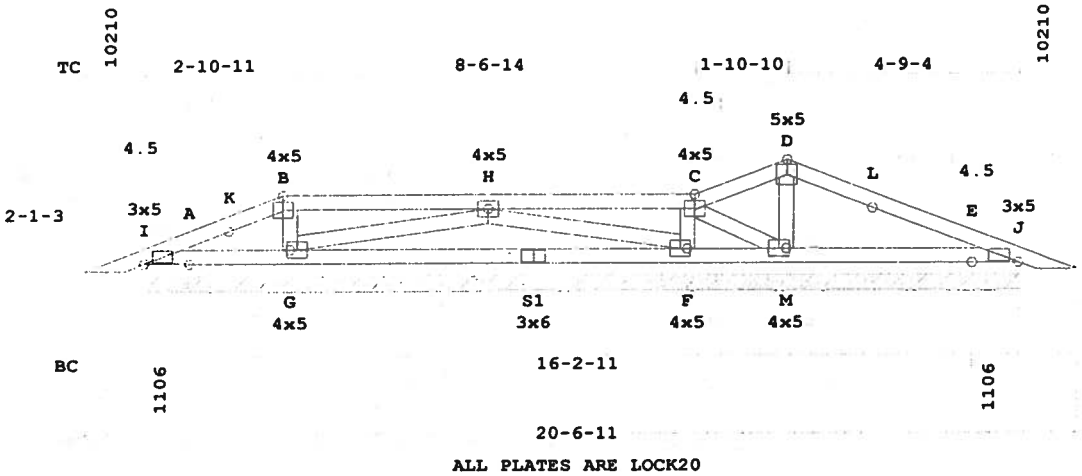
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 613 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T8*	1	SP	200611	4.5	1- 2-10	1- 2-10	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 99.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.17	2x 4	SP-#2
BC	0.14	2x 4	SP-#2
WB	0.17	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	20- 6-11
BC Cont.		0- 0- 0	20- 6-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 18- 1- 7			
	1605	357	Hz =	4

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
I -B	0.04	91	C	0.00	0.04
B -H	0.17	94	T	0.00	0.17
H -C	0.17	186	C	0.00	0.17
C -D	0.08	156	T	0.02	0.06
D -J	0.17	137	C	0.00	0.17
--------Bottom Chords---------					
I -G	0.11	23	C	0.00	0.11
G -S1	0.11	0	T	0.00	0.11
S1-F	0.11	0	T	0.00	0.11
F -M	0.11	0	T	0.00	0.11
M -J	0.14	109	C	0.00	0.14
----------Webs----------					
G -B	0.03	167	C		
G -H	0.17	589	C		
H -F	0.14	490	C		
F -C	0.04	175	C		
C -M	0.02	90	T		
M -D	0.03	132	T		

LL Defl -0.01" in G -S1 L/999
TL Defl -0.02" in M -E L/999
Shear // Grain in B -H 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
I	LOCK	3.0x 5.0	Ctr	Ctr	0.72
B	LOCK	4.0x 5.0	Ctr	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.76
C	LOCK	4.0x 5.0	Ctr	Ctr	0.82
D	LOCK	5.0x 5.0	Ctr	Ctr	0.51
J	LOCK	3.0x 5.0	Ctr	Ctr	0.72
G	LOCK	4.0x 5.0	Ctr	Ctr	0.76
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.76
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60

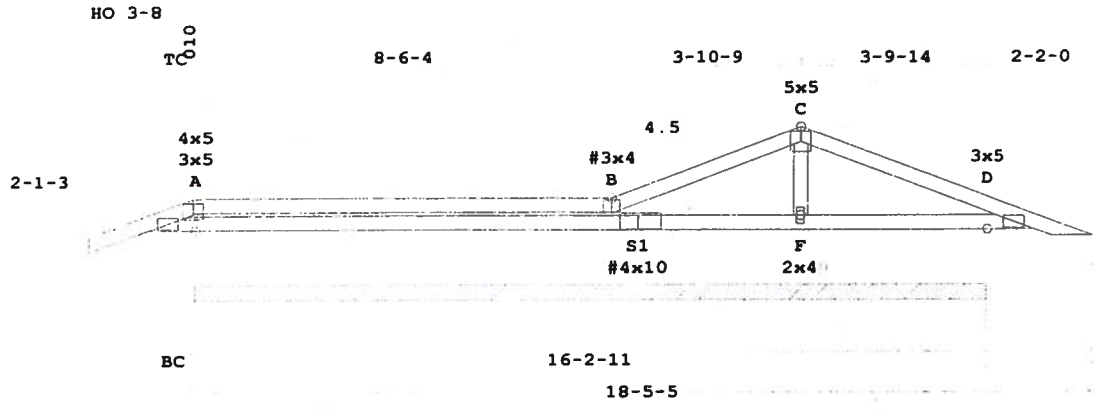
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:

FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 589 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T9*	1	SP	180505	4.5	10	2- 2- 0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 77.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
EX A -B		2x 4	SP-2400
BC	0.78	2x 4	SP-#2
WB	0.23	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	18- 5- 5
BC	Cont.	0- 0- 0	18- 5- 5

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0-11- 6	to	17- 2- 1	
	1534	411	Hz =	5

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.64	1022 T	0.07	0.57
B -C	0.44	1095 T	0.19	0.25
C -D	0.43	1114 T	0.19	0.24
--------Bottom Chords---------				
A -S1	0.78	4 C	0.00	0.78
S1-F	0.59	0 T	0.00	0.59
F -D	0.26	7 C	0.00	0.26
------------Webs------------				
F -C	0.23	1077 C		

LL Defl -0.24" in A -S1 L/823
TL Defl -0.53" in A -S1 L/368
Shear // Grain in A -B 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.72
A	LOCK	4.0x 5.0	Ctr-1.1		0.64
B	LOCK	3.0x 4.0	Ctr-0.2		0.55
C	LOCK	5.0x 5.0	Ctr	Ctr	0.51
D	LOCK	3.0x 5.0	Ctr	Ctr	0.72
S1	LOCK	4.0x10.0	0.6	0.3	0.88
F	LOCK	2.0x 4.0	Ctr	Ctr	0.77

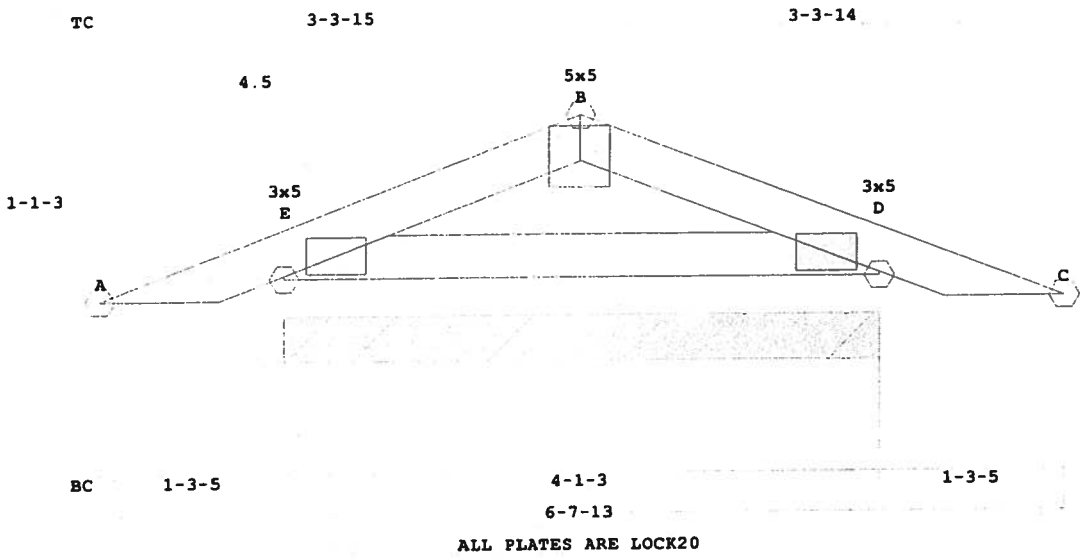
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as

Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1077 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T10*	18	PB	60713	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.05	2x 4	SP-#2
BC	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	6- 7-13
BC	Cont.	0- 0- 0	6- 7-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 4- 1- 3			
	491	171	Hz =	2

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
E -B	0.05	231	T	0.03 0.02
B -D	0.05	231	T	0.03 0.02
--------Bottom Chords---------				
E -D	0.04	18	C	0.00 0.04

LL Defl 0.00" in E -D L/999
TL Defl -0.01" in E -D L/999
Shear // Grain in E -B 0.07

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

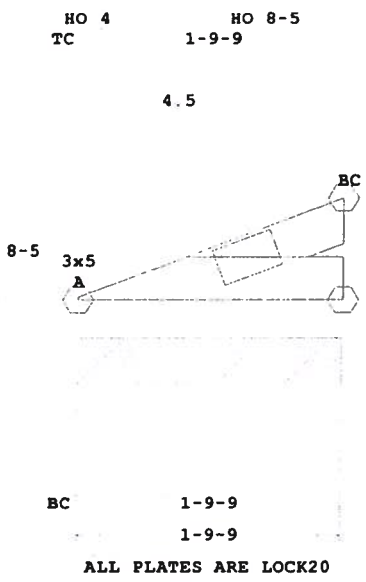
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	Ctr	Ctr	0.51
B	LOCK	5.0x 5.0	Ctr	-1.6	0.33
D	LOCK	3.0x 5.0	Ctr	Ctr	0.51

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 178 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V5*	3	VLM	10909	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1"

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 5.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.08	2x 4	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	1- 9- 9
BC	Cont.	0- 0- 0	1- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 1- 9- 9			
	143	63	Hz =	34

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -A	0.08	19 C	0.00 0.08
A -B	0.08	10 C	0.00 0.08
--------Bottom Chords---------			

LL Defl 0.00" in E -D L/999
TL Defl 0.00" in E -D L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.51

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 19 Lbs
Quality Control Factor 1.25

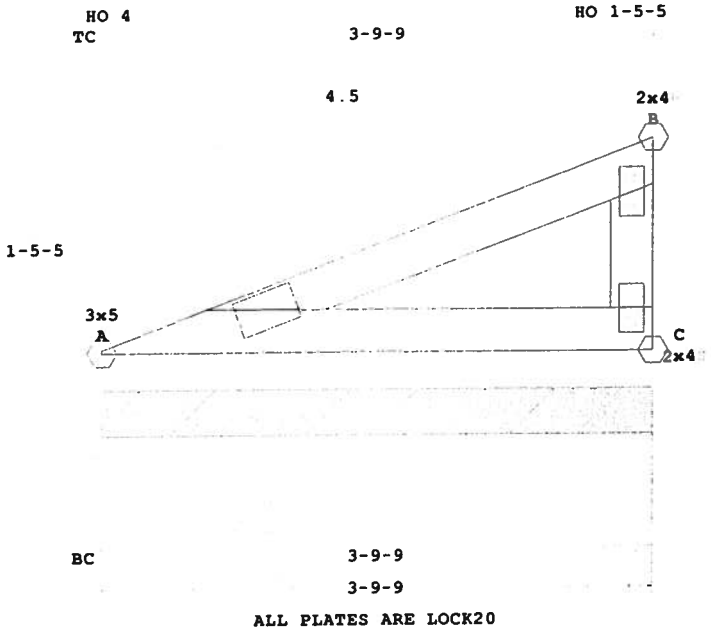
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc./Online Plus™ © 1996-2005 Version 18.0.020 Engineering - Portrait 01/26/06 9:45:57 AM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V6*	3	VLM	30909	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
WB	0.03	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 9- 9
BC	Cont.	0- 0- 0	3- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to		3- 9- 9	
	244	76	Hz =	51

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	51 C	0.00 0.07
--------Bottom Chords---------			
A -C	0.02	0 T	0.00 0.02
------------Webs------------			
C -B	0.03	135 T	

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

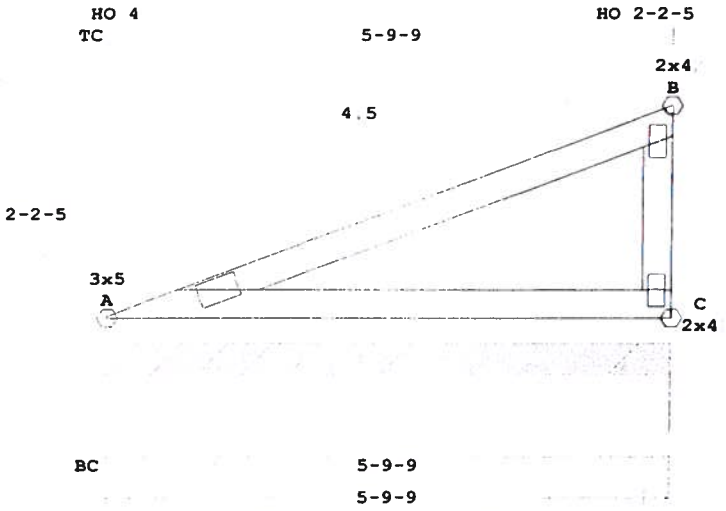
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.51
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 67 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V7*	3	VLM	50909	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 4	SP-#2
BC	0.11	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 9- 9
BC Cont.	0- 0- 0	5- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		5- 9- 9	
	404	126	Hz =	85

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.31	85 C	0.00 0.31
--------Bottom Chords---------			
A -C	0.11	0 T	0.00 0.11
------------Webs------------			
C -B	0.05	224 T	

LL Defl -0.01" in A -C L/999
TL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

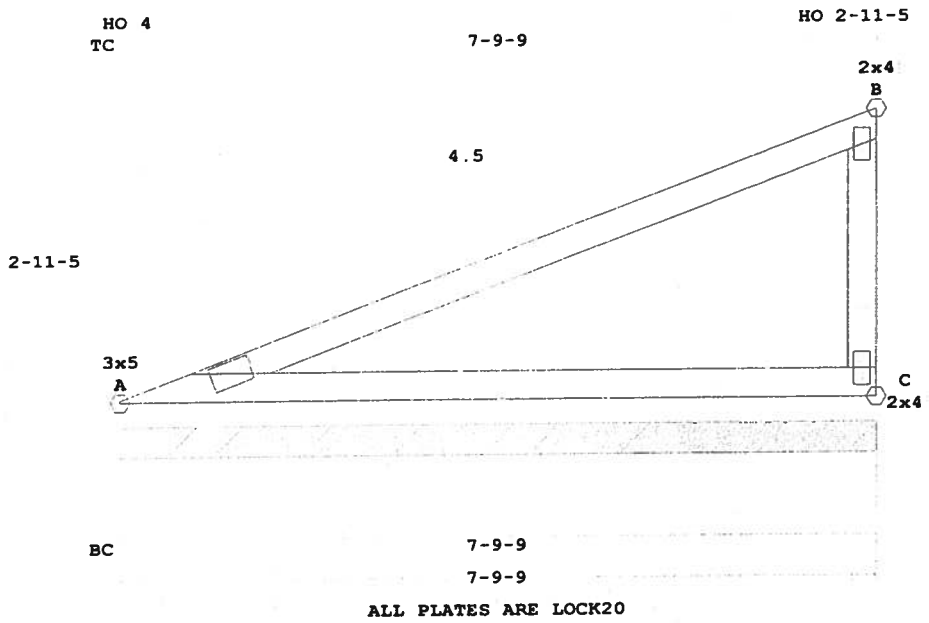
Jt	Type	Plt Size X	Y	JSI
A	LOCK	3.0x 5.0 Ctr	Ctr	0.52
B	LOCK	2.0x 4.0 Ctr	Ctr	0.38
C	LOCK	2.0x 4.0 Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 112 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V8*	3	VLM	70909	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 9- 9
BC	Cont.	0- 0- 0	7- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 7- 9- 9			
	564	176	Hz =	119

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.47	102 C	0.00	0.47
--------Bottom Chords---------				
A -C	0.26	0 T	0.00	0.26
------------Webs------------				
C -B	0.06	264 T		

LL Defl -0.04" in A -C L/999
TL Defl -0.08" in A -C L/999
Shear // Grain in A -B 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

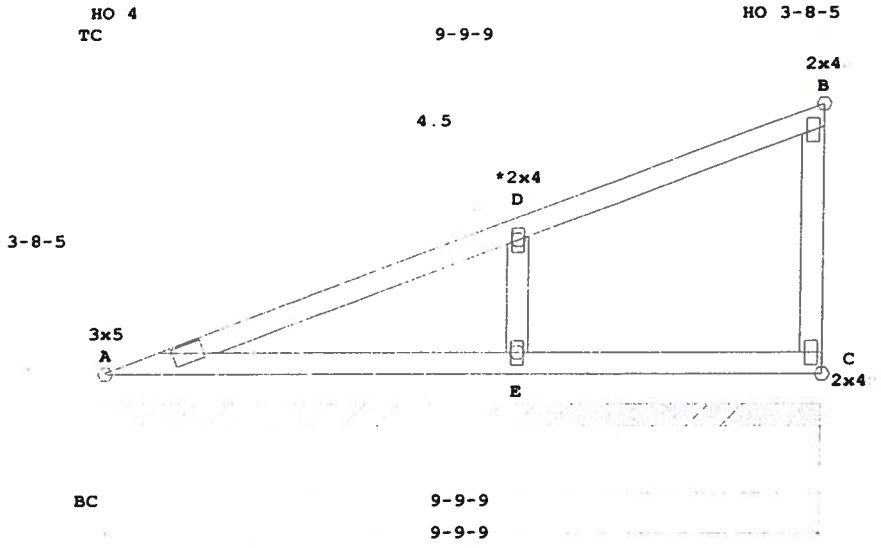
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.55
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 158 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V9*	2	VLM	90909	4.5	0	0	

Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.11	2x 4	SP-#2
WB	0.04	2x 4	SP-#3
GW	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 9- 9
BC Cont.	0- 0- 0	9- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 9- 9- 9			
	724	226	Hz =	153

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.20	200 C	0.00	0.20
D -B	0.20	68 C	0.00	0.20
--------Bottom Chords---------				
A -E	0.11	0 T	0.00	0.11
E -C	0.11	0 T	0.00	0.11
------------Webs------------				
C -B	0.04	177 T		
----------Gable Webs----------				
E -D	0.08	342 T		

LL Defl -0.01" in E -C L/999
TL Defl -0.01" in E -C L/999
Shear // Grain in D -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.59
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 286 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

06176.02

Ridgway Roof Truss Company

(Trusses and Prefabricated Building Components)

Mailing: P.O. Box 1309 – Gainesville, Florida 32602
Physical: 235 SW 11th Place – Gainesville, Florida 32601

Telephone: (352) 376-4436
FAX: (352) 371-3316

Email: Sales@RidgwayTruss.com
www.RidgwayTruss.com

WARNING

THESE TRUSSES MUST BE HANDLED ACCORDING TO BCSI-B1 SUMMARY SHEETS

SEE TABLE OF CONTENTS

M.M. PARRISH CONSTRUCTION CO.
3455 S.W. 42ND AVENUE, GAINESVILLE, FL 32608
PHONE (352) 378-1571 • FAX (352) 377-0669
SUBMITTAL #: 06-2 REVIEWED BY: AC
DATE: 3/22/06
✓ REVIEWED
___ CORRECTIONS NOTED
___ REVISE AND RESUBMIT
REVIEW BY M.M. PARRISH CONSTRUCTION CO. DOES NOT RELIEVE SUBCONTRACTOR AND/OR SUPPLIER FROM ANY REQUIREMENTS OF THE CONTRACT DOCUMENTS. SUBCONTRACTOR AND/OR SUPPLIER IS RESPONSIBLE TO FURNISH AND INSTALL MATERIAL IN STRICT ACCORDANCE WITH THE CONTRACT DOCUMENTS UNLESS SPECIFICALLY WAIVED HEREIN.

SEALED

DESIGN MANUAL

PROJECT NAME: Hospice Office and Care Center
JOB NUMBER: 06-0039
CONTRACTOR: M.M. Parrish Construction Company
DATE: 03-16-06
REVISIONS: _____
COMMENTS: _____

SPUTO and LAMMERT
ENGINEERING, LLC
STRUCTURAL ENGINEERS

☒ REVIEWED
☐ REVIEWED AS NOTED
☐ REVISE AND RESUBMIT
☐ NOT REVIEWED

BY KL DATE 3/22/06

THIS DOCUMENT HAS BEEN REVIEWED FOR GENERAL CONFORMANCE WITH DESIGN CONCEPTS AND GENERAL AGREEMENT WITH CONTRACT DOCUMENTS. SUCH REVIEW DOES NOT RELIEVE THE CONTRACTOR OF ANY RESPONSIBILITY FOR THE WORK.

RIDGWAY ROOF TRUSS COMPANY

P.O. BOX 1309 GAINESVILLE, FL 32602, TELEPHONE: (352) 376-4436 FAX: (352) 371-3316

Job: **Hospice Office and Care Center**

Customer: **M.M. Parrish Construction** Jurisdiction: **Columbia**

Structural Eng. of Record: Sputo & Lammert License # : PE39142 building designer

Address of Eng. Of Record: 10 SW 1st Ave.
Gainesville Florida 32601

SIGNATURE of Building Designer:

I, Gary G. Dounson, Florida Professional Engineer No.35054, hereby certify that I am the truss engineer of record with responsibility for the design of the metal plate connected wood trusses listed herein. The design of these trusses have been performed under my responsible supervision, direction and control and are in accordance with the National Design Standard for Metal Plate Connected Wood Truss Construction (ANSI TPI 1). Building Designer of Record responsibilities shall be required in Article 2.2 of the aforementioned TPI 1 document and shall include but are not limited to the design and detailing of truss supports, anchorage, and permanent truss bracing.

Engineering Program is **Robbins Engineering Online Plus** Building code and chapter: ANSI/ASCE 7-02

Live load: 20 lbs Dead load: 20 lbs Roof Wind speed: 110 mph Design criteria: TPI 1-95

Mean Roof Height: 25 ' Exposure: B Enclosure: Enclosed Category: II

#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date
1	R1	3/16/06	29	R24	3/15/06	57	R43	3/15/06	85	R70	3/15/06	107	R93	3/15/06
2	R2	3/15/06	30	R25	3/15/06	58	R43A	3/15/06	86	R71	3/15/06	108	R94	3/15/06
3	R2A	3/16/06	31	R26	3/15/06	59	R44	3/15/06	87	R72	3/15/06	109	R95	1/26/06
4	R3	3/15/06	32	R27	3/15/06	60	R44A	3/15/06	88	R73	3/15/06	110	R96	1/26/06
5	R4	3/15/06	33	R28	3/15/06	61	R45	3/15/06	89	R74	3/15/06	111	R97	1/26/06
6	R4A	3/15/06	34	R29	3/15/06	62	R45A	3/15/06	90	R75	3/15/06	112	R98	1/26/06
7	R4B	3/15/06	35	R30	3/15/06	63	R46	3/15/06	91	R76	2/16/06	113	R99	1/26/06
8	R5	3/15/06	36	R31	3/15/06	64	R46A	3/15/06	92	R77	1/26/06	114	R100	1/26/06
9	R5A	3/15/06	37	R32	3/15/06	65	R47	3/15/06	93	R77A	2/16/06	115	R101	1/26/06
10	R6	3/15/06	38	R33	3/15/06	66	R47A	3/15/06	94	R78	3/15/06	116	R102	1/26/06
11	R6A	3/15/06	39	R34	3/15/06	67	R48	3/15/06	95	R79	3/15/06	117	R104	3/15/06
12	R7	3/15/06	40	R34A	3/15/06	68	R48A	3/15/06	96	R80	3/15/06	118	R105	3/15/06
13	R11	3/15/06	41	R35	3/15/06	69	R49	3/15/06	97	R81	3/15/06	119	R106	3/15/06
14	R12	3/15/06	42	R35A	2/10/06	70	R49A	3/15/06	98	R83	3/15/06	120	R107	3/15/06
15	R13	3/15/06	43	R36	3/15/06	71	R50	3/15/06	99	R84	3/15/06	121	R108	3/15/06
16	R14	3/15/06	44	R36A	2/10/06	72	R51	3/15/06	100	R86	3/15/06	122	R109	3/15/06
17	R15	3/15/06	45	R37	3/15/06	73	R56	2/13/06	101	R87	2/14/06	123	R110	3/15/06
18	R16	3/14/06	46	37A	2/10/06	74	R57	3/15/06	102	R88	3/15/06	124	R112	3/15/06
19	R17	3/15/06	47	R38	3/15/06	75	R60	3/15/06	103	R89	3/15/06	125	R113	3/15/06
20	R17A	3/15/06	48	R38A	1/25/06	76	R61	3/15/06	104	R90	3/15/06	126	R113A	3/15/06
21	R18	3/15/06	49	R39	3/15/06	77	R62	3/15/06	105	R91	3/15/06	127	R114	3/15/06
22	R18A	3/15/06	50	R39A	1/25/06	78	R63	3/15/06	106	R92	3/15/06	128	R115	3/15/06
23	R18B	3/15/06	51	R40	3/15/06	79	R64	3/15/06						
24	R19	3/15/06	52	R40A	3/15/06	80	R65	3/15/06						
25	R20	3/15/06	53	R41	3/15/06	81	R66	3/15/06						
26	R21	3/15/06	54	R41A	3/15/06	82	R67	3/15/06						
27	R22	3/15/06	55	R42	3/15/06	83	R68	3/15/06						
28	R23	3/15/06	56	R2A	3/15/06	84	R69	3/15/06						

This Package includes 128 individual, dated Truss Design Drawings. With my Seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003 section 5 of the Florida Board of Professional Engineers.

Delegated Engineer (Truss Designer)
GARY DOUNSON, PE 35054
GARY DOUNSON & ASSOCIATES, INC.
2830 NW 41st STREET SUITE D
GAINESVILLE, FL 32606
(352) 375-8593
CA 5201
3/16/2006

Note: The seal on this index sheet indicates acceptance of professional engineering responsibility solely for the Truss Design Drawings listed above and attached. The suitability and use of each component for any particular building is the responsibility of the Building Designer, per ANSI/TPI 1-1995 Section 2.

3-16-06

RIDGWAY ROOF TRUSS COMPANY

P.O. BOX 1309 GAINESVILLE, FL 32602, TELEPHONE: (352) 376-4436 FAX: (352) 371-3316

Job: **Hospice Office and Care Center**

Customer: **M.M. Parrish Construction** Jurisdiction: **Columbia**

Structural Eng. of Record: Sputo & Lammert License # : PE39142 building designer

Address of Eng. Of Record: 10 SW 1st Ave.
Gainesville Florida 32601

SIGNATURE of Building Designer:

I, Gary G. Dounson, Florida Professional Engineer No.35054, hereby certify that I am the truss engineer of record with responsibility for the design of the metal plate connected wood trusses listed herein. The design of these trusses have been performed under my responsible supervision, direction and control and are in accordance with the National Design Standard for Metal Plate Connected Wood Truss Construction (ANSI TPI 1). Building Designer of Record responsibilities shall be required in Article 2.2 of the aforementioned TPI 1 document and shall include but are not limited to the design and detailing of truss supports, anchorage, and permanent truss bracing.

Engineering Program is **Robbins Engineering Online Plus** **Building code and chapter:** ANSI/ASCE 7-02

Live load: 20 lbs **Dead load:** 20 lbs Roof **Wind speed:** 110 mph **Design criteria:** TPI 1-95

Mean Roof Height: 25' **Exposure:** B **Enclosure:** Enclosed **Category:** II

#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date
1	R116	3/15/06	29	R150	3/15/06	57	R178	3/15/06	85	R213	3/15/06	107	T2	1/26/06
2	R117	3/15/06	30	R151	3/15/06	58	R179	3/15/06	86	R214	3/15/06	108	T3	1/26/06
3	R118	3/15/06	31	R152	3/15/06	59	R180	3/15/06	87	R215	3/15/06	109	T4	1/26/06
4	R119	3/15/06	32	R153	3/15/06	60	R181	1/26/06	88	R216	3/15/06	110	T5	1/26/06
5	R121	3/16/06	33	R154	3/16/06	61	R182	1/26/06	89	R217	1/26/06	111	T6	1/26/06
6	R122	3/16/06	34	R154A	3/16/06	62	R183	1/26/06	90	R218	3/15/06	112	T7	1/26/06
7	R123	3/16/06	35	R154B	3/16/06	63	R191	3/15/06	91	R236	1/26/06	113	T8	1/26/06
8	R124	3/15/06	36	R155	3/15/06	64	R192	3/15/06	92	R237	3/16/06	114	T9	1/26/06
9	R125	3/15/06	37	R156	3/15/06	65	R193	3/15/06	93	R238	3/15/06	115	T10	1/26/06
10	R126	3/15/06	38	R157	3/15/06	66	R194	3/15/06	94	R239	3/16/06	116	V1	3/16/06
11	R127	3/15/06	39	R159	2/16/06	67	R195	3/15/06	95	R240	3/15/06	117	V2	3/16/06
12	R128	3/15/06	40	R160	2/16/06	68	R196	3/15/06	96	R241	3/16/06	118	V3	3/16/06
13	R134	3/15/06	41	R161	3/15/06	69	R197	3/15/06	97	R242	3/16/06	119	V4	3/16/06
14	R135	3/15/06	42	R162	1/26/06	70	R198	3/15/06	98	R243	3/16/06	120	V5	1/26/06
15	R136	3/15/06	43	R163	1/26/06	71	R199	3/15/06	99	R244	3/16/06	121	V6	1/26/06
16	R137	3/15/06	44	R164	1/26/06	72	R200	3/15/06	100	R245	2/16/06	122	V7	1/26/06
17	R138	3/15/06	45	R165	1/26/06	73	R201	3/15/06	101	R246	2/16/06	123	V8	1/26/06
18	R139	3/15/06	46	R166	1/26/06	74	R202	3/15/06	102	R247	2/16/06	124	V10	3/16/06
19	R140	3/15/06	47	R167	1/26/06	75	R203	3/15/06	103	R248	3/16/06	125		
20	R141	3/15/06	48	R168	1/26/06	76	R204	3/15/06	104	R249	3/16/06	126		
21	R142	1/26/06	49	R168	1/26/06	77	R205	3/15/06	105	R250	3/16/06	127		
22	R143	1/26/06	50	R170	1/26/06	78	R206	3/15/06	106	T1	1/26/06	128		
23	R144	3/15/06	51	R171	1/26/06	79	R207	3/15/06						
24	R145	3/15/06	52	R172	1/26/06	80	R208	3/15/06						
25	R146	3/15/06	53	R173	1/26/06	81	R209	3/15/06						
26	R147	3/15/06	54	R174	1/26/06	82	R210	3/15/06						
27	R148	3/15/06	55	R175	1/26/06	83	R211	3/15/06						
28	R149	3/15/06	56	R176	3/15/06	84	R212	3/15/06						

Delegated Engineer (Truss Designer)
GARY DOUNSON, PE 35054
GARY DOUNSON & ASSOCIATES, INC.
2830 NW 41st STREET SUITE D
GAINESVILLE, FL 32606
(352) 375-8593
CA 5201
3/16/2006

This Package includes 124 individual, dated Truss Design Drawings. With my Seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003 section 5 of the Florida Board of Professional Engineers.

Note: The seal on this index sheet indicates acceptance of professional engineering responsibility solely for the Truss Design Drawings listed above and attached. The suitability and use of each component for any particular building is the responsibility of the Building Designer, per ANSI/TPI 1-1995 Section 2.

3-16-06

TABLE OF CONTENTS

1. GENERAL NOTES

2. WTCA 1-1995 (STANDARD RESPONSIBILITIES IN THE DESIGN PROCESS INVOLVING METAL PLATE CONNECTED WOOD TRUSSES)

ENGINEERING (INSERT)

BLUEPRINT OF TRUSS PLACEMENT PLAN (INSERT)

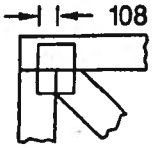
6. **BCSI-B1 SUMMARY SHEET**
HANDLING, INSTALLING & BRACING INFORMATION

8. STANDARD CHORD & WEB REPAIRS

11. EXAMPLES OF PERMANENT WEB BRACING

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



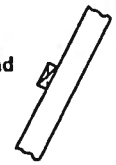
Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6.3 x 8.8

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

LATERAL BRACING

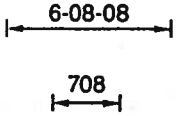


1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

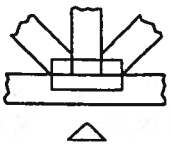
When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Robbins Engineering, Inc.

CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117

Wood Truss Council of America
5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

Standard Responsibilities in the Design Process Involving Metal Plate Connected Wood Trusses

WTCA 1-1995

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPEAND DEFINITIONS

1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice.The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.

1.2 The following definitions shall apply:

1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.

1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.

1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.

1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent bracing, anchorage, connections and field assembly. The term "Contractor" shall include those subcontractors who have a direct contract with the Contractor to perform all or a portion of the storage, handling, and installation of the trusses.

1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.

1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.

1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.

1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.

1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.

1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.

1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

2.1 Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

3.1 Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;

3.2 Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:

3.2.1 All truss orientations and locations;

3.2.2 Information to fully determine all truss profiles;

3.2.3 Adequate support of the Truss and all truss bearing conditions;

3.2.4 Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.

3.2.5 The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;

3.2.6 All Truss anchorage designs required to resist uplift, gravity, and lateral loads;

3.2.7 Allowable vertical and horizontal deflection criteria;

3.2.8 Proper transfer of design loads affecting the Truss; and

3.2.9 Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.

3.3 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.

3.4 Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

4.1 Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.

4.2 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.

4.3 Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.

4.4 Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.

4.5 Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.

4.6 Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.

5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.

5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.2 For each Truss Design Drawing, set forth as a minimum the following:

6.2.1 Slope or depth, span and spacing;

6.2.2 Location of all joints;

6.2.3 Required bearing widths;

6.2.4 Design loads as applicable:

6.2.4.1 Top chord live load (including snow loads);

6.2.4.2 Top chord dead load;

6.2.4.3 Bottom chord live load;

6.2.4.4 Bottom chord dead load;

6.2.4.5 Concentrated loads and their points of application; and

6.2.4.6 Controlling wind and earthquake loads;

6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;

6.2.6 Each reaction force and direction;

6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;

6.2.8 Lumber size, species, and grade for each member;

6.2.9 Connection Requirements for:

(a) Truss to Truss girder;

(b) Truss ply to ply; and

(c) Field splices;

6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;

6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and

6.2.12 Required permanent Truss member bracing location.

7.0 OTHER RESPONSIBILITIES

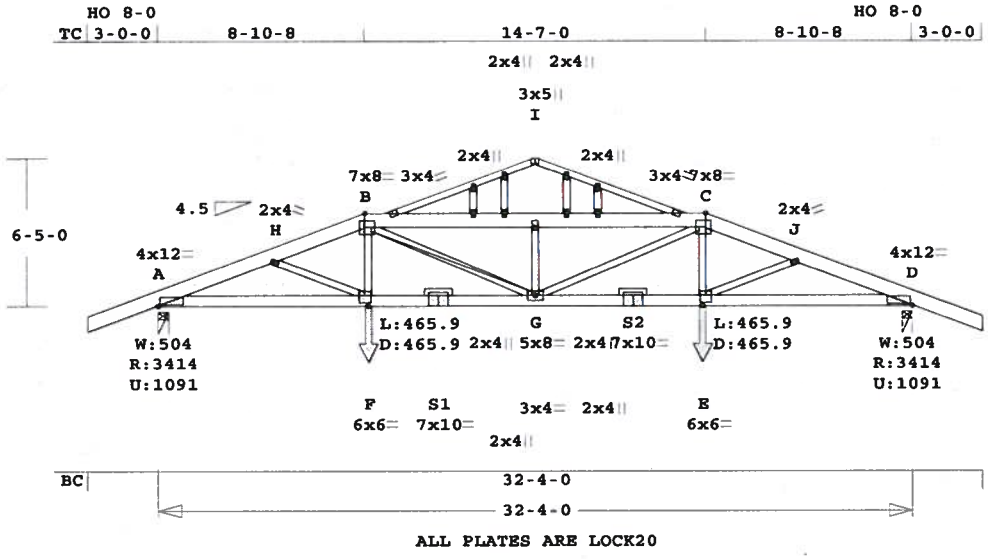
7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R1*	1	HIPP	320400	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 346.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 8	SP-2400
BC	0.49	2x 6	SP-2400
WB	0.76	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	32- 4- 0
BC	Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	32.3'
BC V	0	20	0.0'	32.3'
TC V	69	34	8.9'	23.5'
BC V	0	34	9.0'	23.3'
BC V	466	466	9.0'	CL-LB
BC V	466	466	23.3'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	3415	1092	5- 4	2-13
			Hz =	-38
D	3414	1092	5- 4	2-13

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -H	0.28	7499 C	0.07	0.21
H -B	0.29	7741 C	0.08	0.21
B -I	0.35	9072 C	0.12	0.23
I -C	0.35	9072 C	0.12	0.23
C -J	0.29	7741 C	0.08	0.21
J -D	0.28	7499 C	0.07	0.21
Bottom Chords				
A -F	0.39	6904 T	0.34	0.05
F -S1	0.48	7272 T	0.36	0.12
S1-G	0.49	7272 T	0.36	0.13
G -S2	0.49	7272 T	0.36	0.13
S2-E	0.48	7272 T	0.36	0.12
E -D	0.39	6904 T	0.34	0.05
Webs				
H -F	0.14	440 T		
F -B	0.32	1074 T		
B -G	0.76	1972 T		
G -I	0.24	1201 C		
G -C	0.76	1972 T		
E -C	0.32	1074 T		
E -J	0.14	440 T		

TL Defl -0.64" in S1-G L/586
LL Defl -0.31" in S1-G L/999
Shear // Grain in B -I 0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

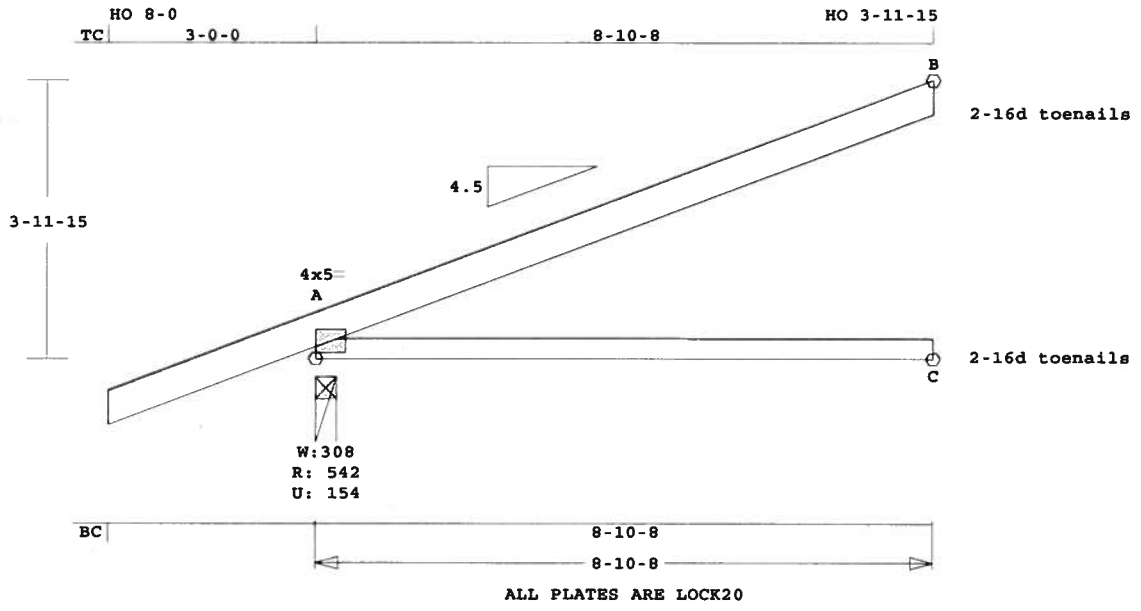
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x12.0	Ctr	Ctr	0.95
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	7.0x 8.0	-0.5	Ctr	0.91
I	LOCK	3.0x 5.0	Ctr	1.1	0.50
C	LOCK	7.0x 8.0	0.5	Ctr	0.91
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
D	LOCK	4.0x12.0	Ctr	Ctr	0.95
F	LOCK	6.0x 6.0	Ctr	-0.8	0.54
S1	LOCK	7.0x10.0	Ctr	0.8	0.94
G	LOCK	5.0x 8.0	Ctr	-0.2	0.84
S2	LOCK	7.0x10.0	Ctr	0.8	0.94
E	LOCK	6.0x 6.0	Ctr	-0.8	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 8-10- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9072 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R2	9	JACK	81008	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 53.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.48	2x 6	SP-#2
BC	0.40	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8-10- 8
BC Cont.	0- 0- 0	8-10- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	542	154	3- 8	1- 8
			Hz = 150	
C	149	0	3- 8	1- 8
B	275	180	3- 8	1- 8
			Hz = 80	

Membr	CSI	P Lbs	Axl-CSI-Bnd
--------Top Chords--------			
A -B	0.48	142 C	0.00 0.48
--------Bottom Chords--------			
A -C	0.40	0 T	0.00 0.40

TL Defl -0.34" in A -C L/291
LL Defl -0.15" in A -C L/650
Shear // Grain in A -C 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

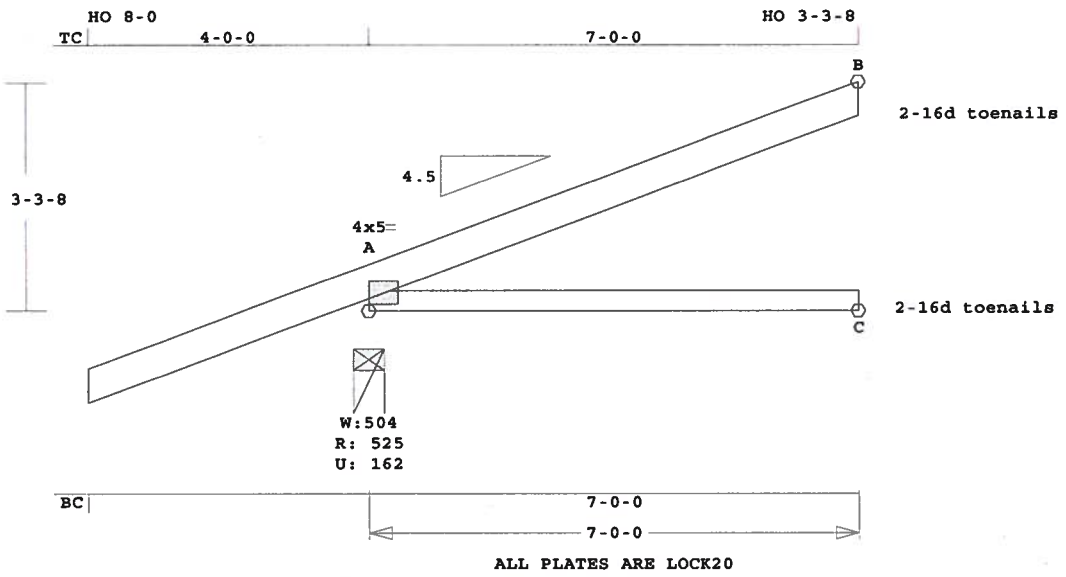
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 142 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R2A	4	JACK	70000	4.5	4- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 47.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 6	SP-#2
BC	0.25	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	525	162	5- 4	1- 8
			Hz =	117
C	113	0	1- 8	1- 8
B	206	136	1- 8	1- 8
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.31	120 C	0.00 0.31
--------Bottom Chords---------			
A -C	0.25	0 T	0.00 0.25

TL Defl -0.14" in A -C L/590
LL Defl -0.06" in A -C L/999
Shear // Grain in A -B 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

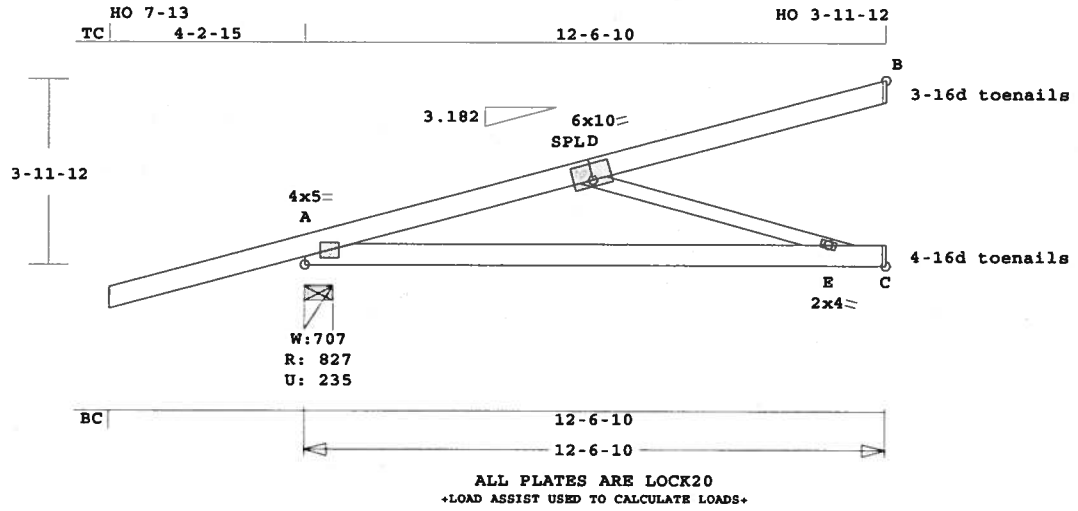
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 120 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R3*	2	KING	120610	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 96.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 6	SP-#2
BC	0.73	2x 6	SP-#2
WB	0.71	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 6-10
BC Cont.	0- 0- 0	12- 6-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	12.6'
BC V	0	20	0.0'	12.6'
TC V	-40	-20	0.0'	
	71	36		12.6'
BC V	0	-20	0.0'	
	0	36		12.6'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	827	236	7- 7	1- 8
			Hz =	209
C	583	42	3- 8	1- 8
			Hz =	112
B	422	296	3- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -D	0.47	1166	C	0.00	0.47
D -B	0.46	141	C	0.00	0.46
--------Bottom Chords---------					
A -E	0.73	1160	T	0.07	0.66
E -C	0.11	112	T	0.00	0.11
-------------Webs-------------					
D -E	0.71	1205	C		

TL Defl -0.49" in A -C L/285
LL Defl -0.24" in A -C L/581
Shear // Grain in C -C 0.42

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71
D	LOCK	6.0x10.0	-0.3	1.2	0.96
E	LOCK	2.0x 4.0	Ctr	Ctr	0.61

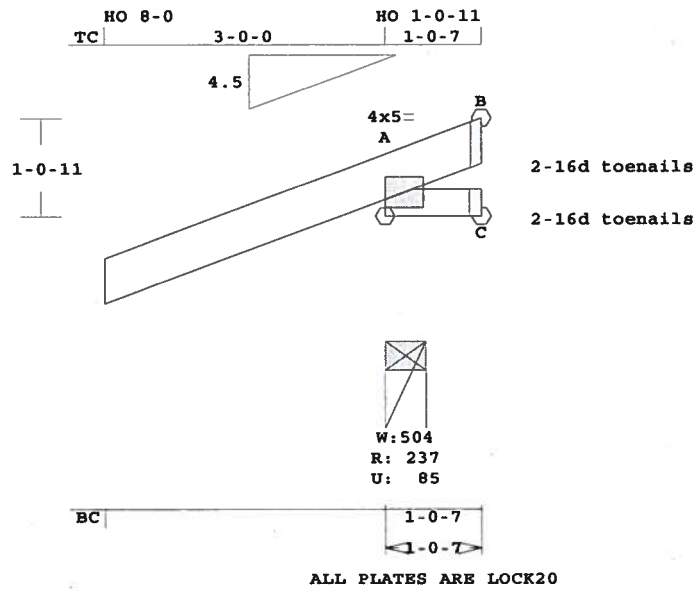
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 8-10- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1205 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R4*	24	JACK1	10007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 14.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 6	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	238	85	3- 8	1- 8
			Hz =	16
B	24	20	1- 8	1- 8
C	14	0	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	21	C
--------Bottom Chords---------			
A -C	0.00	8	T

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.03

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

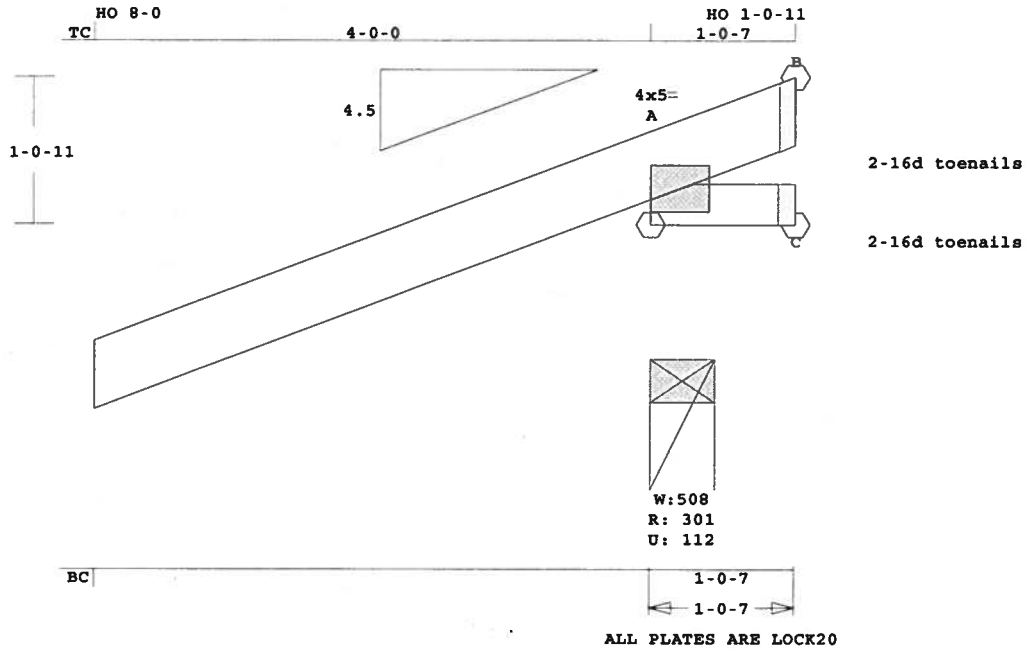
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 21 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R4A*	8	JACK1	10007	4.5	4- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 17.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 6	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	302	112	3- 8	1- 8
			Hz =	16
B	24	20	1- 8	1- 8
C	14	0	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	21 C	
--------Bottom Chords---------			
A -C	0.00	8 T	

TL Defl 0.00" in
LL Defl 0.00" in
Shear // Grain in A -B 0.03

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

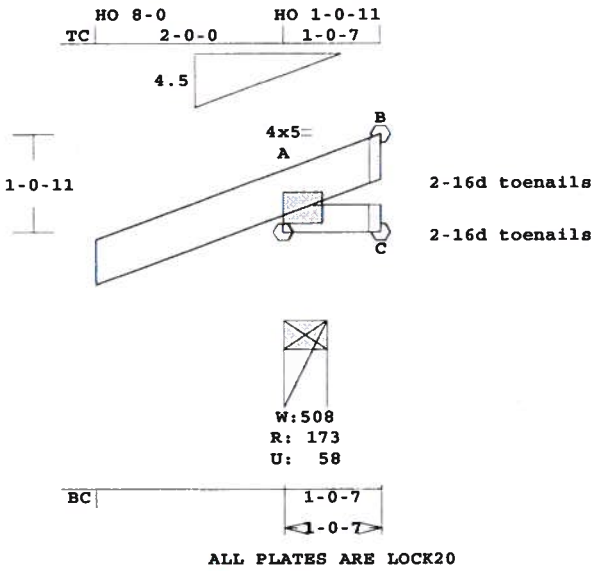
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 21 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R4B	8	JACK1	10007	4.5	2- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 11.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 6	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	174	59	3- 8	1- 8
			Hz =	16
B	24	20	1- 8	1- 8
C	14	0	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	21	C
--------Bottom Chords---------			
A -C	0.00	8	T

TL Defl 0.00" in
LL Defl 0.00" in
Shear // Grain in A -B 0.03

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

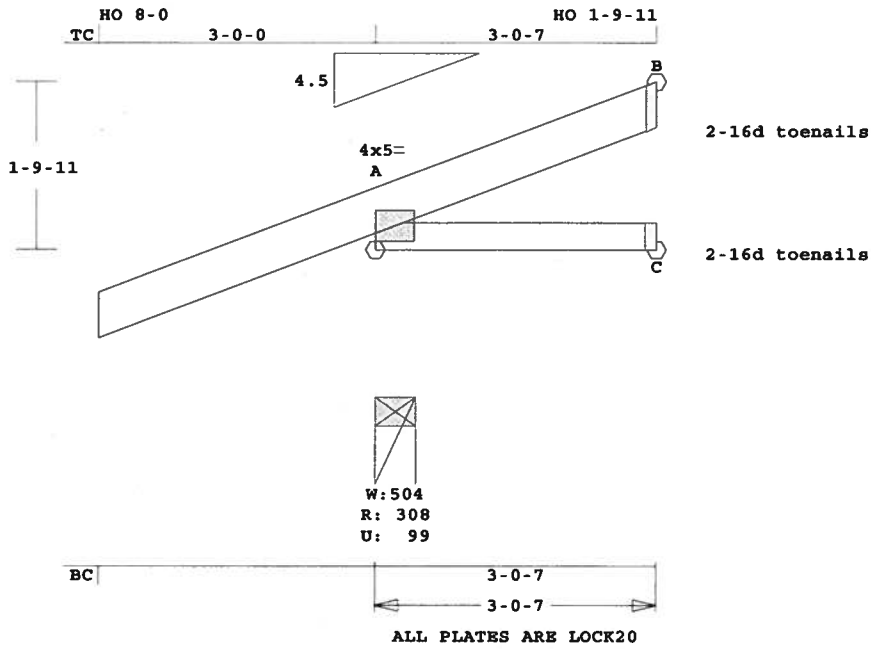
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 21 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R5*	20	JACK1	30007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 6	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	308	99	5- 4	1- 8
			Hz =	51
C	52	0	3- 8	1- 8
B	98	69	3- 8	1- 8
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	73	C 0.00 0.07
--------Bottom Chords---------			
A -C	0.05	0	T 0.00 0.05

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

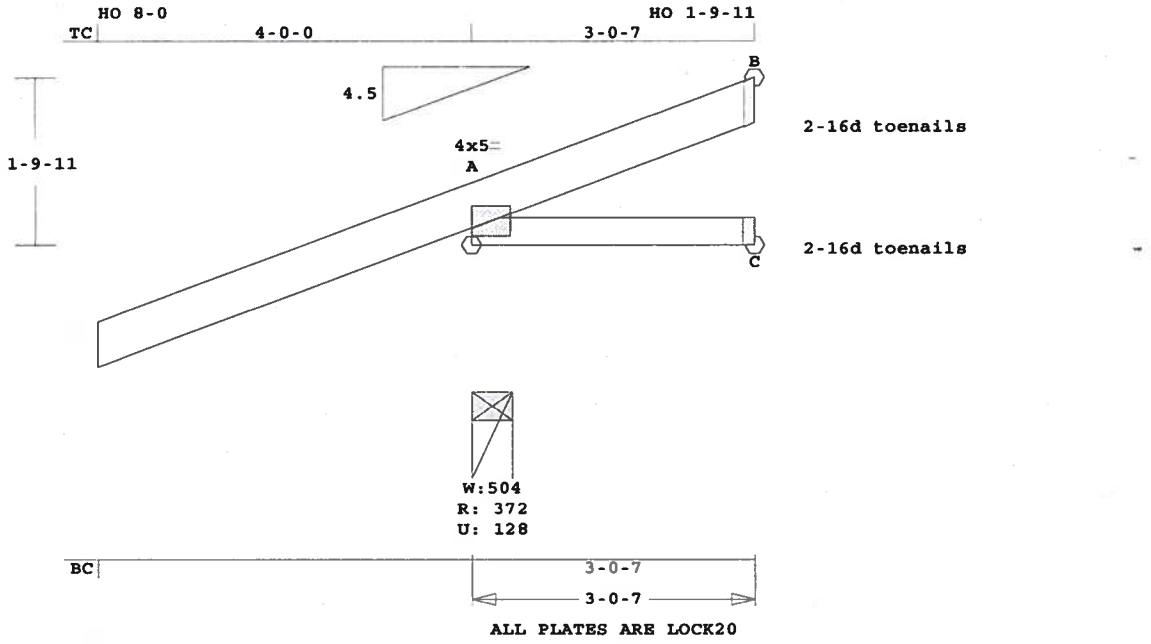
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 73 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R5A*	8	JACK1	30007	4.5	4- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 27.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 6	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	372	129	5- 4	1- 8
			Hz =	51
C	52	0	3- 8	1- 8
B	98	69	3- 8	1- 8
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	73 C	0.00 0.07
--------Bottom Chords---------			
A -C	0.05	0 T	0.00 0.05

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

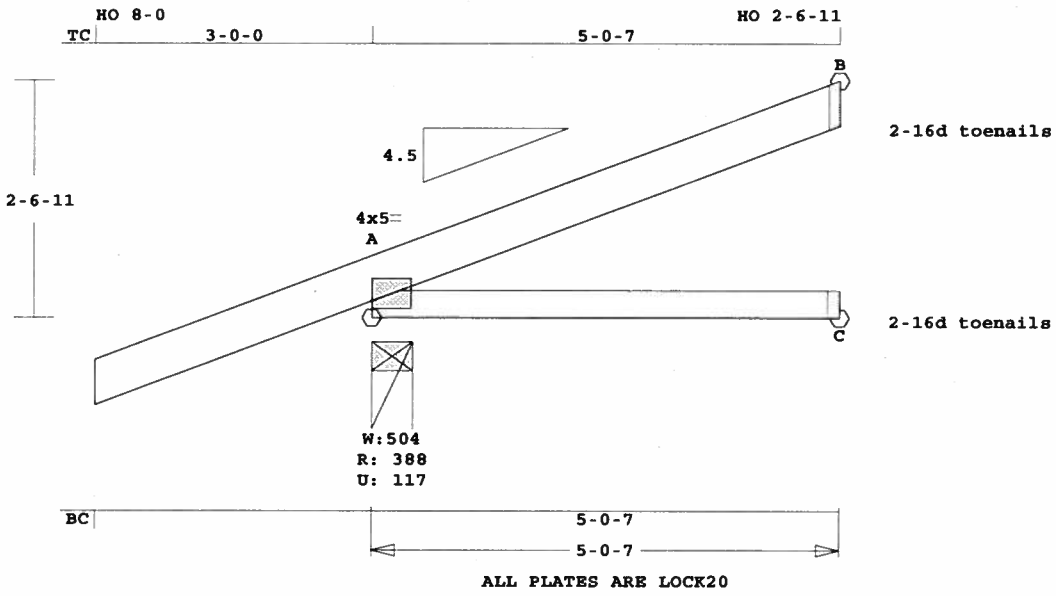
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 73 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R6	16	JACK1	50007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.17	2x 6	SP-#2
BC	0.12	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	389	118	5- 4	1- 8
			Hz =	85
C	85	0	3- 8	1- 8
B	158	107	3- 8	1- 8
			Hz =	46

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.17	108	C 0.00 0.17
Bottom Chords			
A -C	0.12	0	T 0.00 0.12

TL Defl -0.03" in A -C L/999
LL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

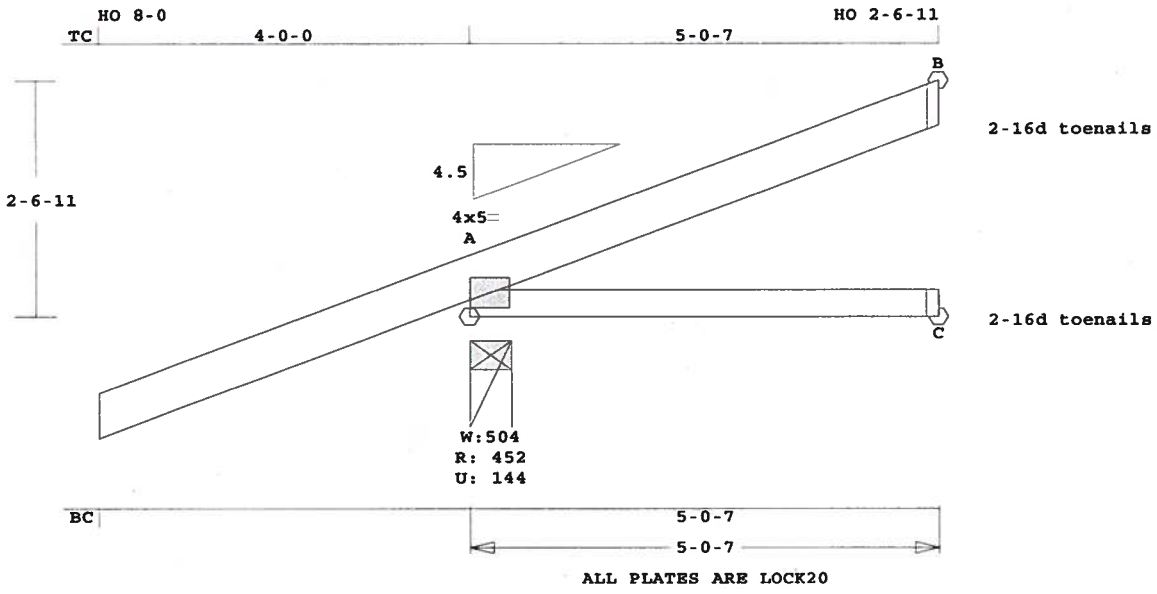
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 108 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R6A	8	JACK1	50007	4.5	4- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.17	2x 6	SP-#2
BC	0.12	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	453	145	5- 4	1- 8
			Hz =	85
C	85	0	3- 8	1- 8
B	158	107	3- 8	1- 8
			Hz =	46

Membr	CSI	P Lbs	Axl-CSI-Bnd
--------Top Chords--------			
A -B	0.17	108 C	0.00 0.17
--------Bottom Chords--------			
A -C	0.12	0 T	0.00 0.12

TL Defl -0.03" in A -C L/999
LL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

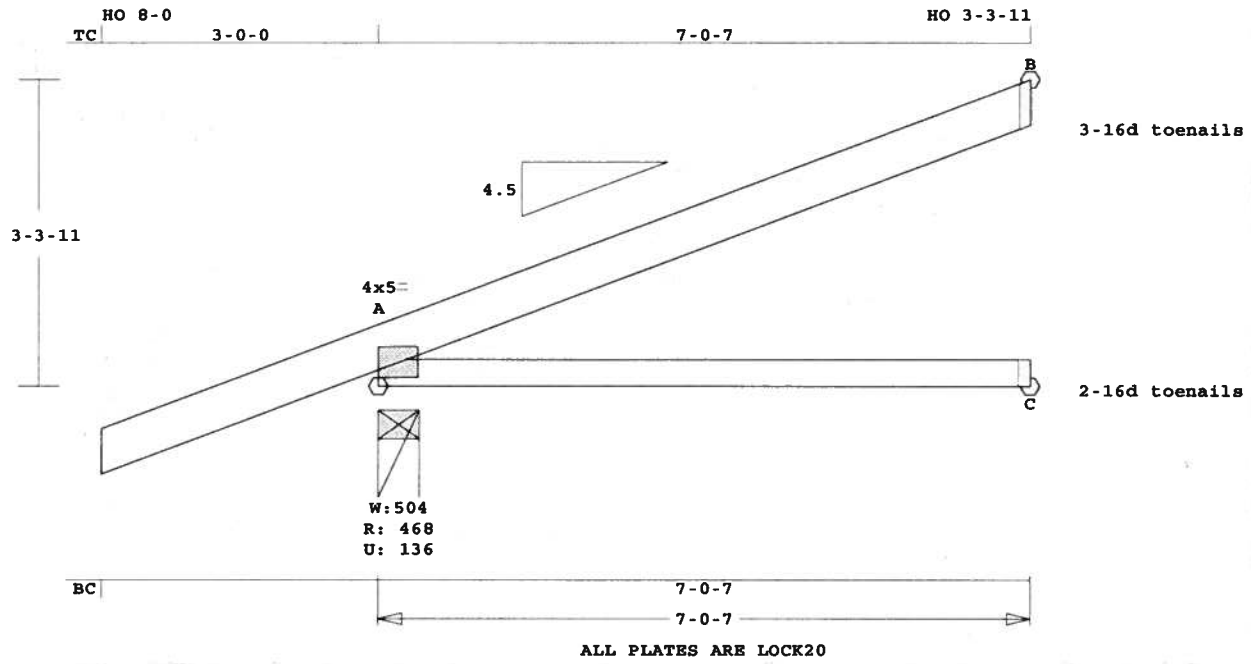
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 108 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R7*	8	JACK1	70007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 6	SP-#2
BC	0.24	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 7
BC	Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	469	137	5- 4	1- 8
			Hz = 119	
C	118	0	3- 8	1- 8
B	219	145	3- 8	1- 8
			Hz = 64	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.29	128 C	0.00 0.29
--------Bottom Chords---------			
A -C	0.24	0 T	0.00 0.24

TL Defl -0.13" in A -C L/581
LL Defl -0.06" in A -C L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

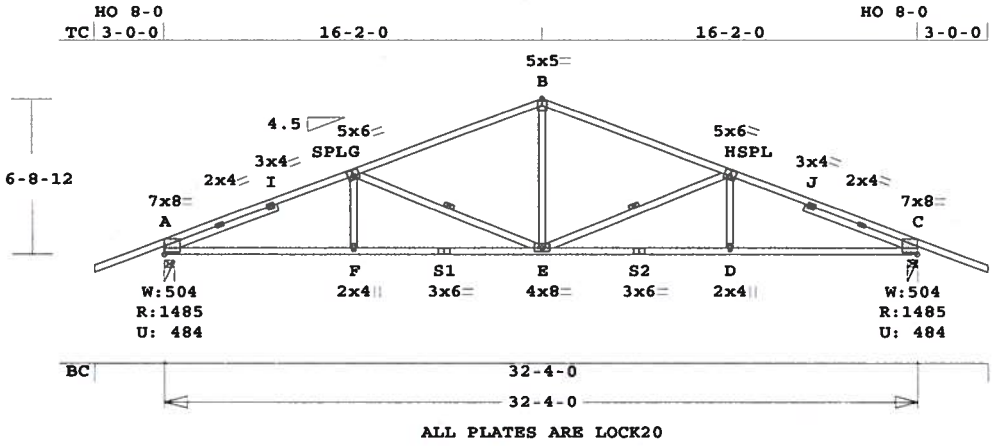
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 128 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R11*	5	TR	320400	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 213.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.67	2x 4	SP-#2
BC	0.62	2x 4	SP-#2
WB	0.23	2x 4	SP-#3
SL	0.42	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

WB 1 rows CLB on G -E
WB 1 rows CLB on E -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1485	484	5- 4	1-12
			Hz =	-70
C	1485	484	5- 4	1-12
			Hz =	71

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -I	0.30	1383 C	0.09 0.21
I -G	0.52	2481 C	0.14 0.38
G -B	0.67	1772 C	0.09 0.58
B -H	0.67	1771 C	0.09 0.58
H -J	0.52	2481 C	0.14 0.38
J -C	0.30	1383 C	0.09 0.21
--------Bottom Chords---------			
A -F	0.57	2342 T	0.39 0.18
F -S1	0.52	2342 T	0.39 0.13
S1-E	0.62	2342 T	0.39 0.23
E -S2	0.62	2342 T	0.39 0.23
S2-D	0.52	2342 T	0.39 0.13
D -C	0.57	2342 T	0.39 0.18
------------Webs------------			
F -G	0.08	301 T	
G -E	0.23	739 C	1 Br
E -B	0.23	757 T	
E -H	0.23	739 C	1 Br
D -H	0.08	301 T	
----------Sliders----------			
A -I	0.42	1185 C	
J -C	0.42	1185 C	

TL Defl -0.28" in S1-E L/999
LL Defl -0.12" in E -S2 L/999
Shear // Grain in G -B 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

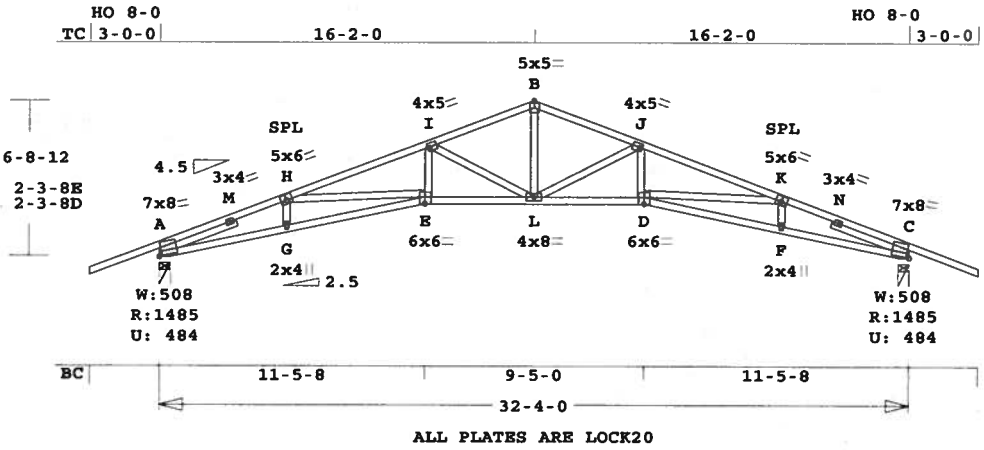
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	3.0x 4.0	Ctr	Ctr	0.57
G	LOCK	5.0x 6.0	-0.2	0.5	0.87
B	LOCK	5.0x 5.0	Ctr	Ctr	0.66
H	LOCK	5.0x 6.0	0.2	0.5	0.87
J	LOCK	3.0x 4.0	Ctr	Ctr	0.57
C	LOCK	7.0x 8.0	-4.0	0.6	0.85
C	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.43
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.58
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
D	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2481 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R12	9	SP	320400	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 213.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.84	2x 4	SP-#2
EX H -B		2x 4	SP-2400
EX B -K		2x 4	SP-2400
BC	0.63	2x 4	SP-2400
EX E -D		2x 4	SP-#2
WB	0.66	2x 4	SP-#3
SL	0.46	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	32- 4- 0
BC	Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1485	484	5- 8	1- 8
			Hz =	-70
C	1485	484	5- 8	1- 8
			Hz =	71

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.74	1974	C	0.02	0.72
M -H	0.84	4319	C	0.12	0.72
H -I	0.59	4284	C	0.16	0.43
I -B	0.24	2680	C	0.03	0.21
B -J	0.24	2681	C	0.03	0.21
J -K	0.59	4284	C	0.16	0.43
K -N	0.84	4319	C	0.12	0.72
N -C	0.74	1974	C	0.02	0.72
Bottom Chords					
A -G	0.63	4039	T	0.28	0.35
G -E	0.47	4060	T	0.29	0.18
E -L	0.80	3997	T	0.67	0.13
L -D	0.80	3997	T	0.67	0.13
D -F	0.47	4060	T	0.29	0.18
F -C	0.63	4038	T	0.28	0.35
Webs					
G -H	0.03	146	T		
H -E	0.03	122	T		
E -I	0.31	967	T		
I -L	0.66	1696	C		
L -B	0.53	1656	T		
L -J	0.66	1696	C		
D -J	0.31	967	T		
D -K	0.03	122	T		
F -K	0.03	146	T		
Sliders					
A -M	0.46	2453	C		
N -C	0.46	2453	C		

TL Defl -0.86" in E -L L/437
LL Defl -0.42" in E -L L/890

Hz Disp	LL	DL	TL
Jt C	0.26"	0.27"	0.53"

Shear // Grain in M -H 0.36

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

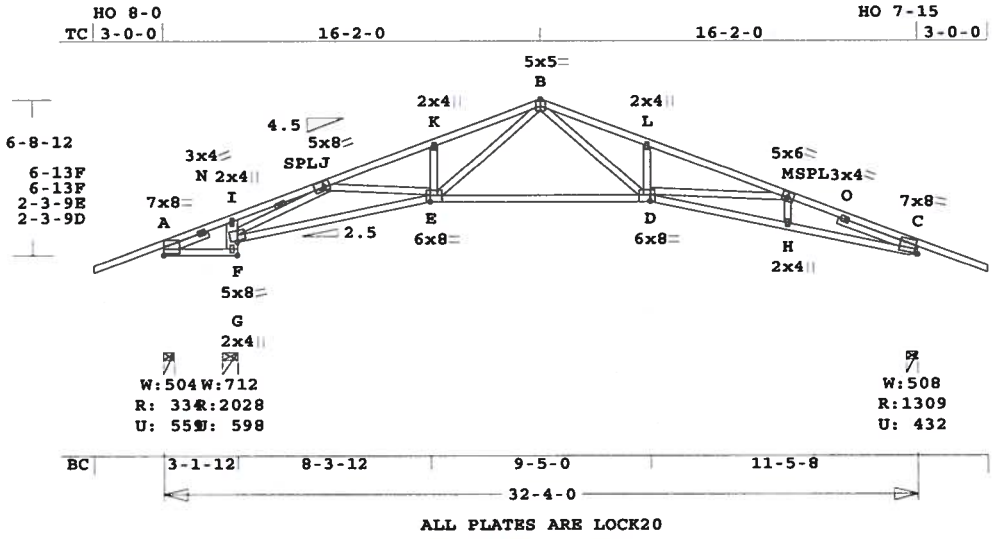
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.6	1.2	0.91
M	LOCK	3.0x 4.0	Ctr	Ctr	0.91
H	LOCK	5.0x 6.0	-0.2	0.5	0.87
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.66
J	LOCK	4.0x 5.0	Ctr	Ctr	0.64
K	LOCK	5.0x 6.0	0.2	0.5	0.87
N	LOCK	3.0x 4.0	Ctr	Ctr	0.91
C	LOCK	7.0x 8.0	-4.6	1.2	0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.43
E	LOCK	6.0x 6.0	Ctr	-0.5	0.69
L	LOCK	4.0x 8.0	Ctr	Ctr	0.61
D	LOCK	6.0x 6.0	Ctr	-0.5	0.69
F	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 4319 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R13	1	SP	320400	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 215.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.83	2x 4	SP-#2
BC	0.70	2x 4	SP-#2
EX D -C		2x 4	SP-2400
CW	0.62	2x 6	SP-#2
WB	0.62	2x 4	SP-#3
SL	0.36	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

WB 1 rows CLB on F -J
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	334	560G	5- 4	1- 8
			Hz =	-70
G	2029	598	7-12	2- 5
C	1310	433	5- 8	1- 8
			Hz =	71

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -N	0.25	568 T	0.09 0.16
N -I	0.69	1254 T	0.21 0.48
I -J	0.73	1546 T	0.25 0.48
J -K	0.33	2752 C	0.16 0.17
K -B	0.34	2767 C	0.17 0.17
B -L	0.42	3467 C	0.21 0.21
L -M	0.83	3457 C	0.18 0.65
M -O	0.62	3600 C	0.09 0.53
O -C	0.54	1781 C	0.01 0.53
--------Bottom Chords---------			
A -G	0.10	1082 C	0.00 0.10
F -E	0.70	1496 T	0.24 0.46
E -D	0.57	1720 T	0.28 0.29
D -H	0.43	3404 T	0.24 0.19
H -C	0.54	3374 T	0.24 0.30
----------Chord-Webs----------			
G -F	0.62	1962 C	0.01 0.61
F -I	0.36	359 C	0.00 0.36
------------Webs-------------			
F -J	0.61	3445 C	1 Br
J -E	0.35	1118 T	
E -K	0.07	323 C	
E -B	0.35	1107 T	
B -D	0.62	1963 T	
D -L	0.08	350 C	
D -M	0.05	209 T	
H -M	0.03	124 T	
----------Sliders------------			
A -N	0.23	717 T	
O -C	0.36	1914 C	

TL Defl -0.78" in E -D L/443
LL Defl -0.31" in E -D L/999

Hz Disp	LL	DL	TL
Jt C	0.15"	0.15"	0.30"

Shear // Grain in G -F 0.89

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

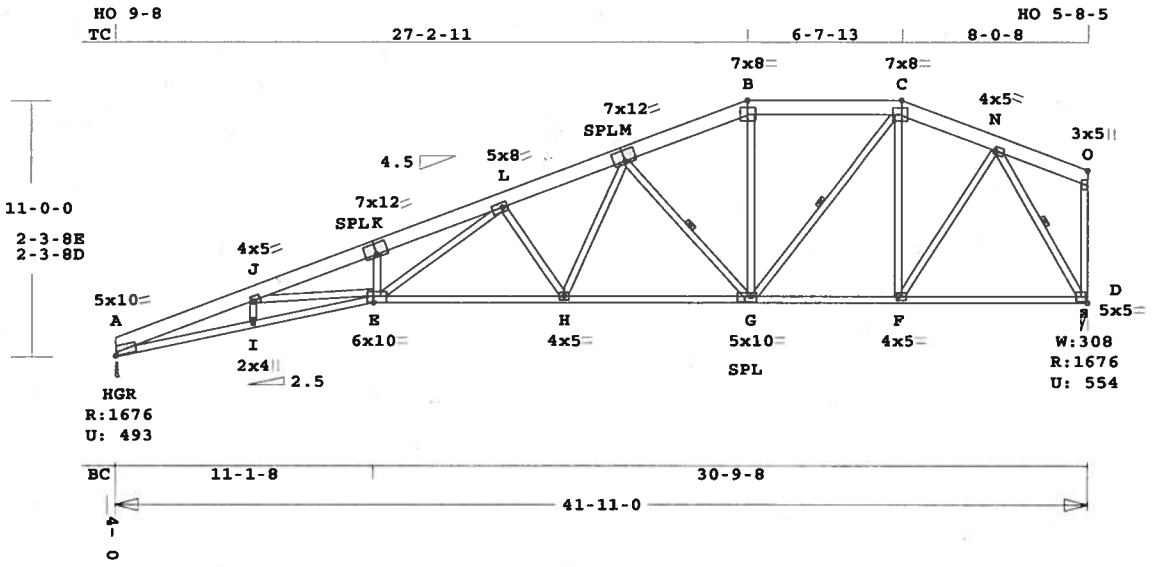
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
N	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	2.0x 4.0	Ctr	Ctr	0.26
J	LOCK	5.0x 8.0-1.1	0.1	0.99	
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.82
L	LOCK	2.0x 4.0	Ctr	Ctr	0.43
M	LOCK	5.0x 6.0	0.2	0.5	0.87
O	LOCK	3.0x 4.0	Ctr	Ctr	0.71
C	LOCK	7.0x 8.0-4.6	1.3	0.91	
G	LOCK	2.0x 4.0	Ctr	Ctr	0.91
F	LOCK	5.0x 8.0	Ctr	Ctr	0.85
E	LOCK	6.0x 8.0	Ctr-0.5	0.66	
D	LOCK	6.0x 8.0	Ctr-0.5	0.75	
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3600 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite L
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R51	5	SP	411100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 396.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 8	SP-2400
BC	0.89	2x 4	SP-#2
EX A -E		2x 4	SP-2400
WB	0.89	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	41-11- 0
BC	Cont.	0- 0- 0	41-11- 0

WB 1 rows CLB on M -G
WB 1 rows CLB on G -C
WB 1 rows CLB on N -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1677	494	3- 8	1- 8
			Hz =	324
D	1677	554	3- 8	2- 0
			Hz =	188

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.32	5798	C	0.03	0.29
J -K	0.33	6492	C	0.04	0.29
K -L	0.11	6527	C	0.05	0.06
L -M	0.06	3493	C	0.04	0.02
M -B	0.05	2039	C	0.02	0.03
B -C	0.06	1909	C	0.00	0.06
C -N	0.03	1414	C	0.01	0.02
N -O	0.02	56	T	0.00	0.02
Bottom Chords					
A -I	0.61	5377	T	0.38	0.23
I -E	0.55	5401	T	0.38	0.17
E -H	0.89	3898	T	0.65	0.24
H -G	0.70	2766	T	0.46	0.24
G -F	0.51	1322	T	0.14	0.37
F -D	0.45	859	T	0.08	0.37
Webs					
I -J	0.03	121	T		
J -E	0.27	856	T		
E -K	0.07	432	C		
E -L	0.89	2801	T		
L -H	0.44	1198	C		
H -M	0.41	1299	T		
M -G	0.37	1306	C		1 Br
G -B	0.11	416	T		
G -C	0.30	947	T		1 Br
F -C	0.53	563	C		
F -N	0.27	860	T		
N -D	0.44	1731	C		1 Br
D -O	0.04	145	T		

TL Defl -0.86" in E -H L/575
LL Defl -0.39" in E -H L/999

Hz Disp	LL	DL	TL
Jt D	0.17"	0.17"	0.35"

Shear // Grain in A -J 0.33

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

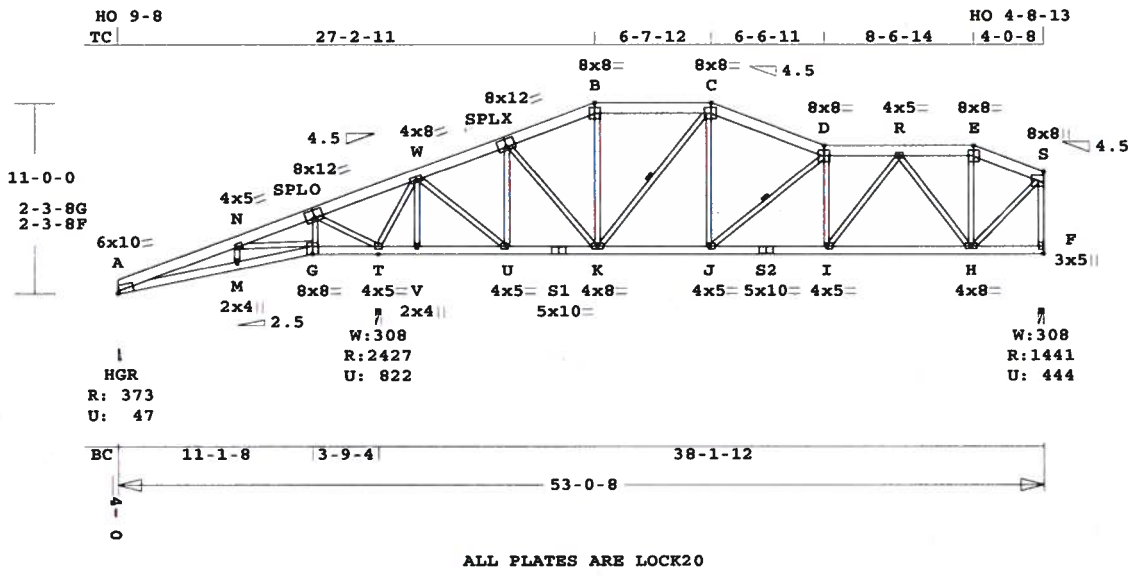
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	0.9	0.3	0.94
J	LOCK	4.0x 5.0-0.5	0.2		0.63
K	LOCK	7.0x12.0-0.7	2.0		0.97
L	LOCK	5.0x 8.0-1.5-0.6			0.97
M	LOCK	7.0x12.0-0.7	2.0		0.97
B	LOCK	7.0x 8.0	Ctr	Ctr	0.86
C	LOCK	7.0x 8.0	1.0	Ctr	0.86
N	LOCK	4.0x 5.0	0.5	0.2	0.72
O	LOCK	3.0x 5.0	Ctr	Ctr	0.88
I	LOCK	2.0x 4.0	Ctr	Ctr	0.49
E	LOCK	6.0x10.0	Ctr-0.5		0.90
H	LOCK	4.0x 5.0	Ctr	Ctr	0.58
G	LOCK	5.0x10.0	1.0-0.5		0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	5.0x 5.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Provide connection to bearing for 323 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6527 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R50	1	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 560.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.10	2x 8	SP-2400
BC	0.31	2x 6	SP-#2
WB	0.76	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 0- 8
BC Cont.		0- 0- 0	53- 0- 8

WB 1 rows CLB on K -C
WB 1 rows CLB on J -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	374	47	3- 8	1- 8
			Hz =	279
T	2428	822	3- 8	2- 9
F	1442	444	3- 8	1-11
			Hz =	176

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.07	469	C	0.00	0.07
N -O	0.08	374	T	0.01	0.07
O -W	0.10	962	T	0.03	0.07
W -X	0.05	1081	C	0.00	0.05
X -B	0.04	1377	C	0.01	0.03
B -C	0.07	1288	C	0.00	0.07
C -D	0.08	1671	C	0.00	0.08
D -R	0.03	2053	C	0.00	0.03
R -E	0.02	914	C	0.00	0.02
E -S	0.03	975	C	0.01	0.02
Bottom Chords					
A -M	0.15	460	T	0.05	0.10
M -G	0.15	465	T	0.05	0.10
G -T	0.15	335	C	0.00	0.15
T -V	0.15	69	T	0.00	0.15
V -U	0.06	69	T	0.00	0.06
U -S1	0.16	1004	T	0.12	0.04
S1-K	0.16	1004	T	0.12	0.04
K -J	0.24	1570	T	0.19	0.05
J -S2	0.30	2062	T	0.25	0.05
S2-I	0.31	2062	T	0.25	0.06
I -H	0.27	1567	T	0.19	0.08
H -F	0.13	175	T	0.00	0.13
Webs					
M -N	0.04	184	T		
N -G	0.24	778	C		
G -O	0.05	211	T		
O -T	0.18	665	C		
T -W	0.67	2087	C		
V -W	0.02	89	T		
W -U	0.41	1291	T		
U -X	0.39	727	C		
X -K	0.14	444	T		
K -B	0.06	229	T		
K -C	0.18	451	C		1 Br
J -C	0.16	557	T		
J -D	0.19	621	C		1 Br
I -D	0.23	520	C		
I -R	0.26	828	T		
R -H	0.76	1113	C		
H -E	0.04	170	T		
H -S	0.42	1325	T		
F -S	0.38	1419	C		

TL Defl -0.15" in S2-I L/999
LL Defl -0.07" in S2-I L/999

Hz Disp	LL	DL	TL
Jt F	0.02"	0.02"	0.04"

Shear // Grain in C -D 0.12

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

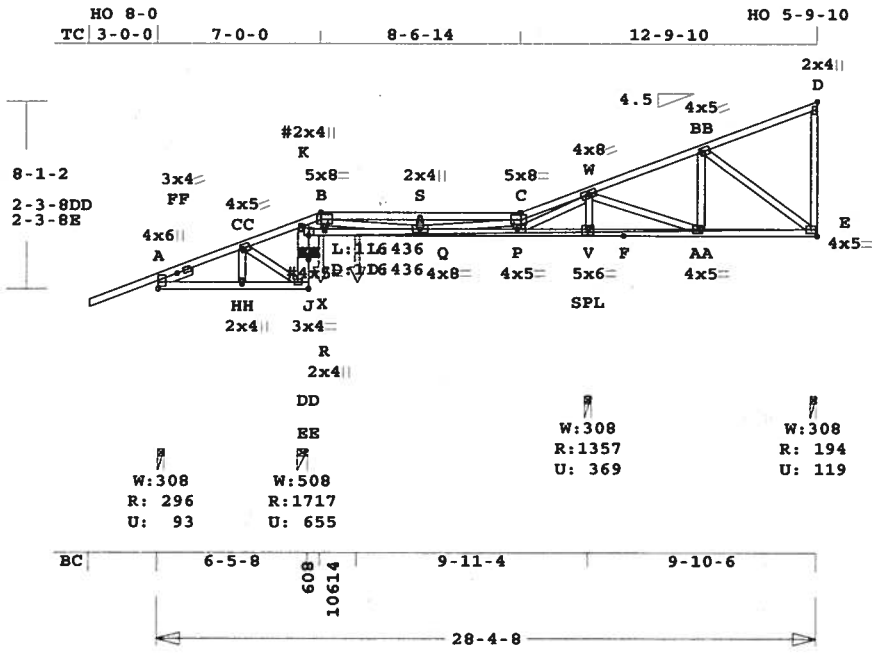
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	5.5	2.2	0.90
N	LOCK	4.0x 5.0-0.5	0.2	0.62	
O	LOCK	8.0x12.0-0.6	1.5	0.97	
W	LOCK	4.0x 8.0-0.5	0.2	0.87	
X	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	8.0x 8.0 Ctr	Ctr	0.86	
C	LOCK	8.0x 8.0 1.0	Ctr	0.86	
D	LOCK	8.0x 8.0 Ctr	Ctr	0.86	
R	LOCK	4.0x 5.0 Ctr	0.5	0.56	
E	LOCK	8.0x 8.0 Ctr	Ctr	0.86	
S	LOCK	8.0x 8.0-0.5	Ctr	0.66	
M	LOCK	2.0x 4.0 Ctr	Ctr	0.56	
G	LOCK	8.0x 8.0 Ctr	-1.5	0.82	
T	LOCK	4.0x 5.0 Ctr	Ctr	0.58	
V	LOCK	2.0x 4.0 Ctr	Ctr	0.56	
U	LOCK	4.0x 5.0 Ctr	Ctr	0.60	
S1	LOCK	5.0x10.0 Ctr	Ctr	0.82	
K	LOCK	4.0x 8.0 Ctr	Ctr	0.50	
J	LOCK	4.0x 5.0 Ctr	Ctr	0.60	
S2	LOCK	5.0x10.0 Ctr	Ctr	0.82	
I	LOCK	4.0x 5.0 Ctr	Ctr	0.60	
H	LOCK	4.0x 8.0 Ctr	Ctr	0.56	
F	LOCK	3.0x 5.0 Ctr	Ctr	0.94	

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2087 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R49A	1*2P	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 196.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 2-Ply Truss *
****

	CSI	Size	Lumber
TC	0.39	2x 4	SP-#2
BC	0.89	2x 4	SP-#2
CW	0.24	2x 6	SP-#2
WB	0.32	2x 4	SP-#3
SL	0.00	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	28.4'
BC V	0	20	0.0'	28.4'
BC V	116	116	7.0'	CL-LB
BC V	436	436	8.6'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	296	94	3- 8	1- 8
		Hz =	334	
J	1717	655	5- 8	1- 8
V	1357	369	3- 8	1- 8
E	195	120	3- 8	1- 8
		Hz =	181	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -FF	0.05	247	C	0.00	0.05
FF-CC	0.07	333	C	0.00	0.07
CC-K	0.39	449	C	0.02	0.37
K -B	0.37	856	C	0.00	0.37
B -S	0.18	1918	C	0.00	0.18
S -C	0.28	1918	C	0.00	0.28
C -W	0.17	227	C	0.01	0.16
W -BB	0.16	78	C	0.00	0.16
BB-D	0.13	67	C	0.00	0.13
Bottom Chords					
A -HH	0.07	101	C	0.00	0.07
HH-J	0.05	101	C	0.00	0.05
LL-R	0.89	633	T	0.05	0.84
R -Q	0.64	556	T	0.05	0.59
Q -P	0.09	79	C	0.00	0.09
P -V	0.07	997	C	0.00	0.07
V -AA	0.08	997	C	0.00	0.08
AA-E	0.08	176	T	0.00	0.08
Chord-Webs					
J -LL	0.24	1615	C	0.00	0.24
LL-K	0.17	1107	C	0.00	0.17
Webs					
HH-CC	0.02	167	T		
CC-J	0.02	202	T		
R -B	0.06	522	T		
B -Q	0.22	1376	T		
Q -S	0.01	121	T		
Q -C	0.32	2007	T		
P -C	0.03	421	C		
P -W	0.15	996	T		
V -W	0.10	1283	C		
W -AA	0.17	1098	T		
AA-BB	0.02	278	C		
BB-E	0.01	148	T		
E -D	0.02	170	T		
Sliders					
A -FF	0.00	105	C		

TL Defl -0.07" in R -Q L/999
LL Defl -0.03" in R -Q L/999
Shear // Grain in R -Q 0.44

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
FF	LOCK	3.0x 4.0	Ctr	Ctr	0.54
CC	LOCK	4.0x 5.0	Ctr	Ctr	0.64
K#	LOCK	2.0x 4.0-1.4-0.6			0.21
B	LOCK	5.0x 8.0	Ctr	Ctr	0.80
S	LOCK	2.0x 4.0	Ctr	Ctr	0.40
C	LOCK	5.0x 8.0-1.0	Ctr	Ctr	0.80
W	LOCK	4.0x 8.0	Ctr	Ctr	0.77
BB	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
HH	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	3.0x 4.0	Ctr	Ctr	1.01
LL#	LOCK	4.0x 5.0	0.1	0.3	0.93
R	LOCK	2.0x 4.0	Ctr	Ctr	0.40
Q	LOCK	4.0x 8.0	Ctr	Ctr	0.79
P	LOCK	4.0x 5.0	Ctr	Ctr	0.60
V	LOCK	5.0x 6.0	Ctr	-0.5	0.88
AA	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

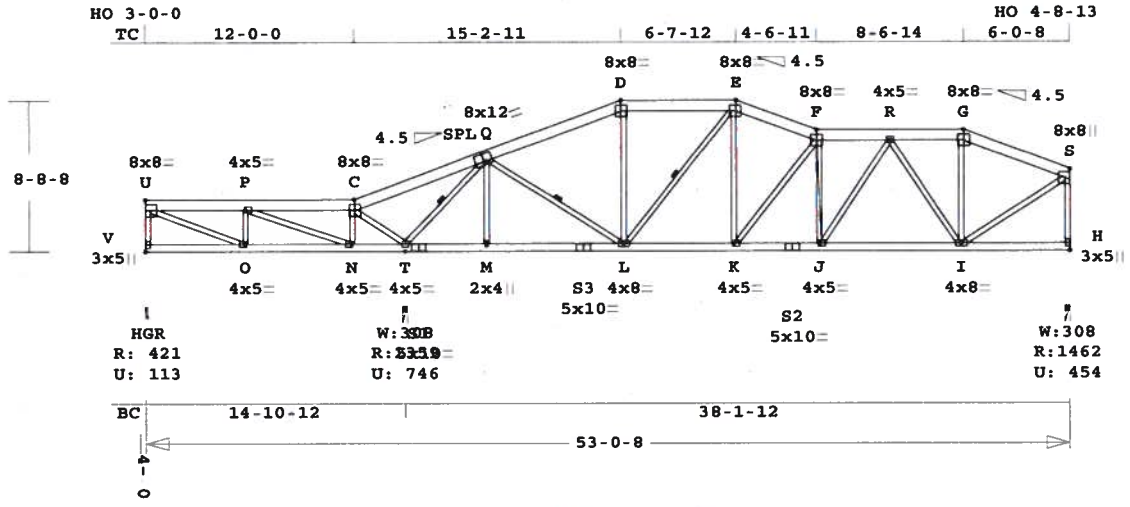
	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 1	12	24	0
WB 1	8	8	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1918 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R49	1	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 568.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 8	SP-2400
BC	0.29	2x 6	SP-#2
WB	0.72	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	53- 0- 8
BC Cont.	0- 0- 0	53- 0- 8

WB 1 rows CLB on T -Q
WB 1 rows CLB on Q -L
WB 1 rows CLB on L -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
V	421	113	3- 8	1- 8
			Hz =	-152
T	2360	746	3- 8	2- 8
H	1462	454	3- 8	1-12
			Hz =	100

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
U -P	0.04	483	C	0.00 0.04
P -C	0.04	221	T	0.00 0.04
C -Q	0.13	857	T	0.03 0.10
Q -D	0.10	1470	C	0.00 0.10
D -E	0.07	1361	C	0.00 0.07
E -F	0.05	1710	C	0.02 0.03
F -R	0.03	1966	C	0.02 0.01
R -G	0.02	1149	C	0.00 0.02
G -S	0.06	1226	C	0.01 0.05
Bottom Chords				
V -O	0.09	121	C	0.00 0.09
O -N	0.12	483	T	0.03 0.09
N -T	0.14	235	C	0.00 0.14
T -S1	0.24	826	T	0.10 0.14
S1-M	0.18	826	T	0.10 0.08
M -S3	0.18	826	T	0.10 0.08
S3-L	0.22	826	T	0.06 0.16
L -K	0.29	1609	T	0.19 0.10
K -S2	0.25	1972	T	0.23 0.02
S2-J	0.28	1972	T	0.23 0.05
J -I	0.28	1634	T	0.19 0.09
I -H	0.16	100	T	0.00 0.16
Webs				
V -U	0.06	369	C	WindLd
U -O	0.16	526	T	
O -P	0.03	133	T	
P -N	0.44	761	C	
N -C	0.10	360	T	
C -T	0.16	676	C	
T -Q	0.55	2419	C	1 Br
M -Q	0.06	232	T	
Q -L	0.20	647	T	1 Br
L -D	0.06	247	T	
L -E	0.16	398	C	1 Br
K -E	0.17	544	T	
K -F	0.50	579	C	
J -F	0.21	383	C	
J -R	0.19	615	T	
R -I	0.72	898	C	
I -G	0.05	217	T	
I -S	0.45	1404	T	
H -S	0.38	1415	C	

TL Defl -0.14" in S2-J L/999
LL Defl -0.07" in S2-J L/999
Shear // Grain in C -Q 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

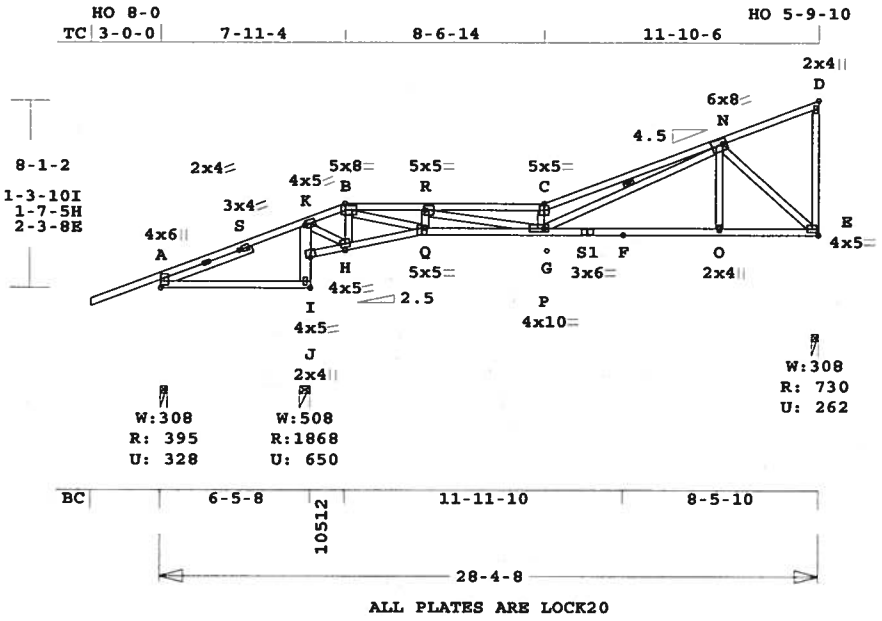
Jt	Type	Plt Size	X	Y	JSI
U	LOCK	8.0x 8.0	0.5	Ctr	0.82
P	LOCK	4.0x 5.0	Ctr	0.5	0.68
C	LOCK	8.0x 8.0	Ctr	Ctr	0.86
Q	LOCK	8.0x12.0-0.6	1.5		0.97
D	LOCK	8.0x 8.0	Ctr	Ctr	0.86
E	LOCK	8.0x 8.0	1.0	Ctr	0.86
F	LOCK	8.0x 8.0	Ctr	Ctr	0.86
R	LOCK	4.0x 5.0	Ctr	0.5	0.58
G	LOCK	8.0x 8.0	Ctr	Ctr	0.86
S	LOCK	8.0x 8.0-0.5	Ctr		0.66
V	LOCK	3.0x 5.0	Ctr	Ctr	0.94
O	LOCK	4.0x 5.0	Ctr	Ctr	0.60
N	LOCK	4.0x 5.0	Ctr	Ctr	0.60
T	LOCK	4.0x 5.0	Ctr	Ctr	0.72
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	2.0x 4.0	Ctr	Ctr	0.56
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 8.0	Ctr	Ctr	0.52
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 8.0	Ctr	Ctr	0.56
H	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2419 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

JOB	MARK	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R48A*	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 197.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.61	2x 4	SP-2400
EX B -C		2x 4	SP-#2
BC	0.56	2x 4	SP-#2
CW	0.76	2x 6	SP-2400
WB	0.75	2x 4	SP-#3
SL	0.35	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	396	329G	3- 8	1- 8
		Hz =	334	
J	1868	651	5- 8	2- 1
E	730	262	3- 8	1- 8
		Hz =	181	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -S	0.25	202 C	0.01	0.24
S -K	0.61	1312 T	0.09	0.52
K -B	0.59	1032 T	0.07	0.52
B -R	0.33	740 C	0.00	0.33
R -C	0.33	2822 C	0.05	0.28
C -N	0.55	2996 C	0.11	0.44
N -D	0.38	46 C	0.00	0.38
Bottom Chords				
A -J	0.20	1143 C	0.00	0.20
I -H	0.56	2002 C	0.02	0.54
H -Q	0.27	1104 C	0.00	0.27
Q -P	0.36	815 T	0.13	0.23
P -S1	0.38	715 T	0.12	0.26
S1-O	0.39	715 T	0.07	0.32
O -E	0.39	715 T	0.07	0.32
Chord-Webs				
J -I	0.76	1808 C	0.00	0.76
I -K	0.62	1278 C	0.00	0.62
Webs				
K -H	0.31	969 T		
H -B	0.10	642 C		
B -Q	0.59	1849 T		
Q -R	0.09	578 C		
R -P	0.65	2051 T		
P -C	0.22	1430 C		
P -N	0.75	2359 T		1 Br
O -N	0.08	307 T		
N -E	0.45	977 C		
E -D	0.02	115 T		
Sliders				
A -S	0.35	1121 T		

TL Defl -0.67" in P -S1 L/384
LL Defl -0.32" in P -S1 L/799

Hz Disp	LL	DL	TL
Jt E	0.02"	0.02"	0.03"

Shear // Grain in J -I 0.87

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

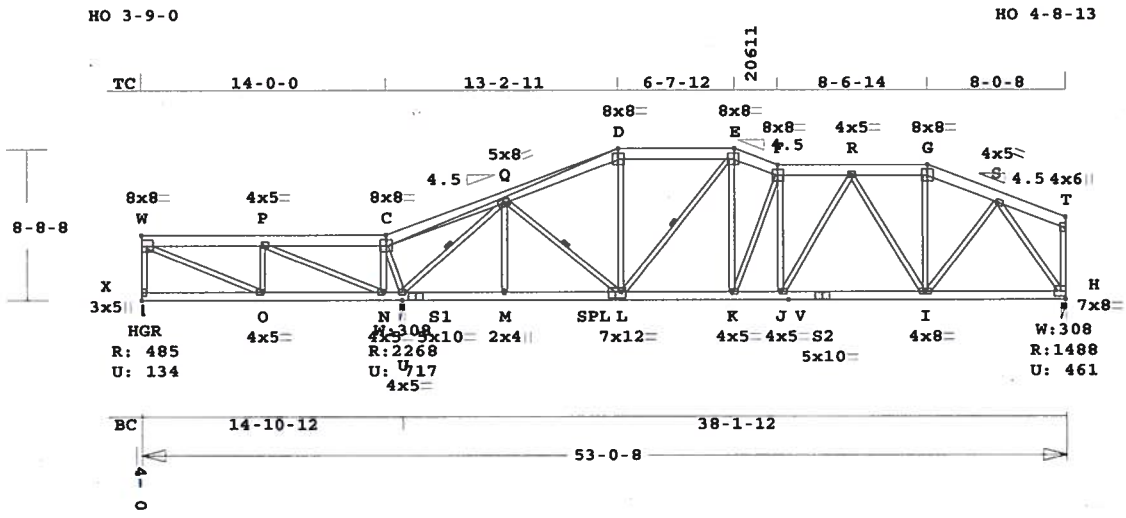
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S	LOCK	3.0x 4.0	Ctr	Ctr	0.84
K	LOCK	4.0x 5.0	Ctr	Ctr	0.78
B	LOCK	5.0x 8.0	Ctr	Ctr	0.85
R	LOCK	5.0x 5.0	Ctr	Ctr	0.93
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
N	LOCK	6.0x 8.0	Ctr	Ctr	0.91
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	Ctr	0.91
I	LOCK	4.0x 5.0	Ctr	0.3	0.80
H	LOCK	4.0x 5.0	Ctr	Ctr	0.61
Q	LOCK	5.0x 5.0	Ctr	-1.0	0.86
P	LOCK	4.0x10.0	-1.0	Ctr	1.00
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
O	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2996 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R48*	1	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 593.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 8	SP-2400
BC	0.34	2x 6	SP-#2
WB	0.97	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	53- 0- 8
BC Cont.	0- 0- 0	53- 0- 8

WB 1 rows CLB on U -Q
WB 1 rows CLB on Q -L
WB 1 rows CLB on L -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
X	486	134	3- 8	1- 8
			Hz =	-169
U	2269	718	3- 8	2- 7
H	1489	462	3- 8	1-12
			Hz =	93

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
W -P	0.06	521 C	0.00	0.06	
P -C	0.06	271 T	0.00	0.06	
C -Q	0.06	490 T	0.01	0.05	
Q -D	0.07	1550 C	0.02	0.05	
D -E	0.07	1442 C	0.00	0.07	
E -F	0.03	1764 C	0.02	0.01	
F -R	0.02	1877 C	0.00	0.02	
R -G	0.02	1309 C	0.00	0.02	
G -S	0.03	1396 C	0.01	0.02	
S -T	0.02	59 T	0.00	0.02	
Bottom Chords					
X -O	0.13	122 T	0.00	0.13	
O -N	0.16	521 T	0.03	0.13	
N -U	0.17	281 C	0.00	0.17	
U -S1	0.30	1094 T	0.13	0.17	
S1-M	0.23	1094 T	0.13	0.10	
M -L	0.22	1094 T	0.13	0.09	
L -K	0.29	1670 T	0.20	0.09	
K -J	0.26	1880 T	0.22	0.04	
J -S2	0.26	1666 T	0.20	0.06	
S2-I	0.34	1666 T	0.12	0.22	
I -H	0.28	881 T	0.06	0.22	
Webs					
X -W	0.10	425 C	WindLd		
W -O	0.18	579 T			
O -P	0.04	165 T			
P -N	0.72	872 C			
N -C	0.10	348 T			
C -U	0.11	549 C			
U -Q	0.58	2169 C			1 Br
M -Q	0.05	214 T			
Q -L	0.14	454 T			1 Br
L -D	0.07	268 T			
L -E	0.14	365 C			1 Br
K -E	0.20	568 T			
K -F	0.43	544 C			
J -F	0.18	255 C			
J -R	0.13	424 T			
R -I	0.68	718 C			
I -G	0.07	272 T			
I -S	0.23	720 T			
S -H	0.97	1571 C			
H -T	0.03	152 T			

TL Defl -0.14" in J -S2 L/999
LL Defl -0.07" in K -J L/999
Shear // Grain in W -P 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

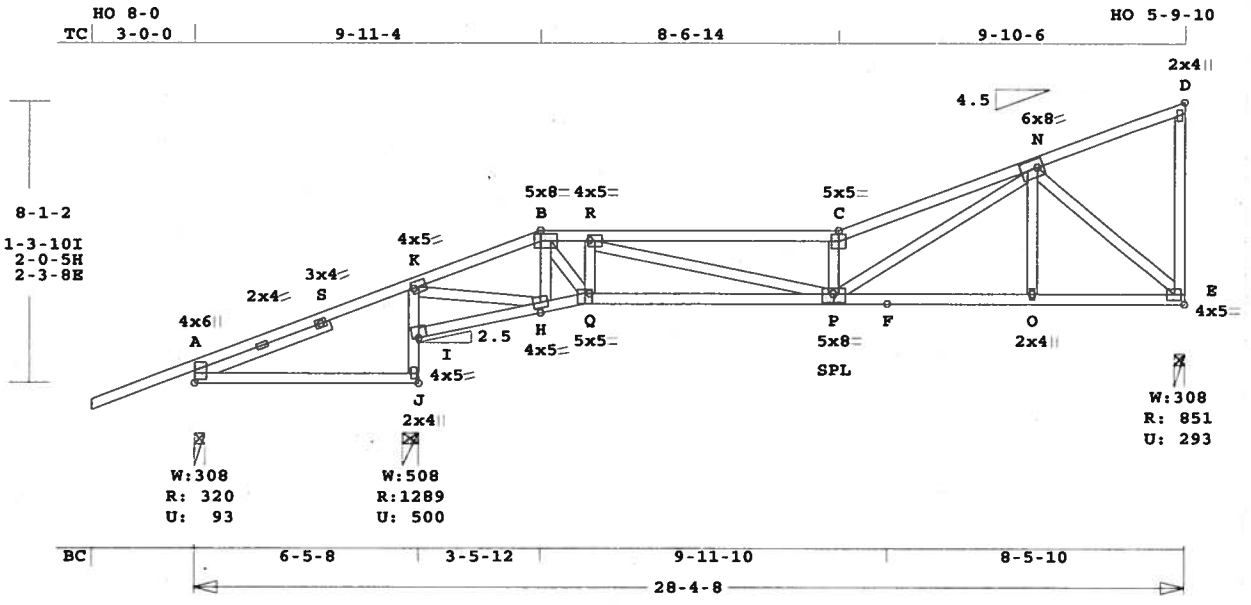
Jt	Type	Plt Size	X	Y	JSI
W	LOCK	8.0x 8.0	0.5	Ctr	0.82
P	LOCK	4.0x 5.0	Ctr	0.5	0.65
C	LOCK	8.0x 8.0	Ctr	Ctr	0.86
Q	LOCK	5.0x 8.0	Ctr	Ctr	0.87
D	LOCK	8.0x 8.0	Ctr	Ctr	0.86
E	LOCK	8.0x 8.0	1.0	Ctr	0.86
F	LOCK	8.0x 8.0	Ctr	Ctr	0.86
R	LOCK	4.0x 5.0	Ctr	0.5	0.60
G	LOCK	8.0x 8.0	Ctr	Ctr	0.86
S	LOCK	4.0x 5.0	0.5	0.2	0.74
T	LOCK	4.0x 6.0	-0.2	Ctr	0.92
X	LOCK	3.0x 5.0	Ctr	Ctr	0.94
O	LOCK	4.0x 5.0	Ctr	Ctr	0.60
N	LOCK	4.0x 5.0	Ctr	Ctr	0.60
U	LOCK	4.0x 5.0	Ctr	Ctr	0.66
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	2.0x 4.0	Ctr	Ctr	0.56
L	LOCK	7.0x12.0	1.0	-0.8	0.97
K	LOCK	4.0x 5.0	Ctr	Ctr	0.61
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.54
H	LOCK	7.0x 8.0	-0.5	Ctr	0.78

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2169 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R47A	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 200.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.54	2x 4	SP-#2
BC	0.43	2x 4	SP-#2
CW	0.93	2x 4	SP-#3
WB	0.67	2x 4	SP-#3
SL	0.14	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 4- 8
BC	Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	321	94	3- 8	1- 8
			Hz =	334
J	1289	500	5- 8	1- 8
E	852	293	3- 8	1- 8
			Hz =	181

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -S	0.30	283 C	0.00		0.30
S -K	0.53	453 C	0.04		0.49
K -B	0.49	1066 C	0.00		0.49
B -R	0.36	1663 C	0.01		0.35
R -C	0.54	2534 C	0.11		0.43
C -N	0.41	2693 C	0.09		0.32
N -D	0.34	55 C	0.00		0.34
Bottom Chords					
A -J	0.19	192 C	0.00		0.19
I -H	0.23	288 C	0.00		0.23
H -Q	0.24	1001 T	0.16		0.08
Q -P	0.43	1695 T	0.28		0.15
P -O	0.25	789 T	0.13		0.12
O -E	0.25	789 T	0.13		0.12
Chord-Webs					
J -I	0.93	1229 C	0.03		0.90
I -K	0.47	1097 C	0.02		0.45
Webs					
K -H	0.40	1262 T			
H -B	0.07	443 C			
B -Q	0.34	1078 T			
Q -R	0.09	568 C			
R -P	0.27	866 T			
P -C	0.21	1300 C			
P -N	0.67	2111 T			
O -N	0.05	218 T			
N -E	0.50	1077 C			
E -D	0.03	140 T			
Sliders					
A -S	0.14	476 T			

TL Defl -0.32" in Q -P L/813
LL Defl -0.13" in Q -P L/999

Hz Disp	LL	DL	TL
Jt E	0.02"	0.02"	0.04"

Shear // Grain in S -K 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

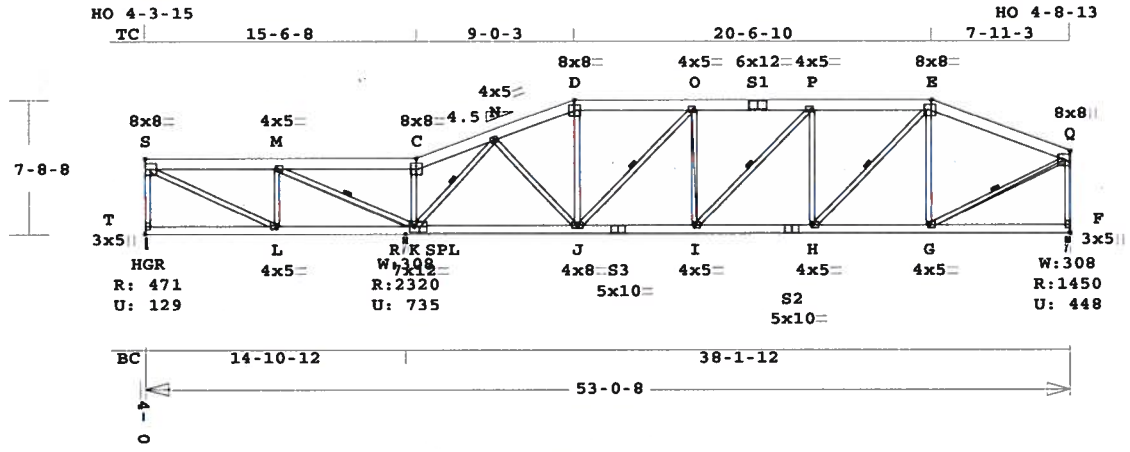
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	4.0x 5.0	Ctr	Ctr	0.70
B	LOCK	5.0x 8.0	Ctr	Ctr	0.99
R	LOCK	4.0x 5.0	Ctr	Ctr	0.77
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
N	LOCK	6.0x 8.0	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	Ctr	0.58
I	LOCK	4.0x 5.0	Ctr	0.3	0.70
H	LOCK	4.0x 5.0	Ctr	Ctr	0.61
Q	LOCK	5.0x 5.0	Ctr-1.0		0.60
P	LOCK	5.0x 8.0	Ctr-0.5		0.88
O	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 5.0	Ctr	Ctr	0.79

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2693 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R47*	1	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 556.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 8	SP-2400
BC	0.30	2x 6	SP-#2
WB	0.46	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	53- 0- 8
BC	Cont.	0- 0- 0	53- 0- 8

WB 1 rows CLB on M -K
WB 1 rows CLB on K -N
WB 1 rows CLB on J -O
WB 1 rows CLB on I -P
WB 1 rows CLB on H -E
WB 1 rows CLB on G -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
T	472	130	3- 8	1- 8
			Hz =	-164
K	2321	735	3- 8	2- 8
F	1451	449	3- 8	1-11
			Hz =	82

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
S -M	0.08	424	C	0.00	0.08
M -C	0.09	539	T	0.01	0.08
C -N	0.04	573	T	0.02	0.02
N -D	0.03	1323	C	0.00	0.03
D -O	0.06	1241	C	0.00	0.06
O -S1	0.06	1776	C	0.00	0.06
S1-P	0.04	1776	C	0.00	0.04
P -E	0.04	1770	C	0.00	0.04
E -Q	0.12	1342	C	0.00	0.12
--------Bottom Chords---------					
T -L	0.14	107	T	0.00	0.14
L -K	0.26	424	T	0.03	0.23
K -J	0.28	734	T	0.05	0.23
J -S3	0.30	1776	T	0.13	0.17
S3-I	0.25	1776	T	0.21	0.04
I -S2	0.27	1770	T	0.21	0.06
S2-H	0.25	1770	T	0.21	0.04
H -G	0.27	1262	T	0.09	0.18
G -F	0.18	81	T	0.00	0.18
------------Webs------------					
T -S	0.12	403	C	WindLd	
S -L	0.15	476	T		
L -M	0.04	160	T		
M -K	0.32	1073	C		1 Br
K -C	0.03	148	C		
K -N	0.46	1972	C		1 Br
N -J	0.24	776	T		
J -D	0.06	231	T		
J -O	0.28	781	C		1 Br
I -O	0.06	253	T		
I -P	0.01	35	C		1 Br
H -P	0.28	399	C		
H -E	0.23	728	T		1 Br
G -E	0.34	497	C		
G -Q	0.45	1423	T		1 Br
F -Q	0.37	1383	C		

TL Defl -0.13" in I -S2 L/999
LL Defl -0.06" in I -S2 L/999
Shear // Grain in S -M 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

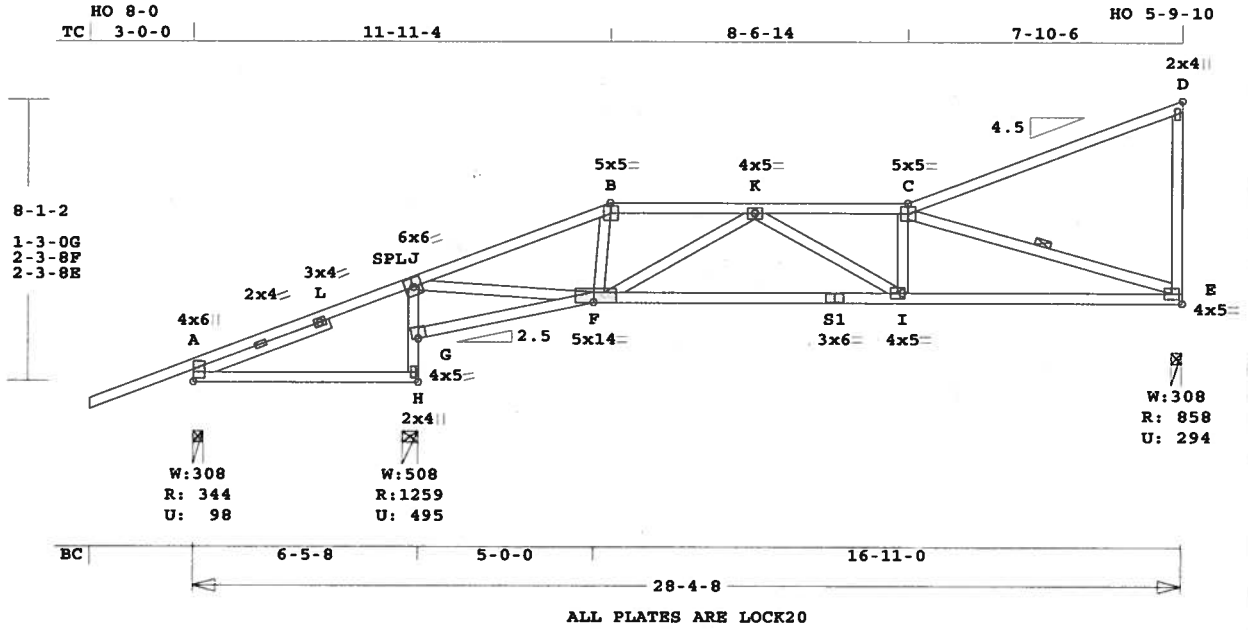
Jt	Type	Plt Size	X	Y	JSI
S	LOCK	8.0x 8.0	0.5	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	0.5	0.63
C	LOCK	8.0x 8.0	Ctr	Ctr	0.86
N	LOCK	4.0x 5.0-0.5	0.2		0.74
D	LOCK	8.0x 8.0	Ctr	Ctr	0.86
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
E	LOCK	8.0x 8.0	1.0	Ctr	0.86
Q	LOCK	8.0x 8.0-0.5	Ctr	Ctr	0.66
T	LOCK	3.0x 5.0	Ctr	Ctr	0.94
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	7.0x12.0-0.5-0.8			0.97
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1972 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R46A	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 190.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.74	2x 4	SP-#2
BC	0.63	2x 4	SP-#2
CW	0.65	2x 4	SP-#3
WB	0.54	2x 4	SP-#3
SL	0.13	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 4- 8
BC	Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on C -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	344	98	3- 8	1- 8
			Hz =	334
H	1259	496	5- 8	1- 8
E	858	294	3- 8	1- 8
			Hz =	180

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -L	0.28	298 C	0.00 0.28
L -J	0.53	446 C	0.03 0.50
J -B	0.51	1390 C	0.01 0.50
B -K	0.17	1262 C	0.01 0.16
K -C	0.66	1816 C	0.02 0.64
C -D	0.74	104 C	0.00 0.74
--------Bottom Chords---------			
A -H	0.19	138 C	0.00 0.19
G -F	0.26	169 C	0.00 0.26
F -S1	0.51	1680 T	0.28 0.23
S1-I	0.60	1680 T	0.16 0.44
I -E	0.63	1811 T	0.19 0.44
----------Chord-Webs----------			
H -G	0.65	1200 C	0.03 0.62
G -J	0.18	1075 C	0.17 0.01
------------Webs-------------			
J -F	0.46	1446 T	
F -B	0.07	285 T	
F -K	0.19	475 C	
K -I	0.06	231 T	
I -C	0.07	262 T	
C -E	0.54	1896 C	1 Br
E -D	0.10	265 T	
----------Sliders------------			
A -L	0.13	448 T	

TL Defl -0.32" in F -S1 L/812
LL Defl -0.14" in F -S1 L/999

Hz Disp	LL	DL	TL
Jt E	0.03"	0.03"	0.06"

Shear // Grain in L -J 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

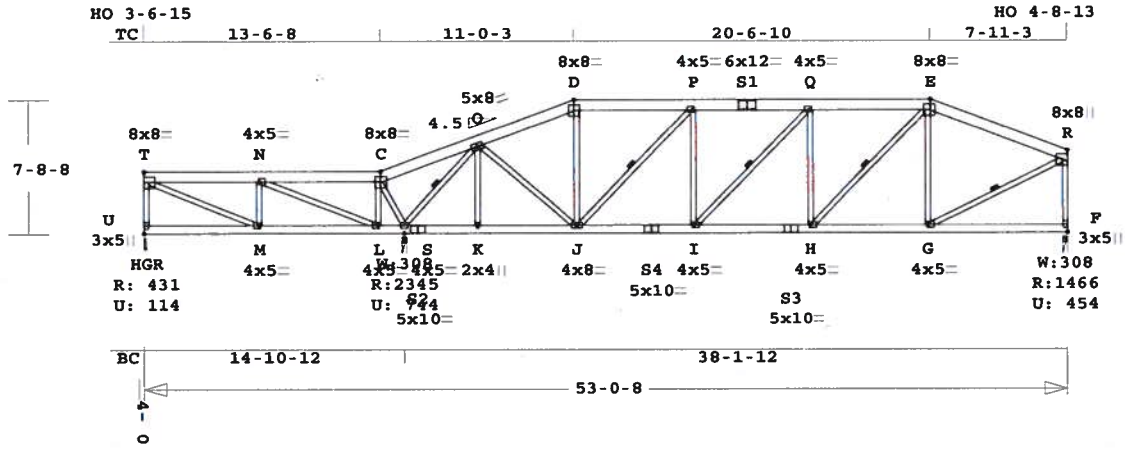
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	3.0x 4.0	Ctr	Ctr	0.54
J	LOCK	6.0x 6.0	Ctr	Ctr	0.87
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
K	LOCK	4.0x 5.0	Ctr	Ctr	0.51
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	2.0x 4.0	Ctr	Ctr	0.58
G	LOCK	4.0x 5.0	Ctr	0.3	0.70
F	LOCK	5.0x14.0	-1.0	-1.0	0.95
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.75

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1896 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R46*	1	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 561.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 8	SP-2400
BC	0.29	2x 6	SP-#2
WB	0.72	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	53- 0- 8
BC Cont.		0- 0- 0	53- 0- 8

WB 1 rows CLB on S -O
WB 1 rows CLB on J -P
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -E
WB 1 rows CLB on G -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
U	431	115	3- 8	1- 8
			Hz =	-147
S	2345	745	3- 8	2- 8
F	1467	454	3- 8	1-12
			Hz =	88

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
T -N	0.05	430	C	0.00	0.05
N -C	0.06	434	T	0.01	0.05
C -O	0.06	750	T	0.02	0.04
O -D	0.04	1398	C	0.00	0.04
D -P	0.06	1302	C	0.00	0.06
P -S1	0.06	1824	C	0.00	0.06
S1-Q	0.04	1824	C	0.00	0.04
Q -E	0.04	1802	C	0.00	0.04
E -R	0.12	1360	C	0.00	0.12
Bottom Chords					
U -M	0.12	102	T	0.00	0.12
M -L	0.15	430	T	0.03	0.12
L -S	0.15	447	C	0.00	0.15
S -S2	0.23	698	T	0.08	0.15
S2-K	0.16	698	T	0.08	0.08
K -J	0.16	698	T	0.05	0.11
J -S4	0.29	1824	T	0.22	0.07
S4-I	0.27	1824	T	0.22	0.05
I -S3	0.26	1802	T	0.21	0.05
S3-H	0.24	1802	T	0.21	0.03
H -G	0.27	1280	T	0.09	0.18
G -F	0.18	88	T	0.00	0.18
Webs					
U -T	0.08	372	C	WindLd	
T -M	0.15	475	T		
M -N	0.03	144	T		
N -L	0.72	947	C		
L -C	0.12	412	T		
C -S	0.10	535	C		
S -O	0.46	2195	C		1 Br
K -O	0.03	147	T		
O -J	0.26	812	T		
J -D	0.06	252	T		
J -P	0.28	762	C		1 Br
I -P	0.06	255	T		
I -Q	0.01	41	T		1 Br
H -Q	0.29	415	C		
H -E	0.24	749	T		1 Br
G -E	0.35	506	C		
G -R	0.46	1443	T		1 Br
F -R	0.37	1399	C		

TL Defl -0.14" in I -S3 L/999
LL Defl -0.06" in I -S3 L/999
Shear // Grain in T -N 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

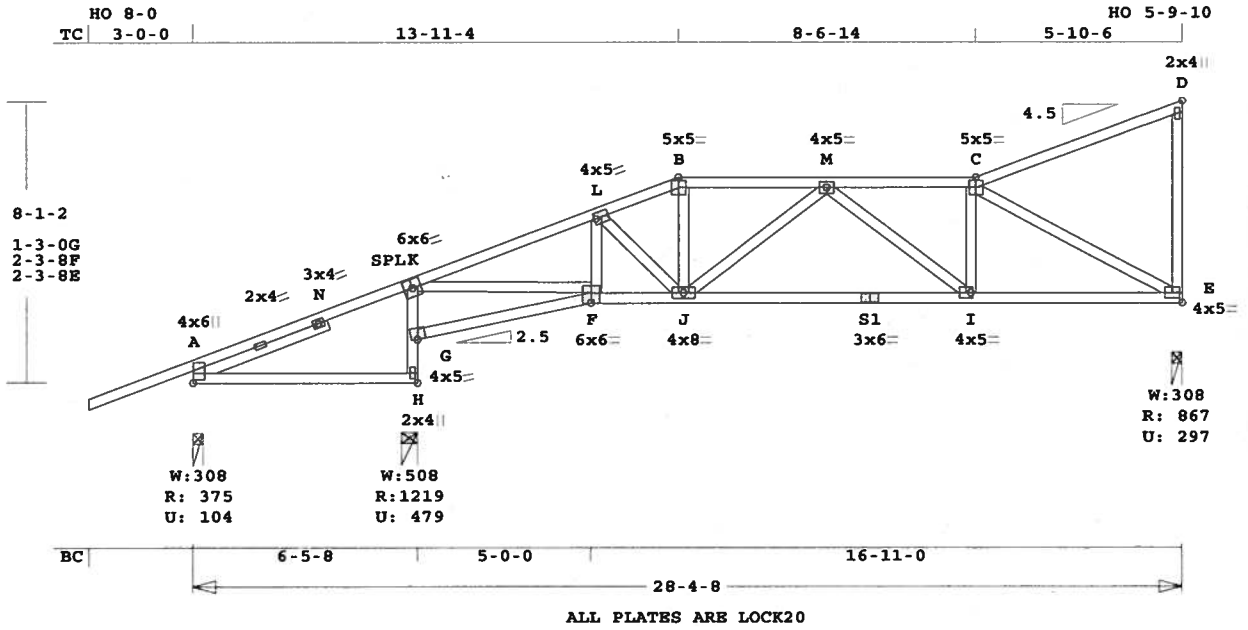
Jt	Type	Plt Size	X	Y	JSI
T	LOCK	8.0x 8.0	0.5	Ctr	0.82
N	LOCK	4.0x 5.0	Ctr	0.5	0.65
C	LOCK	8.0x 8.0	Ctr	Ctr	0.86
O	LOCK	5.0x 8.0	Ctr	Ctr	0.85
D	LOCK	8.0x 8.0	Ctr	Ctr	0.86
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
E	LOCK	8.0x 8.0	1.0	Ctr	0.86
R	LOCK	8.0x 8.0	-0.5	Ctr	0.66
U	LOCK	3.0x 5.0	Ctr	Ctr	0.94
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S	LOCK	4.0x 5.0	Ctr	Ctr	0.64
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	2.0x 4.0	Ctr	Ctr	0.56
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S4	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2195 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R45A	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 200.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.50	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
CW	0.35	2x 4	SP-#3
WB	0.86	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 4- 8
BC	Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	375	105	3- 8	1- 8
			Hz =	334
H	1219	479	5- 8	1- 8
E	867	298	3- 8	1- 8
			Hz =	180

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -N	0.27	317 C	0.00	0.27
N -K	0.50	405 C	0.02	0.48
K -L	0.49	1487 C	0.01	0.48
L -B	0.12	1346 C	0.01	0.11
B -M	0.17	1267 C	0.01	0.16
M -C	0.37	1191 C	0.01	0.36
C -D	0.43	84 C	0.00	0.43
Bottom Chords				
A -H	0.19	79 C	0.00	0.19
G -F	0.31	34 C	0.00	0.31
F -J	0.52	1377 T	0.23	0.29
J -S1	0.39	1365 T	0.13	0.26
S1-I	0.46	1365 T	0.13	0.33
I -E	0.45	1182 T	0.12	0.33
Chord-Webs				
H -G	0.35	1160 C	0.03	0.32
G -K	0.31	1044 C	0.02	0.29
Webs				
K -F	0.44	1399 T		
F -L	0.01	40 T		
L -J	0.04	196 T		
J -B	0.08	263 T		
J -M	0.05	125 C		
M -I	0.09	223 C		
I -C	0.09	373 T		
C -E	0.86	1346 C		
E -D	0.08	216 T		
Sliders				
A -N	0.11	418 T		

TL Defl -0.08" in A -H L/853
LL Defl -0.03" in A -H L/999

Hz Disp	LL	DL	TL
Jt E	0.03"	0.03"	0.07"

Shear // Grain in N -K 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

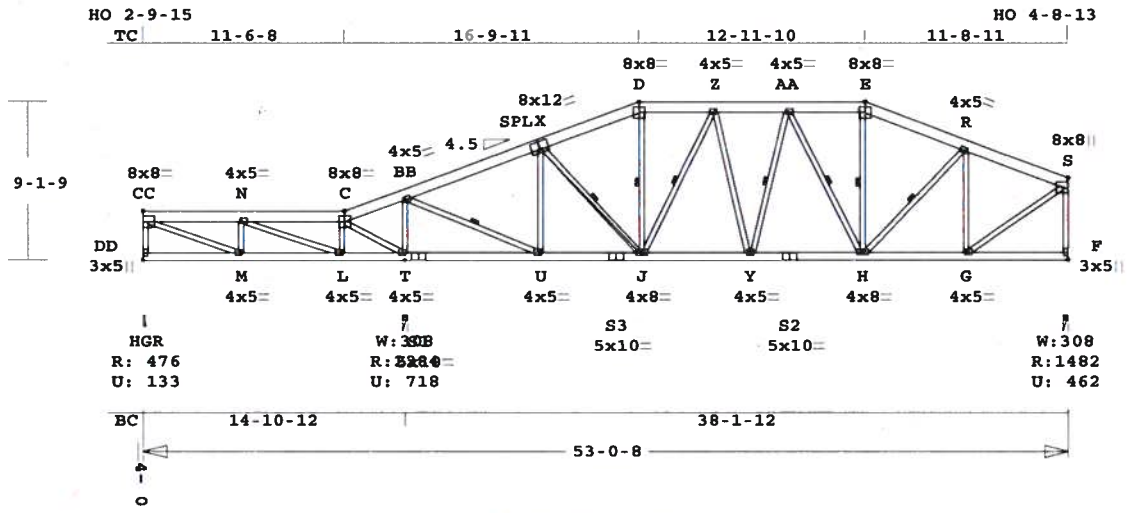
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	6.0x 6.0	Ctr	Ctr	0.87
L	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	4.0x 5.0	Ctr	Ctr	0.48
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	2.0x 4.0	Ctr	Ctr	0.58
G	LOCK	4.0x 5.0	0.3		0.70
F	LOCK	6.0x 6.0	Ctr-0.5		0.65
J	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1487 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R45*	1	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 587.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.11	2x 8	SP-2400
BC	0.23	2x 6	SP-#2
WB	0.57	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	53- 0- 8
BC	Cont.	0- 0- 0	53- 0- 8

WB 1 rows CLB on BB-U
WB 1 rows CLB on X -J
WB 1 rows CLB on J -D
WB 1 rows CLB on J -Z
WB 1 rows CLB on Y -AA
WB 1 rows CLB on AA-H
WB 1 rows CLB on H -E
WB 1 rows CLB on H -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
DD	476	133	3- 8	1- 8
			Hz =	-157
T	2284	718	3- 8	2- 7
F	1483	462	3- 8	1-12
			Hz =	110

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
CC-N	0.04	641	C	0.00	0.04
N -C	0.04	109	C	0.00	0.04
C -BB	0.11	436	T	0.01	0.10
BB-X	0.10	1363	C	0.00	0.10
X -D	0.06	1519	C	0.02	0.04
D -Z	0.03	1414	C	0.02	0.01
Z -AA	0.02	1569	C	0.00	0.02
AA-E	0.03	1376	C	0.02	0.01
E -R	0.05	1477	C	0.02	0.03
R -S	0.05	1214	C	0.01	0.04
Bottom Chords					
DD-M	0.08	126	C	0.00	0.08
M -L	0.12	641	T	0.07	0.05
L -T	0.17	97	T	0.00	0.17
T -S1	0.17	362	C	0.00	0.17
S1-U	0.10	362	C	0.00	0.10
U -S3	0.19	1266	T	0.09	0.10
S3-J	0.20	1266	T	0.15	0.05
J -Y	0.23	1560	T	0.18	0.05
Y -S2	0.22	1547	T	0.18	0.04
S2-H	0.23	1547	T	0.18	0.05
H -G	0.18	1146	T	0.13	0.05
G -F	0.09	110	T	0.00	0.09
Webs					
DD-CC	0.07	424	C	WindLd	
CC-M	0.22	695	T		
M -N	0.04	162	T		
N -L	0.31	571	C		
L -C	0.08	282	T		
C -T	0.13	531	C		
T -BB	0.39	1847	C		
BB-U	0.57	1787	T	1 Br	
U -X	0.35	615	C		
X -J	0.07	222	T	1 Br	
J -D	0.08	286	T	1 Br	
J -Z	0.11	329	C	1 Br	
Z -Y	0.03	117	T		
Y -AA	0.03	147	T	1 Br	
AA-H	0.13	386	C	1 Br	
H -E	0.07	280	T	1 Br	
H -R	0.10	334	T	1 Br	
G -R	0.40	724	C		
G -S	0.45	1427	T		
F -S	0.38	1431	C		

TL Defl -0.11" in J -Y L/999
LL Defl -0.05" in Y -S2 L/999
Shear // Grain in BB-X 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

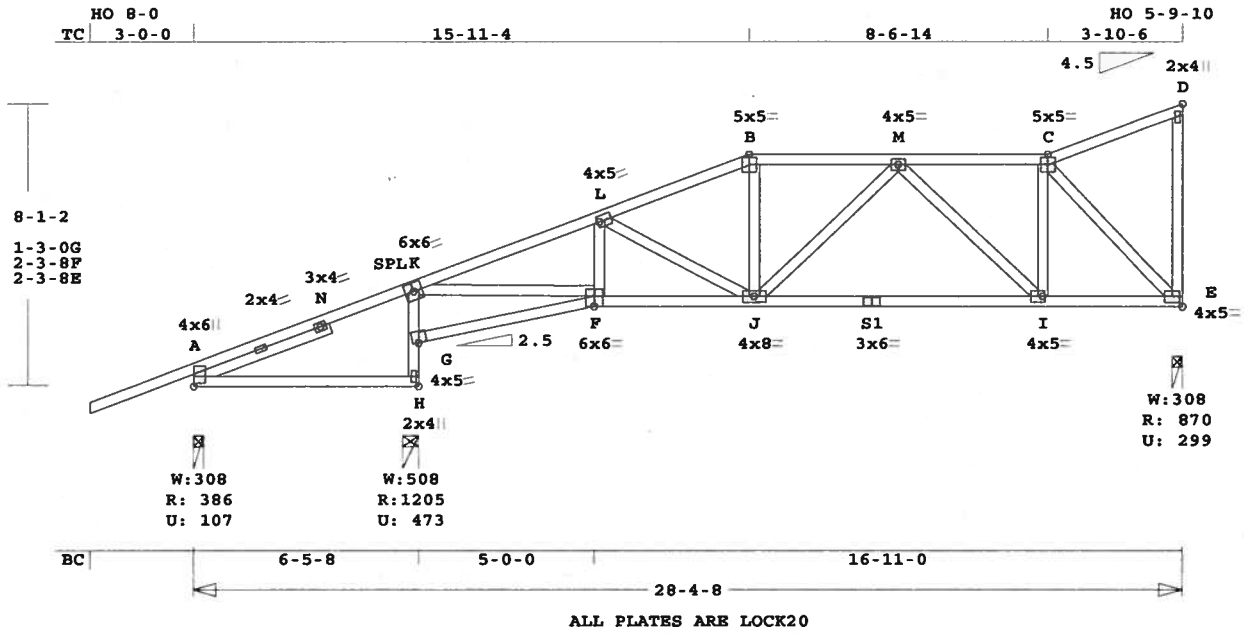
Jt	Type	Plt Size	X	Y	JSI
CC	LOCK	8.0x 8.0	0.5	Ctr	0.82
N	LOCK	4.0x 5.0	Ctr	0.5	0.68
C	LOCK	8.0x 8.0	Ctr	Ctr	0.86
BB	LOCK	4.0x 5.0	-0.5	0.2	0.82
X	LOCK	8.0x12.0	-0.6	1.5	0.97
D	LOCK	8.0x 8.0	Ctr	Ctr	0.86
Z	LOCK	4.0x 5.0	Ctr	0.5	0.62
AA	LOCK	4.0x 5.0	Ctr	0.5	0.62
E	LOCK	8.0x 8.0	Ctr	Ctr	0.86
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S	LOCK	8.0x 8.0	-0.5	Ctr	0.66
DD	LOCK	3.0x 5.0	Ctr	Ctr	0.94
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
U	LOCK	4.0x 5.0	Ctr	Ctr	0.75
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 8.0	Ctr	Ctr	0.57
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.62
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 8.0	Ctr	Ctr	0.57
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1847 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R44A	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 206.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 4	SP-#2
BC	0.49	2x 4	SP-#2
CW	0.40	2x 4	SP-#3
WB	0.45	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 4- 8
BC	Cont.	0- 0- 0	28- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	386	107	3- 8	1- 8
			Hz =	334
H	1206	474	5- 8	1- 8
E	870	299	3- 8	1- 8
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.28	322	C	0.00	0.28
N -K	0.47	393	C	0.02	0.45
K -L	0.46	1518	C	0.01	0.45
L -B	0.21	1219	C	0.06	0.15
B -M	0.14	1142	C	0.00	0.14
M -C	0.18	720	C	0.00	0.18
C -D	0.19	61	C	0.00	0.19
Bottom Chords					
A -H	0.19	66	C	0.00	0.19
G -F	0.31	48	C	0.00	0.31
F -J	0.49	1410	T	0.23	0.26
J -S1	0.36	1058	T	0.10	0.26
S1-I	0.40	1058	T	0.10	0.30
I -E	0.37	708	T	0.07	0.30
Chord-Webs					
H -G	0.26	1146	C	0.01	0.25
G -K	0.40	1034	C	0.02	0.38
Webs					
K -F	0.44	1397	T		
F -L	0.02	89	T		
L -J	0.11	304	C		
J -B	0.06	228	T		
J -M	0.04	168	T		
M -I	0.23	473	C		
I -C	0.15	485	T		
C -E	0.45	1017	C		
E -D	0.05	158	T		
Sliders					
A -N	0.11	408	T		

TL Defl -0.08" in A -H L/847
LL Defl -0.03" in A -H L/999

Hz Disp	LL	DL	TL
Jt E	0.03"	0.03"	0.06"

Shear // Grain in N -K 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

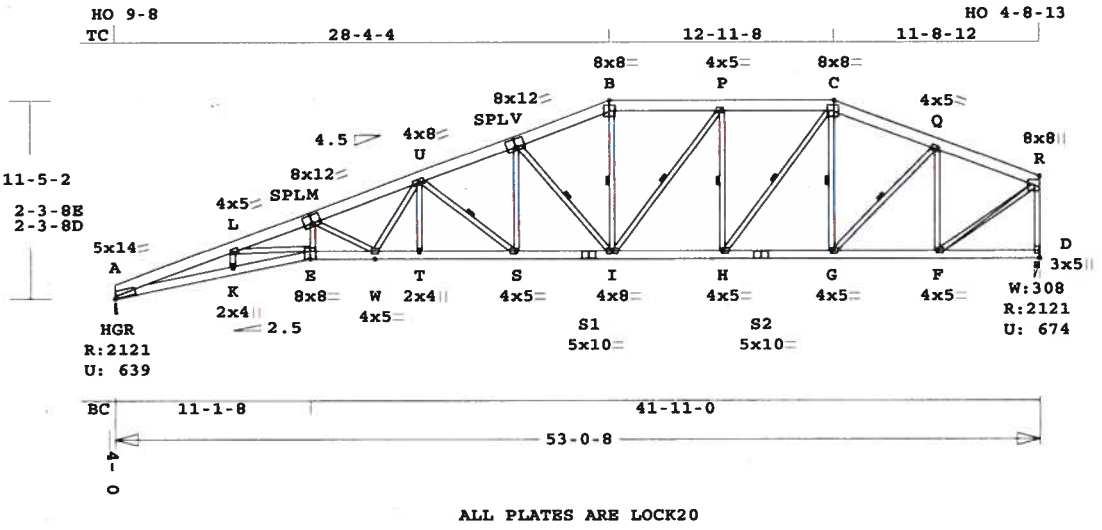
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.0	0.3	0.88
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	3.0x 4.0	Ctr	Ctr	0.54
K	LOCK	6.0x 6.0	Ctr	Ctr	0.87
L	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
M	LOCK	4.0x 5.0	Ctr	Ctr	0.47
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	2.0x 4.0	Ctr	Ctr	0.58
G	LOCK	4.0x 5.0	Ctr	0.3	0.70
F	LOCK	6.0x 6.0	Ctr	-0.5	0.65
J	LOCK	4.0x 8.0	Ctr	Ctr	0.54
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1518 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R44*	2	SP	530008	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 568.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.32	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
EX	S1-S2	2x 6	SP-#2
EX	S2-D	2x 6	SP-#2
WB	0.72	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	53- 0- 8
BC Cont.	0- 0- 0	53- 0- 8

WB 1 rows CLB on U -S
WB 1 rows CLB on V -I
WB 1 rows CLB on I -B
WB 1 rows CLB on I -P
WB 1 rows CLB on H -P
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
WB 1 rows CLB on G -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2122	639	3- 8	1-12
		Hz =	279	
D	2122	675	3- 8	2- 8
		Hz =	180	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.30	8529	C	0.08	0.22
L -M	0.32	9052	C	0.09	0.23
M -U	0.16	6614	C	0.05	0.11
U -V	0.07	4473	C	0.05	0.02
V -B	0.07	3445	C	0.04	0.03
B -P	0.07	3227	C	0.01	0.06
P -C	0.05	2933	C	0.01	0.04
C -Q	0.07	2403	C	0.03	0.04
Q -R	0.06	1826	C	0.02	0.04
--------Bottom Chords---------					
A -K	0.55	8021	T	0.36	0.19
K -E	0.55	8064	T	0.36	0.19
E -W	0.45	8443	T	0.38	0.07
W -T	0.29	5482	T	0.25	0.04
T -S	0.28	5482	T	0.25	0.03
S -S1	0.21	4183	T	0.19	0.02
S1-I	0.56	4183	T	0.50	0.06
I -H	0.41	2933	T	0.35	0.06
H -S2	0.30	2244	T	0.27	0.03
S2-G	0.33	2244	T	0.27	0.06
G -F	0.26	1720	T	0.20	0.06
F -D	0.09	179	T	0.00	0.09
------------Webs------------					
K -L	0.03	156	T		
L -E	0.20	631	T		
E -M	0.58	1825	T		
M -W	0.72	2594	C		
W -U	0.47	1481	T		
T -U	0.05	189	T		
U -S	0.38	1686	C		1 Br
S -V	0.37	1176	T		
V -I	0.43	1473	C		1 Br
I -B	0.27	855	T		1 Br
I -P	0.15	495	T		1 Br
H -P	0.23	795	C		1 Br
H -C	0.36	1144	T		1 Br
G -C	0.12	419	C		1 Br
G -Q	0.24	756	T		1 Br
F -Q	0.64	1153	C		
F -R	0.68	2141	T		
D -R	0.55	2069	C		

TL Defl -1.00" in W -T L/631
LL Defl -0.50" in W -T L/999

Hz Disp	LL	DL	TL
Jt D	0.20"	0.20"	0.40"

Shear // Grain in A -L 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

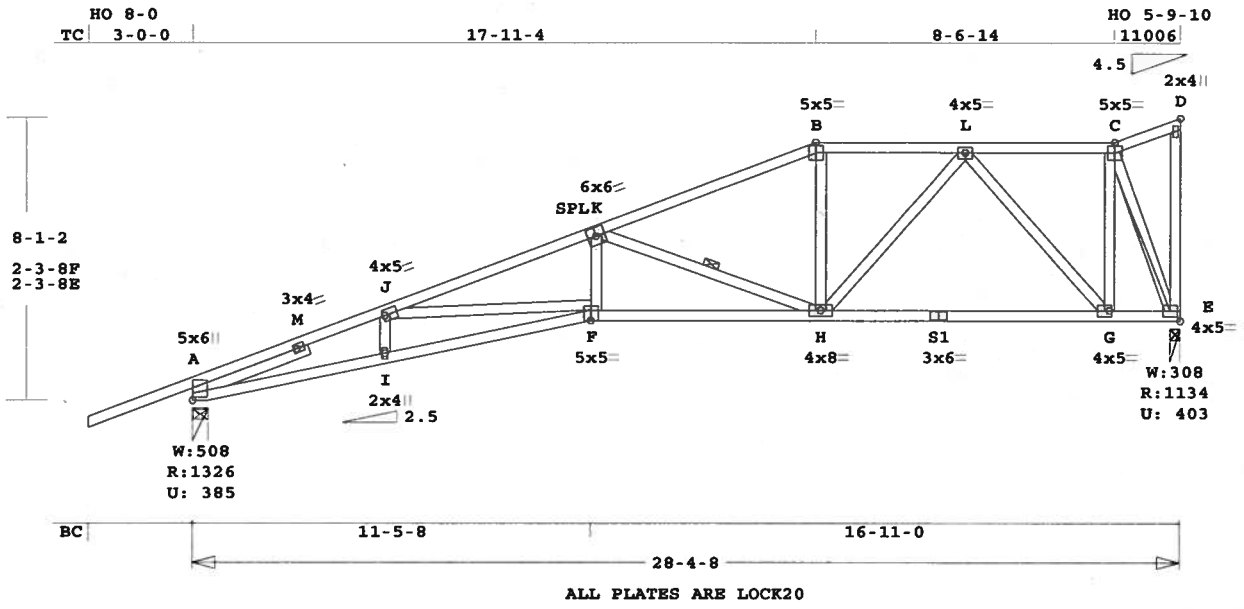
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x14.0	7.4	2.8	0.97
L	LOCK	4.0x 5.0-0.5	0.2	0.62	
M	LOCK	8.0x12.0-0.6	1.5	0.97	
U	LOCK	4.0x 8.0-0.5	0.2	0.95	
V	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	8.0x 8.0	Ctr	Ctr	0.86
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	8.0x 8.0	1.0	Ctr	0.86
Q	LOCK	4.0x 5.0	0.5	0.2	0.56
R	LOCK	8.0x 8.0-0.5	Ctr	Ctr	0.69
K	LOCK	2.0x 4.0	Ctr	Ctr	0.56
E	LOCK	8.0x 8.0	Ctr-1.5		0.82
W	LOCK	4.0x 5.0	Ctr	Ctr	0.90
T	LOCK	2.0x 4.0	Ctr	Ctr	0.56
S	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 5.0	Ctr	Ctr	0.78
D	LOCK	3.0x 5.0	Ctr	Ctr	0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9052 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R43A	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 211.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.80	2x 4	SP-#2
BC	0.73	2x 4	SP-#2
EX A -F		2x 4	SP-2400
WB	0.59	2x 4	SP-#3
SL	0.39	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on K -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1327	385	5- 8	1- 8
			Hz =	334
E	1135	404	3- 8	1- 8
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.60	1725	C	0.01	0.59
M -J	0.68	3697	C	0.09	0.59
J -K	0.80	3489	C	0.19	0.61
K -B	0.44	1662	C	0.01	0.43
B -L	0.22	1549	C	0.10	0.12
L -C	0.14	437	C	0.00	0.14
C -D	0.05	31	C	0.00	0.05
Bottom Chords					
A -I	0.53	3463	T	0.24	0.29
I -F	0.40	3482	T	0.25	0.15
F -H	0.73	3257	T	0.54	0.19
H -S1	0.44	1103	T	0.11	0.33
S1-G	0.39	1103	T	0.11	0.28
G -E	0.32	419	T	0.04	0.28
Webs					
I -J	0.04	152	T		
J -F	0.06	134	T		
F -K	0.27	844	T		
K -H	0.42	1824	C		1 Br
H -B	0.09	341	T		
H -L	0.21	683	T		
L -G	0.59	1020	C		
G -C	0.29	918	T		
C -E	0.45	1166	C		
E -D	0.03	82	T		
Sliders					
A -M	0.39	2070	C		

TL Defl -0.55" in I -F L/599
LL Defl -0.26" in I -F L/999

Hz Disp	LL	DL	TL
Jt E	0.10"	0.10"	0.20"

Shear // Grain in M -J 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

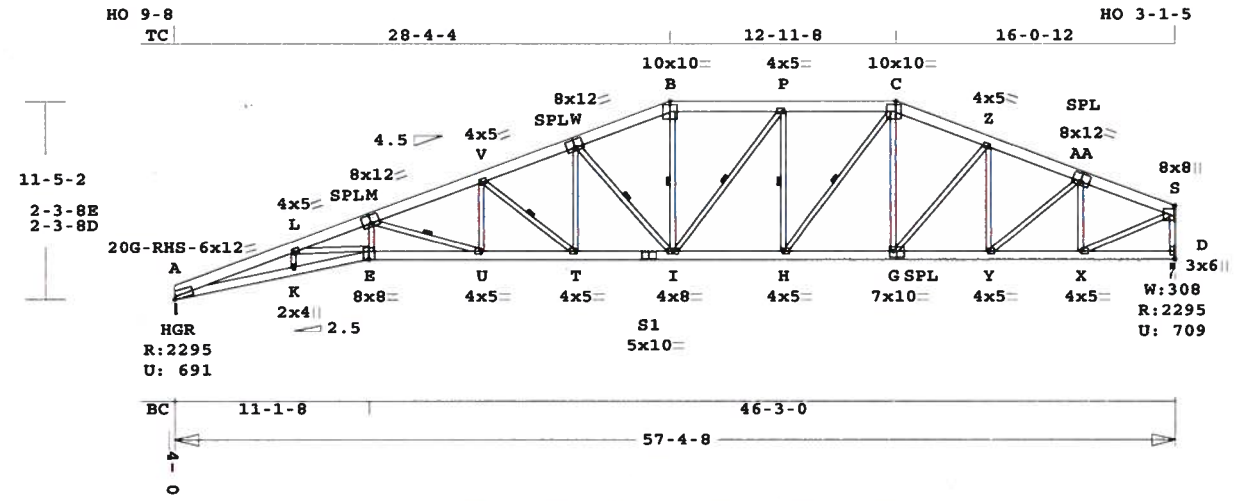
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 6.0	2.5	0.7	0.95
M	LOCK	3.0x 4.0	Ctr	Ctr	0.77
J	LOCK	4.0x 5.0	Ctr	Ctr	0.64
K	LOCK	6.0x 6.0	Ctr	Ctr	0.88
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
L	LOCK	4.0x 5.0	Ctr	Ctr	0.49
C	LOCK	5.0x 5.0	Ctr	Ctr	0.80
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
I	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	5.0x 5.0	Ctr	-1.0	0.98
H	LOCK	4.0x 8.0	Ctr	Ctr	0.66
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	4.0x 5.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 334 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3697 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R43	4	SP	570408	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 608.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.36	2x 8	SP-2400
BC	0.61	2x 6	SP-2400
EX	S1-G	2x 6	SP-#2
EX	G -D	2x 6	SP-#2
WB	0.85	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 4- 8
BC	Cont.	0- 0- 0	57- 4- 8

WB 1 rows CLB on M -U
WB 1 rows CLB on V -T
WB 1 rows CLB on W -I
WB 1 rows CLB on I -B
WB 1 rows CLB on I -P
WB 1 rows CLB on H -P
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2295	692	3- 8	1-14
		Hz =	204	
D	2295	710	3- 8	2-11
		Hz =	173	

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.33	9344	C	0.10 0.23
L -M	0.36	9895	C	0.11 0.25
M -V	0.21	6542	C	0.05 0.16
V -W	0.09	5121	C	0.06 0.03
W -B	0.08	4051	C	0.05 0.03
B -P	0.07	3798	C	0.01 0.06
P -C	0.06	3633	C	0.01 0.05
C -Z	0.07	3280	C	0.04 0.03
Z -AA	0.05	3210	C	0.03 0.02
AA-S	0.05	2546	C	0.02 0.03
Bottom Chords				
A -K	0.61	8785	T	0.40 0.21
K -E	0.61	8832	T	0.40 0.21
E -U	0.48	9214	T	0.42 0.06
U -T	0.31	6166	T	0.28 0.03
T -S1	0.22	4780	T	0.21 0.01
S1-I	0.64	4780	T	0.58 0.06
I -H	0.50	3632	T	0.44 0.06
H -G	0.42	3066	T	0.37 0.05
G -Y	0.41	3005	T	0.36 0.05
Y -X	0.33	2387	T	0.29 0.04
X -D	0.07	172	T	0.00 0.07
Webs				
K -L	0.04	160	T	
L -E	0.21	655	T	
E -M	0.63	1966	T	
M -U	0.69	3229	C	1 Br
U -V	0.36	1149	T	
V -T	0.41	1800	C	1 Br
T -W	0.40	1255	T	
W -I	0.44	1514	C	1 Br
I -B	0.34	1061	T	1 Br
I -P	0.08	277	T	1 Br
H -P	0.18	629	C	1 Br
H -C	0.30	940	T	1 Br
G -C	0.04	177	T	
G -Z	0.05	135	T	
Y -Z	0.25	425	C	
Y -AA	0.26	811	T	
X -AA	0.32	1087	C	
X -S	0.85	2671	T	
D -S	0.39	2244	C	

TL Defl -1.20" in E -U L/569
LL Defl -0.59" in E -U L/999

Hz Disp	LL	DL	TL
Jt D	0.25"	0.25"	0.49"

Shear // Grain in A -L 0.26

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

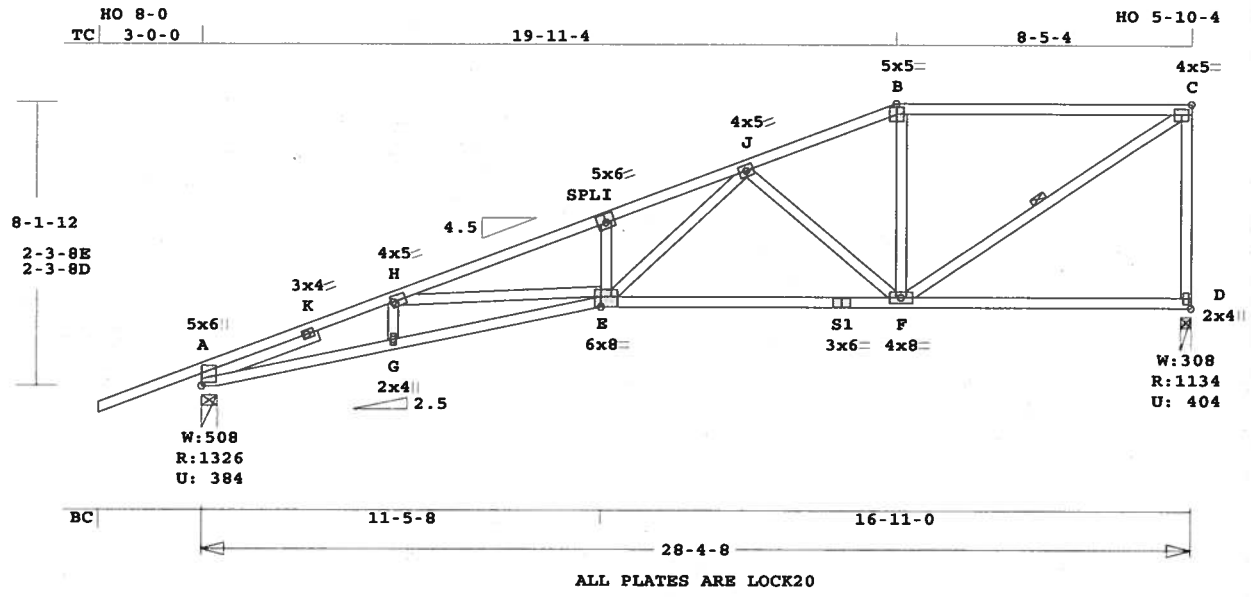
Jt	Type	Plt Size	X	Y	JSI
A	RHS	6.0x12.0	6.6	2.8	0.97
L	LOCK	4.0x 5.0-0.5	0.2	0.62	
M	LOCK	8.0x12.0-0.6	1.5	0.97	
V	LOCK	4.0x 5.0-0.5	0.2	0.86	
W	LOCK	8.0x12.0-0.6	1.5	0.97	
B	LOCK	10.0x10.0	Ctr	Ctr	0.72
P	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.72
Z	LOCK	4.0x 5.0	0.5	0.2	0.56
AA	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	8.0x 8.0-0.5	Ctr	Ctr	0.86
K	LOCK	2.0x 4.0	Ctr	Ctr	0.58
E	LOCK	8.0x 8.0	Ctr-1.5		0.87
U	LOCK	4.0x 5.0-0.5	Ctr	Ctr	0.93
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	7.0x10.0	Ctr-0.8		0.97
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.60
X	LOCK	4.0x 5.0	1.0	Ctr	1.00
D	LOCK	3.0x 6.0	Ctr	Ctr	0.84

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9895 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

JOB	MARK	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R42A*	1	SP	280408	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 199.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.91	2x 4	SP-#2
BC	0.71	2x 4	SP-#2
EX A -E		2x 4	SP-2400
WB	0.59	2x 4	SP-#3
SL	0.38	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 4- 8
BC Cont.	0- 0- 0	28- 4- 8

WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1327	385	5- 8	1- 8
			Hz =	337
D	1135	405	3- 8	1- 8
			Hz =	182

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -K	0.58	1753 C	0.01 0.57
K -H	0.66	3658 C	0.09 0.57
H -I	0.86	3535 C	0.20 0.66
I -J	0.42	3561 C	0.09 0.33
J -B	0.39	1303 C	0.01 0.38
B -C	0.91	1235 C	0.04 0.87
--------Bottom Chords--------			
A -G	0.55	3426 T	0.24 0.31
G -E	0.41	3453 T	0.24 0.17
E -S1	0.53	1963 T	0.32 0.21
S1-F	0.71	1963 T	0.20 0.51
F -D	0.51	181 T	0.00 0.51
------------Webs------------			
G -H	0.03	130 T	
H -E	0.03	129 T	
E -I	0.07	387 C	
E -J	0.59	1854 T	
J -F	0.50	970 C	
F -B	0.05	198 T	
F -C	0.47	1495 T	1 Br
D -C	0.49	1071 C	
-----------Sliders-----------			
A -K	0.38	2002 C	

TL Defl -0.55" in E -S1 L/597
LL Defl -0.26" in G -E L/999

Hz Disp	LL	DL	TL
Jt D	0.09"	0.09"	0.18"

Shear // Grain in B -C 0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

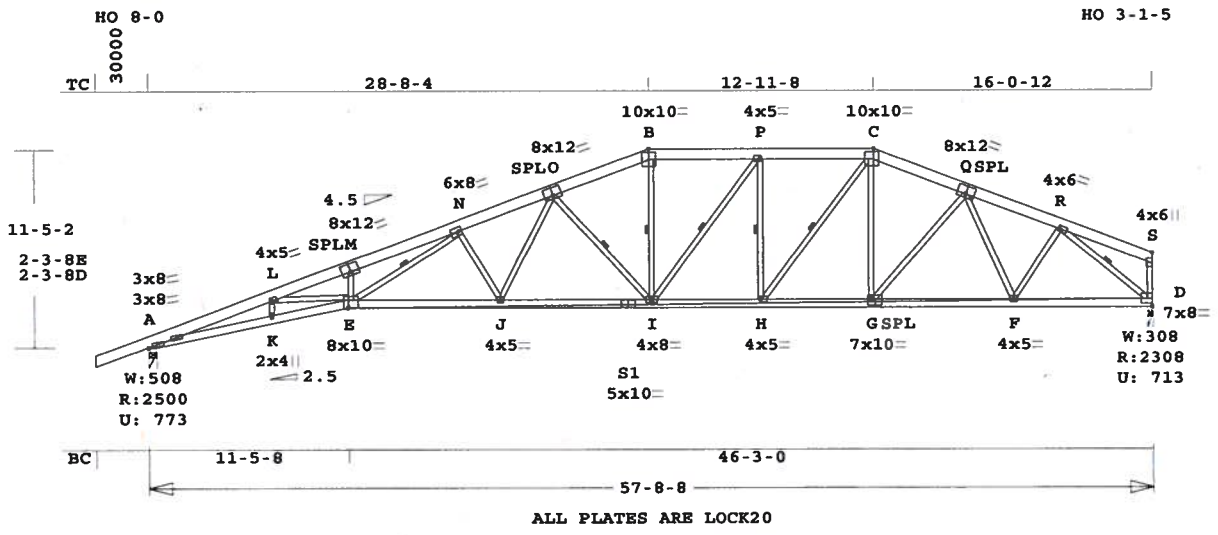
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 6.0	2.5	0.7	0.94
K	LOCK	3.0x 4.0	Ctr	Ctr	0.74
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
I	LOCK	5.0x 6.0	-0.2	0.5	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.83
B	LOCK	5.0x 5.0	Ctr	Ctr	0.80
C	LOCK	4.0x 5.0	Ctr	Ctr	0.61
G	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	6.0x 8.0	Ctr	-0.5	0.76
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 8.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.46

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing for 336 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3658 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R42	4	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 607.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.41	2x 8	SP-2400
BC	0.96	2x 6	SP-#2
EX	A -E	2x 6	SP-2400
WB	0.69	2x 4	SP-#3
EX	E -N	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on E -N
WB 1 rows CLB on O -I
WB 1 rows CLB on I -B
WB 1 rows CLB on I -P
WB 1 rows CLB on H -P
WB 1 rows CLB on H -C
WB 1 rows CLB on R -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2500	773	5- 8	2- 1
			Hz =	210
D	2308	714	3- 8	2-12
			Hz =	175

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.38	9579	C	0.10	0.28
L -M	0.41	10405	C	0.12	0.29
M -N	0.30	10382	C	0.13	0.17
N -O	0.13	5976	C	0.04	0.09
O -B	0.08	4096	C	0.05	0.03
B -P	0.07	3843	C	0.01	0.06
P -C	0.06	3667	C	0.01	0.05
C -Q	0.07	3310	C	0.04	0.03
Q -R	0.05	2976	C	0.01	0.04
R -S	0.05	75	T	0.00	0.05
--------Bottom Chords---------					
A -K	0.72	9029	T	0.41	0.31
K -E	0.72	9104	T	0.41	0.31
E -J	0.96	6302	T	0.76	0.20
J -S1	0.74	4844	T	0.58	0.16
S1-I	0.64	4844	T	0.58	0.06
I -H	0.50	3666	T	0.44	0.06
H -G	0.45	3095	T	0.37	0.08
G -F	0.44	3015	T	0.36	0.08
F -D	0.38	2367	T	0.28	0.10
------------Webs------------					
K -L	0.05	276	C		
L -E	0.28	901	T		
E -M	0.06	265	C		
E -N	0.77	4196	T		1 Br
N -J	0.57	1545	C		
J -O	0.56	1745	T		
O -I	0.46	1536	C		1 Br
I -B	0.34	1068	T		1 Br
I -P	0.09	297	T		1 Br
H -P	0.19	645	C		1 Br
H -C	0.30	949	T		1 Br
G -C	0.05	190	T		
G -Q	0.04	151	T		
Q -F	0.41	595	C		
F -R	0.26	838	T		
R -D	0.69	3150	C		1 Br
D -S	0.04	192	T		

TL Defl -1.36" in E -J L/502
LL Defl -0.66" in E -J L/999

Hz Disp	LL	DL	TL
Jt D	0.26"	0.27"	0.53"

Shear // Grain in A -L 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

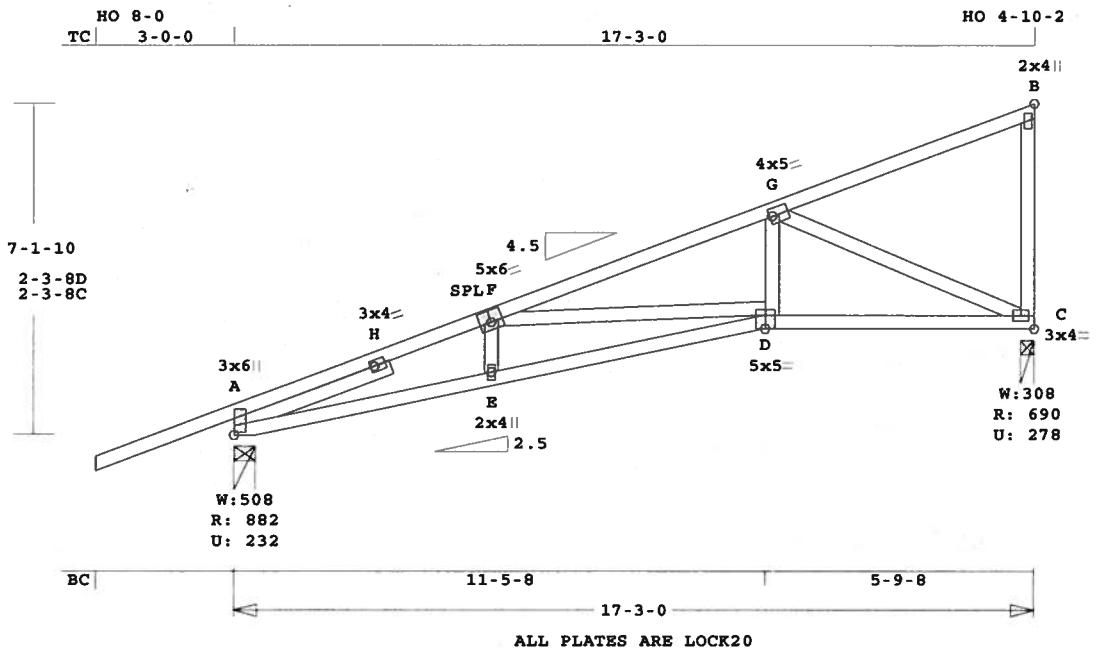
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.88
A	LOCK	3.0x 8.0	Ctr	Ctr	0.76
L	LOCK	4.0x 5.0-0.5	0.2		0.62
M	LOCK	8.0x12.0-0.6	1.5		0.97
N	LOCK	6.0x 8.0	Ctr	Ctr	0.89
O	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
P	LOCK	4.0x 5.0	Ctr		0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 6.0	0.5	0.2	0.86
S	LOCK	4.0x 6.0-0.2	Ctr		1.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x10.0	Ctr-1.5		0.94
J	LOCK	4.0x 5.0	Ctr	Ctr	0.76
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	7.0x10.0	Ctr-0.8		0.97
F	LOCK	4.0x 5.0	Ctr	Ctr	0.58
D	LOCK	7.0x 8.0-0.5	Ctr		0.85

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10405 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R41A*	8	SP	170300	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 116.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.42	2x 4	SP-#2
BC	0.50	2x 4	SP-#2
WB	0.64	2x 4	SP-#3
SL	0.18	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17- 3- 0
BC Cont.	0- 0- 0	17- 3- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	882	233	5- 8	1- 8
			Hz =	291
C	690	278	3- 8	1- 8
			Hz =	156

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -H	0.20	1013 C	0.00 0.20
H -F	0.22	1925 C	0.02 0.20
F -G	0.42	1214 C	0.00 0.42
G -B	0.42	72 C	0.00 0.42
--------Bottom Chords---------			
A -E	0.50	1823 T	0.30 0.20
E -D	0.47	1834 T	0.30 0.17
D -C	0.34	1132 T	0.19 0.15
------------Webs------------			
E -F	0.04	177 T	
F -D	0.34	642 C	
D -G	0.16	531 T	
G -C	0.64	1237 C	
C -B	0.04	182 T	
----------Sliders----------			
A -H	0.18	984 C	

TL Defl -0.18" in E -D L/999
LL Defl -0.08" in E -D L/999

Hz Disp	LL	DL	TL
Jt C	0.03"	0.03"	0.06"

Shear // Grain in F -G 0.26

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

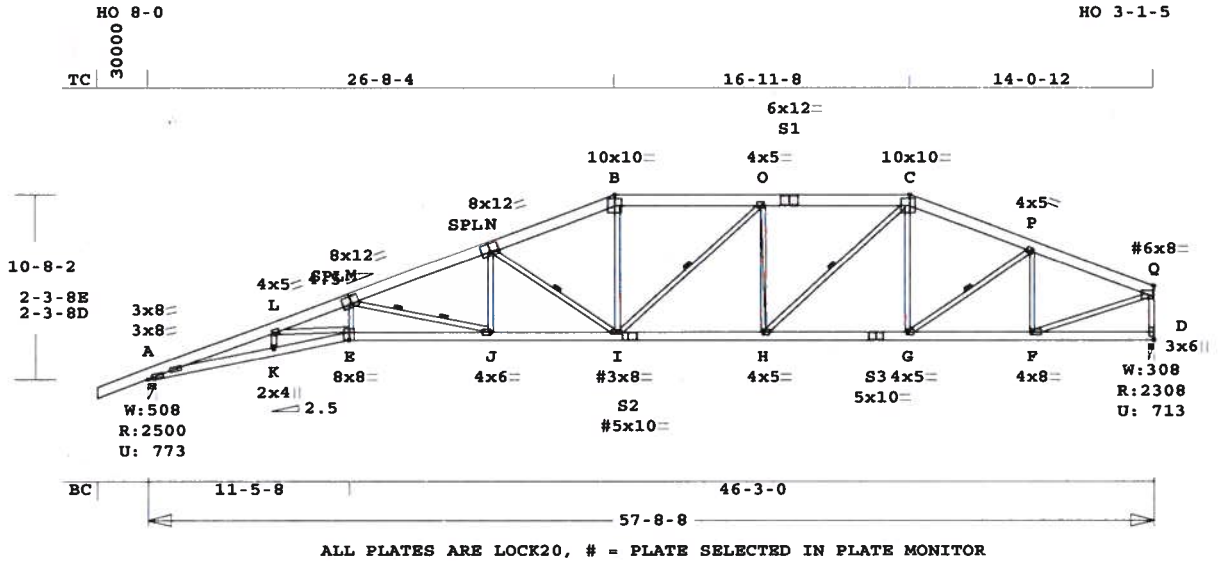
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	1.5	0.3	0.81
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	5.0x 6.0-0.2	0.5		0.87
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr-1.0		0.79
C	LOCK	3.0x 4.0	Ctr	Ctr	0.67

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1925 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R41	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 583.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S2-S3	2x 6	SP-#2
EX	S3-D	2x 6	SP-#2
WB	0.93	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	57- 8- 8
BC Cont.	0- 0- 0	57- 8- 8

WB 2 rows CLB on M -J
WB 1 rows CLB on N -I
WB 1 rows CLB on I -O
WB 1 rows CLB on H -C
WB 1 rows CLB on G -P
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2501	773	5- 8	2- 1
			Hz =	210
D	2309	714	3- 8	2-12
			Hz =	160

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.34	9860	C	0.11	0.23
L -M	0.37	10192	C	0.12	0.25
M -N	0.23	6173	C	0.05	0.18
N -B	0.12	4495	C	0.02	0.10
B -O	0.11	4196	C	0.02	0.09
O -S1	0.09	4048	C	0.04	0.05
S1-C	0.08	4048	C	0.01	0.07
C -P	0.08	3335	C	0.03	0.05
P -Q	0.08	2906	C	0.03	0.05
Bottom Chords					
A -K	0.63	9309	T	0.42	0.21
K -E	0.63	9349	T	0.42	0.21
E -J	0.47	9488	T	0.43	0.04
J -I	0.31	5819	T	0.26	0.05
I -S2	0.23	4048	T	0.18	0.05
S2-H	0.56	4048	T	0.49	0.07
H -S3	0.44	3114	T	0.37	0.07
S3-G	0.47	3114	T	0.37	0.10
G -F	0.43	2733	T	0.33	0.10
F -D	0.12	160	T	0.00	0.12
Webs					
K -L	0.03	143	T		
L -E	0.13	428	T		
E -M	0.66	2082	T		
M -J	0.72	3805	C		2 Br
J -N	0.36	1142	T		
N -I	0.63	1984	C		1 Br
I -B	0.37	1172	T		
I -O	0.07	202	T		1 Br
H -O	0.58	694	C		
H -C	0.40	1266	T		1 Br
G -C	0.09	130	T		
G -P	0.15	468	T		1 Br
F -P	0.33	897	C		
F -Q	0.93	2923	T		
D -Q	0.39	2244	C		

TL Defl -1.34" in E -J L/511
LL Defl -0.65" in E -J L/999

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Shear // Grain in A -L 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.91
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0	-0.5	0.2	0.62
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
C	LOCK	10.0x10.0	0.5	Ctr	0.73
P	LOCK	4.0x 5.0	0.5	0.2	0.56
Q#	LOCK	6.0x 8.0	-1.0	0.5	0.94
K	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
J	LOCK	4.0x 6.0	-1.0	Ctr	0.97
I#	LOCK	3.0x 8.0	-0.8	0.1	0.69
S2#	LOCK	5.0x10.0	0.9	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 8.0	1.0	Ctr	1.00
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

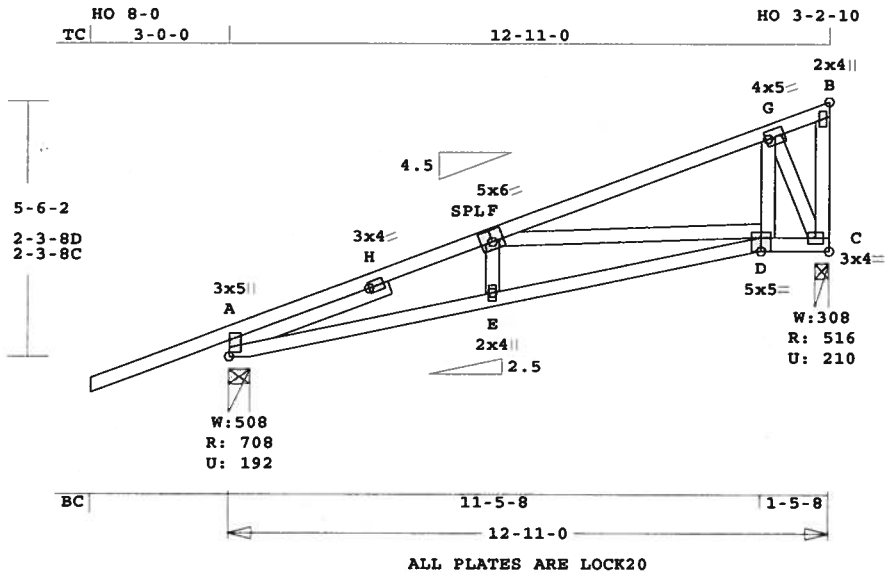
\# = Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10192 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R40A	9	SP	121100	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 90.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.46	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	709	192	5- 8	1- 8
			Hz =	218
C	517	211	3- 8	1- 8
			Hz =	117

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -H	0.10	746 C	0.04 0.06
H -F	0.24	1242 C	0.07 0.17
F -G	0.33	285 C	0.00 0.33
G -B	0.33	54 C	0.00 0.33
--------Bottom Chords---------			
A -E	0.29	1192 T	0.19 0.10
E -D	0.36	1198 T	0.20 0.16
D -C	0.17	264 T	0.04 0.13
------------Webs------------			
E -F	0.05	202 T	
F -D	0.46	897 C	
D -G	0.08	304 T	
G -C	0.11	609 C	
C -B	0.03	112 T	
----------Sliders----------			
A -H	0.10	558 C	

TL Defl -0.09" in E -D L/999
LL Defl -0.04" in E -D L/999

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"

Shear // Grain in F -G 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	5.0x 6.0	-0.2	0.5	0.87
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.76
C	LOCK	3.0x 4.0	Ctr	Ctr	0.58

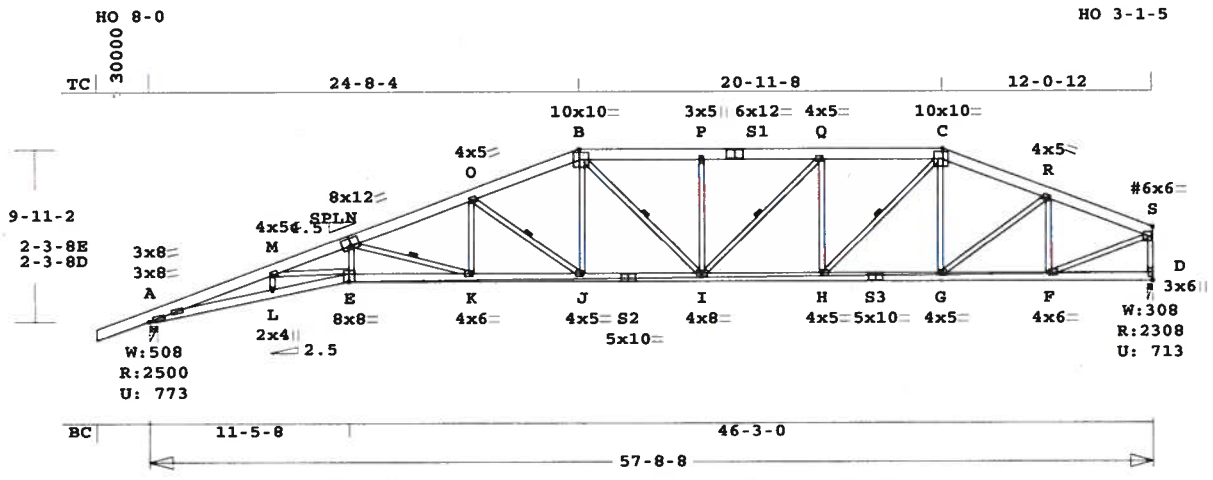
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1242 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc./Online Plus™ © 1996-2006 Version 19.0.009 Engineering - Portrait 3/16/2006 1:16:34 PM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R40	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 593.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX S2-S3		2x 6	SP-#2
EX S3-D		2x 6	SP-#2
WB	0.89	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8
WB	1 rows	CLB on N -K	
WB	1 rows	CLB on O -J	
WB	1 rows	CLB on B -I	
WB	1 rows	CLB on I -Q	
WB	1 rows	CLB on H -C	

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2501	773	5- 8	2- 1
			Hz =	210
D	2309	714	3- 8	2-12
			Hz =	146

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.34	9854	C	0.11	0.23
M -N	0.37	10196	C	0.12	0.25
N -O	0.22	6450	C	0.05	0.17
O -B	0.10	4884	C	0.05	0.05
B -P	0.09	4638	C	0.02	0.07
P -S1	0.08	4638	C	0.02	0.06
S1-Q	0.07	4638	C	0.05	0.02
Q -C	0.06	4130	C	0.01	0.05
C -R	0.07	3300	C	0.03	0.04
R -S	0.07	2718	C	0.03	0.04
Bottom Chords					
A -L	0.63	9301	T	0.42	0.21
L -E	0.63	9342	T	0.42	0.21
E -K	0.48	9491	T	0.43	0.05
K -J	0.30	6076	T	0.27	0.03
J -S2	0.22	4550	T	0.20	0.02
S2-I	0.61	4550	T	0.55	0.06
I -H	0.56	4130	T	0.50	0.06
H -S3	0.42	3084	T	0.37	0.05
S3-G	0.44	3084	T	0.37	0.07
G -F	0.38	2553	T	0.31	0.07
F -D	0.09	146	T	0.00	0.09
Webs					
L -M	0.03	147	T		
M -E	0.14	438	T		
E -N	0.66	2079	T		
N -K	0.85	3581	C		1 Br
K -O	0.38	1185	T		
O -J	0.49	1886	C		1 Br
J -B	0.40	1273	T		
B -I	0.04	142	T		1 Br
I -P	0.32	470	C		
I -Q	0.23	730	T		1 Br
H -Q	0.62	913	C		
H -C	0.47	1476	T		1 Br
G -C	0.18	265	C		
G -R	0.21	668	T		
F -R	0.33	1017	C		
F -S	0.89	2793	T		
D -S	0.39	2251	C		

TL Defl -1.32" in E -K L/516
LL Defl -0.65" in E -K L/999

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Shear // Grain in A -M 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.91
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
M	LOCK	4.0x 5.0	-0.5	0.2	0.62
N	LOCK	8.0x12.0	-0.6	1.5	0.97
O	LOCK	4.0x 5.0	-0.5	0.2	0.90
B	LOCK	10.0x10.0	-0.5	Ctr	0.73
P	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S#	LOCK	6.0x 6.0	-0.2	-0.2	0.98
L	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
K	LOCK	4.0x 6.0	Ctr	Ctr	0.98
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 6.0	1.0	Ctr	0.99
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

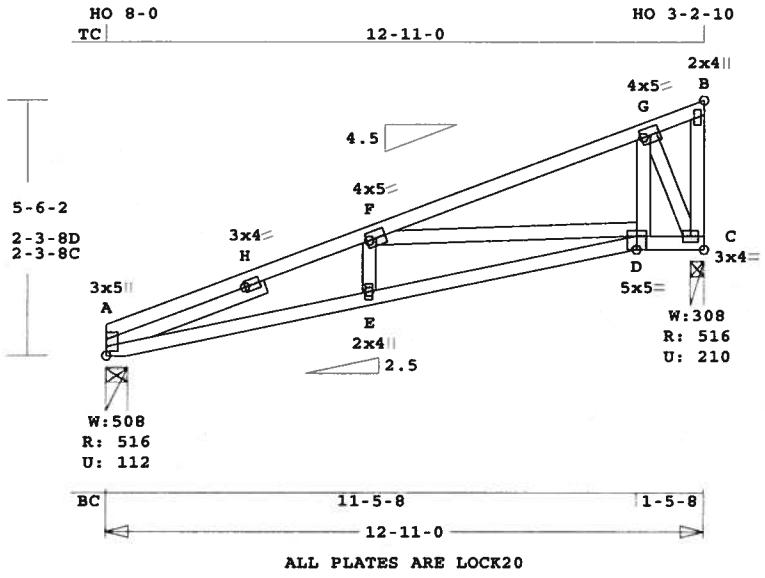
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10196 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R39A	1	SP	121100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.46	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11- 0
BC	Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	112	5- 8	1- 0
			Hz =	218
C	517	211	3- 8	1- 0
			Hz =	117

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -H	0.10	746 C	0.04 0.06
H -F	0.24	1242 C	0.07 0.17
F -G	0.33	285 C	0.00 0.33
G -B	0.33	54 C	0.00 0.33
--------Bottom Chords---------			
A -E	0.29	1192 T	0.19 0.10
E -D	0.36	1198 T	0.20 0.16
D -C	0.17	264 T	0.04 0.13
------------Webs------------			
E -F	0.05	202 T	
F -D	0.46	897 C	
D -G	0.08	304 T	
G -C	0.11	609 C	
C -B	0.03	112 T	
----------Sliders----------			
A -H	0.10	558 C	

LL Defl -0.04" in E -D L/999
TL Defl -0.09" in E -D L/999
Shear // Grain in F -G 0.23

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

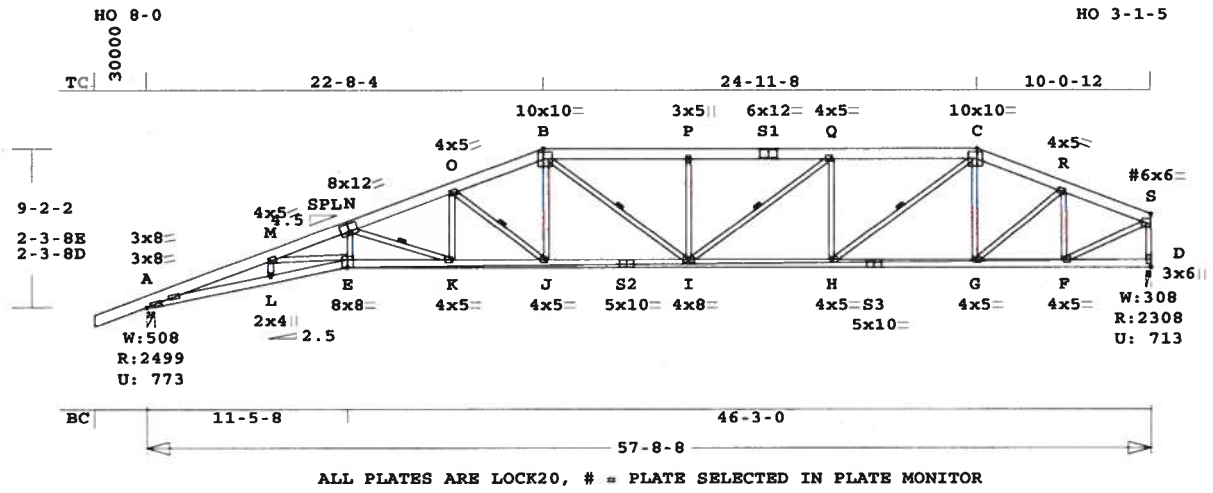
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.64
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.76
C	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1242 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R39*	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 580.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S2-S3	2x 6	SP-#2
EX	S3-D	2x 6	SP-#2
WB	0.84	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on B -I
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2500	773	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	141

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.34	9842	C	0.11	0.23
M -N	0.37	10199	C	0.12	0.25
N -O	0.20	6756	C	0.05	0.15
O -B	0.09	5315	C	0.06	0.03
B -P	0.13	5228	C	0.03	0.10
P -S1	0.11	5228	C	0.02	0.09
S1-Q	0.09	5228	C	0.03	0.06
Q -C	0.09	4547	C	0.02	0.07
C -R	0.06	3210	C	0.03	0.03
R -S	0.05	2484	C	0.02	0.03
Bottom Chords					
A -L	0.63	9290	T	0.42	0.21
L -E	0.63	9332	T	0.42	0.21
E -K	0.48	9495	T	0.43	0.05
K -J	0.32	6362	T	0.29	0.03
J -S2	0.26	4959	T	0.22	0.04
S2-I	0.68	4959	T	0.60	0.08
I -H	0.63	4547	T	0.55	0.08
H -S3	0.42	2999	T	0.36	0.06
S3-G	0.46	2999	T	0.36	0.10
G -F	0.38	2333	T	0.28	0.10
F -D	0.07	140	T	0.00	0.07
Webs					
L -M	0.03	150	T		
M -E	0.14	453	T		
E -N	0.66	2081	T		
N -K	0.69	3340	C		1 Br
K -O	0.39	1223	T		
O -J	0.39	1784	C		1 Br
J -B	0.41	1283	T		
B -I	0.10	335	T		1 Br
I -P	0.30	554	C		
I -Q	0.27	860	T		1 Br
H -Q	0.54	993	C		
H -C	0.61	1927	T		1 Br
G -C	0.22	404	C		
G -R	0.27	868	T		
F -R	0.33	1170	C		
F -S	0.84	2646	T		
D -S	0.39	2255	C		

TL Defl -1.32" in E -K L/518
LL Defl -0.65" in E -K L/999

Hz Disp	LL	DL	TL
Jt D	0.27"	0.27"	0.54"

Shear // Grain in A -M 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
M	LOCK	4.0x 5.0-0.5	0.2		0.62
N	LOCK	8.0x12.0-0.6	1.5		0.97
O	LOCK	4.0x 5.0-0.5	0.2		0.85
B	LOCK	10.0x10.0-0.5	Ctr		0.73
P	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	10.0x10.0	0.5	Ctr	0.73
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S#	LOCK	6.0x 6.0	0.2	0.3	0.85
L	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	Ctr-1.5		0.90
K	LOCK	4.0x 5.0-0.5	Ctr		0.95
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.70
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	4.0x 5.0	0.5	Ctr	0.99
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

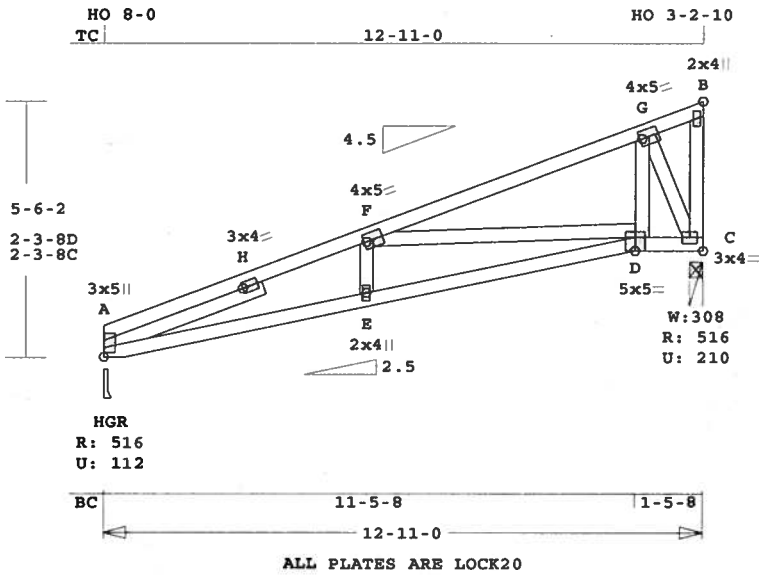
\# = Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10199 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R38A	1	SP	121100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 25-JAN-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.46	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11- 0
BC	Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	112	3- 8	1- 0
			Hz =	218
C	517	211	3- 8	1- 0
			Hz =	117

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -H	0.10	746 C	0.04 0.06
H -F	0.24	1242 C	0.07 0.17
F -G	0.33	285 C	0.00 0.33
G -B	0.33	54 C	0.00 0.33
--------Bottom Chords---------			
A -E	0.29	1192 T	0.19 0.10
E -D	0.36	1198 T	0.20 0.16
D -C	0.17	264 T	0.04 0.13
------------Webs------------			
E -F	0.05	202 T	
F -D	0.46	897 C	
D -G	0.08	304 T	
G -C	0.11	609 C	
C -B	0.03	112 T	
----------Sliders----------			
A -H	0.10	558 C	

LL Defl -0.04" in E -D L/999
TL Defl -0.09" in E -D L/999
Shear // Grain in F -G 0.23

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

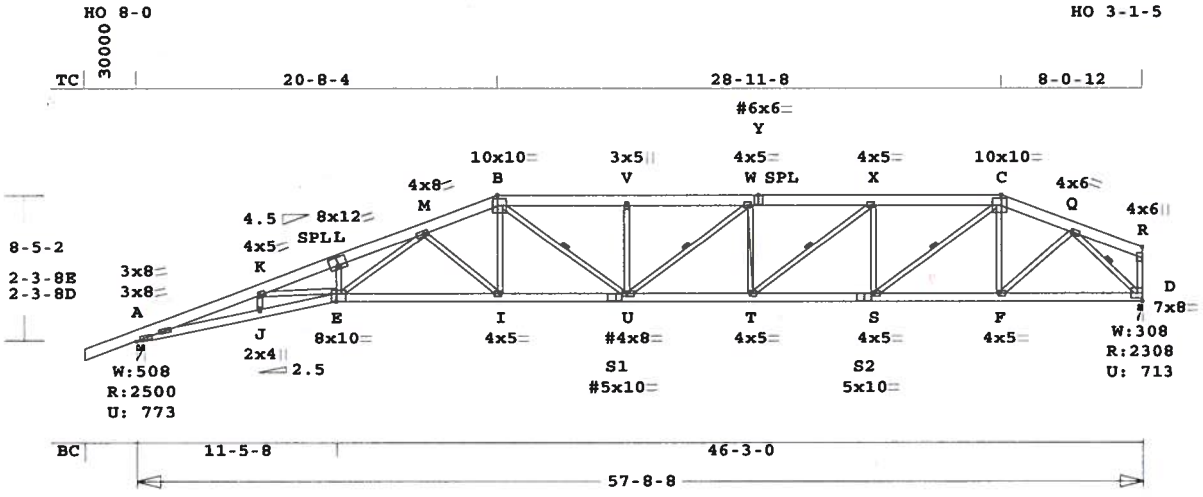
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.64
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.76
C	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1242 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R38	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 574.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.41	2x 8	SP-2400
BC	1.00	2x 6	SP-#2
EX A -E		2x 6	SP-2400
WB	0.82	2x 4	SP-#3
EX E -M		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8

WB 1 rows CLB on B -U
WB 1 rows CLB on U -W
WB 1 rows CLB on T -X
WB 1 rows CLB on S -C
WB 1 rows CLB on Q -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2500	773	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	135

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -K	0.37	9591 C	0.10	0.27
K -L	0.41	10421 C	0.12	0.29
L -M	0.26	10414 C	0.12	0.14
M -B	0.07	5820 C	0.03	0.04
B -V	0.12	5924 C	0.04	0.08
V -W	0.09	5924 C	0.04	0.05
W -Y	0.08	5637 C	0.06	0.02
Y -X	0.11	5637 C	0.04	0.07
X -C	0.08	4620 C	0.02	0.06
C -Q	0.06	3037 C	0.03	0.03
Q -R	0.02	57 T	0.00	0.02
Bottom Chords				
A -J	0.71	9039 T	0.41	0.30
J -E	0.71	9111 T	0.41	0.30
E -I	1.00	6710 T	0.81	0.19
I -S1	0.80	5430 T	0.66	0.14
S1-U	0.69	5430 T	0.66	0.03
U -T	0.75	5636 T	0.68	0.07
T -S2	0.64	4619 T	0.56	0.08
S2-S	0.57	4619 T	0.56	0.01
S -F	0.46	2840 T	0.34	0.12
F -D	0.36	2017 T	0.24	0.12
Webs				
J -K	0.05	256 C		
K -E	0.29	910 T		
E -L	0.05	269 C		
E -M	0.75	3890 T		
M -I	0.82	1682 C		
I -B	0.42	1331 T		
B -U	0.19	614 T		1 Br
U -V	0.20	468 C		
U -W	0.11	366 T		1 Br
T -W	0.28	654 C		
T -X	0.41	1295 T		1 Br
S -X	0.51	1193 C		
S -C	0.70	2212 T		1 Br
F -C	0.25	590 C		
F -Q	0.36	1124 T		
Q -D	0.55	2934 C		1 Br
D -R	0.03	148 T		

TL Defl -1.43" in E -I L/477
LL Defl -0.69" in E -I L/995

Hz Disp	LL	DL	TL
Jt D	0.28"	0.28"	0.56"

Shear // Grain in A -K 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.88
A	LOCK	3.0x 8.0	Ctr	Ctr	0.76
K	LOCK	4.0x 5.0-0.5	0.2		0.62
L	LOCK	8.0x12.0-0.6	1.5		0.97
M	LOCK	4.0x 8.0-1.2-0.1			0.89
B	LOCK	10.0x10.0-0.5	Ctr		0.73
V	LOCK	3.0x 5.0	Ctr	Ctr	0.39
W	LOCK	4.0x 5.0	Ctr	0.5	0.60
Y#	LOCK	6.0x 6.0	Ctr	Ctr	0.91
X	LOCK	4.0x 5.0	Ctr	0.5	0.63
C	LOCK	10.0x10.0	0.5	Ctr	0.74
Q	LOCK	4.0x 6.0	0.5	0.2	0.79
R	LOCK	4.0x 6.0-0.2	Ctr		1.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x10.0	Ctr-1.5		0.92
I	LOCK	4.0x 5.0	Ctr	Ctr	0.62
S1#	LOCK	5.0x10.0-0.9	0.1		0.82
U#	LOCK	4.0x 8.0	0.9	0.1	0.61
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
S	LOCK	4.0x 5.0	Ctr	Ctr	0.81
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	7.0x 8.0-0.5	Ctr		0.85

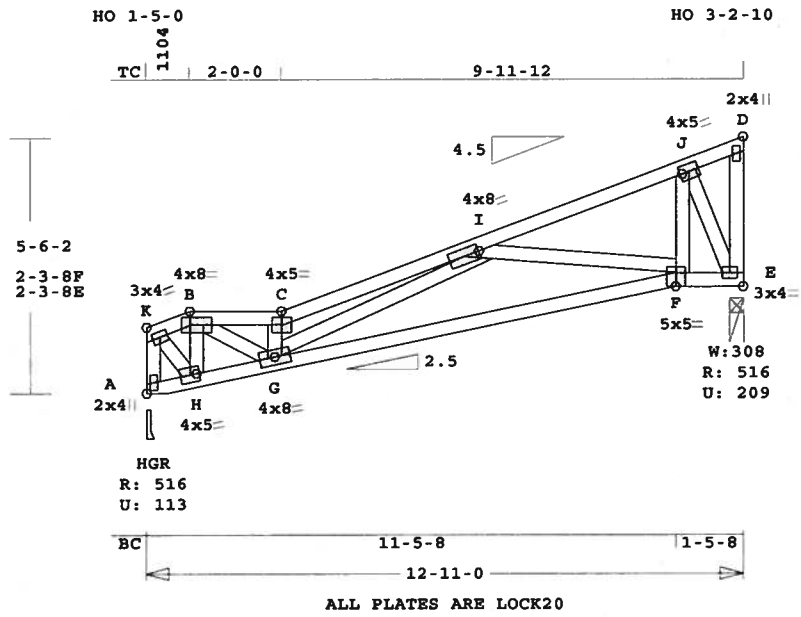
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10421 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R37A*	1	SP	121100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 10-FEB-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.41	2x 4	SP-#2
WB	0.31	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
K -B	0.06	324	C	0.00	0.06
B -C	0.13	1196	C	0.01	0.12
C -I	0.22	1263	C	0.01	0.21
I -J	0.21	270	C	0.00	0.21
J -D	0.14	19	C	0.00	0.14
--------Bottom Chords---------					
A -H	0.02	307	C	0.00	0.02
H -G	0.36	453	C	0.03	0.33
G -F	0.41	979	T	0.08	0.33
F -E	0.14	236	T	0.00	0.14
------------Webs------------					
A -K	0.08	508	C	WindLd	
K -H	0.13	424	T		
H -B	0.06	390	C		
B -G	0.31	982	T		
G -C	0.09	585	C		
G -I	0.12	480	T		
I -F	0.26	715	C		
F -J	0.12	440	T		
J -E	0.09	545	C		
E -D	0.00	24	C		

LL Defl -0.14" in G -F L/999
TL Defl -0.29" in G -F L/517
Shear // Grain in C -I 0.19

Hz Disp	LL	DL	TL
Jt E	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

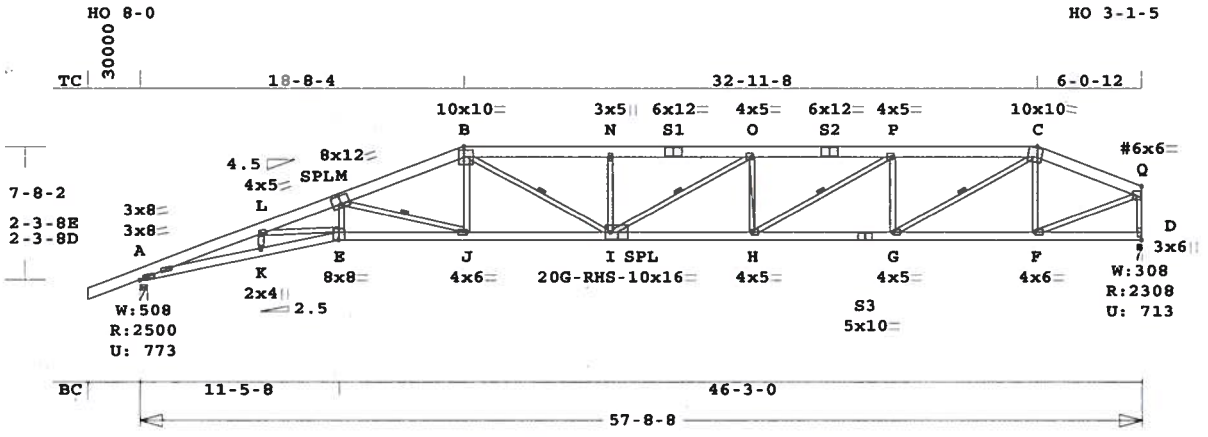
Jt	Type	Plt Size	X	Y	JSI
K	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 8.0	Ctr	Ctr	0.89
J	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.61
G	LOCK	4.0x 8.0	Ctr	Ctr	0.81
F	LOCK	5.0x 5.0	Ctr	Ctr-1.0	0.81
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1263 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R37*	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 553.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX I -S3		2x 6	SP-#2
EX S3-D		2x 6	SP-#2
WB	0.92	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	57- 8- 8
BC Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on M -J
WB 1 rows CLB on B -I
WB 1 rows CLB on I -O
WB 1 rows CLB on H -P
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2500	773	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	130

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.34	9850	C	0.11	0.23
L -M	0.37	10187	C	0.12	0.25
M -B	0.23	6361	C	0.05	0.18
B -N	0.14	6823	C	0.05	0.09
N -S1	0.13	6823	C	0.06	0.07
S1-O	0.13	6823	C	0.06	0.07
O -S2	0.15	6526	C	0.05	0.10
S2-P	0.15	6526	C	0.05	0.10
P -C	0.12	5086	C	0.02	0.10
C -Q	0.10	2724	C	0.00	0.10
Bottom Chords					
A -K	0.63	9296	T	0.42	0.21
K -E	0.63	9337	T	0.42	0.21
E -J	0.47	9482	T	0.43	0.04
J -I	0.31	5959	T	0.27	0.04
I -H	0.88	6526	T	0.79	0.09
H -S3	0.72	5085	T	0.61	0.11
S3-G	0.67	5085	T	0.61	0.06
G -F	0.44	2558	T	0.31	0.13
F -D	0.18	129	T	0.00	0.18
Webs					
K -L	0.03	142	T		
L -E	0.13	433	T		
E -M	0.66	2069	T		
M -J	0.92	3648	C		1 Br
J -B	0.38	1206	T		
B -I	0.31	990	T		1 Br
I -N	0.16	503	C		
I -O	0.11	345	T		1 Br
H -O	0.22	685	C		
H -P	0.53	1675	T		1 Br
G -P	0.42	1277	C		
G -C	0.92	2898	T		1 Br
F -C	0.30	911	C		
F -Q	0.88	2762	T		
D -Q	0.39	2269	C		

TL Defl -1.50" in J -I L/457
LL Defl -0.74" in J -I L/924

Hz Disp	LL	DL	TL
Jt D	0.29"	0.30"	0.59"

Shear // Grain in A -L 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0	-0.5	0.2	0.63
M	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	-0.5	Ctr	0.73
N	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	4.0x 5.0	Ctr	0.5	0.79
C	LOCK	10.0x10.0	-1.6	-5.8	0.96
Q#	LOCK	6.0x 6.0	0.1	0.1	0.89
K	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
J	LOCK	4.0x 6.0	-1.0	Ctr	0.95
I	RHS	10.0x16.0	-1.0	Ctr	0.87
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
G	LOCK	4.0x 5.0	1.0	Ctr	1.00
F	LOCK	4.0x 6.0	1.0	Ctr	0.99
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

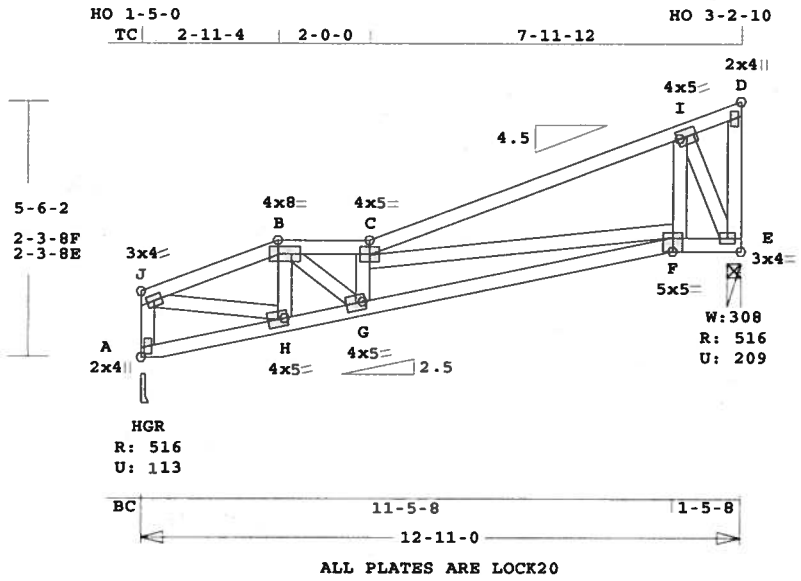
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10187 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R36A*	1	SP	121100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 92.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 10-FEB-06

	CSI	-Size-	----Lumber----
TC	0.55	2x 4	SP-#2
BC	0.42	2x 4	SP-#2
WB	0.64	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
J -B	0.17	723 C	0.05 0.12
B -C	0.32	1226 C	0.01 0.31
C -I	0.55	336 C	0.00 0.55
I -D	0.55	91 C	0.00 0.55
--------Bottom Chords---------			
A -H	0.05	330 C	0.00 0.05
H -G	0.19	696 T	0.11 0.08
G -F	0.42	1290 T	0.21 0.21
F -E	0.14	324 T	0.05 0.09
------------Webs------------			
A -J	0.09	490 C	WindLd
J -H	0.22	690 T	
H -B	0.03	196 C	
B -G	0.21	661 T	
G -C	0.08	483 C	
C -F	0.64	928 C	
F -I	0.08	310 T	
I -E	0.15	748 C	
E -D	0.06	217 T	

LL Defl -0.06" in G -F L/999
TL Defl -0.15" in G -F L/969
Shear // Grain in I -D 0.36

Hz Disp	LL	DL	TL
Jt E	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

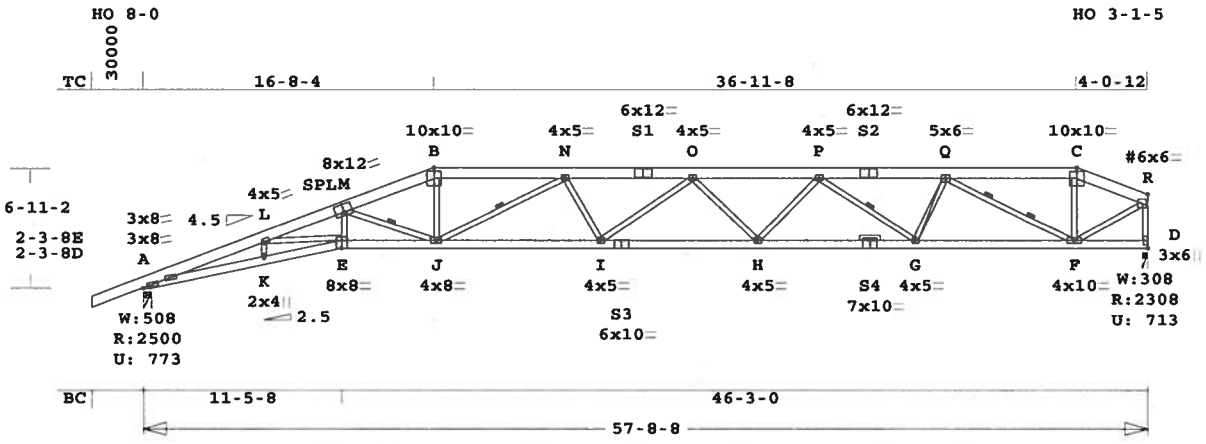
Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.74
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr-1.0		0.69
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1226 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R36*	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 542.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.38	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S4-D	2x 6	SP-#2
WB	0.82	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on M -J
WB 1 rows CLB on J -N
WB 1 rows CLB on P -G
WB 2 rows CLB on Q -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2500	773	5- 8	2- 1
			Hz =	211
D	2308	714	3- 8	2-12
			Hz =	124

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.34	9837 C	0.11	0.23	
L -M	0.38	10167 C	0.12	0.26	
M -B	0.21	7054 C	0.05	0.16	
B -N	0.14	6657 C	0.05	0.09	
N -S1	0.15	7933 C	0.07	0.08	
S1-O	0.14	7933 C	0.07	0.07	
O -P	0.15	7577 C	0.07	0.08	
P -S2	0.10	5668 C	0.04	0.06	
S2-Q	0.11	5668 C	0.04	0.07	
Q -C	0.08	2150 C	0.00	0.08	
C -R	0.04	2290 C	0.02	0.02	
Bottom Chords					
A -K	0.63	9285 T	0.42	0.21	
K -E	0.63	9327 T	0.42	0.21	
E -J	0.48	9461 T	0.43	0.05	
J -I	0.42	7834 T	0.35	0.07	
I -S3	0.39	7965 T	0.36	0.03	
S3-H	0.42	7965 T	0.36	0.06	
H -S4	0.38	7061 T	0.32	0.06	
S4-G	0.93	7062 T	0.85	0.08	
G -F	0.72	5167 T	0.62	0.10	
F -D	0.16	124 T	0.00	0.16	
Webs					
K -L	0.03	151 T			
L -E	0.13	421 T			
E -M	0.66	2058 T			
M -J	0.58	3035 C			1 Br
J -B	0.62	1958 T			
J -N	0.40	1343 C			1 Br
N -I	0.09	357 T			
I -O	0.04	180 T			
O -H	0.25	569 C			
H -P	0.25	791 T			
P -G	0.40	1733 C			1 Br
G -Q	0.42	1309 T			
Q -F	0.67	3444 C			2 Br
F -C	0.14	477 T			
F -R	0.82	2558 T			
D -R	0.40	2293 C			

TL Defl -1.66" in J -I L/411
LL Defl -0.82" in I -S3 L/833

Hz Disp	LL	DL	TL
Jt D	0.32"	0.32"	0.65"

Shear // Grain in A -L 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
L	LOCK	4.0x 5.0	-0.5	0.2	0.63
M	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.73
N	LOCK	4.0x 5.0	Ctr	0.5	0.61
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
O	LOCK	4.0x 5.0	Ctr	0.5	0.57
P	LOCK	4.0x 5.0	Ctr	0.5	0.73
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	5.0x 6.0	Ctr	Ctr	0.89
C	LOCK	10.0x10.0	Ctr	Ctr	0.73
R#	LOCK	6.0x 6.0	-0.6	0.2	0.95
K	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
J	LOCK	4.0x 8.0	-0.5	Ctr	0.90
I	LOCK	4.0x 5.0	Ctr	Ctr	0.55
S3	LOCK	6.0x10.0	Ctr	0.2	0.99
H	LOCK	4.0x 5.0	Ctr	Ctr	0.48
S4	LOCK	7.0x10.0	Ctr	0.8	0.86
G	LOCK	4.0x 5.0	Ctr	Ctr	0.58
F	LOCK	4.0x10.0	Ctr	Ctr	0.86
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

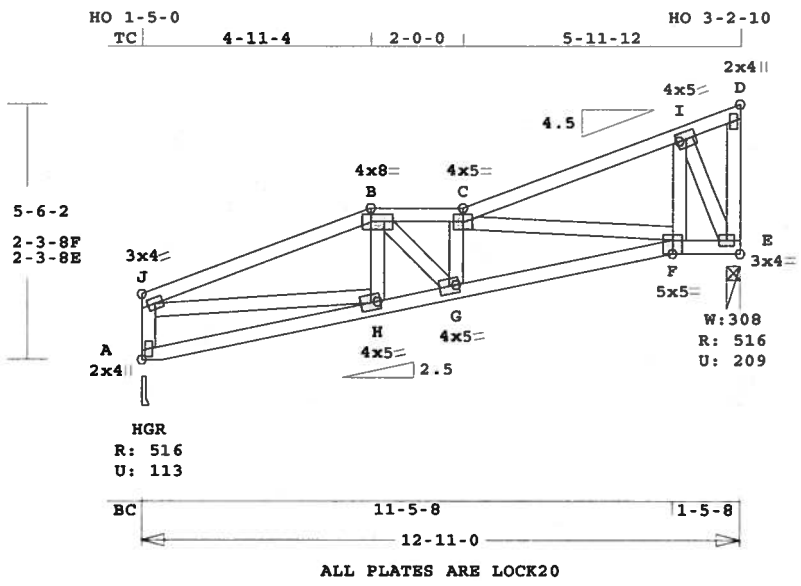
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10167 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R35A*	1	SP	121100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 10-FEB-06

	CSI	-Size-	----Lumber----
TC	0.40	2x 4	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.26	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
J -B	0.40	847 C	0.08 0.32
B -C	0.22	989 C	0.09 0.13
C -I	0.31	268 C	0.02 0.29
I -D	0.29	43 C	0.00 0.29
--------Bottom Chords---------			
A -H	0.13	393 C	0.00 0.13
H -G	0.20	883 C	0.13 0.07
G -F	0.31	1033 T	0.17 0.14
F -E	0.14	274 C	0.04 0.10
------------Webs------------			
A -J	0.11	472 C	WindLd
J -H	0.25	797 T	
H -B	0.03	122 T	
B -G	0.08	257 T	
G -C	0.05	224 C	
C -F	0.26	744 C	
F -I	0.09	307 T	
I -E	0.15	632 T	
E -D	0.02	108 C	

LL Defl -0.03" in H -G L/999
TL Defl -0.07" in H -G L/999
Shear // Grain in C -I 0.25

Hz Disp	LL	DL	TL
Jt E	0.02"	0.00"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

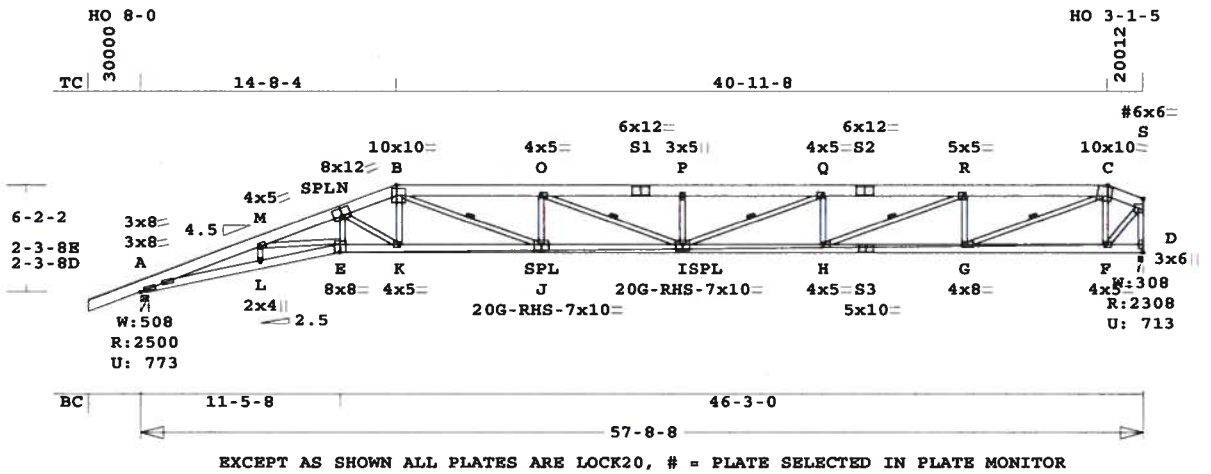
Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.74
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr	-1.0	0.81
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 999 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R35*	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 540.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 8	SP-2400
BC	0.63	2x 6	SP-2400
EX	S3-D	2x 6	SP-#2
WB	0.68	2x 4	SP-#3
EX	H -R	2x 4	SP-#2
EX	G -C	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	57- 8- 8
BC Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on B -J
WB 1 rows CLB on O -I
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -R
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2501	773	5- 8	2- 1
			Hz =	210
D	2308	713	3- 8	2-12
			Hz =	119

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.34	9819	C	0.11	0.23
M -N	0.37	10200	C	0.12	0.25
N -B	0.21	7912	C	0.07	0.14
B -O	0.22	9415	C	0.11	0.11
O -S1	0.25	9811	C	0.12	0.13
S1-P	0.21	9811	C	0.12	0.09
P -Q	0.22	9811	C	0.12	0.10
Q -S2	0.18	8615	C	0.09	0.09
S2-R	0.24	8615	C	0.09	0.15
R -C	0.20	5666	C	0.03	0.17
C -S	0.13	1459	C	0.00	0.13
Bottom Chords					
A -L	0.63	9265	T	0.42	0.21
L -E	0.63	9308	T	0.42	0.21
E -K	0.49	9494	T	0.43	0.06
K -J	0.39	7416	T	0.33	0.06
J -I	0.51	9415	T	0.43	0.08
I -H	0.45	8615	T	0.39	0.06
H -S3	0.31	5666	T	0.25	0.06
S3-G	0.85	5666	T	0.68	0.17
G -F	0.44	1326	T	0.16	0.28
F -D	0.28	118	T	0.00	0.28
Webs					
L -M	0.03	157	T		
M -E	0.15	473	T		
E -N	0.65	2053	T		
N -K	0.57	2431	C		
K -B	0.46	1464	T		
B -J	0.68	2137	T		1 Br
J -O	0.12	610	C		
O -I	0.13	427	T		1 Br
I -P	0.09	493	C		
I -Q	0.41	1293	T		1 Br
H -Q	0.20	1032	C		
H -R	0.58	3189	T		1 Br
G -R	0.31	1544	C		
G -C	0.85	4639	T		1 Br
F -C	0.26	1309	C		
F -S	0.65	2051	T		
D -S	0.42	2435	C		

TL Defl -2.20" in J -I L/310
LL Defl -1.09" in J -I L/627

Hz Disp	LL	DL	TL
Jt D	0.34"	0.34"	0.68"

Shear // Grain in A -M 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.90
A	LOCK	3.0x 8.0	Ctr	Ctr	0.80
M	LOCK	4.0x 5.0	-0.5	0.2	0.63
N	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	-0.5	Ctr	0.90
O	LOCK	4.0x 5.0	Ctr	0.5	0.66
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	3.0x 5.0	Ctr	Ctr	0.39
Q	LOCK	4.0x 5.0	Ctr	0.5	0.66
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
R	LOCK	5.0x 5.0	Ctr	Ctr	0.96
C	LOCK	10.0x10.0	-2.1	-5.8	0.98
S#	LOCK	6.0x 6.0	0.4	0.1	0.87
L	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
K	LOCK	4.0x 5.0	Ctr	Ctr	0.78
J	RHS	7.0x10.0	Ctr	-0.8	0.97
I	RHS	7.0x10.0	Ctr	-0.8	0.97
H	LOCK	4.0x 5.0	0.5	Ctr	0.91
S3	LOCK	5.0x10.0	Ctr	Ctr	0.85
G	LOCK	4.0x 8.0	0.5	Ctr	0.92
F	LOCK	4.0x 5.0	Ctr	Ctr	0.80
D	LOCK	3.0x 6.0	Ctr	Ctr	0.85

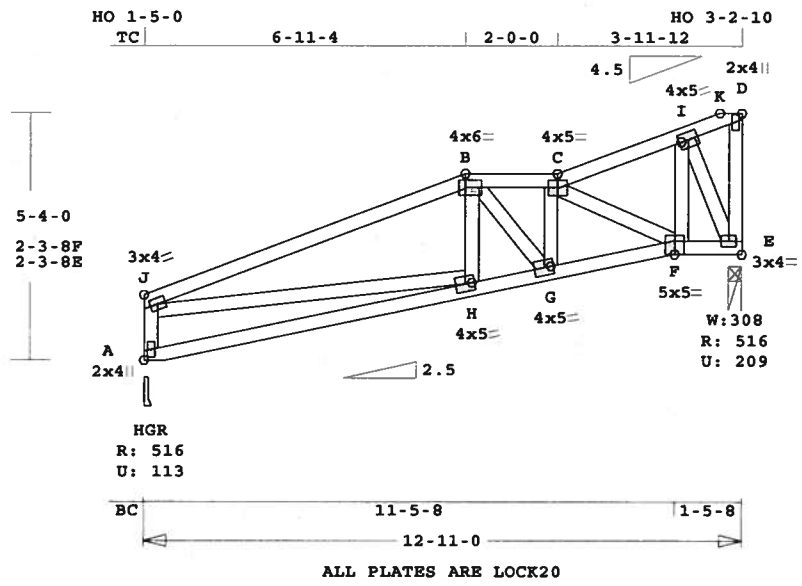
\# = Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 10200 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R34A	1	SP	121100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 95.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 10-FEB-06

	CSI	-Size-	----Lumber----
TC	0.56	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.25	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11- 0
BC Cont.	0- 0- 0	12-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	517	113	3- 8	1- 0
			Hz =	214
E	517	210	3- 8	1- 0
			Hz =	124

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
J -B	0.56	818 C	0.01 0.55
B -C	0.28	691 C	0.06 0.22
C -I	0.11	251 C	0.00 0.11
I -D	0.10	15 C	0.00 0.10
--------Bottom Chords---------			
A -H	0.28	341 C	0.00 0.28
H -G	0.36	770 T	0.08 0.28
G -F	0.18	709 T	0.11 0.07
F -E	0.07	231 T	0.03 0.04
------------Webs------------			
A -J	0.09	457 C	WindLd
J -H	0.25	772 T	
H -B	0.05	212 T	
B -G	0.03	136 T	
G -C	0.01	72 C	
C -F	0.10	478 C	
F -I	0.12	378 T	
I -E	0.12	533 C	
E -D	0.00	11 T	

LL Defl -0.07" in A -H L/999
TL Defl -0.15" in A -H L/966
Shear // Grain in J -B 0.27

Hz Disp	LL	DL	TL
Jt E	0.01"	0.00"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

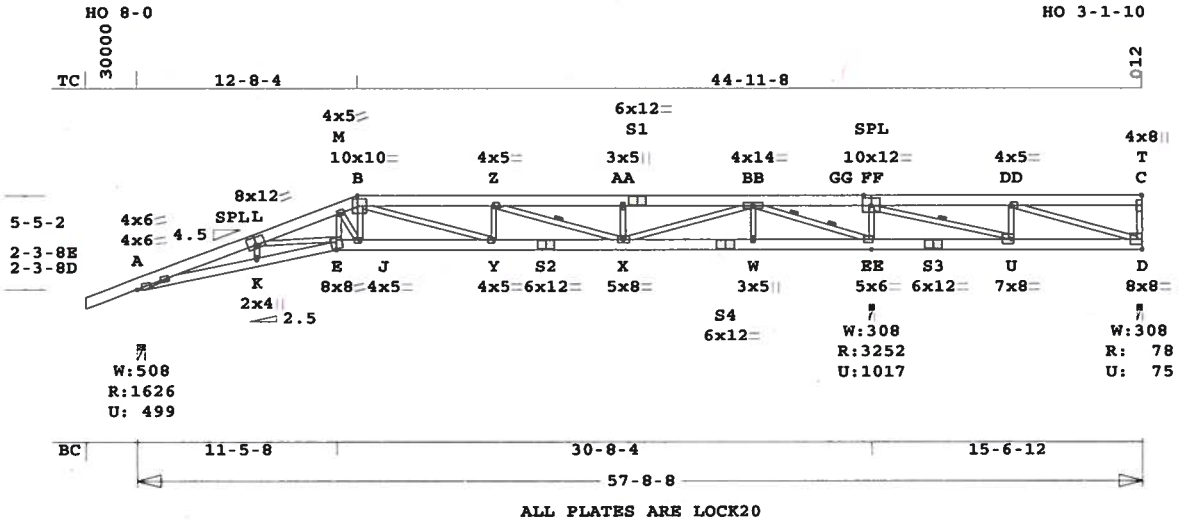
Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x 4.0	Ctr	Ctr	0.58
B	LOCK	4.0x 6.0	-0.5	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H	LOCK	4.0x 5.0	Ctr	Ctr	0.74
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	5.0x 5.0	Ctr	-1.0	0.46
E	LOCK	3.0x 4.0	Ctr	Ctr	0.58

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 818 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R34	1	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 568.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 8	SP-2400
BC	0.22	2x 8	SP-2400
EX	A -E	2x 6	SP-#2
WB	0.88	2x 4	SP-#3
EX	X -BB	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	57- 8- 8
BC	Cont.	0- 0- 0	57- 8- 8

WB 1 rows CLB on Z -X
WB 2 rows CLB on BB-EE
WB 1 rows CLB on FF-U
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1627	500	5- 8	1-15
			Hz =	212
EE	3253	1018	3- 8	2- 7
D	78	75	3- 8	1- 8
			Hz =	115

Membr	CSI	P Lbs	Axl	CSI	Bnd
	Top Chords				
A -L	0.18	5462	C	0.03	0.15
L -M	0.20	5428	C	0.03	0.17
M -B	0.09	4801	C	0.02	0.07
B -Z	0.11	4913	C	0.02	0.09
Z -AA	0.08	3493	C	0.01	0.07
AA-S1	0.05	3493	C	0.03	0.02
S1-BB	0.08	3493	C	0.01	0.07
BB-FF	0.33	3677	T	0.12	0.21
FF-DD	0.24	1054	T	0.03	0.21
DD-T	0.08	0	T	0.00	0.08
	Bottom Chords				
A -K	0.92	5156	T	0.62	0.30
K -E	0.93	5194	T	0.63	0.30
E -J	0.20	5058	T	0.17	0.03
J -Y	0.20	4581	T	0.15	0.05
Y -S2	0.22	4913	T	0.17	0.05
S2-X	0.22	4913	T	0.17	0.05
X -S4	0.05	248	T	0.00	0.05
S4-W	0.03	248	T	0.00	0.03
W -EE	0.18	248	T	0.00	0.18
EE-S3	0.19	3677	C	0.01	0.18
S3-U	0.06	3677	C	0.03	0.03
U -D	0.05	1054	C	0.00	0.05
	Webs				
K -L	0.03	158	T		
L -E	0.03	112	T		
E -M	0.35	1099	T		
M -J	0.13	824	C		
J -B	0.29	935	T		
B -Y	0.15	346	T		
Y -Z	0.05	222	T		
Z -X	0.39	1499	C	1 Br	
X -AA	0.07	454	C		
X -BB	0.99	3427	T		
W -BB	0.05	197	T		
BB-EE	0.75	4186	C	2 Br	
EE-FF	0.24	1453	C		
FF-U	0.88	2749	T	1 Br	
U -DD	0.13	769	C		
DD-D	0.41	1115	T		
D -T	0.03	205	C		

TL Defl -0.81" in J -Y L/617
LL Defl -0.40" in J -Y L/999

Hz Disp	LL	DL	TL
Jt EE	0.13"	0.13"	0.26"

Shear // Grain in M -B 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

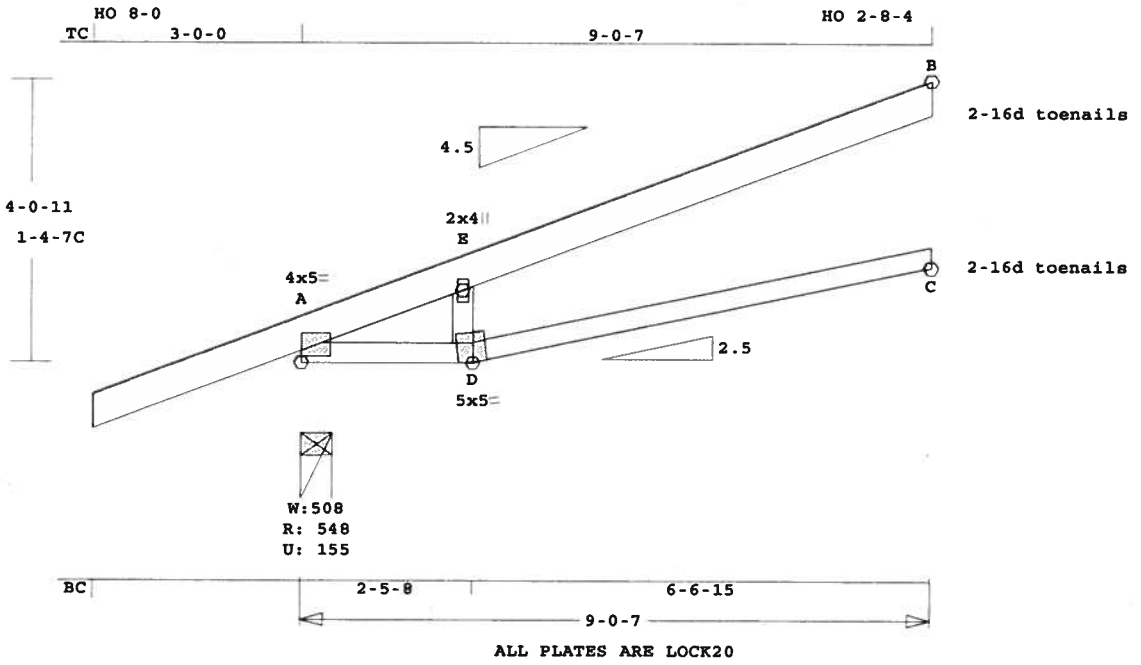
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.75
A	LOCK	4.0x 6.0	Ctr	Ctr	0.34
L	LOCK	8.0x12.0	-0.6	1.5	0.97
M	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	10.0x10.0	-0.5	Ctr	0.90
Z	LOCK	4.0x 5.0	Ctr	0.5	0.71
AA	LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
BB	LOCK	4.0x14.0	Ctr	0.5	0.87
FF	LOCK	10.0x12.0	Ctr	0.5	0.99
DD	LOCK	4.0x 5.0	Ctr	0.5	0.71
T	LOCK	4.0x 8.0	-0.2	Ctr	0.85
K	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	8.0x 8.0	0.2	-1.0	0.72
J	LOCK	4.0x 5.0	Ctr	-0.5	0.60
Y	LOCK	4.0x 5.0	Ctr	-0.5	0.72
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
X	LOCK	5.0x 8.0	0.5	Ctr	0.96
S4	LOCK	6.0x12.0	Ctr	Ctr	0.91
W	LOCK	3.0x 5.0	Ctr	Ctr	0.39
EE	LOCK	5.0x 6.0	-0.5	Ctr	0.99
S3	LOCK	6.0x12.0	Ctr	Ctr	0.91
U	LOCK	7.0x 8.0	-0.5	Ctr	0.90
D	LOCK	8.0x 8.0	-0.5	Ctr	0.88

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5462 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R33*	1	SP	90007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.50	2x 6	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	549	156	5- 8	1- 8
			Hz =	153
C	133	0	3- 8	1- 8
B	281	171	3- 8	1- 8
			Hz =	82

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.39	186 C	0.00 0.39
E -B	0.50	134 C	0.00 0.50
--------Bottom Chords---------			
A -D	0.28	23 C	0.00 0.28
D -C	0.31	6 C	0.00 0.31
------------Webs------------			
D -E	0.05	187 T	

TL Defl	-0.26" in D -C	L/389	
LL Defl	-0.10" in D -C	L/974	
Hz Disp	LL	DL	TL
Jt C	0.02"	0.02"	0.04"
Shear // Grain	in A -E	0.20	

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	-0.3	2.7	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

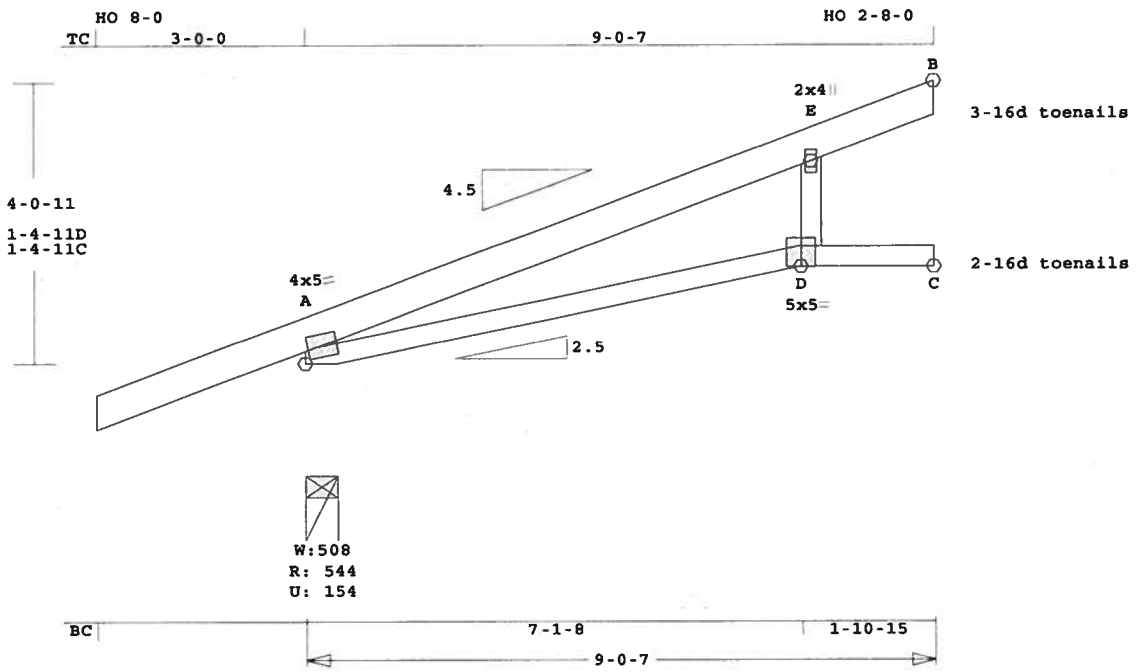
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 186 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R32*	1	SP	90007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 56.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.52	2x 6	SP-#2
BC	0.29	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	9- 0- 7
BC Cont.		0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	544	154	5- 8	1- 8
			Hz = 152	
C	53	37	3- 8	1- 8
B	318	114	3- 8	1- 8
			Hz = 82	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.52	172 C	0.00 0.52
E -B	0.33	102 C	0.00 0.33
--------Bottom Chords---------			
A -D	0.29	15 T	0.00 0.29
D -C	0.20	0 T	0.00 0.20
------------Webs------------			
D -E	0.05	192 T	

TL Defl	-0.24" in A -D		L/408
LL Defl	-0.11" in A -D		L/943
Hz Disp	LL	DL	TL
Jt B	0.02"	0.02"	0.04"
Shear // Grain	in E -B		0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr-1.0		0.46

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

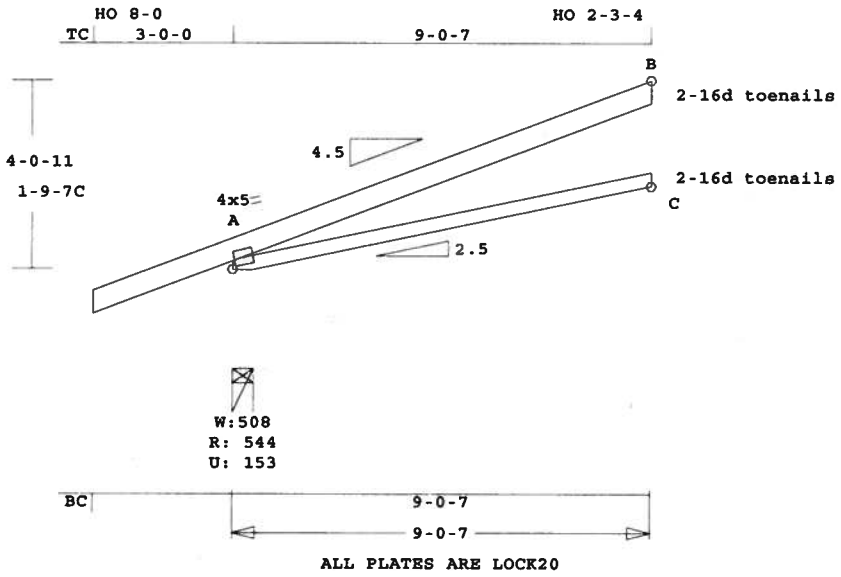
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 172 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R31*	9	HFSC	90007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 54.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 6	SP-#2
BC	0.42	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	545	153	5- 8	1- 8
			Hz = 153	
C	153	0	3- 8	1- 8
B	283	186	3- 8	1- 8
			Hz = 82	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.51	145 C	0.00 0.51
--------Bottom Chords---------			
A -C	0.42	5 C	0.00 0.42

TL Defl -0.39" in A -C L/252
LL Defl -0.18" in A -C L/560

Hz Disp	LL	DL	TL
Jt B	0.02"	0.02"	0.04"

Shear // Grain in A -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

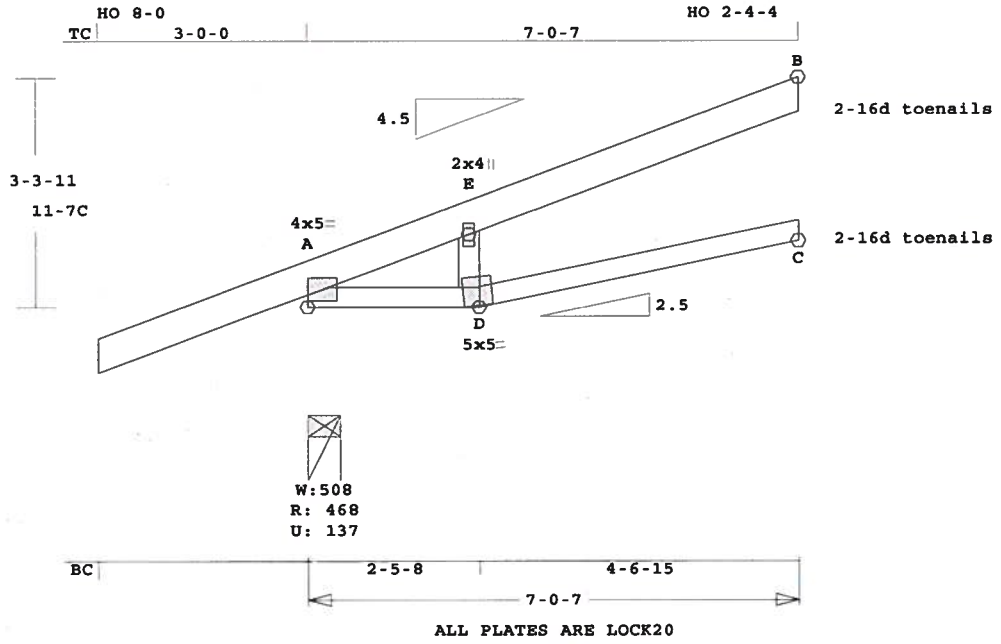
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph

Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 145 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R30*	1	SP	70007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 6	SP-#2
BC	0.18	2x 4	SP-#2
WB	0.03	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	7- 0- 7
BC Cont.		0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	469	137	5- 8	1- 8
			Hz =	119
C	99	0	3- 8	1- 8
B	220	131	3- 8	1- 8
			Hz =	64

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.27	156 C	0.00 0.27
E -B	0.30	118 C	0.00 0.30
--------Bottom Chords---------			
A -D	0.18	15 C	0.00 0.18
D -C	0.18	5 C	0.00 0.18
------------Webs------------			
D -E	0.03	124 T	

TL Defl	-0.08" in D -C		L/899
LL Defl	-0.04" in D -C		L/999
Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"
Shear // Grain	in E -B		0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0-0.3	2.7		0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

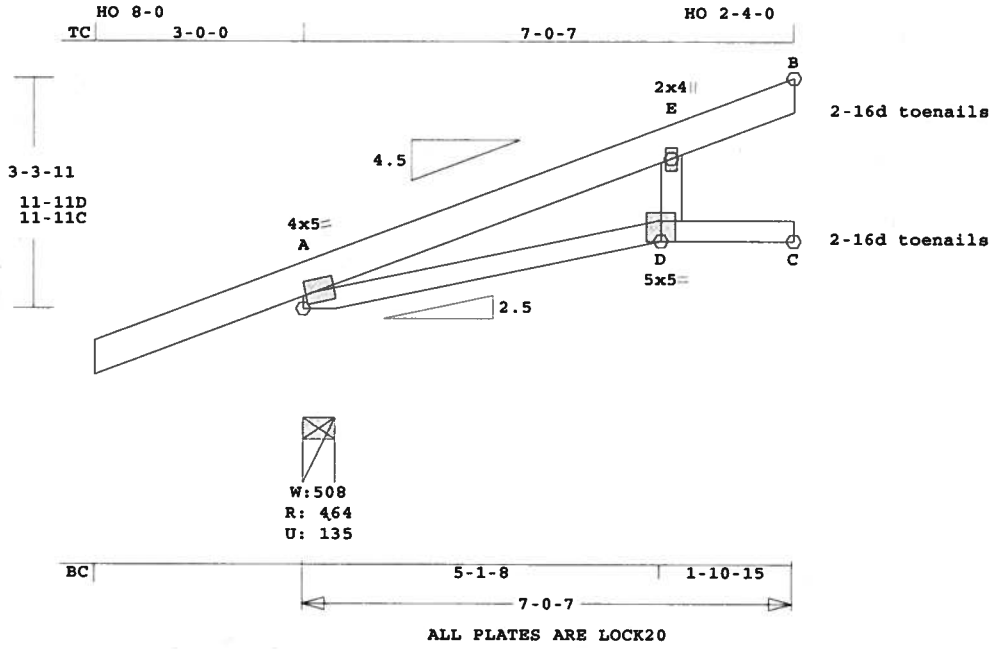
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 156 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R29	1	SP	70007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 6	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.03	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 7
BC	Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	464	135	5- 8	1- 8
			Hz =	119
C	48	17	3- 8	1- 8
B	243	104	3- 8	1- 8
			Hz =	64

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.31	155 C	0.00 0.31
E -B	0.23	101 C	0.00 0.23
--------Bottom Chords---------			
A -D	0.17	13 T	0.00 0.17
D -C	0.14	0 T	0.00 0.14
------------Webs------------			
D -E	0.03	132 T	

TL Defl -0.08" in A -D L/899
LL Defl -0.04" in A -D L/999

Hz Disp	LL	DL	TL
Jt B	0.02"	0.00"	0.02"

Shear // Grain in E -B 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.46

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

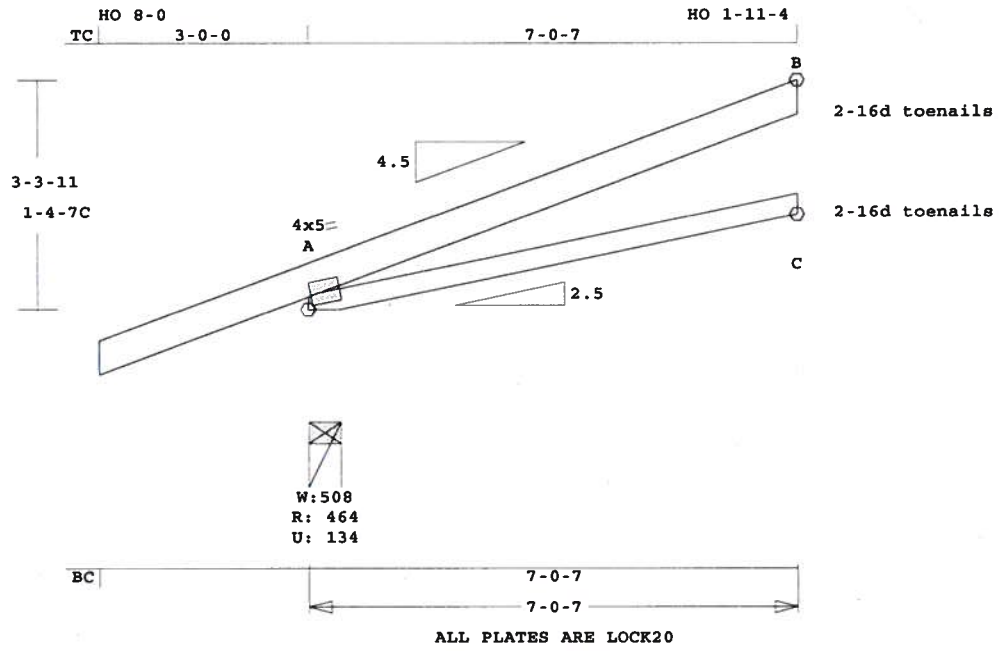
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 155 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R28*	7	VJACK	70007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 6	SP-#2
BC	0.25	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 7
BC Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	465	134	5- 8	1- 8
			Hz = 119	
C	120	0	3- 8	1- 8
B	222	148	3- 8	1- 8
			Hz = 64	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.30	130 C	0.00 0.30
--------Bottom Chords---------			
A -C	0.25	4 C	0.00 0.25

TL Defl -0.14" in A -C L/531
LL Defl -0.06" in A -C L/999

Hz Disp	LL	DL	TL
Jt B	0.02"	0.00"	0.02"

Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

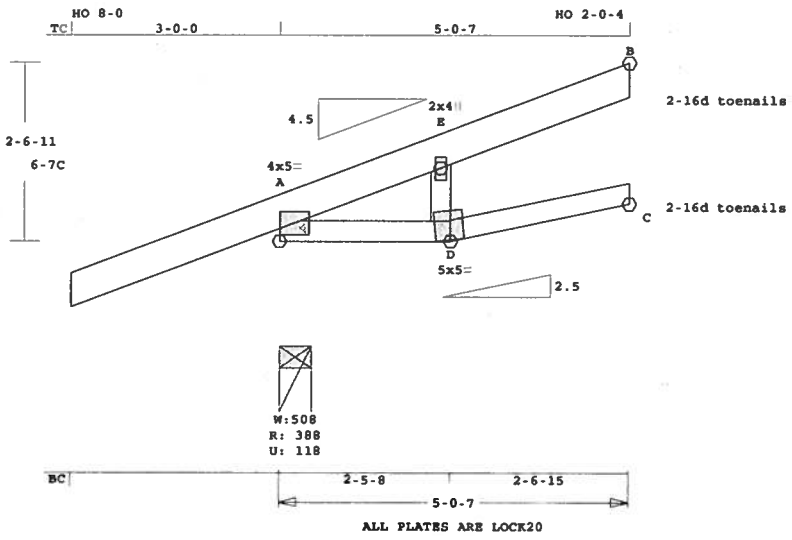
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph

Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 130 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R27	1	SP	50007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 36.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.14	2x 6	SP-#2
BC	0.11	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	389	118	5- 8	1- 8
			Hz = 85	
C	65	0	3- 8	1- 8
B	159	91	3- 8	1- 8
			Hz = 46	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.14	122 C	0.00 0.14
E -B	0.14	95 C	0.00 0.14
--------Bottom Chords---------			
A -D	0.11	11 C	0.00 0.11
D -C	0.11	6 C	0.00 0.11
------------Webs------------			
D -E	0.02	88 T	

TL Defl	-0.02" in D -C	L/999	
LL Defl	-0.01" in D -C	L/999	
Hz Disp	LL	DL	TL
Jt B	0.01"	0.00"	0.01"
Shear // Grain	in E -B	0.13	

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	4.0x 5.0	0.7	0.3	0.65
E LOCK	2.0x 4.0	Ctr	Ctr	0.38
D LOCK	5.0x 5.0	-0.3	2.7	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

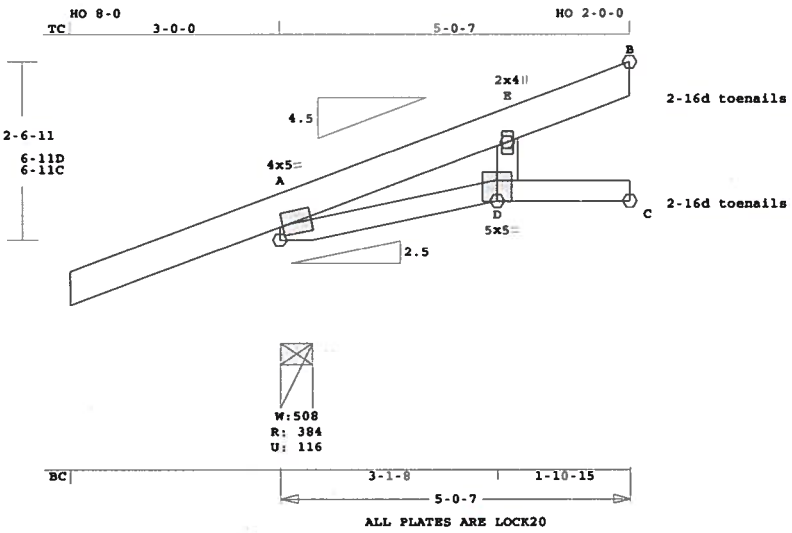
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 122 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R26*	1	SP	50007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 35.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 6	SP-#2
BC	0.09	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	5- 0- 7
BC Cont.		0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	384	116	5- 8	1- 8
			Hz =	85
C	47	2	3- 8	1- 8
B	171	87	3- 8	1- 8
			Hz =	46

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.16	135 C	0.00 0.16
E -B	0.14	93 C	0.00 0.14
--------Bottom Chords---------			
A -D	0.09	12 T	0.00 0.09
D -C	0.09	0 T	0.00 0.09
-------------Webs-------------			
D -E	0.02	84 T	

TL Defl	-0.02" in A -D		L/999
LL Defl	-0.01" in A -D		L/999
Hz Disp	LL	DL	TL
Jt B	0.01"	0.00"	0.01"
Shear // Grain	in E -B		0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.46

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

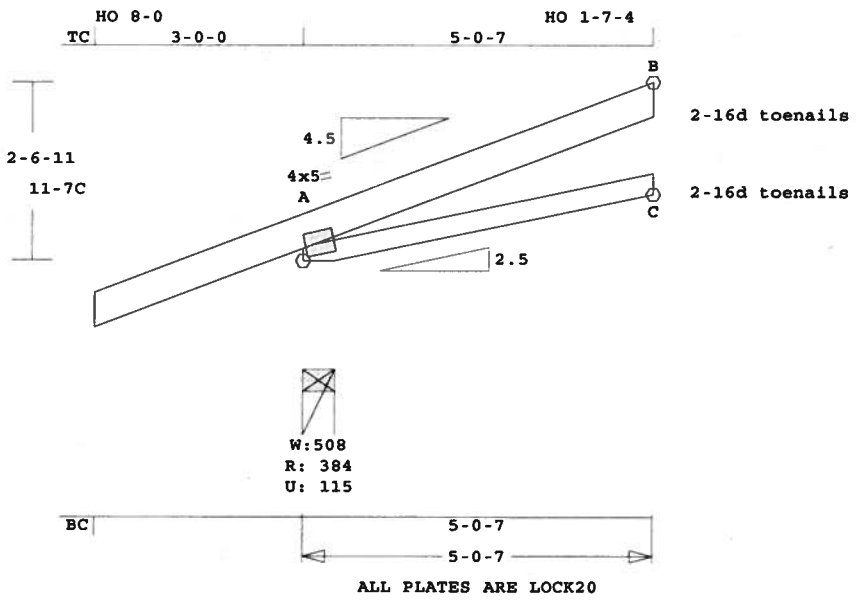
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 135 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R25	12	VJACK	50007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 6	SP-#2
BC	0.13	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	384	116	5- 8	1- 8
			Hz =	85
C	87	0	3- 8	1- 8
B	162	109	3- 8	1- 8
			Hz =	46

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.18	110 C	0.00 0.18
--------Bottom Chords---------			
A -C	0.13	3 C	0.00 0.13

TL Defl -0.04" in A -C L/999
LL Defl -0.02" in A -C L/999

Hz Disp	LL	DL	TL
Jt B	0.01"	0.00"	0.01"

Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

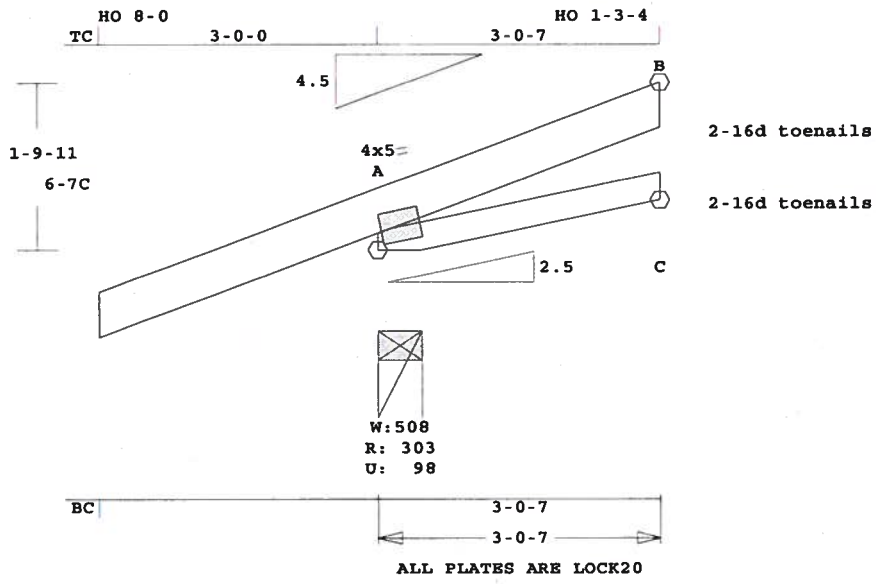
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph

Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 110 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R24*	12	VJACK	30007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 6	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	304	99	5- 8	1- 8
			Hz =	51
C	54	0	3- 8	1- 8
B	101	71	3- 8	1- 8
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	76 C	0.00 0.07
--------Bottom Chords---------			
A -C	0.05	2 C	0.00 0.05

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Shear // Grain in A -B 0.10

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

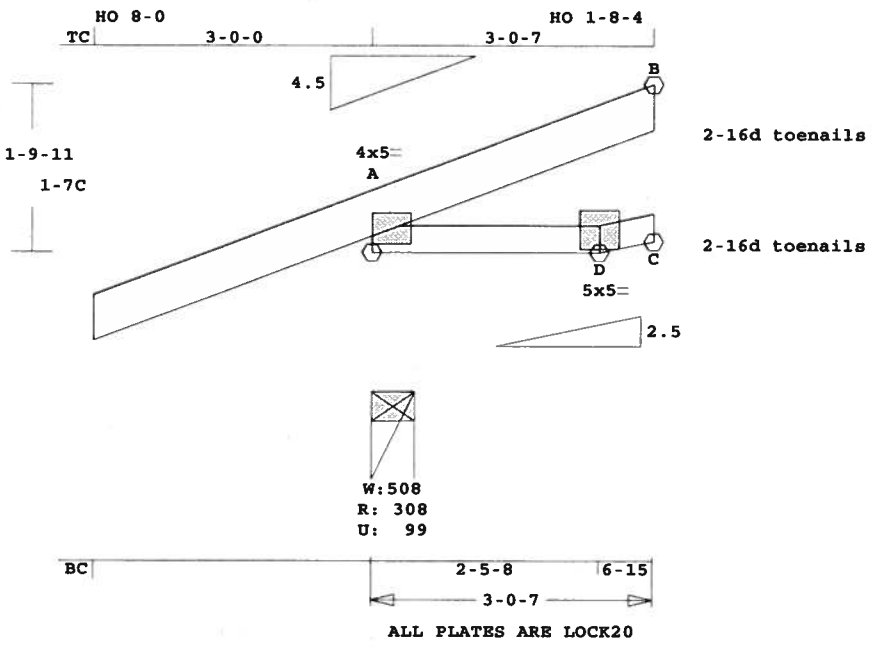
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph

Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 76 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R23	1	SP	30007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 6	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 0- 7
BC	Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	308	99	5- 8	1- 8
			Hz =	51
C	52	0	3- 8	1- 8
B	98	69	3- 8	1- 8
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	73 C	0.00 0.07
--------Bottom Chords---------			
A -D	0.05	0 T	0.00 0.05
D -C	0.02	7 T	0.00 0.02

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Shear // Grain in A -B 0.09

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
D	LOCK	5.0x 5.0	Ctr	1.2	0.39

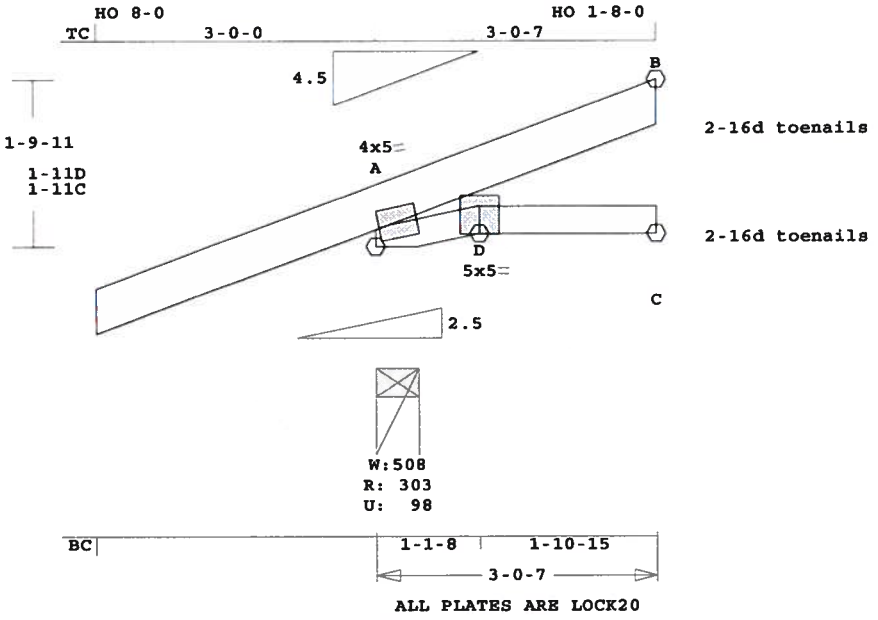
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 73 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R22*	1	SP	30007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 6	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 0- 7
BC	Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	304	99	5- 8	1- 8
			Hz =	51
C	54	0	3- 8	1- 8
B	101	71	3- 8	1- 8
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	76 C	0.00 0.07
--------Bottom Chords---------			
A -D	0.05	8 C	0.00 0.05
D -C	0.04	0 T	0.00 0.04

TL Defl 0.00" in D -C L/999
LL Defl 0.00" in D -C L/999

Hz Disp	LL	DL	TL
Jt B	0.00"	0.00"	0.00"

Shear // Grain in A -B 0.10

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
D	LOCK	5.0x 5.0	Ctr	0.8	0.33

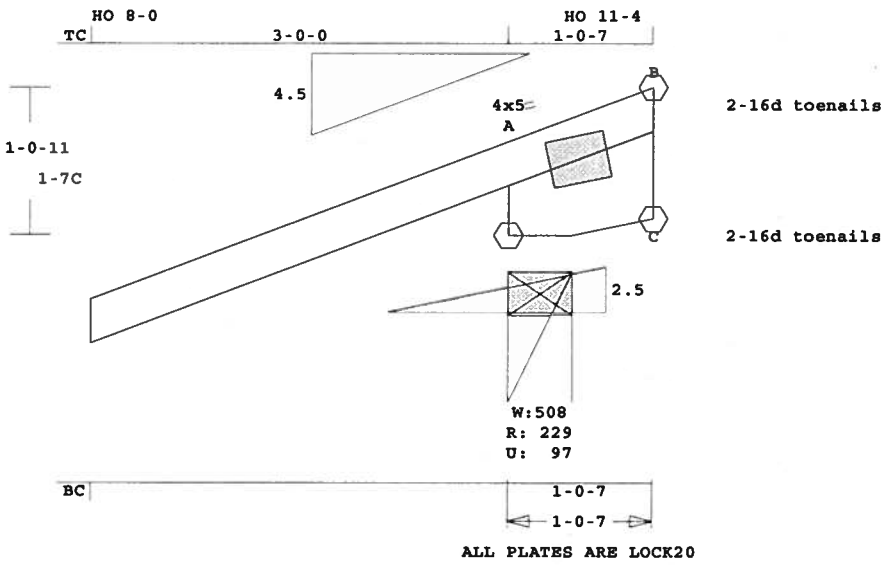
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 76 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R21*	12	VJACK	10007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 12.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.05	2x 4	SP-#2
BC	0.00	2x 8	SP-2400

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	230	98	3- 8	1- 8
			Hz =	16
B	24	18	1- 8	1- 8
C	26	0	1- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -A	0.05	11	C 0.00	0.05	
A -B	0.01	18	C 0.00	0.01	

--------Bottom Chords---------

TL Defl 0.00" in D -E L/999
LL Defl 0.00" in D -E L/999

Hz Disp	LL	DL	TL
Jt C	0.00"	0.00"	0.00"

Shear // Grain in B -A 0.09

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	4.0x 5.0 Ctr	Ctr	1.00

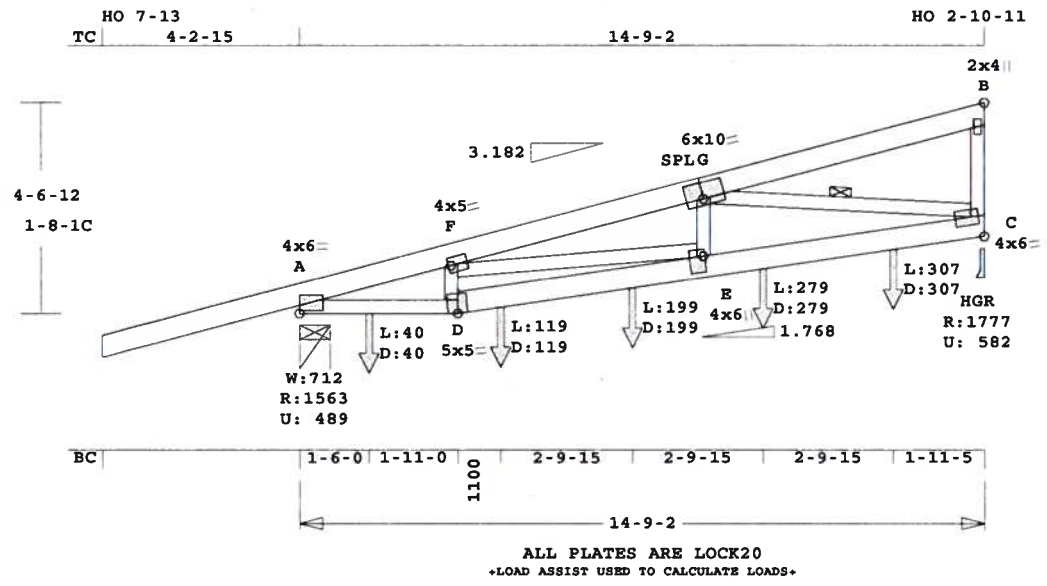
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Excessive Left OH condition.
Max allowable length is 2- 9- 5. Support end or provide level return.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 18 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R20*	1	SP	140902	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 123.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 6	SP-#2
BC	0.81	2x 4	SP-#2
EX D -C		2x 6	SP-2400
WB	0.76	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 9- 2
BC Cont.	0- 0- 0	14- 9- 2

WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	14.8'
BC V 0	20	0.0'	14.8'
BC V 40	40	1.5'	CL-LB
BC V 119	119	4.3'	CL-LB
BC V 199	199	7.2'	CL-LB
BC V 279	279	10.0'	CL-LB
BC V 307	307	12.8'	CL-LB

Plus 8 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	1563	490	7-12	1-14
			Hz =	176
C	1777	583	3- 8	1- 8
			Hz =	95

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -F	0.51	3135	C	0.03	0.48
F -G	0.41	3868	C	0.06	0.35
G -B	0.23	59	C	0.00	0.23
Bottom Chords					
A -D	0.81	2958	T	0.54	0.27
D -E	0.45	3009	T	0.15	0.30
E -C	0.64	3773	T	0.19	0.45
Webs					
D -F	0.03	154	T		
F -E	0.25	800	T		
E -G	0.36	1266	T		
G -C	0.76	3790	C		1 Br
C -B	0.05	208	T		

TL Defl -0.19" in D -E L/884
LL Defl -0.09" in D -E L/999

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.03"

Shear // Grain in E -C 0.59

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

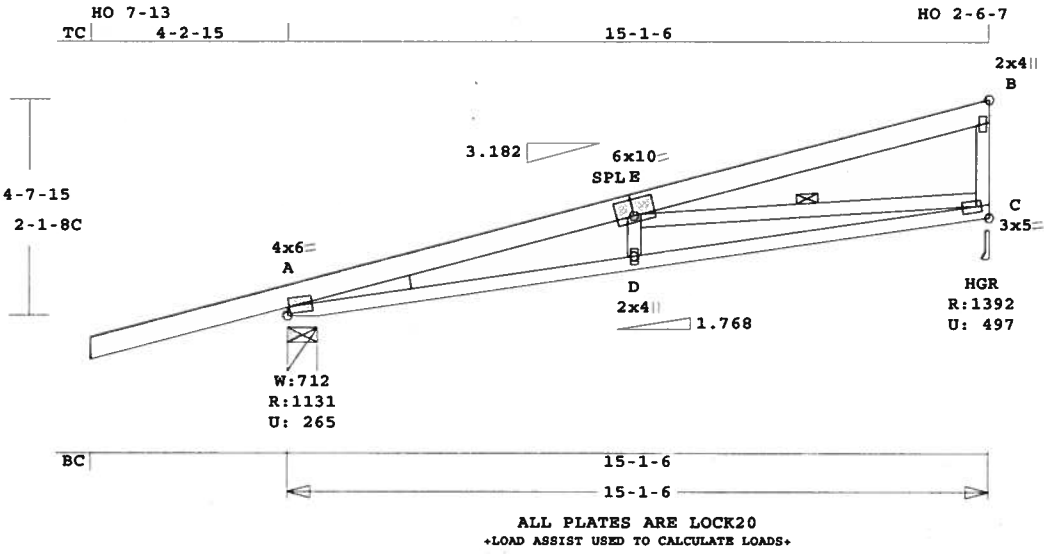
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.4	Ctr	0.74
F	LOCK	4.0x 5.0	Ctr	Ctr	0.73
G	LOCK	6.0x10.0	0.2	1.4	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.46
D	LOCK	5.0x 5.0	-0.2	2.7	0.55
E	LOCK	4.0x 6.0	0.2	-1.0	0.77
C	LOCK	4.0x 6.0	-1.0	-0.1	0.96

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3868 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R19*	1	HFSC	150106	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 109.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.90	2x 6	SP-#2
EX E -B		2x 6	SP-2400
BC	0.77	2x 4	SP-2400
WB	0.78	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	15- 1- 6
BC Cont.	0- 0- 0	15- 1- 6

WB 1 rows CLB on E -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	15.1'
BC V 0	20	0.0'	15.1'
TC V -40	-20	0.0'	
97	49		15.1'
BC V 0	-20	0.0'	
0	49		15.1'

Plus 8 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1132	265	7-12	1- 8
			Hz =	309
C	1392	498	3- 8	1- 8
			Hz =	166

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.90	3165 C	0.04	0.86
E -B	0.57	181 C	0.00	0.57
--------Bottom Chords--------				
A -D	0.59	3076 T	0.24	0.35
D -C	0.77	3080 T	0.15	0.62
------------Webs------------				
D -E	0.15	566 T		
E -C	0.78	3046 C		1 Br
C -B	0.15	666 C		

TL Defl -0.73" in D -C L/234
LL Defl -0.32" in D -C L/536

Hz Disp	LL	DL	TL
Jt C	0.03"	0.03"	0.06"

Shear // Grain in E -B 0.47

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	2.6	0.6	0.82
E	LOCK	6.0x10.0	0.2	1.4	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.46
D	LOCK	2.0x 4.0	Ctr	Ctr	0.51
C	LOCK	3.0x 5.0	-1.0	-0.1	0.99

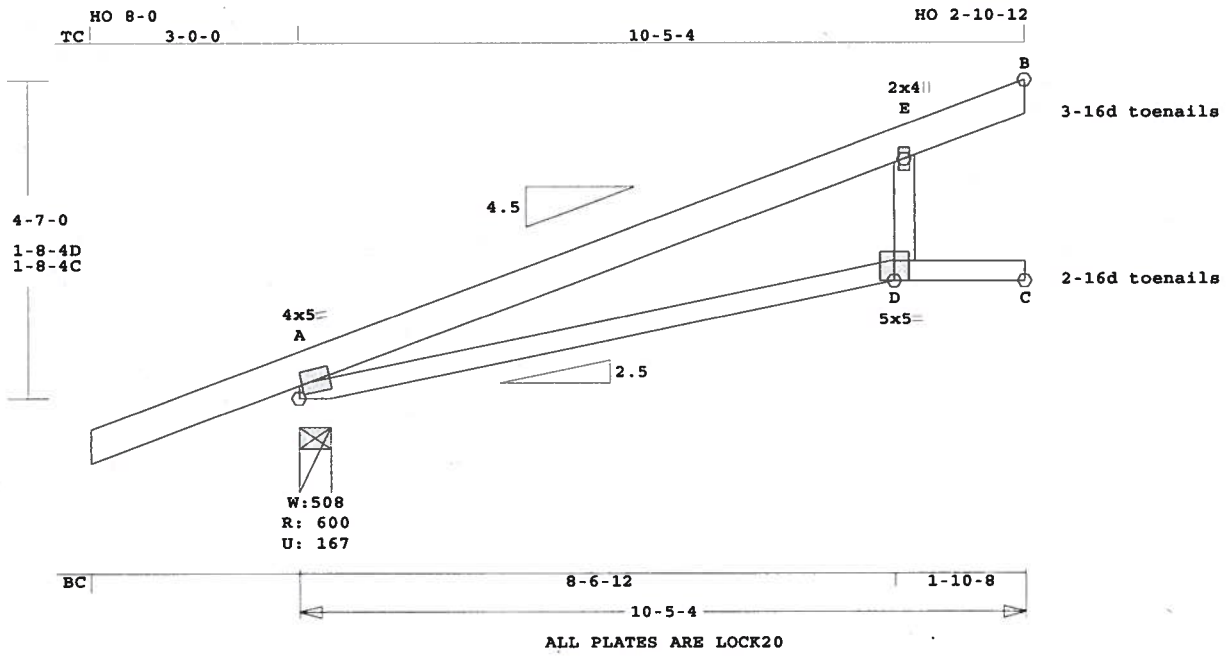
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 10- 8- 4
OH Loading
Soffit psf 2.0
Provide connection to bearing for 308 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3165 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R18B*	1	SP	100504	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 6	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 5- 4
BC Cont.	0- 0- 0	10- 5- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	600	168	5- 8	1- 8
			Hz =	176
C	53	57	3- 8	1- 8
B	374	116	3- 8	1- 8
			Hz =	95

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.71	183 C	0.00 0.71
E -B	0.40	109 T	0.01 0.39
--------Bottom Chords---------			
A -D	0.40	17 T	0.00 0.40
D -C	0.25	0 T	0.00 0.25
------------Webs------------			
D -E	0.06	245 T	

TL Defl	-0.45" in A -D		L/260
LL Defl	-0.19" in A -D		L/612
Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"
Shear // Grain	in E -B		0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr-1.0		0.46

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

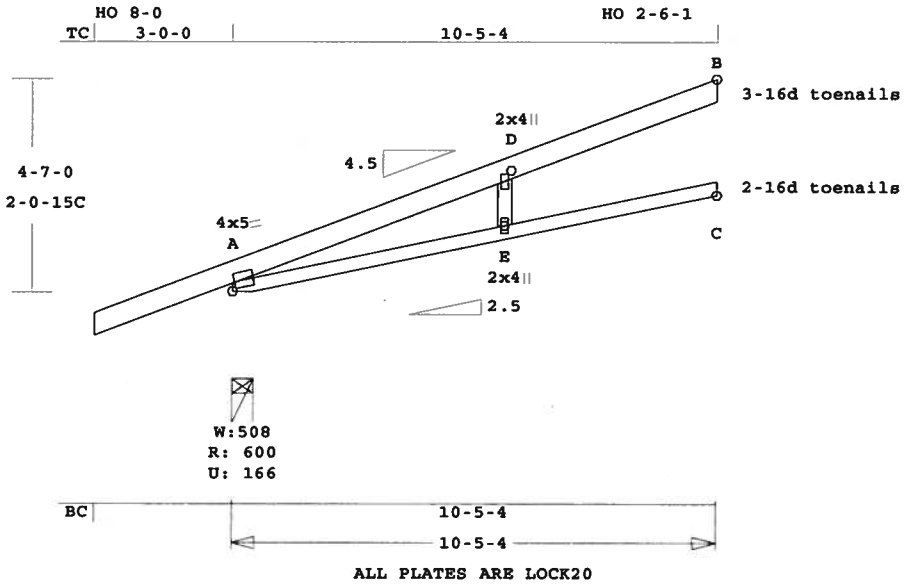
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 183 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R18A	2	HFSC	100504	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 63.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 6	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 5- 4
BC Cont.	0- 0- 0	10- 5- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	601	167	5- 8	1- 8
			Hz = 176	
C	114	2	3- 8	1- 8
B	334	172	3- 8	1- 8
			Hz = 95	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.71	188 C	0.00 0.71
D -B	0.70	128 C	0.00 0.70
--------Bottom Chords---------			
A -E	0.40	7 C	0.00 0.40
E -C	0.38	9 C	0.00 0.38
------------Webs------------			
E -D	0.04	157 T	

TL Defl	-0.39" in A -E		L/300
LL Defl	-0.18" in A -E		L/628
Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"
Shear // Grain	in D -B		0.24

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

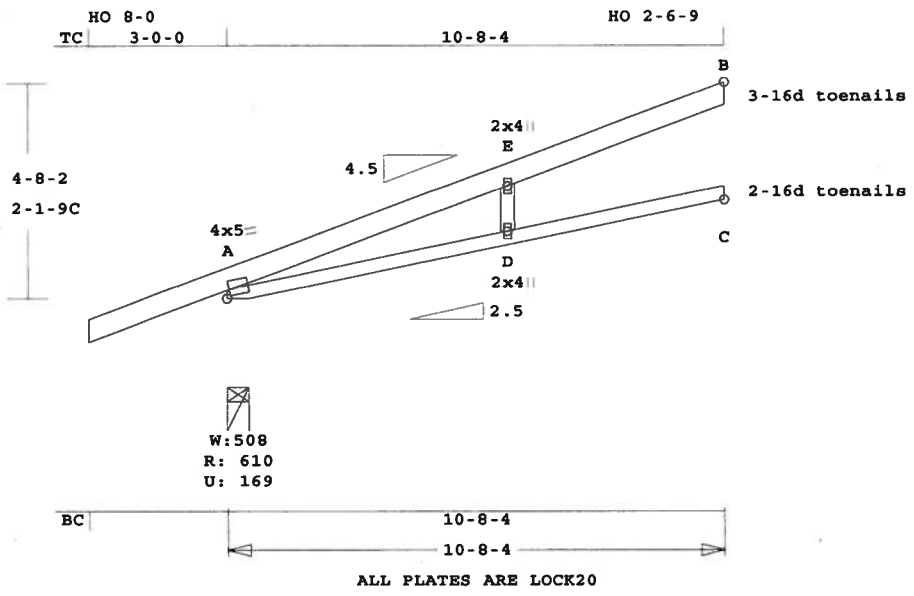
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 188 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R18*	24	VJACK	100804	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.75	2x 6	SP-#2
BC	0.42	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 8- 4
BC	Cont.	0- 0- 0	10- 8- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	611	169	5- 8	1- 8
			Hz = 180	
C	116	2	3- 8	1- 8
			Hz = 97	
B	342	176	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.75	190 C	0.00 0.75
E -B	0.74	130 C	0.00 0.74
--------Bottom Chords---------			
A -D	0.42	99 T	0.00 0.42
D -C	0.39	96 T	0.00 0.39
------------Webs------------			
D -E	0.04	161 T	

TL Defl -0.43" in A -D L/279
LL Defl -0.20" in A -D L/584

Hz Disp	LL	DL	TL
Jt B	0.03"	0.03"	0.06"

Shear // Grain in E -B 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

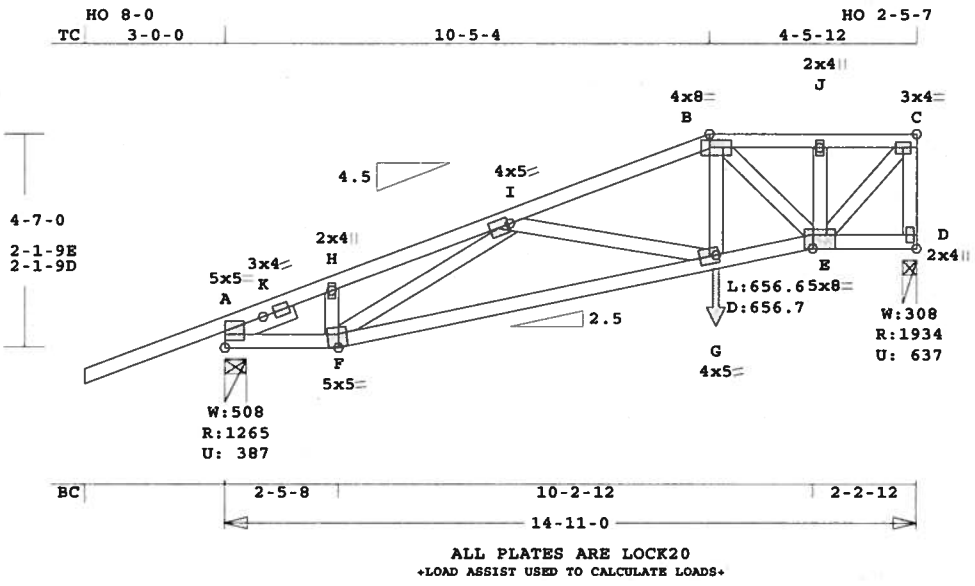
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 190 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R17A*	1*2P	SP	141100	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 107.7 LBS

```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.20  2x 4  SP-#2
BC  0.40  2x 4  SP-#2
WB  0.40  2x 4  SP-#3
SL  0.06  2x 4  SP-#3

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 14-11- 0
 BC Cont.      0- 0- 0 14-11- 0

Loading   Live   Dead  (psf)
TC        20.0   10.0
BC         0.0   10.0
Total     20.0   20.0   40.0
Spacing                 24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.00  Fc=1.00  Ft=1.00
BC Fb=1.00  Fc=1.00  Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor   1.25
Plate  Duration Factor   1.25
plf - Live  Dead  From    To
TC V    40    20   0.0'  14.9'
BC V     0    20   0.0'  14.9'
TC V    84    42  10.4'  13.9'
TC V   -40   -20  13.9'  14.9'
BC V     0    42  10.6'  13.9'
BC V     0   -20  13.9'  14.9'
BC V   657   657  10.6'  CL-LB


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
     Lbs   Lbs In-Sx In-Sx
A    1265   388  5- 8  1- 8
                Hz =   181
D    1934   638  3- 8  1- 8
                Hz =    98
```

```
Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -K  0.20  1152 C  0.00  0.20
K -H  0.20  1834 C  0.00  0.20
H -I  0.11  1766 C  0.00  0.11
I -B  0.18  2803 C  0.01  0.17
B -J  0.16  1769 C  0.00  0.16
J -C  0.10  1769 C  0.06  0.04
--------Bottom Chords---------
A -F  0.30  1651 T  0.15  0.15
F -G  0.35  2615 T  0.24  0.11
G -E  0.40  2648 T  0.24  0.16
E -D  0.18    98 T  0.00  0.18
------------Webs-------------
F -H  0.02   155 T
F -I  0.09  1087 C
I -G  0.02   179 T
G -B  0.21  1360 T
B -E  0.08  1058 C
E -J  0.04   492 C
E -C  0.40  2529 T
D -C  0.14  1819 C
-----------Sliders-----------
A -K  0.06   827 C

TL Defl  -0.15" in F -G  L/999
LL Defl  -0.07" in F -G  L/999
Hz Disp   LL      DL      TL
  Jt D   0.01"   0.01"   0.02"
Shear // Grain  in B -J   0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  5.0x 5.0 2.5 0.5 0.94
K  LOCK  3.0x 4.0 Ctr Ctr 0.50
H  LOCK  2.0x 4.0 Ctr Ctr 0.38
I  LOCK  4.0x 5.0 0.5 0.2 1.00
B  LOCK  4.0x 8.0 Ctr Ctr 0.76
J  LOCK  2.0x 4.0 Ctr Ctr 0.38
C  LOCK  3.0x 4.0 Ctr Ctr 0.81
F  LOCK  5.0x 5.0-0.3 2.7 0.53
G  LOCK  4.0x 5.0 Ctr Ctr 0.61
E  LOCK  5.0x 8.0 Ctr-1.0 0.72
D  LOCK  2.0x 4.0 Ctr Ctr 0.39
```

```
NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
Girder    Half Hip
  Framing King Jacks
  Jack    Open Faced
  Setback 10- 5- 4
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3   screws -OR- 10d nails
  as each layer is applied.)
                ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  1    12      24       0
  BC  1    12      24       0
  WB  1     8       8
Plus clusters of nails where
  shown.
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:    B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  TC Dead Load:        4.0 psf
  BC Dead Load:        6.0 psf
Max comp. force     2803 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
    Designer)
    Gary Dounson, PE 35054
    Gary Dounson & Associates,
    Inc.
    2830 NW 41st Street Suite D
    Gainesville, FL 32606
    (352)375-8593
    CA 5201
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R17*	1*4P	SP	570808	4.5	3- 0- 0	0	

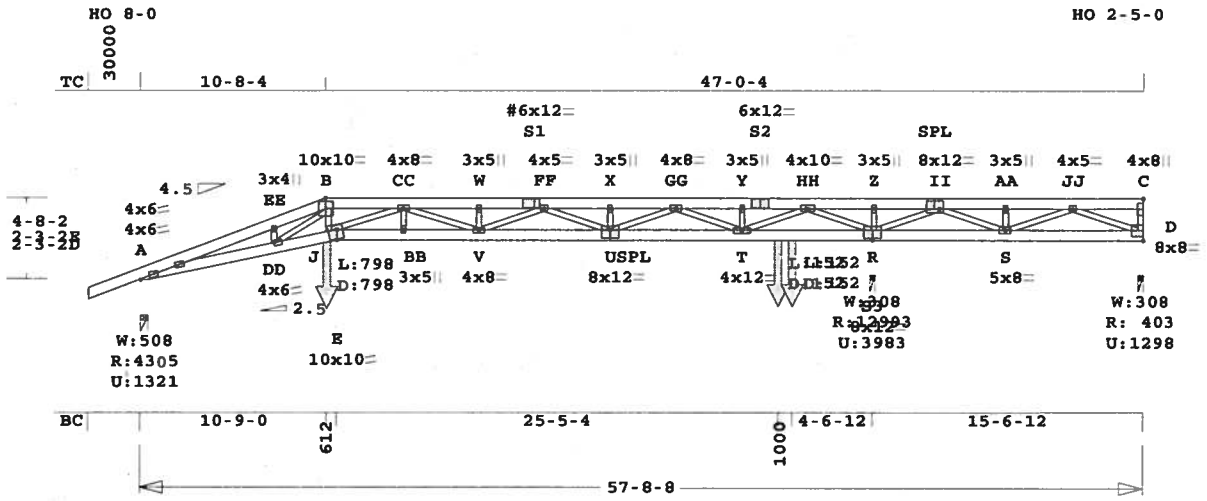
+Office and Care Center, Hospice, M.M. Parrish

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed: 110 mph
  Mean Roof Height: 25-0
  Exposure Category: B
  Occupancy Factor : 1.00
  Building Type: Enclosed
  TC Dead Load: 4.0 psf
  BC Dead Load: 6.0 psf
Max comp. force 23802 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
    Designer)
    Gary Dounson, PE 35054
    Gary Dounson & Associates,
    Inc.
    2830 NW 41st Street Suite D
    Gainesville, FL 32606
    (352)375-8593
    CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R17	1*4P	SP	570808	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 568.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 4-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.36	2x 8	SP-2400
BC	0.27	2x 8	SP-2400
WB	0.71	2x 4	SP-#3
EX T -HH		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	57- 8- 8
BC Cont.		0- 0- 0	57- 8- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	57.7'
BC V	0	20	0.0'	57.7'
BC V	107	107	11.8'	35.8'
BC V	119	119	38.6'	54.7'
BC V	798	798	10.8'	CL-LB
BC V	152	152	36.8'	CL-LB
BC V	152	152	37.6'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	4305	1321	5- 8	1- 8
			Hz =	179
S3	12994	3983	3- 8	2- 7
D	404	1298G	3- 8	1- 8
			Hz =	97

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -EE	0.08	20000 C	0.02 0.06
EE-B	0.08	19866 C	0.02 0.06
B -CC	0.09	21775 C	0.03 0.06
CC-W	0.11	23802 C	0.04 0.07
W -S1	0.10	23802 C	0.04 0.06
S1-FF	0.08	23802 C	0.04 0.04
FF-X	0.06	16647 C	0.01 0.05
X -GG	0.06	16647 C	0.01 0.05
GG-Y	0.01	275 C	0.00 0.01
Y -S2	0.00	275 C	
S2-HH	0.01	275 C	0.00 0.01
HH-Z	0.36	23427 T	0.20 0.16
Z -II	0.36	23427 T	0.20 0.16
II-AA	0.08	7816 T	0.06 0.02
AA-JJ	0.07	7816 T	0.06 0.01
JJ-C	0.01	0 T	0.00 0.01
--------Bottom Chords---------			
A -DD	0.25	18976 T	0.16 0.09
DD-E	0.27	20907 T	0.18 0.09
E -BB	0.24	23810 T	0.20 0.04
BB-V	0.25	23810 T	0.20 0.05
V -U	0.27	20623 T	0.17 0.10
U -T	0.14	8826 T	0.07 0.07
T -S3	0.22	11938 C	0.01 0.21
S3-S	0.22	15867 C	0.01 0.21
S -D	0.08	3563 C	0.00 0.08
------------Webs------------			
DD-EE	0.01	280 T	
DD-B	0.08	2116 C	
E -B	0.61	7634 T	
E -CC	0.08	2167 C	
BB-CC	0.06	935 T	
CC-V	0.01	176 T	
V -W	0.00	164 C	
V -FF	0.28	3522 T	
FF-U	0.17	4404 C	
U -X	0.01	162 T	
U -GG	0.69	8665 T	
GG-T	0.38	9507 C	
T -Y	0.00	194 C	
T -HH	0.62	13497 T	
HH-S3	0.51	12727 C	
S3-Z	0.05	1419 C	
S3-II	0.33	8376 C	
II-S	0.71	8917 T	
S -AA	0.00	114 T	
S -JJ	0.19	4697 C	
JJ-D	0.31	3931 T	
D -C	0.00	73 C	

TL Defl -0.89" in BB-V L/560
LL Defl -0.44" in BB-V L/999

Hz Disp	LL	DL	TL
Jt S3	0.13"	0.13"	0.25"

Shear // Grain in T -S3 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	4.0x 6.0	Ctr	Ctr	0.75
A LOCK	4.0x 6.0	Ctr	Ctr	0.31
EE LOCK	3.0x 4.0	Ctr	Ctr	0.39
B LOCK	10.0x10.0	Ctr	Ctr	0.73
CC LOCK	4.0x 8.0	Ctr	0.5	0.72
W LOCK	3.0x 5.0	Ctr	Ctr	0.39
S1#LOCK	6.0x12.0-2.0		0.1	0.91
FF LOCK	4.0x 5.0	Ctr	0.5	0.68
X LOCK	3.0x 5.0	Ctr	Ctr	0.39
GG LOCK	4.0x 8.0	Ctr	0.5	0.91
Y LOCK	3.0x 5.0	Ctr	Ctr	0.39
S2 LOCK	6.0x12.0	Ctr	Ctr	0.91
HH LOCK	4.0x10.0	Ctr	0.5	0.90
Z LOCK	3.0x 5.0	Ctr	Ctr	0.39
II LOCK	8.0x12.0	0.5	1.5	0.99
AA LOCK	3.0x 5.0	Ctr	Ctr	0.39
JJ LOCK	4.0x 5.0	Ctr	0.5	0.68
C LOCK	4.0x 8.0-0.2	Ctr		0.85
DD LOCK	4.0x 6.0	1.0-0.2		0.63
E LOCK	10.0x10.0-0.7-1.6			0.85
BB LOCK	3.0x 5.0	Ctr	Ctr	0.39
V LOCK	4.0x 8.0	Ctr-0.5		0.72
U LOCK	8.0x12.0	Ctr-1.5		0.99
T LOCK	4.0x12.0	Ctr-0.5		0.90
S3 LOCK	8.0x12.0	Ctr-1.5		0.99
S LOCK	5.0x 8.0-0.5	Ctr		0.96
D LOCK	8.0x 8.0-0.5	Ctr		0.88

= Plate Monitor used

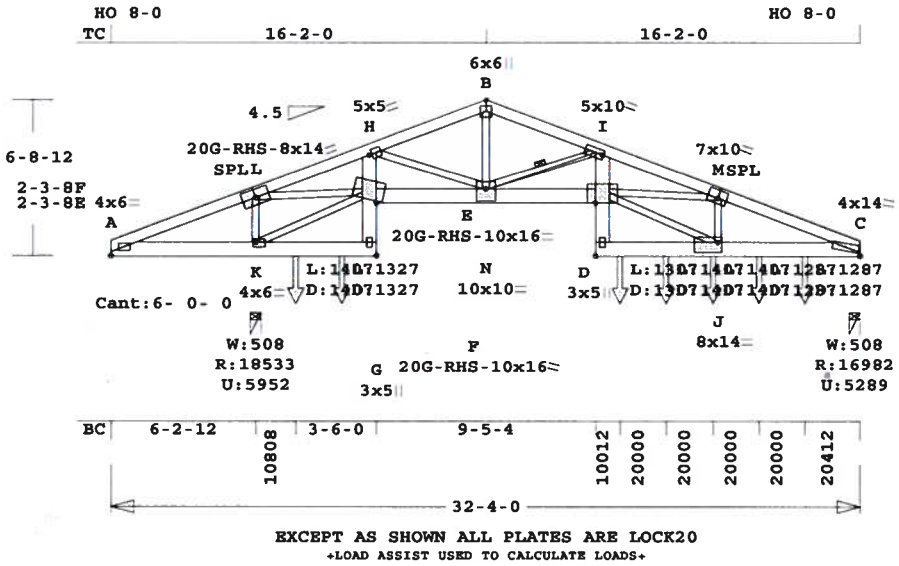
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	24
BC 2	12	24	24
WB 1	8	8	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R16*	1*4P	BOXTRAY	320400	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 318.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 14-MAR-06
****
* 4-Ply Truss *
****

	CSI	Size	Lumber
TC	0.66	2x 6	SP-#2
EX B -M		2x 6	SP-2400
BC	0.93	2x 8	SP-2400
CW	0.84	2x 8	SP-2400
WB	1.00	2x 4	SP-#3
EX L -F		2x 4	SP-2400
EX N -B		2x 4	SP-2400
EX E -M		2x 4	SP-#2
EX E -J		2x 4	SP-2400

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

WB 1 rows CLB on N -I
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	32.3'
BC V	0	20	0.0'	32.3'
BC V	704	704	10.9'	20.9'
BC V	1407	1407	7.9'	CL-LB
BC V	1327	1327	9.9'	CL-LB
BC V	1307	1307	21.9'	CL-LB
BC V	1407	1407	23.9'	CL-LB
BC V	1407	1407	25.9'	CL-LB
BC V	1287	1287	27.9'	CL-LB
BC V	1287	1287	29.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
K	18534	5953	5- 8	3- 9
C	16982	5289	5- 8	3- 8

Hz = -70
Hz = 71

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -L	0.15	1068 C	0.00	0.15
L -H	0.66	35522 C	0.27	0.39
H -B	0.62	32985 C	0.23	0.39
B -I	0.40	32942 C	0.14	0.26
I -M	0.72	56064 C	0.43	0.29
M -C	0.46	37975 C	0.31	0.15
Bottom Chords				
A -K	0.21	1016 T	0.00	0.21
K -G	0.23	2525 T	0.02	0.21
F -N	0.67	34980 T	0.30	0.37
N -E	0.93	54606 T	0.47	0.46
D -J	0.32	4649 T	0.04	0.28
J -C	0.49	35498 T	0.30	0.19
Chord-Webs				
G -F	0.46	2731 T	0.02	0.44
F -H	0.30	3162 T	0.02	0.28
D -E	0.84	3459 T	0.03	0.81
E -I	0.51	18062 T	0.17	0.34
Webs				
K -L	0.55	13988 C		
L -F	0.63	32219 T		
K -F	0.08	1793 C		
H -N	0.17	4258 C		
N -B	0.43	21977 T		
N -I	1.00	25411 C		1 Br
E -M	0.78	16893 T		
E -J	0.67	33951 T		
J -M	0.28	7269 C		

TL Defl -0.99" in D -E L/307
LL Defl -0.50" in D -E L/614
LL Cant 0.24" in A -K L/294
Shear // Grain in N -E 0.85

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0 Ctr	Ctr		0.85
L	RHS	8.0x14.0-0.1	0.3		0.95
H	LOCK	5.0x 5.0 Ctr	Ctr		0.46
B	LOCK	6.0x 6.0 Ctr	Ctr		0.95
I	LOCK	5.0x10.0-0.5	0.2		1.00
M	LOCK	7.0x10.0-0.2	1.0		0.95
C	LOCK	4.0x14.0-0.4	0.1		0.99
K	LOCK	4.0x 6.0 Ctr	-0.5		0.91
G	LOCK	3.0x 5.0 Ctr	Ctr		0.96
F	RHS	10.0x16.0 Ctr	3.1		0.97
N	LOCK	10.0x10.0 Ctr	-1.2		0.97
E	RHS	10.0x16.0 Ctr	1.4		0.86
D	LOCK	3.0x 5.0 Ctr	Ctr		0.96
J	LOCK	8.0x14.0-3.5	-1.3		0.84

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

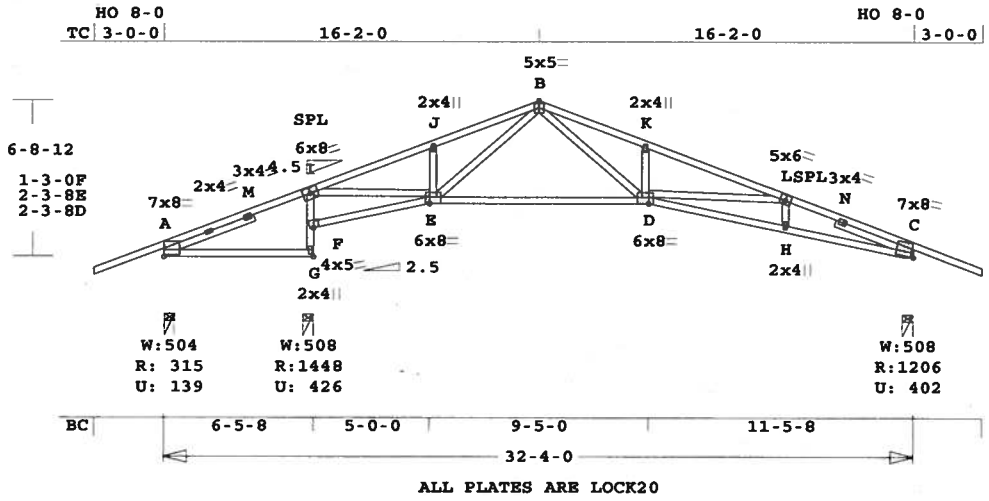
Rows	Nails	Screws	Bolts
TC 2	12	24	24
BC 2	4	6.5	10
WB 1	8	8	

Web Connection Exception -- Use 4" spacing for screws or nails on the following webs
N- I
No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 56064 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

JOB	MARK	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R15*	4	SP	320400	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 211.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.70	2x 4	SP-#2
BC	0.91	2x 4	SP-#2
CW	0.95	2x 4	SP-#3
WB	0.61	2x 4	SP-#3
SL	0.33	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	32- 4- 0
BC Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	315	140	5- 4	1- 8
			Hz =	-70
G	1449	426	5- 8	1-10
C	1206	402	5- 8	1- 8
			Hz =	71

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.30	272	C	0.00	0.30
M -I	0.55	274	T	0.05	0.50
I -J	0.52	1810	C	0.02	0.50
J -B	0.30	1808	C	0.03	0.27
B -K	0.39	2907	C	0.05	0.34
K -L	0.70	2902	C	0.12	0.58
L -N	0.52	3188	C	0.06	0.46
N -C	0.47	1544	C	0.01	0.46
--------Bottom Chords---------					
A -G	0.19	202	C	0.00	0.19
F -E	0.37	254	C	0.00	0.37
E -D	0.60	1342	T	0.22	0.38
D -H	0.79	3017	T	0.50	0.29
H -C	0.91	2992	T	0.50	0.41
----------Chord-Webs----------					
G -F	0.95	1388	C	0.04	0.91
F -I	0.23	1228	C	0.04	0.19
------------Webs------------					
I -E	0.61	1922	T		
E -J	0.07	291	C		
E -B	0.13	435	T		
B -D	0.57	1779	T		
D -K	0.07	334	C		
D -L	0.11	271	T		
H -L	0.02	113	T		
----------Sliders-----------					
A -M	0.14	493	T		
N -C	0.33	1734	C		

TL Defl -0.63" in E -D L/484
LL Defl -0.27" in E -D L/999

Hz Disp	LL	DL	TL
Jt C	0.10"	0.09"	0.19"

Shear // Grain in M -I 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

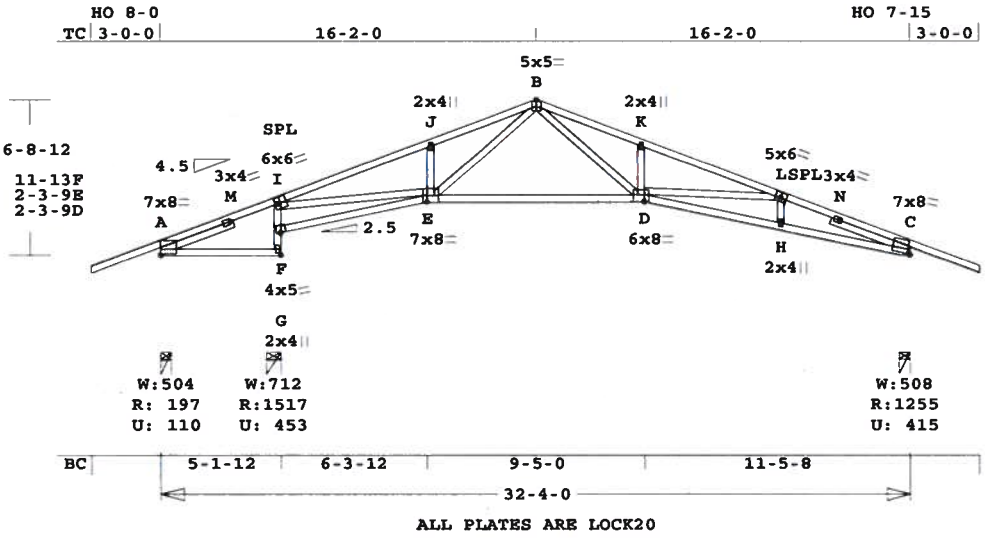
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	6.0x 8.0	Ctr	Ctr	0.87
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.75
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
L	LOCK	5.0x 6.0	0.2	0.5	0.87
N	LOCK	3.0x 4.0	Ctr	Ctr	0.64
C	LOCK	7.0x 8.0-4.6	1.2		0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.58
F	LOCK	4.0x 5.0	Ctr	0.3	0.75
E	LOCK	6.0x 8.0	Ctr-0.5		0.89
D	LOCK	6.0x 8.0	Ctr-0.5		0.66
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3188 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R14	1	SP	320400	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 212.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.76	2x 4	SP-#2
BC	0.95	2x 4	SP-#2
CW	0.60	2x 4	SP-#2
WB	0.79	2x 4	SP-#3
SL	0.34	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	32- 4- 0
BC	Cont.	0- 0- 0	32- 4- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	198	111	5- 4	1- 8
			Hz = -70	
G	1517	453	7-12	1-11
C	1255	416	5- 8	1- 8
			Hz = 71	

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -M	0.14	106 T	0.00	0.14
M -I	0.60	381 T	0.06	0.54
I -J	0.57	2307 C	0.03	0.54
J -B	0.37	2309 C	0.15	0.22
B -K	0.40	3172 C	0.19	0.21
K -L	0.76	3163 C	0.15	0.61
L -N	0.57	3395 C	0.07	0.50
N -C	0.51	1662 C	0.01	0.50
--------Bottom Chords---------				
A -G	0.11	300 C	0.00	0.11
F -E	0.49	338 C	0.00	0.49
E -D	0.67	1525 T	0.25	0.42
D -H	0.82	3210 T	0.54	0.28
H -C	0.95	3184 T	0.53	0.42
----------Chord-Webs----------				
G -F	0.60	1474 C	0.01	0.59
F -I	0.15	1281 C	0.09	0.06
-------------Webs-------------				
I -E	0.79	2472 T		
E -J	0.08	342 C		
E -B	0.25	802 T		
B -D	0.59	1859 T		
D -K	0.08	347 C		
D -L	0.08	247 T		
H -L	0.02	115 T		
-----------Sliders-----------				
A -M	0.13	425 T		
N -C	0.34	1825 C		

TL Defl -0.68" in E -D L/470
LL Defl -0.28" in E -D L/999

Hz Disp	LL	DL	TL
Jt C	0.13"	0.12"	0.24"

Shear // Grain in G -F 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	7.0x 8.0	4.0	0.6	0.85
M	LOCK	3.0x 4.0	Ctr	Ctr	0.57
I	LOCK	6.0x 6.0	Ctr	Ctr	0.91
J	LOCK	2.0x 4.0	Ctr	Ctr	0.43
B	LOCK	5.0x 5.0	Ctr	Ctr	0.78
K	LOCK	2.0x 4.0	Ctr	Ctr	0.43
L	LOCK	5.0x 6.0	0.2	0.5	0.87
N	LOCK	3.0x 4.0	Ctr	Ctr	0.68
C	LOCK	7.0x 8.0	-4.6	1.3	0.91
G	LOCK	2.0x 4.0	Ctr	Ctr	0.58
F	LOCK	4.0x 5.0	Ctr	0.3	0.75
E	LOCK	7.0x 8.0	Ctr	Ctr	0.79
D	LOCK	6.0x 8.0	Ctr	-0.5	0.71
H	LOCK	2.0x 4.0	Ctr	Ctr	0.43

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3395 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

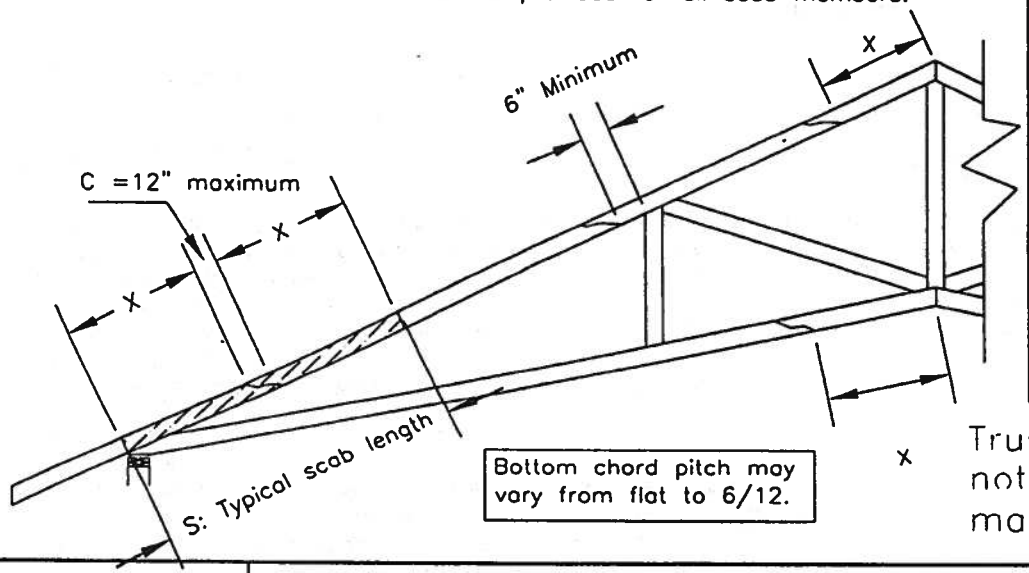
General detail for repair of broken chords and damaged or missing chord splices that meet the following conditions:

* Chord size must be maximum 2x6 and southern pine species.
* Scab must be the same size and grade as broken chord.
* Truss must be single ply.
* No more than two broken or cracked chords per truss.
* Perimeter of break area must be minimum of X distance from any heel or peak, and minimum of 6" away from any interior joint locations.

C = Maximum length of damaged area not to exceed 12".
S = Overall length of scab member (shaded) must be equal or greater than 2(X)+C.
X = Minimum length of scab member at each end of break area.
Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.
For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Use 2 rows of 10d common nails 3" on center each row and staggered into 2x4 members.
   Use 3 rows of 10d common nails 3" on center each row and staggered into 2x6 members.
   Use 6" spacing if scabs applied to each face of truss.
   Minimum end distance of 2" must be provided for all scab members.

THOMAS ALBANI
CERTIFICATE
No. 29387
STATE OF
FLORIDA
PROFESSIONAL ENGINEER



Bottom chord pitch may vary from flat to 6/12.

TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		X (INCHES)	MAXIMUM FORCE (LBS.) AT 33% LOAD DURATION	
2X4 SYP	2X6 SYP		2X4 SYP	2X6 SYP
13	20	24"	1924	2886
17	26	30"	2501	3752
21	32	36"	3079	4618
25	38	42"	3656	5484
29	44	48"	4233	6349

Truss manufacturer must be notified if field conditions do not match this general detail.

ROBBINS
Engineering Inc.
P.O.Box 280055, Tampa, FL 33682

ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "-", all slots in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Designed By: MG
Checked By: TAA
Rev. Date: 12/01/03
Dwg. No: GD-62-33

General detail for repair of broken webs that meet the following conditions.

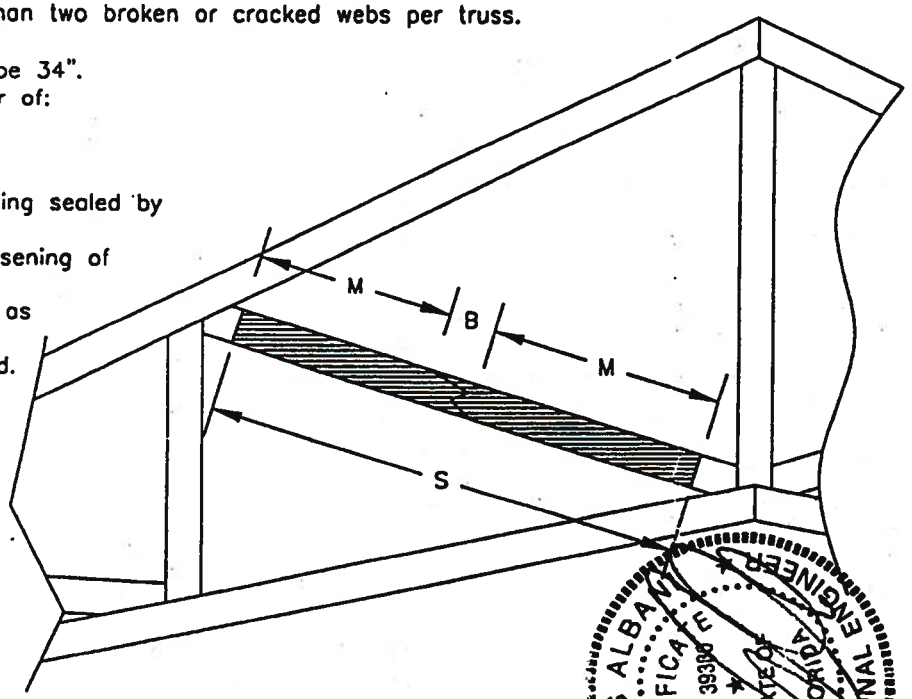
* Web size and grade must be maximum 2x4, #3, standard, or stud grade of any species.
* Truss must be single ply.
* No more than one crack or break per web and no more than two broken or cracked webs per truss.

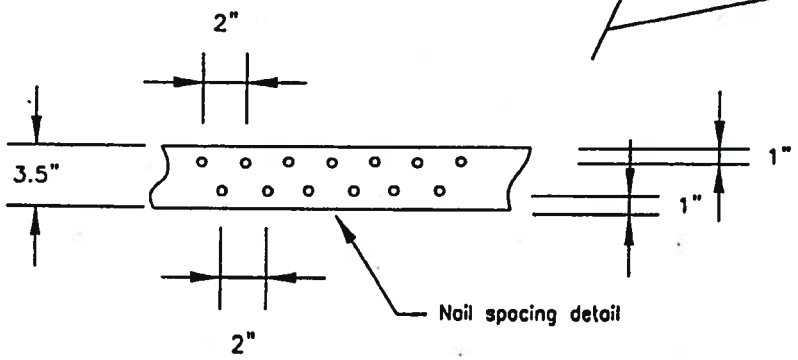
B = Maximum length of damaged area not to exceed 12".
M = Minimum length of scab at each end of break area to be 34".
S = Overall length of scab member (shaded) must be greater of:
S = 68" + B
S = 80% of original web length.

For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Attach 2x4 scab (shaded), minimum same size and grade as original web member, to one face of truss using 2 rows of 10d common nails 2" on center each row and staggered.

Truss manufacturer must be notified if field conditions do not match this general detail.





THOMAS ALBANESE CERTIFICATE No. 39306 STATE OF FLORIDA PROFESSIONAL ENGINEER

ROBBINS

Engineering Inc.

P.O.Box 280035, Tampa, FL 33682

ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "-", all slots in plates run parallel with the chords or horizontally at the peak end / or heel. No loose knots or voids in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg., Co. bears no responsibility for the erection of trusses. Field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "domino-ing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Designed By: MG
Checked By: TAA
Date: 5/23/02
Dwg. No: GD-41

BCSI-B1 SUMMARY SHEET 2 of 2

BRACING FOR THREE PLANES OF ROOF
EL ARRIOSTRE EN TRES PLANOS DE TECHO

☑ This bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.
Este metodo de arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2

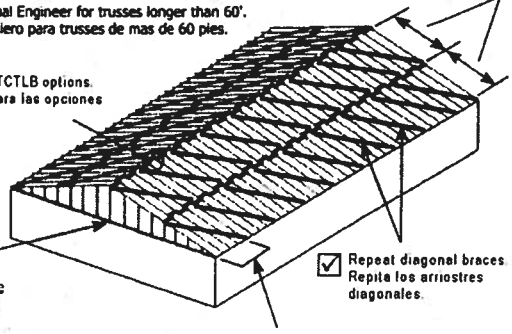
1) TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Brace (TCTLB) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un ingeniero para trusses de mas de 60 pies.

☑ See BCSI-B2 for TCTLB options.
Vea el BCSI-B2 para las opciones de TCTLB.

⚠ Refer to BCSI-B6 Summary Sheet - Gable End Frame Bracing.
Vea el resúmen BCSI-B6 - Arriostre del truss terminal de un techo a dos aguas.

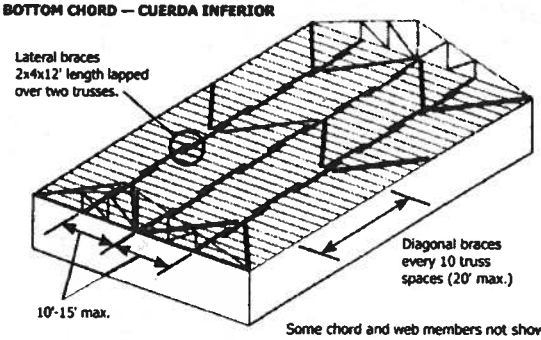


☑ Repeat diagonal braces.
Repita los arriostres diagonales.

☑ Set first five trusses with spacer pieces, then add diagonals. Repeat process on groups of four trusses until all trusses are set.

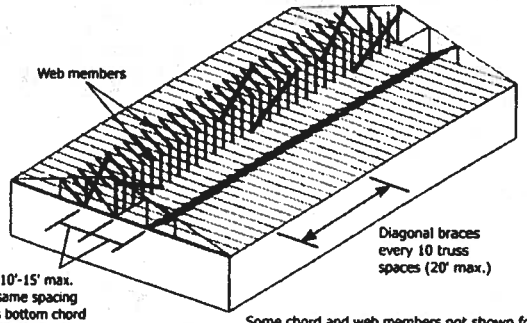
Instale los cinco primeros trusses con espaciadores, luego los arriostres diagonales. Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses esten instalados.

2) BOTTOM CHORD — CUERDA INFERIOR



Some chord and web members not shown for clarity.

3) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS



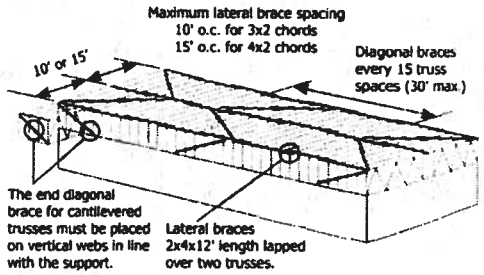
Some chord and web members not shown for clarity.

⚠ DIAGONAL BRACING IS VERY IMPORTANT ⚠
¡EL ARRIOSTRE DIAGONAL ES MUY IMPORTANTE!

BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3x2 Y 4x2

⚠ Refer to BCSI-B7 Summary Sheet - Temporary and Permanent Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Arriostre temporal y permanente de trusses de cuerdas paralelas para mayor información.



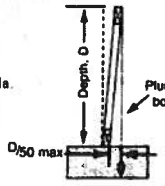
INSTALLING — INSTALACION

☑ Tolerances for Out-of-Plane. Tolerancias para Fuera-de-Plano

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥ 33.3'

☑ Tolerances for Out-of-Plumb

Tolerances para Fuerta-de-Plomada



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥ 8'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

⚠ Do not proceed with construction until all bracing is securely and properly in place.

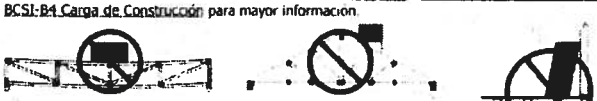
No proceda con la construcción hasta que todos los arriostres estén colocados en forma apropiada y segura.

Maximum Stack Height for Materials on Trusses

Material	Height (h)
Gypsum Board	12'
Plywood or OSB	16'
Asphalt Shingles	2 bundles
Concrete Block	8'
Clay Tile	3-4 tiles high

⊘ Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resúmen BCSI-B4 Carga de Construcción para mayor informacion.

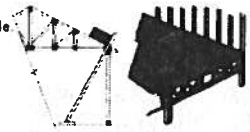


⊘ Do not overload small groups or single trusses.

No sobrecargue pequeños grupos o trusses individuales.

☑ Place loads over as many trusses as possible
Cologue las cargas sobre tantos trusses como sea posible.

☑ Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.



ALTERATIONS — ALTERACIONES

⚠ Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications and Installation Errors.

Vea el resúmen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

⊘ Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perfore ningun miembro estructural de los trusses, a menos que esté especificamente permitido en el dibujo del diseño del truss.

⚠ Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construccion o han sido alterados sin una autorizacion previa del Fabricante de Trusses, pueden reducir o eliminar la garantia del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer must rely on the fact that the Contractor and crane operator (if applicable) are capable to undertake the work they have agreed to do on a particular project. The Contractor should seek any required assistance regarding construction practices from a competent party. The methods and procedures outlined are intended to ensure that the overall construction techniques employed will put floor and roof trusses into place SAFELY. These recommendations for handling, installing and bracing wood trusses are based upon the collective experience of leading technical personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Erection/Installation Contractor. It is not intended that these recommendations be interpreted as superior to any design specification (provided by either an Architect, Engineer, the Building Designer, the Erection/Installation Contractor or otherwise) for handling, installing and bracing wood trusses and it does not preclude the use of other equivalent methods for bracing and providing stability for the walls and columns as may be determined by the truss Erection/Installation Contractor. Thus, the Wood Truss Council of America and the Truss Plate Institute expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

WOOD TRUSS COUNCIL OF AMERICA
One WTCA Center • 6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.woodtruss.com

TRUSS PLATE INSTITUTE
583 D'Onofrio Drive • Madison, WI 53719
608/833-5900 • www.tpinst.org

BCSI-B1 SUMMARY SHEET 1 of 2

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary bracing. Follow the recommendations for handling, installing and temporary bracing of trusses. Refer to BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent bracing on individual compression members. Refer to the BCSI-B3 Summary Sheet - Web Member Permanent Bracing/Web Reinforcement for more information. All other permanent bracing design is the responsibility of the Building Designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de los arriostres (bracing) temporales. Use las recomendaciones de manejo, instalación y arriostre temporal de los trusses. Vea el folleto BCSI 1-03 Guía de Buena Práctica para el Manejo, Instalación y Arriostre de los Trusses de Madera Conectados con Placas de Metalpara para mayor información.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de los arriostres permanentes en los miembros individuales en compresión. Vea la hoja resumen BCSI-B3 para los arriostres permanentes y refuerzos de los miembros secundarios (webs) para mayor información. El resto de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

⚠ The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, instalación y arriostre inadecuados, puede ser la caída de la estructura o aun peor, muertos o heridos.

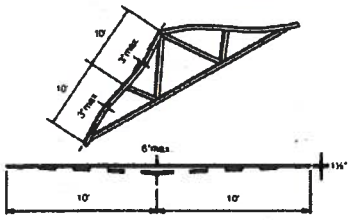
⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

HANDLING — MANEJO

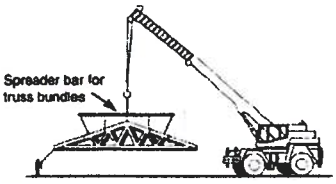
☑ Allow no more than 3" of deflection for every 10' of span.

No permita mas de 3 pulgadas de pandeo por cada 10 pies de tramo.



⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



☑ Check banding prior to moving bundles.

Revise los empaques antes de mover los paquetes de trusses.

☑ Pick up vertical bundles at the top chord.

Levante de la cuerda superior los grupos verticales de trusses.

⚠ Avoid lateral bending. — Evite la flexión lateral.

☑ Bundles stored on the ground for one week or more should be raised by blocking at 8' to 10' on center.

Los paquetes almacenados en la tierra por una semana o más deben ser elevados con bloques a cada 8 o 10 pies.

☑ For long term storage, cover bundles to prevent moisture gain but allow for ventilation.

Para almacen-amiento por mayor tiempo, cubra los paquetes para prevenir aumento de humedad pero permita ventilacion.

⊘ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.

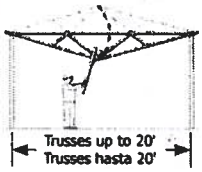
⊘ Do not store on uneven ground.

No almacene en tierra desigual.

HAND ERECTION — LEVANTAMIENTO A MANO

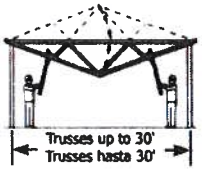
☑ Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



☑ Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING — LEVANTAMIENTO

☑ Hold each truss in position with the erection equipment until temporary bracing is installed and truss is fastened to the bearing points.

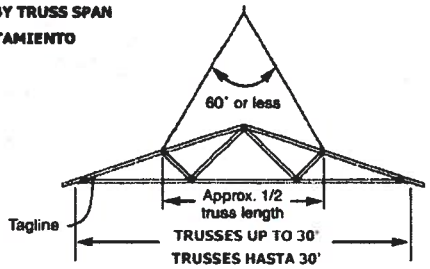
Sostenga cada truss en posición con la grúa hasta que el arriostre temporal esté instalado y el truss asegurado en los soportes.

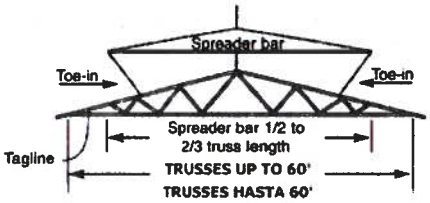
⊘ Do not lift trusses over 30' by the peak.

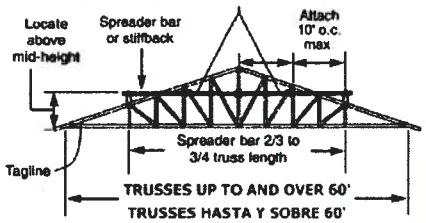
No levante del pico los trusses de más de 30 pies.

HOISTING RECOMMENDATIONS BY TRUSS SPAN

RECOMMENDACIONES DE LEVANTAMIENTO POR LONGITUD DEL TRUSS







BRACING — ARRIOSTRE

⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation and Temporary Bracing for more information.

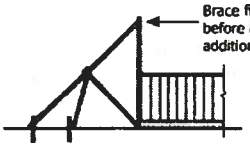
Vea el resúmen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

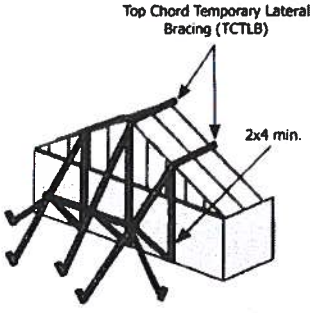
⊘ Do not walk on unbraced trusses.

No camine en trusses sueltos.

☑ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral bracing.

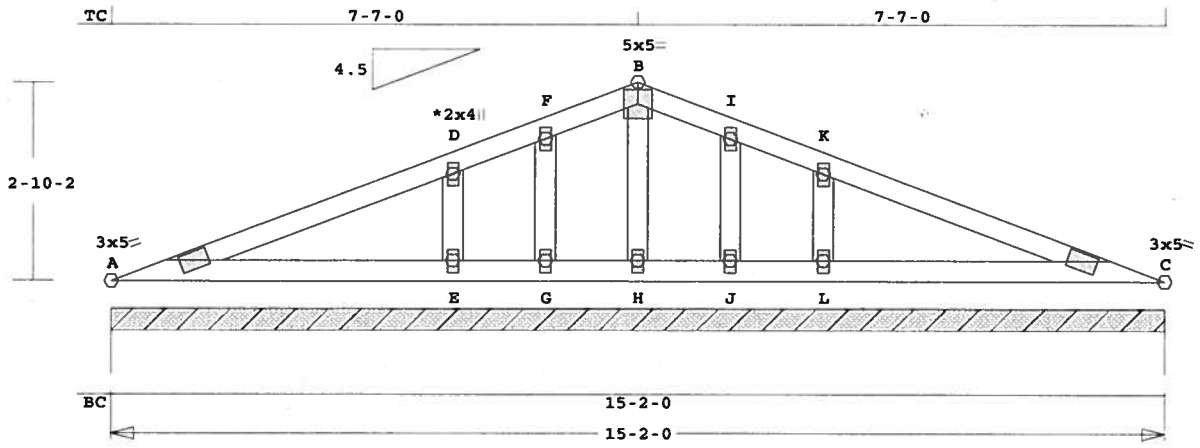
Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de arriostres laterales temporales de la cuerda superior.





JOB	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V10*	1	VL	150200	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 72.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.10	2x 4	SP-#2
GW	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	15- 2- 0
BC Cont.	0- 0- 0	15- 2- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 15- 2- 0			
	1223	371	Hz =	55

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -D	0.13	43 C	0.00		0.13
D -F	0.13	66 T	0.00		0.13
F -B	0.01	74 T	0.00		0.01
B -I	0.01	74 T	0.00		0.01
I -K	0.13	66 T	0.00		0.13
K -C	0.13	43 C	0.00		0.13
--------Bottom Chords---------					
A -E	0.10	3 T	0.00		0.10
E -G	0.08	0 T	0.00		0.08
G -H	0.01	0 T	0.00		0.01
H -J	0.01	0 T	0.00		0.01
J -L	0.08	0 T	0.00		0.08
L -C	0.10	3 T	0.00		0.10
----------Gable Webs----------					
E -D	0.05	225 C			
G -F	0.01	77 T			
H -B	0.01	77 C			
J -I	0.01	77 T			
L -K	0.05	225 C			

TL Defl -0.01" in A -E L/999
LL Defl -0.01" in A -E L/999
Shear // Grain in A -D 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.67
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	5.0x 5.0	Ctr	Ctr	0.48
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	3.0x 5.0	Ctr	Ctr	0.67
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	2.0x 4.0	Ctr	Ctr	0.00

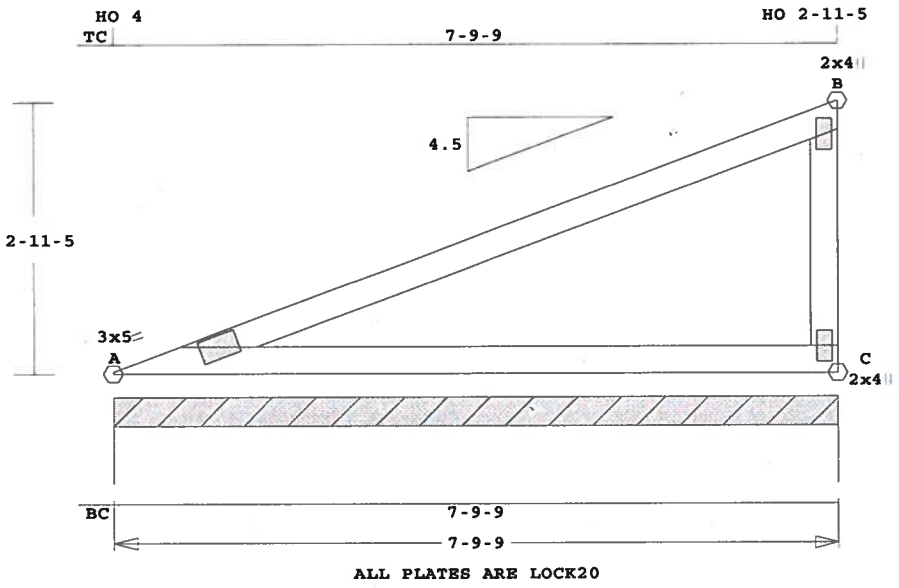
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 225 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V8*	2	VLM	70909	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.47	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 9- 9
BC	Cont.	0- 0- 0	7- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 7- 9- 9			
	564	176	Hz =	119

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.47	102 C	0.00	0.47
--------Bottom Chords---------				
A -C	0.26	0 T	0.00	0.26
----------Webs----------				
C -B	0.06	264 T		

LL Defl -0.04" in A -C L/999
TL Defl -0.08" in A -C L/999
Shear // Grain in A -B 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.55
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

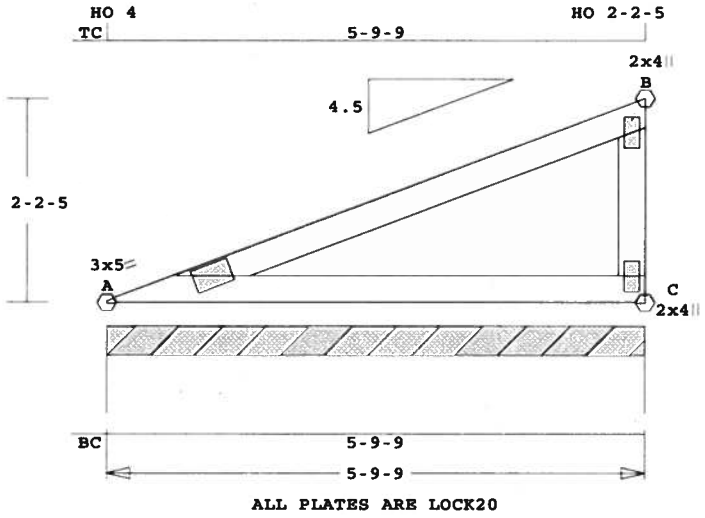
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 158 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	V7	2	VLM	50909	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 4	SP-#2
BC	0.11	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 9- 9
BC Cont.	0- 0- 0	5- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		5- 9- 9	
	404	126	Hz =	85

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.31	85 C	0.00 0.31
--------Bottom Chords---------			
A -C	0.11	0 T	0.00 0.11

----------Webs----------
C -B 0.05 224 T

LL Defl -0.01" in A -C L/999
TL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.52
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

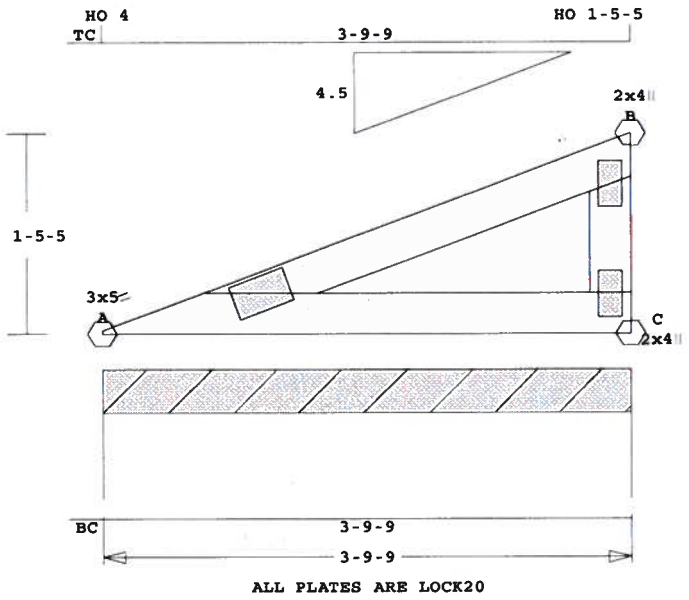
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 112 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V6*	2	VLM	30909	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
WB	0.03	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 9- 9
BC Cont.	0- 0- 0	3- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to		3- 9- 9	
	244	76	Hz =	51

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	51 C	0.00 0.07
--------Bottom Chords---------			
A -C	0.02	0 T	0.00 0.02
----------Webs----------			
C -B	0.03	135 T	

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.51
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

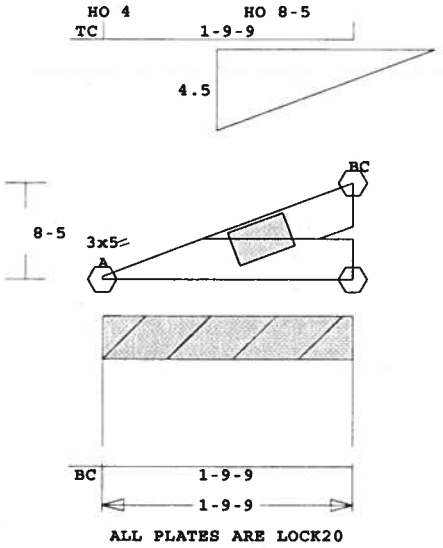
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 67 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V5*	2	VLM	10909	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 5.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.08	2x 4	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 9- 9
BC Cont.	0- 0- 0	1- 9- 9

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		1- 9- 9	
	143	63	Hz =	34

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -A	0.08	19 C	0.00	0.08
A -B	0.08	10 C	0.00	0.08
--------Bottom Chords---------				

LL Defl 0.00" in E -D L/999
TL Defl 0.00" in E -D L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.51

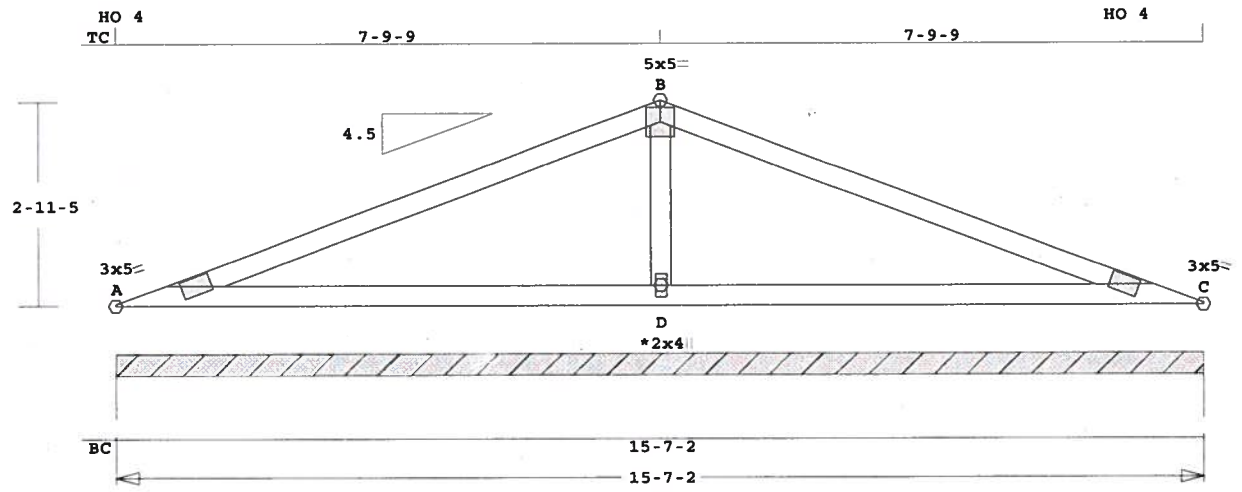
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 19 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V4*	1	VL	150702	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 62.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.36	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
GW	0.07	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	15- 7- 2
BC	Cont.	0- 0- 0	15- 7- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 15- 7- 2			
	1398	448	Hz =	127

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.36	129 T	0.00	0.36
B -C	0.36	129 T	0.00	0.36
--------Bottom Chords---------				
A -D	0.36	3 C	0.00	0.36
D -C	0.36	3 C	0.00	0.36
----------Gable Webs----------				
D -B	0.07	351 C		

TL Defl -0.10" in A -D L/999
LL Defl -0.04" in A -D L/999
Shear // Grain in A -B 0.26

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.68
B	LOCK	5.0x 5.0	Ctr	Ctr	0.48
C	LOCK	3.0x 5.0	Ctr	Ctr	0.68
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00

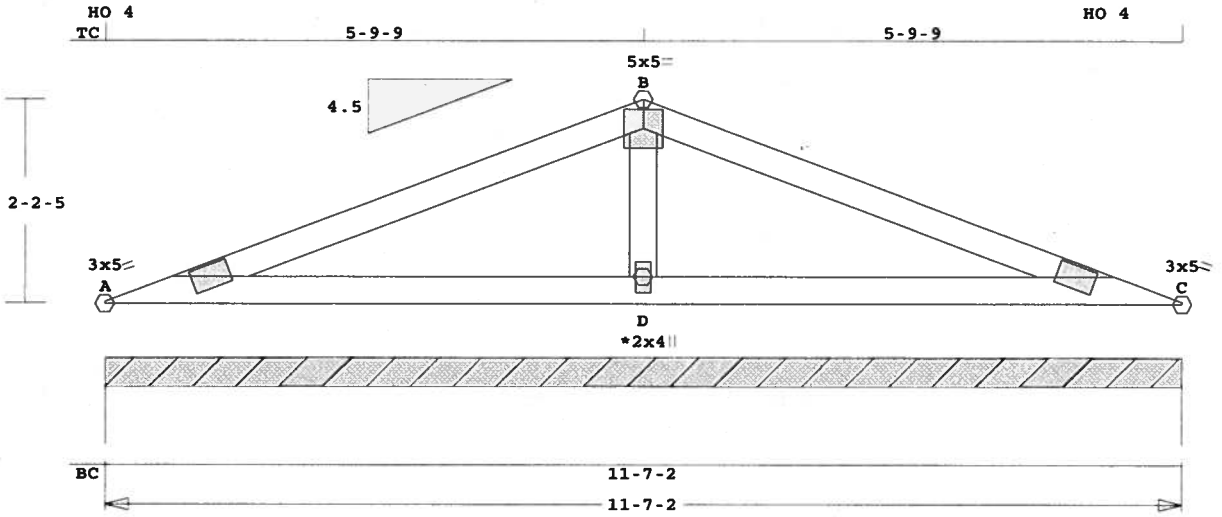
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 351 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	V3	1	VL	110702	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.19	2x 4	SP-#2
GW	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 7- 2
BC	Cont.	0- 0- 0	11- 7- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 11- 7- 2			
	1000	319	Hz =	83

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.20	102 T	0.01	0.19
B -C	0.20	102 T	0.01	0.19
--------Bottom Chords---------				
A -D	0.19	3 T	0.00	0.19
D -C	0.19	3 T	0.00	0.19
----------Gable Webs----------				
D -B	0.06	258 T		

TL Defl -0.03" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in A -B 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.61
B	LOCK	5.0x 5.0	Ctr	Ctr	0.44
C	LOCK	3.0x 5.0	Ctr	Ctr	0.61
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 250 Lbs

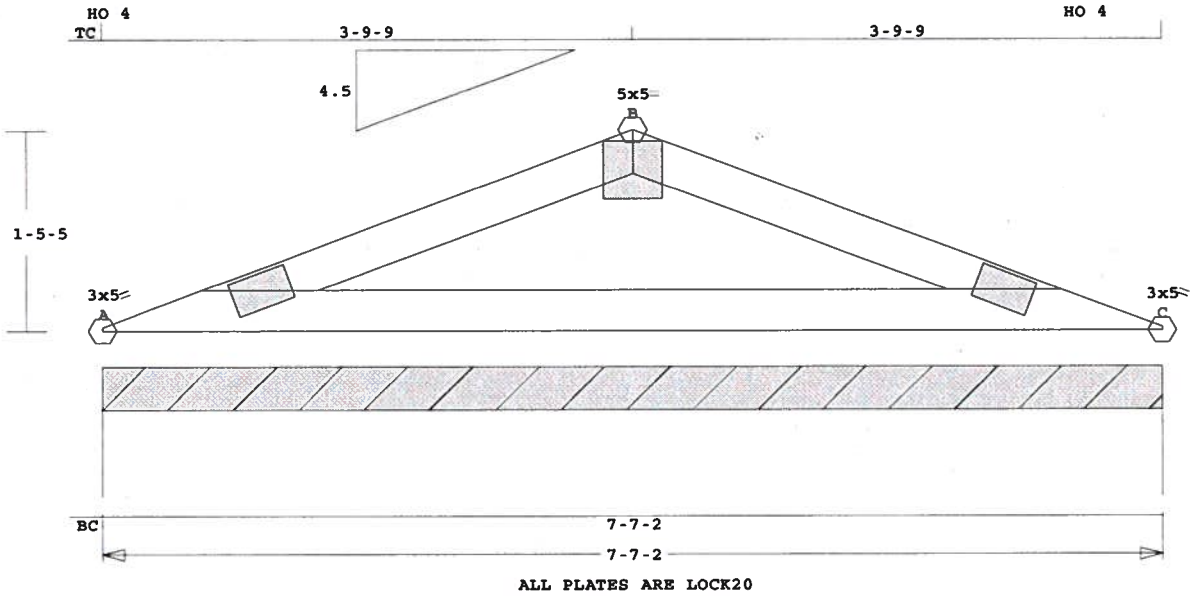
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc./Online Plus™ © 1996-2006 Version 19.0.009 Engineering - Portrait 3/16/2006 1:17:55 PM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*V2*	1	VL	70702	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 26.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.12	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 7- 2
BC Cont.	0- 0- 0	7- 7- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to	7- 7- 2		
	488	152	Hz =	3

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.13	364 T	0.00	0.13
B -C	0.13	364 T	0.00	0.13
--------Bottom Chords---------				
A -C	0.12	2 T	0.00	0.12

TL Defl -0.05" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.55
B	LOCK	5.0x 5.0	Ctr	-1.6	0.36
C	LOCK	3.0x 5.0	Ctr	Ctr	0.55

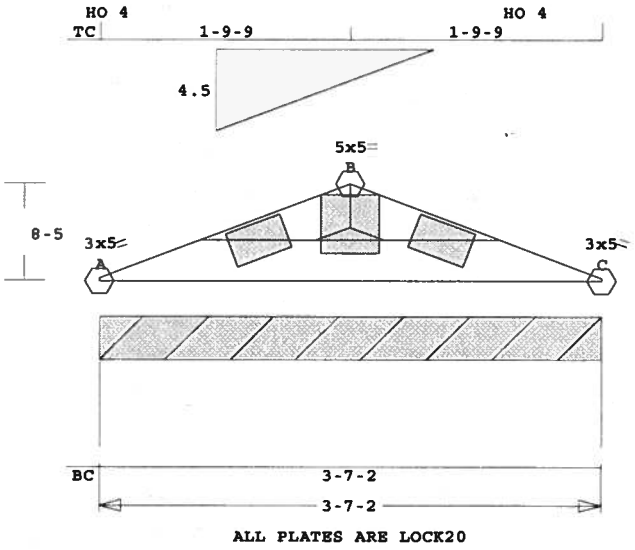
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 325 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	V1	1	VL	30702	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 11.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 4	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 7- 2
BC Cont.	0- 0- 0	3- 7- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to		3- 7- 2	
	168	52	Hz =	1

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	57	T
B -C	0.00	57	T
--------Bottom Chords---------			
A -C	0.00	0	T

TL Defl 0.00" in B -B L/999
LL Defl 0.00" in B -B L/999
Shear // Grain in B -B 0.02

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.51
B	LOCK	5.0x 5.0	Ctr	-1.6	0.33
C	LOCK	3.0x 5.0	Ctr	Ctr	0.51

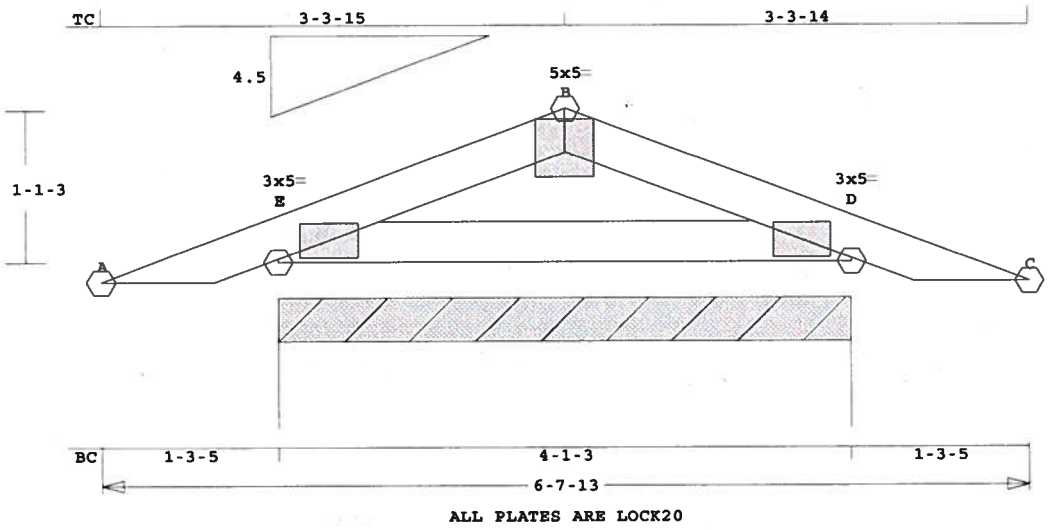
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 31 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T10*	18	PB	60713	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.05	2x 4	SP-#2
BC	0.04	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 7-13
BC Cont.	0- 0- 0	6- 7-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		4- 1- 3	
	491	171	Hz =	2

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
E -B	0.05	231 T	0.03	0.02
B -D	0.05	231 T	0.03	0.02
--------Bottom Chords---------				
E -D	0.04	18 C	0.00	0.04

LL Defl 0.00" in E -D L/999
TL Defl -0.01" in E -D L/999
Shear // Grain in E -B 0.07

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
E LOCK	3.0x 5.0	Ctr	Ctr	0.51
B LOCK	5.0x 5.0	Ctr	-1.6	0.33
D LOCK	3.0x 5.0	Ctr	Ctr	0.51

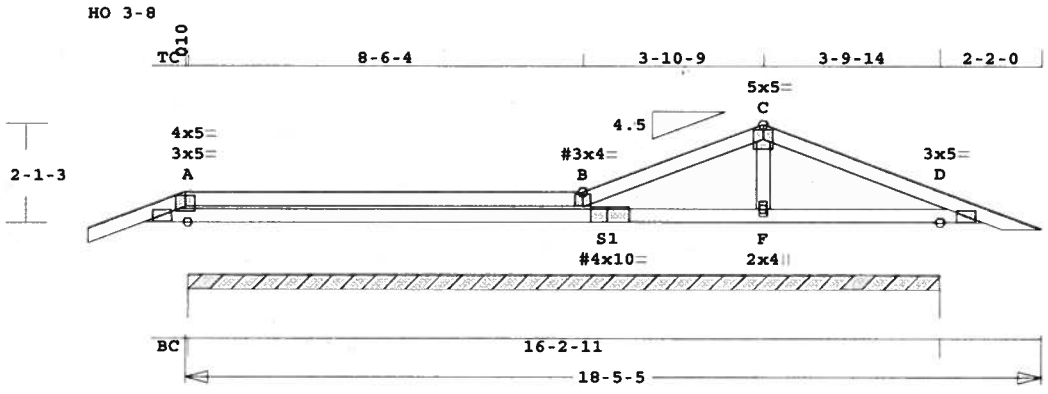
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 178 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	T9	1	SP	180505	4.5	10	2- 2- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 77.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
EX A -B		2x 4	SP-2400
BC	0.78	2x 4	SP-#2
WB	0.23	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	18- 5- 5
BC	Cont.	0- 0- 0	18- 5- 5

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg	0-11- 6	to	17- 2- 1
	1534	411	Hz =	5

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.64	1022 T	0.07	0.57
B -C	0.44	1095 T	0.19	0.25
C -D	0.43	1114 T	0.19	0.24
--------Bottom Chords---------				
A -S1	0.78	4 C	0.00	0.78
S1-F	0.59	0 T	0.00	0.59
F -D	0.26	7 C	0.00	0.26
------------Webs------------				
F -C	0.23	1077 C		

LL Defl -0.24" in A -S1 L/823
TL Defl -0.53" in A -S1 L/368
Shear // Grain in A -B 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.72
A	LOCK	4.0x 5.0	Ctr	-1.1	0.64
B	LOCK	3.0x 4.0	Ctr	-0.2	0.55
C	LOCK	5.0x 5.0	Ctr	Ctr	0.51
D	LOCK	3.0x 5.0	Ctr	Ctr	0.72
S1	LOCK	4.0x10.0	0.6	0.3	0.88
F	LOCK	2.0x 4.0	Ctr	Ctr	0.77

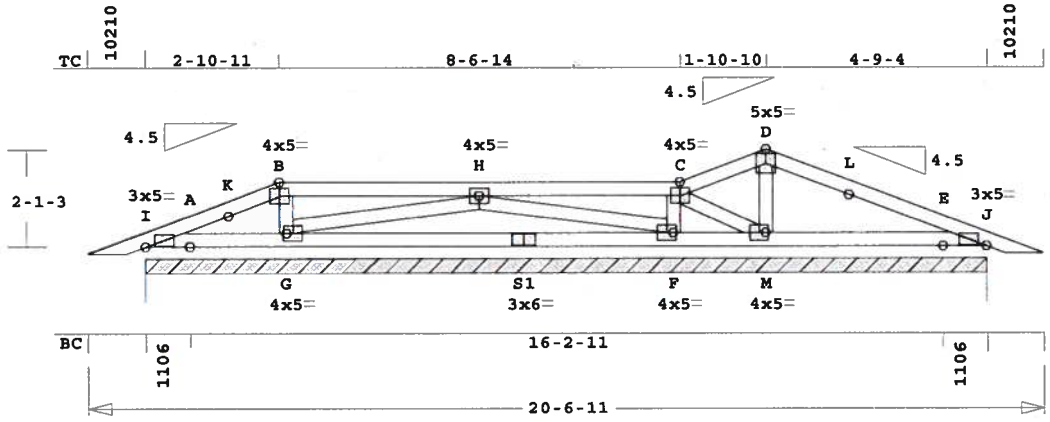
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1077 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	T8	1	SP	200611	4.5	1- 2-10	1- 2-10	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 99.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.17	2x 4	SP-#2
BC	0.14	2x 4	SP-#2
WB	0.17	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 6-11
BC	Cont.	0- 0- 0	20- 6-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg	0- 0- 0 to	18- 1- 7	
	1605	357	Hz =	4

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
I -B	0.04	91	C	0.00	0.04
B -H	0.17	94	T	0.00	0.17
H -C	0.17	186	C	0.00	0.17
C -D	0.08	156	T	0.02	0.06
D -J	0.17	137	C	0.00	0.17
--------Bottom Chords---------					
I -G	0.11	23	C	0.00	0.11
G -S1	0.11	0	T	0.00	0.11
S1-F	0.11	0	T	0.00	0.11
F -M	0.11	0	T	0.00	0.11
M -J	0.14	109	C	0.00	0.14
------------Webs------------					
G -B	0.03	167	C		
G -H	0.17	589	C		
H -F	0.14	490	C		
F -C	0.04	175	C		
C -M	0.02	90	T		
M -D	0.03	132	T		

LL Defl -0.01" in G -S1 L/999
TL Defl -0.02" in M -E L/999
Shear // Grain in B -H 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

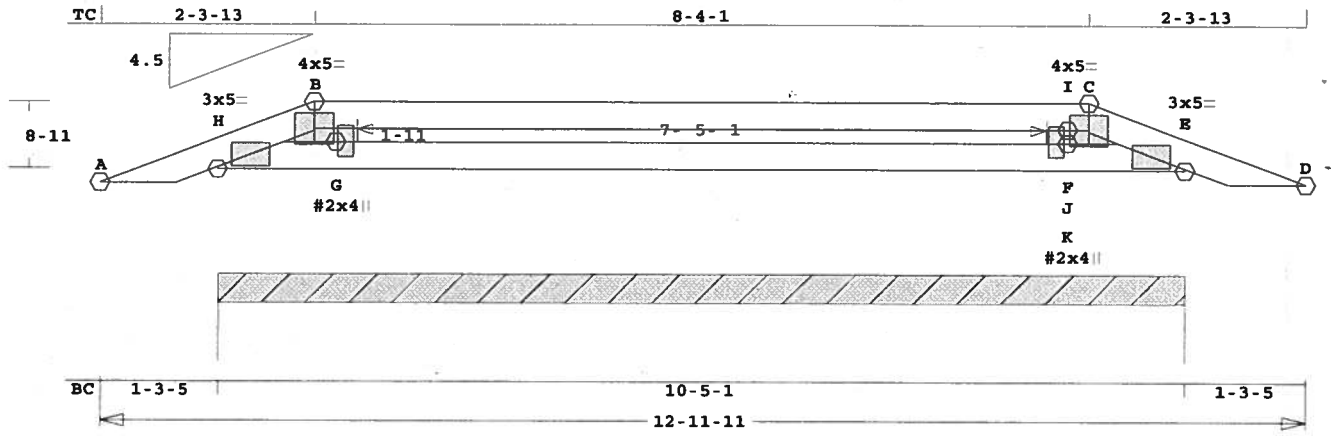
Jt	Type	Plt Size	X	Y	JSI
I	LOCK	3.0x 5.0	Ctr	Ctr	0.72
B	LOCK	4.0x 5.0	Ctr	Ctr	0.82
H	LOCK	4.0x 5.0	Ctr	Ctr	0.76
C	LOCK	4.0x 5.0	Ctr	Ctr	0.82
D	LOCK	5.0x 5.0	Ctr	Ctr	0.51
J	LOCK	3.0x 5.0	Ctr	Ctr	0.72
G	LOCK	4.0x 5.0	Ctr	Ctr	0.76
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.76
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 589 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T7*	1	PBH	121111	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
WB	0.05	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 997	0- 0- 0 to 329	Hz =	10- 5- 1 1

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
H -B	0.13	145	T	0.02	0.11
B -I	0.49	35	T	0.00	0.49
I -E	0.13	145	T	0.02	0.11
Bottom Chords					
H -G	0.29	126	T	0.00	0.29
G -K	0.29	0	T	0.00	0.29
K -E	0.29	126	T	0.00	0.29
Webs					
G -B	0.05	613	C		
K -I	0.05	613	C		

LL Defl -0.06" in G -F L/999
TL Defl -0.11" in G -F L/999
Shear // Grain in B -B 0.42

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt	Size	X	Y	JSI
H	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	Ctr	0.73
I	LOCK	4.0x 5.0	Ctr	Ctr	0.73
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
G#	LOCK	2.0x 4.0	1.2	0.1	0.45
K#	LOCK	2.0x 4.0-1.5		0.2	0.45

= Plate Monitor used

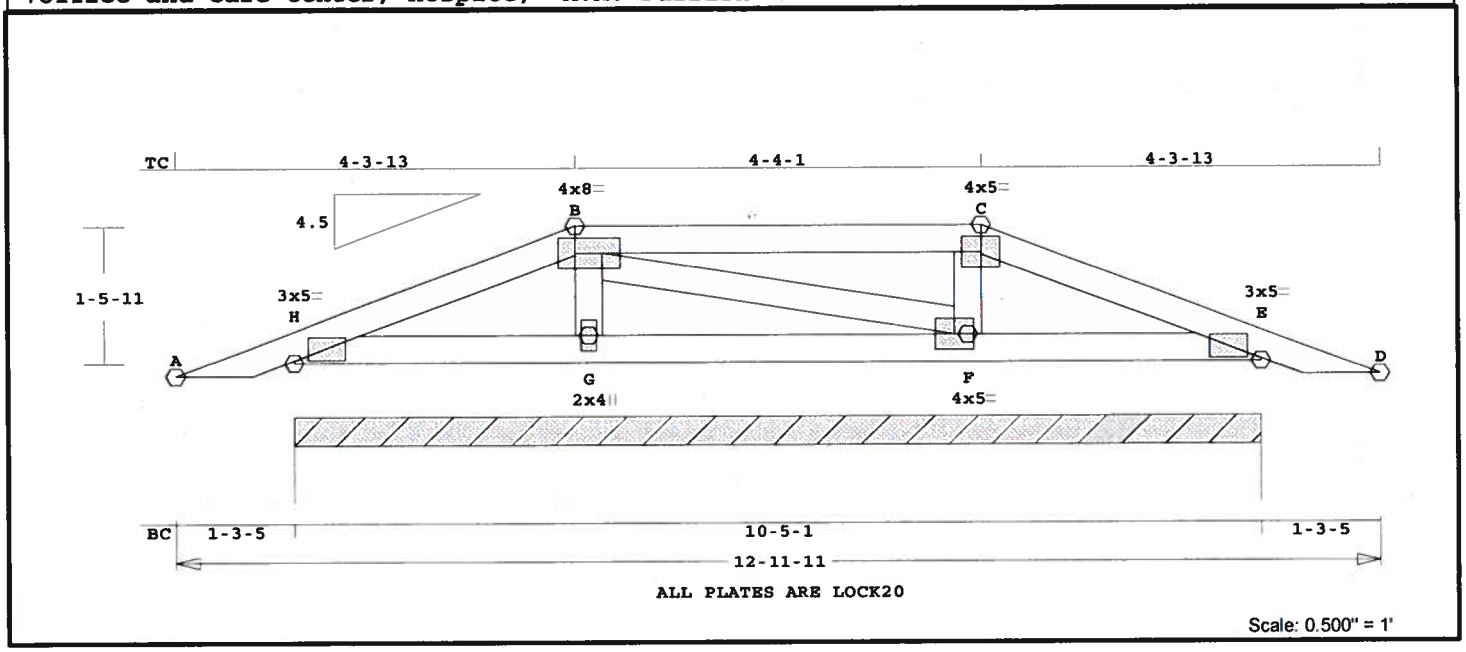
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 613 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T6*	1	PBH	121111	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 10- 5- 1			
	997	329	Hz =	3

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
H -B	0.07	137 T	0.01	0.06
B -C	0.13	165 T	0.00	0.13
C -E	0.07	130 T	0.01	0.06
--------Bottom Chords---------				
H -G	0.07	30 T	0.00	0.07
G -F	0.07	0 T	0.00	0.07
F -E	0.07	30 T	0.00	0.07

----------Webs----------			
G -B	0.04	197 C	
B -F	0.01	47 C	
F -C	0.04	177 T	

LL Defl 0.00" in G -F L/999
TL Defl -0.01" in G -F L/999
Shear // Grain in B -C 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

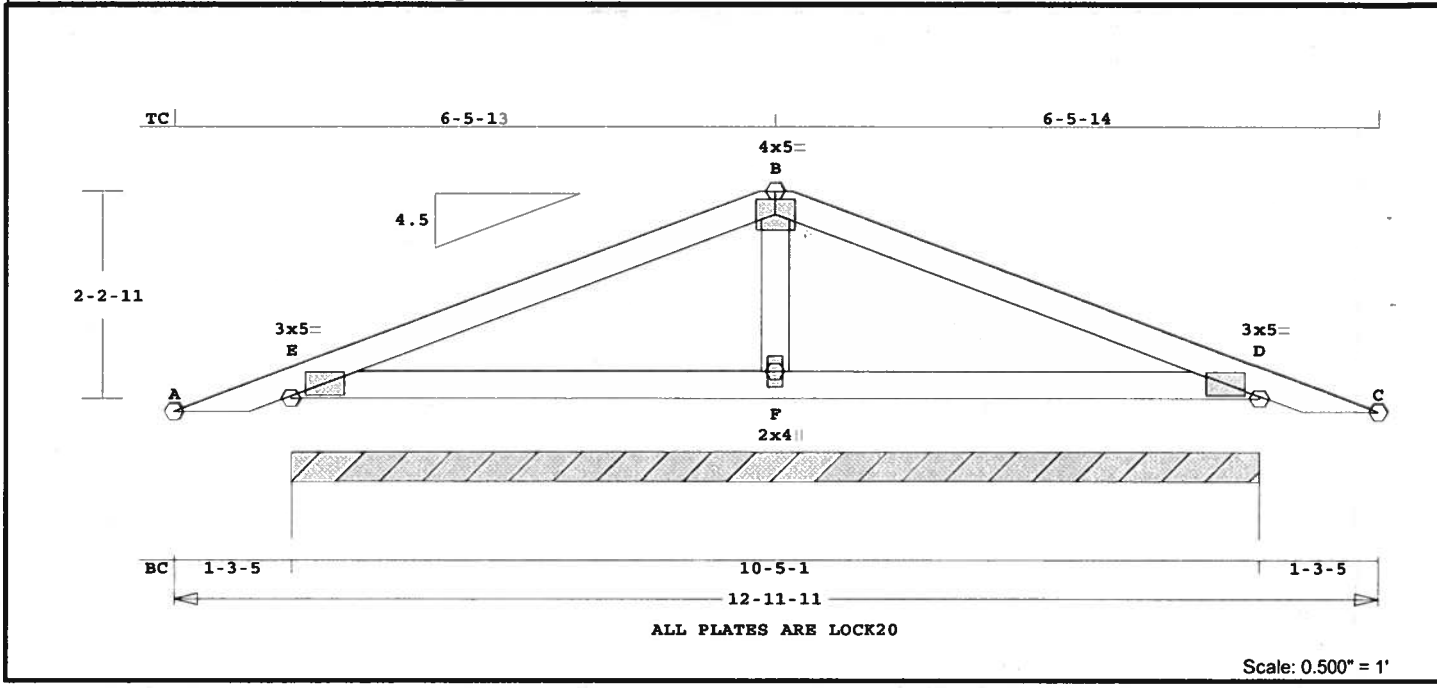
Jt	Type	Plt Size	X	Y	JSI
H	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 8.0	Ctr	Ctr	0.68
C	LOCK	4.0x 5.0	Ctr	Ctr	0.68
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.76

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 197 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T5*	1	PB	121111	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 10- 5- 1			
	997	329	Hz =	4

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
E -B	0.21	347 C	0.03	0.18
B -D	0.21	347 C	0.03	0.18
--------Bottom Chords---------				
E -F	0.17	61 C	0.00	0.17
F -D	0.17	61 C	0.00	0.17

----------Webs----------
F -B 0.02 90 T

LL Defl -0.01" in F -D L/999
TL Defl -0.02" in F -D L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

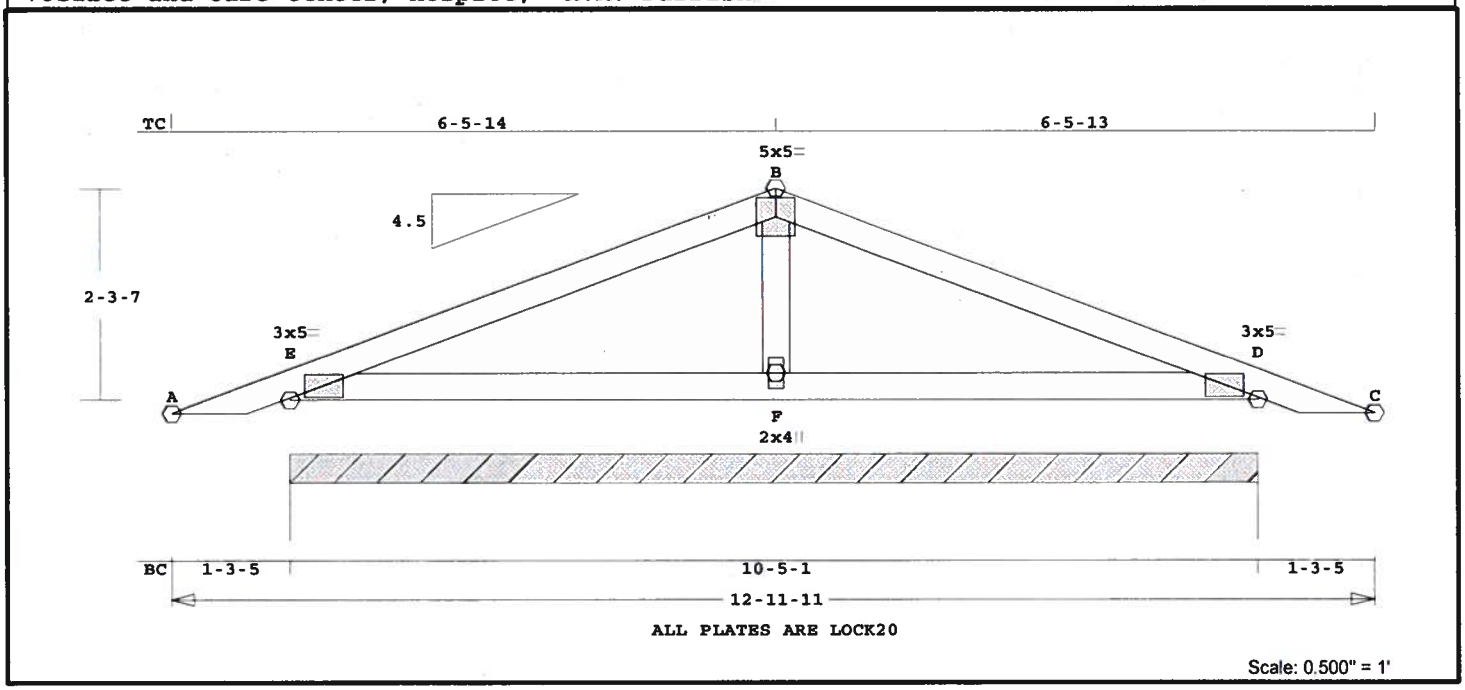
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	Ctr	0.50
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 347 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T4*	4	PB	121111	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 10- 5- 1			
	997	329	Hz =	4

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
E -B	0.21	347	C 0.03	0.18
B -D	0.21	347	C 0.03	0.18
Bottom Chords				
E -F	0.17	61	C 0.00	0.17
F -D	0.17	61	C 0.00	0.17

Webs

F -B	0.02	91	T

LL Defl -0.01" in F -D L/999
TL Defl -0.02" in F -D L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

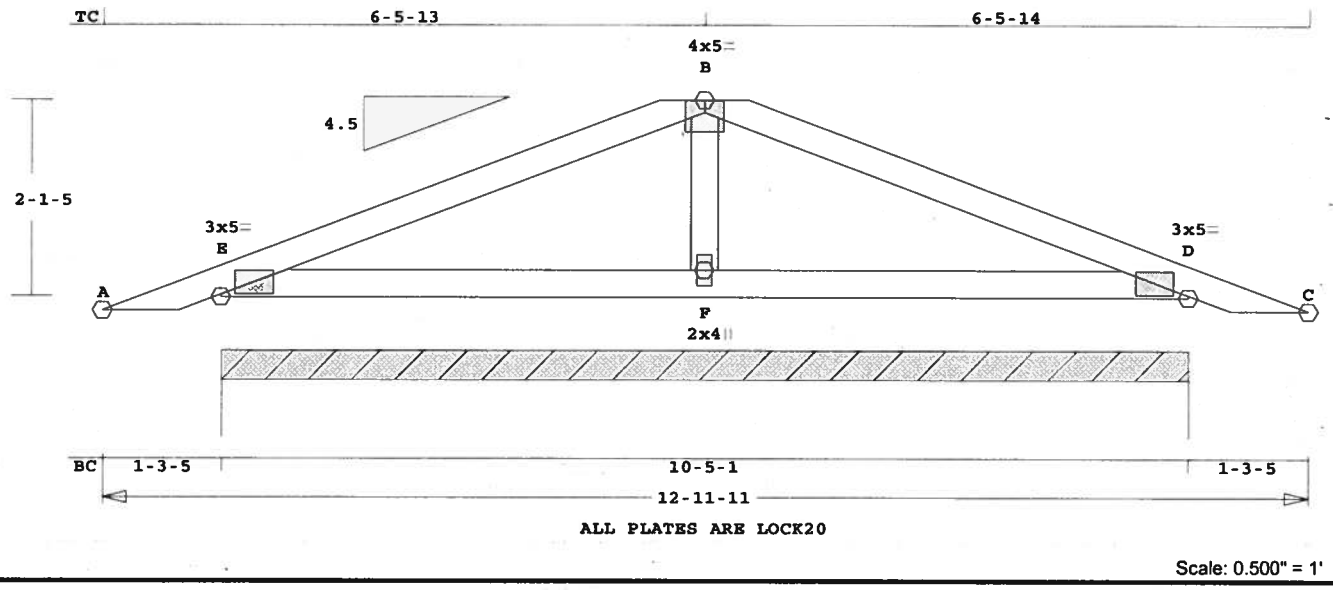
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	5.0x 5.0	Ctr	Ctr	0.42
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 347 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T3*	1	PB	121111	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12-11-11
BC Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0		to 10- 5- 1	
	997	329	Hz =	4

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
E -B	0.21	355	C	0.03	0.18
B -D	0.21	355	C	0.03	0.18
--------Bottom Chords---------					
E -F	0.17	61	C	0.00	0.17
F -D	0.17	61	C	0.00	0.17

----------Webs----------

F -B	0.02	84	T

LL Defl -0.01" in F -D L/999
TL Defl -0.02" in F -D L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

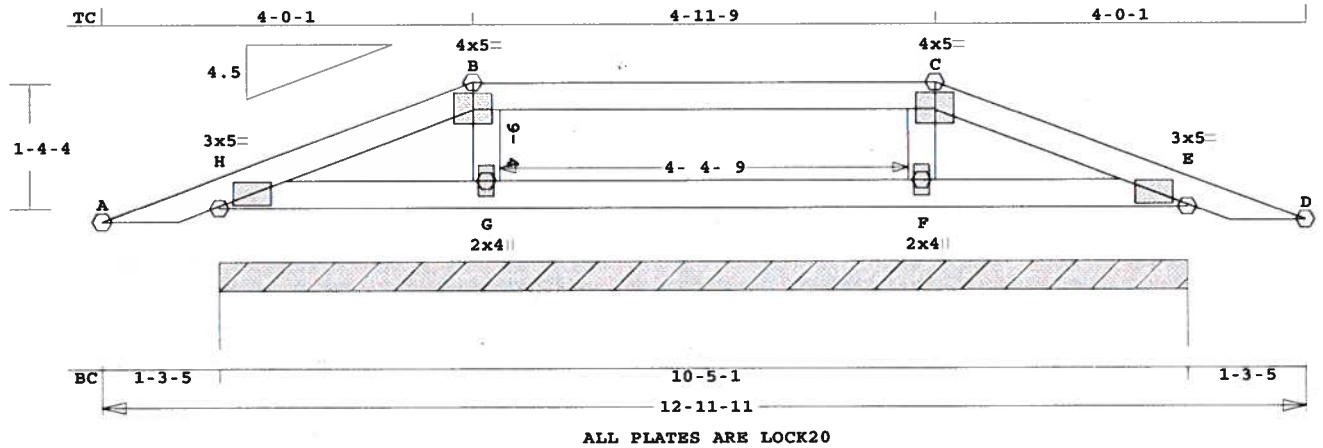
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	-0.4	0.58
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 355 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T2*	1	PBH	121111	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.19	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12-11-11
BC	Cont.	0- 0- 0	12-11-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 997	0- 0- 0 to 329	Hz =	10- 5- 1 2

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
H -B	0.05	88	T	0.00	0.05
B -C	0.19	124	T	0.00	0.19
C -E	0.05	88	T	0.00	0.05
Bottom Chords					
H -G	0.09	21	T	0.00	0.09
G -F	0.09	0	T	0.00	0.09
F -E	0.09	21	T	0.00	0.09
Webs					
G -B	0.05	213	C		
F -C	0.05	213	C		

LL Defl -0.01" in G -F L/999
TL Defl -0.02" in G -F L/999
Shear // Grain in B -C 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
H	LOCK	3.0x 5.0	Ctr	Ctr	0.60
B	LOCK	4.0x 5.0	Ctr	Ctr	0.68
C	LOCK	4.0x 5.0	Ctr	Ctr	0.68
E	LOCK	3.0x 5.0	Ctr	Ctr	0.60
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

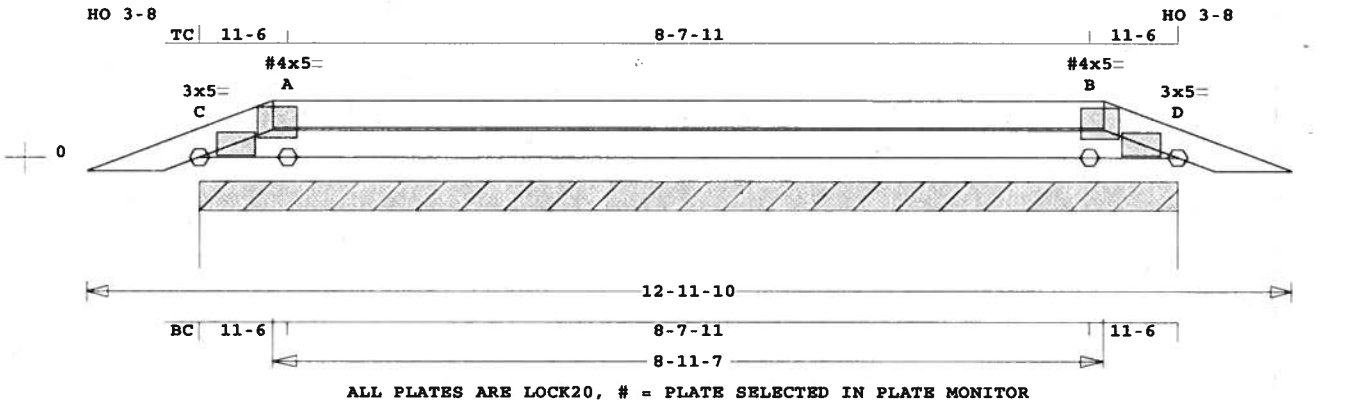
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed

TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 213 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*T1*	1	SP	81107	0	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.94	2x 4	SP-#2
BC	0.37	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8-11- 7
BC	Cont.	0- 0- 0	8-11- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0		to 10- 6- 7	
	872	320	Hz =	0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.94	1685 C	0.09 0.85
--------Bottom Chords---------			
C -D	0.37	106 C	0.00 0.37

LL Defl -0.11" in A -B L/999
TL Defl -0.24" in A -B L/530
Shear // Grain in A -B 0.33

Plates for each ply each face.

ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
C	LOCK	3.0x 5.0	Ctr	Ctr	0.60
A#	LOCK	4.0x 5.0	0.5-0.9		0.51
B#	LOCK	4.0x 5.0	-0.6-1.2		0.53
D	LOCK	3.0x 5.0	Ctr	Ctr	0.60

= Plate Monitor used

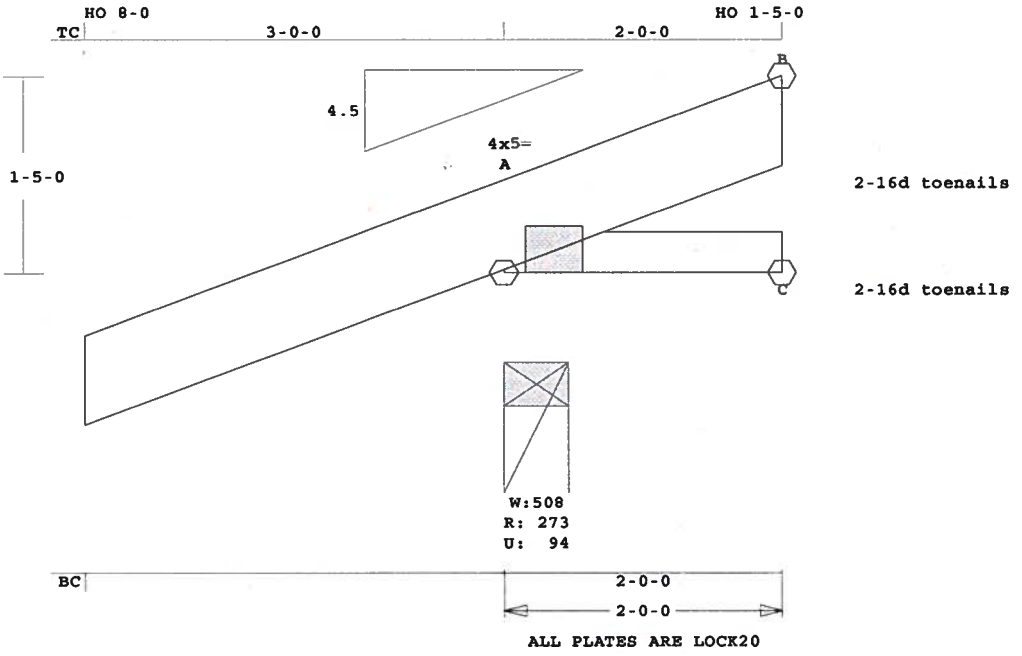
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1685 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R250*	3	MONO	20000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 8	SP-2400
BC	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	2- 0- 0
BC	Cont.	0- 0- 0	2- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	274	94	3- 8	1- 8
			Hz =	34
B	62	41	1- 8	1- 8
C	25	0	1- 8	1- 8
			Hz =	18

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	44	C
--------Bottom Chords---------			
A -C	0.01	18	T 0.00 0.01

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.04

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0 Ctr	0.1	0.84	

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

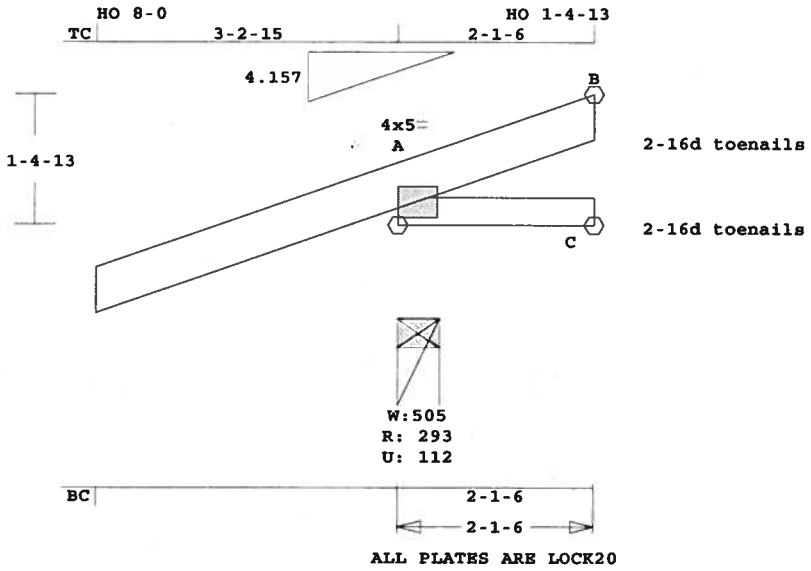
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 44 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R249	2	MONO	20106	4.157	3- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 20.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 6	SP-#2
BC	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	2- 1- 6
BC Cont.	0- 0- 0	2- 1- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	294	113	5- 5	1- 8
			Hz =	33
C	32	0	1- 8	1- 8
B	63	45	1- 8	1- 8
			Hz =	18

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.03	44 C	0.00 0.03
--------Bottom Chords---------			
A -C	0.02	0 T	0.00 0.02

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.6	0.2	0.68

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

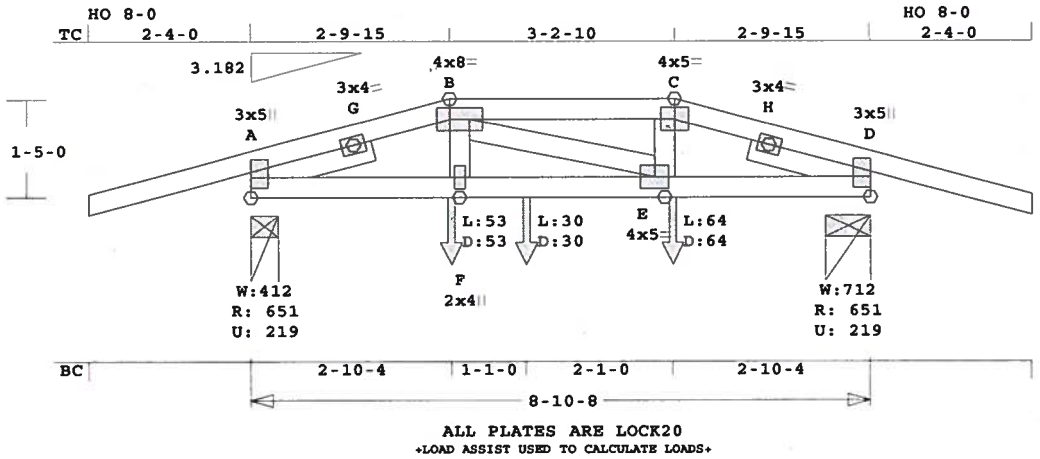
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 44 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc./Online Plus™ © 1996-2006 Version 19.0.009 Engineering - Portrait 3/16/2006 1:17:51 PM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R248	1	HIPP	81008	3.182	2- 4- 0	2- 4- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 58.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.05	2x 4	SP-#3
SL	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8-10- 8
BC Cont.	0- 0- 0	8-10- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	8.9'
BC V 0	20	0.0'	8.9'
BC V 53	53	2.9'	CL-LB
BC V 30	30	3.9'	CL-LB
BC V 64	64	6.0'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	651	219	4-12	1- 8
D	652	219	7-12	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -G	0.17	683 C	0.08	0.09
G -B	0.21	950 C	0.12	0.09
B -C	0.24	934 C	0.12	0.12
C -H	0.22	980 C	0.12	0.10
H -D	0.19	684 C	0.00	0.19
Bottom Chords				
A -F	0.32	907 T	0.16	0.16
F -E	0.33	896 T	0.16	0.17
E -D	0.33	934 T	0.17	0.16
Webs				
F -B	0.05	196 T		
B -E	0.01	40 T		
E -C	0.05	210 T		
Sliders				
A -G	0.08	341 C		
H -D	0.08	376 C		

TL Defl -0.05" in F -E L/999
LL Defl -0.02" in F -E L/999
Shear // Grain in H -H 0.16

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

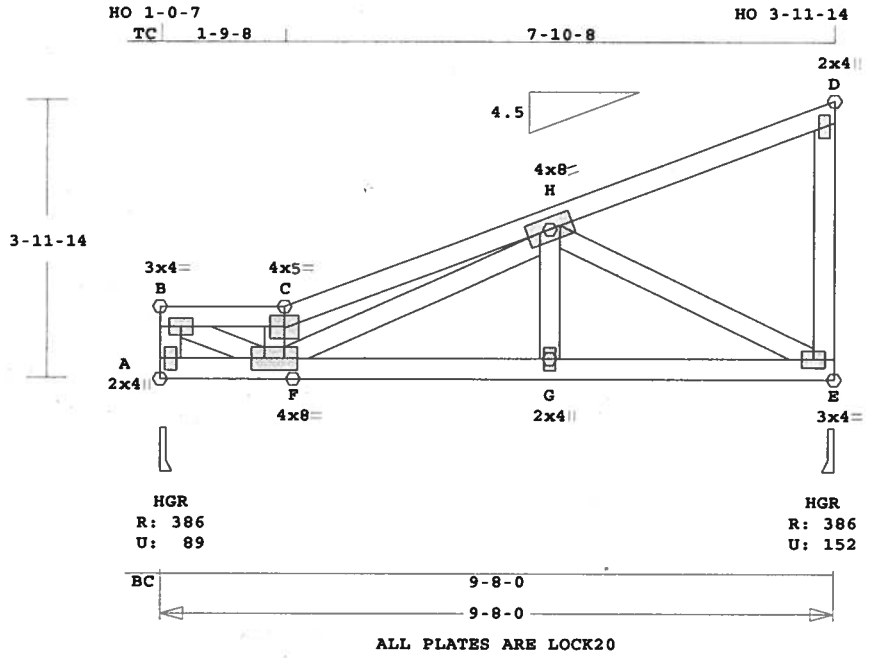
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.2	0.71
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 8.0	Ctr	Ctr	0.65
C	LOCK	4.0x 5.0	Ctr	Ctr	0.65
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	3.0x 5.0-1.5	0.2		0.71
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	4.0x 5.0	Ctr	Ctr	0.77

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 980 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R247*	1	SP	90800	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 67.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	-Size-	----Lumber----
TC	0.23	2x 4	SP-#2
BC	0.14	2x 4	SP-#2
WB	0.23	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 8- 0
BC Cont.	0- 0- 0	9- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	387	89	3- 8	1- 0
			Hz =	149
E	387	153	3- 8	1- 0
			Hz =	84

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -C	0.06	653 C	0.03	0.03
C -H	0.23	685 C	0.04	0.19
H -D	0.19	67 C	0.00	0.19
--------Bottom Chords---------				
A -F	0.04	228 C	0.00	0.04
F -G	0.14	464 T	0.07	0.07
G -E	0.14	464 T	0.04	0.10
------------Webs------------				
A -B	0.05	352 C	WindLd	
B -F	0.23	729 T		
F -C	0.06	406 C		
F -H	0.06	215 T		
G -H	0.05	191 T		
H -E	0.16	529 C		
E -D	0.04	176 T		

LL Defl -0.01" in F -G L/999
TL Defl -0.03" in F -G L/999
Shear // Grain in H -D 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.53
C	LOCK	4.0x 5.0	Ctr	Ctr	0.67
H	LOCK	4.0x 8.0	Ctr	Ctr	0.78
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 8.0	Ctr	Ctr	0.59
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	3.0x 4.0	Ctr	Ctr	0.53

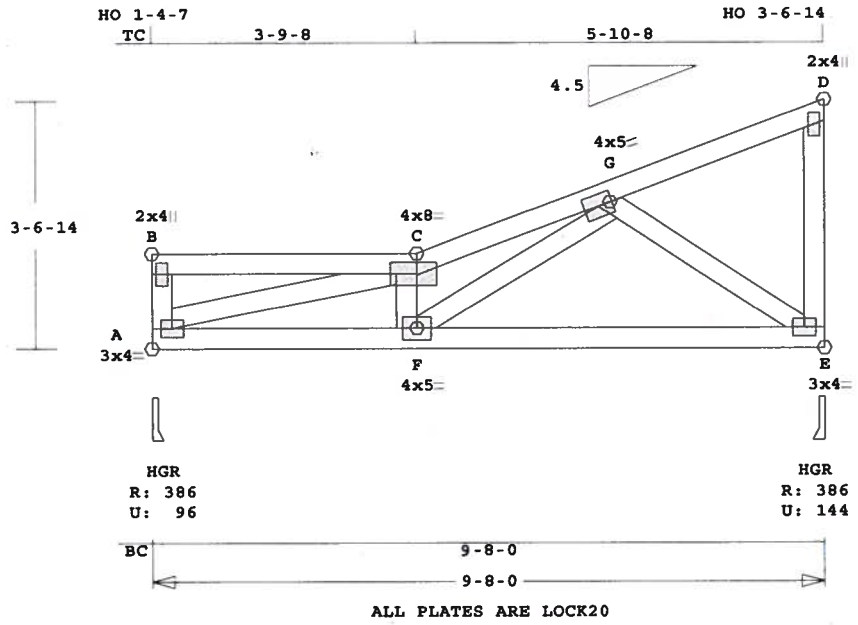
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 685 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R246*	1	SP	90800	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 65.0 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.21	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 8- 0
BC Cont.	0- 0- 0	9- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	387	97	3- 8	1- 0
			Hz =	124
E	387	145	3- 8	1- 0
			Hz =	73

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.15	20 T	0.00 0.15
C -G	0.20	909 C	0.07 0.13
G -D	0.13	51 C	0.00 0.13
--------Bottom Chords---------			
A -F	0.26	905 T	0.10 0.16
F -E	0.22	422 C	0.03 0.19
-------------Webs-------------			
A -B	0.02	119 T	WindLd
A -C	0.21	938 C	
F -C	0.07	308 T	
F -G	0.20	626 T	
G -E	0.13	521 T	
E -D	0.03	132 T	

LL Defl -0.05" in F -E L/999
TL Defl -0.10" in F -E L/999
Shear // Grain in G -D 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

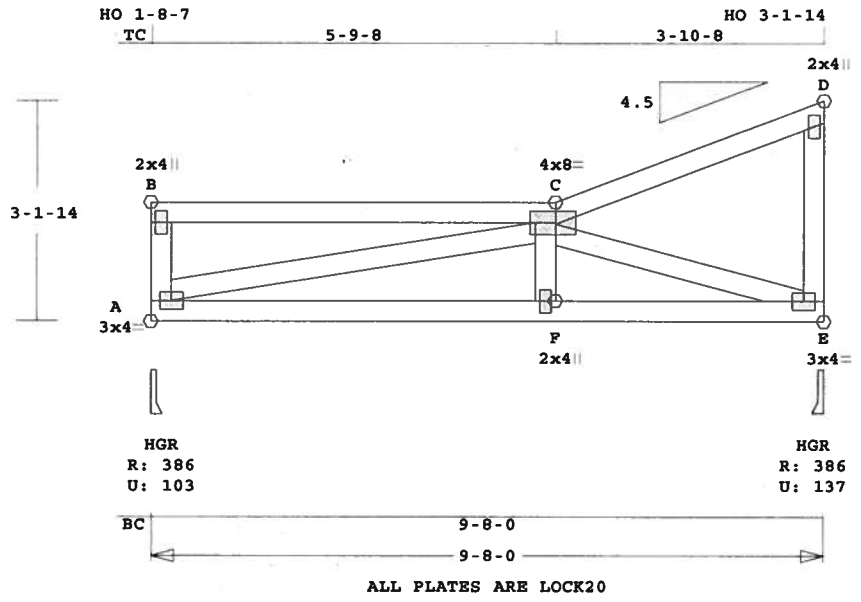
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.67
G	LOCK	4.0x 5.0	Ctr	Ctr	0.75
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.54
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 4.0	Ctr	Ctr	0.53

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 938 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R245*	1	SP	90800	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 63.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	Size	Lumber
TC	0.33	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.39	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 8- 0
BC	Cont.	0- 0- 0	9- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	387	104	3- 8	1- 0
			Hz =	99
E	387	138	3- 8	1- 0
			Hz =	62

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
B -C	0.33	29 T	0.00	0.33
C -D	0.20	71 C	0.00	0.20
Bottom Chords				
A -F	0.26	829 T	0.09	0.17
F -E	0.23	829 T	0.09	0.14
Webs				
A -B	0.05	180 T	WindLd	
A -C	0.39	847 C		
F -C	0.06	227 T		
C -E	0.23	865 C		
E -D	0.04	186 T		

LL Defl -0.04" in A -F L/999
TL Defl -0.08" in A -F L/999
Shear // Grain in B -C 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.68
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.54
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	3.0x 4.0	Ctr	Ctr	0.53

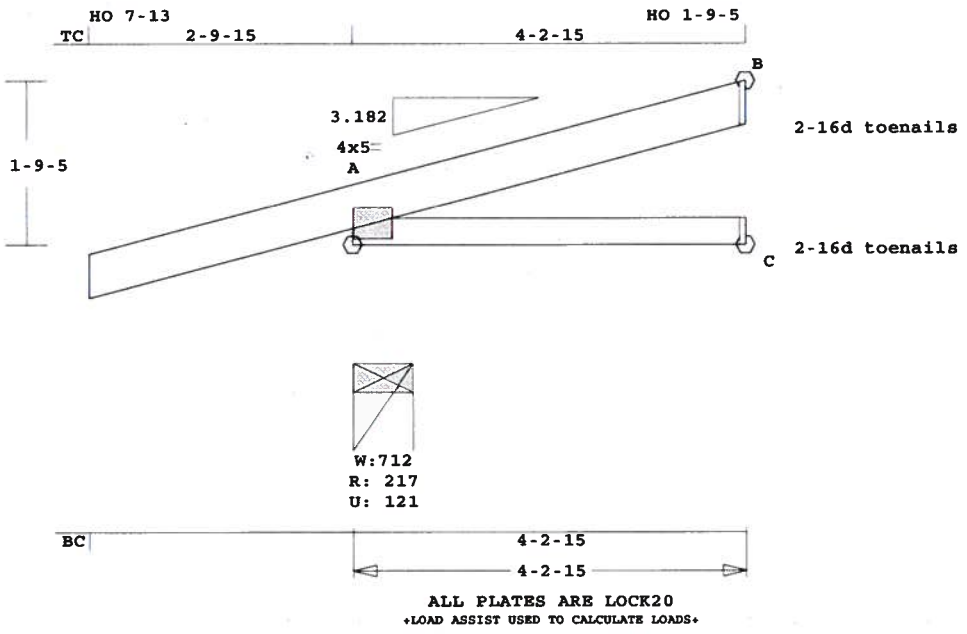
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 865 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R244*	4	KING	40215	3.182	2- 9-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.05	2x 6	SP-#2
BC	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 2-15
BC Cont.	0- 0- 0	4- 2-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	4.2'
BC V	0	20	0.0'	4.2'
TC V	-40	-20	0.0'	
	-12	-6		4.2'
BC V	0	-20	0.0'	
	0	-6		4.2'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	217	121	7-12	1- 8
			Hz =	18
C	36	0	3- 8	1- 8
B	64	41	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.05	34 C	0.00	0.05
--------Bottom Chords---------				
A -C	0.03	0 T	0.00	0.03

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71

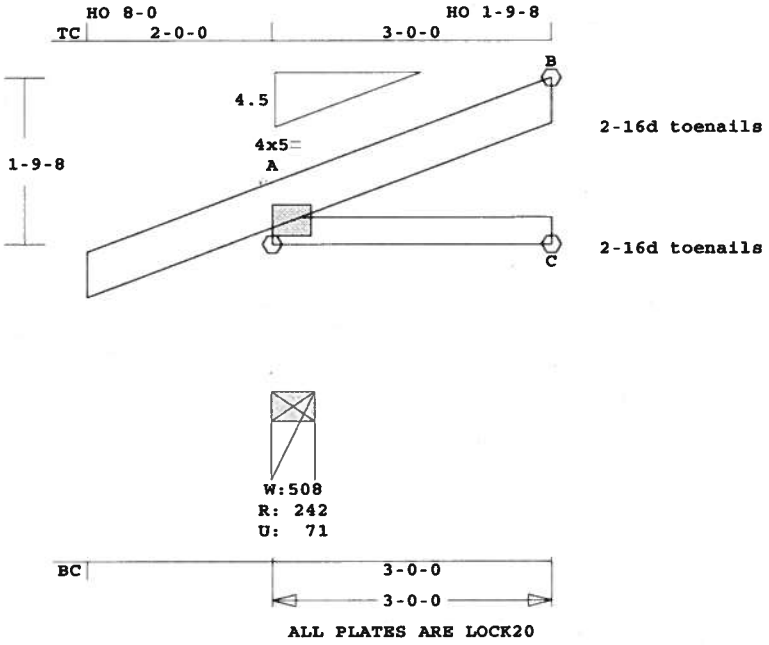
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 3- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 34 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R243*	6	JACK	30000	4.5	2- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.06	2x 6	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	243	72	5- 8	1- 8
			Hz = 51	
C	51	0	3- 8	1- 8
B	97	68	3- 8	1- 8
			Hz = 27	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.06	72 C	0.00 0.06
--------Bottom Chords---------			
A -C	0.05	0 T	0.00 0.05

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

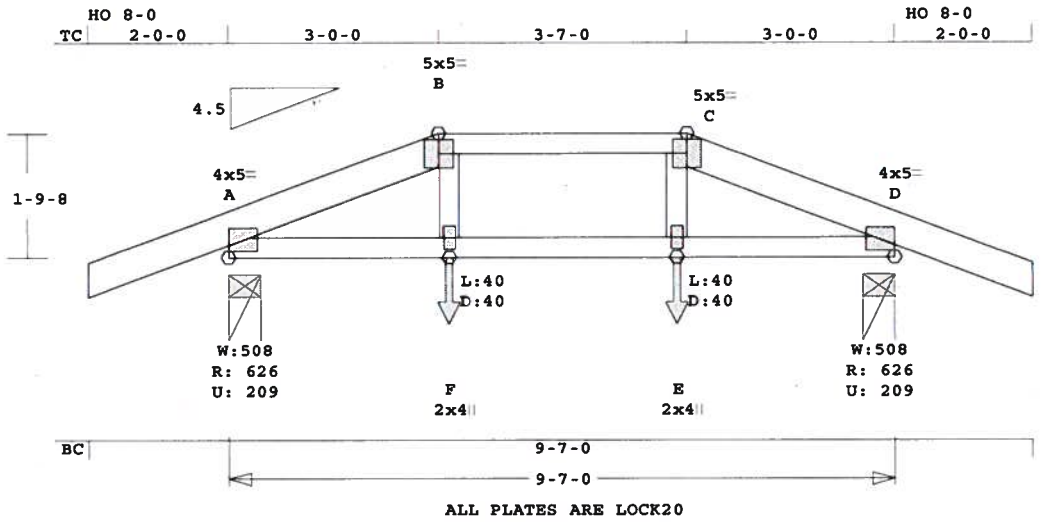
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 72 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R242*	2	HIPP	90700	4.5	2- 0- 0	2- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 60.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 6	SP-#2
EX B -C		2x 4	SP-#2
BC	0.23	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 7- 0
BC Cont.	0- 0- 0	9- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.6'
BC V	0	20	0.0'	9.6'
TC V	10	5	3.0'	6.6'
BC V	0	5	3.1'	6.4'
BC V	40	40	3.1'	CL-LB
BC V	40	40	6.4'	CL-LB

Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	626	210	5- 8	1- 8
D	626	210	5- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI-Bnd	
----------Top Chords----------					
A -B	0.16	830	C	0.07	0.09
B -C	0.20	756	C	0.09	0.11
C -D	0.16	830	C	0.07	0.09
--------Bottom Chords---------					
A -F	0.23	756	T	0.13	0.10
F -E	0.21	756	T	0.13	0.08
E -D	0.23	756	T	0.13	0.10
------------Webs------------					
F -B	0.04	186	T		
E -C	0.04	186	T		

TL Defl -0.04" in F -E L/999
LL Defl -0.02" in F -E L/999
Shear // Grain in B -C 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	5.0x 5.0	Ctr	Ctr	0.53
C	LOCK	5.0x 5.0	Ctr	Ctr	0.53
D	LOCK	4.0x 5.0	-0.7	0.3	0.65
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

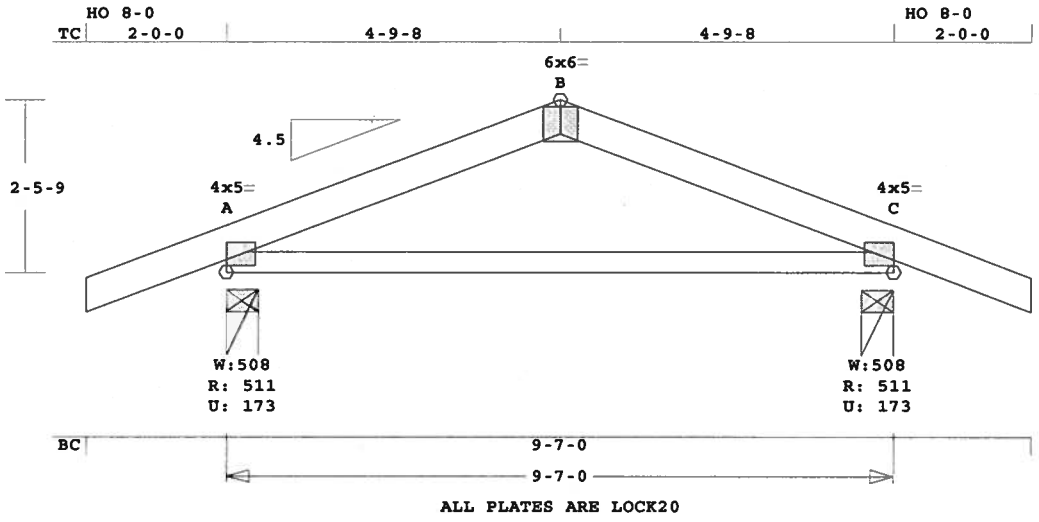
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 3- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 830 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Plus 9 Wind Load Case(s)

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R241*	5	TR	90700	4.5	2- 0- 0	2- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 60.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 6	SP-#2
BC	0.42	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 7- 0
BC	Cont.	0- 0- 0	9- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	511	173	5- 8	1- 8
C	511	173	5- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.12	424 C	0.03	0.09
B -C	0.12	424 C	0.03	0.09
--------Bottom Chords---------				
A -C	0.42	401 T	0.03	0.39

TL Defl -0.20" in A -C L/528
LL Defl -0.10" in A -C L/999
Shear // Grain in A -C 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

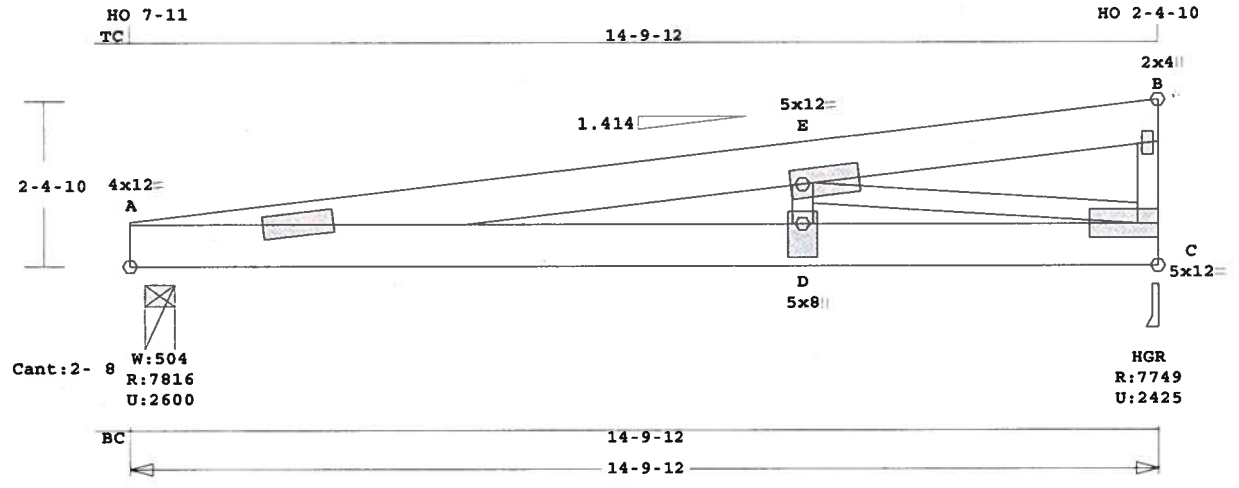
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr-1.2	0.3	0.33
C	LOCK	4.0x 5.0	-0.7	0.3	0.65

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 424 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R240*	1*3P	MONO	140912	1.414	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 124.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.50	2x 8	SP-2400
BC	0.73	2x 8	SP-2400
WB	0.68	2x 4	SP-#3
EX E -C		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	14- 9-12
BC Cont.		0- 0- 0	14- 9-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.8'
BC V	485	505	0.0'	14.8'

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	7816	2600	5- 4	2- 3
			Hz =	105
C	7749	2426	3- 8	2- 2
			Hz =	95

Membr	CSI	P Lbs	Axl-CSI-Bnd	
		Top Chords		
A -E	0.50	21674 C	0.06	0.44
E -B	0.06	30 C	0.00	0.06
		Bottom Chords		
A -D	0.73	21655 T	0.25	0.48
D -C	0.46	21655 T	0.25	0.21
		Webs		
D -E	0.68	6406 T		
E -C	0.74	22250 C		
C -B	0.01	312 C		

TL Defl -0.55" in A -D L/302
LL Defl -0.28" in A -D L/605
Shear // Grain in A -D 0.70

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x12.0	Ctr	Ctr	0.89
E	LOCK	5.0x12.0	2.2	0.3	0.91
B	LOCK	2.0x 4.0	Ctr	Ctr	0.64
D	LOCK	5.0x 8.0	Ctr	-1.8	0.92
C	LOCK	5.0x12.0	-2.5	Ctr	0.91

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 50- 6- 8
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

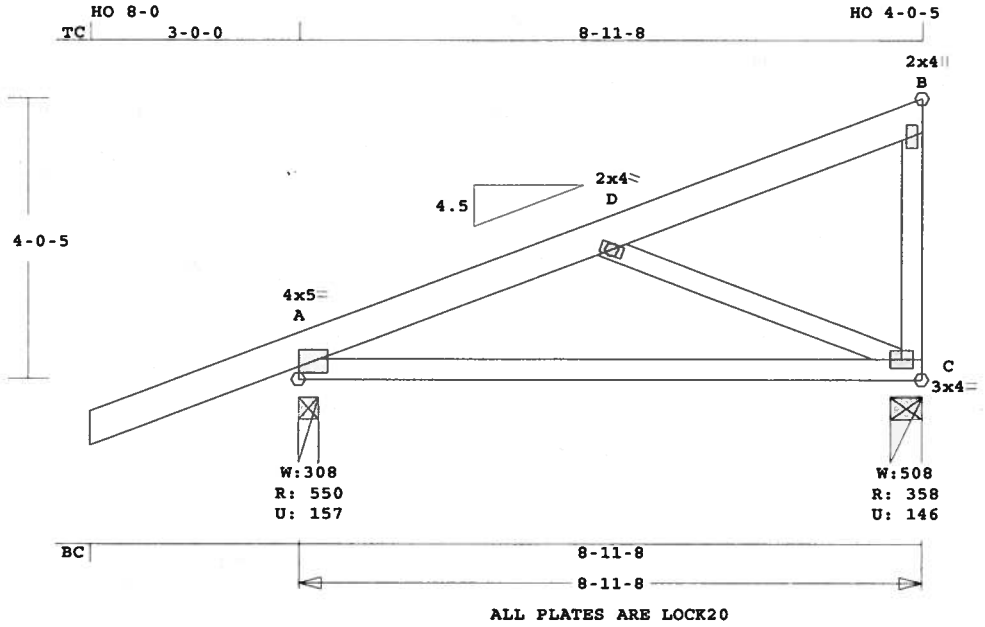
	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	5	7.5	0
WB 1	8	8	

Web Connection Exception -- Use 4" spacing for screws or nails on the following webs
E- C
No bolts in 2x4s or smaller.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 22250 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R239	2	MONO	81108	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.14	2x 6	SP-#2
BC	0.53	2x 4	SP-#2
WB	0.16	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8-11- 8
BC Cont.	0- 0- 0	8-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	550	158	3- 8	1- 8
			Hz =	151
C	358	146	5- 8	1- 8
			Hz =	81

Membr	CSI	P Lbs	Axl-CSI-Bnd
--------Top Chords---------			
A -D	0.14	432 C	0.00 0.14
D -B	0.13	85 C	0.00 0.13
--------Bottom Chords---------			
A -C	0.53	418 T	0.03 0.50
----------Webs----------			
D -C	0.16	449 C	
C -B	0.05	222 T	

TL Defl -0.28" in A -C L/354
LL Defl -0.14" in A -C L/712
Shear // Grain in A -C 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

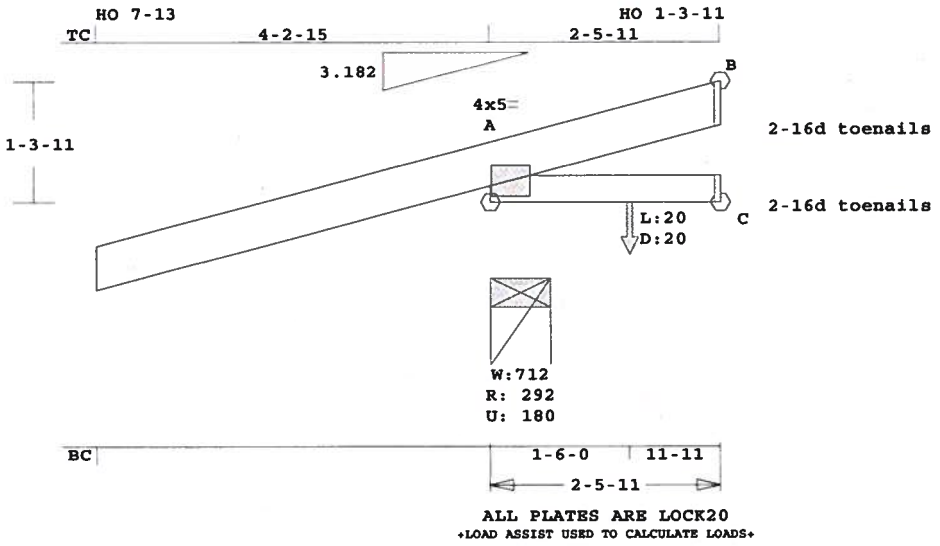
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
B	LOCK	2.0x 4.0	Ctr	Ctr	0.39
C	LOCK	3.0x 4.0	Ctr	Ctr	0.52

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 449 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R238*	1	KING	20511	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.01	2x 6	SP-#2
BC	0.03	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	2- 5-11
BC	Cont.	0- 0- 0	2- 5-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	2.5'
BC V 0	20	0.0'	2.5'
TC V -40	-20	0.0'	
-31	-15		2.5'
BC V 0	-20	0.0'	
0	-15		2.5'
BC V 20	20	1.5'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	293	181	7-12	1- 8
C	27	4	3- 8	1- 8
B	19	10	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.01	8 C	0.00 0.01
--------Bottom Chords---------			
A -C	0.03	0 T	0.00 0.03

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -C 0.02

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71

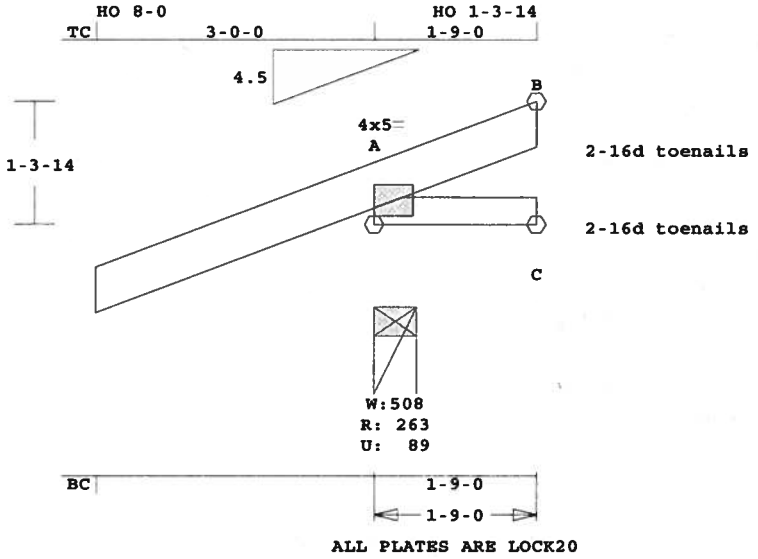
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 1- 7- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R237*	5	JACK	10900	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 18.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.02	2x 6	SP-#2
BC	0.01	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 9- 0
BC Cont.	0- 0- 0	1- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	264	90	3- 8	1- 8
			Hz =	30
B	52	39	1- 8	1- 8
C	26	0	1- 8	1- 8
			Hz =	16

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.02	41 C	0.00 0.02
--------Bottom Chords---------			
A -C	0.01	15 T	0.00 0.01

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.05

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

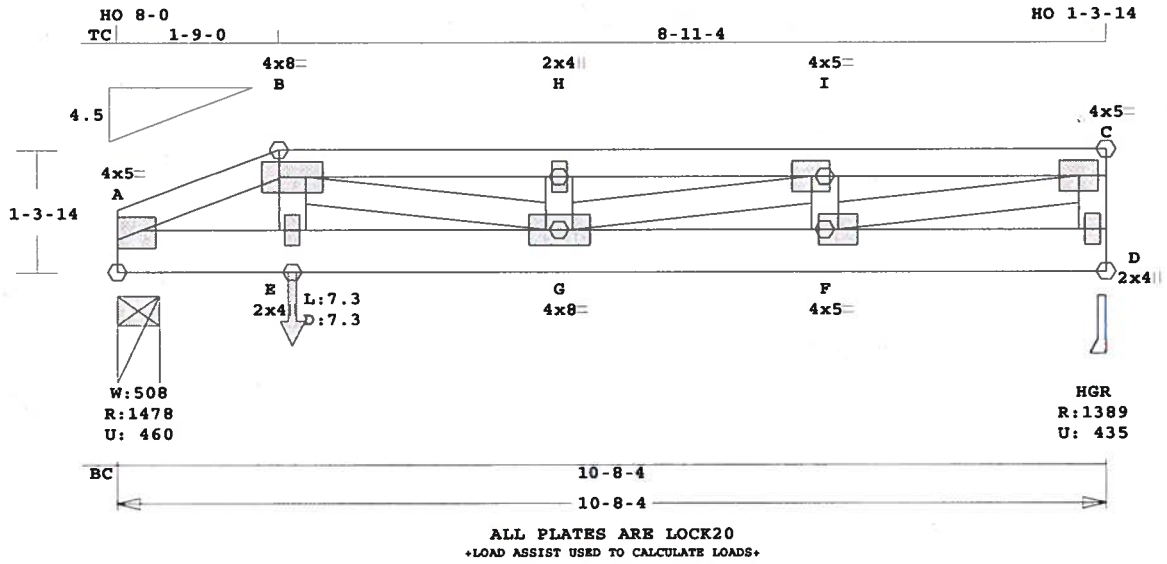
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 41 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc./Online Plus™ © 1996-2006 Version 19.0.009 Engineering - Portrait 3/16/2006 1:17:47 PM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R236*	1*2P	HHIP	100804	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 70.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.39	2x 6	SP-#2
WB	0.50	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 8- 4
BC	Cont.	0- 0- 0	10- 8- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	10.7'
BC V	0	20	0.0'	10.7'
BC V	100	100	0.0'	10.7'
TC V	-4	-2	1.8'	9.7'
TC V	-40	-20	9.7'	10.7'
BC V	0	-2	1.9'	9.7'
BC V	0	-20	9.7'	10.7'
BC V	7	7	1.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd			
	Lbs	Lbs	In-Sx		In-Sx	
A	1478	460	5- 8		1- 0	
			Hz =	30		
D	1390	436	3- 8		1- 0	
			Hz =	17		

Membr	CSI	P Lbs	Axl-CSI-Bnd	
	Top Chords			
A -B	0.16	2240 C	0.09	0.07
B -H	0.19	3526 C	0.15	0.04
H -I	0.20	3526 C	0.15	0.05
I -C	0.16	2991 C	0.12	0.04
	Bottom Chords			
A -E	0.31	2048 T	0.13	0.18
E -G	0.30	2016 T	0.13	0.17
G -F	0.39	2991 T	0.20	0.19
F -D	0.13	16 T	0.00	0.13
	Webs			
E -B	0.05	373 T		
B -G	0.25	1560 T		
G -H	0.01	96 T		
G -I	0.09	563 T		
F -I	0.03	343 C		
F -C	0.50	3148 T		
D -C	0.08	1029 C		

LL Defl -0.04" in G -F L/999
TL Defl -0.09" in G -F L/999
Shear // Grain in A -E 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.8	0.3	0.79
B	LOCK	4.0x 8.0	Ctr	Ctr	0.69
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
I	LOCK	4.0x 5.0	Ctr	Ctr	0.75
C	LOCK	4.0x 5.0	Ctr	Ctr	0.89
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	4.0x 8.0	Ctr	Ctr	0.75
F	LOCK	4.0x 5.0	Ctr	Ctr	0.89
D	LOCK	2.0x 4.0	Ctr	Ctr	0.45

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 1- 7- 8
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

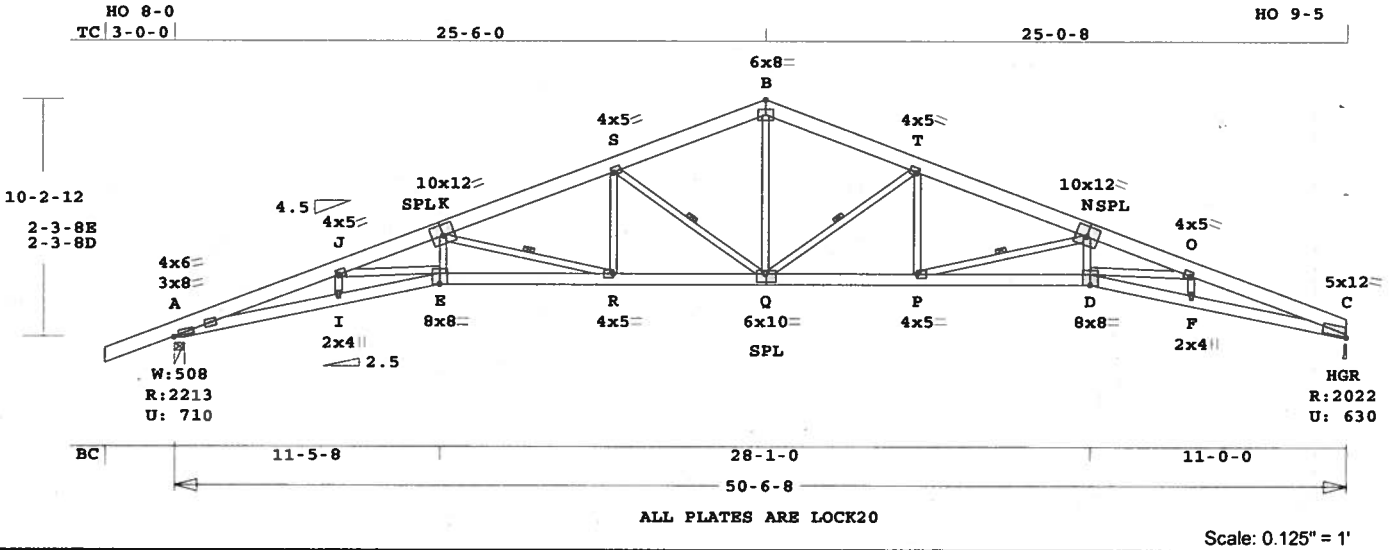
	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	1	12	24	0
BC	2	12	24	0
WB	1	4	4	

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3526 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R218	6	SP	500608	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 474.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 6- 8
BC	Cont.	0- 0- 0	50- 6- 8

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2214	711	5- 8	1-13
			Hz =	-175
C	2022	631	3- 8	1-11
			Hz =	177

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -J	0.28	8458 C	0.08	0.20
J -K	0.30	8612 C	0.09	0.21
K -S	0.17	5176 C	0.03	0.14
S -B	0.09	3691 C	0.04	0.05
B -T	0.09	3691 C	0.04	0.05
T -N	0.18	5121 C	0.03	0.15
N -O	0.30	8370 C	0.08	0.22
O -C	0.27	7939 C	0.07	0.20
Bottom Chords				
A -I	0.54	7988 T	0.36	0.18
I -E	0.54	8024 T	0.36	0.18
E -R	0.39	8019 T	0.36	0.03
R -Q	0.27	4881 T	0.22	0.05
Q -P	0.27	4830 T	0.22	0.05
P -D	0.40	7798 T	0.35	0.05
D -F	0.52	7499 T	0.34	0.18
F -C	0.52	7457 T	0.34	0.18
Webs				
I -J	0.03	133 T		
J -E	0.07	245 T		
E -K	0.58	1810 T		
K -R	0.84	3273 C		1 Br
R -S	0.33	1054 T		
S -Q	0.50	1794 C		1 Br
Q -B	0.72	2264 T		
Q -T	0.48	1730 C		1 Br
P -T	0.31	995 T		
P -N	0.79	3096 C		1 Br
D -N	0.54	1691 T		
D -O	0.17	532 T		
F -O	0.03	147 T		

TL Defl -1.33" in R -Q L/448
LL Defl -0.66" in R -Q L/903

Hz Disp	LL	DL	TL
Jt C	0.41"	0.41"	0.82"

Shear // Grain in O -C 0.25

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

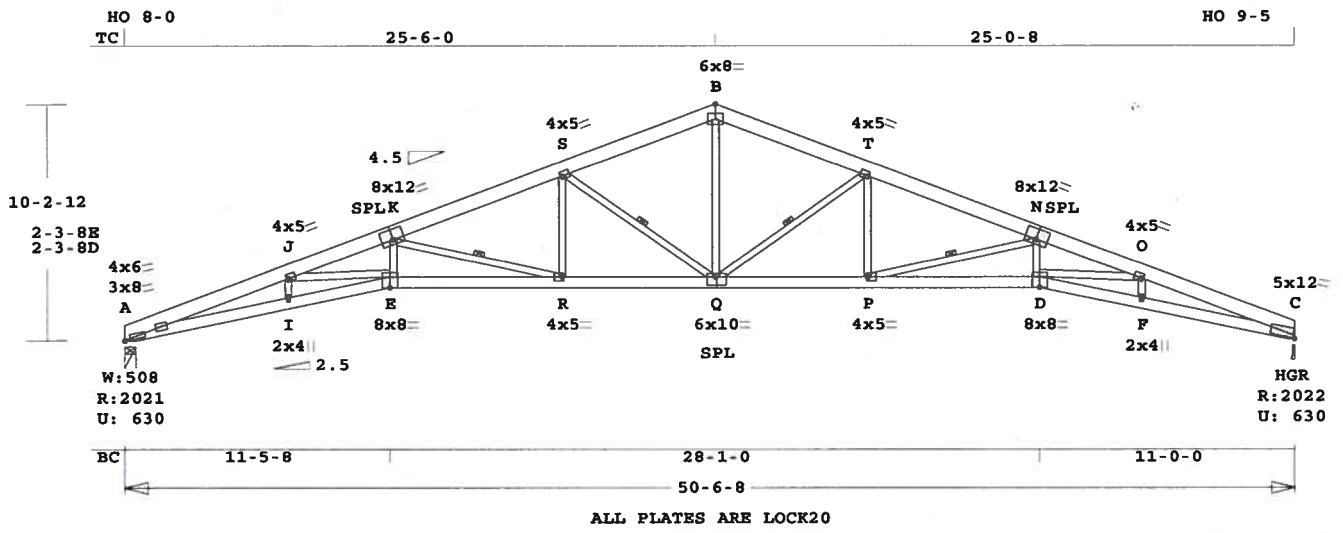
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0 Ctr	Ctr		0.80
A	LOCK	4.0x 6.0 Ctr	Ctr		0.74
J	LOCK	4.0x 5.0-0.5	0.2		0.62
K	LOCK	10.0x12.0-0.2	0.6		0.97
S	LOCK	4.0x 5.0-0.5	0.2		0.85
B	LOCK	6.0x 8.0 Ctr	Ctr		0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.82
N	LOCK	10.0x12.0	0.2	0.6	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	5.0x12.0-6.4	2.4		0.99
I	LOCK	2.0x 4.0 Ctr	Ctr		0.54
E	LOCK	8.0x 8.0 Ctr-1.5			0.80
R	LOCK	4.0x 5.0-0.5	Ctr		0.95
Q	LOCK	6.0x10.0 Ctr-1.2			0.97
P	LOCK	4.0x 5.0	0.5	Ctr	0.90
D	LOCK	8.0x 8.0 Ctr-1.5			0.80
F	LOCK	2.0x 4.0 Ctr	Ctr		0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8612 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R217*	1	SP	500608	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 462.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 6- 8
BC Cont.	0- 0- 0	50- 6- 8

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2022	631	5- 8	1-11
			Hz =	-175
C	2022	631	3- 8	1-11
			Hz =	177

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -J	0.28	8458 C	0.08 0.20
J -K	0.30	8612 C	0.09 0.21
K -S	0.17	5176 C	0.03 0.14
S -B	0.09	3691 C	0.04 0.05
B -T	0.09	3691 C	0.04 0.05
T -N	0.18	5121 C	0.03 0.15
N -O	0.30	8370 C	0.08 0.22
O -C	0.27	7939 C	0.07 0.20
Bottom Chords			
A -I	0.54	7989 T	0.36 0.18
I -E	0.54	8025 T	0.36 0.18
E -R	0.39	8019 T	0.36 0.03
R -Q	0.27	4882 T	0.22 0.05
Q -P	0.27	4830 T	0.22 0.05
P -D	0.40	7799 T	0.35 0.05
D -F	0.52	7500 T	0.34 0.18
F -C	0.52	7457 T	0.34 0.18
Webs			
I -J	0.03	133 T	
J -E	0.07	245 T	
E -K	0.58	1810 T	
K -R	0.84	3273 C	1 Br
R -S	0.33	1054 T	
S -Q	0.50	1794 C	1 Br
Q -B	0.72	2264 T	
Q -T	0.48	1730 C	1 Br
P -T	0.31	995 T	
P -N	0.79	3097 C	1 Br
D -N	0.54	1691 T	
D -O	0.17	532 T	
F -O	0.03	147 T	

LL Defl -0.66" in R -Q L/903
TL Defl -1.33" in R -Q L/448
Shear // Grain in O -C 0.25

Hz Disp	LL	DL	TL
Jt C	0.41"	0.41"	0.82"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.80
A	LOCK	4.0x 6.0	Ctr	Ctr	0.74
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	5.0x12.0	-6.4	2.4	0.99
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.95
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.90
D	LOCK	8.0x 8.0	Ctr	-1.5	0.80
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8612 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R216*	1*4P	SP	650704	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish

S1 LOCK 6.0x12.0 Ctr Ctr 0.91
A4 LOCK 4.0x 6.0 Ctr-0.5 0.84
VV LOCK 5.0x 6.0 Ctr Ctr 0.95
PP LOCK 5.0x 8.0 Ctr-1.8 0.94

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

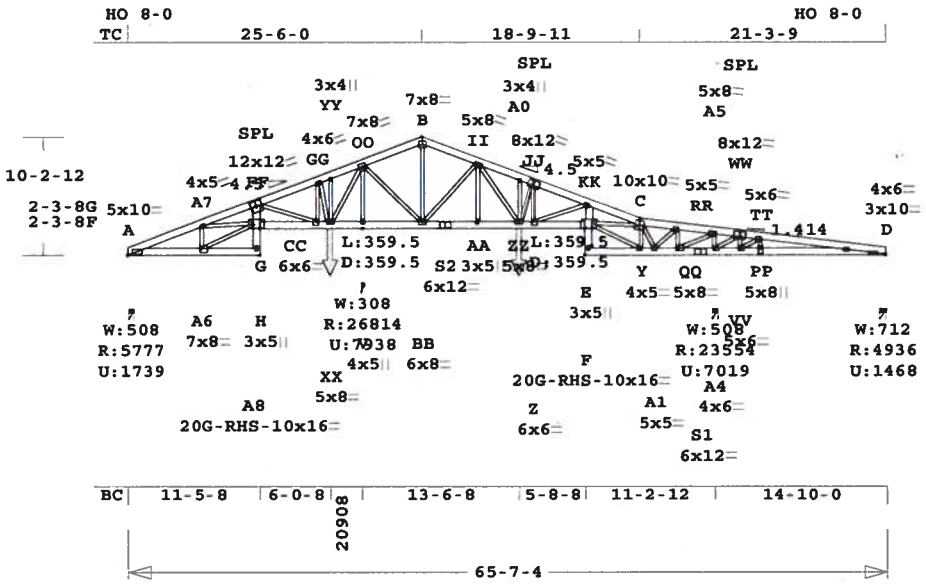
	----Spacing (In)----		
	Rows	Nails	Screws	Bolts
TC	2	12	24	24
BC	2	6	9.5	14.5
WB	1	8	8	

Web Connection Exception --
Use 4" spacing for screws or nails on the following webs
F- C V- O O- B B- I
No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
Plus use 1/2 In (ASTM-A307) thru bolts at each panel point and on each side of splices in 2x6 or larger chords only.
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 22614 Lbs
Quality Control Factor 1.25
5. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R216*	1*4P	SP	650704	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



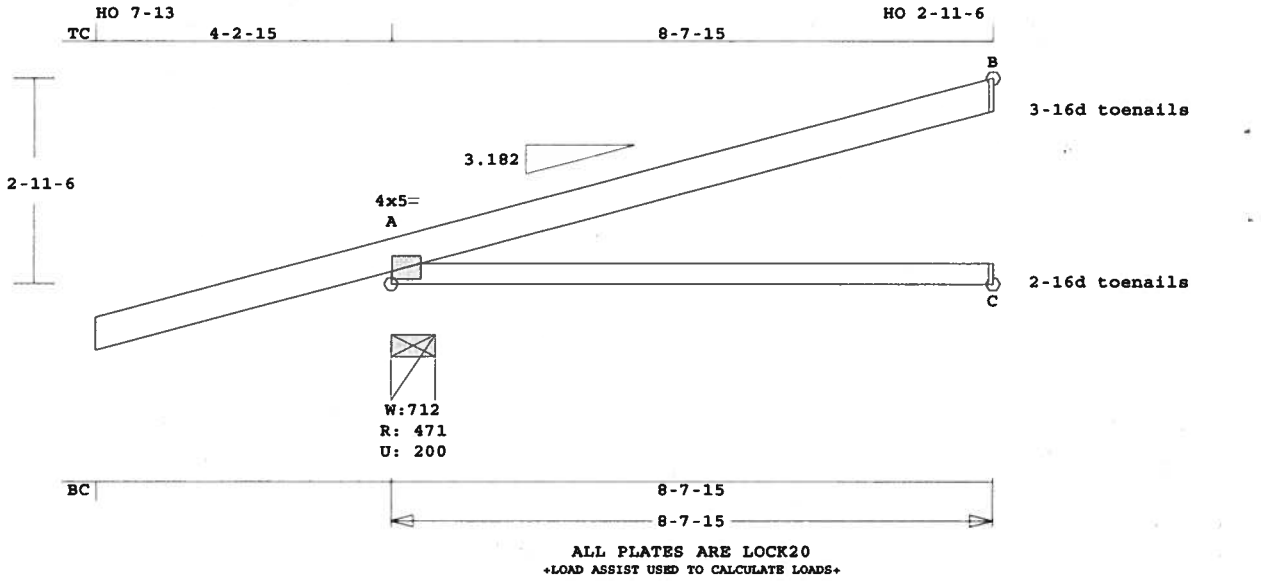
Scale: 0.063" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 743.6 LBS

```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
***************
* 4-Ply Truss *
***************

     CSI  -Size-  ----Lumber----
TC  0.38  2x 8  SP-2400
BC  0.45  2x 8  SP-2400
CW  0.30  2x 8  SP-2400
WB  0.93  2x 4  SP-#3
EX OO-BB  2x 4  SP-#2
EX QQ-A5  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 65- 7- 4
 BC Cont.      0- 0- 0 65- 7- 4

Loading   Live   Dead  (psf)
TC         20.0   10.0
BC          0.0   10.0
Total      20.0   20.0   40.0
Spacing                  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor   1.25
Plate  Duration Factor   1.25
plf - Live  Dead  From    To
TC V    40    20   0.0'  65.6'
BC V     0    20   0.0'  65.6'
BC V   490   490   0.0'  17.5'
BC V   187   187  17.5'  33.8'
BC V   490   490  33.8'  50.8'
BC V   490   490  50.8'  65.6'
BC V   360   360  17.5'  CL-LB
BC V   360   360  33.8'  CL-LB


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A    5777  1740  5- 8  1- 8
                Hz =  -169
V   26814  7939  3- 8  5- 0 **
A4  23554  7020  5- 8  4- 9
D    4937  1468  7-12  1- 8
                Hz =   179

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -A7  0.10  6988 C  0.00  0.10
A7-FF  0.03  1179 T  0.01  0.02
FF-GG  0.13 12980 T  0.11  0.02
GG-YY  0.15 15404 T  0.13  0.02
YY-OO  0.24 15562 T  0.13  0.11
OO-B   0.16  6503 T  0.05  0.11
B -II  0.08  6581 T  0.05  0.03
II-A0  0.03  5211 C  0.00  0.03
A0-JJ  0.02  5459 C  0.00  0.02
JJ-KK  0.18  7435 C  0.00  0.18
KK-C   0.19 14378 C  0.01  0.18
C -RR  0.08  3399 C  0.00  0.08
RR-A5  0.20  4111 T  0.03  0.17
A5-WW  0.35 20841 T  0.18  0.17
WW-TT  0.38 13154 T  0.11  0.27
TT-D   0.33   239 C  0.00  0.33
--------Bottom Chords---------
A -A6  0.22  6667 T  0.05  0.17
A6-H   0.14   226 C  0.00  0.14
A8-CC  0.17   883 C  0.00  0.17
CC-XX  0.06 12120 C  0.01  0.05
XX-V   0.32 17795 C  0.02  0.30
V -BB  0.32 17795 C  0.02  0.30
BB-S2  0.07   542 C  0.00  0.07
S2-AA  0.06   542 C  0.00  0.06
AA-ZZ  0.05   542 C  0.00  0.05
ZZ-Z   0.12  7181 T  0.06  0.06
Z -F   0.32 14615 T  0.12  0.20
E -Y   0.15  1669 T  0.01  0.14
Y -A1  0.13  6479 T  0.05  0.08
A1-QQ  0.08  4179 C  0.00  0.08
QQ-S1  0.09 20647 C  0.02  0.07
S1-A4  0.30 20647 C  0.02  0.28
A4-VV  0.29 13223 C  0.01  0.28
VV-PP  0.45   430 T  0.00  0.45
PP-D   0.45   430 T  0.00  0.45
----------Chord-Webs----------
H -A8  0.04  1915 T  0.01  0.03
A8-FF  0.10  8212 T  0.07  0.03
E -F   0.30  2085 T  0.01  0.29
F -KK  0.27  6616 T  0.06  0.21
-------------Webs-------------
A6-A7  0.19  2635 T
A6-A8  0.61  7665 T
A7-A8  0.31  7800 C
FF-CC  0.49 11987 C
CC-GG  0.67  8406 T
GG-XX  0.32  8111 C
XX-YY  0.02   341 T
XX-OO  0.58  7340 T
V -OO  0.93 22614 C
OO-BB  0.76 16627 T
BB-B   0.19  4470 C
BB-II  0.39  8793 C
AA-II  0.15  1941 T
II-ZZ  0.80  9976 T
ZZ-A0  0.04   599 T
ZZ-JJ  0.29  7320 C
Z -JJ  0.51  6460 T
Z -KK  0.33  8011 C
F -C   0.53  6683 T
F -Y   0.43  5363 T
Y -C   0.05   810 T
C -A1  0.26  6641 C
A1-RR  0.88 11091 T
QQ-RR  0.34  8853 C
QQ-A5  0.87 18994 T
A4-A5  0.50 12823 C
A4-WW  0.36  9213 C
VV-WW  0.64  8018 T
VV-TT  0.67 17121 C
PP-TT  0.69  8697 T

TL Defl  -0.24" in PP-D  L/707
LL Defl  -0.12" in PP-D  L/999
Shear // Grain  in PP-D   0.69

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  5.0x10.0 Ctr Ctr 0.96
A7 LOCK  4.0x 5.0-0.5 0.2 0.78
FF LOCK 12.0x12.0 Ctr Ctr 0.91
GG LOCK  4.0x 6.0-0.5 0.2 0.87
YY LOCK  3.0x 4.0 Ctr Ctr 0.42
OO LOCK  7.0x 8.0 Ctr Ctr 0.94
B  LOCK  7.0x 8.0 Ctr Ctr 0.84
II LOCK  5.0x 8.0 0.5-0.2 0.93
A0 LOCK  3.0x 4.0 Ctr Ctr 0.42
JJ LOCK  8.0x12.0 0.6 1.5 0.97
KK LOCK  5.0x 5.0 0.5-0.2 0.65
C  LOCK 10.0x10.0 Ctr Ctr 0.80
RR LOCK  5.0x 5.0 Ctr Ctr 0.89
A5 LOCK  5.0x 8.0-0.2 Ctr 0.95
WW LOCK  8.0x12.0 0.2 1.5 0.99
TT LOCK  5.0x 6.0 Ctr Ctr 0.96
D  LOCK  3.0x10.0 Ctr Ctr 0.95
D  LOCK  4.0x 6.0 Ctr Ctr 0.21
A6 LOCK  7.0x 8.0 0.5-1.1 0.61
H  LOCK  3.0x 5.0 Ctr Ctr 0.96
A8 RHS  10.0x16.0 Ctr 1.4 0.68
CC LOCK  6.0x 6.0 Ctr-0.6 0.79
XX LOCK  5.0x 8.0 Ctr Ctr 0.70
V  LOCK  4.0x 5.0 Ctr Ctr 0.94
BB LOCK  6.0x 8.0-0.5 Ctr 0.97
S2 LOCK  6.0x12.0 Ctr Ctr 0.91
AA LOCK  3.0x 5.0 Ctr Ctr 0.42
ZZ LOCK  5.0x 8.0 Ctr Ctr 0.89
Z  LOCK  6.0x 6.0 Ctr-0.6 0.60
F  RHS  10.0x16.0 Ctr 1.4 0.68
E  LOCK  3.0x 5.0 Ctr Ctr 0.96
Y  LOCK  4.0x 5.0 Ctr-0.5 0.62
A1 LOCK  5.0x 5.0 Ctr Ctr 0.92
QQ LOCK  5.0x 8.0 0.5 Ctr 0.90
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R215	2	KING	80715	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 54.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.48	2x 6	SP-#2
BC	0.42	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 7-15
BC Cont.	0- 0- 0	8- 7-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	8.7'
BC V	0	20	0.0'	8.7'
TC V	-40	-20	0.0'	
	32	16		8.7'
BC V	0	-20	0.0'	
	0	16		8.7'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	472	201	7-12	1- 8
			Hz =	94
C	186	0	3- 8	1- 8
B	326	202	3- 8	1- 8
			Hz =	50

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.48	113 C	0.00 0.48
--------Bottom Chords---------			
A -C	0.42	0 T	0.00 0.42

TL Defl -0.29" in A -C L/325
LL Defl -0.13" in A -C L/724
Shear // Grain in A -B 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

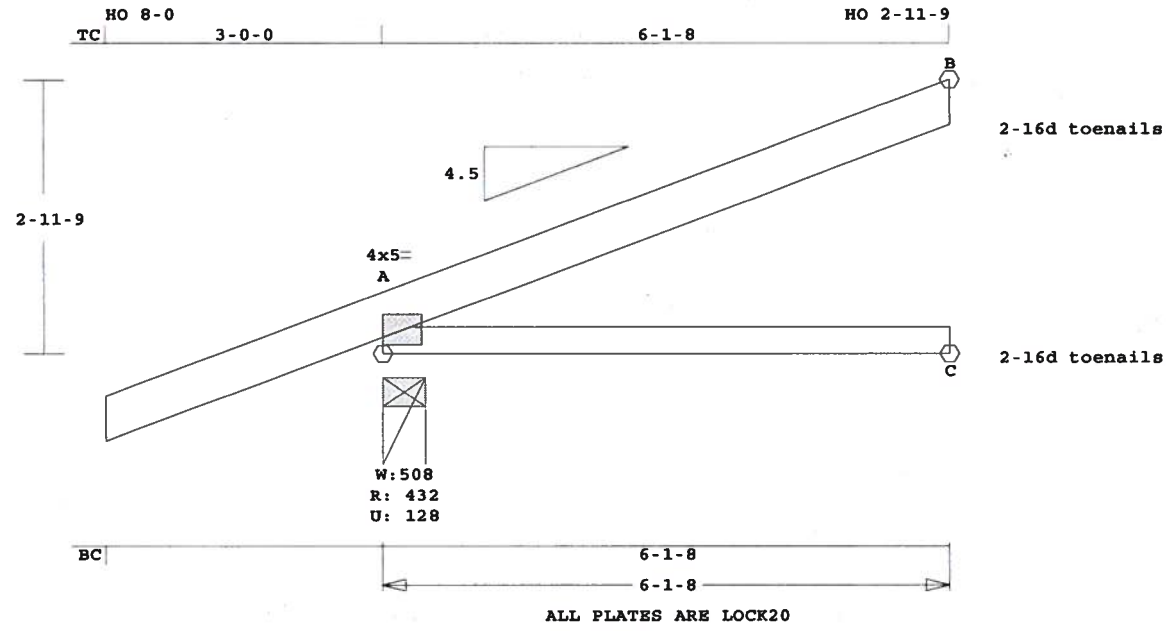
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder King Jack
Loading TC and BC
Setback 6- 1- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 113 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R214*	2	JACK	60108	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 39.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.23	2x 6	SP-#2
BC	0.18	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	6- 1- 8
BC	Cont.	0- 0- 0	6- 1- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	432	128	5- 8	1- 8
			Hz = 103	
C	103	0	3- 8	1- 8
B	191	127	3- 8	1- 8
			Hz = 56	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.23	120 C	0.00 0.23
--------Bottom Chords---------			
A -C	0.18	0 T	0.00 0.18

TL Defl -0.07" in A -C L/883
LL Defl -0.03" in A -C L/999
Shear // Grain in A -B 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

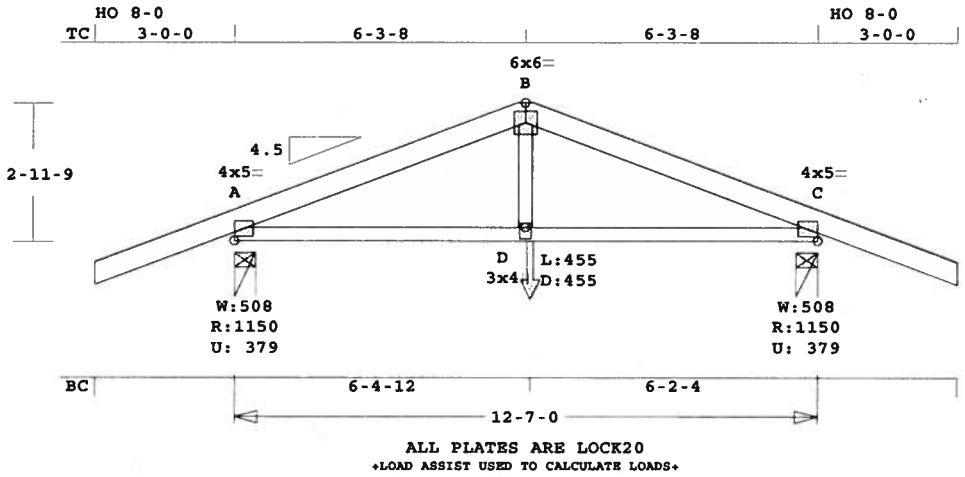
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 120 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R213*	1	TR	120700	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 85.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 6	SP-#2
BC	0.51	2x 4	SP-#2
WB	0.31	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	12.6'
BC V	0	20	0.0'	12.6'
BC V	455	455	6.4'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1150	380	5- 8	1- 8
C	1150	380	5- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.30	1771 C	0.12	0.18
B -C	0.30	1771 C	0.12	0.18
--------Bottom Chords---------				
A -D	0.51	1637 T	0.30	0.21
D -C	0.51	1637 T	0.30	0.21
-------------Webs-------------				
D -B	0.31	1101 T		

TL Defl -0.07" in D -C L/999
LL Defl -0.03" in D -C L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C	LOCK	4.0x 5.0	-0.7	0.3	0.65
D	LOCK	3.0x 4.0	Ctr	-0.9	0.79

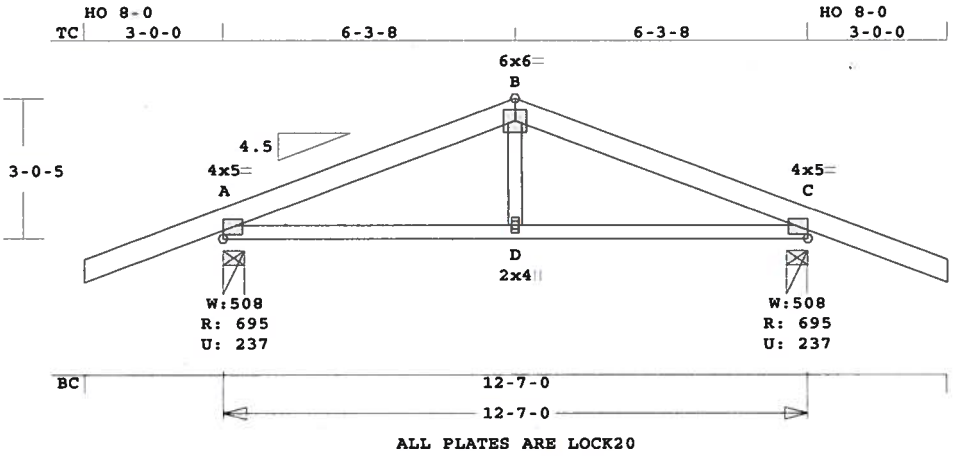
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1771 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R212*	2	TR	120700	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 85.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.19	2x 6	SP-#2
BC	0.25	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	695	237	5- 8	1- 8
C	695	237	5- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.19	672 C	0.04	0.15
B -C	0.19	672 C	0.04	0.15
--------Bottom Chords---------				
A -D	0.25	624 T	0.06	0.19
D -C	0.25	624 T	0.06	0.19
------------Webs------------				
D -B	0.06	247 T		

TL Defl -0.04" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C	LOCK	4.0x 5.0	-0.7	0.3	0.65
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

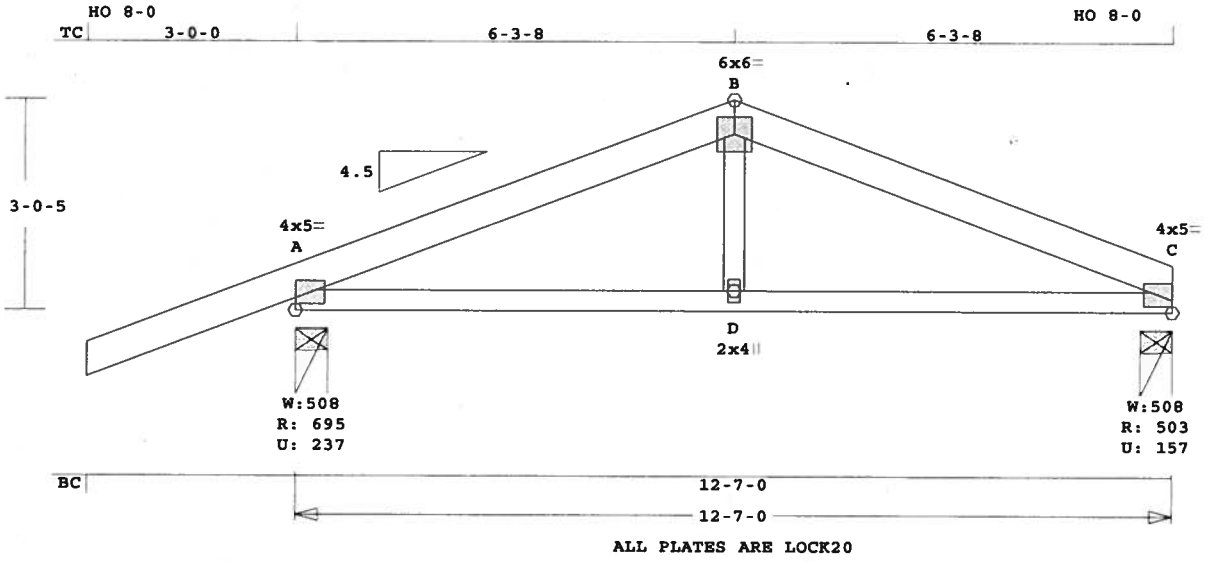
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00

Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 672 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R211*	1	TR	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 76.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.19	2x 6	SP-#2
BC	0.25	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	695	237	5- 8	1- 8
C	503	157	5- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -B	0.19	672 C	0.04	0.15
B -C	0.19	672 C	0.04	0.15
Bottom Chords				
A -D	0.25	624 T	0.06	0.19
D -C	0.25	624 T	0.06	0.19
Webs				
D -B	0.06	247 T		

TL Defl -0.04" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

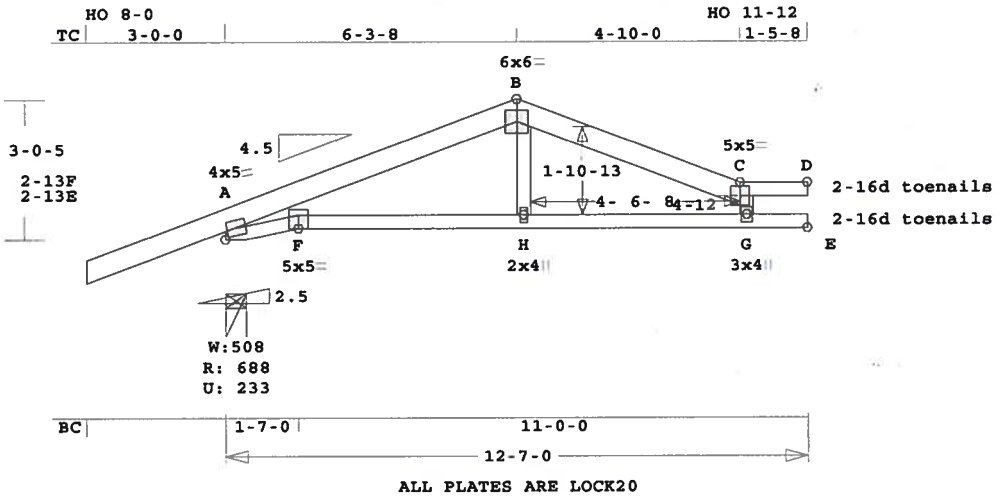
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C	LOCK	4.0x 5.0	-0.7	0.3	0.65
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 672 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R210*	1	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 74.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 6	SP-#2
EX C -D		2x 4	SP-#2
BC	0.52	2x 4	SP-#2
WB	0.95	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	688	234	5- 8	1- 8
			Hz =	29
E	253	79	3- 8	1- 8
D	258	83	3- 8	1- 8
			Hz =	12

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.21	752 C	0.04 0.17
B -C	0.25	756 C	0.06 0.19
C -D	0.42	11 T	0.00 0.42
Bottom Chords			
A -F	0.48	684 T	0.11 0.37
F -H	0.29	705 T	0.11 0.18
H -G	0.29	715 T	0.12 0.17
G -E	0.52	0 T	0.00 0.52
Webs			
H -B	0.13	261 T	0.05 0.08
G -C	0.95	357 C	0.00 0.95

TL Defl -0.10" in F -H L/999
LL Defl -0.04" in H -G L/999

Hz Disp	LL	DL	TL
Jt E	0.01"	0.02"	0.03"

Shear // Grain in G -E 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C	LOCK	5.0x 5.0	Ctr	Ctr	0.58
F	LOCK	5.0x 5.0	Ctr	0.8	0.41
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	3.0x 4.0	Ctr	Ctr	0.25

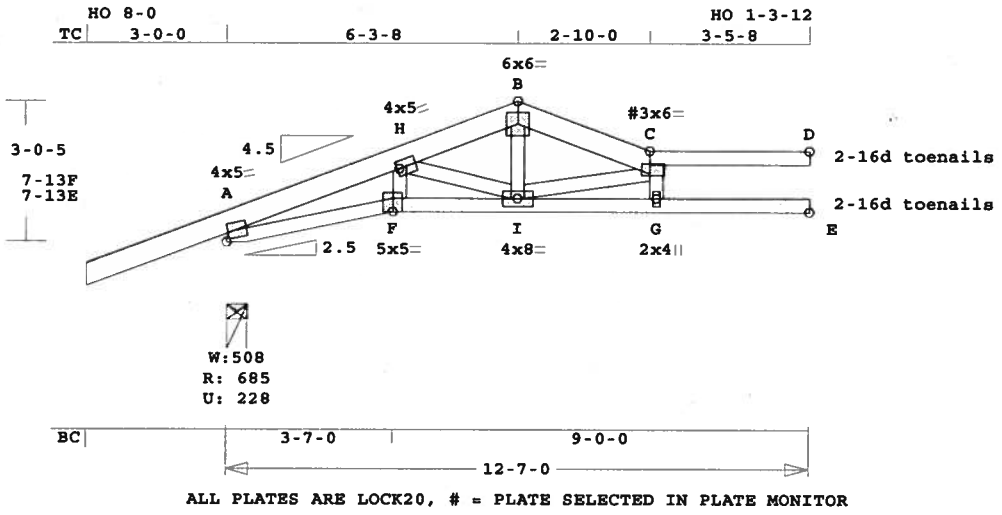
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 756 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R209*	1	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 83.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 6	SP-#2
EX C -C		2x 4	SP-#2
BC	0.35	2x 4	SP-#2
EX F -G		2x 4	SP-2400
WB	0.29	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	685	229	5- 8	1- 8
			Hz =	63
E	241	51	3- 8	1- 8
D	272	114	3- 8	1- 8
			Hz =	31

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -H	0.24	1163 C	0.00	0.24
H -B	0.16	903 C	0.00	0.16
B -C	0.89	866 C	0.00	0.89
C -D	0.89	30 T	0.00	0.89
--------Bottom Chords---------				
A -F	0.35	1061 T	0.17	0.18
F -I	0.24	1047 T	0.07	0.17
I -G	0.73	0 T	0.00	0.73
G -E	0.73	0 T	0.00	0.73
------------Webs------------				
F -H	0.05	207 T		
H -I	0.05	211 T		
I -B	0.04	164 T		
I -C	0.29	932 T		
G -C	0.08	402 C		

TL Defl	-0.53" in I -G		L/268
LL Defl	-0.26" in I -G		L/539
Hz Disp	LL	DL	TL
Jt D	0.06"	0.06"	0.12"
Shear // Grain		in B -C	0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C#	LOCK	3.0x 6.0	0.9	1.2	1.00
F	LOCK	5.0x 5.0	Ctr-1.0		0.46
I	LOCK	4.0x 8.0	Ctr	Ctr	0.77
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38

= Plate Monitor used

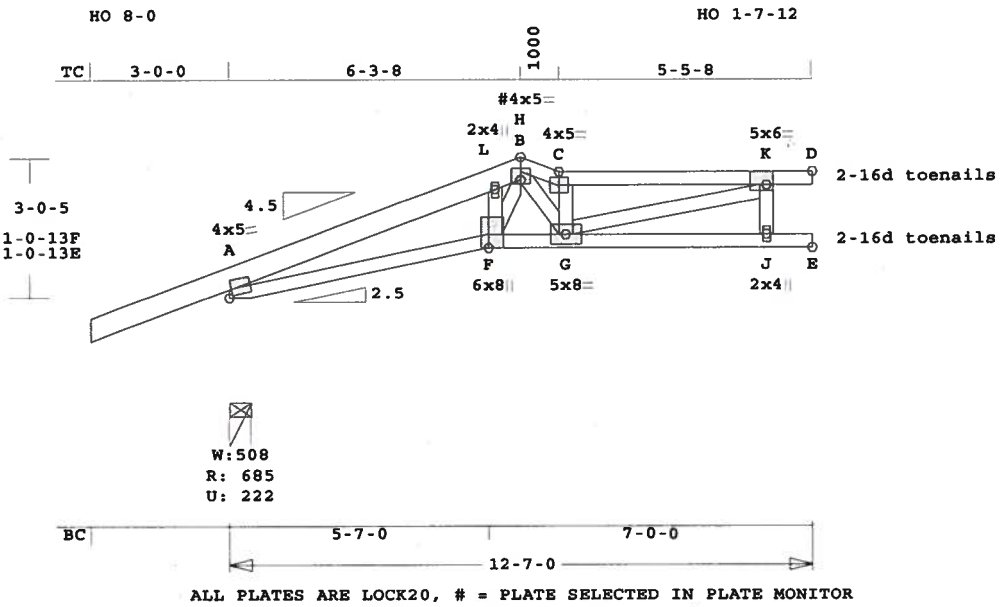
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1163 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R208*	1	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.36	2x 4	SP-#2
EX A -H		2x 6	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.39	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	685	223	5- 8	1- 8
			Hz =	97
E	286	120	3- 8	1- 8
D	227	52	3- 8	1- 8
			Hz =	51

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -L	0.26	1191	C	0.00	0.26
L -H	0.09	1161	C	0.07	0.02
H -C	0.15	1212	C	0.10	0.05
C -K	0.36	1162	C	0.01	0.35
K -D	0.23	50	T	0.00	0.23
--------Bottom Chords---------					
A -F	0.36	1114	T	0.18	0.18
F -G	0.25	905	T	0.15	0.10
G -J	0.36	0	T	0.00	0.36
J -E	0.36	0	T	0.00	0.36
------------Webs------------					
F -L	0.06	240	T		
F -H	0.13	410	T		
H -G	0.13	433	T		
G -C	0.09	562	C		
G -K	0.39	1218	T		
J -K	0.06	280	C		

TL Defl -0.15" in G -J L/969
LL Defl -0.07" in G -J L/999

Hz Disp	LL	DL	TL
Jt E	0.02"	0.02"	0.04"

Shear // Grain in J -E 0.36

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79
L	LOCK	2.0x 4.0	Ctr	Ctr	0.38
H#	LOCK	4.0x 5.077	42.9		0.81
C	LOCK	4.0x 5.0	Ctr	Ctr	0.70
K	LOCK	5.0x 6.0	0.5	2.8	0.93
F	LOCK	6.0x 8.0	1.0	0.5	0.58
G	LOCK	5.0x 8.0	Ctr	Ctr	1.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.38

= Plate Monitor used

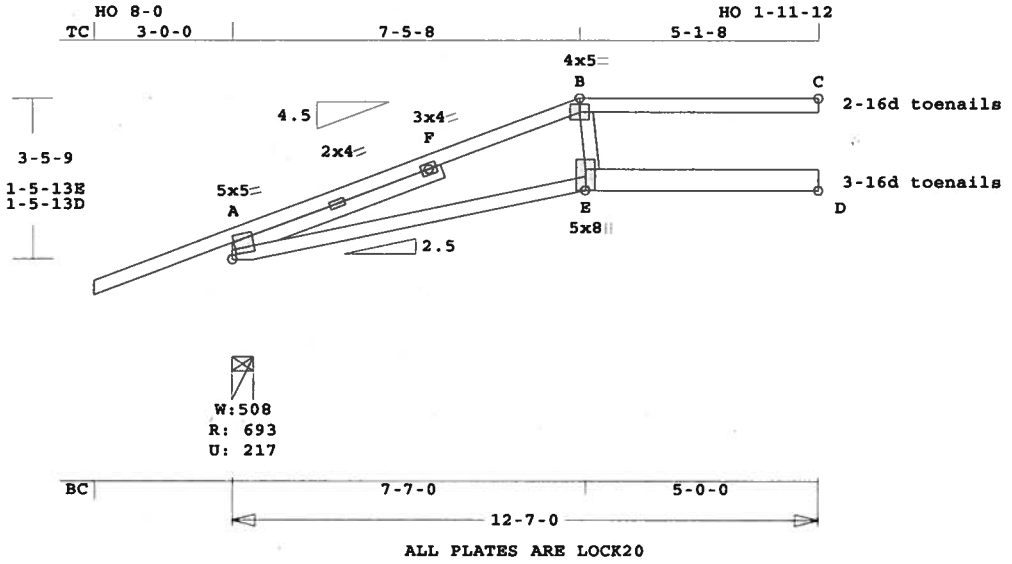
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1212 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R207	1	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 71.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 4	SP-#2
BC	0.00	2x 4	SP-#2
EX	E -D	2x 6	SP-2400
WB	0.09	2x 4	SP-#3
SL	0.14	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	693	217	5- 8	1- 8
			Hz =	126
D	326	84	3- 8	1- 8
C	179	94	3- 8	1- 8
			Hz =	68

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.71	514 C	0.00	0.71
F -B	0.71	67 C	0.00	0.71
B -C	0.39	67 T	0.00	0.39
--------Bottom Chords---------				
A -E	0.64	20 C	0.00	0.64
E -D	0.64	0 T	0.00	0.64
------------Webs------------				
B -E	0.09	379 T		
-----------Sliders-----------				
A -F	0.14	485 T		

TL Defl -0.74" in A -E L/190
LL Defl -0.37" in A -E L/387

Hz Disp	LL	DL	TL
Jt C	0.21"	0.21"	0.41"

Shear // Grain in A -F 0.33

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	1.00
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.71
E	LOCK	5.0x 8.0	Ctr	0.4	0.42

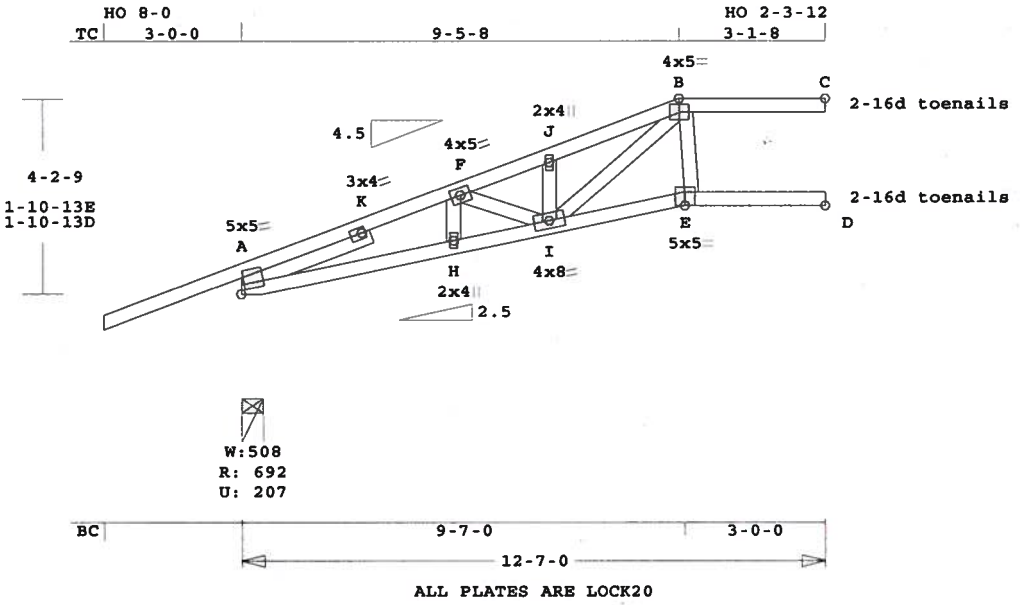
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 514 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R206*	1	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 79.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.88	2x 4	SP-#2
BC	0.40	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.47	2x 4	SP-#3
SL	0.09	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	692	207	5- 8	1- 8
			Hz =	160
D	234	64	3- 8	1- 8
C	272	123	3- 8	1- 8
			Hz =	86

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -K	0.05	676 C	0.00	0.05
K -F	0.12	1153 C	0.08	0.04
F -J	0.25	1154 C	0.09	0.16
J -B	0.88	1260 C	0.01	0.87
B -C	0.87	85 T	0.00	0.87
--------Bottom Chords---------				
A -H	0.25	1087 T	0.18	0.07
H -I	0.40	1106 T	0.18	0.22
I -E	0.68	24 C	0.00	0.68
E -D	0.68	0 T	0.00	0.68
------------Webs------------				
H -F	0.02	95 T		
F -I	0.01	64 C		
I -J	0.10	447 C		
I -B	0.47	1481 T		
B -E	0.09	375 C		
----------Sliders----------				
A -K	0.09	538 C		

TL Defl -0.49" in I -E L/287
LL Defl -0.24" in I -E L/584

Hz Disp	LL	DL	TL
Jt C	0.10"	0.10"	0.20"

Shear // Grain in J -B 0.36

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	1.00
K	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	2.0x 4.0	Ctr	Ctr	0.38
B	LOCK	4.0x 5.0	Ctr	Ctr	0.85
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
I	LOCK	4.0x 8.0	Ctr	Ctr	0.65
E	LOCK	5.0x 5.0	Ctr	-1.0	0.44

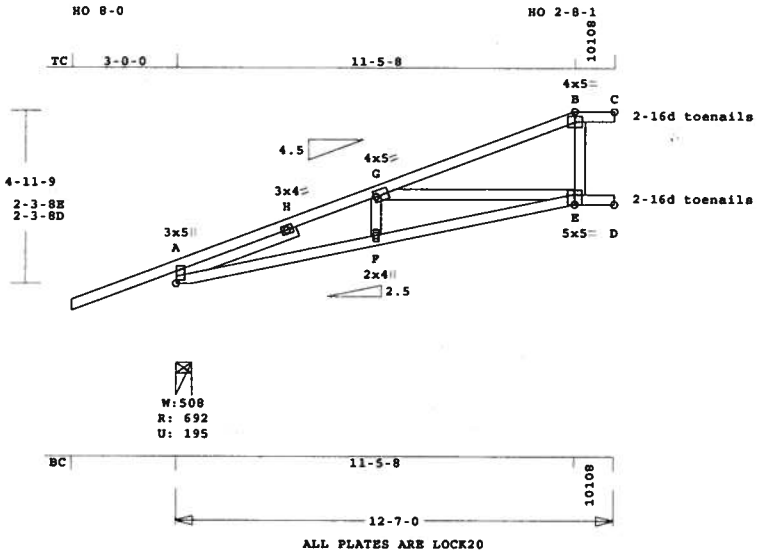
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1260 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R205*	1	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 79.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.64	2x 4	SP-#2
WB	0.61	2x 4	SP-#3
SL	0.12	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	692	196	5- 8	1- 8
			Hz =	194
D	254	137	3- 8	1- 8
C	253	62	3- 8	1- 8
			Hz =	104

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords					
A -H	0.13	702	C	0.00	0.13
H -G	0.34	1280	C	0.08	0.26
G -B	0.51	84	T	0.00	0.51
B -C	0.31	104	T	0.00	0.31
Bottom Chords					
A -F	0.33	1236	T	0.20	0.13
F -E	0.64	1224	T	0.20	0.44
E -D	0.44	0	T	0.00	0.44
Webs					
F -G	0.06	258	T		
G -E	0.61	1210	C		
E -B	0.05	196	T		
Sliders					
A -H	0.12	643	C		

TL Defl -0.28" in F -E L/515
LL Defl -0.13" in F -E L/999

Hz Disp	LL	DL	TL
Jt C	0.03"	0.02"	0.05"

Shear // Grain in E -D 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	4.0x 5.0	Ctr	Ctr	0.72
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	5.0x 5.0	Ctr	-1.0	0.59

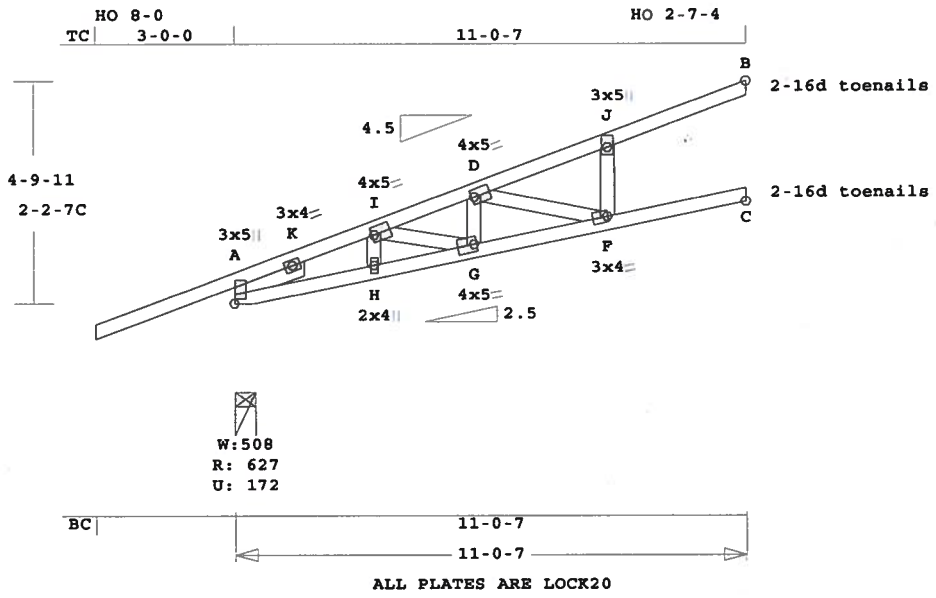
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1280 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R204*	1	HFSC	110007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.67	2x 4	SP-#2
BC	0.63	2x 4	SP-2400
WB	0.20	2x 4	SP-#3
SL	0.04	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 0- 7
BC	Cont.	0- 0- 0	11- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	627	172	5- 8	1- 8
			Hz =	186
C	210	60	3- 8	1- 8
B	238	124	3- 8	1- 8
			Hz =	100

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -K	0.12	676	C	0.00	0.12
K -I	0.12	871	C	0.00	0.12
I -D	0.22	1065	C	0.00	0.22
D -J	0.67	102	C	0.00	0.67
J -B	0.67	94	C	0.00	0.67
--------Bottom Chords---------					
A -H	0.13	795	T	0.05	0.08
H -G	0.18	825	T	0.05	0.13
G -F	0.63	1004	T	0.07	0.56
F -C	0.56	36	T	0.00	0.56
------------Webs------------					
H -I	0.02	104	C		
I -G	0.07	238	T		
G -D	0.07	239	T		
D -F	0.20	1056	C		
F -J	0.05	188	T		
----------Sliders----------					
A -K	0.04	299	C		

TL Defl -0.35" in G -F L/353
LL Defl -0.17" in G -F L/712

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.02"

Shear // Grain in D -J 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.71
K	LOCK	3.0x 4.0	Ctr	Ctr	0.50
I	LOCK	4.0x 5.0	Ctr	Ctr	0.64
D	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	3.0x 5.0	Ctr	0.7	0.37
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	4.0x 5.0	Ctr	Ctr	0.61
F	LOCK	3.0x 4.0	Ctr	Ctr	0.56

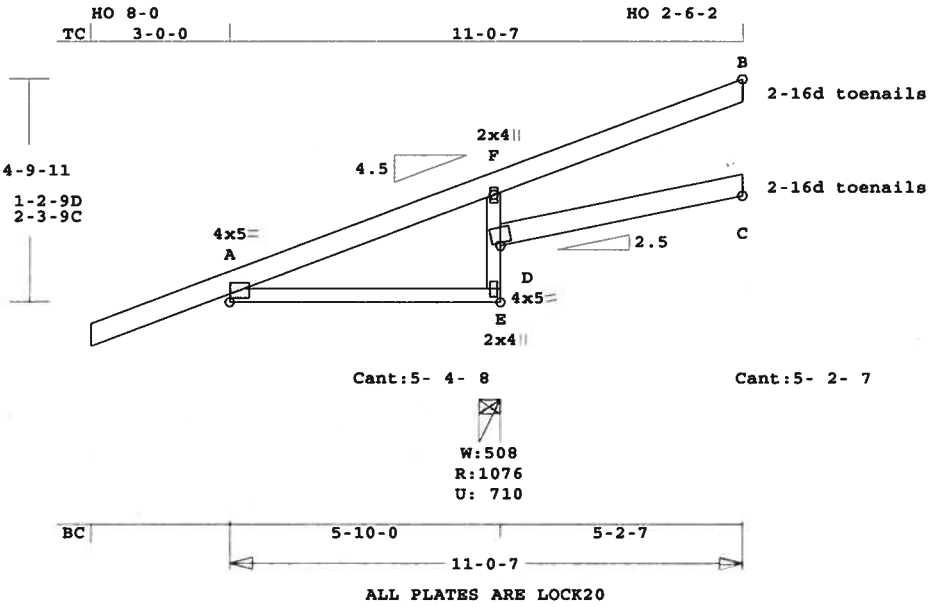
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1065 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R203*	1	SP	110007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 73.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.78	2x 6	SP-#2
BC	0.47	2x 4	SP-#2
EX D -C		2x 6	SP-#2
CW	0.59	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 7
BC Cont.	0- 0- 0	11- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
D	1077	710	5- 8	1- 8
			Hz =	187
C	113	44G	3- 8	1- 8
B	63	27G	3- 8	1- 8
			Hz =	100

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.78	562 C	0.00	0.78
F -B	0.78	86 T	0.00	0.78
--------Bottom Chords---------				
A -E	0.47	498 T	0.06	0.41
E -D	0.49	161 C	0.00	0.49
D -C	0.73	39 T	0.00	0.73
----------Chord-Webs----------				
E -D	0.36	102 C	0.00	0.36
D -F	0.59	910 T	0.12	0.47

TL Defl 0.04" in D -C L/999
LL Defl 0.02" in D -C L/999
LL Cant -0.22" in A -E L/291

Hz Disp	LL	DL	TL
Jt B	0.22"	0.03"	0.19"

Shear // Grain in A -E 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
F	LOCK	2.0x 4.0	Ctr	Ctr	0.45
E	LOCK	2.0x 4.0	Ctr	Ctr	0.58
D	LOCK	4.0x 5.0	Ctr	Ctr	0.48

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

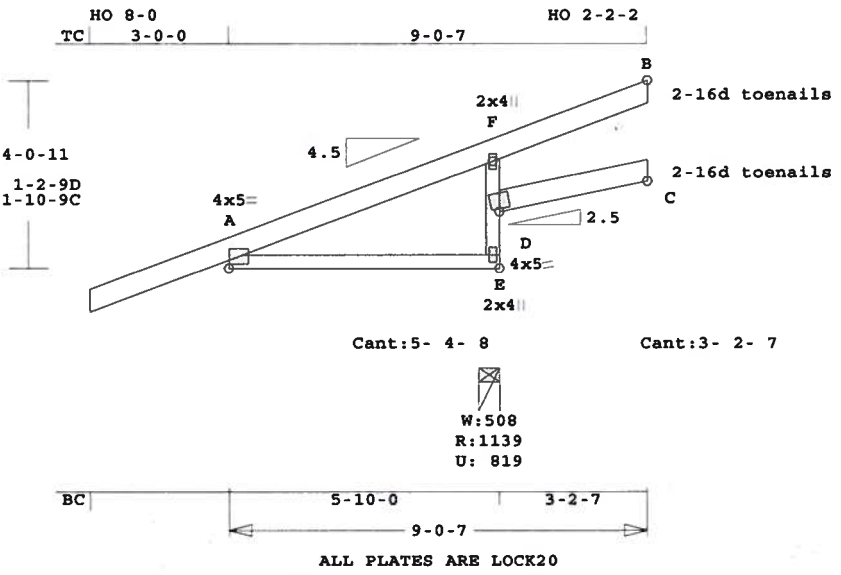
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 667 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R202*	1	SP	90007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.90	2x 6	SP-#2
BC	0.59	2x 4	SP-#2
EX D -C		2x 6	SP-#2
CW	0.85	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 7
BC Cont.	0- 0- 0	9- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
D	1139	819	5- 8	1- 8
		Hz =	153	
C	163	128G	3- 8	1- 8
B	95	106G	3- 8	1- 8
		Hz =	82	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -F	0.90	668 C	0.00 0.90
F -B	0.90	107 T	0.00 0.90
Bottom Chords			
A -E	0.59	586 T	0.07 0.52
E -D	0.59	202 C	0.00 0.59
D -C	0.93	78 T	0.00 0.93
Chord-Webs			
E -D	0.43	128 C	0.00 0.43
D -F	0.85	1182 T	0.16 0.69

TL Defl	0.02" in D -C	L/999
LL Defl	0.01" in D -C	L/999
LL Cant	-0.19" in A -E	L/337

Hz Disp	LL	DL	TL
Jt B	0.22"	0.02"	0.19"

Shear // Grain in A -E 0.35

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
F	LOCK	2.0x 4.0	Ctr	Ctr	0.59
E	LOCK	2.0x 4.0	Ctr	Ctr	0.58
D	LOCK	4.0x 5.0	Ctr	Ctr	0.46

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

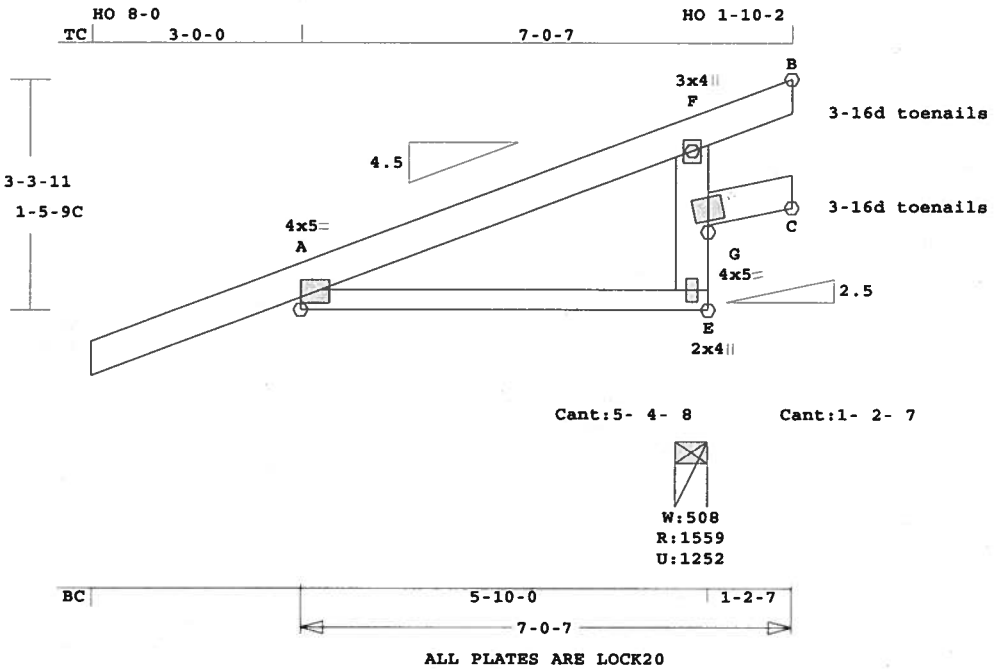
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 684 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R201*	1	SP	70007	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.98	2x 6	SP-#2
BC	0.72	2x 4	SP-#2
EX G -C		2x 6	SP-2400
CW	0.70	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 7
BC	Cont.	0- 0- 0	7- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
G	1560	1252	5- 8	1- 8
			Hz =	119
C	362	379G	3- 8	1- 8
B	379	426G	3- 8	1- 8
			Hz =	64

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.95	854 C	0.00	0.95
F -B	0.98	396 T	0.03	0.95
--------Bottom Chords---------				
A -E	0.72	754 T	0.09	0.63
E -G	0.27	223 C	0.00	0.27
G -C	0.61	233 T	0.00	0.61
----------Chord-Webs----------				
E -G	0.49	198 C	0.00	0.49
G -F	0.70	2070 T	0.20	0.50

TL Defl	0.00"	in S1-E	L/999
LL Defl	0.00"	in S1-E	L/999
LL Cant	-0.13"	in A -E	L/504

Hz Disp	LL	DL	TL
Jt B	0.16"	0.01"	0.15"

Shear // Grain in F -B 0.77

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
F	LOCK	3.0x 4.0	Ctr	Ctr	0.68
E	LOCK	2.0x 4.0	Ctr	Ctr	0.91
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

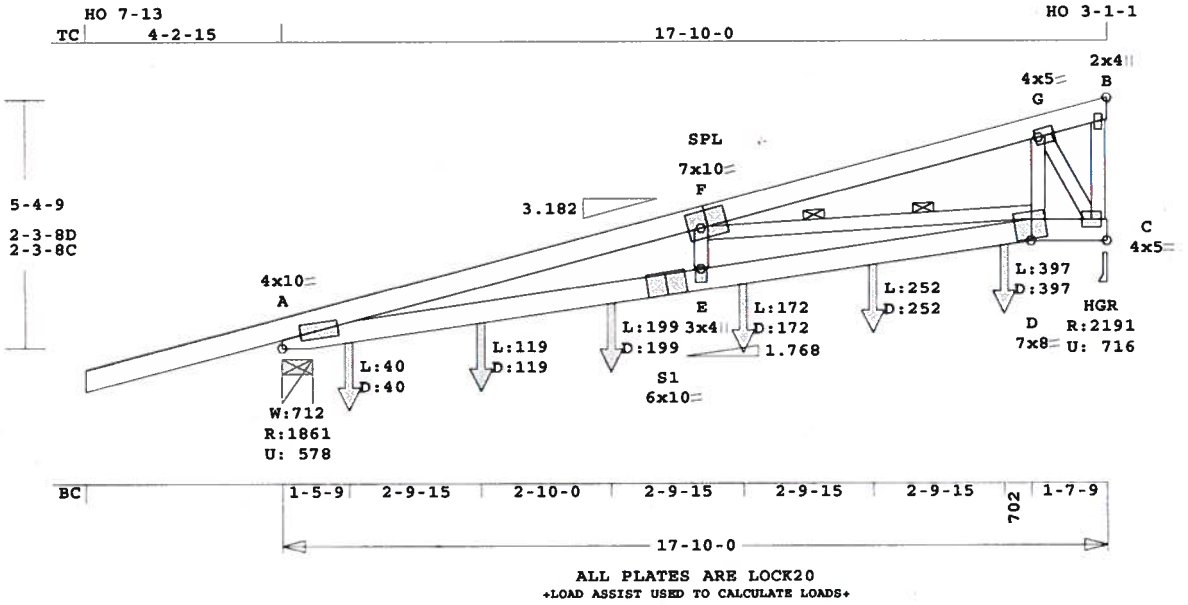
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 894 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R200*	1	SP	171000	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 142.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.90	2x 6	SP-#2
BC	0.79	2x 6	SP-2400
EX D -C		2x 6	SP-#2
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17-10- 0
BC Cont.	0- 0- 0	17-10- 0

WB 2 rows CLB on F -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	17.8'
BC V	0	20	0.0'	17.8'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.1'	CL-LB
BC V	172	172	10.0'	CL-LB
BC V	252	252	12.8'	CL-LB
BC V	397	397	15.6'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	1861	578	7-12	1- 9
			Hz =	212
C	2192	717	3- 8	2- 9
			Hz =	114

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -F	0.90	6270	C	0.20	0.70
F -G	0.62	1394	C	0.00	0.62
G -B	0.62	121	C	0.00	0.62
Bottom Chords					
A -S1	0.65	6108	T	0.30	0.35
S1-E	0.39	6165	T	0.31	0.08
E -D	0.79	6150	T	0.30	0.49
D -C	0.60	1314	T	0.17	0.43
Webs					
E -F	0.32	1194	T		
F -D	0.84	4658	C		2 Br
D -G	0.56	1785	T		
G -C	0.45	2613	C		
C -B	0.13	426	T		

TL Defl -0.70" in S1-E L/291
LL Defl -0.35" in S1-E L/583

Hz Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.09"

Shear // Grain in E -D 0.90

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

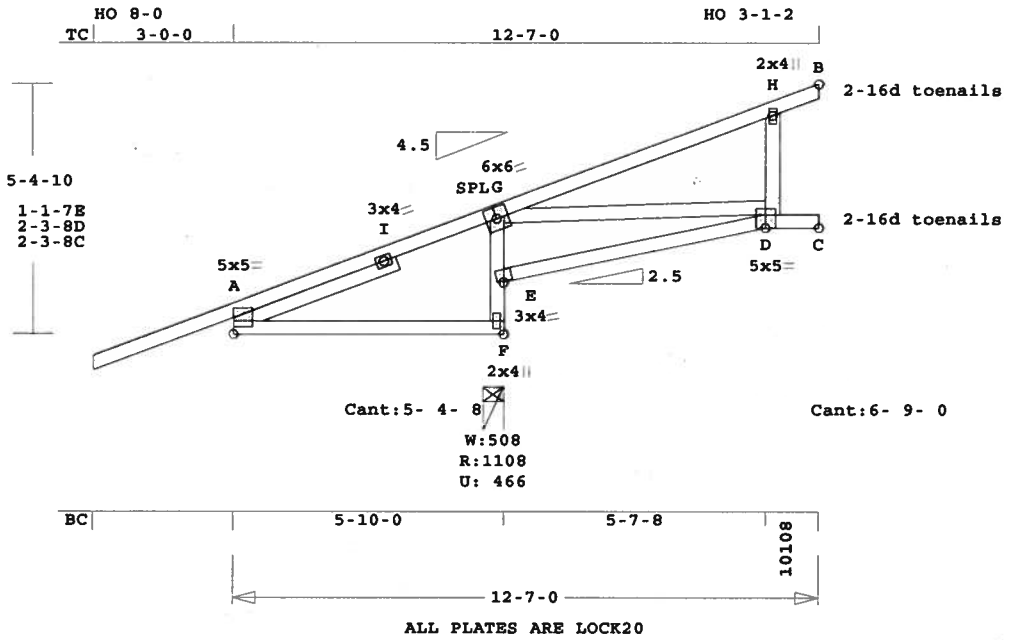
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0 Ctr	Ctr		0.93
F	LOCK	7.0x10.0-0.2	0.8		0.96
G	LOCK	4.0x 5.0 Ctr	Ctr		0.96
B	LOCK	2.0x 4.0 Ctr	Ctr		0.49
S1	LOCK	6.0x10.0 Ctr	0.2		0.77
E	LOCK	3.0x 4.0 Ctr	-1.4		0.92
D	LOCK	7.0x 8.0-0.3	-1.6		0.96
C	LOCK	4.0x 5.0 Ctr	Ctr		0.73

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6270 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R199*	10	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.64	2x 4	SP-#2
BC	0.37	2x 4	SP-#2
CW	0.89	2x 4	SP-#2
WB	0.17	2x 4	SP-#3
SL	0.17	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 7- 0
BC Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
F	1109	466	5- 8	1- 8
			Hz =	212
C	77	72	3- 8	1- 8
B	72	0	3- 8	1- 8
			Hz =	114

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -I	0.25	192 T	0.02	0.23
I -G	0.64	486 C	0.08	0.56
G -H	0.56	125 C	0.00	0.56
H -B	0.09	130 T	0.00	0.09
--------Bottom Chords---------				
A -F	0.22	487 T	0.06	0.16
E -D	0.37	581 C	0.00	0.37
D -C	0.18	0 T	0.00	0.18
----------Chord-Webs----------				
F -E	0.89	864 C	0.00	0.89
E -G	0.50	644 C	0.00	0.50
-------------Webs-------------				
G -D	0.17	550 T		
D -H	0.06	269 T		
-----------Sliders-----------				
A -I	0.17	694 C		

TL Defl -0.03" in E -D L/999
LL Defl -0.01" in E -D L/999
LL Cant -0.11" in A -F L/611

Hz Disp	LL	DL	TL
Jt B	0.04"	0.04"	0.09"

Shear // Grain in F -E 0.51

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.89
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	6.0x 6.0	Ctr	Ctr	0.87
H	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	2.0x 4.0	Ctr	Ctr	0.58
E	LOCK	3.0x 4.0	Ctr	Ctr	0.64
D	LOCK	5.0x 5.0	Ctr	-1.0	0.77

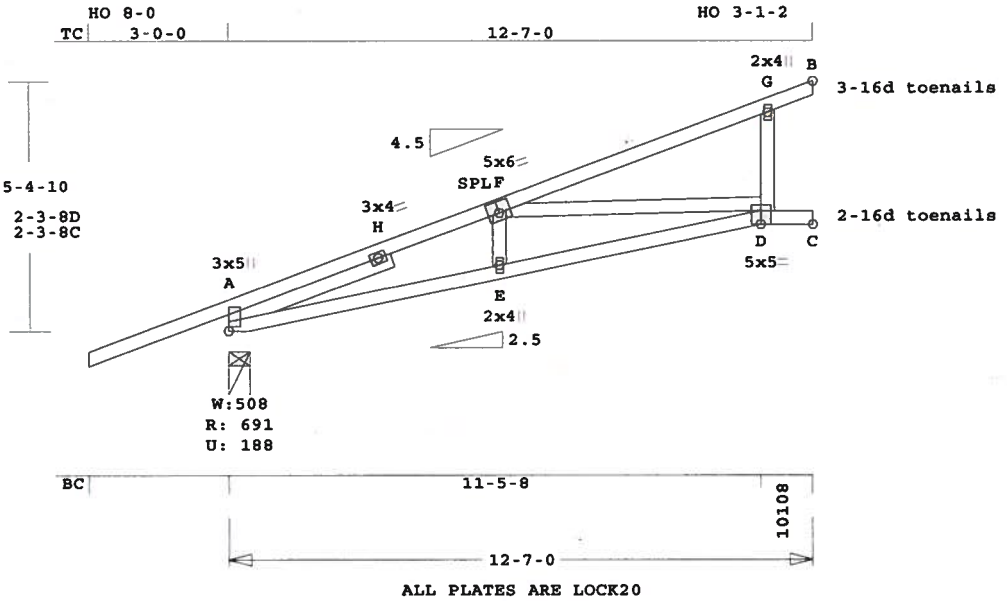
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 864 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R198*	4	SP	120700	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 79.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.56	2x 4	SP-#2
WB	0.59	2x 4	SP-#3
SL	0.12	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 7- 0
BC	Cont.	0- 0- 0	12- 7- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	692	188	5- 8	1- 8
			Hz =	212
C	208	122	3- 8	1- 8
B	299	84	3- 8	1- 8
			Hz =	114

Membr CSI P Lbs Axl-CSI-Bnd

----------Top Chords----------

A -H	0.11	712 C	0.00	0.11
H -F	0.33	1262 C	0.07	0.26
F -G	0.51	111 C	0.00	0.51
G -B	0.33	119 T	0.01	0.32

--------Bottom Chords---------

A -E	0.31	1220 T	0.20	0.11
E -D	0.56	1211 T	0.20	0.36
D -C	0.36	0 T	0.00	0.36

-------------Webs-------------

E -F	0.06	247 T		
F -D	0.59	1188 C		
D -G	0.06	242 T		

-----------Sliders------------

A -H	0.12	615 C		

TL Defl -0.24" in E -D L/595
LL Defl -0.11" in E -D L/999

Hz Disp	LL	DL	TL
Jt C	0.02"	0.02"	0.04"

Shear // Grain in G -B 0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.80
H	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	5.0x 6.0	-0.2	0.5	0.87
G	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 5.0	Ctr	-1.0	0.78

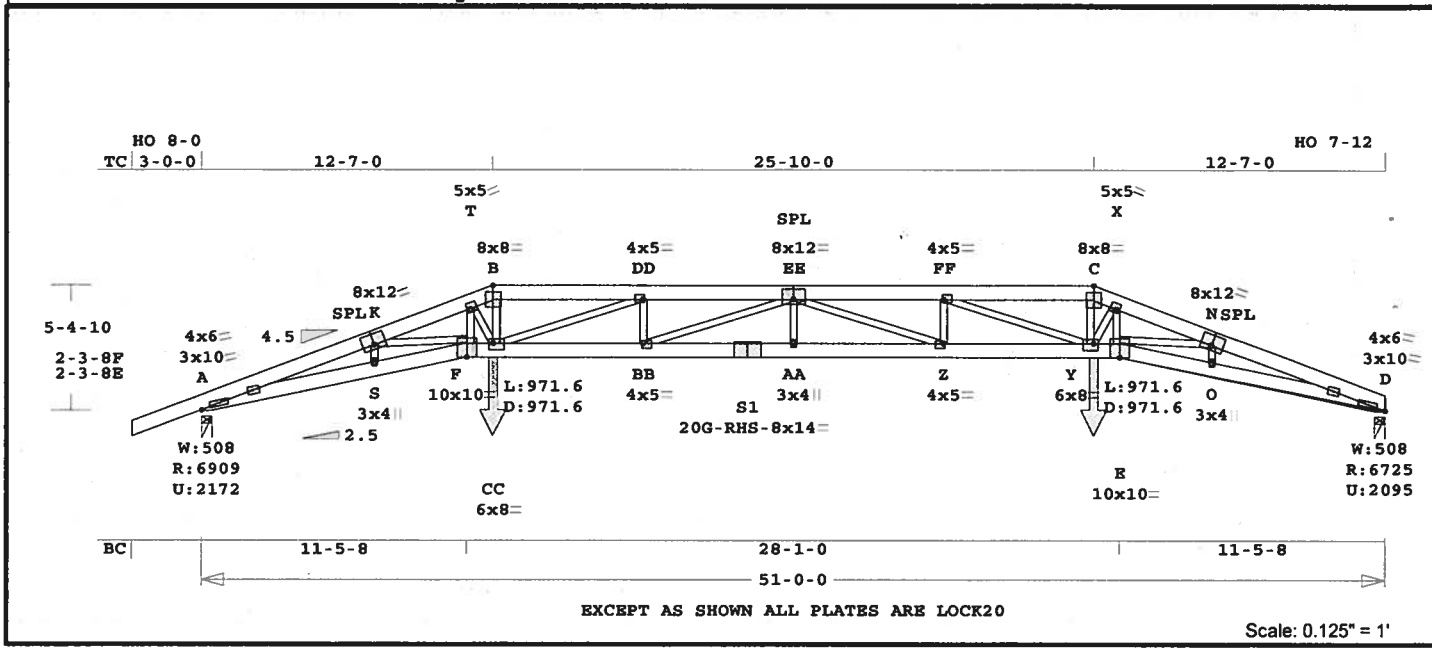
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1262 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R197*	1*3P	HPSC4	510000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 504.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.44	2x 8	SP-2400
BC	0.60	2x 8	SP-2400
WB	0.79	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	106	106	12.6'	38.4'
BC V	972	972	12.6'	CL-LB
BC V	972	972	38.4'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	6909	2173	5- 8	1-15
			Hz =	-91
D	6726	2095	5- 8	1-14
			Hz =	90

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -K	0.31	33166	C	0.14	0.17
K -T	0.44	35561	C	0.16	0.28
T -B	0.39	32367	C	0.12	0.27
B -DD	0.23	31618	C	0.13	0.10
DD-EE	0.32	37845	C	0.19	0.13
EE-FF	0.32	37845	C	0.19	0.13
FF-C	0.23	31616	C	0.13	0.10
C -X	0.39	32361	C	0.12	0.27
X -N	0.44	35568	C	0.16	0.28
N -D	0.31	33221	C	0.14	0.17
--------Bottom Chords---------					
A -S	0.58	31432	T	0.36	0.22
S -F	0.58	31645	T	0.36	0.22
F -CC	0.44	32925	T	0.38	0.06
CC-BB	0.54	37861	T	0.43	0.11
BB-S1	0.60	40397	T	0.46	0.14
S1-AA	0.58	40397	T	0.46	0.12
AA-Z	0.60	40397	T	0.46	0.14
Z -Y	0.54	37861	T	0.43	0.11
Y -E	0.44	32927	T	0.38	0.06
E -O	0.58	31695	T	0.36	0.22
O -D	0.58	31485	T	0.36	0.22
----------Webs----------					
S -K	0.05	961	C		
K -F	0.24	2332	T		
F -T	0.76	7119	T		
T -CC	0.14	2737	C		
CC-B	0.79	7405	T		
CC-DD	0.39	6694	C		
BB-DD	0.24	2337	T		
BB-EE	0.15	2717	C		
AA-EE	0.16	1496	T		
EE-Z	0.15	2717	C		
Z -FF	0.25	2338	T		
FF-Y	0.39	6696	C		
Y -C	0.79	7393	T		
Y -X	0.14	2744	C		
E -X	0.76	7145	T		
E -N	0.24	2283	T		
O -N	0.04	948	C		

TL Defl -2.67" in S1-AA L/224
LL Defl -1.33" in S1-AA L/450

Hz Disp	LL	DL	TL
Jt D	0.59"	0.59"	1.19"

Shear // Grain in C -X 0.73

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x10.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.80
K	LOCK	8.0x12.0	-0.6	1.5	0.97
T	LOCK	5.0x 5.0	-1.2	0.3	0.87
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
DD	LOCK	4.0x 5.0	Ctr	0.5	0.97
EE	LOCK	8.0x12.0	Ctr	1.5	0.99
FF	LOCK	4.0x 5.0	Ctr	0.5	0.97
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
X	LOCK	5.0x 5.0	1.2	0.3	0.87
N	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	3.0x10.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.80
S	LOCK	3.0x 4.0	Ctr	Ctr	0.36
F	LOCK	10.0x10.0	Ctr	-2.2	0.83
CC	LOCK	6.0x 8.0	Ctr	-0.6	0.79
BB	LOCK	4.0x 5.0	Ctr	-0.5	0.69
S1	RHS	8.0x14.0	Ctr	0.4	0.94
AA	LOCK	3.0x 4.0	Ctr	Ctr	0.36
Z	LOCK	4.0x 5.0	Ctr	-0.5	0.69
Y	LOCK	6.0x 8.0	Ctr	-0.6	0.79
E	LOCK	10.0x10.0	Ctr	-2.2	0.83
O	LOCK	3.0x 4.0	Ctr	Ctr	0.36

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Closed Face
Setback 12- 7- 0
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

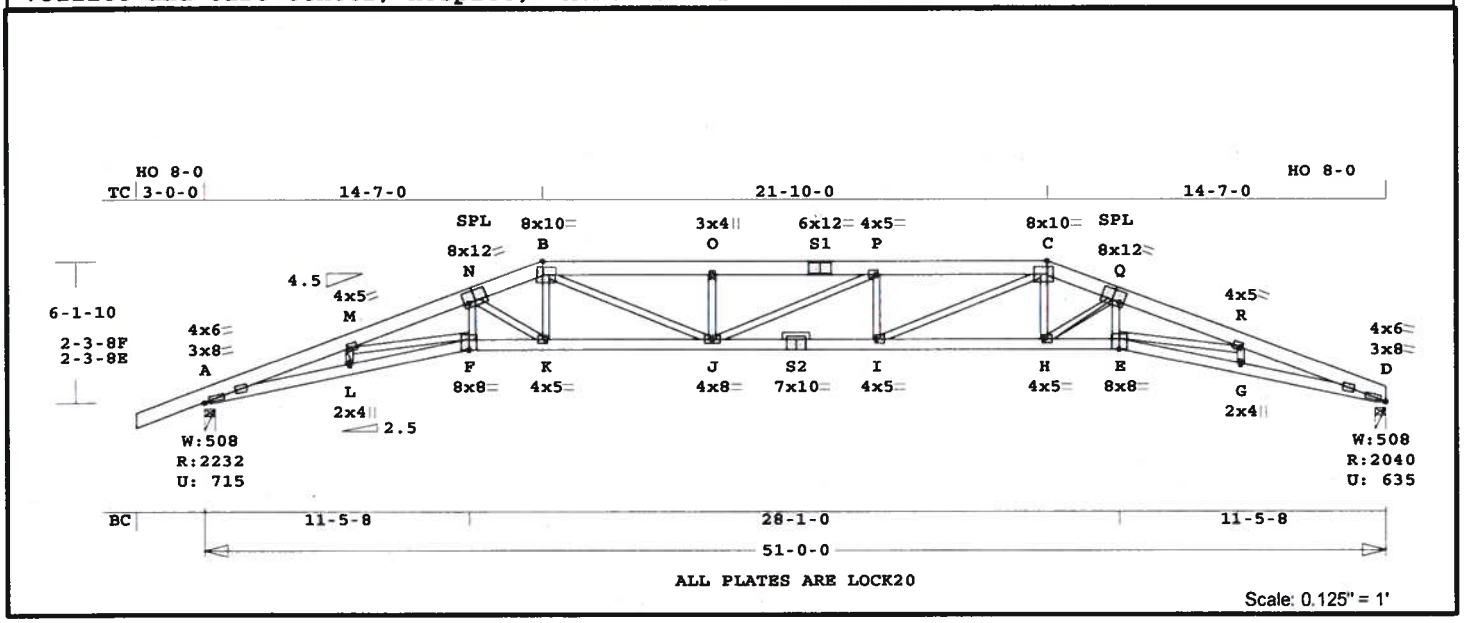
	Rows	Nails	----Spacing (In)---- Screws	Bolts
TC	2	12	24	0
BC	2	12	24	0
WB	1	8	8	

Web Connection Exception --
Use 4" spacing for screws or nails on the following webs
C- D F- Y
No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 37845 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R196*	1	HPSC4	510000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 466.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.32	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.58	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	51- 0- 0
BC	Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	2232	715	5- 8	1-14
			Hz =	-102
D	2040	636	5- 8	1-11
			Hz =	102

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.26	8461	C	0.08	0.18
M -N	0.32	8735	C	0.09	0.23
N -B	0.16	6722	C	0.05	0.11
B -O	0.15	7625	C	0.07	0.08
O -S1	0.16	7625	C	0.07	0.09
S1-P	0.15	7625	C	0.07	0.08
P -C	0.16	7612	C	0.07	0.09
C -Q	0.16	6722	C	0.05	0.11
Q -R	0.32	8732	C	0.09	0.23
R -D	0.26	8459	C	0.08	0.18
--------Bottom Chords---------					
A -L	0.54	7979	T	0.36	0.18
L -F	0.54	8018	T	0.36	0.18
F -K	0.42	8129	T	0.37	0.05
K -J	0.33	6304	T	0.28	0.05
J -S2	0.40	7614	T	0.34	0.06
S2-I	0.40	7614	T	0.34	0.06
I -H	0.33	6304	T	0.28	0.05
H -E	0.42	8126	T	0.37	0.05
E -G	0.54	8015	T	0.36	0.18
G -D	0.54	7977	T	0.36	0.18
----------Webs----------					
L -M	0.03	149	T		
M -F	0.11	360	T		
F -N	0.58	1810	T		
N -K	0.49	2158	C		
K -B	0.42	1310	T		
B -J	0.48	1435	T		
J -O	0.08	421	C		
J -P	0.09	119	T		
I -P	0.08	416	C		
I -C	0.48	1421	T		
H -C	0.42	1311	T		
H -Q	0.49	2153	C		
E -Q	0.58	1808	T		
E -R	0.11	359	T		
G -R	0.03	149	T		

TL Defl -1.71" in S2-I L/350
LL Defl -0.85" in S2-I L/707

Hz Disp	LL	DL	TL
Jt D	0.44"	0.44"	0.88"

Shear // Grain in A -M 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

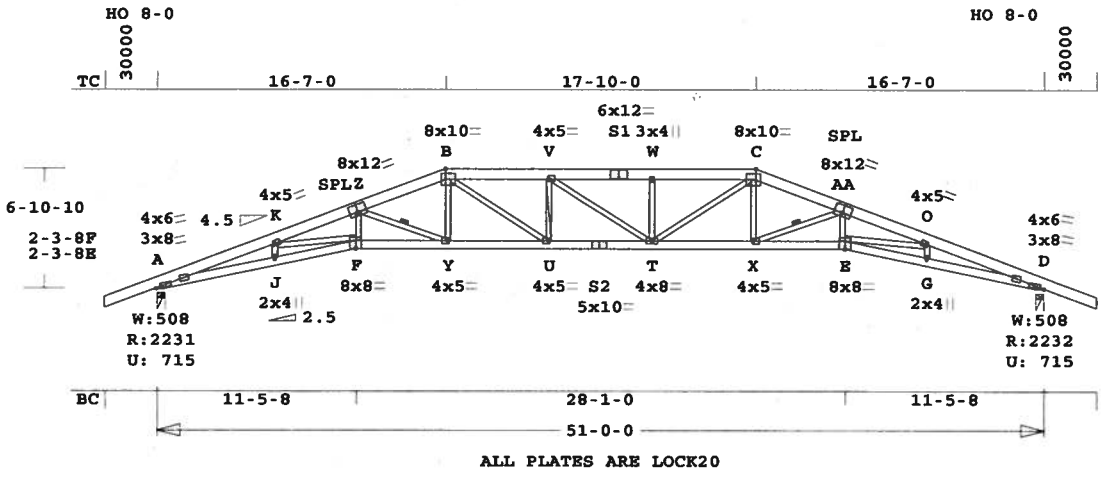
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.74
M	LOCK	4.0x 5.0	-0.5	0.2	0.85
N	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	3.0x 4.0	Ctr	Ctr	0.36
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
P	LOCK	4.0x 5.0	Ctr	0.5	0.64
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.85
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.74
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr	-1.5	0.77
K	LOCK	4.0x 5.0	Ctr	Ctr	0.69
J	LOCK	4.0x 8.0	Ctr	Ctr	0.62
S2	LOCK	7.0x10.0	Ctr	0.8	0.87
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	4.0x 5.0	Ctr	Ctr	0.69
E	LOCK	8.0x 8.0	Ctr	-1.5	0.77
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8735 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R195*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 483.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.58	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

WB 1 rows CLB on Z -Y
WB 1 rows CLB on X -AA
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2232	715	5- 8	1-14
			Hz = -114	
D	2232	715	5- 8	1-14
			Hz = 117	

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -K	0.27	8524 C	0.08 0.19
K -Z	0.31	8694 C	0.09 0.22
Z -B	0.17	5909 C	0.04 0.13
B -V	0.09	6247 C	0.04 0.05
V -S1	0.10	6252 C	0.04 0.06
S1-W	0.08	6252 C	0.04 0.04
W -C	0.09	6252 C	0.04 0.05
C -AA	0.17	5910 C	0.04 0.13
AA-O	0.31	8696 C	0.09 0.22
O -D	0.27	8525 C	0.08 0.19
Bottom Chords			
A -J	0.54	8043 T	0.36 0.18
J -F	0.54	8080 T	0.36 0.18
F -Y	0.41	8085 T	0.37 0.04
Y -U	0.28	5539 T	0.25 0.03
U -S2	0.31	6245 T	0.28 0.03
S2-T	0.32	6245 T	0.28 0.04
T -X	0.28	5539 T	0.25 0.03
X -E	0.41	8086 T	0.37 0.04
E -G	0.54	8081 T	0.36 0.18
G -D	0.54	8044 T	0.36 0.18
Webs			
J -K	0.03	138 T	
K -F	0.08	256 T	
F -Z	0.58	1816 T	
Z -Y	0.51	2729 C	1 Br
Y -B	0.37	1162 T	
B -U	0.27	844 T	
U -V	0.08	349 C	
V -T	0.07	110 T	
T -W	0.08	352 C	
T -C	0.27	850 T	
X -C	0.37	1161 T	
X -AA	0.51	2731 C	1 Br
E -AA	0.58	1817 T	
E -O	0.08	257 T	
G -O	0.03	138 T	

TL Defl -1.50" in U -S2 L/400
LL Defl -0.74" in U -S2 L/807

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Shear // Grain in A -K 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

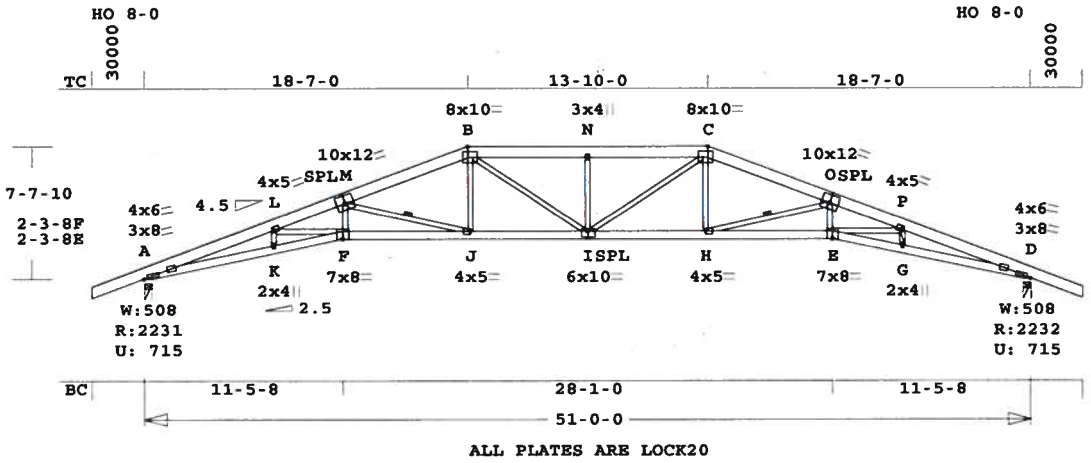
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
K	LOCK	4.0x 5.0	-0.5	0.2	0.84
Z	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
V	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
W	LOCK	3.0x 4.0	Ctr	Ctr	0.36
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
AA	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.84
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr	-1.5	0.76
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.89
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.93
T	LOCK	4.0x 8.0	Ctr	Ctr	0.52
X	LOCK	4.0x 5.0	Ctr	Ctr	0.90
E	LOCK	8.0x 8.0	Ctr	-1.5	0.76
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8696 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R194*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 479.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.57	2x 6	SP-2400
WB	0.80	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on M -J
WB 1 rows CLB on H -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-129
D	2233	716	5- 8	1-14
			Hz =	131

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.30	8497	C	0.08	0.22
L -M	0.31	8764	C	0.09	0.22
M -B	0.17	5302	C	0.03	0.14
B -N	0.09	5381	C	0.03	0.06
N -C	0.09	5381	C	0.03	0.06
C -O	0.17	5302	C	0.03	0.14
O -P	0.31	8765	C	0.09	0.22
P -D	0.30	8498	C	0.08	0.22
Bottom Chords					
A -K	0.57	8025	T	0.36	0.21
K -F	0.57	8069	T	0.36	0.21
F -J	0.41	8087	T	0.37	0.04
J -I	0.26	4963	T	0.22	0.04
I -H	0.26	4963	T	0.22	0.04
H -E	0.41	8087	T	0.37	0.04
E -G	0.57	8070	T	0.36	0.21
G -D	0.57	8026	T	0.36	0.21
Webs					
K -L	0.03	142	T		
L -F	0.11	361	T		
F -M	0.58	1814	T		
M -J	0.80	3238	C		1 Br
J -B	0.35	1106	T		
B -I	0.22	502	T		
I -N	0.14	444	C		
I -C	0.22	501	T		
H -C	0.35	1106	T		
H -O	0.80	3239	C		1 Br
E -O	0.58	1814	T		
E -P	0.11	361	T		
G -P	0.03	142	T		

TL Defl -1.41" in I -H L/425
LL Defl -0.70" in I -H L/857

Hz Disp	LL	DL	TL
Jt D	0.42"	0.42"	0.85"

Shear // Grain in A -L 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

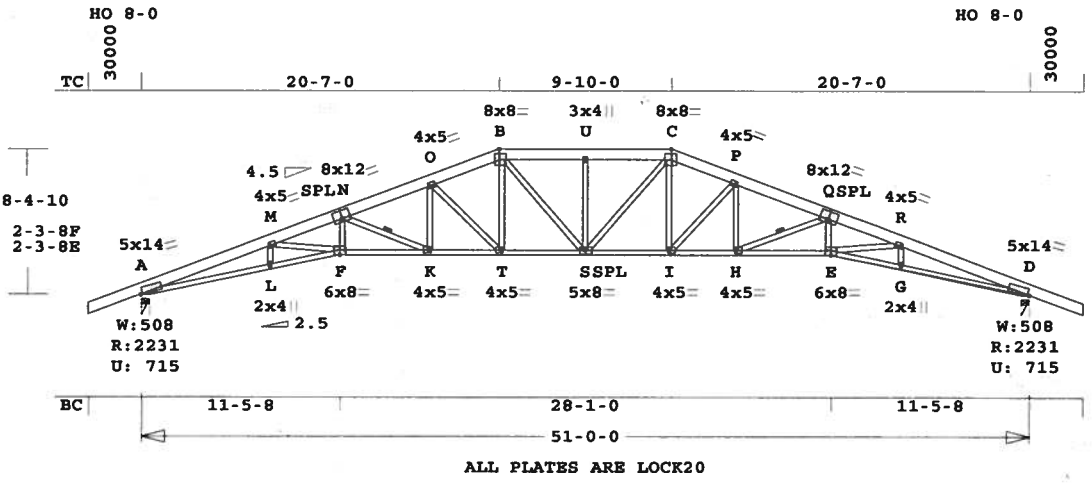
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.75
L	LOCK	4.0x 5.0-0.5	0.2		0.83
M	LOCK	10.0x12.0-0.2	0.6		0.97
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
N	LOCK	3.0x 4.0	Ctr	Ctr	0.36
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	10.0x12.0	0.2	0.6	0.97
P	LOCK	4.0x 5.0	0.5	0.2	0.83
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.75
K	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	7.0x 8.0	Ctr-2.0		0.92
J	LOCK	4.0x 5.0-0.5	Ctr		0.95
I	LOCK	6.0x10.0	Ctr-1.2		0.97
H	LOCK	4.0x 5.0	0.5	Ctr	0.95
E	LOCK	7.0x 8.0	Ctr-2.0		0.92
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8765 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R193	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 461.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 8	SP-2400
BC	0.77	2x 4	SP-2400
WB	0.70	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-143
D	2231	715	5- 8	1-14
			Hz =	145

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -M	0.42	7941 C		0.07	0.35
M -N	0.43	8399 C		0.08	0.35
N -O	0.15	5803 C		0.04	0.11
O -B	0.06	4685 C		0.05	0.01
B -U	0.06	4576 C		0.05	0.01
U -C	0.06	4576 C		0.05	0.01
C -P	0.06	4685 C		0.05	0.01
P -Q	0.15	5708 C		0.04	0.11
Q -R	0.43	8394 C		0.08	0.35
R -D	0.42	7947 C		0.07	0.35
--------Bottom Chords---------					
A -L	0.77	7440 T		0.53	0.24
L -F	0.71	7458 T		0.53	0.18
F -K	0.64	7840 T		0.56	0.08
K -T	0.42	5453 T		0.39	0.03
T -S	0.36	4381 T		0.31	0.05
S -I	0.36	4379 T		0.31	0.05
I -H	0.40	5365 T		0.38	0.02
H -E	0.64	7833 T		0.56	0.08
E -G	0.70	7464 T		0.53	0.17
G -D	0.77	7446 T		0.53	0.24
------------Webs------------					
L -M	0.03	149 T			
M -F	0.20	652 T			
F -N	0.50	1566 T			
N -K	0.49	2621 C			1 Br
K -O	0.37	1156 T			
O -T	0.70	1486 C			
T -B	0.36	1138 T			
B -S	0.10	294 T			
S -U	0.15	339 C			
S -C	0.10	296 T			
I -C	0.36	1145 T			
I -P	0.65	1433 C			
H -P	0.36	1134 T			
H -Q	0.52	2684 C			1 Br
E -Q	0.50	1570 T			
E -R	0.20	639 T			
G -R	0.04	150 T			

TL Defl -1.65" in S -I L/363
LL Defl -0.82" in S -I L/734

Hz Disp	LL	DL	TL
Jt D	0.53"	0.54"	1.07"

Shear // Grain in A -M 0.33

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

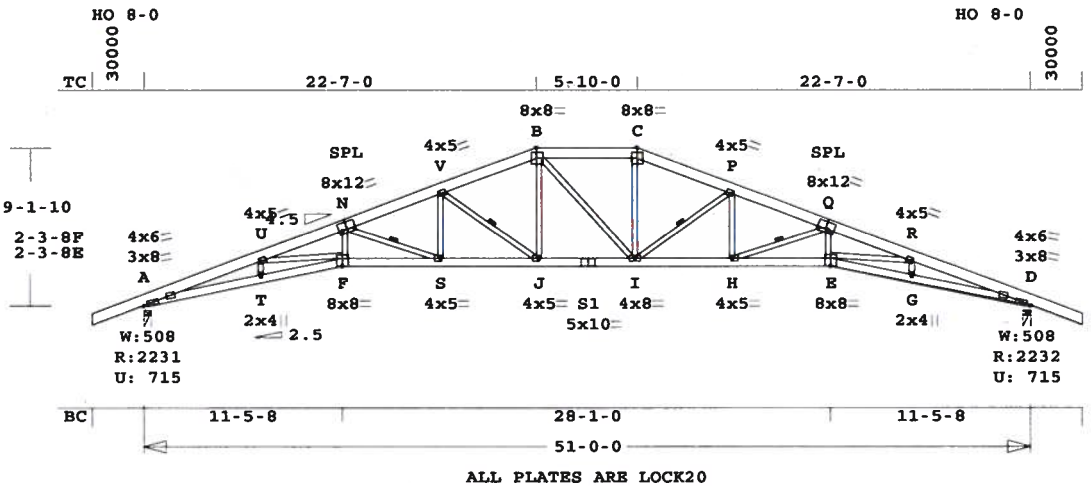
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x14.0	0.9	1.6	0.99
M	LOCK	4.0x 5.0	-0.5	0.2	0.57
N	LOCK	8.0x12.0	-0.6	1.5	0.97
O	LOCK	4.0x 5.0	-0.5	0.2	0.71
B	LOCK	8.0x 8.0	-0.5	Ctr	0.84
U	LOCK	3.0x 4.0	Ctr	Ctr	0.36
C	LOCK	8.0x 8.0	1.0	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.69
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.57
D	LOCK	5.0x14.0	-0.9	1.6	0.99
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	6.0x 8.0	Ctr	-0.5	0.99
K	LOCK	4.0x 5.0	Ctr	Ctr	0.86
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S	LOCK	5.0x 8.0	Ctr	-0.5	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	4.0x 5.0	Ctr	Ctr	0.88
E	LOCK	6.0x 8.0	Ctr	-0.5	0.99
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8399 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R192*	1	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 499.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.54	2x 6	SP-2400
WB	0.58	2x 4	SP-#3

Brace trusses as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -S
WB 1 rows CLB on V -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-158
D	2232	715	5- 8	1-14
			Hz =	158

Membr	CSI	P Lbs	Axl-CSI-Bnd	
		Top Chords		
A -U	0.28	8517 C	0.08	0.20
U -N	0.31	8730 C	0.09	0.22
N -V	0.16	5737 C	0.04	0.12
V -B	0.08	4310 C	0.05	0.03
B -C	0.07	4041 C	0.01	0.06
C -P	0.08	4317 C	0.05	0.03
P -Q	0.16	5739 C	0.04	0.12
Q -R	0.31	8728 C	0.09	0.22
R -D	0.28	8516 C	0.08	0.20
		Bottom Chords		
A -T	0.54	8040 T	0.36	0.18
T -F	0.54	8077 T	0.36	0.18
F -S	0.41	8128 T	0.37	0.04
S -J	0.27	5403 T	0.24	0.03
J -S1	0.19	4022 T	0.18	0.01
S1-I	0.22	4022 T	0.18	0.04
I -H	0.28	5407 T	0.24	0.04
H -E	0.41	8125 T	0.37	0.04
E -G	0.54	8075 T	0.36	0.18
G -D	0.54	8038 T	0.36	0.18
		Webs		
T -U	0.03	140 T		
U -F	0.09	303 T		
F -N	0.58	1823 T		
N -S	0.58	2926 C		1 Br
S -V	0.36	1147 T		
V -J	0.39	1716 C		1 Br
J -B	0.36	1151 T		
B -I	0.12	148 T		
I -C	0.37	1166 T		
I -P	0.39	1719 C		1 Br
H -P	0.36	1128 T		
H -Q	0.58	2918 C		1 Br
E -Q	0.58	1825 T		
E -R	0.09	302 T		
G -R	0.03	140 T		

TL Defl -1.35" in S -J L/443
LL Defl -0.67" in S -J L/894

Hz Disp	LL	DL	TL
Jt D	0.41"	0.41"	0.82"

Shear // Grain in A -U 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

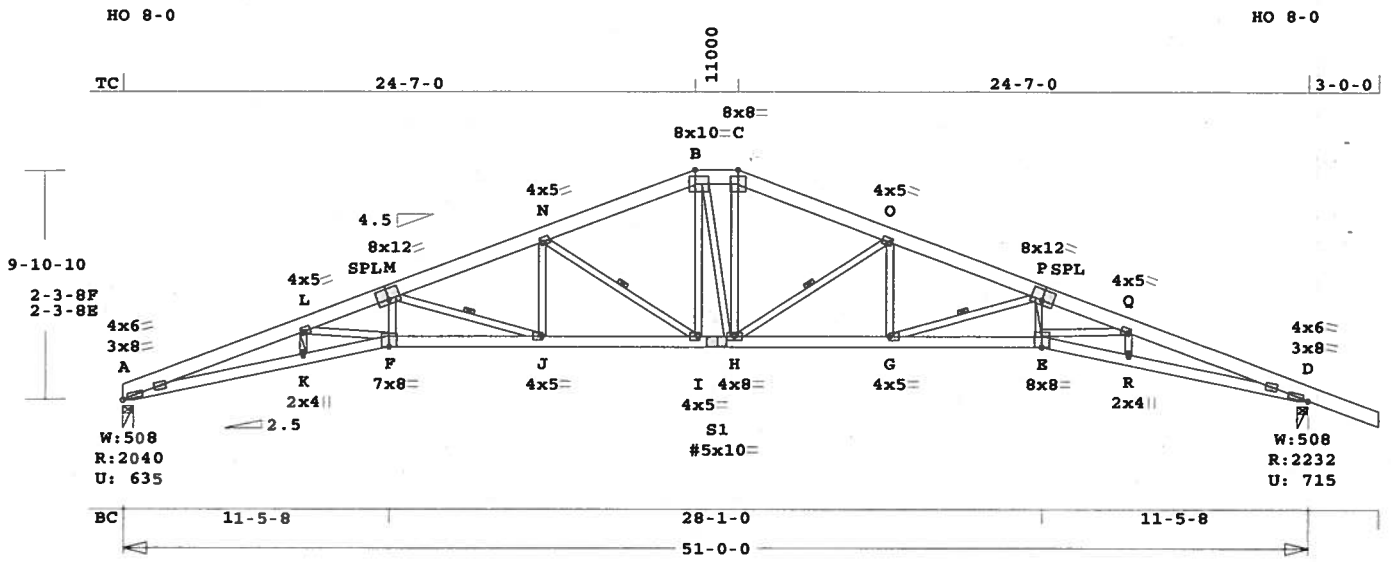
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.75
U	LOCK	4.0x 5.0-0.5	0.2		0.63
N	LOCK	8.0x12.0-0.6	1.5		0.97
V	LOCK	4.0x 5.0-0.5	0.2		0.82
B	LOCK	8.0x 8.0-1.0	Ctr		0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.82
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.75
T	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr-1.5		0.78
S	LOCK	4.0x 5.0	Ctr	Ctr	0.96
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.60
H	LOCK	4.0x 5.0	Ctr	Ctr	0.96
E	LOCK	8.0x 8.0	Ctr-1.5		0.78
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8730 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R191	1	HPSC4	510000	4.5	0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 494.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.57	2x 6	SP-2400
WB	0.68	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on M -J
WB 1 rows CLB on N -I
WB 1 rows CLB on H -O
WB 1 rows CLB on G -P
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2040	636	5- 8	1-11
			Hz =	-172
D	2233	716	5- 8	1-14
			Hz =	172

Membr	CSI	P Lbs	Axl	CSI-Bnd
--------Top Chords--------				
A -L	0.30	8532	C	0.08 0.22
L -M	0.31	8752	C	0.09 0.22
M -N	0.16	5499	C	0.03 0.13
N -B	0.08	3903	C	0.04 0.04
B -C	0.04	3661	C	0.04 0.00
C -O	0.09	3919	C	0.04 0.05
O -P	0.17	5494	C	0.03 0.14
P -Q	0.30	8692	C	0.09 0.21
Q -D	0.30	8590	C	0.09 0.21
--------Bottom Chords--------				
A -K	0.56	8067	T	0.36 0.20
K -F	0.57	8110	T	0.37 0.20
F -J	0.40	8101	T	0.37 0.03
J -I	0.27	5183	T	0.23 0.04
I -S1	0.19	3631	T	0.16 0.03
S1-H	0.22	3631	T	0.16 0.06
H -G	0.29	5184	T	0.23 0.06
G -E	0.40	8096	T	0.37 0.03
E -R	0.55	8161	T	0.37 0.18
R -D	0.55	8124	T	0.37 0.18
----------Webs----------				
K -L	0.03	147	T	
L -F	0.10	315	T	
F -M	0.58	1815	T	
M -J	0.68	3081	C	1 Br
J -N	0.36	1125	T	
N -I	0.50	1852	C	1 Br
I -B	0.33	1045	T	
B -H	0.07	165	T	
H -C	0.34	1063	T	
H -O	0.50	1840	C	1 Br
G -O	0.34	1084	T	
G -P	0.68	3075	C	1 Br
E -P	0.58	1831	T	
E -Q	0.06	193	T	
R -Q	0.03	136	T	

TL Defl -1.38" in J -I L/434
LL Defl -0.68" in J -I L/878

Hz Disp	LL	DL	TL
Jt D	0.42"	0.42"	0.84"

Shear // Grain in A -L 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
L	LOCK	4.0x 5.0	-0.5	0.2	0.57
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	4.0x 5.0	-0.5	0.2	0.88
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
O	LOCK	4.0x 5.0	0.5	0.2	0.87
P	LOCK	8.0x12.0	0.6	1.5	0.97
Q	LOCK	4.0x 5.0	0.5	0.2	0.59
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.77
K	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	7.0x 8.0	Ctr	-2.0	0.88
J	LOCK	4.0x 5.0	-0.5	Ctr	0.89
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1#	LOCK	5.0x10.0	-0.4	Ctr	0.82
H	LOCK	4.0x 8.0	Ctr	Ctr	0.67
G	LOCK	4.0x 5.0	0.5	Ctr	0.89
E	LOCK	8.0x 8.0	Ctr	-1.5	0.86
R	LOCK	2.0x 4.0	Ctr	Ctr	0.54

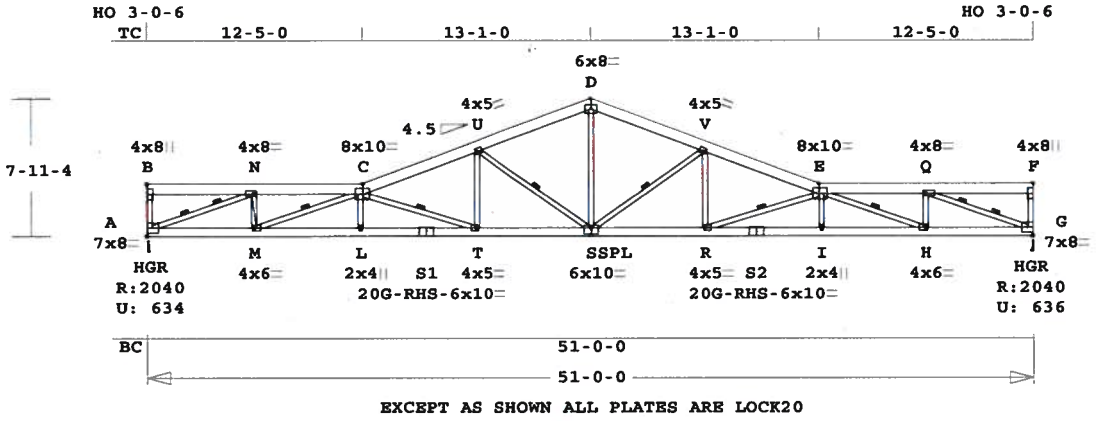
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8752 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R183*	1	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 497.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	Size	Lumber
TC	0.11	2x 8	SP-2400
BC	0.49	2x 6	SP-2400
WB	0.81	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	51- 0- 0
BC	Cont.	0- 0- 0	51- 0- 0

WB 2 rows CLB on A -N
WB 1 rows CLB on M -C
WB 1 rows CLB on C -T
WB 1 rows CLB on U -S
WB 1 rows CLB on S -V
WB 1 rows CLB on R -E
WB 1 rows CLB on E -H
WB 2 rows CLB on Q -G
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	634	3- 8	1-11
			Hz =	-155
G	2040	637	3- 8	1-11
			Hz =	96

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
B -N	0.07	34	T 0.00	0.07
N -C	0.08	4352	C 0.02	0.06
C -U	0.11	5258	C 0.03	0.08
U -D	0.09	3749	C 0.04	0.05
D -V	0.09	3749	C 0.04	0.05
V -E	0.11	5258	C 0.03	0.08
E -Q	0.08	4353	C 0.02	0.06
Q -F	0.07	0	T 0.00	0.07
Bottom Chords				
A -M	0.22	4352	T 0.19	0.03
M -L	0.49	7857	T 0.35	0.14
L -S1	0.49	7858	T 0.35	0.14
S1-T	0.42	7858	T 0.35	0.07
T -S	0.28	4945	T 0.22	0.06
S -R	0.28	4945	T 0.22	0.06
R -S2	0.42	7858	T 0.35	0.07
S2-I	0.49	7858	T 0.35	0.14
I -H	0.49	7858	T 0.35	0.14
H -G	0.22	4352	T 0.19	0.03
Webs				
A -B	0.05	189	C WindLd	
A -N	0.81	4708	C	2 Br
M -N	0.46	1443	T	
M -C	0.79	3723	C	1 Br
L -C	0.04	156	T	
C -T	0.71	3061	C	1 Br
T -U	0.36	1148	T	
U -S	0.50	1809	C	1 Br
S -D	0.73	2289	T	
S -V	0.50	1809	C	1 Br
R -V	0.36	1148	T	
R -E	0.71	3061	C	1 Br
I -E	0.04	156	T	
E -H	0.79	3723	C	1 Br
H -Q	0.46	1443	T	
Q -G	0.81	4708	C	2 Br
G -F	0.03	189	C	

LL Defl -0.57" in R -S2 L/999
TL Defl -1.16" in R -S2 L/522
Shear // Grain in C -U 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

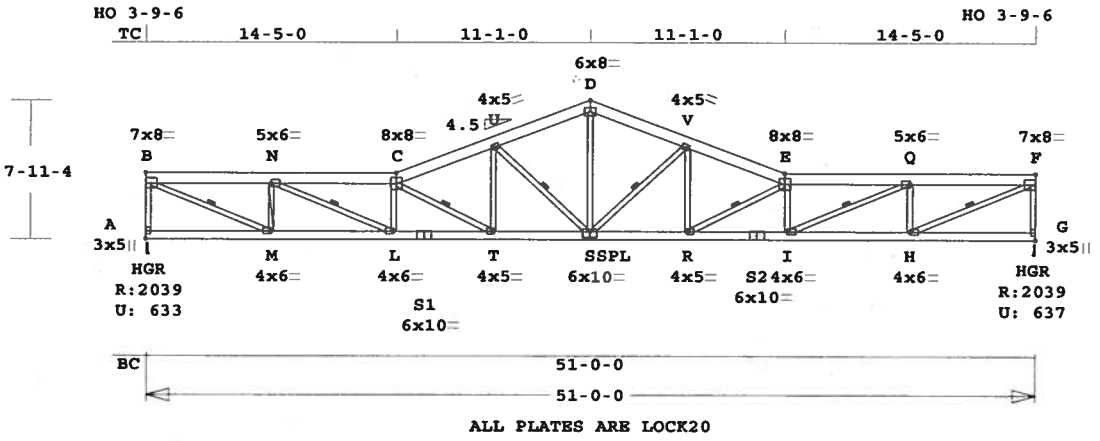
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	4.0x 8.0	0.2	Ctr	0.76
N	LOCK	4.0x 8.0-0.5	0.5		0.83
C	LOCK	8.0x10.0-0.5	Ctr		0.85
U	LOCK	4.0x 5.0-0.5	0.2		0.57
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
V	LOCK	4.0x 5.0	0.5	0.2	0.57
E	LOCK	8.0x10.0	0.5	Ctr	0.85
Q	LOCK	4.0x 8.0	0.5	0.5	0.83
F	LOCK	4.0x 8.0-0.2	Ctr		0.76
A	LOCK	7.0x 8.0	0.5	Ctr	0.83
M	LOCK	4.0x 6.0	Ctr	Ctr	0.89
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
S1	RHS	6.0x10.0	Ctr	0.2	0.75
T	LOCK	4.0x 5.0	Ctr	Ctr	0.89
S	LOCK	6.0x10.0	Ctr	-1.2	0.97
R	LOCK	4.0x 5.0	Ctr	Ctr	0.89
S2	RHS	6.0x10.0	Ctr	0.2	0.75
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
H	LOCK	4.0x 6.0	Ctr	Ctr	0.89
G	LOCK	7.0x 8.0	-0.5	Ctr	0.83

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5258 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R182	1	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 511.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 8	SP-2400
BC	0.93	2x 6	SP-#2
WB	0.90	2x 4	SP-#3
EX B -M		2x 4	SP-#2
EX H -F		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on B -M
WB 1 rows CLB on N -L
WB 1 rows CLB on C -T
WB 1 rows CLB on U -S
WB 1 rows CLB on S -V
WB 1 rows CLB on R -E
WB 1 rows CLB on I -Q
WB 1 rows CLB on H -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	633	3- 8	2- 7
			Hz =	-164
G	2040	638	3- 8	2- 7
			Hz =	84

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -N	0.09	3851 C	0.01 0.08
N -C	0.12	6411 C	0.04 0.08
C -U	0.13	4949 C	0.02 0.11
U -D	0.08	3749 C	0.04 0.04
D -V	0.08	3748 C	0.04 0.04
V -E	0.13	4949 C	0.02 0.11
E -Q	0.12	6412 C	0.04 0.08
Q -F	0.09	3851 C	0.01 0.08
--------Bottom Chords---------			
A -M	0.15	116 T	0.00 0.15
M -L	0.64	3851 T	0.46 0.18
L -S1	0.93	6449 T	0.78 0.15
S1-T	0.92	6449 T	0.78 0.14
T -S	0.62	4673 T	0.56 0.06
S -R	0.62	4674 T	0.56 0.06
R -S2	0.92	6449 T	0.78 0.14
S2-I	0.93	6449 T	0.78 0.15
I -H	0.64	3851 T	0.46 0.18
H -G	0.15	84 T	0.00 0.15
------------Webs------------			
A -B	0.38	1979 C	WindLd
B -M	0.78	4239 T	1 Br
M -N	0.31	1580 C	
N -L	0.90	2818 T	1 Br
L -C	0.22	1130 C	
C -T	0.42	2002 C	1 Br
T -U	0.34	1082 T	
U -S	0.41	1628 C	1 Br
S -D	0.75	2359 T	
S -V	0.41	1629 C	1 Br
R -V	0.34	1083 T	
R -E	0.42	2002 C	1 Br
I -E	0.22	1131 C	
I -Q	0.90	2818 T	1 Br
H -Q	0.31	1580 C	
H -F	0.78	4240 T	1 Br
G -F	0.38	1979 C	

LL Defl -0.47" in R -S2 L/999
TL Defl -0.94" in R -S2 L/640
Shear // Grain in C -U 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

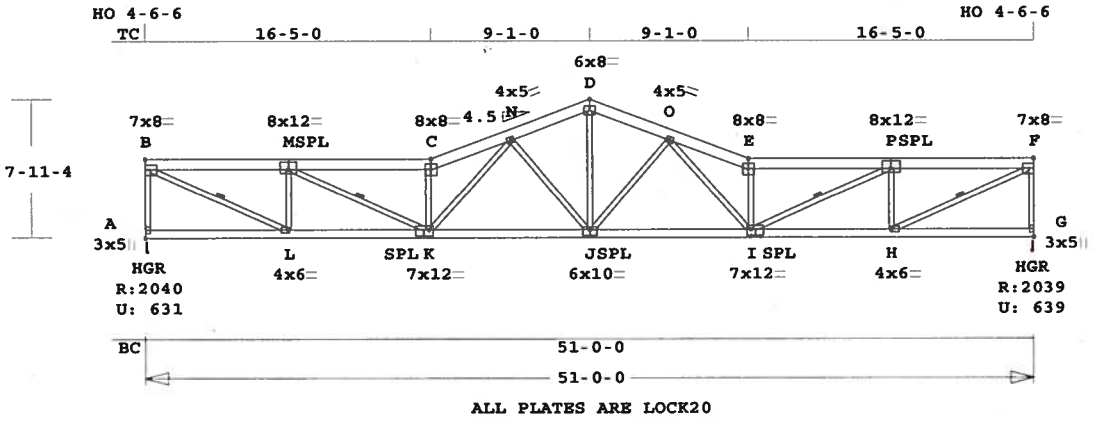
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
N	LOCK	5.0x 6.0	1.0	Ctr	0.98
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
U	LOCK	4.0x 5.0	-0.5	0.2	0.56
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
V	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	5.0x 6.0	-1.0	Ctr	0.98
F	LOCK	7.0x 8.0	-0.5	Ctr	0.90
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
M	LOCK	4.0x 6.0	-1.0	Ctr	0.98
L	LOCK	4.0x 6.0	-1.0	Ctr	0.98
S1	LOCK	6.0x10.0	Ctr	0.2	0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S	LOCK	6.0x10.0	Ctr	-1.2	0.97
R	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	6.0x10.0	Ctr	0.2	0.80
I	LOCK	4.0x 6.0	1.0	Ctr	0.98
H	LOCK	4.0x 6.0	1.0	Ctr	0.98
G	LOCK	3.0x 5.0	Ctr	Ctr	0.90

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6412 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R181*	1	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 511.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 8	SP-2400
BC	0.71	2x 6	SP-#2
WB	1.00	2x 4	SP-#3
EX B -L		2x 4	SP-#2
EX H -F		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on B -L
WB 1 rows CLB on M -K
WB 1 rows CLB on I -P
WB 1 rows CLB on H -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	632	3- 8	2- 7
			Hz =	-172
G	2040	639	3- 8	2- 7
			Hz =	77

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -M	0.10	3535 C	0.01 0.09
M -C	0.10	5629 C	0.03 0.07
C -N	0.10	5985 C	0.07 0.03
N -D	0.06	3762 C	0.04 0.02
D -O	0.06	3762 C	0.04 0.02
O -E	0.10	5985 C	0.07 0.03
E -P	0.10	5629 C	0.03 0.07
P -F	0.10	3535 C	0.01 0.09
--------Bottom Chords---------			
A -L	0.18	113 T	0.00 0.18
L -K	0.53	3535 T	0.42 0.11
K -J	0.71	4387 T	0.53 0.18
J -I	0.71	4387 T	0.53 0.18
I -H	0.53	3535 T	0.42 0.11
H -G	0.18	77 T	0.00 0.18
------------Webs------------			
A -B	0.48	1969 C	WindLd
B -L	0.72	3947 T	1 Br
L -M	0.38	1564 C	
M -K	0.74	2337 T	1 Br
K -C	0.59	2417 C	
K -N	0.62	1956 T	
N -J	1.00	1386 C	
J -D	0.76	2390 T	
J -O	1.00	1386 C	
O -I	0.62	1956 T	
I -E	0.59	2417 C	
I -P	0.74	2337 T	1 Br
H -P	0.38	1564 C	
H -F	0.72	3946 T	1 Br
G -F	0.48	1969 C	

LL Defl -0.40" in K -J L/999
TL Defl -0.83" in K -J L/725
Shear // Grain in B -M 0.16

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
M	LOCK	8.0x12.0	Ctr	1.5	0.99
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
N	LOCK	4.0x 5.0	-0.5	0.2	0.87
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
O	LOCK	4.0x 5.0	0.5	0.2	0.87
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	8.0x12.0	Ctr	1.5	0.99
F	LOCK	7.0x 8.0	-0.5	Ctr	0.90
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
L	LOCK	4.0x 6.0	-0.5	Ctr	0.98
K	LOCK	7.0x12.0	0.5	-0.8	0.97
J	LOCK	6.0x10.0	Ctr	-1.2	0.97
I	LOCK	7.0x12.0	-0.5	-0.8	0.97
H	LOCK	4.0x 6.0	0.5	Ctr	0.98
G	LOCK	3.0x 5.0	Ctr	Ctr	0.90

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5985 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

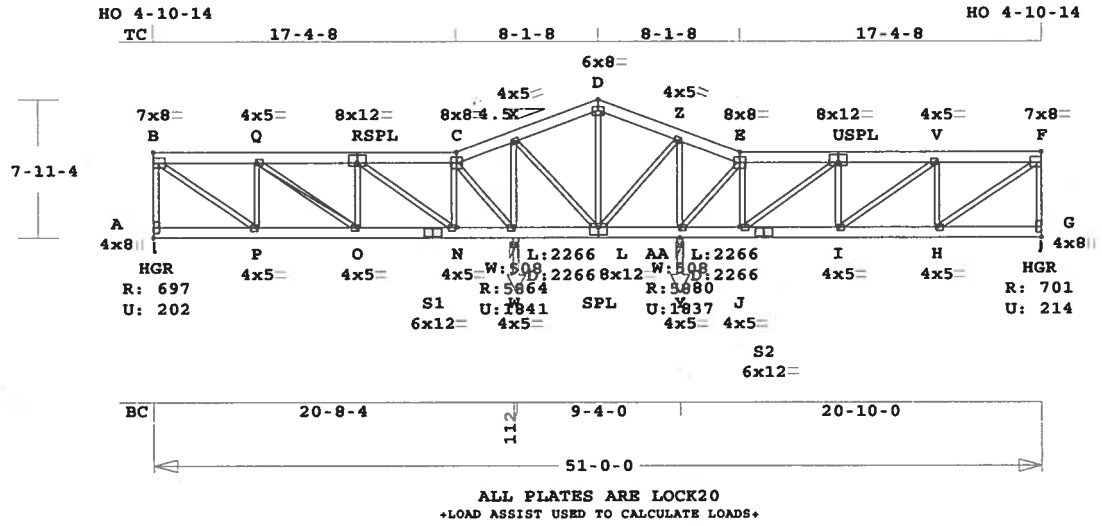
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R180*	1*2P	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish

CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R180*	1*2P	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

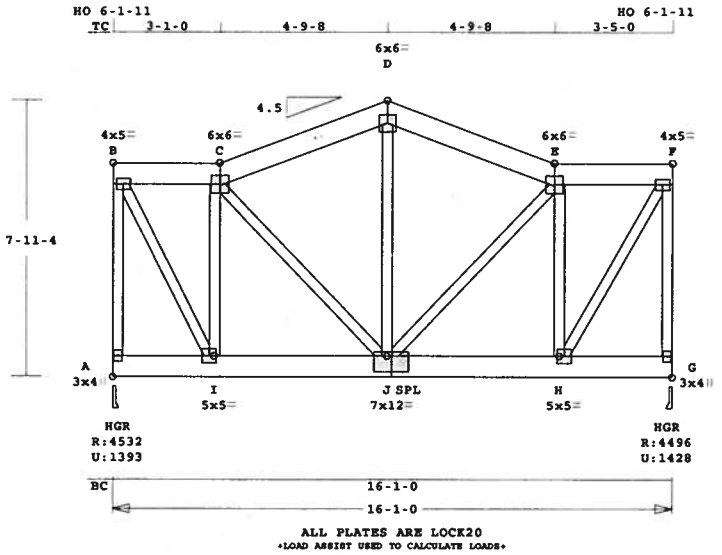
Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 596.3 LBS

```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.03  2x 8  SP-2400
BC  0.02  2x 8  SP-2400
WB  0.13  2x 4  SP-#3

Brace truss as follows:
      O.C.      From      To
 TC Cont.      0- 0- 0 51- 0- 0
 BC Cont.      0- 0- 0 51- 0- 0

Loading   Live   Dead  (psf)
TC         20.0   10.0
BC          0.0   10.0
Total      20.0   20.0   40.0
Spacing                  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.00  Fc=1.00  Ft=1.00
BC Fb=1.00  Fc=1.00  Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor   1.25
Plate  Duration Factor   1.25
plf - Live  Dead  From    To
TC V    40    20   0.0'  51.0'
BC V     0    20   0.0'  51.0'
BC V  2266  2266  20.8'  CL-LB
BC V  2266  2266  30.2'  CL-LB


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A     697   202  3- 8  1- 8
                Hz =  -175
W    5864  1841  5- 8  2- 4
Y    5881  1837  5- 8  2- 4
G     702   214  3- 8  1- 8
                Hz =    80

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
B -Q  0.02   655 C  0.00  0.02
Q -R  0.02   631 C  0.00  0.02
R -C  0.01    27 C  0.00  0.01
C -X  0.03   500 T  0.01  0.02
X -D  0.02   271 T  0.00  0.02
D -Z  0.02   271 T  0.00  0.02
Z -E  0.03   503 T  0.01  0.02
E -U  0.01    45 C  0.00  0.01
U -V  0.02   644 C  0.00  0.02
V -F  0.02   662 C  0.00  0.02
--------Bottom Chords---------
A -P  0.01   111 T  0.00  0.01
P -O  0.01   655 T  0.01  0.00
O -S1 0.02   631 T  0.01  0.01
S1-N  0.01   631 T  0.01  0.00
N -W  0.02    62 T  0.00  0.02
W -L  0.02   443 C  0.00  0.02
L -Y  0.02   447 C  0.00  0.02
Y -J  0.02    79 T  0.00  0.02
J -S2 0.01   644 T  0.01  0.00
S2-I  0.02   644 T  0.01  0.01
I -H  0.01   662 T  0.01  0.00
H -G  0.01    79 T  0.00  0.01
------------Webs-------------
A -B  0.08   639 C WindLd
B -P  0.13   814 T
P -Q  0.03   361 C
Q -O  0.00    40 T
O -R  0.03   243 T
R -N  0.08   766 C
N -C  0.08   513 T
C -W  0.06   707 C
W -X  0.05   621 C
X -L  0.04   267 T
L -D  0.04   409 C
L -Z  0.04   278 T
Y -Z  0.05   634 C
Y -E  0.06   724 C
J -E  0.08   509 T
J -U  0.08   758 C
I -U  0.03   239 T
I -V  0.00    22 C
H -V  0.03   366 C
H -F  0.13   821 T
G -F  0.05   644 C

TL Defl  -0.01" in P -O  L/999
LL Defl   0.00" in P -O  L/999
Shear // Grain  in B -Q   0.05

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
B  LOCK  7.0x 8.0 0.5 Ctr 0.90
Q  LOCK  4.0x 5.0 Ctr 0.5 0.60
R  LOCK  8.0x12.0 Ctr 1.5 0.99
C  LOCK  8.0x 8.0 Ctr Ctr 0.60
X  LOCK  4.0x 5.0-0.5 0.2 0.56
D  LOCK  6.0x 8.0 Ctr Ctr 0.80
Z  LOCK  4.0x 5.0 0.5 0.2 0.56
E  LOCK  8.0x 8.0 Ctr Ctr 0.60
U  LOCK  8.0x12.0 Ctr 1.5 0.99
V  LOCK  4.0x 5.0 Ctr 0.5 0.60
F  LOCK  7.0x 8.0-0.5 Ctr 0.90
A  LOCK  4.0x 8.0 0.2 Ctr 0.76
P  LOCK  4.0x 5.0 Ctr-0.5 0.60
O  LOCK  4.0x 5.0 Ctr-0.5 0.60
S1 LOCK  6.0x12.0 Ctr Ctr 0.91
N  LOCK  4.0x 5.0 Ctr-0.5 0.60
W  LOCK  4.0x 5.0 Ctr-0.5 0.60
L  LOCK  8.0x12.0 Ctr-1.5 0.99
Y  LOCK  4.0x 5.0 Ctr-0.5 0.60
J  LOCK  4.0x 5.0 Ctr-0.5 0.60
S2 LOCK  6.0x12.0 Ctr Ctr 0.91
I  LOCK  4.0x 5.0 Ctr-0.5 0.60
H  LOCK  4.0x 5.0 Ctr-0.5 0.60
G  LOCK  4.0x 8.0-0.2 Ctr 0.76


NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3   screws -OR- 10d nails
  as each layer is applied.)
                ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  2     12      24       0
  BC  2     12      24       0
  WB  1      8       8
Plus clusters of nails where
  shown.
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
  bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:    B
  Occupancy Factor   : 1.00
  Building Type: Enclosed
  TC Dead Load:        4.0 psf
  BC Dead Load:        6.0 psf
Max comp. force      766 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
    Designer)
    Gary Dounson, PE 35054
    Gary Dounson & Associates,
    Inc.
    2830 NW 41st Street Suite D
    Gainesville, FL 32606
    (352)375-8593
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R179*	1*2P	SP	160100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 222.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.09	2x 8	SP-2400
BC	0.16	2x 8	SP-2400
WB	0.69	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16- 1- 0
BC	Cont.	0- 0- 0	16- 1- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	16.1'
BC V	187	207	0.0'	16.1'
TC V	90	90	3.1'	12.7'

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	4533	1393	3- 8	1-14
			Hz =	-153
G	4497	1429	3- 8	1-14
			Hz =	107

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
B -C	0.01	1955 C	0.00 0.01
C -D	0.09	2894 C	0.03 0.06
D -E	0.09	2892 C	0.00 0.09
E -F	0.02	2139 C	0.02 0.00
Bottom Chords			
A -I	0.04	114 C	0.00 0.04
I -J	0.15	1998 T	0.03 0.12
J -H	0.16	2178 T	0.04 0.12
H -G	0.05	106 T	0.00 0.05
Webs			
A -B	0.43	3986 C	WindLd
B -I	0.69	4343 T	
I -C	0.22	2424 C	
C -J	0.16	1047 T	
J -D	0.13	865 T	
J -E	0.12	783 T	
H -E	0.20	2247 C	
H -F	0.69	4352 T	
G -F	0.36	3895 C	

LL Defl -0.04" in J -H L/999
TL Defl -0.07" in J -H L/999
Shear // Grain in I -J 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	4.0x 5.0	Ctr	Ctr	0.94
C	LOCK	6.0x 6.0	Ctr	Ctr	0.80
D	LOCK	6.0x 6.0	Ctr	Ctr	0.62
E	LOCK	6.0x 6.0	Ctr	Ctr	0.80
F	LOCK	4.0x 5.0	Ctr	Ctr	0.92
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
I	LOCK	5.0x 5.0	Ctr	Ctr	0.85
J	LOCK	7.0x12.0	Ctr-2.0		0.99
H	LOCK	5.0x 5.0	Ctr	Ctr	0.82
G	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 20- 8- 8
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

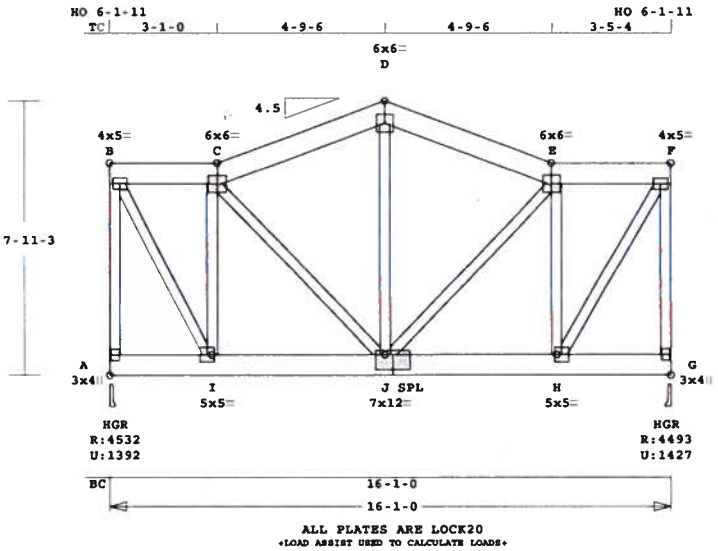
	Rows	Nails	Screws	Bolts
	----Spacing (In)----			
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3986 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R178*	1*2P	SP	160100	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 222.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.09	2x 8	SP-2400
BC	0.16	2x 8	SP-2400
WB	0.69	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16- 1- 0
BC	Cont.	0- 0- 0	16- 1- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	16.1'
BC V	187	207	0.0'	16.1'
TC V	90	90	3.1'	12.6'

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	4532	1393	3- 8	1-14
			Hz =	-153
G	4494	1428	3- 8	1-14
			Hz =	107

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
B -C	0.01	1954 C	0.00 0.01
C -D	0.09	2894 C	0.03 0.06
D -E	0.09	2892 C	0.00 0.09
E -F	0.02	2149 C	0.02 0.00
Bottom Chords			
A -I	0.04	114 C	0.00 0.04
I -J	0.15	1997 T	0.03 0.12
J -H	0.16	2188 T	0.04 0.12
H -G	0.05	106 T	0.00 0.05
Webs			
A -B	0.43	3984 C	WindLd
B -I	0.69	4341 T	
I -C	0.22	2424 C	
C -J	0.16	1049 T	
J -D	0.13	866 T	
J -E	0.12	769 T	
H -E	0.20	2236 C	
H -F	0.69	4351 T	
G -F	0.36	3888 C	

LL Defl -0.04" in J -H L/999
TL Defl -0.07" in J -H L/999
Shear // Grain in I -J 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	4.0x 5.0	Ctr	Ctr	0.94
C	LOCK	6.0x 6.0	Ctr	Ctr	0.80
D	LOCK	6.0x 6.0	Ctr	Ctr	0.62
E	LOCK	6.0x 6.0	Ctr	Ctr	0.80
F	LOCK	4.0x 5.0	Ctr	Ctr	0.91
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
I	LOCK	5.0x 5.0	Ctr	Ctr	0.85
J	LOCK	7.0x12.0	Ctr-2.0		0.99
H	LOCK	5.0x 5.0	Ctr	Ctr	0.81
G	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 20- 8- 8
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

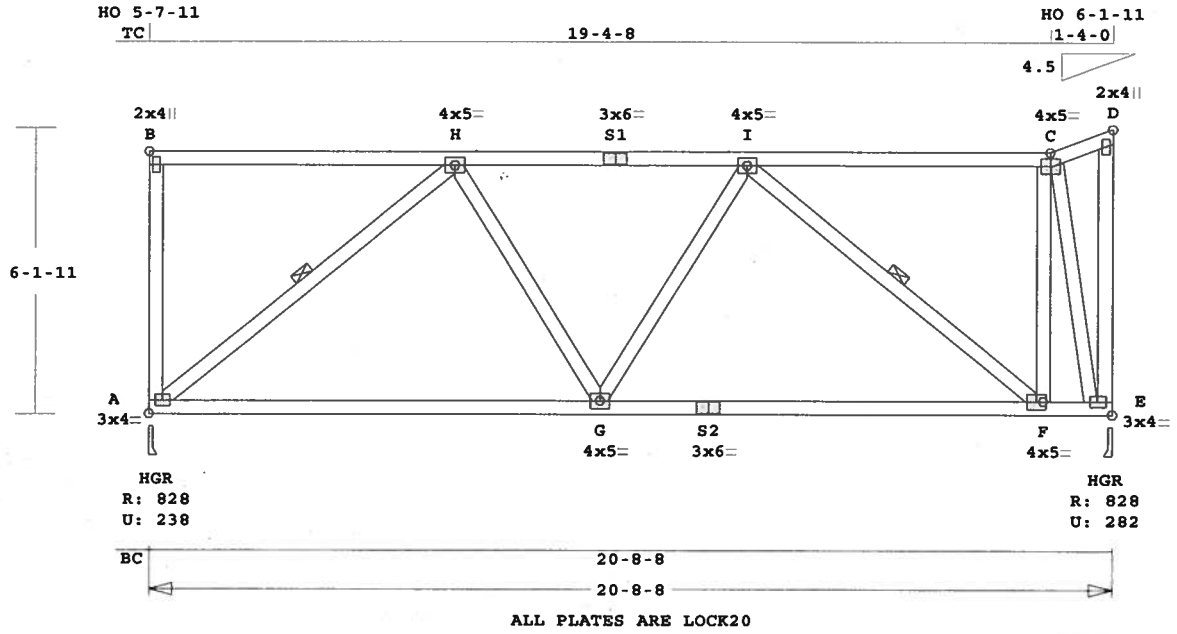
	Rows	Nails	Screws	Bolts
	----Spacing (In)----			
TC	2	12	24	0
BC	2	12	24	0
WB	1	4	4	

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3984 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R176*	1	SP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 15-FEB-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.69	2x 4	SP-#2
WB	0.41	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -H
WB 1 rows CLB on I -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	238	3- 8	1- 0
			Hz =	163
E	828	283	3- 8	1- 0
			Hz =	122

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -H	0.43	109 T	0.00	0.43
H -S1	0.43	801 C	0.00	0.43
S1-I	0.38	801 C	0.00	0.38
I -C	0.38	224 C	0.00	0.38
C -D	0.12	22 C	0.00	0.12
--------Bottom Chords---------				
A -G	0.68	705 T	0.06	0.62
G -S2	0.69	768 T	0.07	0.62
S2-F	0.40	768 T	0.07	0.33
F -E	0.35	207 T	0.02	0.33
------------Webs------------				
A -B	0.27	165 C	WindLd	
A -H	0.27	917 C		1 Br
H -G	0.08	312 T		
G -I	0.05	198 T		
I -F	0.21	706 C		1 Br
F -C	0.20	657 T		
C -E	0.41	933 C		
E -D	0.01	31 C		

LL Defl -0.22" in A -G L/999
TL Defl -0.44" in A -G L/546
Shear // Grain in B -H 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

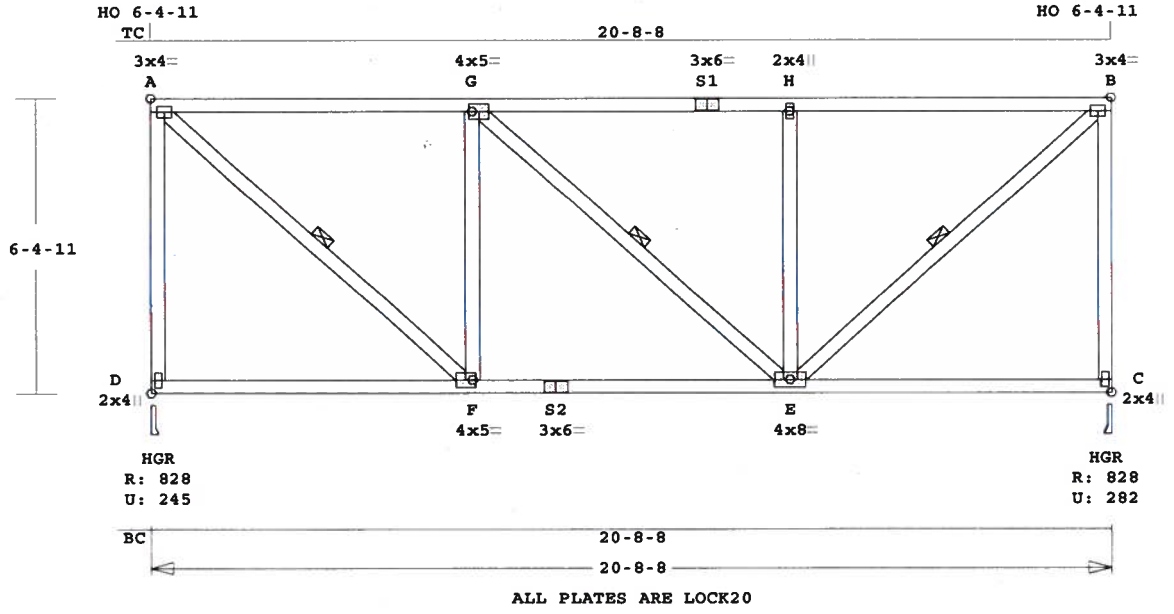
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 933 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R175*	1	FLAT	200808	60411	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 171.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.42	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	246	3- 8	1- 0
			Hz =	-178
C	828	282	3- 8	1- 0
			Hz =	126

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	666 C	0.00 0.49
G -S1	0.49	665 C	0.00 0.49
S1-H	0.49	665 C	0.00 0.49
H -B	0.49	665 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	125 C	0.00 0.28
F -S2	0.34	666 T	0.06 0.28
S2-E	0.34	666 T	0.06 0.28
E -C	0.28	126 T	0.00 0.28
------------Webs------------			
D -A	0.42	770 C	WindLd
A -F	0.28	894 T	1 Br
F -G	0.24	448 C	
G -E	0.01	56 T	1 Br
E -H	0.24	447 C	
E -B	0.28	894 T	1 Br
C -B	0.42	769 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

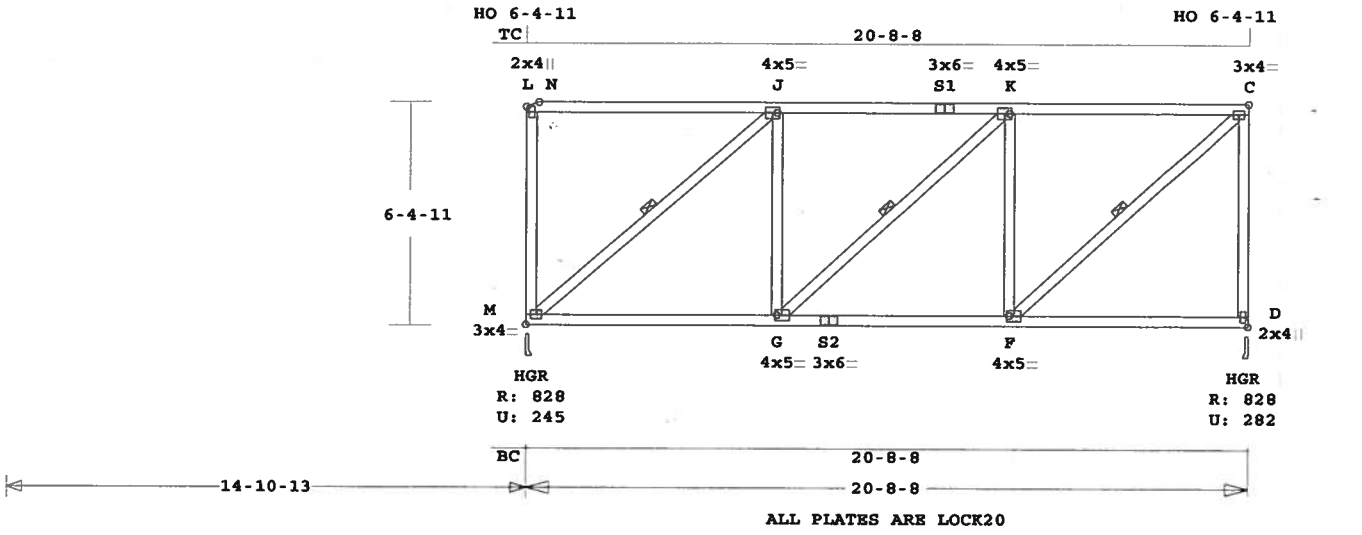
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R174	1	HHIP	200808	0	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 171.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.42	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on M -J
WB 1 rows CLB on G -K
WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	828	246	3- 8	1- 0
			Hz =	-178
D	828	282	3- 8	1- 0
			Hz =	126

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
L -J	0.51	125 T	0.00	0.51
J -S1	0.51	679 C	0.00	0.51
S1-K	0.46	679 C	0.00	0.46
K -C	0.46	657 C	0.00	0.46
--------Bottom Chords---------				
M -G	0.36	679 T	0.07	0.29
G -S2	0.35	657 T	0.06	0.29
S2-F	0.33	657 T	0.06	0.27
F -D	0.27	126 T	0.00	0.27
------------Webs------------				
M -L	0.35	179 C	WindLd	
M -J	0.31	898 C		1 Br
G -J	0.07	288 T		
G -K	0.02	65 C		1 Br
F -K	0.24	455 C		
F -C	0.28	889 T		1 Br
D -C	0.42	770 C		

LL Defl -0.08" in M -G L/999
TL Defl -0.16" in M -G L/999
Shear // Grain in L -J 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

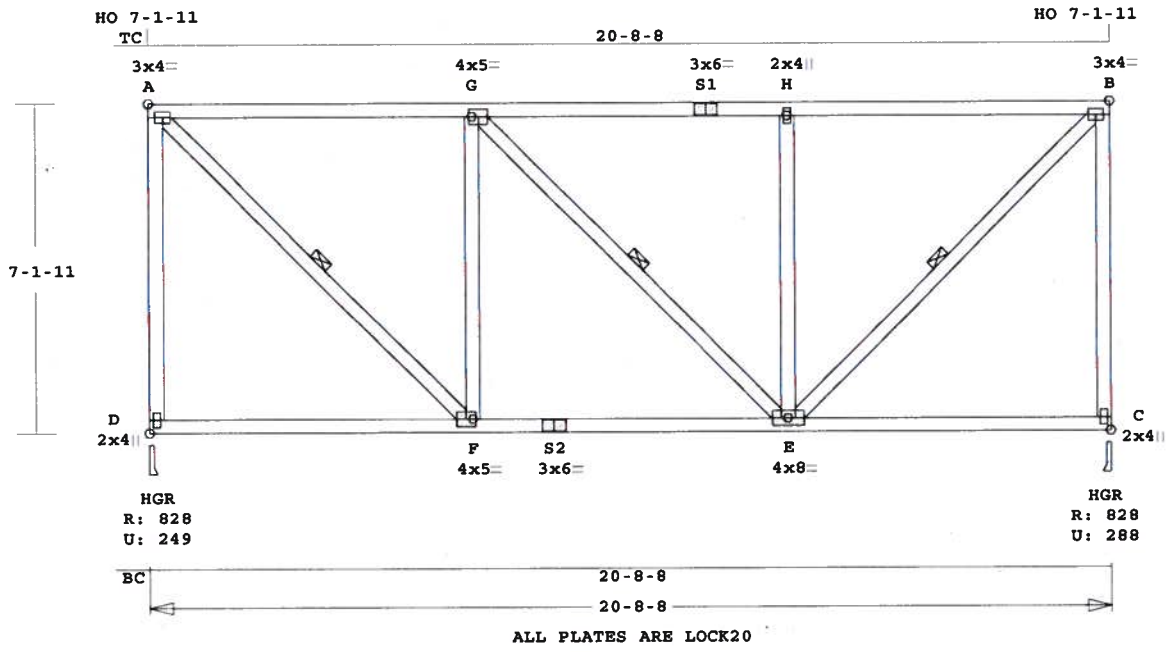
Jt	Type	Plt Size	X	Y	JSI
L	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
M	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 898 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R173*	1	FLAT	200808	70111	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 180.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.52	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	250	3- 8	1- 0
			Hz = -201	
C	828	289	3- 8	1- 0
			Hz = 142	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	593 C	0.00 0.49
G -S1	0.49	593 C	0.00 0.49
S1-H	0.49	593 C	0.00 0.49
H -B	0.49	593 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	141 C	0.00 0.28
F -S2	0.34	593 T	0.06 0.28
S2-E	0.34	593 T	0.06 0.28
E -C	0.28	142 T	0.00 0.28
------------Webs------------			
D -A	0.52	770 C	WindLd
A -F	0.27	842 T	1 Br
F -G	0.30	448 C	
G -E	0.01	67 T	1 Br
E -H	0.30	447 C	
E -B	0.26	841 T	1 Br
C -B	0.52	770 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	3.0x 4.0	Ctr	Ctr	0.69
G LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1 LOCK	3.0x 6.0	Ctr	Ctr	0.88
H LOCK	2.0x 4.0	Ctr	Ctr	0.40
B LOCK	3.0x 4.0	Ctr	Ctr	0.69
D LOCK	2.0x 4.0	Ctr	Ctr	0.40
F LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2 LOCK	3.0x 6.0	Ctr	Ctr	0.88
E LOCK	4.0x 8.0	Ctr	Ctr	0.50
C LOCK	2.0x 4.0	Ctr	Ctr	0.40

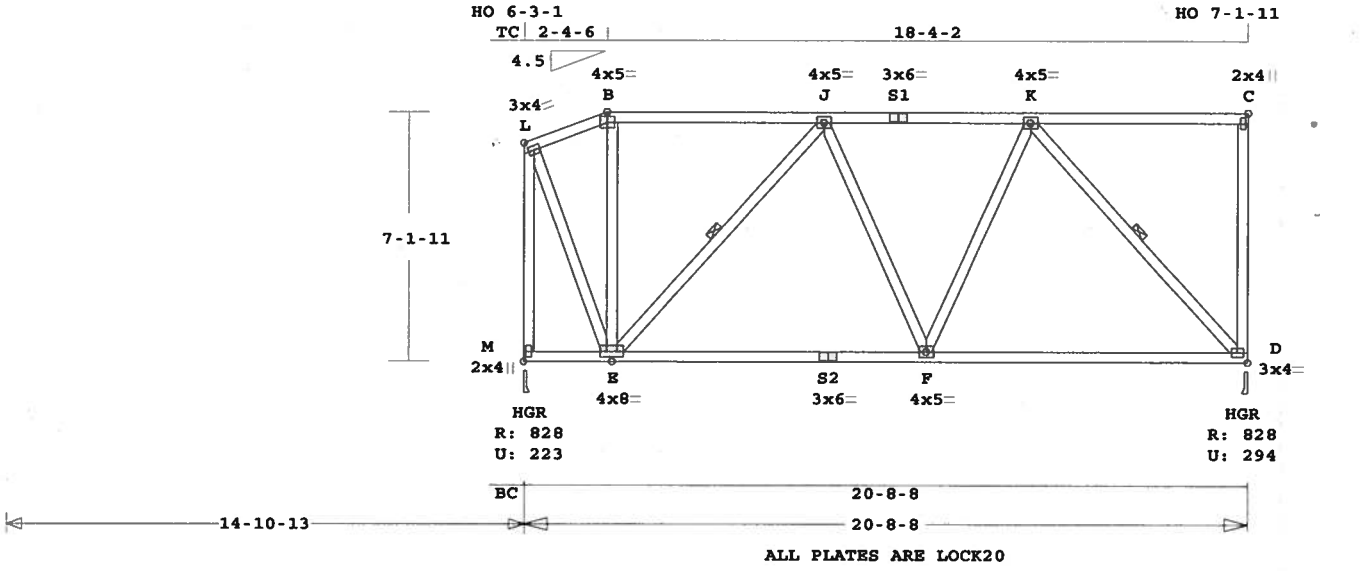
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R172*	1	HHIP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 186.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.39	2x 4	SP-#2
BC	0.62	2x 4	SP-#2
WB	0.45	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on K -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	828	223	3- 8	1- 0
			Hz =	200
D	828	294	3- 8	1- 0
			Hz =	146

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
L -B	0.10	281 C	0.00	0.10
B -J	0.34	272 C	0.00	0.34
J -S1	0.34	618 C	0.00	0.34
S1-K	0.39	618 C	0.00	0.39
K -C	0.39	0 T	0.00	0.39
--------Bottom Chords---------				
M -E	0.28	195 C	0.00	0.28
E -S2	0.34	614 T	0.06	0.28
S2-F	0.62	614 T	0.06	0.56
F -D	0.61	532 T	0.05	0.56
------------Webs------------				
M -L	0.45	837 C	WindLd	
L -E	0.23	743 T		
E -B	0.11	168 C		
E -J	0.17	513 C		1 Br
J -F	0.04	164 T		
F -K	0.08	314 T		
K -D	0.27	801 C		1 Br
D -C	0.10	156 C		

LL Defl -0.17" in F -D L/999
TL Defl -0.35" in F -D L/689
Shear // Grain in K -C 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

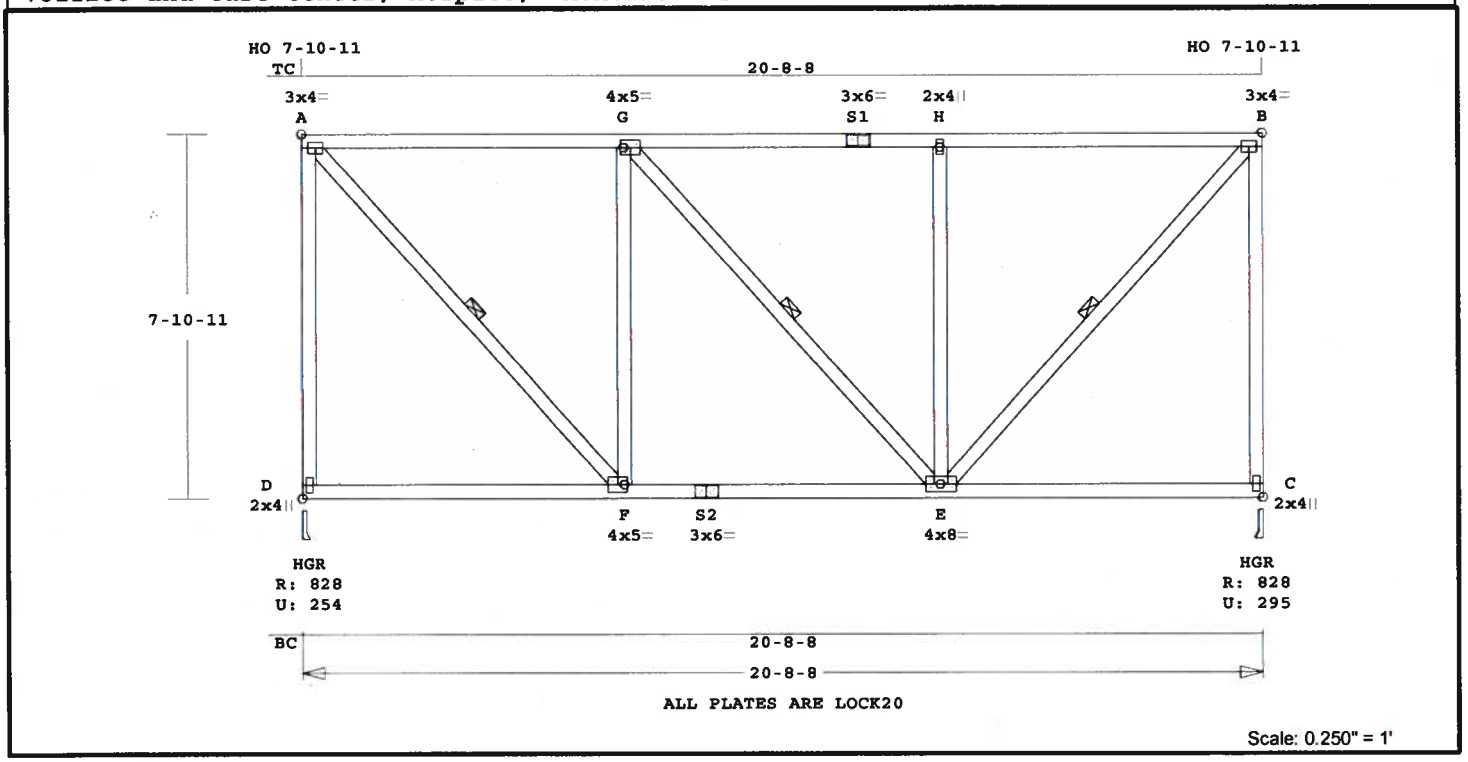
Jt	Type	Plt Size	X	Y	JSI
L	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.58
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40
M	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.62
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.58
D	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 837 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R171	1	FLAT	200808	71011	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 188.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.64	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	254	3- 8	1- 0
			Hz =	-223
C	828	296	3- 8	1- 0
			Hz =	158

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	535 C	0.00 0.49
G -S1	0.49	534 C	0.00 0.49
S1-H	0.49	534 C	0.00 0.49
H -B	0.49	534 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	158 C	0.00 0.28
F -S2	0.33	535 T	0.05 0.28
S2-E	0.33	535 T	0.05 0.28
E -C	0.28	157 T	0.00 0.28
------------Webs------------			
D -A	0.64	770 C	WindLd
A -F	0.25	802 T	1 Br
F -G	0.37	448 C	
G -E	0.02	79 T	1 Br
E -H	0.37	447 C	
E -B	0.25	801 T	1 Br
C -B	0.64	770 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.13" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

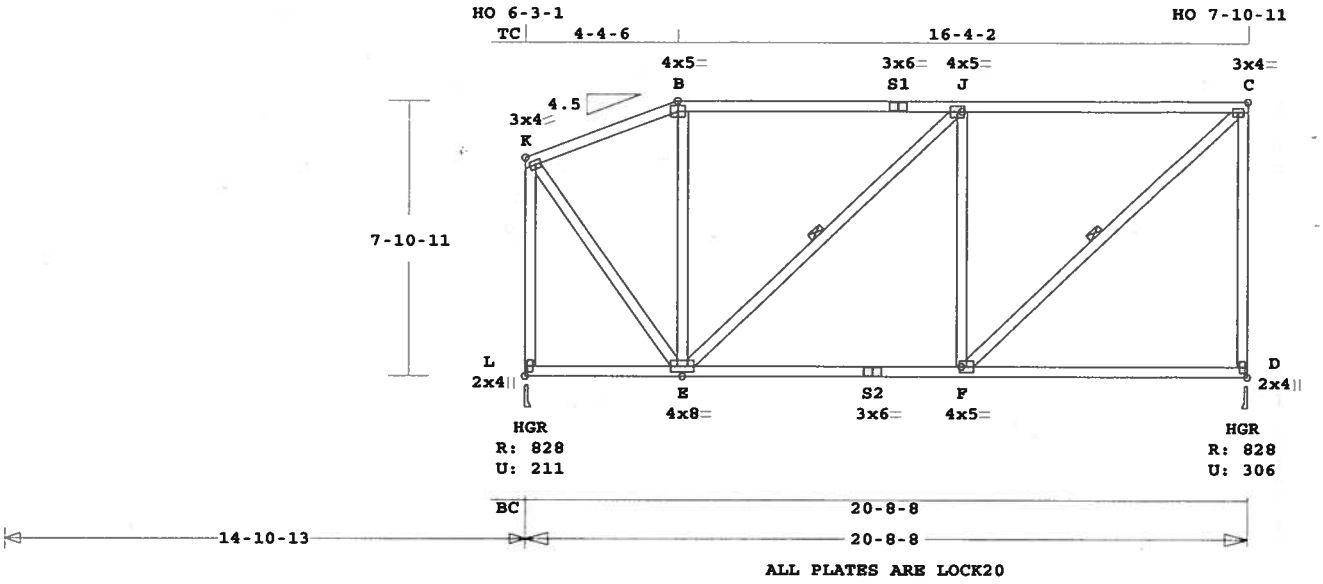
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

NOTES:
Trusses Manufactured by:

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	*Engineering*
060039	*R170*	1	HHIP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 184.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.80	2x 4	SP-#2
BC	0.50	2x 4	SP-#2
WB	0.64	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
L	828	211	3- 8	1- 0
			Hz =	234
D	828	306	3- 8	1- 0
			Hz =	164

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
K -B	0.22	413 C	0.03	0.19
B -S1	0.39	395 C	0.00	0.39
S1-J	0.80	395 C	0.00	0.80
J -C	0.80	599 C	0.00	0.80
Bottom Chords				
L -E	0.23	255 C	0.00	0.23
E -S2	0.29	599 T	0.06	0.23
S2-F	0.50	599 T	0.06	0.44
F -D	0.44	164 T	0.00	0.44
Webs				
L -K	0.43	798 C	WindLd	
K -E	0.21	672 T		
E -B	0.18	218 C		
E -J	0.13	281 C		1 Br
F -J	0.32	399 T		
F -C	0.26	825 T		1 Br
D -C	0.64	762 C		

LL Defl -0.11" in F -D L/999
TL Defl -0.23" in F -D L/999
Shear // Grain in J -C 0.38

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

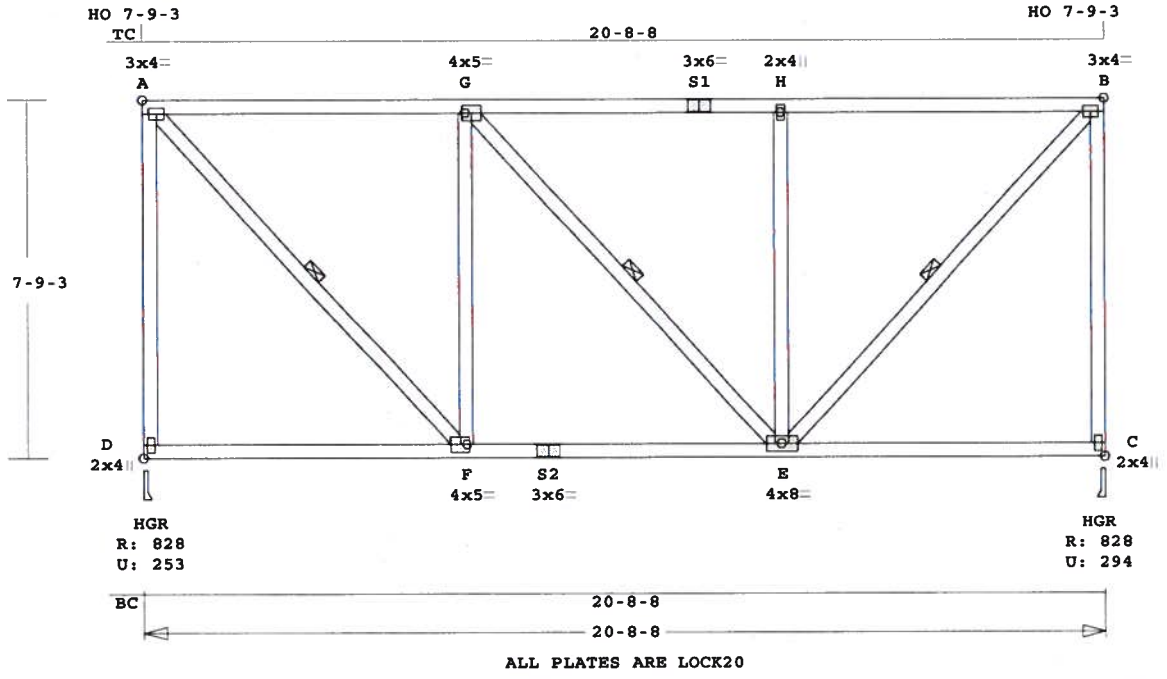
Jt	Type	Plt Size	X	Y	JSI
K	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
L	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.52
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 798 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R169*	1	FLAT	200808	70903	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 187.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.62	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
D	828	253	3- 8	1- 0
			Hz =	-220
C	828	295	3- 8	1- 0
			Hz =	155

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	544 C	0.00 0.49
G -S1	0.49	543 C	0.00 0.49
S1-H	0.49	543 C	0.00 0.49
H -B	0.49	543 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	155 C	0.00 0.28
F -S2	0.33	544 T	0.05 0.28
S2-E	0.33	544 T	0.05 0.28
E -C	0.28	155 T	0.00 0.28
------------Webs------------			
D -A	0.62	770 C	WindLd
A -F	0.25	808 T	1 Br
F -G	0.36	448 C	
G -E	0.02	77 T	1 Br
E -H	0.36	447 C	
E -B	0.25	807 T	1 Br
C -B	0.62	770 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

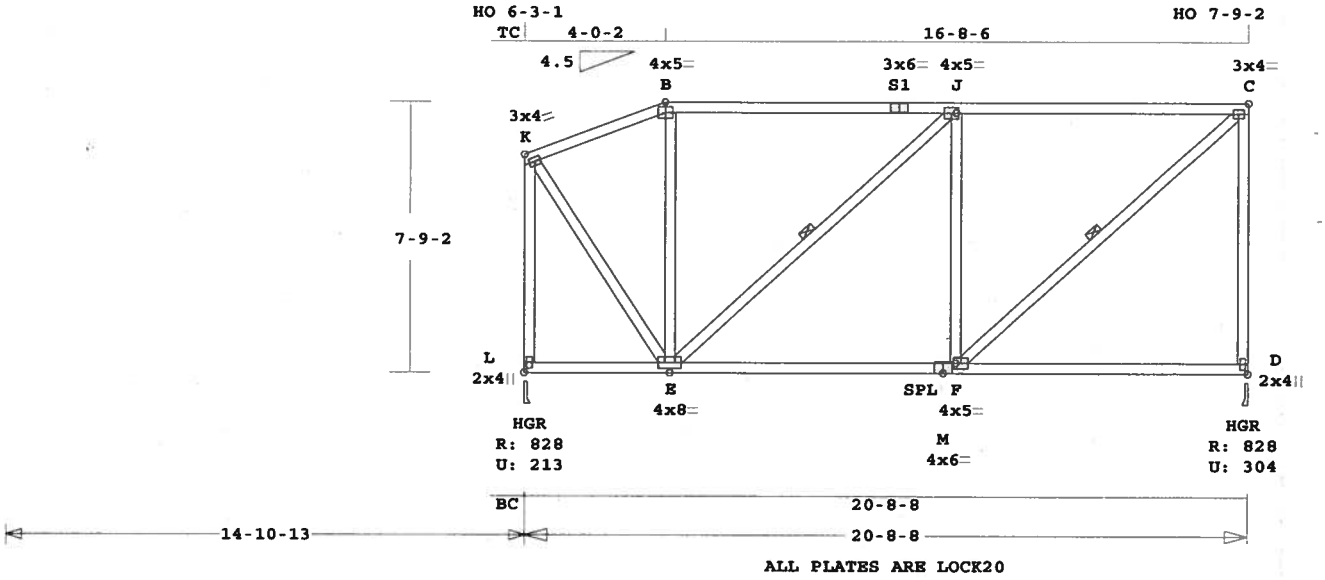
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R168	1	HHIP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 183.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.84	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
WB	0.61	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on F -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
L	828	213	3- 8	1- 0
			Hz =	228
D	828	304	3- 8	1- 0
			Hz =	161

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
K -B	0.19	395 C	0.00	0.19
B -S1	0.42	379 C	0.00	0.42
S1-J	0.84	379 C	0.00	0.84
J -C	0.84	617 C	0.00	0.84
--------Bottom Chords---------				
L -E	0.23	244 C	0.00	0.23
E -M	0.34	617 T	0.06	0.28
M -F	0.52	617 T	0.06	0.46
F -D	0.46	161 T	0.00	0.46
------------Webs------------				
L -K	0.43	803 C	WindLd	
K -E	0.21	683 T		
E -B	0.17	220 C		
E -J	0.15	322 C		1 Br
F -J	0.30	394 T		
F -C	0.26	835 T		1 Br
D -C	0.61	760 C		

LL Defl -0.12" in F -D L/999
TL Defl -0.25" in F -D L/981
Shear // Grain in J -C 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

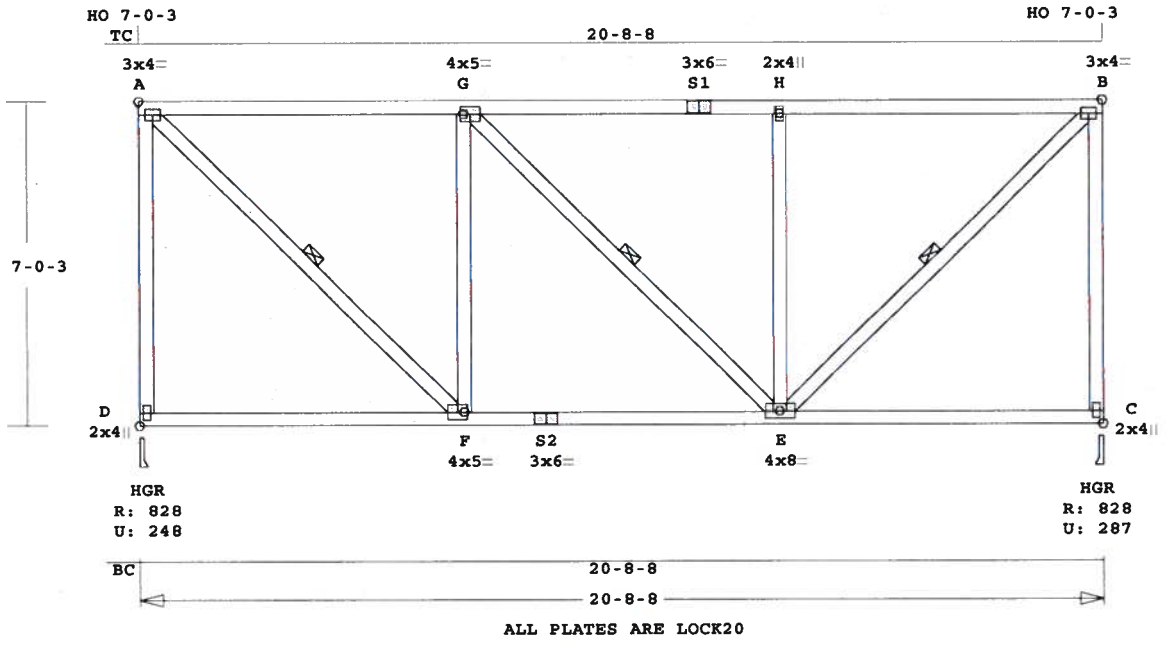
Jt	Type	Plt Size	X	Y	JSI
K	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
L	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.53
M	LOCK	4.0x 6.0	Ctr	0.2	0.91
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 803 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R167*	1	FLAT	200808	70003	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 178.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.50	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	249	3- 8	1- 0
			Hz =	-197
C	828	288	3- 8	1- 0
			Hz =	139

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	604 C	0.00 0.49
G -S1	0.49	604 C	0.00 0.49
S1-H	0.49	604 C	0.00 0.49
H -B	0.49	604 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	139 C	0.00 0.28
F -S2	0.34	604 T	0.06 0.28
S2-E	0.34	604 T	0.06 0.28
E -C	0.28	139 T	0.00 0.28
------------Webs------------			
D -A	0.50	770 C	WindLd
A -F	0.27	850 T	1 Br
F -G	0.29	448 C	
G -E	0.01	65 T	1 Br
E -H	0.29	447 C	
E -B	0.27	849 T	1 Br
C -B	0.50	769 C	

LL Defl -0.06" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

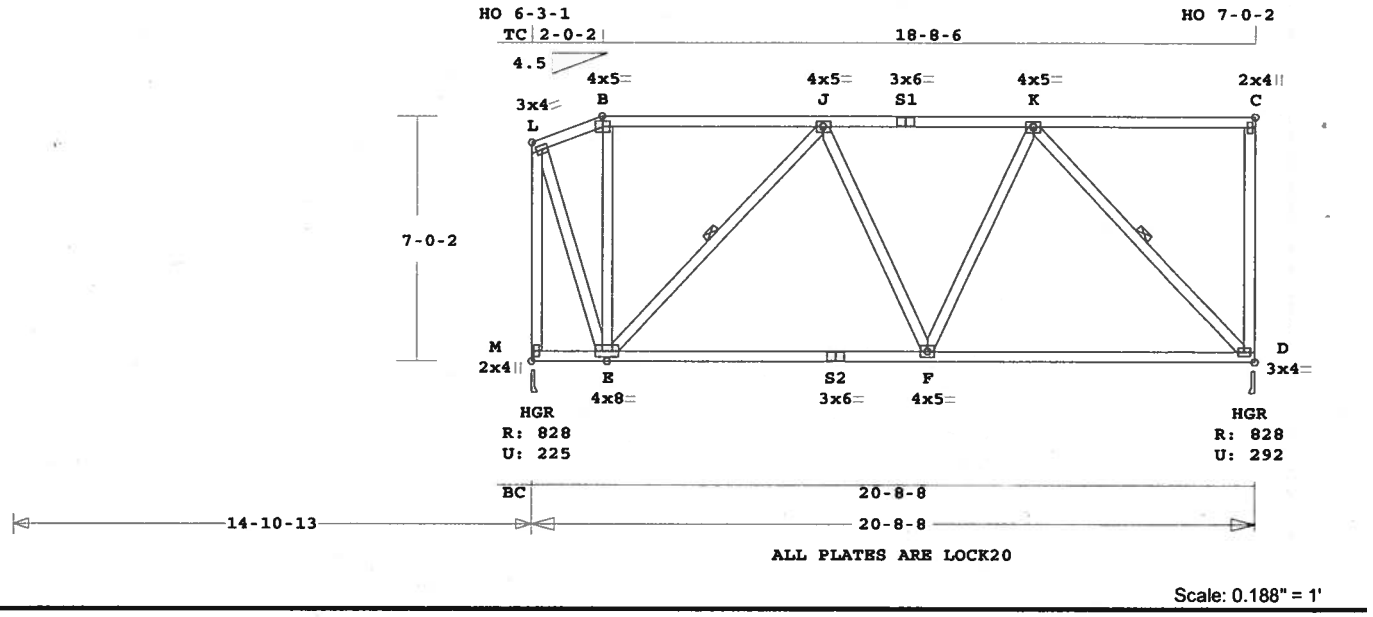
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R166	1	HHIP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 185.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.40	2x 4	SP-#2
BC	0.64	2x 4	SP-#2
WB	0.45	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	20- 8- 8
BC	Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on E -J
WB 1 rows CLB on K -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	828	225	3- 8	1- 0
			Hz =	194
D	828	293	3- 8	1- 0
			Hz =	143

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
L -B	0.11	249 C	0.00 0.11
B -J	0.36	244 C	0.00 0.36
J -S1	0.36	633 C	0.00 0.36
S1-K	0.40	633 C	0.00 0.40
K -C	0.40	0 T	0.00 0.40
Bottom Chords			
M -E	0.29	184 C	0.00 0.29
E -S2	0.35	621 T	0.06 0.29
S2-F	0.64	621 T	0.06 0.58
F -D	0.63	549 T	0.05 0.58
Webs			
M -L	0.45	849 C	WindLd
L -E	0.24	771 T	
E -B	0.11	178 C	
E -J	0.18	555 C	1 Br
J -F	0.04	158 T	
F -K	0.08	310 T	
K -D	0.27	809 C	1 Br
D -C	0.10	159 C	

LL Defl -0.19" in F -D L/999
TL Defl -0.38" in F -D L/637
Shear // Grain in K -C 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

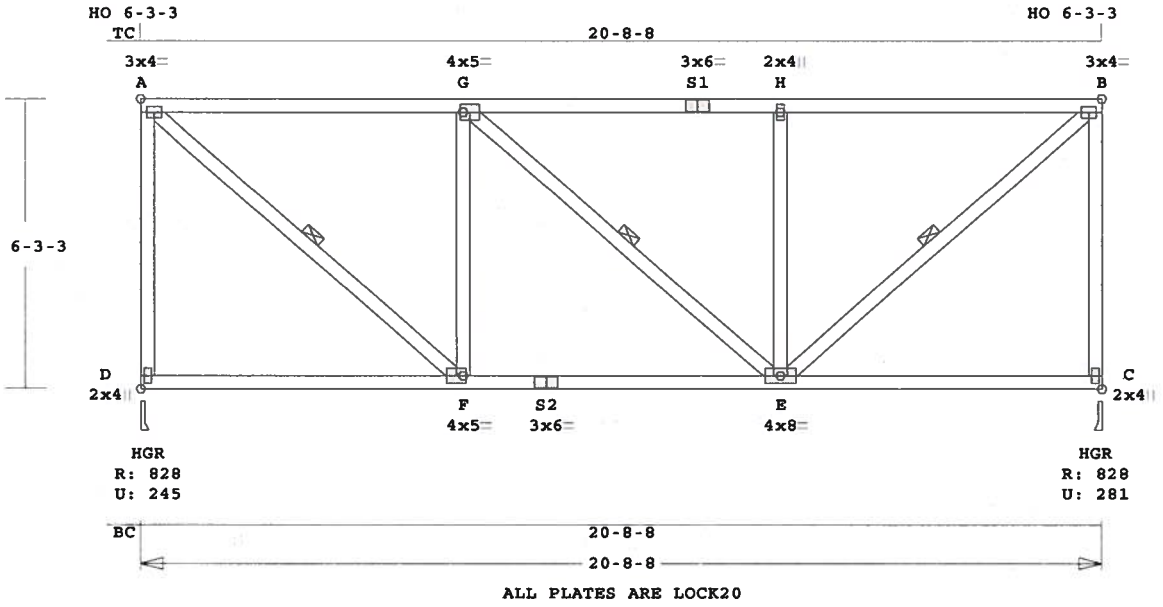
Jt	Type	Plt Size	X	Y	JSI
L	LOCK	3.0x 4.0	Ctr	Ctr	0.69
B	LOCK	4.0x 5.0	Ctr	Ctr	0.87
J	LOCK	4.0x 5.0	Ctr	Ctr	0.58
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40
M	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	4.0x 8.0	Ctr	Ctr	0.65
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.58
D	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 849 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R165*	1	FLAT	200808	60303	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.40	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on A -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	828	245	3- 8	1- 0
			Hz =	-175
C	828	281	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.49	680 C	0.00 0.49
G -S1	0.49	679 C	0.00 0.49
S1-H	0.49	679 C	0.00 0.49
H -B	0.49	679 C	0.00 0.49
--------Bottom Chords---------			
D -F	0.28	122 C	0.00 0.28
F -S2	0.35	680 T	0.07 0.28
S2-E	0.35	680 T	0.07 0.28
E -C	0.28	123 T	0.00 0.28
------------Webs------------			
D -A	0.40	770 C	WindLd
A -F	0.29	905 T	1 Br
F -G	0.23	448 C	
G -E	0.01	54 T	1 Br
E -H	0.23	447 C	
E -B	0.29	904 T	1 Br
C -B	0.40	769 C	

LL Defl -0.07" in D -F L/999
TL Defl -0.14" in E -C L/999
Shear // Grain in A -G 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
C	LOCK	2.0x 4.0	Ctr	Ctr	0.40

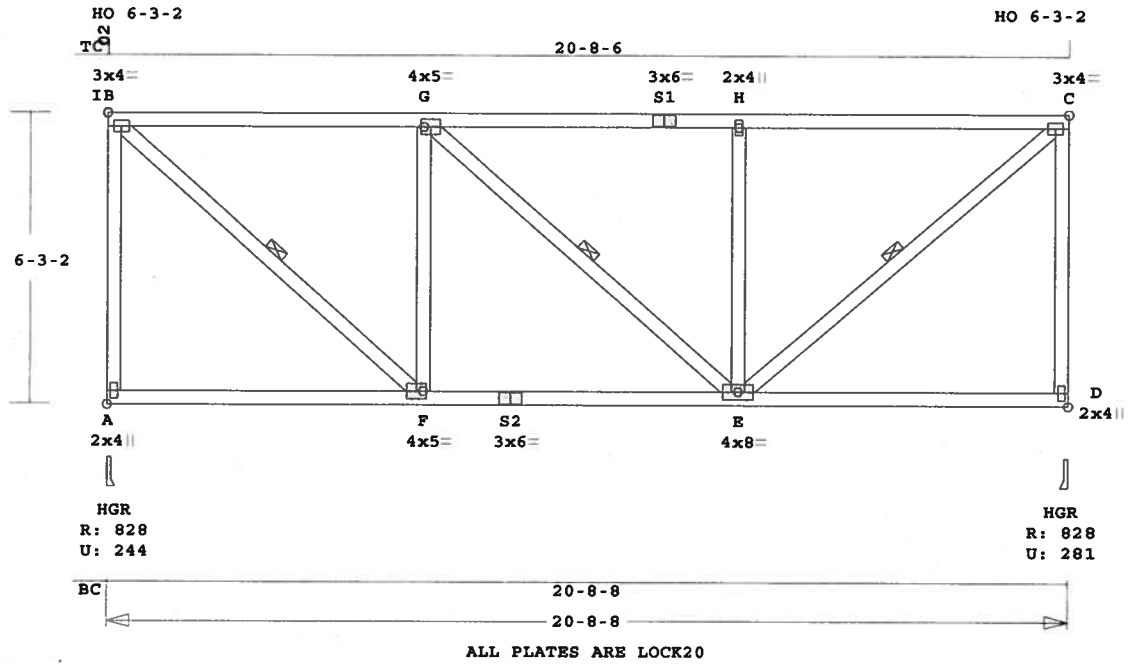
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 770 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

NOTES:
Trusses Manufactured by:

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R164	1	SP	200808	0	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 170.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.40	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

WB 1 rows CLB on B -F
WB 1 rows CLB on G -E
WB 1 rows CLB on E -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	828	244	3- 8	1- 0
			Hz =	-174
D	828	281	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -G	0.47	672 C	0.00 0.47
G -S1	0.47	689 C	0.00 0.47
S1-H	0.51	689 C	0.00 0.51
H -C	0.51	689 C	0.00 0.51
--------Bottom Chords---------			
A -F	0.27	122 C	0.00 0.27
F -S2	0.34	672 T	0.07 0.27
S2-E	0.36	672 T	0.07 0.29
E -D	0.29	123 T	0.00 0.29
------------Webs------------			
A -B	0.40	771 C	WindLd
B -F	0.28	902 T	1 Br
F -G	0.23	455 C	
G -E	0.01	47 T	1 Br
E -H	0.23	455 C	
E -C	0.29	908 T	1 Br
D -C	0.40	768 C	

LL Defl -0.07" in E -D L/999
TL Defl -0.15" in E -D L/999
Shear // Grain in H -C 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.40
C	LOCK	3.0x 4.0	Ctr	Ctr	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
E	LOCK	4.0x 8.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:

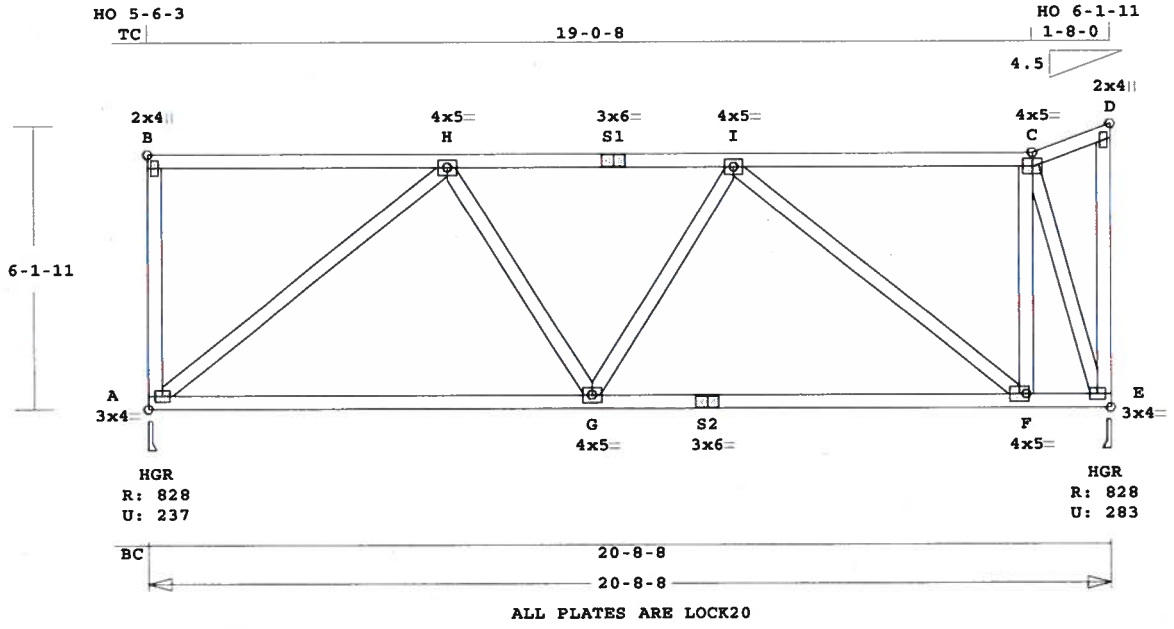
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 771 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Robbins Engineering, Inc./Online Plus™ © 1996-2006 Version 19.0.009 Engineering - Portrait 3/16/2006 1:17:27 PM Page 1

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R163*	1	SP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 169.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.42	2x 4	SP-#2
BC	0.67	2x 4	SP-#2
WB	0.90	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	20- 8- 8
BC Cont.		0- 0- 0	20- 8- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	237	3- 8	1- 0
			Hz = 165	
E	828	284	3- 8	1- 0
			Hz = 123	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -H	0.42	106 T	0.00	0.42
H -S1	0.42	817 C	0.00	0.42
S1-I	0.36	817 C	0.00	0.36
I -C	0.36	276 C	0.00	0.36
C -D	0.07	18 C	0.00	0.07
--------Bottom Chords---------				
A -G	0.67	715 T	0.07	0.60
G -S2	0.67	792 T	0.07	0.60
S2-F	0.38	792 T	0.07	0.31
F -E	0.34	266 T	0.03	0.31
------------Webs------------				
A -B	0.26	162 C	WindLd	
A -H	0.90	926 C		
H -G	0.08	319 T		
G -I	0.04	184 T		
I -F	0.64	668 C		
F -C	0.18	605 T		
C -E	0.38	885 C		
E -D	0.01	46 T		

LL Defl -0.20" in A -G L/999
TL Defl -0.41" in A -G L/585
Shear // Grain in B -H 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

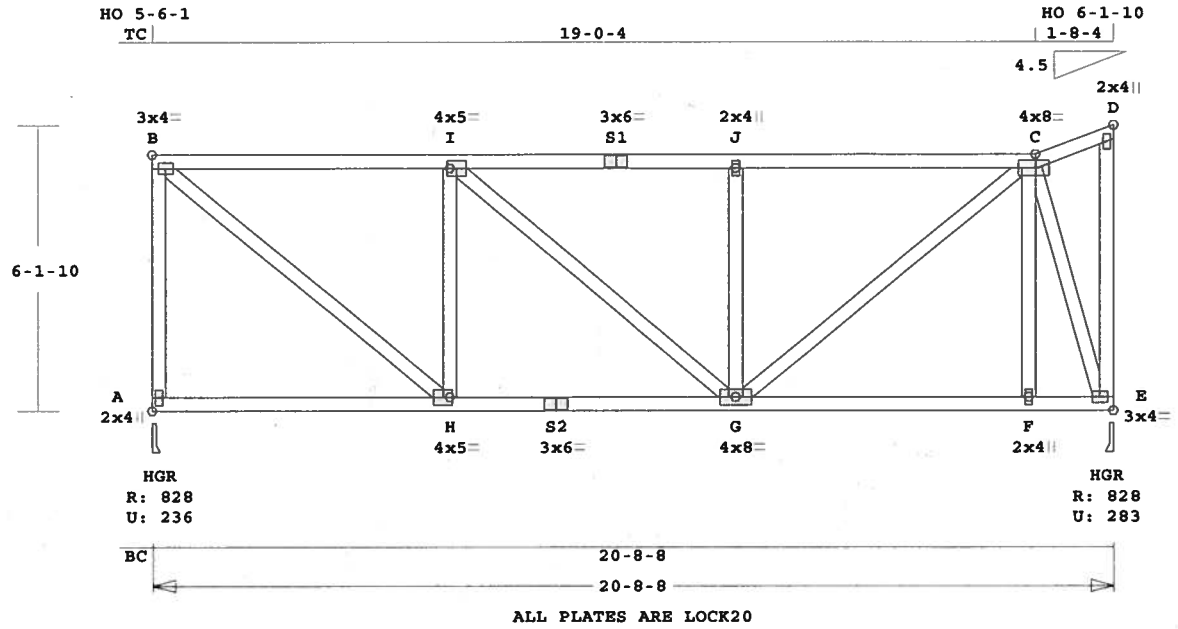
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.53
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.53
C	LOCK	4.0x 5.0	Ctr	Ctr	0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
G	LOCK	4.0x 5.0	Ctr	Ctr	0.53
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 926 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R162	1	SP	200808	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 179.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.42	2x 4	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.35	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 8
BC Cont.	0- 0- 0	20- 8- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	828	237	3- 8	1- 0
			Hz =	166
E	828	284	3- 8	1- 0
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -I	0.42	741 C	0.00 0.42
I -S1	0.42	808 C	0.00 0.42
S1-J	0.33	808 C	0.00 0.33
J -C	0.33	808 C	0.00 0.33
C -D	0.06	27 C	0.00 0.06
--------Bottom Chords---------			
A -H	0.24	155 C	0.00 0.24
H -S2	0.31	741 T	0.07 0.24
S2-G	0.26	741 T	0.07 0.19
G -F	0.21	259 T	0.02 0.19
F -E	0.17	259 T	0.02 0.15
------------Webs------------			
A -B	0.31	774 C	WindLd
B -H	0.30	962 T	
H -I	0.19	475 C	
I -G	0.03	87 T	
G -J	0.15	370 C	
G -C	0.28	701 T	
F -C	0.05	215 T	
C -E	0.35	808 C	
E -D	0.03	72 T	

LL Defl -0.05" in A -H L/999
TL Defl -0.10" in A -H L/999
Shear // Grain in B -I 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

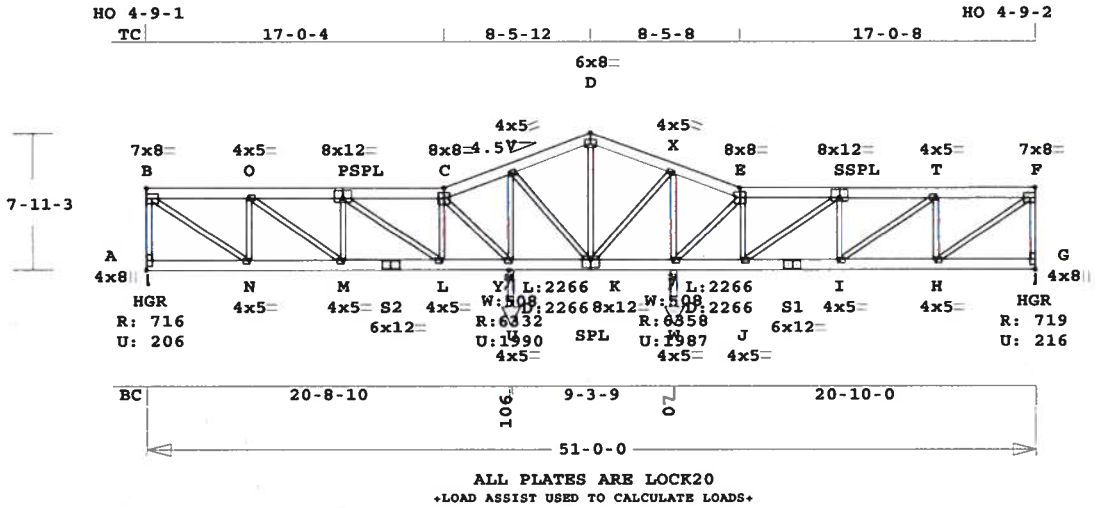
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.69
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	2.0x 4.0	Ctr	Ctr	0.40
C	LOCK	4.0x 8.0-0.5	Ctr		0.87
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.40
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
G	LOCK	4.0x 8.0	Ctr	Ctr	0.50
F	LOCK	2.0x 4.0	Ctr	Ctr	0.40
E	LOCK	3.0x 4.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 808 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R161	1*2P	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 592.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.05	2x 8	SP-2400
BC	0.02	2x 8	SP-2400
WB	0.13	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	51- 0- 0
BC	Cont.	0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
TC V	50	50	20.7'	30.5'
BC V	2266	2266	20.8'	CL-LB
BC V	2266	2266	30.2'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	716	207	3- 8	1- 8
			Hz =	-173
U	6333	1990	5- 8	2- 7
W	6358	1988	5- 8	2- 7
G	719	216	3- 8	1- 8
			Hz =	75

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -O	0.02	687 C	0.00 0.02
O -P	0.02	728 C	0.00 0.02
P -C	0.02	162 C	0.00 0.02
C -V	0.05	426 T	0.00 0.05
V -D	0.05	148 T	0.00 0.05
D -X	0.05	141 T	0.00 0.05
X -E	0.05	425 T	0.00 0.05
E -S	0.02	171 C	0.00 0.02
S -T	0.02	734 C	0.00 0.02
T -F	0.02	691 C	0.00 0.02
--------Bottom Chords---------			
A -N	0.01	111 T	0.00 0.01
N -M	0.01	687 T	0.01 0.00
M -S2	0.02	728 T	0.01 0.01
S2-L	0.02	728 T	0.01 0.01
L -U	0.02	151 T	0.00 0.02
U -K	0.02	361 C	0.00 0.02
K -W	0.02	357 C	0.00 0.02
W -J	0.02	160 T	0.00 0.02
J -S1	0.02	734 T	0.01 0.01
S1-I	0.02	734 T	0.01 0.01
I -H	0.01	691 T	0.01 0.00
H -G	0.01	74 T	0.00 0.01
----------Webs----------			
A -B	0.08	660 C	WindLd
B -N	0.13	854 T	
N -O	0.03	393 C	
O -M	0.00	51 T	
M -P	0.02	211 T	
P -L	0.07	703 C	
L -C	0.07	463 T	
C -U	0.06	720 C	
U -V	0.10	1093 C	
V -K	0.06	427 T	
K -D	0.08	688 C	
K -X	0.06	421 T	
W -X	0.10	1115 C	
W -E	0.06	725 C	
J -E	0.07	462 T	
J -S	0.07	700 C	
I -S	0.02	210 T	
I -T	0.00	54 T	
H -T	0.03	395 C	
H -F	0.13	858 T	
G -F	0.05	663 C	

TL Defl -0.02" in M -S2 L/999
LL Defl -0.01" in M -S2 L/999
Shear // Grain in V -D 0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
O	LOCK	4.0x 5.0	Ctr	0.5	0.60
P	LOCK	8.0x12.0	Ctr	1.5	0.99
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
V	LOCK	4.0x 5.0	-0.5	0.2	0.56
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
X	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
S	LOCK	8.0x12.0	Ctr	1.5	0.99
T	LOCK	4.0x 5.0	Ctr	0.5	0.60
F	LOCK	7.0x 8.0	-0.5	Ctr	0.90
A	LOCK	4.0x 8.0	0.2	Ctr	0.76
N	LOCK	4.0x 5.0	Ctr	-0.5	0.60
M	LOCK	4.0x 5.0	Ctr	-0.5	0.60
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
L	LOCK	4.0x 5.0	Ctr	-0.5	0.60
U	LOCK	4.0x 5.0	Ctr	-0.5	0.60
K	LOCK	8.0x12.0	Ctr	-1.5	0.99
W	LOCK	4.0x 5.0	Ctr	-0.5	0.60
J	LOCK	4.0x 5.0	Ctr	-0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
I	LOCK	4.0x 5.0	Ctr	-0.5	0.60
H	LOCK	4.0x 5.0	Ctr	-0.5	0.60
G	LOCK	4.0x 8.0	-0.2	Ctr	0.76

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

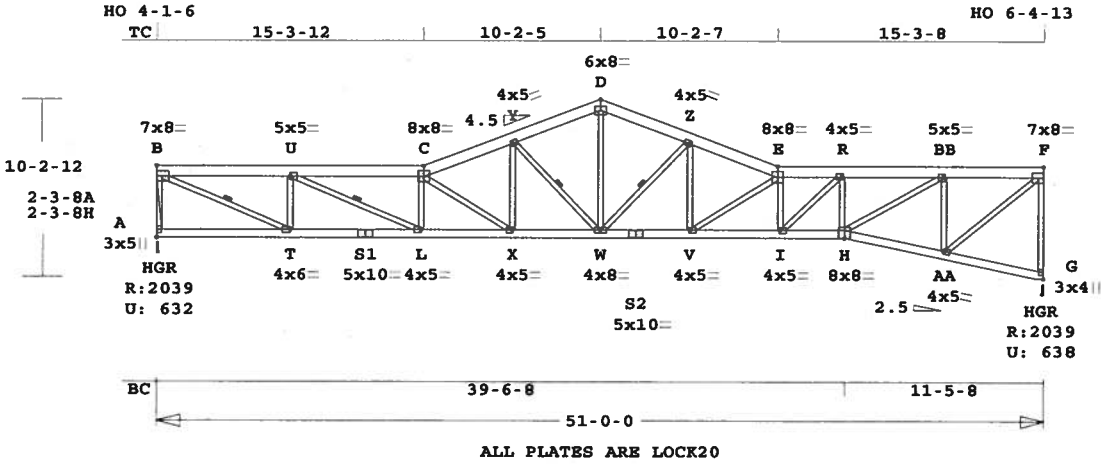
	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	8	8	

Plus clusters of nails where shown.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1115 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R160	1	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 534.7 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 8	SP-2400
BC	0.96	2x 6	SP-#2
WB	0.97	2x 4	SP-#3
EX B	-T	2x 4	SP-#2
EX H	-BB	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

WB 1 rows CLB on B -T
WB 1 rows CLB on U -L
WB 1 rows CLB on Y -W
WB 1 rows CLB on W -Z
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	633	3- 8	2- 7
			Hz =	-168
G	2040	638	3- 8	2- 7
			Hz =	79

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
B -U	0.10	3698	C	0.01	0.09
U -C	0.11	6035	C	0.04	0.07
C -Y	0.11	4821	C	0.02	0.09
Y -D	0.07	3746	C	0.04	0.03
D -Z	0.07	3746	C	0.04	0.03
Z -E	0.11	4828	C	0.02	0.09
E -R	0.11	6004	C	0.04	0.07
R -BB	0.11	5018	C	0.02	0.09
BB-F	0.06	2216	C	0.00	0.06
Bottom Chords					
A -T	0.17	115	T	0.00	0.17
T -S1	0.58	3698	T	0.44	0.14
S1-L	0.59	3698	T	0.44	0.15
L -X	0.85	6065	T	0.73	0.12
X -W	0.60	4550	T	0.55	0.05
W -S2	0.60	4555	T	0.55	0.05
S2-V	0.61	4555	T	0.55	0.06
V -I	0.96	6036	T	0.73	0.23
I -H	0.94	5048	T	0.61	0.33
H -AA	0.58	2246	T	0.27	0.31
AA-G	0.16	82	T	0.00	0.16
Webs					
A -B	0.42	1975	C	WindLd	
B -T	0.75	4097	T		1 Br
T -U	0.33	1569	C		
U -L	0.83	2589	T		1 Br
L -C	0.22	1050	C		
C -X	0.97	1791	C		
X -Y	0.34	1088	T		
Y -W	0.37	1543	C		1 Br
W -D	0.75	2365	T		
W -Z	0.37	1549	C		1 Br
V -Z	0.35	1099	T		
V -E	0.95	1750	C		
I -E	0.23	1115	C		
I -R	0.43	1358	T		
H -R	0.21	981	C		
H -BB	0.78	3238	T		
AA-BB	0.68	2109	C		
AA-F	0.93	2904	T		
G -F	0.97	2009	C		

LL Defl -0.44" in V -I L/999
TL Defl -0.89" in V -I L/679
Shear // Grain in I -H 0.17

Hz Disp	LL	DL	TL
Jt G	0.15"	0.15"	0.31"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

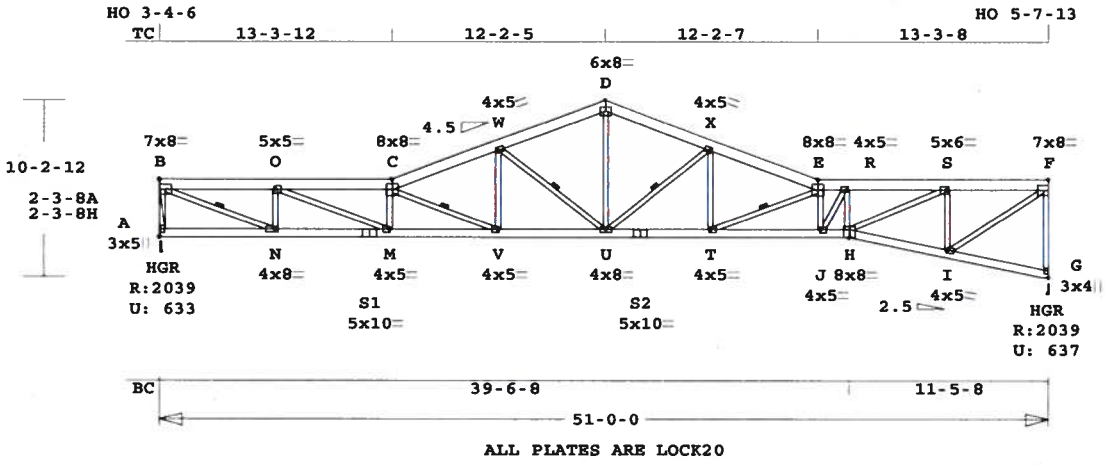
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5	Ctr	0.90
U	LOCK	5.0x 5.0	0.5	Ctr	1.00
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Y	LOCK	4.0x 5.0	-0.5	0.2	0.56
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
Z	LOCK	4.0x 5.0	0.5	0.2	0.56
E	LOCK	8.0x 8.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	Ctr	0.5	0.68
BB	LOCK	5.0x 5.0	Ctr	Ctr	0.95
F	LOCK	7.0x 8.0	-0.5	Ctr	0.93
A	LOCK	3.0x 5.0	Ctr	Ctr	0.90
T	LOCK	4.0x 6.0	-1.0	Ctr	0.94
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 5.0	-0.5	Ctr	1.00
X	LOCK	4.0x 5.0	Ctr	Ctr	0.60
W	LOCK	4.0x 8.0	Ctr	Ctr	0.79
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
V	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	8.0x 8.0	Ctr	-1.5	0.61
AA	LOCK	4.0x 5.0	0.5	-0.1	0.97
G	LOCK	3.0x 4.0	Ctr	Ctr	1.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6035 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R159*	1	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 518.9 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 8	SP-2400
BC	0.72	2x 6	SP-#2
EX	S1-S2	2x 6	SP-2400
EX	S2-H	2x 6	SP-2400
WB	0.75	2x 4	SP-#3
EX	B -N	2x 4	SP-#2
EX	O -M	2x 4	SP-#2
EX	H -S	2x 4	SP-#2
EX	I -F	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on B -N
WB 1 rows CLB on C -V
WB 1 rows CLB on W -U
WB 1 rows CLB on U -X
WB 1 rows CLB on T -E
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	634	3- 8	2- 7
			Hz =	-160
G	2040	637	3- 8	2- 7
			Hz =	91

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -O	0.09	4082 C	0.01 0.08
O -C	0.15	6977 C	0.05 0.10
C -W	0.16	5123 C	0.03 0.13
W -D	0.10	3751 C	0.01 0.09
D -X	0.09	3750 C	0.04 0.05
X -E	0.15	5130 C	0.03 0.12
E -R	0.18	6928 C	0.05 0.13
R -S	0.18	6315 C	0.04 0.14
S -F	0.08	2681 C	0.00 0.08
--------Bottom Chords---------			
A -N	0.13	118 T	0.00 0.13
N -S1	0.72	4083 T	0.49 0.23
S1-M	0.30	4083 T	0.18 0.12
M -V	0.44	7027 T	0.32 0.12
V -U	0.27	4843 T	0.22 0.05
U -S2	0.24	4845 T	0.22 0.02
S2-T	0.29	4845 T	0.22 0.07
T -J	0.51	6994 T	0.32 0.19
J -H	0.50	6339 T	0.29 0.21
H -I	0.71	2717 T	0.33 0.38
I -G	0.18	95 T	0.00 0.18
----------Webs----------			
A -B	0.35	1983 C	WindLd
B -N	0.82	4452 T	1 Br
N -O	0.28	1584 C	
O -M	0.85	3157 T	
M -C	0.22	1243 C	
C -V	0.52	2358 C	1 Br
V -W	0.34	1084 T	
W -U	0.46	1755 C	1 Br
U -D	0.75	2344 T	
U -X	0.46	1756 C	1 Br
T -X	0.35	1101 T	
T -E	0.51	2319 C	1 Br
J -E	0.29	1652 C	
J -R	0.39	1233 T	
H -R	0.10	565 C	
H -S	0.86	3990 T	
I -S	0.54	2197 C	
I -F	0.77	3282 T	
G -F	0.75	2013 C	

LL Defl -0.53" in T -J L/999
TL Defl -1.08" in T -J L/560
Shear // Grain in J -H 0.43

Hz Disp	LL	DL	TL
Jt G	0.18"	0.18"	0.36"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

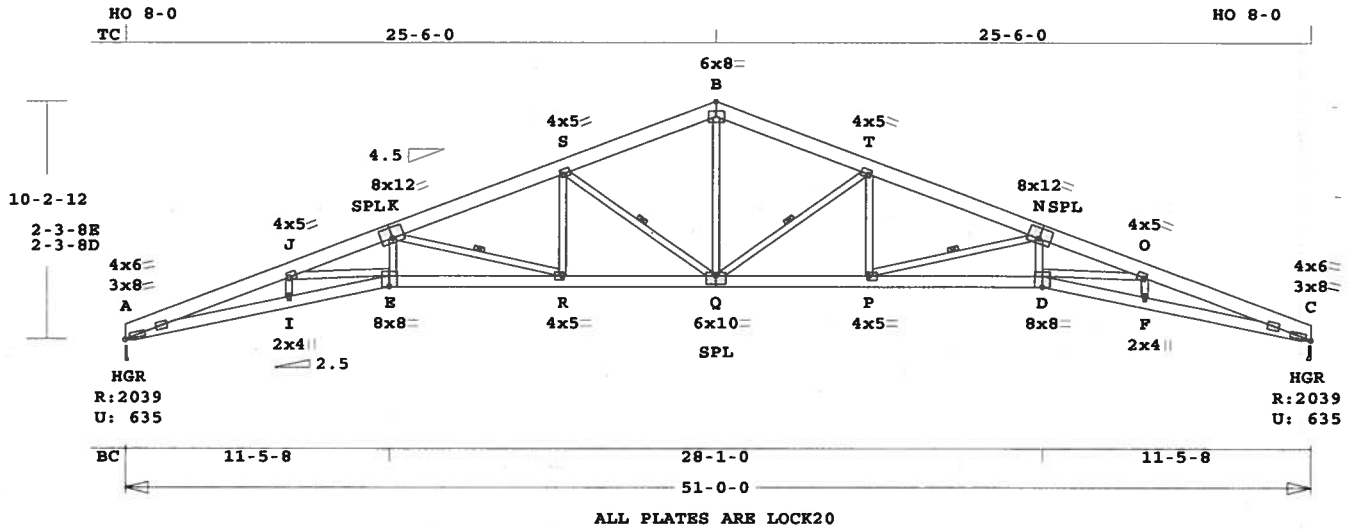
Jt	Type	Plt Size	X	Y	JSI
B	LOCK	7.0x 8.0	0.5 Ctr	0.90	
O	LOCK	5.0x 5.0	Ctr Ctr	0.94	
C	LOCK	8.0x 8.0	Ctr Ctr	0.84	
W	LOCK	4.0x 5.0-0.5	0.2	0.56	
D	LOCK	6.0x 8.0	Ctr Ctr	0.80	
X	LOCK	4.0x 5.0	0.5 0.2	0.56	
E	LOCK	8.0x 8.0	Ctr Ctr	0.84	
R	LOCK	4.0x 5.0	Ctr 0.5	0.71	
S	LOCK	5.0x 6.0	Ctr Ctr	0.97	
F	LOCK	7.0x 8.0-0.5	Ctr	0.90	
A	LOCK	3.0x 5.0	Ctr Ctr	0.90	
N	LOCK	4.0x 8.0-0.5	Ctr	0.87	
S1	LOCK	5.0x10.0	Ctr Ctr	0.82	
M	LOCK	4.0x 5.0-0.5	Ctr	0.90	
V	LOCK	4.0x 5.0	Ctr Ctr	0.66	
U	LOCK	4.0x 8.0	Ctr Ctr	0.79	
S2	LOCK	5.0x10.0	Ctr Ctr	0.82	
T	LOCK	4.0x 5.0	Ctr Ctr	0.65	
J	LOCK	4.0x 5.0	Ctr Ctr	0.60	
H	LOCK	8.0x 8.0	Ctr-1.5	0.74	
I	LOCK	4.0x 5.0	0.5-0.1	0.99	
G	LOCK	3.0x 4.0	Ctr Ctr	1.00	

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6977 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R157	9	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 465.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 15-FEB-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2040	636	3- 8	1-11
			Hz =	-178
C	2040	635	3- 8	1-11
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.28	8548	C	0.08	0.20
J -K	0.31	8715	C	0.09	0.22
K -S	0.18	5251	C	0.03	0.15
S -B	0.09	3759	C	0.04	0.05
B -T	0.09	3759	C	0.04	0.05
T -N	0.18	5250	C	0.03	0.15
N -O	0.30	8715	C	0.09	0.21
O -C	0.28	8562	C	0.08	0.20
Bottom Chords					
A -I	0.55	8076	T	0.37	0.18
I -E	0.55	8112	T	0.37	0.18
E -R	0.40	8116	T	0.37	0.03
R -Q	0.27	4953	T	0.22	0.05
Q -P	0.27	4953	T	0.22	0.05
P -D	0.40	8116	T	0.37	0.03
D -F	0.55	8125	T	0.37	0.18
F -C	0.55	8090	T	0.37	0.18
Webs					
I -J	0.03	134	T		
J -E	0.08	257	T		
E -K	0.58	1828	T		
K -R	0.84	3299	C		1 Br
R -S	0.34	1061	T		
S -Q	0.50	1803	C		1 Br
Q -B	0.74	2311	T		
Q -T	0.50	1802	C		1 Br
P -T	0.34	1060	T		
P -N	0.84	3300	C		1 Br
D -N	0.58	1830	T		
D -O	0.07	244	T		
F -O	0.03	132	T		

LL Defl -0.69" in Q -P L/872
TL Defl -1.40" in Q -P L/433
Shear // Grain in A -J 0.20

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

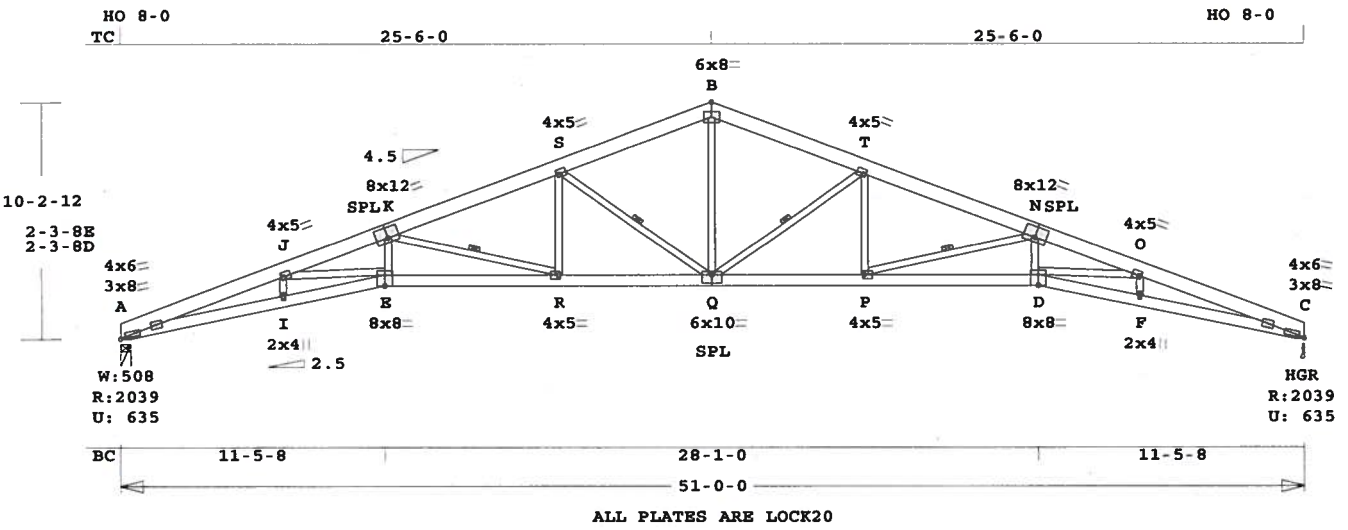
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr-1.5		0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x10.0	Ctr-1.2		0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.96
D	LOCK	8.0x 8.0	Ctr-1.5		0.81
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8715 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R156*	5	SP	510000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 465.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 15-FEB-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	51- 0- 0
BC	Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2040	636	5- 8	1-11
			Hz =	-178
C	2040	635	3- 8	1-11
			Hz =	179

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -J	0.28	8548 C	0.08 0.20
J -K	0.31	8715 C	0.09 0.22
K -S	0.18	5251 C	0.03 0.15
S -B	0.09	3759 C	0.04 0.05
B -T	0.09	3759 C	0.04 0.05
T -N	0.18	5250 C	0.03 0.15
N -O	0.30	8715 C	0.09 0.21
O -C	0.28	8562 C	0.08 0.20
Bottom Chords			
A -I	0.55	8076 T	0.37 0.18
I -E	0.55	8112 T	0.37 0.18
E -R	0.40	8116 T	0.37 0.03
R -Q	0.27	4953 T	0.22 0.05
Q -P	0.27	4953 T	0.22 0.05
P -D	0.40	8116 T	0.37 0.03
D -F	0.55	8125 T	0.37 0.18
F -C	0.55	8090 T	0.37 0.18
Webs			
I -J	0.03	134 T	
J -E	0.08	257 T	
E -K	0.58	1828 T	
K -R	0.84	3299 C	1 Br
R -S	0.34	1061 T	
S -Q	0.50	1803 C	1 Br
Q -B	0.74	2311 T	
Q -T	0.50	1802 C	1 Br
P -T	0.34	1060 T	
P -N	0.84	3300 C	1 Br
D -N	0.58	1830 T	
D -O	0.07	244 T	
F -O	0.03	132 T	

LL Defl -0.69" in Q -P L/870
TL Defl -1.40" in Q -P L/431
Shear // Grain in A -J 0.20

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

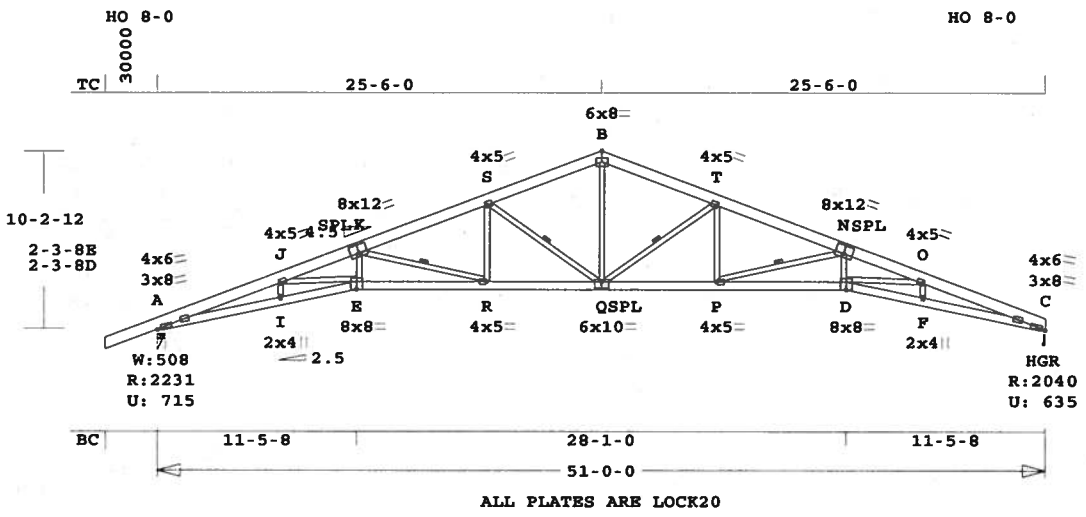
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0-0.5	0.2		0.62
K	LOCK	8.0x12.0-0.6	1.5		0.97
S	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.56
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr-1.5		0.80
R	LOCK	4.0x 5.0	Ctr	Ctr	0.96
Q	LOCK	6.0x10.0	Ctr-1.2		0.97
P	LOCK	4.0x 5.0	Ctr	Ctr	0.96
D	LOCK	8.0x 8.0	Ctr-1.5		0.81
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8715 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R155	6	SP	510000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 477.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2232	715	5- 8	1-14
			Hz =	-178
C	2040	636	3- 8	1-11
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.28	8548 C	0.08		0.20
J -K	0.30	8714 C	0.09		0.21
K -S	0.18	5250 C	0.03		0.15
S -B	0.09	3758 C	0.04		0.05
B -T	0.09	3758 C	0.04		0.05
T -N	0.18	5250 C	0.03		0.15
N -O	0.30	8714 C	0.09		0.21
O -C	0.28	8562 C	0.08		0.20
Bottom Chords					
A -I	0.55	8074 T	0.37		0.18
I -E	0.55	8110 T	0.37		0.18
E -R	0.40	8114 T	0.37		0.03
R -Q	0.27	4952 T	0.22		0.05
Q -P	0.27	4951 T	0.22		0.05
P -D	0.40	8114 T	0.37		0.03
D -F	0.55	8123 T	0.37		0.18
F -C	0.55	8088 T	0.37		0.18
Webs					
I -J	0.03	134 T			
J -E	0.08	257 T			
E -K	0.58	1828 T			
K -R	0.84	3299 C			1 Br
R -S	0.34	1060 T			
S -Q	0.50	1802 C			1 Br
Q -B	0.74	2311 T			
Q -T	0.50	1802 C			1 Br
P -T	0.34	1060 T			
P -N	0.84	3299 C			1 Br
D -N	0.58	1830 T			
D -O	0.07	244 T			
F -O	0.03	132 T			

TL Defl -1.40" in Q -P L/432
LL Defl -0.69" in Q -P L/870

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Shear // Grain in A -J 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES TO BE MANUFACTURED BY ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

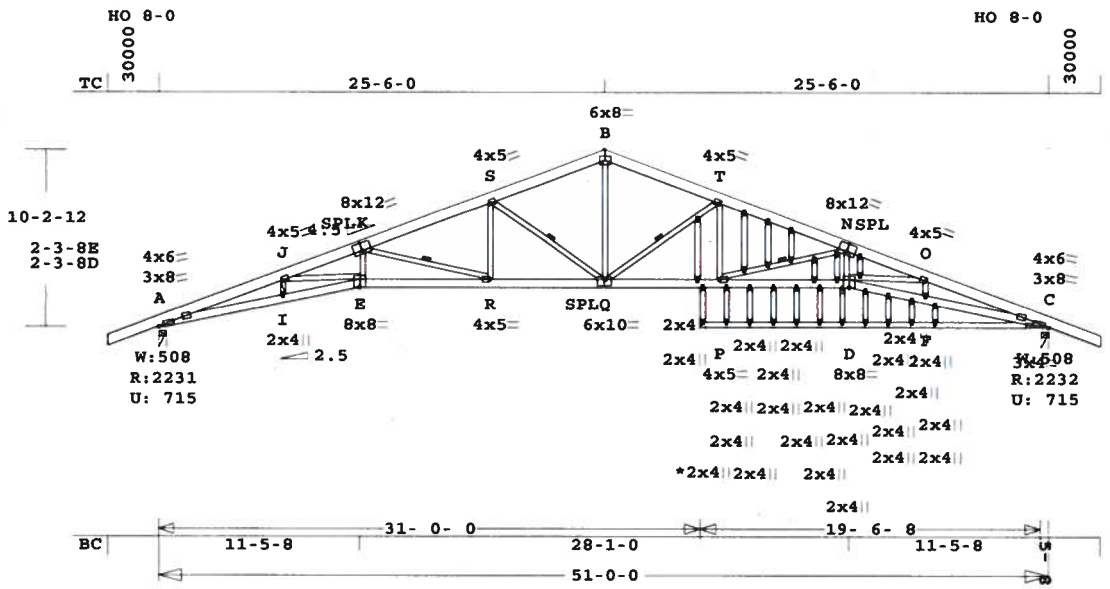
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.86
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.86
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	-1.0	Ctr	1.00
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	1.0	Ctr	1.00
D	LOCK	8.0x 8.0	Ctr	-1.5	0.81
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8714 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R154B	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

See * For Typical Gable Plate Size and Placement

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 592.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-177
C	2232	715	5- 8	1-14
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -J	0.28	8546	C	0.08 0.20
J -K	0.30	8712	C	0.09 0.21
K -S	0.18	5249	C	0.03 0.15
S -B	0.09	3758	C	0.04 0.05
B -T	0.09	3758	C	0.04 0.05
T -N	0.18	5249	C	0.03 0.15
N -O	0.30	8711	C	0.09 0.21
O -C	0.28	8545	C	0.08 0.20
--------Bottom Chords---------				
A -I	0.55	8070	T	0.37 0.18
I -E	0.55	8106	T	0.37 0.18
E -R	0.40	8110	T	0.37 0.03
R -Q	0.27	4949	T	0.22 0.05
Q -P	0.27	4949	T	0.22 0.05
P -D	0.40	8109	T	0.37 0.03
D -F	0.55	8106	T	0.37 0.18
F -C	0.55	8070	T	0.37 0.18
------------Webs------------				
I -J	0.03	134	T	
J -E	0.08	257	T	
E -K	0.58	1827	T	
K -R	0.84	3298	C	1 Br
R -S	0.34	1060	T	
S -Q	0.50	1802	C	1 Br
Q -B	0.74	2311	T	
Q -T	0.50	1802	C	1 Br
P -T	0.34	1060	T	
P -N	0.84	3297	C	1 Br
D -N	0.58	1826	T	
D -O	0.08	257	T	
F -O	0.03	134	T	

TL Defl -1.39" in R -Q L/431
LL Defl -0.69" in R -Q L/868

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Shear // Grain in A -J 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0 Ctr	Ctr		0.81
A	LOCK	4.0x 6.0 Ctr	Ctr		0.76
J	LOCK	4.0x 5.0-0.5	0.2		0.62
K	LOCK	8.0x12.0-0.6	1.5		0.97
S	LOCK	4.0x 5.0-0.5	0.2		0.86
B	LOCK	6.0x 8.0 Ctr	Ctr		0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.86
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0 Ctr	Ctr		0.81
C	LOCK	4.0x 6.0 Ctr	Ctr		0.76
I	LOCK	2.0x 4.0 Ctr	Ctr		0.54
E	LOCK	8.0x 8.0 Ctr	-1.5		0.80
R	LOCK	4.0x 5.0	-1.0	Ctr	1.00
Q	LOCK	6.0x10.0 Ctr	-1.2		0.97
P	LOCK	4.0x 5.0	1.0	Ctr	1.00
D	LOCK	8.0x 8.0 Ctr	-1.5		0.80
F	LOCK	2.0x 4.0 Ctr	Ctr		0.54

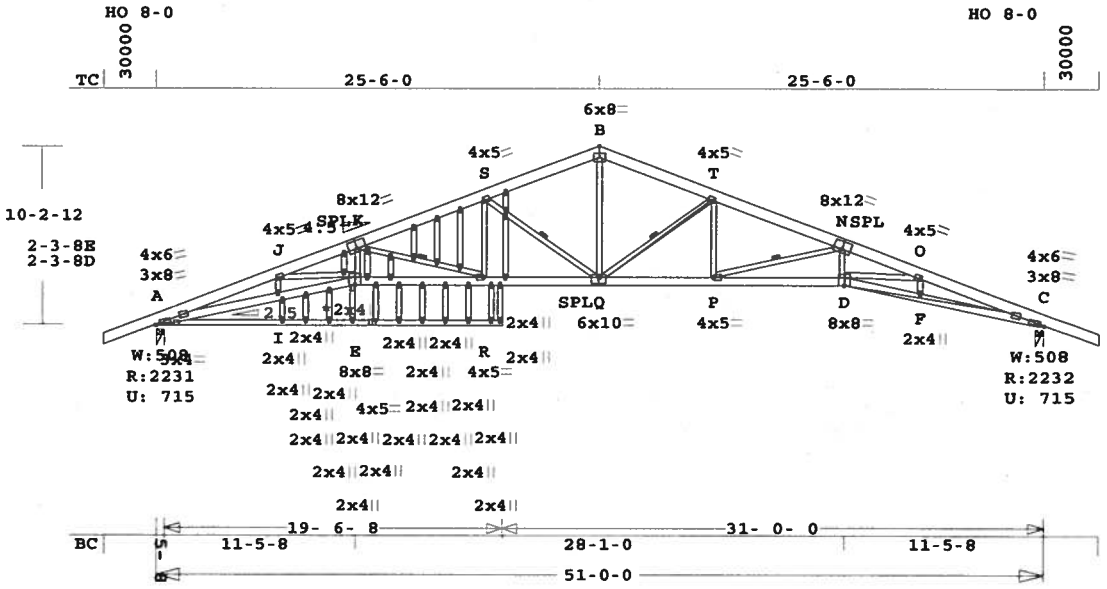
7 Gable studs to be attached with 2.0x4.0 plates each end.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8712 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R154A	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

See * For Typical Gable Plate Size and Placement

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 595.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	Size	Lumber
TC	0.30	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-177
C	2232	715	5- 8	1-14
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.28	8546	C	0.08	0.20
J -K	0.30	8712	C	0.09	0.21
K -S	0.18	5249	C	0.03	0.15
S -B	0.09	3758	C	0.04	0.05
B -T	0.09	3758	C	0.04	0.05
T -N	0.18	5249	C	0.03	0.15
N -O	0.30	8711	C	0.09	0.21
O -C	0.28	8545	C	0.08	0.20
Bottom Chords					
A -I	0.55	8070	T	0.37	0.18
I -E	0.55	8106	T	0.37	0.18
E -R	0.40	8110	T	0.37	0.03
R -Q	0.27	4949	T	0.22	0.05
Q -P	0.27	4949	T	0.22	0.05
P -D	0.40	8109	T	0.37	0.03
D -F	0.55	8106	T	0.37	0.18
F -C	0.55	8070	T	0.37	0.18
Webs					
I -J	0.03	134	T		
J -E	0.08	257	T		
E -K	0.58	1827	T		
K -R	0.84	3298	C		1 Br
R -S	0.34	1060	T		
S -Q	0.50	1802	C		1 Br
Q -B	0.74	2311	T		
Q -T	0.50	1802	C		1 Br
P -T	0.34	1060	T		
P -N	0.84	3297	C		1 Br
D -N	0.58	1826	T		
D -O	0.08	257	T		
F -O	0.03	134	T		

TL Defl -1.39" in R -Q L/431
LL Defl -0.69" in R -Q L/868

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Shear // Grain in A -J 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.86
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.86
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	-1.0	Ctr	1.00
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	1.0	Ctr	1.00
D	LOCK	8.0x 8.0	Ctr	-1.5	0.80
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

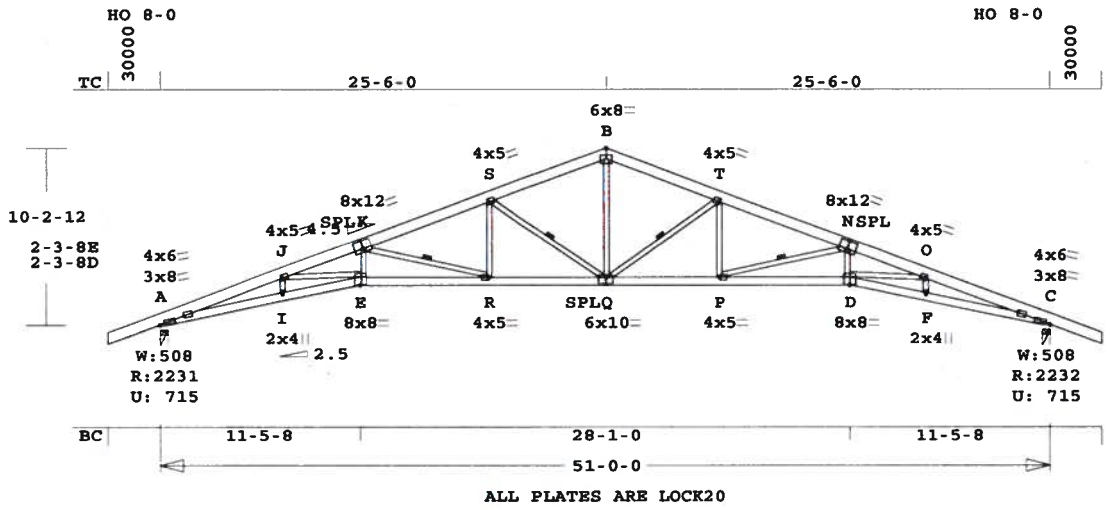
8 Gable studs to be attached with 2.0x4.0 plates each end.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8712 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R154*	41	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 489.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.84	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	51- 0- 0
BC	Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on K -R
WB 1 rows CLB on S -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on P -N
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-177
C	2232	715	5- 8	1-14
			Hz =	179

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -J	0.28	8546	C	0.08	0.20
J -K	0.30	8712	C	0.09	0.21
K -S	0.18	5249	C	0.03	0.15
S -B	0.09	3758	C	0.04	0.05
B -T	0.09	3758	C	0.04	0.05
T -N	0.18	5249	C	0.03	0.15
N -O	0.30	8711	C	0.09	0.21
O -C	0.28	8545	C	0.08	0.20
--------Bottom Chords---------					
A -I	0.55	8070	T	0.37	0.18
I -E	0.55	8106	T	0.37	0.18
E -R	0.40	8110	T	0.37	0.03
R -Q	0.27	4949	T	0.22	0.05
Q -P	0.27	4949	T	0.22	0.05
P -D	0.40	8109	T	0.37	0.03
D -F	0.55	8106	T	0.37	0.18
F -C	0.55	8070	T	0.37	0.18
------------Webs------------					
I -J	0.03	134	T		
J -E	0.08	257	T		
E -K	0.58	1827	T		
K -R	0.84	3298	C		1 Br
R -S	0.34	1060	T		
S -Q	0.50	1802	C		1 Br
Q -B	0.74	2311	T		
Q -T	0.50	1802	C		1 Br
P -T	0.34	1060	T		
P -N	0.84	3297	C		1 Br
D -N	0.58	1826	T		
D -O	0.08	257	T		
F -O	0.03	134	T		

TL Defl -1.39" in R -Q L/431
LL Defl -0.69" in R -Q L/868

Hz Disp	LL	DL	TL
Jt C	0.43"	0.43"	0.86"

Shear // Grain in A -J 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

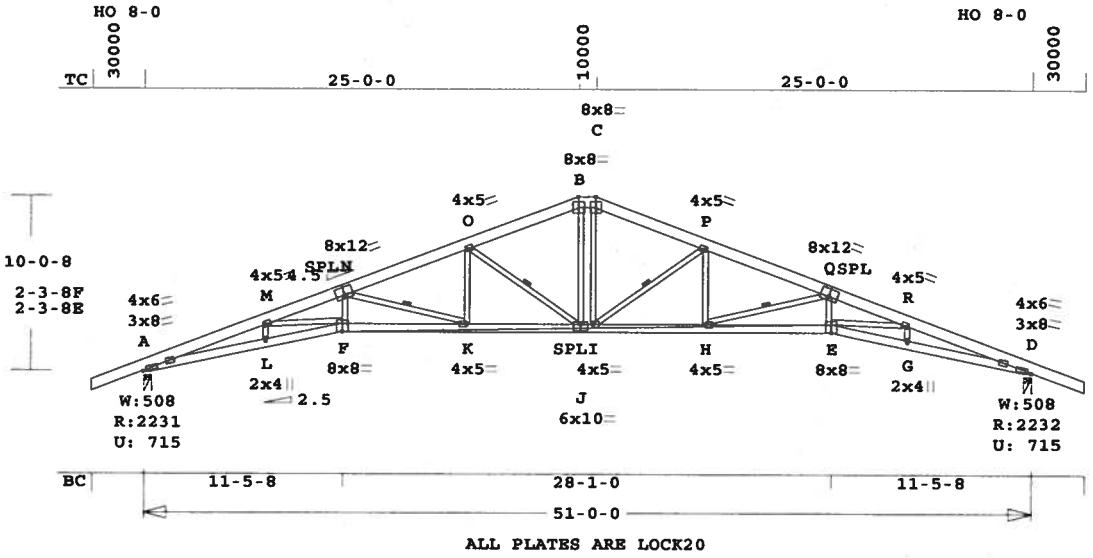
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
J	LOCK	4.0x 5.0	-0.5	0.2	0.62
K	LOCK	8.0x12.0	-0.6	1.5	0.97
S	LOCK	4.0x 5.0	-0.5	0.2	0.86
B	LOCK	6.0x 8.0	Ctr	Ctr	0.80
T	LOCK	4.0x 5.0	0.5	0.2	0.86
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	4.0x 5.0	0.5	0.2	0.62
C	LOCK	3.0x 8.0	Ctr	Ctr	0.81
C	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	2.0x 4.0	Ctr	Ctr	0.54
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
R	LOCK	4.0x 5.0	-1.0	Ctr	1.00
Q	LOCK	6.0x10.0	Ctr	-1.2	0.97
P	LOCK	4.0x 5.0	1.0	Ctr	1.00
D	LOCK	8.0x 8.0	Ctr	-1.5	0.80
F	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8712 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R153	2	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 500.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.79	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	51- 0- 0
BC	Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-174
D	2232	715	5- 8	1-14
			Hz =	175

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.28	8543	C	0.08	0.20
M -N	0.31	8708	C	0.09	0.22
N -O	0.17	5315	C	0.03	0.14
O -B	0.09	3827	C	0.04	0.05
B -C	0.04	3571	C	0.04	0.00
C -P	0.09	3827	C	0.04	0.05
P -Q	0.17	5315	C	0.03	0.14
Q -R	0.31	8707	C	0.09	0.22
R -D	0.28	8542	C	0.08	0.20
Bottom Chords					
A -L	0.54	8065	T	0.36	0.18
L -F	0.55	8101	T	0.37	0.18
F -K	0.40	8105	T	0.37	0.03
K -J	0.26	5009	T	0.22	0.04
J -I	0.20	3570	T	0.16	0.04
I -H	0.26	5009	T	0.22	0.04
H -E	0.40	8105	T	0.37	0.03
E -G	0.55	8101	T	0.37	0.18
G -D	0.54	8065	T	0.36	0.18
Webs					
L -M	0.03	135	T		
M -F	0.08	256	T		
F -N	0.58	1825	T		
N -K	0.79	3239	C		1 Br
K -O	0.34	1070	T		
O -J	0.48	1793	C		1 Br
J -B	0.36	1152	T		
I -C	0.36	1152	T		
I -P	0.48	1793	C		1 Br
H -P	0.34	1070	T		
H -Q	0.79	3239	C		1 Br
E -Q	0.58	1825	T		
E -R	0.08	256	T		
G -R	0.03	135	T		

TL Defl -1.38" in K -J L/434
LL Defl -0.69" in K -J L/876

Hz Disp	LL	DL	TL
Jt D	0.42"	0.43"	0.85"

Shear // Grain in A -M 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

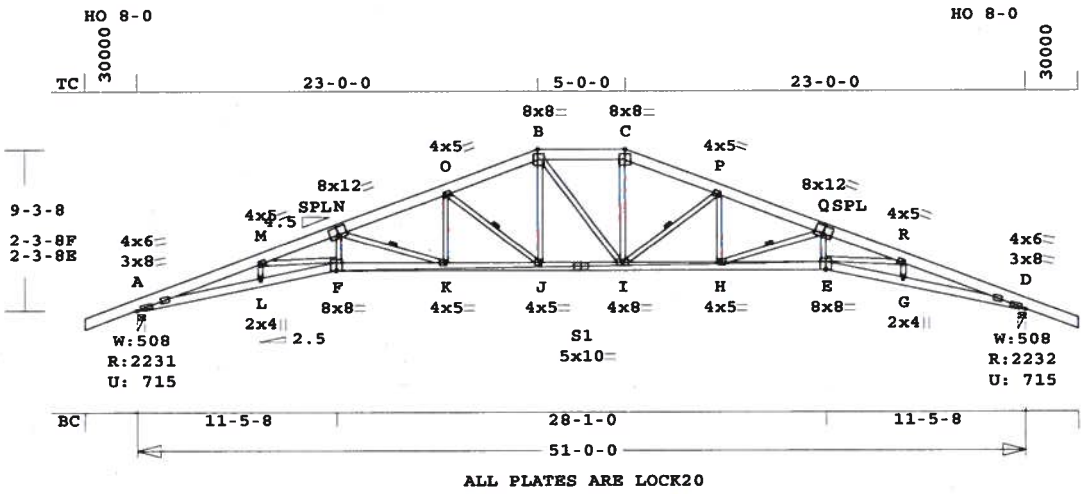
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	4.0x 5.0-0.5	0.2	0.2	0.62
N	LOCK	8.0x12.0-0.6	1.5	0.2	0.97
O	LOCK	4.0x 5.0-0.5	0.2	0.2	0.85
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.85
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.76
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr-1.5		0.80
K	LOCK	4.0x 5.0-1.0	Ctr		1.00
J	LOCK	6.0x10.0	Ctr-1.3		0.97
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	4.0x 5.0	1.0	Ctr	1.00
E	LOCK	8.0x 8.0	Ctr-1.5		0.80
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8708 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R152	2	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 501.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.63	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on N -K
WB 1 rows CLB on O -J
WB 1 rows CLB on I -P
WB 1 rows CLB on H -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2232	715	5- 8	1-14
			Hz =	-160
D	2232	715	5- 8	1-14
			Hz =	161

Membr	CSI	P Lbs	Axl	CSI	Bnd
--------Top Chords--------					
A -M	0.28	8538	C	0.08	0.20
M -N	0.31	8714	C	0.09	0.22
N -O	0.16	5592	C	0.04	0.12
O -B	0.08	4219	C	0.05	0.03
B -C	0.06	3954	C	0.01	0.05
C -P	0.08	4228	C	0.05	0.03
P -Q	0.16	5594	C	0.04	0.12
Q -R	0.31	8714	C	0.09	0.22
R -D	0.28	8538	C	0.08	0.20
--------Bottom Chords---------					
A -L	0.54	8061	T	0.36	0.18
L -F	0.55	8097	T	0.37	0.18
F -K	0.41	8113	T	0.37	0.04
K -J	0.27	5266	T	0.24	0.03
J -S1	0.18	3932	T	0.18	0.00
S1-I	0.21	3932	T	0.18	0.03
I -H	0.27	5270	T	0.24	0.03
H -E	0.41	8112	T	0.37	0.04
E -G	0.55	8098	T	0.37	0.18
G -D	0.54	8062	T	0.36	0.18
------------Webs------------					
L -M	0.03	137	T		
M -F	0.08	268	T		
F -N	0.58	1821	T		
N -K	0.63	3025	C		1 Br
K -O	0.36	1122	T		
O -J	0.38	1691	C		1 Br
J -B	0.36	1152	T		
B -I	0.10	144	T		
I -C	0.37	1168	T		
I -P	0.38	1691	C		1 Br
H -P	0.35	1103	T		
H -Q	0.63	3020	C		1 Br
E -Q	0.58	1825	T		
E -R	0.08	266	T		
G -R	0.03	137	T		

TL Defl -1.36" in K -J L/440
LL Defl -0.68" in K -J L/888

Hz Disp	LL	DL	TL
Jt D	0.41"	0.42"	0.83"

Shear // Grain in A -M 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

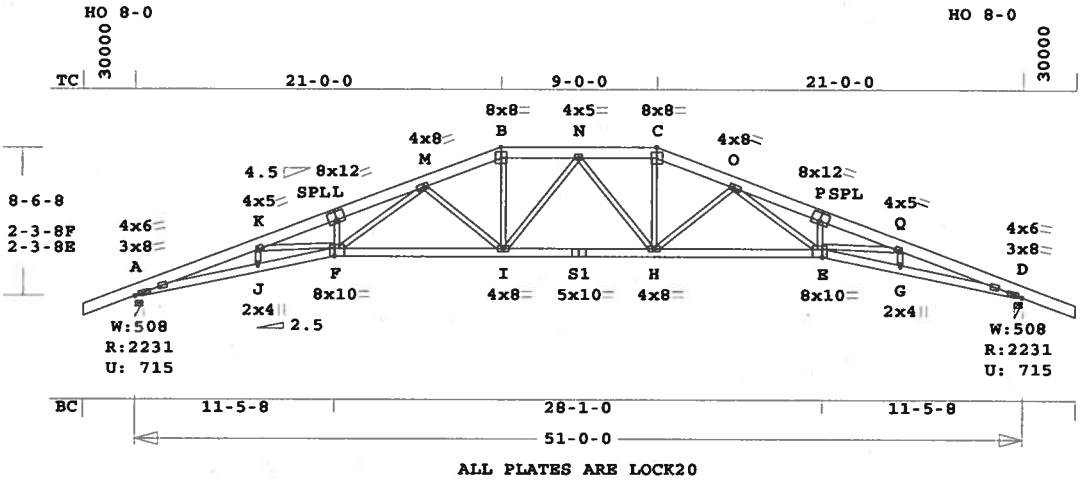
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	4.0x 5.0	-0.5	0.2	0.62
N	LOCK	8.0x12.0	-0.6	1.5	0.97
O	LOCK	4.0x 5.0	-0.5	0.2	0.80
B	LOCK	8.0x 8.0	-1.0	Ctr	0.84
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
P	LOCK	4.0x 5.0	0.5	0.2	0.81
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.76
L	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x 8.0	Ctr	-1.5	0.80
K	LOCK	4.0x 5.0	Ctr	Ctr	1.00
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr	Ctr	0.59
H	LOCK	4.0x 5.0	Ctr	Ctr	1.00
E	LOCK	8.0x 8.0	Ctr	-1.5	0.80
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8714 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R151*	2	HPSC4	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 490.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 8	SP-2400
BC	0.62	2x 6	SP-2400
EX F -S1		2x 6	SP-#2
EX S1-E		2x 6	SP-#2
WB	0.80	2x 4	SP-#3
EX F -M		2x 4	SP-#2
EX O -E		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2232	715	5- 8	1-14
			Hz =	-146
D	2231	715	5- 8	1-14
			Hz =	147

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -K	0.31	8299 C	0.07	0.24
K -L	0.34	8904 C	0.09	0.25
L -M	0.21	8893 C	0.09	0.12
M -B	0.07	4693 C	0.05	0.02
B -N	0.06	4392 C	0.05	0.01
N -C	0.06	4392 C	0.05	0.01
C -O	0.07	4693 C	0.05	0.02
O -P	0.21	8893 C	0.09	0.12
P -Q	0.34	8904 C	0.09	0.25
Q -D	0.31	8299 C	0.07	0.24
--------Bottom Chords---------				
A -J	0.61	7824 T	0.35	0.26
J -F	0.62	7888 T	0.36	0.26
F -I	0.87	5570 T	0.67	0.20
I -S1	0.70	4477 T	0.54	0.16
S1-H	0.70	4477 T	0.54	0.16
H -E	0.87	5570 T	0.67	0.20
E -G	0.62	7888 T	0.36	0.26
G -D	0.61	7824 T	0.35	0.26
------------Webs------------				
J -K	0.04	222 C		
K -F	0.21	679 T		
F -L	0.05	263 C		
F -M	0.66	3507 T		
M -I	0.80	1569 C		
I -B	0.43	1362 T		
I -N	0.10	148 T		
N -H	0.10	148 T		
H -C	0.43	1362 T		
H -O	0.80	1569 C		
O -E	0.66	3507 T		
E -P	0.05	263 C		
E -Q	0.21	678 T		
G -Q	0.04	222 C		

TL Defl -1.41" in F -I L/425
LL Defl -0.68" in F -I L/890

Hz Disp	LL	DL	TL
Jt D	0.40"	0.41"	0.81"

Shear // Grain in A -K 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

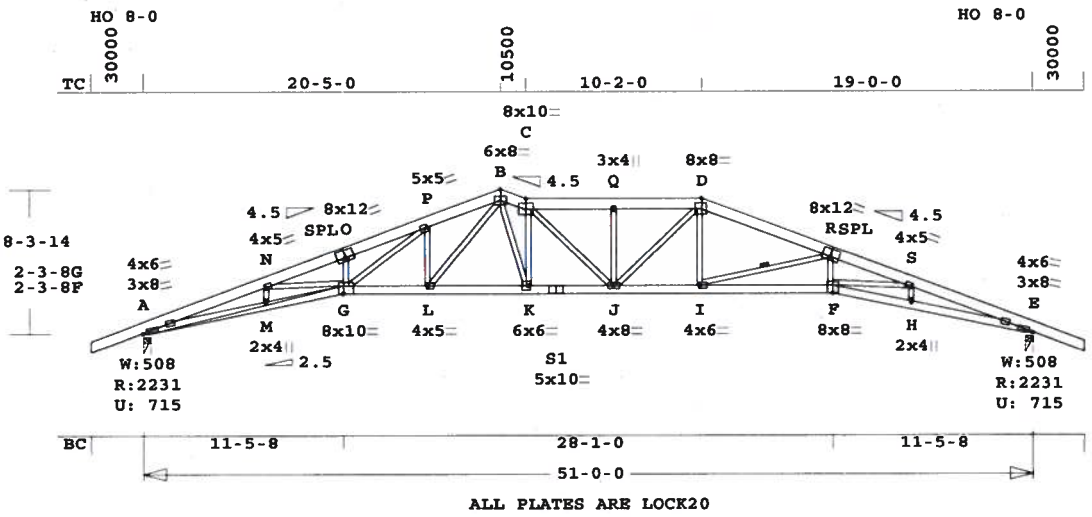
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.70
K	LOCK	4.0x 5.0	-0.5	0.2	0.62
L	LOCK	8.0x12.0	-0.6	1.5	0.97
M	LOCK	4.0x 8.0	-1.2	-0.1	0.81
B	LOCK	8.0x 8.0	Ctr	Ctr	0.84
N	LOCK	4.0x 5.0	Ctr	0.5	0.56
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
O	LOCK	4.0x 8.0	1.2	-0.1	0.81
P	LOCK	8.0x12.0	0.6	1.5	0.97
Q	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.81
D	LOCK	4.0x 6.0	Ctr	Ctr	0.70
J	LOCK	2.0x 4.0	Ctr	Ctr	0.54
F	LOCK	8.0x10.0	Ctr	-1.5	0.81
I	LOCK	4.0x 8.0	Ctr	Ctr	0.54
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
H	LOCK	4.0x 8.0	Ctr	Ctr	0.54
E	LOCK	8.0x10.0	Ctr	-1.5	0.81
G	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8904 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R150*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 504.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.33	2x 8	SP-2400
BC	0.61	2x 6	SP-2400
EX G -S1		2x 6	SP-#2
WB	0.89	2x 4	SP-#3
EX G -P		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	2232	715	5- 8	1-14
			Hz =	-141
E	2232	715	5- 8	1-14
			Hz =	143

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.30	8339	C	0.07	0.23
N -O	0.33	8902	C	0.09	0.24
O -P	0.21	8909	C	0.09	0.12
P -B	0.09	6029	C	0.04	0.05
B -C	0.09	5374	C	0.03	0.06
C -Q	0.08	5128	C	0.02	0.06
Q -D	0.08	5128	C	0.02	0.06
D -R	0.19	5183	C	0.03	0.16
R -S	0.30	8714	C	0.09	0.21
S -E	0.28	8543	C	0.08	0.20
Bottom Chords					
A -M	0.61	7865	T	0.36	0.25
M -G	0.61	7925	T	0.36	0.25
G -L	0.81	5667	T	0.68	0.13
L -K	0.59	4449	T	0.54	0.05
K -S1	0.67	5145	T	0.62	0.05
S1-J	0.26	5145	T	0.23	0.03
J -I	0.25	4857	T	0.22	0.03
I -F	0.40	8113	T	0.37	0.03
F -H	0.55	8103	T	0.37	0.18
H -E	0.54	8068	T	0.36	0.18
Webs					
M -N	0.04	201	C		
N -G	0.20	641	T		
G -O	0.05	309	C		
G -P	0.63	3419	T		
L -P	0.32	1356	C		
L -B	0.62	1932	T		
B -K	0.70	2191	T		
K -C	0.67	1962	C		
C -J	0.07	109	C		
J -Q	0.06	196	C		
J -D	0.12	377	T		
I -D	0.34	1083	T		
I -R	0.89	3365	C	1 Br	
F -R	0.58	1822	T		
F -S	0.08	262	T		
H -S	0.03	129	T		

TL Defl -1.48" in S1-J L/407
LL Defl -0.73" in S1-J L/818

Hz Disp	LL	DL	TL
Jt E	0.42"	0.43"	0.85"

Shear // Grain in A -N 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

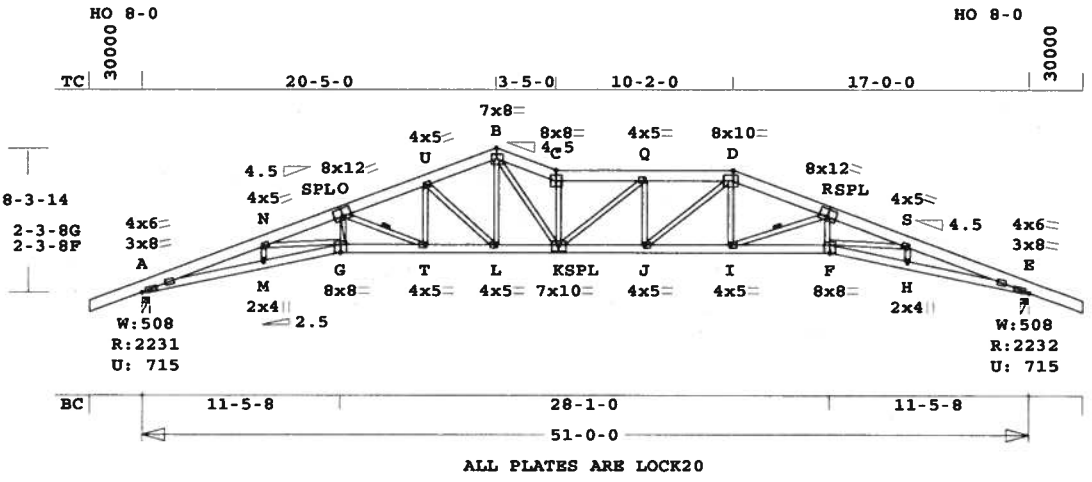
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.71
N	LOCK	4.0x 5.0-0.5	0.2		0.62
O	LOCK	8.0x12.0-0.6	1.5		0.97
P	LOCK	5.0x 5.0-0.5-0.2			0.95
B	LOCK	6.0x 8.0	Ctr	Ctr	0.95
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
Q	LOCK	3.0x 4.0	Ctr	Ctr	0.36
D	LOCK	8.0x 8.0	1.0	Ctr	0.84
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.62
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x10.0	Ctr-1.5		0.81
L	LOCK	4.0x 5.0	Ctr	Ctr	0.76
K	LOCK	6.0x 6.0	Ctr	Ctr	0.95
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
I	LOCK	4.0x 6.0	0.5	Ctr	0.96
F	LOCK	8.0x 8.0	Ctr-1.5		0.79
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8909 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R149*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 501.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.90	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on O -T
WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	2232	715	5- 8	1-14
			Hz =	-141
E	2233	716	5- 8	1-14
			Hz =	143

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.28	8527	C	0.08	0.20
N -O	0.31	8721	C	0.09	0.22
O -U	0.15	6014	C	0.04	0.11
U -B	0.07	4843	C	0.06	0.01
B -C	0.10	6447	C	0.04	0.06
C -Q	0.09	6082	C	0.04	0.05
Q -D	0.08	5984	C	0.04	0.04
D -R	0.17	5767	C	0.04	0.13
R -S	0.31	8696	C	0.09	0.22
S -E	0.28	8530	C	0.08	0.20
Bottom Chords					
A -M	0.54	8051	T	0.36	0.18
M -G	0.55	8088	T	0.37	0.18
G -T	0.41	8119	T	0.37	0.04
T -L	0.32	5666	T	0.25	0.07
L -K	0.28	4499	T	0.20	0.08
K -J	0.35	5983	T	0.27	0.08
J -I	0.26	5407	T	0.24	0.02
I -F	0.41	8091	T	0.37	0.04
F -H	0.55	8089	T	0.37	0.18
H -E	0.54	8052	T	0.36	0.18
Webs					
M -N	0.03	141	T		
N -G	0.09	284	T		
G -O	0.58	1833	T		
O -T	0.49	2702	C		1 Br
T -U	0.36	1126	T		
U -L	0.69	1581	C		
L -B	0.42	1318	T		
B -K	0.90	2807	T		
K -C	0.64	2450	C		
K -Q	0.06	129	T		
J -Q	0.08	316	C		
J -D	0.23	738	T		
I -D	0.36	1125	T		
I -R	0.56	2851	C		1 Br
F -R	0.58	1820	T		
F -S	0.08	254	T		
H -S	0.03	140	T		

TL Defl -1.56" in K -J L/384
LL Defl -0.78" in K -J L/775

Hz Disp	LL	DL	TL
Jt E	0.42"	0.43"	0.85"

Shear // Grain in A -N 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

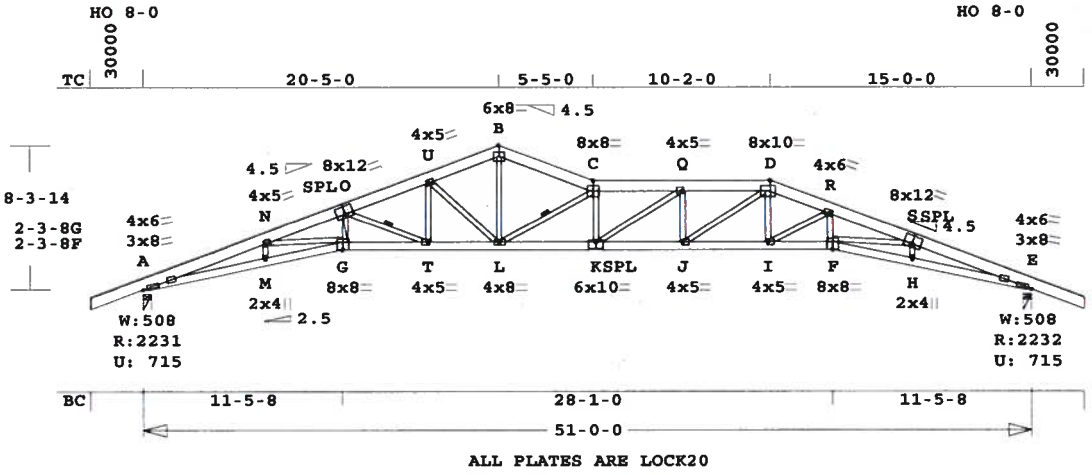
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
N	LOCK	4.0x 5.0-0.5	0.2		0.62
O	LOCK	8.0x12.0-0.6	1.5		0.97
U	LOCK	4.0x 5.0-0.5	0.2		0.76
B	LOCK	7.0x 8.0	0.5	Ctr	0.96
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.63
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.76
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x 8.0	Ctr-1.5		0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.88
L	LOCK	4.0x 5.0	Ctr	Ctr	0.62
K	LOCK	7.0x10.0	Ctr-0.8		0.97
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.94
F	LOCK	8.0x 8.0	Ctr-1.5		0.79
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8721 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R148*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 492.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.55	2x 6	SP-2400
WB	0.95	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

WB 1 rows CLB on O -T
WB 1 rows CLB on L -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2231	715	5- 8	1-14
			Hz =	-141
E	2232	715	5- 8	1-14
			Hz =	143

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -N	0.28	8526 C	0.08 0.20
N -O	0.31	8727 C	0.09 0.22
O -U	0.15	5994 C	0.04 0.11
U -B	0.07	4865 C	0.06 0.01
B -C	0.18	4830 C	0.02 0.16
C -Q	0.20	7482 C	0.06 0.14
Q -D	0.13	7014 C	0.05 0.08
D -R	0.17	6511 C	0.04 0.13
R -S	0.31	8697 C	0.09 0.22
S -E	0.28	8514 C	0.08 0.20
Bottom Chords			
A -M	0.54	8050 T	0.36 0.18
M -G	0.55	8087 T	0.37 0.18
G -T	0.41	8126 T	0.37 0.04
T -L	0.35	5640 T	0.25 0.10
L -K	0.47	7493 T	0.34 0.13
K -J	0.45	7014 T	0.32 0.13
J -I	0.31	6116 T	0.28 0.03
I -F	0.41	8091 T	0.37 0.04
F -H	0.55	8076 T	0.36 0.19
H -E	0.55	8037 T	0.36 0.19
Webs			
M -N	0.03	140 T	
N -G	0.09	292 T	
G -O	0.58	1835 T	
O -T	0.50	2738 C	1 Br
T -U	0.35	1117 T	
U -L	0.65	1490 C	
L -B	0.95	2975 T	
L -C	0.71	3375 C	1 Br
K -C	0.07	379 C	
K -Q	0.18	571 T	
J -Q	0.08	405 C	
J -D	0.34	1071 T	
I -D	0.39	1215 T	
I -R	0.59	2269 C	
F -R	0.58	1827 T	
F -S	0.08	269 T	
H -S	0.03	147 T	

TL Defl -1.78" in K -J L/337
LL Defl -0.88" in K -J L/679

Hz Disp	LL	DL	TL
Jt E	0.45"	0.45"	0.90"

Shear // Grain in A -N 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

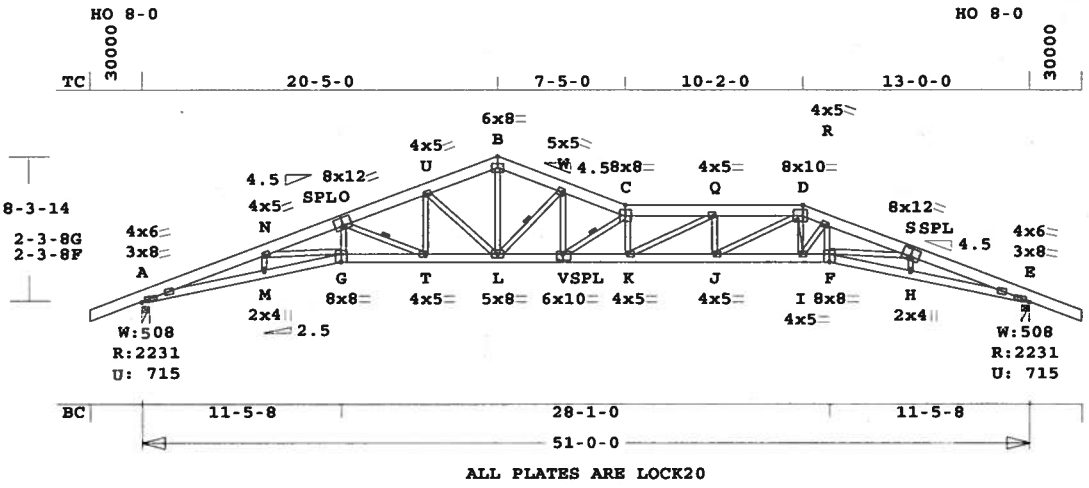
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
N	LOCK	4.0x 5.0-0.5	0.2		0.62
O	LOCK	8.0x12.0-0.6	1.5		0.97
U	LOCK	4.0x 5.0-0.5	0.2		0.71
B	LOCK	6.0x 8.0	Ctr	Ctr	0.95
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 6.0	0.5	0.2	0.89
S	LOCK	8.0x12.0	0.6	1.5	0.97
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.75
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x 8.0	Ctr-1.5		0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.89
L	LOCK	4.0x 8.0	0.5	Ctr	1.00
K	LOCK	6.0x10.0	Ctr-1.2		0.97
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.73
F	LOCK	8.0x 8.0	Ctr-1.5		0.79
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8727 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R147*	2	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 496.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.62	2x 6	SP-2400
WB	0.69	2x 4	SP-#3
EX L -B		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51- 0- 0
BC Cont.	0- 0- 0	51- 0- 0

WB 1 rows CLB on O -T
WB 1 rows CLB on L -W
WB 1 rows CLB on V -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2231	715	5- 8	1-14
			Hz =	-141
E	2232	715	5- 8	1-14
			Hz =	143

Membr	CSI	P Lbs	Axl-CSI-Bnd	
		Top Chords		
A -N	0.28	8528 C	0.08	0.20
N -O	0.31	8725 C	0.09	0.22
O -U	0.15	6004 C	0.04	0.11
U -B	0.07	4835 C	0.05	0.02
B -W	0.09	4862 C	0.02	0.07
W -C	0.21	6744 C	0.04	0.17
C -Q	0.23	9422 C	0.09	0.14
Q -D	0.19	8490 C	0.08	0.11
D -R	0.18	7523 C	0.06	0.12
R -S	0.31	8706 C	0.09	0.22
S -E	0.28	8497 C	0.08	0.20
		Bottom Chords		
A -M	0.54	8052 T	0.36	0.18
M -G	0.55	8089 T	0.37	0.18
G -T	0.41	8125 T	0.37	0.04
T -L	0.29	5652 T	0.25	0.04
L -V	0.33	6404 T	0.29	0.04
V -K	0.62	9447 T	0.43	0.19
K -J	0.57	8489 T	0.38	0.19
J -I	0.35	7127 T	0.32	0.03
I -F	0.42	8100 T	0.37	0.05
F -H	0.55	8059 T	0.36	0.19
H -E	0.55	8020 T	0.36	0.19
		Webs		
M -N	0.03	140 T		
N -G	0.09	289 T		
G -O	0.58	1830 T		
O -T	0.49	2723 C		1 Br
T -U	0.37	1154 T		
U -L	0.68	1554 C		
L -B	0.59	3211 T		
L -W	0.51	2748 C		1 Br
V -W	0.69	2161 T		
V -C	0.63	3623 C		1 Br
K -C	0.11	628 C		
K -Q	0.34	1064 T		
J -Q	0.08	499 C		
J -D	0.48	1520 T		
I -D	0.40	1263 T		
I -R	0.27	1575 C		
F -R	0.59	1854 T		
F -S	0.09	294 T		
H -S	0.03	154 T		

TL Defl -2.03" in K -J L/295
LL Defl -1.01" in K -J L/596

Hz Disp	LL	DL	TL
Jt E	0.48"	0.48"	0.96"

Shear // Grain in D -R 0.26

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.81
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
N	LOCK	4.0x 5.0	-0.5	0.2	0.62
O	LOCK	8.0x12.0	-0.6	1.5	0.97
U	LOCK	4.0x 5.0	-0.5	0.2	0.74
B	LOCK	6.0x 8.0	Ctr	Ctr	0.81
W	LOCK	5.0x 5.0	0.5	-0.2	0.86
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
Q	LOCK	4.0x 5.0	Ctr	0.5	0.62
D	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 5.0	0.5	0.2	0.91
S	LOCK	8.0x12.0	0.6	1.5	0.97
E	LOCK	3.0x 8.0	Ctr	Ctr	0.81
E	LOCK	4.0x 6.0	Ctr	Ctr	0.75
M	LOCK	2.0x 4.0	Ctr	Ctr	0.54
G	LOCK	8.0x 8.0	Ctr	-1.5	0.80
T	LOCK	4.0x 5.0	Ctr	Ctr	0.89
L	LOCK	5.0x 8.0	Ctr	Ctr	0.86
V	LOCK	6.0x10.0	0.5	-1.2	0.97
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
J	LOCK	4.0x 5.0	Ctr	Ctr	0.61
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	8.0x 8.0	Ctr	-1.5	0.78
H	LOCK	2.0x 4.0	Ctr	Ctr	0.54

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9422 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R146*	2*3P	SP	510000	4.5	3- 0- 0	3- 0- 0	

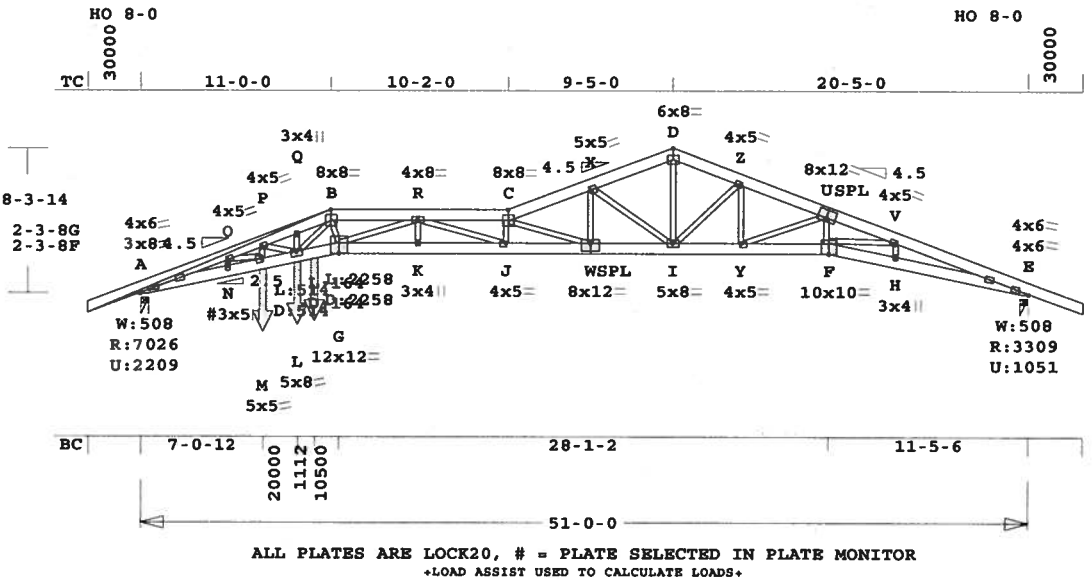
+Office and Care Center, Hospice, M.M. Parrish

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 35023 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R146	2*3P	SP	510000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 536.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.42	2x 8	SP-2400
BC	0.73	2x 8	SP-2400
WB	0.86	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 0- 0
BC Cont.		0- 0- 0	51- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	514	514	7.1'	CL-LB
BC V	164	164	9.1'	CL-LB
BC V	2258	2258	10.0'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	7026	2209	5- 8	1-15
			Hz =	-144
E	3309	1051	5- 8	1- 8
			Hz =	143

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -O	0.38	28818 C	0.10 0.28
O -P	0.42	33654 C	0.14 0.28
P -Q	0.31	35023 C	0.15 0.16
Q -B	0.28	34852 C	0.15 0.13
B -R	0.32	32729 C	0.12 0.20
R -C	0.30	28962 C	0.10 0.20
C -X	0.27	15955 C	0.03 0.24
X -D	0.14	9320 C	0.01 0.13
D -Z	0.03	9204 C	0.03 0.00
Z -U	0.09	10731 C	0.01 0.08
U -V	0.11	15182 C	0.02 0.09
V -E	0.09	14622 C	0.02 0.07
Bottom Chords			
A -N	0.61	27012 T	0.31 0.30
N -M	0.61	27367 T	0.31 0.30
M -L	0.54	32612 T	0.37 0.17
L -G	0.59	29774 T	0.33 0.26
G -K	0.40	29922 T	0.34 0.06
K -J	0.73	29922 T	0.34 0.39
J -W	0.72	29065 T	0.33 0.39
W -I	0.29	15254 T	0.17 0.12
I -Y	0.13	10120 T	0.11 0.02
Y -F	0.18	14086 T	0.16 0.02
F -H	0.25	13968 T	0.16 0.09
H -E	0.25	13883 T	0.16 0.09
Webs			
N -O	0.08	1696 C	
O -M	0.56	5285 T	
M -P	0.08	1579 C	
P -L	0.12	1197 T	
L -Q	0.03	448 T	
L -B	0.54	5115 T	
B -G	0.86	8070 T	
G -R	0.31	3344 T	
K -R	0.05	656 T	
R -J	0.06	1466 C	
J -C	0.07	1422 C	
C -W	0.80	14540 C	
W -X	0.65	6091 T	
X -I	0.47	8364 C	
I -D	0.69	6485 T	
I -Z	0.11	2108 C	
Y -Z	0.17	1629 T	
Y -U	0.23	4314 C	
F -U	0.33	3179 T	
F -V	0.06	586 T	
H -V	0.01	331 C	

TL Defl -2.26" in K -J L/266
LL Defl -1.12" in K -J L/534

Hz Disp	LL	DL	TL
Jt E	0.43"	0.43"	0.85"

Shear // Grain in L -G 0.51

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.88
A	LOCK	4.0x 6.0	Ctr	Ctr	0.82
O	LOCK	4.0x 5.0	-0.5	0.2	0.85
P	LOCK	4.0x 5.0	-0.5	0.2	0.56
Q	LOCK	3.0x 4.0	Ctr	Ctr	0.36
B	LOCK	8.0x 8.0	Ctr	Ctr	0.97
R	LOCK	4.0x 8.0	Ctr	0.5	0.74
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
X	LOCK	5.0x 5.0	Ctr	Ctr	0.97
D	LOCK	6.0x 8.0	Ctr	Ctr	0.80
Z	LOCK	4.0x 5.0	0.5	0.2	0.56
U	LOCK	8.0x12.0	0.6	1.5	0.97
V	LOCK	4.0x 5.0	0.5	0.2	0.61
E	LOCK	4.0x 6.0	Ctr	Ctr	0.69
E	LOCK	4.0x 6.0	Ctr	Ctr	0.30
N#	LOCK	3.0x 5.0	-0.1	-0.6	0.36
M	LOCK	5.0x 5.0	Ctr	Ctr	0.78
L	LOCK	5.0x 8.0	0.2	-1.0	0.97
G	LOCK	12.0x12.0	Ctr	-1.2	0.92
K	LOCK	3.0x 4.0	Ctr	Ctr	0.36
J	LOCK	4.0x 5.0	Ctr	-0.5	0.71
W	LOCK	8.0x12.0	Ctr	-1.5	0.99
I	LOCK	5.0x 8.0	Ctr	Ctr	0.79
Y	LOCK	4.0x 5.0	Ctr	-0.5	0.68
F	LOCK	10.0x10.0	Ctr	-2.2	0.75
H	LOCK	3.0x 4.0	Ctr	Ctr	0.36

= Plate Monitor used

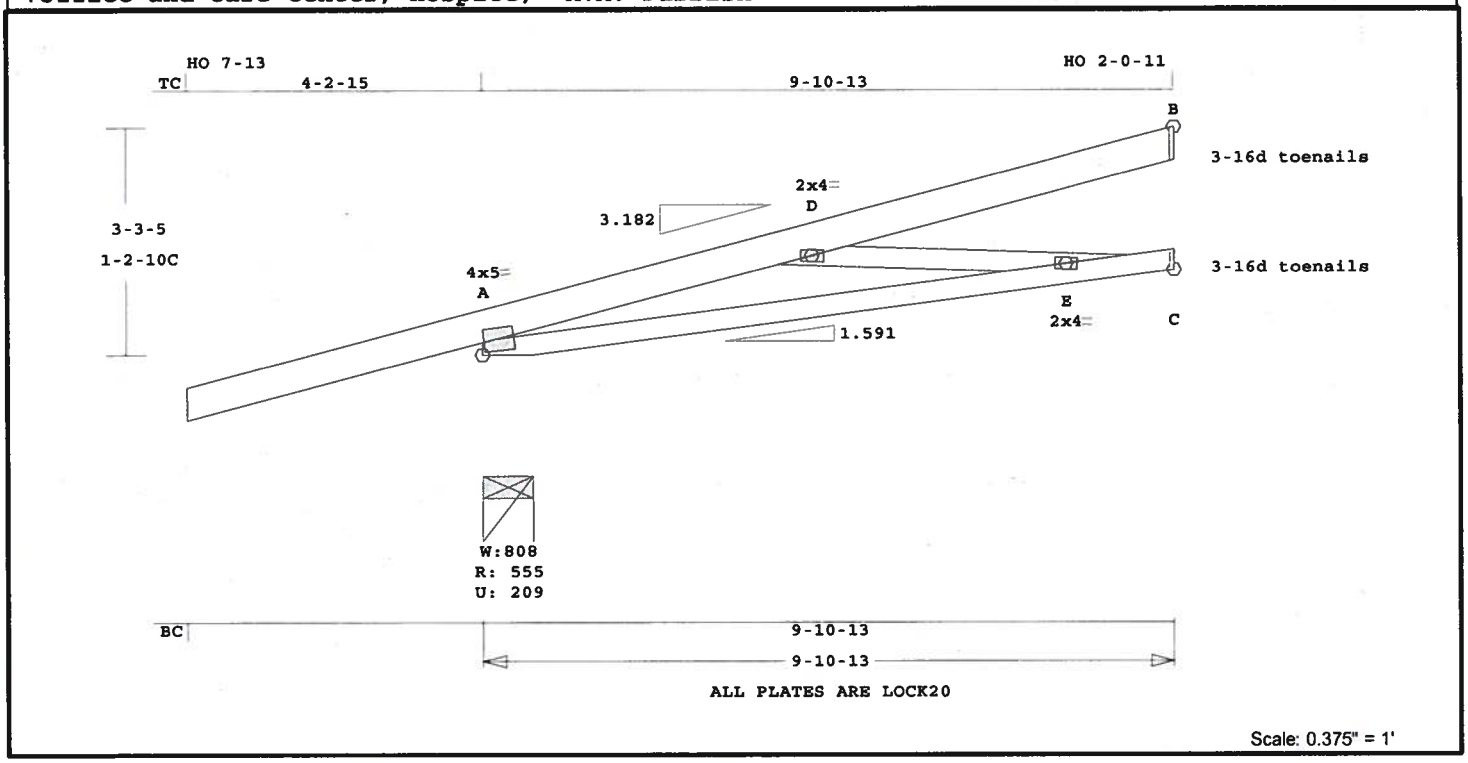
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	8	8	

Web Connection Exception --
Use 4" spacing for screws or nails on the following webs
C- W X- I
No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R145*	2	VKING	91013	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 70.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.36	2x 6	SP-#2
BC	0.77	2x 4	SP-#2
WB	0.35	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9-10-13
BC	Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
TC V	-40	-20	0.0'	
	45	22		9.9'
BC V	0	-20	0.0'	
	0	22		9.9'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	556	209	8- 8	1- 8
			Hz =	125
C	314	0	3- 8	1- 8
			Hz =	68
B	323	211	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.27	869 C	0.05	0.22
D -B	0.36	113 C	0.00	0.36
--------Bottom Chords---------				
A -E	0.77	844 T	0.06	0.71
E -C	0.24	72 T	0.00	0.24
------------Webs------------				
D -E	0.35	838 C		

TL Defl -0.53" in A -C L/200
LL Defl -0.26" in A -C L/416

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.01"

Shear // Grain in C -C 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.7	0.3	0.83
D	LOCK	2.0x 4.0	Ctr	Ctr	0.47
E	LOCK	2.0x 4.0	Ctr	Ctr	0.56

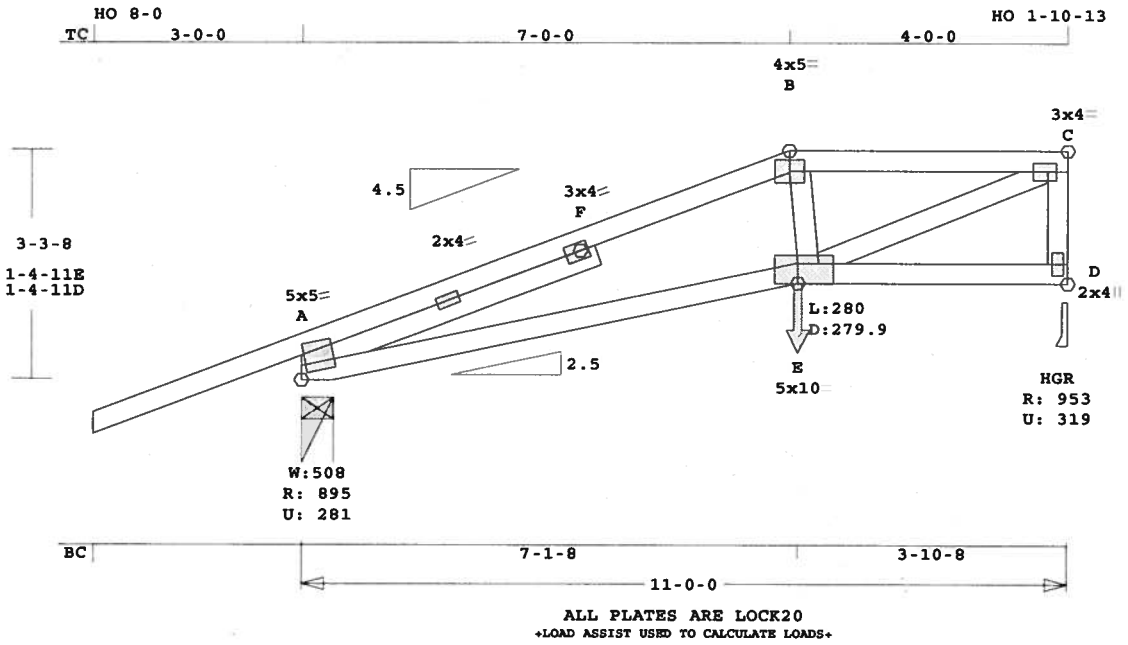
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 869 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R144*	2	SP	110000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
WB	0.64	2x 4	SP-#3
SL	0.21	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 0- 0
BC Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	11.0'
BC V	0	20	0.0'	11.0'
TC V	50	25	7.0'	10.0'
TC V	-40	-20	10.0'	11.0'
BC V	0	25	7.1'	10.0'
BC V	0	-20	10.0'	11.0'
BC V	280	280	7.1'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd		In-Sx	In-Sx
A	896	281			5- 8	1- 8
				Hz =	121	
D	953	320			3- 8	1- 8
				Hz =	66	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -F	0.21	1114 C	0.08	0.13
F -B	0.36	1913 C	0.19	0.17
B -C	0.45	1853 C	0.18	0.27
--------Bottom Chords---------				
A -E	0.52	1832 T	0.33	0.19
E -D	0.15	66 T	0.00	0.15
------------Webs------------				
B -E	0.10	400 T		
E -C	0.64	2009 T		
D -C	0.16	913 C		
----------Sliders----------				
A -F	0.21	873 C		

TL Defl -0.11" in A -E L/999
LL Defl -0.05" in A -E L/999

Hz Disp	LL	DL	TL
Jt D	0.02"	0.02"	0.04"

Shear // Grain in B -C 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

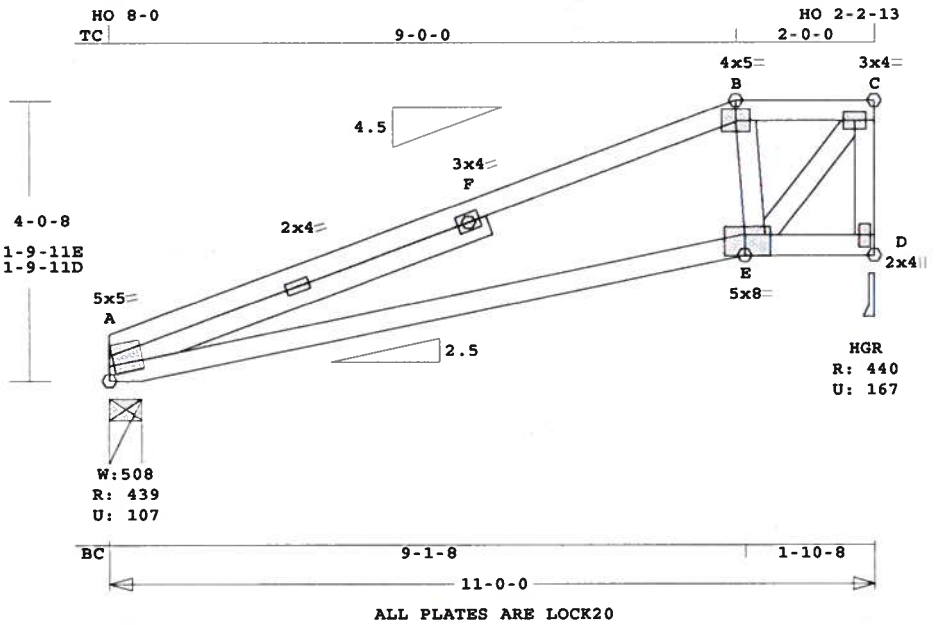
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	0.97
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.69
C	LOCK	3.0x 4.0-0.5	Ctr		0.97
E	LOCK	5.0x10.0	1.0-1.0		0.95
D	LOCK	2.0x 4.0	Ctr	Ctr	0.55

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1913 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R143*	2	SP	110000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.63	2x 4	SP-#2
BC	0.43	2x 4	SP-#2
WB	0.22	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 0- 0
BC	Cont.	0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	440	107	5- 8	1- 0
			Hz =	152
D	440	168	3- 8	1- 0
			Hz =	83

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -F	0.63	801 C	0.00 0.63
F -B	0.61	480 C	0.00 0.61
B -C	0.36	459 C	0.00 0.36
--------Bottom Chords---------			
A -E	0.43	510 T	0.00 0.43
E -D	0.16	82 T	0.00 0.16
------------Webs------------			
B -E	0.09	381 T	
E -C	0.22	714 T	
D -C	0.09	476 C	
----------Sliders----------			
A -F	0.11	416 T	

LL Defl -0.12" in A -E L/999
TL Defl -0.28" in A -E L/443
Shear // Grain in A -F 0.31

Hz Disp	LL	DL	TL
Jt D	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

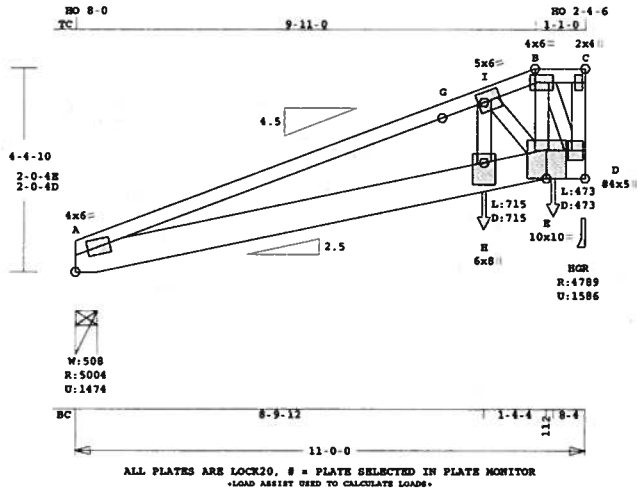
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	3.0	0.8	0.97
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 5.0	Ctr	Ctr	0.69
C	LOCK	3.0x 4.0	Ctr	Ctr	0.55
E	LOCK	5.0x 8.0	0.5-1.0		0.89
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 801 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R142*	2*2P	SP	110000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



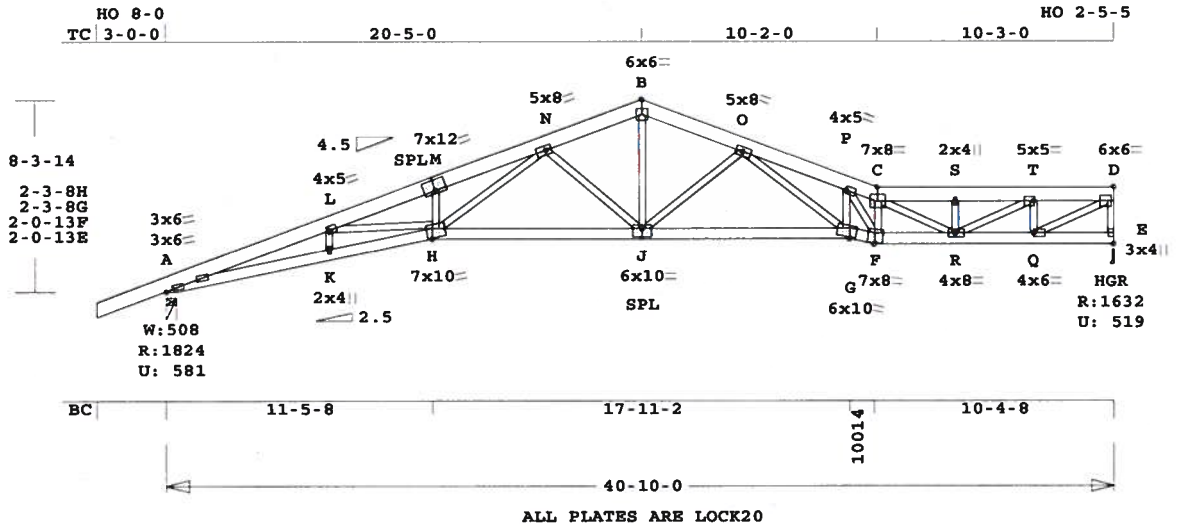
Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 78.9 LBS

```
Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.66  2x 4  SP-2400
EX B -C   2x 4  SP-#2
BC  0.90  2x 8  SP-2400
WB  0.59  2x 4  SP-#3
EX H -I   2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 11- 0- 0
 BC Cont.      0- 0- 0 11- 0- 0

Loading   Live   Dead  (psf)
TC        20.0   10.0
BC         0.0   10.0
Total     20.0   20.0   40.0
Spacing                 24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.00  Fc=1.00  Ft=1.00
BC Fb=1.00  Fc=1.00  Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor   1.25
Plate  Duration Factor   1.25
plf -  Live  Dead  From    To
TC V    40    20   0.0'  11.0'
BC V     0    20   0.0'  11.0'
BC V   418   418   0.0'   7.8'
BC V   715   715   8.8'  CL-LB
BC V   473   473  10.3'  CL-LB


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A    5004  1474  5- 8  2- 1
                      Hz =   934
D    4789  1586  3- 8  2- 0
                      Hz =   535

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -I  0.66  7092 C  0.20  0.46
I -B  0.21  1717 C  0.00  0.21
B -C  0.16     0 T  0.00  0.16
--------Bottom Chords---------
A -H  0.90  6825 T  0.13  0.77
H -E  0.61  6367 T  0.12  0.49
E -D  0.16  1189 T  0.02  0.14
------------Webs-------------
H -I  0.61  6784 T
I -E  0.58  7227 C
E -B  0.59  3681 T
B -D  0.24  3055 C
D -C  0.02   284 C

LL Defl -0.18" in A -H  L/681
TL Defl -0.36" in A -H  L/337
Shear // Grain in A -H   0.86
Hz Disp   LL      DL      TL
  Jt D   0.02"    0.02"   0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  4.0x 6.0 Ctr Ctr 0.97
I  LOCK  5.0x 6.0-0.5-0.2 0.93
B  LOCK  4.0x 6.0 1.0 Ctr 0.85
C  LOCK  2.0x 4.0 Ctr Ctr 0.38
H  LOCK  6.0x 8.0 Ctr-1.6 0.94
E  LOCK 10.0x10.0 Ctr-2.3 0.63
D  LOCK  4.0x 5.0 0.9 Ctr 0.57


NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004

2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3   screws -OR- 10d nails
  as each layer is applied.)
          ----Spacing (In)----
    Rows  Nails  Screws  Bolts
TC  1      12      24       0
BC  2     7.5      20       0
WB  1      4       4
Plus clusters of nails where
  shown.
Provide connection to bearing
  for   933 Lbs Horiz Reaction
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
  bearing locations.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:     B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  TC Dead Load:         4.0 psf
  BC Dead Load:         6.0 psf
Max comp. force     7227 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
    Designer)
    Gary Dounson, PE 35054
    Gary Dounson & Associates,
    Inc.
    2830 NW 41st Street Suite D
    Gainesville, FL 32606
    (352)375-8593
    CA 5201
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R141*	2	SP	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 392.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 8	SP-2400
BC	0.85	2x 6	SP-#2
EX A -H		2x 6	SP-2400
WB	0.93	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	40-10- 0
BC Cont.		0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1825	581	5- 8	1- 8
			Hz =	174
E	1633	519	3- 8	1-15
			Hz =	114

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.22	6380 C	0.04		0.18
L -M	0.24	6591 C	0.05		0.19
M -N	0.14	6577 C	0.05		0.09
N -B	0.06	3267 C	0.04		0.02
B -O	0.06	3262 C	0.04		0.02
O -P	0.14	6464 C	0.04		0.10
P -C	0.15	6963 C	0.05		0.10
C -S	0.06	4739 C	0.05		0.01
S -T	0.06	4739 C	0.05		0.01
T -D	0.03	2546 C	0.00		0.03
Bottom Chords					
A -K	0.48	6022 T	0.27		0.21
K -H	0.48	6072 T	0.27		0.21
H -J	0.69	4022 T	0.48		0.21
J -G	0.70	4086 T	0.49		0.21
G -F	0.82	6259 T	0.76		0.06
F -R	0.85	6681 T	0.81		0.04
R -Q	0.36	2546 T	0.30		0.06
Q -E	0.04	113 T	0.00		0.04
Webs					
K -L	0.04	172 T			
L -H	0.08	278 T			
H -M	0.05	244 C			
H -N	0.88	2757 T			
N -J	0.60	1335 C			
J -B	0.66	2067 T			
J -O	0.64	1375 C			
O -G	0.83	2601 T			
G -P	0.03	172 C			
P -F	0.20	625 T			
F -C	0.30	1899 C			
C -R	0.50	2159 C			
R -S	0.04	261 C			
R -T	0.80	2522 T			
Q -T	0.22	1381 C			
Q -D	0.93	2927 T			
E -D	0.25	1586 C			

TL Defl -0.94" in H -J L/510
LL Defl -0.45" in H -J L/999

Hz Disp	LL	DL	TL
Jt E	0.18"	0.18"	0.36"

Shear // Grain in P -C 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

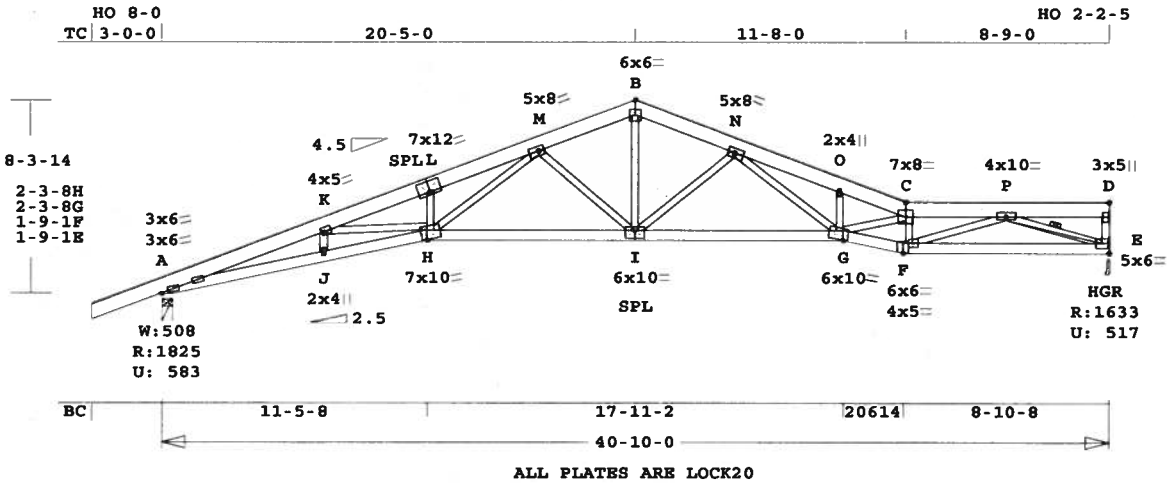
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	Ctr	Ctr	0.78
A	LOCK	3.0x 6.0	Ctr	Ctr	0.56
L	LOCK	4.0x 5.0-0.5	0.2		0.62
M	LOCK	7.0x12.0-0.7	2.0		0.97
N	LOCK	5.0x 8.0-1.1-0.4			0.98
B	LOCK	6.0x 6.0	Ctr	Ctr	0.77
O	LOCK	5.0x 8.0	0.4-0.2		0.96
P	LOCK	4.0x 5.0	0.5	0.2	0.58
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
S	LOCK	2.0x 4.0	Ctr	Ctr	0.48
T	LOCK	5.0x 5.0-0.5	Ctr		0.95
D	LOCK	6.0x 6.0-1.0	Ctr		0.97
K	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	7.0x10.0	0.2-1.2		0.95
J	LOCK	6.0x10.0	Ctr-1.2		0.97
G	LOCK	6.0x10.0-1.4-1.6			0.94
F	LOCK	7.0x 8.0	0.4	3.9	0.81
R	LOCK	4.0x 8.0	0.5	Ctr	1.00
Q	LOCK	4.0x 6.0	1.0	Ctr	0.97
E	LOCK	3.0x 4.0	Ctr	Ctr	0.92

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6963 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R140*	2	SP	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



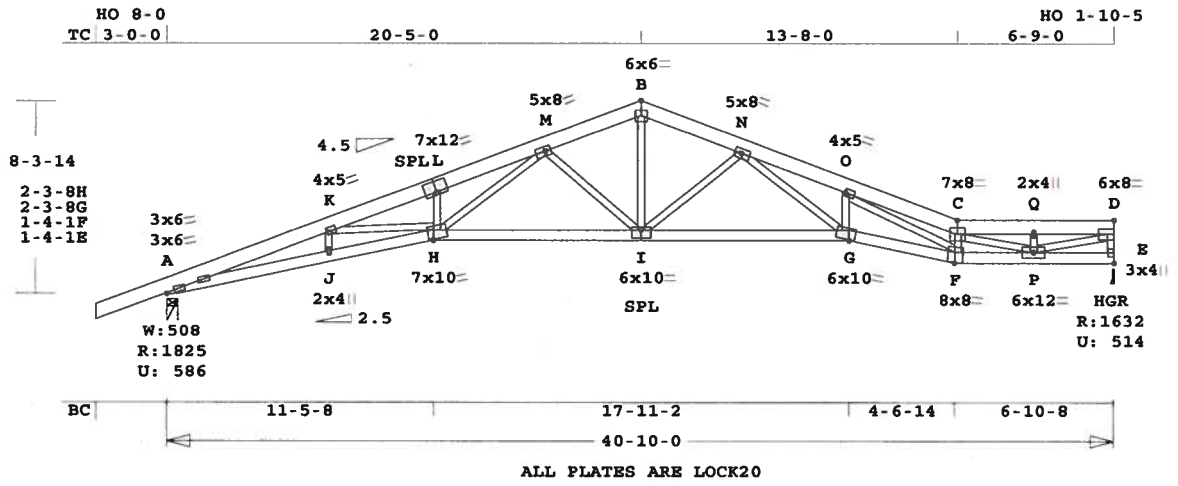
Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 385.3 LBS

```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

     CSI  -Size-  ----Lumber----
TC  0.24  2x 8  SP-2400
BC  0.74  2x 6  SP-#2
EX A -H   2x 6  SP-2400
EX G -F   2x 6  SP-2400
WB  0.88  2x 4  SP-#3
EX F -P   2x 4  SP-#2

Brace truss as follows:
      O.C.      From        To
 TC Cont.      0- 0- 0 40-10- 0
 BC Cont.      0- 0- 0 40-10- 0
 WB 1 rows CLB on P -E
Attach CLB with (2)-10d nails
  at each web.

Loading    Live    Dead   (psf)
TC         20.0    10.0
BC          0.0    10.0
Total      20.0    20.0    40.0
Spacing                    24.0"
Lumber Duration Factor     1.25
Plate  Duration Factor     1.25
TC Fb=1.15   Fc=1.10   Ft=1.10
BC Fb=1.10   Fc=1.10   Ft=1.10


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A    1825   584  5- 8  1- 8
                 Hz =   149
E    1633   517  3- 8  1-15
                 Hz =   118

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -K  0.22  6384 C  0.04  0.18
K -L  0.24  6590 C  0.05  0.19
L -M  0.14  6576 C  0.05  0.09
M -B  0.06  3276 C  0.04  0.02
B -N  0.06  3275 C  0.04  0.02
N -O  0.11  6556 C  0.04  0.07
O -C  0.12  6594 C  0.04  0.08
C -P  0.10  6438 C  0.04  0.06
P -D  0.06     0 T  0.00  0.06
--------Bottom Chords---------
A -J  0.47  6026 T  0.27  0.20
J -H  0.47  6077 T  0.27  0.20
H -I  0.73  4027 T  0.48  0.25
I -G  0.74  4085 T  0.49  0.25
G -F  0.41  6790 T  0.31  0.10
F -E  0.70  3401 T  0.41  0.29
------------Webs-------------
J -K  0.04   170 T
K -H  0.08   273 T
H -L  0.05   245 C
H -M  0.88  2748 T
M -I  0.60  1331 C
I -B  0.66  2084 T
I -N  0.64  1362 C
N -G  0.85  2679 T
G -O  0.03   131 T
G -C  0.09   478 C
F -C  0.40  2538 C
F -P  0.60  3263 T
P -E  0.64  3655 C        1 Br
E -D  0.02   181 C

TL Defl   -0.98" in I -G  L/492
LL Defl   -0.47" in I -G  L/999
Hz Disp    LL      DL      TL
  Jt E   0.19"   0.19"   0.38"
Shear // Grain  in A -K   0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  3.0x 6.0 Ctr Ctr 0.78
A  LOCK  3.0x 6.0 Ctr Ctr 0.56
K  LOCK  4.0x 5.0-0.5 0.2 0.62
L  LOCK  7.0x12.0-0.7 2.0 0.97
M  LOCK  5.0x 8.0-1.1-0.4 0.97
B  LOCK  6.0x 6.0 Ctr Ctr 0.77
N  LOCK  5.0x 8.0 0.4-0.2 0.99
O  LOCK  2.0x 4.0 Ctr Ctr 0.48
C  LOCK  7.0x 8.0 Ctr Ctr 0.85
P  LOCK  4.0x10.0 Ctr 0.5 0.98
D  LOCK  3.0x 5.0 Ctr Ctr 0.99
J  LOCK  2.0x 4.0 Ctr Ctr 0.48
H  LOCK  7.0x10.0 0.2-1.2 0.95
I  LOCK  6.0x10.0 Ctr-1.2 0.97
G  LOCK  6.0x10.0-0.3-1.6 0.96
F  LOCK  6.0x 6.0 Ctr 0.9 0.83
F  LOCK  4.0x 5.0 0.5 Ctr 0.98
E  LOCK  5.0x 6.0 Ctr Ctr 0.91

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:    B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  TC Dead Load:        4.0 psf
  BC Dead Load:        6.0 psf
Max comp. force     6594 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
    Designer)
    Gary Dounson, PE 35054
    Gary Dounson & Associates,
    Inc.
    2830 NW 41st Street Suite D
    Gainesville, FL 32606
    (352)375-8593
    CA 5201
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R139	2	SP	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 386.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 8	SP-2400
BC	1.00	2x 6	SP-#2
EX	A -H	2x 6	SP-2400
WB	0.88	2x 4	SP-#3
EX	P -D	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1825	586	5- 8	1- 8
			Hz =	115
E	1633	514	3- 8	1-15
			Hz =	123

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -K	0.22	6380 C	0.04 0.18
K -L	0.24	6590 C	0.05 0.19
L -M	0.14	6576 C	0.05 0.09
M -B	0.06	3267 C	0.04 0.02
B -N	0.06	3260 C	0.04 0.02
N -O	0.11	6549 C	0.04 0.07
O -C	0.13	7251 C	0.06 0.07
C -Q	0.16	4250 C	0.01 0.15
Q -D	0.16	4250 C	0.01 0.15
--------Bottom Chords---------			
A -J	0.47	6021 T	0.27 0.20
J -H	0.47	6072 T	0.27 0.20
H -I	0.69	4021 T	0.48 0.21
I -G	0.70	4080 T	0.49 0.21
G -F	1.00	6318 T	0.76 0.24
F -P	1.00	7212 T	0.87 0.13
P -E	0.13	122 T	0.00 0.13
------------Webs------------			
J -K	0.04	170 T	
K -H	0.08	278 T	
H -L	0.05	244 C	
H -M	0.88	2756 T	
M -I	0.60	1335 C	
I -B	0.66	2062 T	
I -N	0.64	1367 C	
N -G	0.86	2696 T	
G -O	0.05	250 C	
O -F	0.22	691 T	
F -C	0.27	1728 C	
C -P	0.66	3090 C	
P -Q	0.10	686 C	
P -D	0.84	4581 T	
E -D	0.25	1639 C	

TL Defl -1.03" in H -I L/465
LL Defl -0.50" in I -G L/964

Hz Disp	LL	DL	TL
Jt E	0.20"	0.20"	0.40"

Shear // Grain in C -Q 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

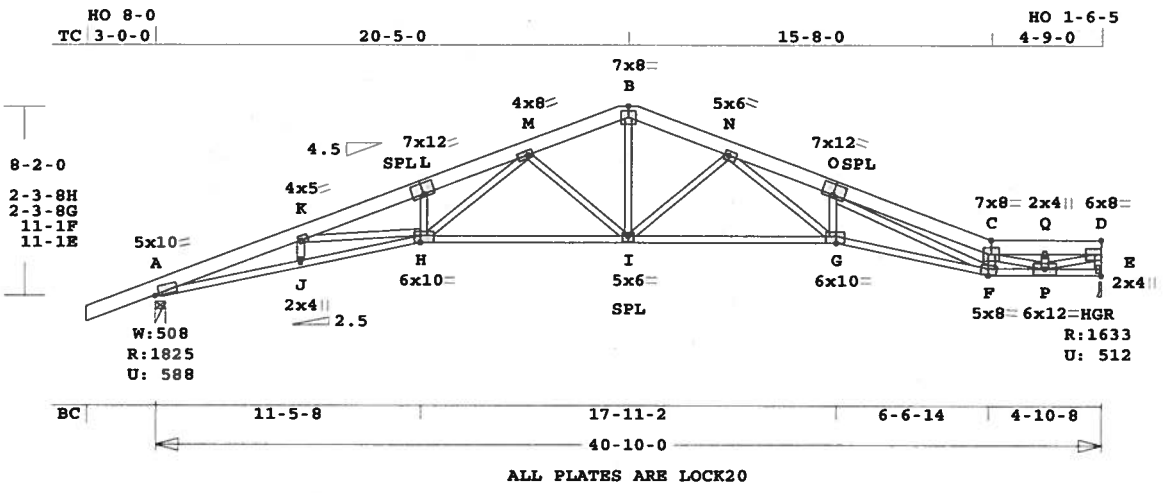
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	Ctr	Ctr	0.78
A	LOCK	3.0x 6.0	Ctr	Ctr	0.56
K	LOCK	4.0x 5.0-0.5	0.2		0.62
L	LOCK	7.0x12.0-0.7	2.0		0.97
M	LOCK	5.0x 8.0-1.1-0.4			0.98
B	LOCK	6.0x 6.0	Ctr	Ctr	0.77
N	LOCK	5.0x 8.0	0.4-0.2		1.00
O	LOCK	4.0x 5.0	0.5	0.2	0.93
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.48
D	LOCK	6.0x 8.0-0.5	Ctr		1.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	7.0x10.0	0.2-1.2		0.95
I	LOCK	6.0x10.0	Ctr-1.2		0.97
G	LOCK	6.0x10.0-1.4-1.6			0.98
F	LOCK	8.0x 8.0	0.5	4.4	0.88
P	LOCK	6.0x12.0	Ctr	Ctr	1.00
E	LOCK	3.0x 4.0	Ctr	Ctr	0.92

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 7251 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R138*	2	SP	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 349.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.32	2x 8	SP-2400
BC	0.61	2x 4	SP-2400
EX H -I		2x 4	SP-#2
EX I -G		2x 4	SP-#2
WB	0.86	2x 4	SP-#3
EX P -D		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10		Ft=1.10
BC Fb=1.10	Fc=1.10		Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	1825	588	5- 8	1- 8
			Hz = -118	
E	1633	513	3- 8	1- 8
			Hz = 128	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.30	5931	C	0.03	0.27
K -L	0.32	6384	C	0.04	0.28
L -M	0.09	6427	C	0.04	0.05
M -B	0.06	3195	C	0.04	0.02
B -N	0.06	3192	C	0.04	0.02
N -O	0.11	6298	C	0.04	0.07
O -C	0.17	6557	C	0.05	0.12
C -Q	0.19	3965	C	0.01	0.18
Q -D	0.19	3965	C	0.01	0.18
Bottom Chords					
A -J	0.57	5535	T	0.39	0.18
J -H	0.55	5558	T	0.39	0.16
H -I	0.96	3975	T	0.66	0.30
I -G	0.96	3975	T	0.66	0.30
G -F	0.61	6057	T	0.43	0.18
F -P	0.54	6689	T	0.48	0.06
P -E	0.05	127	T	0.00	0.05
Webs					
J -K	0.03	123	T		
K -H	0.19	601	T		
H -L	0.07	426	C		
H -M	0.86	2682	T		
M -I	0.64	1315	C		
I -B	0.62	1953	T		
I -N	0.64	1315	C		
N -G	0.82	2563	T		
G -O	0.06	273	C		
O -F	0.09	225	T		
F -C	0.21	1385	C		
C -P	0.49	2858	C		
P -Q	0.14	929	C		
P -D	0.80	4378	T		
E -D	0.25	1627	C		

TL Defl -1.21" in H -I L/398
LL Defl -0.55" in H -I L/875

Hz Disp	LL	DL	TL
Jt E	0.27"	0.27"	0.55"

Shear // Grain in C -Q 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

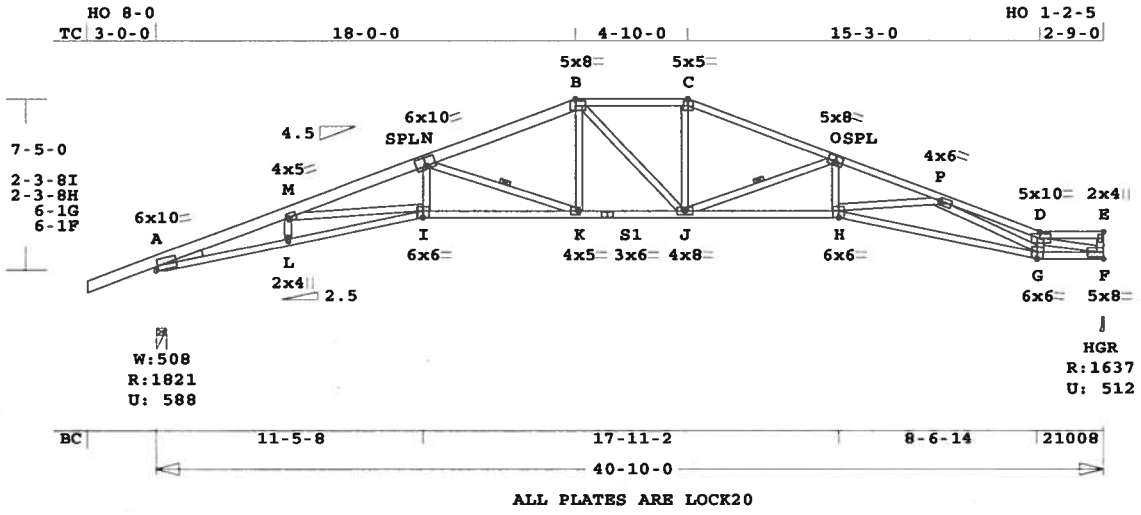
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0 Ctr	0.8	0.94	
K	LOCK	4.0x 5.0-0.5	0.2	0.63	
L	LOCK	7.0x12.0-0.7	2.0	0.97	
M	LOCK	4.0x 8.0-1.3-0.1		1.00	
B	LOCK	7.0x 8.0 Ctr	Ctr	0.64	
N	LOCK	5.0x 6.0	0.9-0.4		0.96
O	LOCK	7.0x12.0	0.7	2.0	0.97
C	LOCK	7.0x 8.0 Ctr	Ctr		0.85
Q	LOCK	2.0x 4.0 Ctr	Ctr		0.48
D	LOCK	6.0x 8.0-0.5	Ctr		0.96
J	LOCK	2.0x 4.0 Ctr	Ctr		0.48
H	LOCK	6.0x10.0 Ctr-0.5			0.87
I	LOCK	5.0x 6.0 Ctr-0.5			0.88
G	LOCK	6.0x10.0-0.5-0.5			0.96
F	LOCK	5.0x 8.0	0.3	2.9	0.95
P	LOCK	6.0x12.0	Ctr	Ctr	0.96
E	LOCK	2.0x 4.0 Ctr	Ctr		0.56

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6557 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R137*	2	SP	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 290.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.97	2x 4	SP-#2
EX A -N		2x 6	SP-2400
EX N -B		2x 6	SP-#2
BC	0.93	2x 4	SP-2400
WB	0.87	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0
WB	1 rows CLB on N -K		
WB	1 rows CLB on J -O		

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1822	588	5- 8	1- 8
			Hz =	-110
F	1637	513	3- 8	1- 8
			Hz =	117

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.76	5875	C	0.06	0.70
M -N	0.43	6268	C	0.09	0.34
N -B	0.31	3672	C	0.15	0.16
B -C	0.31	3394	C	0.20	0.11
C -O	0.65	3622	C	0.22	0.43
O -P	0.63	6021	C	0.36	0.27
P -D	0.64	4809	C	0.16	0.48
D -E	0.97	0	T	0.00	0.97
Bottom Chords					
A -L	0.93	5463	T	0.39	0.54
L -I	0.63	5489	T	0.39	0.24
I -K	0.57	5866	T	0.42	0.15
K -S1	0.27	3413	T	0.24	0.03
S1-J	0.28	3413	T	0.24	0.04
J -H	0.50	5602	T	0.40	0.10
H -G	0.59	5733	T	0.41	0.18
G -F	0.53	4658	T	0.33	0.20
Webs					
L -M	0.03	138	T		
M -I	0.17	540	T		
I -N	0.41	1289	T		
N -K	0.59	2583	C		1 Br
K -B	0.30	960	T		
B -J	0.07	130	C		
J -C	0.30	944	T		
J -O	0.54	2356	C		1 Br
H -O	0.42	1319	T		
H -P	0.03	137	T		
P -G	0.51	1287	C		
G -D	0.08	533	C		
D -F	0.87	4820	C		
F -E	0.04	305	C		

TL Defl	-1.38"	in S1-J	L/349
LL Defl	-0.69"	in S1-J	L/700

Hz Disp	LL	DL	TL
Jt F	0.36"	0.36"	0.72"

Shear // Grain in A -M 0.40

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

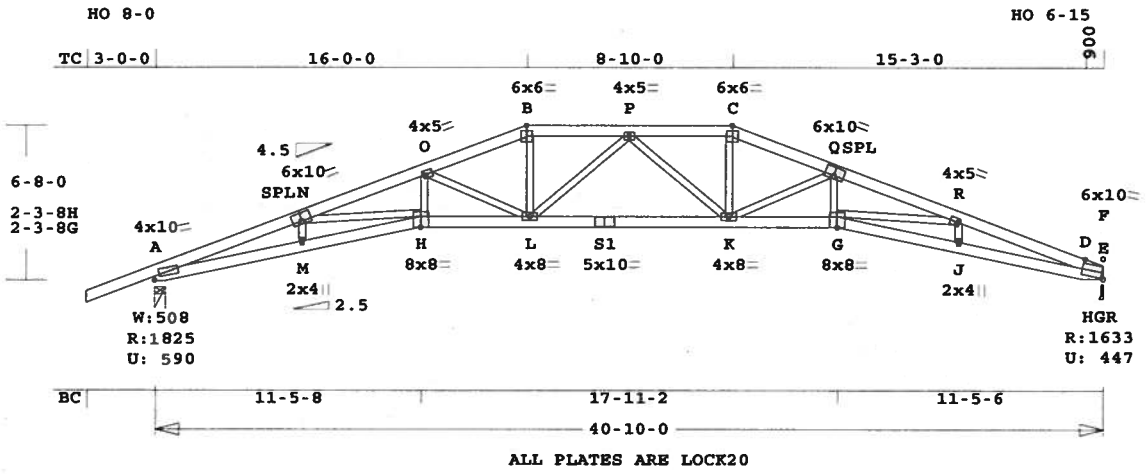
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
M	LOCK	4.0x 5.0	Ctr	Ctr	0.64
N	LOCK	6.0x10.0	-0.5	1.2	0.95
B	LOCK	5.0x 8.0	-0.5	Ctr	0.97
C	LOCK	5.0x 5.0	Ctr	Ctr	0.96
O	LOCK	5.0x 8.0	0.2	0.5	0.89
P	LOCK	4.0x 6.0	-0.9	0.4	0.91
D	LOCK	5.0x10.0	0.5	Ctr	0.98
E	LOCK	2.0x 4.0	Ctr	Ctr	0.56
L	LOCK	2.0x 4.0	Ctr	Ctr	0.48
I	LOCK	6.0x 6.0	Ctr	-0.5	0.93
K	LOCK	4.0x 5.0	Ctr	Ctr	0.85
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
J	LOCK	4.0x 8.0	Ctr	Ctr	0.86
H	LOCK	6.0x 6.0	Ctr	-0.5	0.96
G	LOCK	6.0x 6.0	0.3	3.3	0.98
F	LOCK	5.0x 8.0	-0.5	Ctr	0.89

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6268 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R136	2	SP	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 329.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 6	SP-#2
BC	0.49	2x 6	SP-2400
EX	H -S1	2x 6	SP-#2
EX	S1-G	2x 6	SP-#2
WB	0.78	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1825	590	5- 8	1- 8
			Hz =	-106
F	1633	447	3- 8	1- 8
			Hz =	107

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -N	0.50	6250	C	0.11	0.39
N -O	0.52	6374	C	0.14	0.38
O -B	0.25	4288	C	0.17	0.08
B -P	0.20	4028	C	0.16	0.04
P -C	0.20	4028	C	0.16	0.04
C -Q	0.24	4288	C	0.17	0.07
Q -R	0.53	6377	C	0.14	0.39
R -F	0.49	6256	C	0.11	0.38
Bottom Chords					
A -M	0.48	5882	T	0.26	0.22
M -H	0.49	5939	T	0.27	0.22
H -L	0.78	5932	T	0.72	0.06
L -S1	0.59	4175	T	0.50	0.09
S1-K	0.61	4175	T	0.50	0.11
K -G	0.79	5935	T	0.72	0.07
G -J	0.48	5943	T	0.27	0.21
J -F	0.48	5887	T	0.27	0.21
Webs					
M -N	0.04	187	T		
N -H	0.05	183	T		
H -O	0.44	1386	T		
O -L	0.78	2123	C		
L -B	0.38	1204	T		
L -P	0.09	198	C		
P -K	0.09	198	C		
K -C	0.38	1205	T		
K -Q	0.78	2127	C		
G -Q	0.44	1387	T		
G -R	0.05	182	T		
J -R	0.04	188	T		

TL Defl -1.12" in S1-K L/427
LL Defl -0.55" in S1-K L/878

Hz Disp	LL	DL	TL
Jt F	0.31"	0.32"	0.63"

Shear // Grain in A -N 0.24

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

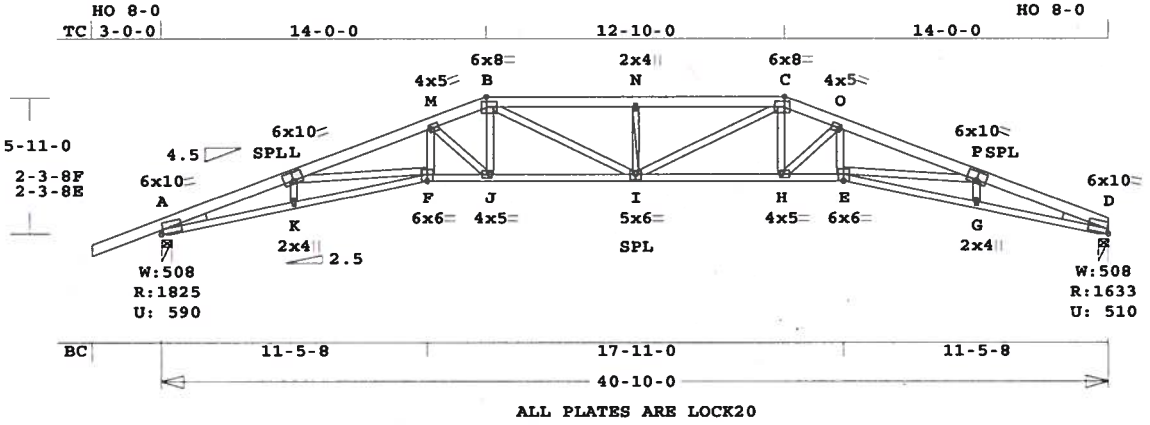
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x10.0	Ctr	Ctr	0.99
N	LOCK	6.0x10.0-0.5	1.2		0.95
O	LOCK	4.0x 5.0	Ctr	Ctr	0.70
B	LOCK	6.0x 6.0	Ctr	Ctr	0.90
P	LOCK	4.0x 5.0	Ctr	Ctr	0.48
C	LOCK	6.0x 6.0	Ctr	Ctr	0.90
Q	LOCK	6.0x10.0	0.5	1.2	0.95
R	LOCK	4.0x 5.0	Ctr	Ctr	0.64
F	LOCK	6.0x10.0-5.5	1.2		0.94
M	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	8.0x 8.0	Ctr-1.5		0.72
L	LOCK	4.0x 8.0	Ctr	Ctr	0.77
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.77
G	LOCK	8.0x 8.0	Ctr-1.5		0.72
J	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6377 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R135*	2	HPSC4	401000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 301.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.80	2x 6	SP-#2
EX A -L		2x 6	SP-2400
EX P -D		2x 6	SP-2400
BC	0.95	2x 4	SP-2400
WB	0.41	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40-10- 0
BC	Cont.	0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1825	590	5- 8	1- 8
			Hz =	-95
D	1634	511	5- 8	1- 8
			Hz =	96

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -L	0.76	5929 C	0.06	0.70
L -M	0.80	6262 C	0.14	0.66
M -B	0.24	4924 C	0.19	0.05
B -N	0.29	5188 C	0.09	0.20
N -C	0.29	5188 C	0.09	0.20
C -O	0.24	4924 C	0.19	0.05
O -P	0.80	6262 C	0.14	0.66
P -D	0.76	5928 C	0.06	0.70
Bottom Chords				
A -K	0.95	5517 T	0.39	0.56
K -F	0.63	5545 T	0.39	0.24
F -J	0.59	5853 T	0.42	0.17
J -I	0.40	4584 T	0.32	0.08
I -H	0.40	4584 T	0.32	0.08
H -E	0.59	5853 T	0.42	0.17
E -G	0.63	5545 T	0.39	0.24
G -D	0.95	5517 T	0.39	0.56
Webs				
K -L	0.03	139 T		
L -F	0.15	473 T		
F -M	0.41	1305 T		
M -J	0.35	1664 C		
J -B	0.36	1143 T		
B -I	0.21	670 T		
I -N	0.08	437 C		
I -C	0.21	670 T		
H -C	0.36	1143 T		
H -O	0.35	1664 C		
E -O	0.41	1305 T		
E -P	0.15	474 T		
G -P	0.03	139 T		

TL Defl -1.50" in I -H L/319
LL Defl -0.73" in I -H L/652

Hz Disp	LL	DL	TL
Jt D	0.43"	0.43"	0.86"

Shear // Grain in A -L 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

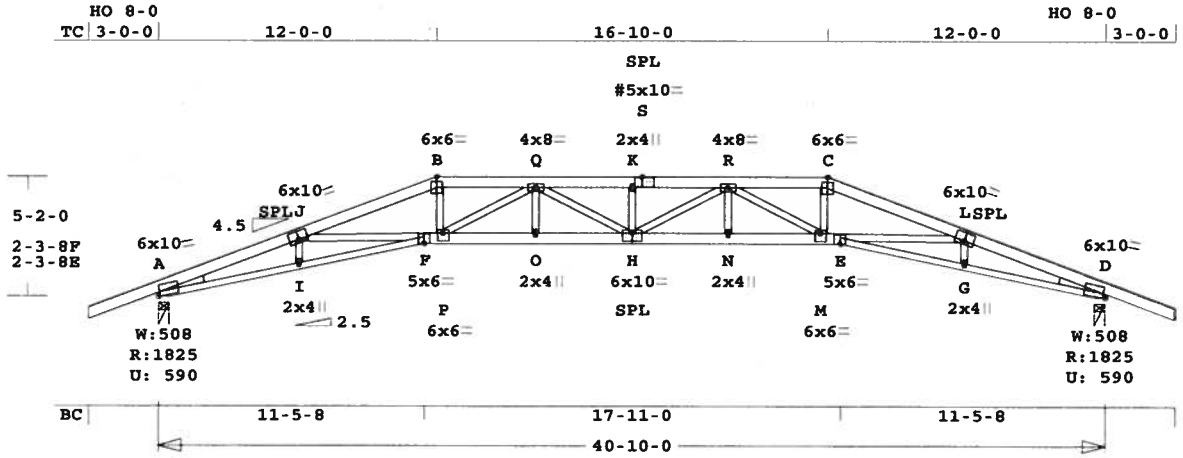
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
L	LOCK	6.0x10.0-0.5	1.2		0.95
M	LOCK	4.0x 5.0 Ctr	Ctr		0.65
B	LOCK	6.0x 8.0-0.5	Ctr		0.90
N	LOCK	2.0x 4.0 Ctr	Ctr		0.48
C	LOCK	6.0x 8.0 0.5	Ctr		0.90
O	LOCK	4.0x 5.0 Ctr	Ctr		0.65
P	LOCK	6.0x10.0 0.5	1.2		0.95
D	LOCK	6.0x10.0-4.6	1.5		0.91
K	LOCK	2.0x 4.0 Ctr	Ctr		0.48
F	LOCK	6.0x 6.0 Ctr-0.5			0.94
J	LOCK	4.0x 5.0 Ctr	Ctr		0.60
I	LOCK	5.0x 6.0 Ctr-0.5			0.88
H	LOCK	4.0x 5.0 Ctr	Ctr		0.60
E	LOCK	6.0x 6.0 Ctr-0.5			0.94
G	LOCK	2.0x 4.0 Ctr	Ctr		0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6262 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R134*	2	HPSC4	401000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 316.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.88	2x 6	SP-#2
EX A -J		2x 6	SP-2400
EX L -D		2x 6	SP-2400
BC	0.96	2x 4	SP-2400
EX F -H		2x 6	SP-2400
EX H -E		2x 6	SP-2400
WB	0.56	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	40-10- 0
BC Cont.		0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1826	591	5- 8	1- 8
		Hz =	-82	
D	1825	590	5- 8	1- 8
		Hz =	83	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -J	0.75	5879 C	0.06		0.69
J -B	0.88	6352 C	0.15		0.73
B -Q	0.26	5998 C	0.23		0.03
Q -K	0.29	6835 C	0.15		0.14
K -S	0.26	6835 C	0.25		0.01
S -R	0.29	6836 C	0.15		0.14
R -C	0.26	5997 C	0.23		0.03
C -L	0.88	6351 C	0.15		0.73
L -D	0.75	5877 C	0.06		0.69
Bottom Chords					
A -I	0.96	5478 T	0.39		0.57
I -F	0.66	5516 T	0.39		0.27
F -P	0.55	6002 T	0.27		0.28
P -O	0.57	6502 T	0.29		0.28
O -H	0.38	6502 T	0.29		0.09
H -N	0.38	6501 T	0.29		0.09
N -M	0.57	6501 T	0.29		0.28
M -E	0.55	6001 T	0.27		0.28
E -G	0.66	5514 T	0.39		0.27
G -D	0.96	5476 T	0.39		0.57
Webs					
I -J	0.03	158 T			
J -F	0.19	615 T			
P -B	0.56	1769 T			
P -Q	0.19	578 C			
O -Q	0.03	146 C			
Q -H	0.12	390 T			
H -K	0.04	238 C			
H -R	0.12	391 T			
N -R	0.03	146 C			
R -M	0.19	579 C			
M -C	0.56	1769 T			
E -L	0.19	615 T			
G -L	0.03	158 T			

TL Defl -1.66" in O -H L/288
LL Defl -0.82" in O -H L/580

Hz Disp	LL	DL	TL
Jt D	0.45"	0.45"	0.90"

Shear // Grain in F -P 0.95

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	6.0x10.0	4.6	1.5	0.91
J	LOCK	6.0x10.0	-0.5	1.2	0.95
B	LOCK	6.0x 6.0	Ctr	Ctr	0.90
Q	LOCK	4.0x 8.0	Ctr	Ctr	0.54
K	LOCK	2.0x 4.0	Ctr	Ctr	0.48
S#	LOCK	5.0x10.0	1.2	Ctr	0.82
R	LOCK	4.0x 8.0	Ctr	Ctr	0.54
C	LOCK	6.0x 6.0	Ctr	Ctr	0.90
L	LOCK	6.0x10.0	0.5	1.2	0.95
D	LOCK	6.0x10.0	-4.6	1.5	0.91
I	LOCK	2.0x 4.0	Ctr	Ctr	0.48
F	LOCK	5.0x 6.0	Ctr	-1.1	0.93
P	LOCK	6.0x 6.0	Ctr	-1.2	0.79
O	LOCK	2.0x 4.0	Ctr	Ctr	0.48
H	LOCK	6.0x10.0	Ctr	-1.2	0.97
N	LOCK	2.0x 4.0	Ctr	Ctr	0.48
M	LOCK	6.0x 6.0	Ctr	-1.2	0.79
E	LOCK	5.0x 6.0	Ctr	-1.1	0.93
G	LOCK	2.0x 4.0	Ctr	Ctr	0.48

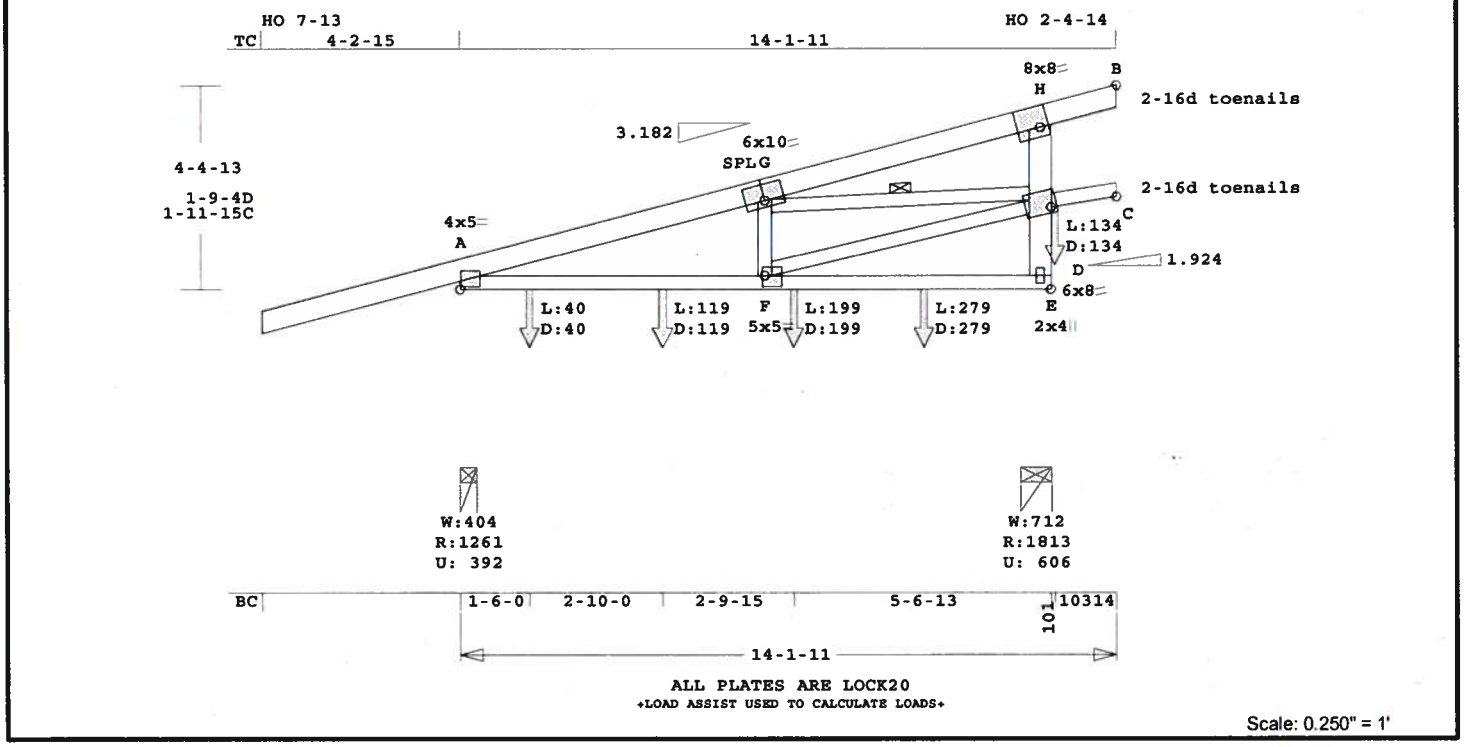
\# = Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6836 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R128*	2	SP	140111	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 115.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 6	SP-#2
BC	0.70	2x 4	SP-2400
CW	0.25	2x 6	SP-#2
WB	0.57	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 1-11
BC Cont.	0- 0- 0	14- 1-11

WB 1 rows CLB on G -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.1'
BC V	0	20	0.0'	14.1'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.2'	CL-LB
BC V	279	279	10.0'	CL-LB
BC V	134	134	12.8'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	1262	393	4- 4	1- 8
			Hz =	168
H	1813	607	7-12	1- 8
C	46	0	3- 8	1- 8
B	44	155G	3- 8	1- 8
			Hz =	90

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -G	0.26	2114 C	0.01		0.25
G -H	0.35	81 C	0.00		0.35
H -B	0.29	94 T	0.00		0.29
Bottom Chords					
A -F	0.70	2028 T	0.16		0.54
F -E	0.69	319 T	0.02		0.67
E -D	0.68	206 T	0.01		0.67
D -C	0.10	20 C	0.00		0.10
Chord-Webs					
E -D	0.15	288 T	0.03		0.12
D -H	0.25	1311 T	0.17		0.08
Webs					
F -G	0.15	587 T			
G -D	0.41	2039 C		1	Br
F -D	0.57	1802 T			

TL Defl 0.14" in F -E L/981
LL Defl 0.08" in F -E L/999

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.01"

Shear // Grain in F -E 0.76

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71
G	LOCK	6.0x10.0	-0.3	1.2	0.96
H	LOCK	8.0x 8.0	-2.1	1.0	0.36
F	LOCK	5.0x 5.0	Ctr	-0.3	0.94
E	LOCK	2.0x 4.0	Ctr	Ctr	0.91
D	LOCK	6.0x 8.0	Ctr	Ctr	0.73

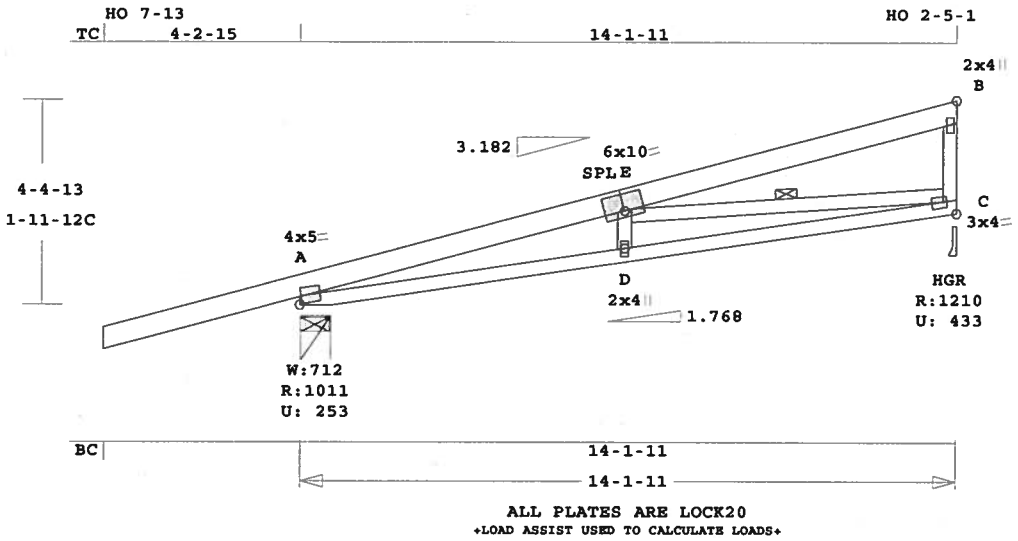
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2114 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R127*	2	HFSC	140111	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 98.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.88	2x 6	SP-#2
BC	0.63	2x 4	SP-2400
WB	0.61	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 1-11
BC Cont.	0- 0- 0	14- 1-11

WB 1 rows CLB on E -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	14.1'
BC V 0	20	0.0'	14.1'
TC V -40	-20	0.0'	
87	44		14.1'
BC V 0	-20	0.0'	
0	44		14.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1011	253	7-12	1- 8
			Hz =	268
C	1211	433	3- 8	1- 8
			Hz =	145

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -E	0.75	2683	C	0.02	0.73
E -B	0.88	163	C	0.00	0.88
Bottom Chords					
A -D	0.50	2607	T	0.20	0.30
D -C	0.63	2611	T	0.12	0.51
Webs					
D -E	0.13	486	T		
E -C	0.61	2582	C		1 Br
C -B	0.14	579	C		

TL Defl -0.53" in D -C L/297
LL Defl -0.23" in D -C L/687

Hz Disp	LL	DL	TL
Jt C	0.02"	0.02"	0.05"

Shear // Grain in E -B 0.44

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

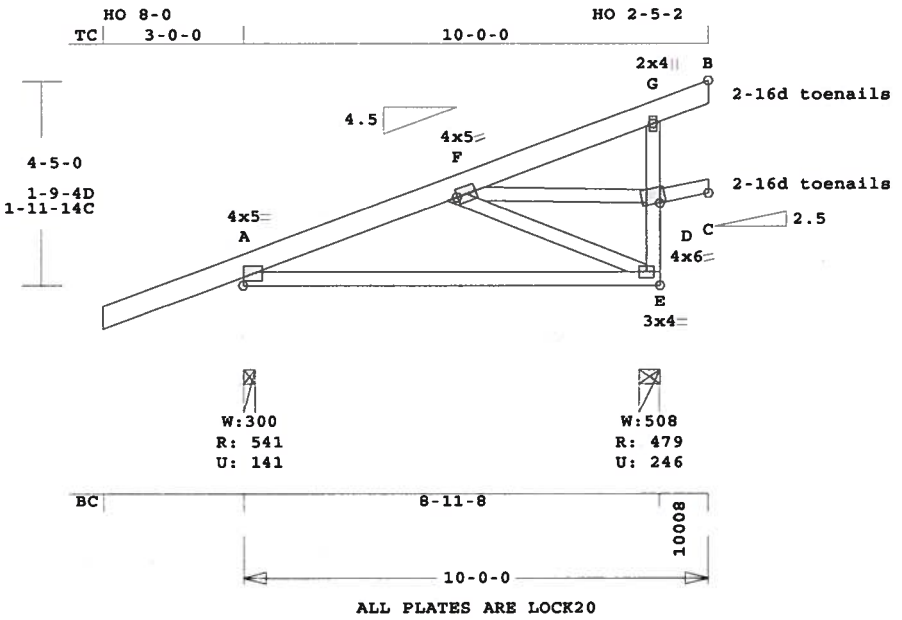
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.1	0.4	0.86
E	LOCK	6.0x10.0	-0.3	1.2	0.96
B	LOCK	2.0x 4.0	Ctr	Ctr	0.45
D	LOCK	2.0x 4.0	Ctr	Ctr	0.44
C	LOCK	3.0x 4.0	-1.0	-0.1	0.99

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 10- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2683 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R126*	6	SP	100000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 80.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 6	SP-#2
BC	0.36	2x 4	SP-#2
CW	0.11	2x 4	SP-#3
WB	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 0- 0
BC Cont.	0- 0- 0	10- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	541	142	3- 0	1- 8
			Hz =	169
G	480	247	5- 8	1- 8
C	50	0	3- 8	1- 8
B	61	51G	3- 8	1- 8
			Hz =	91

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -F	0.10	396 C	0.00	0.10
F -G	0.08	125 C	0.00	0.08
G -B	0.08	104 T	0.00	0.08
--------Bottom Chords---------				
A -E	0.36	378 T	0.03	0.33
E -D	0.34	117 T	0.01	0.33
D -C	0.03	4 T	0.00	0.03
----------Chord-Webs----------				
E -D	0.06	102 T	0.02	0.04
D -G	0.11	223 T	0.07	0.04
-------------Webs-------------				
F -E	0.11	357 T		
F -D	0.03	146 C		

TL Defl -0.16" in A -E L/608
LL Defl -0.08" in A -E L/999

Hz Disp	LL	DL	TL
Jt C	0.00"	0.00"	0.00"

Shear // Grain in A -E 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
F	LOCK	4.0x 5.0	Ctr	Ctr	0.76
G	LOCK	2.0x 4.0	Ctr	Ctr	0.15
E	LOCK	3.0x 4.0	Ctr	Ctr	0.77
D	LOCK	4.0x 6.0	Ctr	Ctr	0.39

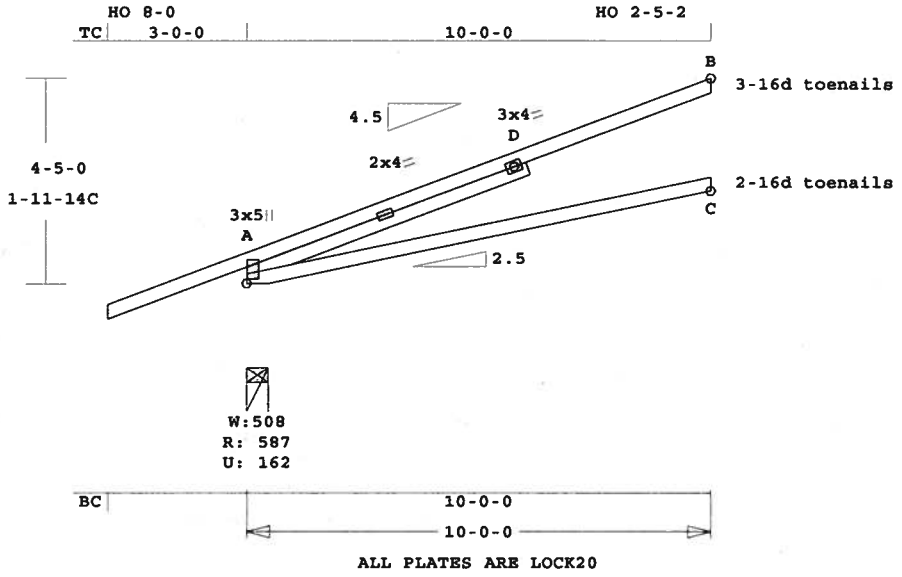
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 396 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R125*	18	VJACK	100000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 58.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.72	2x 4	SP-2400
BC	0.58	2x 4	SP-#2
SL	0.28	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 0- 0
BC Cont.	0- 0- 0	10- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	587	162	5- 8	1- 8
			Hz =	169
C	164	0	3- 8	1- 8
			Hz =	91
B	298	192	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.72	935 C	0.00 0.72
D -B	0.72	137 C	0.00 0.72
--------Bottom Chords---------			
A -C	0.58	88 T	0.00 0.58
----------Sliders-----------			
A -D	0.28	942 T	

TL Defl -0.55" in A -C L/203
LL Defl -0.22" in A -C L/502

Hz Disp	LL	DL	TL
Jt B	0.05"	0.04"	0.08"

Shear // Grain in A -D 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.75
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.67

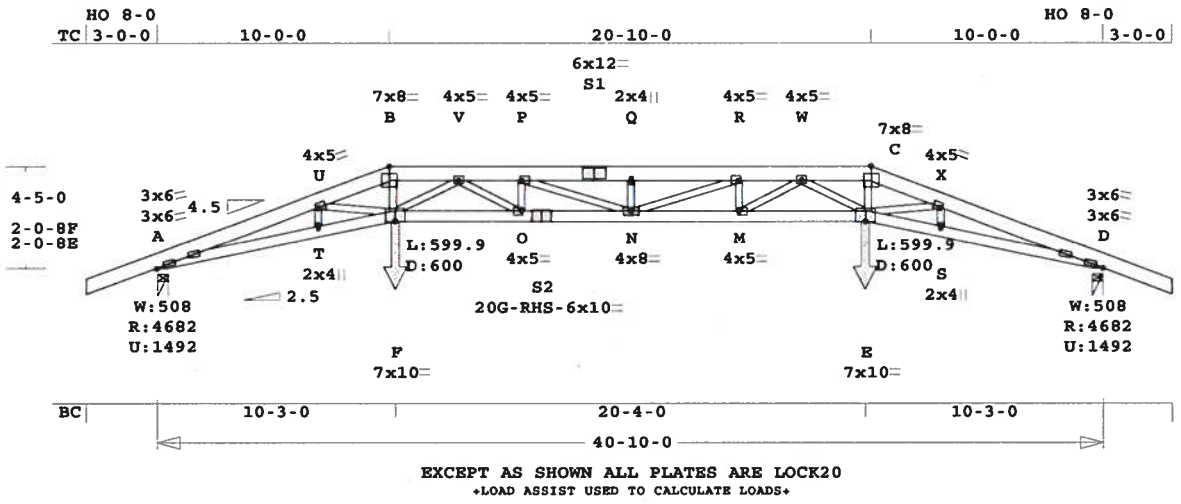
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 935 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
060039	R124	2*3P	HPSC3	401000	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 369.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 3-Ply Truss *
****

	CSI	Size	Lumber
TC	0.28	2x 8	SP-2400
BC	0.51	2x 6	SP-2400
WB	0.68	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	40-10- 0
BC Cont.		0- 0- 0	40-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	40.8'
BC V 0	20	0.0'	40.8'
TC V 80	40	10.0'	30.8'
BC V 0	40	10.2'	30.6'
BC V 600	600	10.2'	CL-LB
BC V 600	600	30.6'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	4682	1492	5- 8	1- 8
			Hz =	-69
D	4683	1493	5- 8	1- 8
			Hz =	70

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -U	0.26	19889 C	0.04	0.22
U -B	0.28	23324 C	0.06	0.22
B -V	0.18	22116 C	0.06	0.12
V -P	0.22	26889 C	0.09	0.13
P -S1	0.21	28503 C	0.10	0.11
S1-Q	0.20	28503 C	0.10	0.10
Q -R	0.21	28503 C	0.10	0.11
R -W	0.22	26890 C	0.09	0.13
W -C	0.18	22117 C	0.05	0.13
C -X	0.28	23326 C	0.06	0.22
X -D	0.26	19890 C	0.04	0.22
Bottom Chords				
A -T	0.44	18710 T	0.28	0.16
T -F	0.44	18815 T	0.28	0.16
F -O	0.46	24527 T	0.37	0.09
O -S2	0.51	26894 T	0.41	0.10
S2-N	0.51	26894 T	0.41	0.10
N -M	0.51	26895 T	0.41	0.10
M -E	0.46	24528 T	0.37	0.09
E -S	0.44	18816 T	0.28	0.16
S -D	0.44	18711 T	0.28	0.16
Webs				
T -U	0.02	437 C		
U -F	0.41	3850 T		
F -B	0.68	6396 T		
F -V	0.14	2764 C		
V -O	0.30	2861 T		
O -P	0.08	1543 C		
P -N	0.18	1733 T		
N -Q	0.05	858 C		
N -R	0.18	1732 T		
M -R	0.08	1542 C		
M -W	0.30	2860 T		
W -E	0.14	2763 C		
E -C	0.68	6398 T		
E -X	0.41	3851 T		
S -X	0.02	437 C		

TL Defl -1.66" in S2-N L/288
LL Defl -0.83" in S2-N L/576

Hz Disp	LL	DL	TL
Jt D	0.39"	0.39"	0.78"

Shear // Grain in U -B 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 6.0	Ctr	Ctr	0.81
A	LOCK	3.0x 6.0	Ctr	Ctr	0.48
U	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	7.0x 8.0	Ctr	Ctr	0.85
V	LOCK	4.0x 5.0	Ctr	0.5	0.61
P	LOCK	4.0x 5.0	Ctr	0.5	0.70
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.48
R	LOCK	4.0x 5.0	Ctr	0.5	0.70
W	LOCK	4.0x 5.0	Ctr	0.5	0.61
C	LOCK	7.0x 8.0	Ctr	Ctr	0.85
X	LOCK	4.0x 5.0	0.5	0.2	0.56
D	LOCK	3.0x 6.0	Ctr	Ctr	0.81
D	LOCK	3.0x 6.0	Ctr	Ctr	0.48
T	LOCK	2.0x 4.0	Ctr	Ctr	0.48
F	LOCK	7.0x10.0	Ctr	-0.6	0.87
O	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	RHS	6.0x10.0	Ctr	0.2	0.83
N	LOCK	4.0x 8.0	Ctr	Ctr	0.63
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	7.0x10.0	Ctr	-0.6	0.87
S	LOCK	2.0x 4.0	Ctr	Ctr	0.48

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 10- 0- 0
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

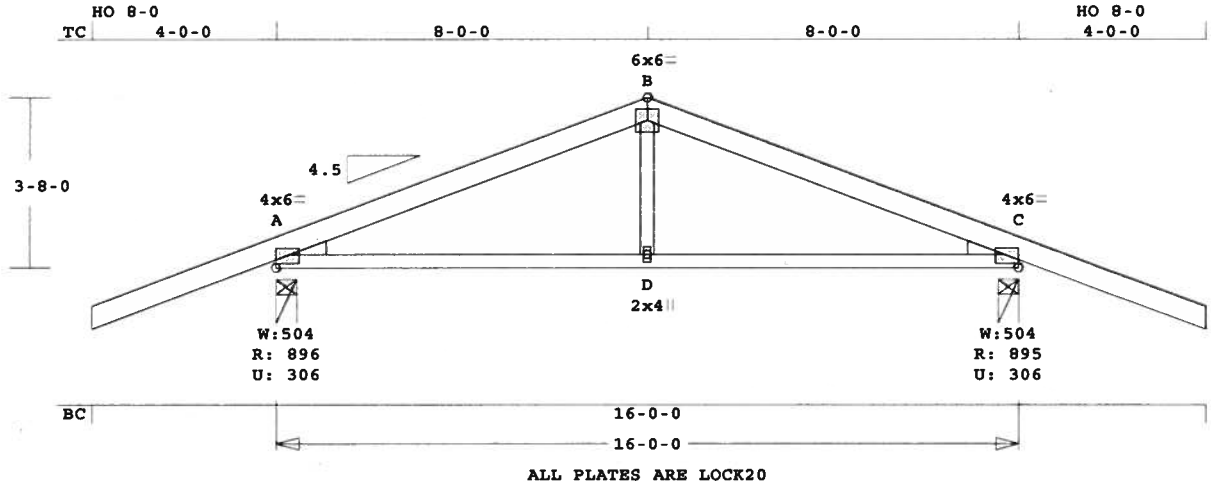
	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	2	12	24	0
BC	2	12	24	0
WB	1	8	8	

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 28503 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R123	3	TR	160000	4.5	4- 0- 0	4- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 112.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.25	2x 6	SP-#2
BC	0.42	2x 4	SP-#2
WB	0.08	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16- 0- 0
BC	Cont.	0- 0- 0	16- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	896	307	5- 4	1- 8
C	896	307	5- 4	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -B	0.25	892 C	0.00	0.25
B -C	0.25	892 C	0.00	0.25
Bottom Chords				
A -D	0.42	831 T	0.09	0.33
D -C	0.42	831 T	0.09	0.33
Webs				
D -B	0.08	322 T		

TL Defl -0.11" in D -C L/999
LL Defl -0.05" in D -C L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	1.2	0.3	0.66
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C	LOCK	4.0x 6.0	-1.2	0.3	0.66
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38

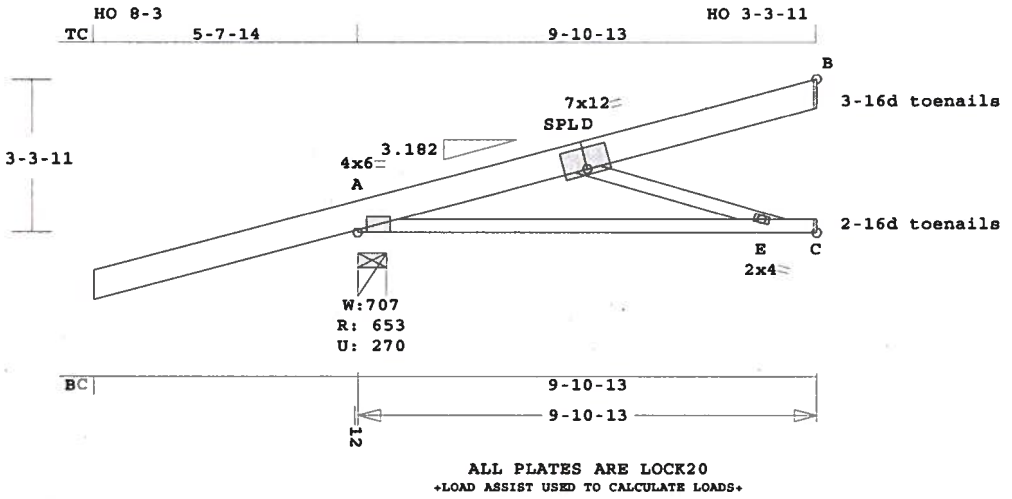
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf

Max comp. force 892 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R122*	4	KING	91013	3.182	5- 7-14	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 88.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 8	SP-2400
BC	0.66	2x 4	SP-#2
WB	0.18	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9-10-13
BC	Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
TC V	-40	-20	0.0'	
	45	22		9.9'
BC V	0	-20	0.0'	
	0	22		9.9'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	654	270	7- 7	1- 8
			Hz =	125
C	291	21	3- 8	1- 8
			Hz =	68
B	311	184	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.10	487 C	0.00	0.10
D -B	0.10	103 C	0.00	0.10
--------Bottom Chords---------				
A -E	0.66	479 T	0.03	0.63
E -C	0.27	67 T	0.00	0.27
------------Webs------------				
D -E	0.18	503 C		

TL Defl -0.44" in A -E L/247
LL Defl -0.21" in A -E L/523
Shear // Grain in E -C 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.89
D	LOCK	7.0x12.0-0.5	2.0	0.96	
E	LOCK	2.0x 4.0	Ctr	Ctr	0.48

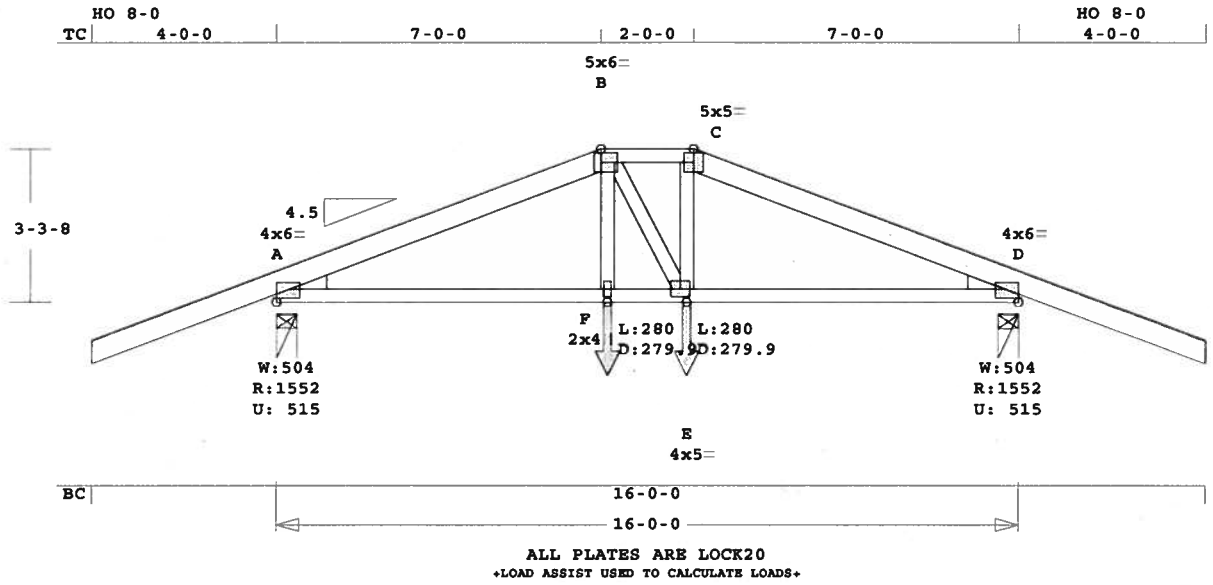
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 503 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R121	2	HIPP	160000	4.5	4- 0- 0	4- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 120.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 6	SP-#2
EX B -C		2x 4	SP-#2
BC	0.75	2x 4	SP-#2
WB	0.20	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16- 0- 0
BC	Cont.	0- 0- 0	16- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	16.0'
BC V 0	20	0.0'	16.0'
TC V 50	25	7.0'	9.0'
BC V 0	25	7.1'	8.9'
BC V 280	280	7.1'	CL-LB
BC V 280	280	8.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1552	515	5- 4	1-13
D	1552	515	5- 4	1-13

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.45	2574 C	0.02	0.43
B -C	0.33	2374 C	0.20	0.13
C -D	0.45	2566 C	0.02	0.43
--------Bottom Chords---------				
A -F	0.75	2382 T	0.44	0.31
F -E	0.49	2361 T	0.43	0.06
E -D	0.73	2374 T	0.43	0.30
------------Webs------------				
F -B	0.20	634 T		
B -E	0.00	39 C		
E -C	0.19	617 T		

TL Defl -0.16" in F -E L/999
LL Defl -0.08" in F -E L/999
Shear // Grain in A -B 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

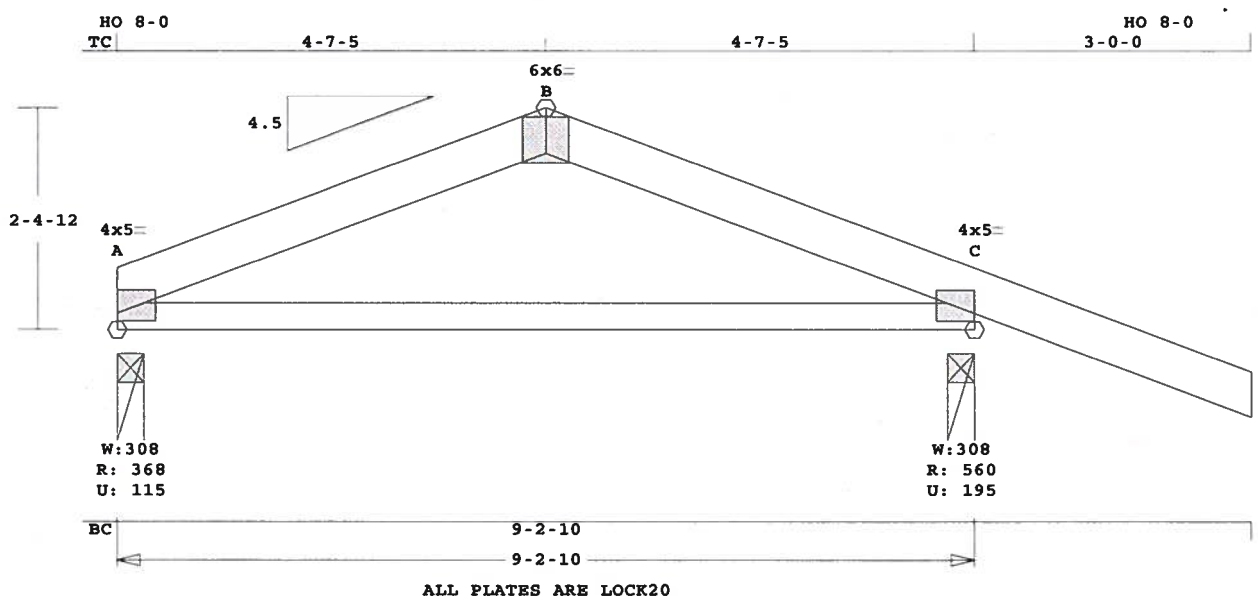
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	1.2	0.3	0.66
B	LOCK	5.0x 6.0-0.5	Ctr		0.64
C	LOCK	5.0x 5.0	Ctr	Ctr	0.63
D	LOCK	4.0x 6.0-1.2	0.3		0.66
F	LOCK	2.0x 4.0	Ctr	Ctr	0.68
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2574 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R119*	1	TR	90210	4.5	0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 6	SP-#2
BC	0.39	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 2-10
BC	Cont.	0- 0- 0	9- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	369	115	3- 8	1- 8
C	561	195	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.11	415 T	0.00	0.11
B -C	0.11	415 T	0.00	0.11
--------Bottom Chords---------				
A -C	0.39	382 T	0.03	0.36

TL Defl -0.17" in A -C L/617
LL Defl -0.08" in A -C L/999
Shear // Grain in A -C 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

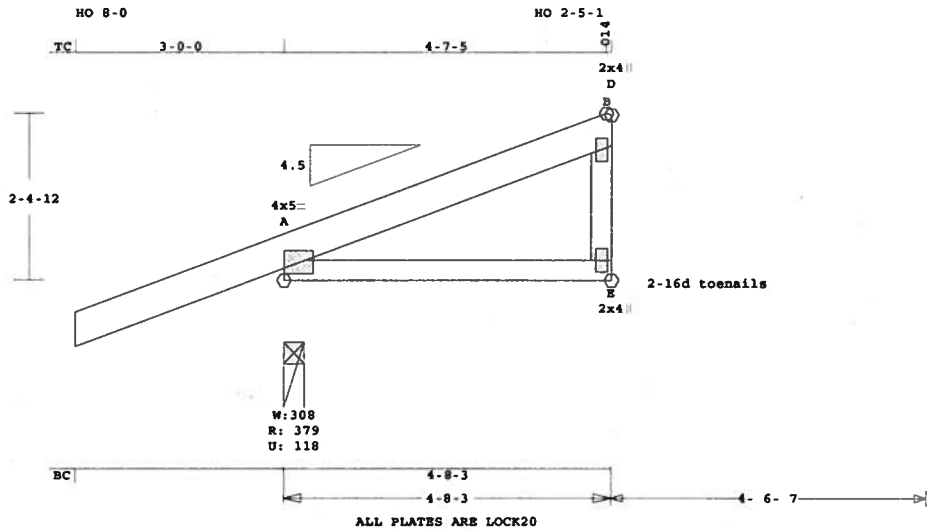
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr-1.2		0.33
C	LOCK	4.0x 5.0	-0.7	0.3	0.65

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 405 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R118*	1	TR	40803	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 35.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.17	2x 6	SP-#2
BC	0.12	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	4- 8- 3
BC	Cont.	0- 0- 0	4- 8- 3

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	379	119	3- 8	1- 8
			Hz =	78
E	187	72	3- 8	1- 8
			Hz =	42

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.17	95 C	0.00 0.17
--------Bottom Chords---------			
A -E	0.12	41 T	0.00 0.12
----------Webs----------			
E -D	0.06	251 T	

TL Defl -0.02" in A -E L/999
LL Defl -0.01" in A -E L/999
Shear // Grain in A -B 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

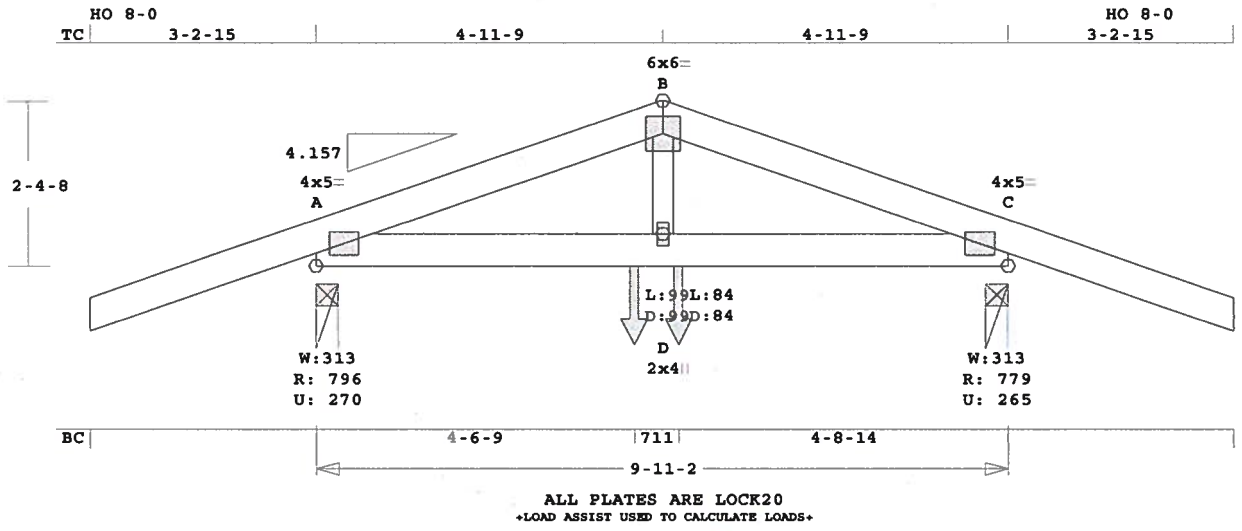
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 141 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R117*	1	TR	91102	4.157	3- 2-15	3- 2-15	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.18	2x 6	SP-#2
BC	0.20	2x 6	SP-#2
WB	0.13	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9-11- 2
BC	Cont.	0- 0- 0	9-11- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
BC V	99	99	4.5'	CL-LB
BC V	84	84	5.2'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	796	271	3-13	1- 8
C	779	265	3-13	1- 8
			Hz =	-27
			Hz =	28

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.18	999 C	0.08	0.10
B -C	0.18	1000 C	0.08	0.10
--------Bottom Chords---------				
A -D	0.20	940 T	0.12	0.08
D -C	0.14	940 T	0.12	0.02
-------------Webs-------------				
D -B	0.13	496 T		

TL Defl -0.02" in A -D L/999
LL Defl -0.01" in A -D L/999
Shear // Grain in A -D 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.67
B	LOCK	6.0x 6.0	Ctr	Ctr	0.48
C	LOCK	4.0x 5.0	Ctr	Ctr	0.67
D	LOCK	2.0x 4.0	Ctr	Ctr	0.53

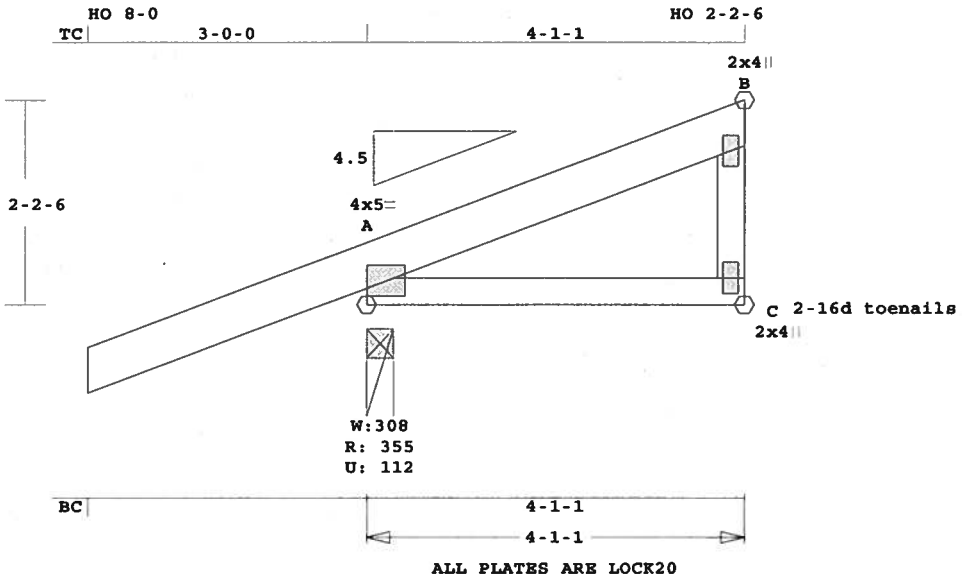
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1000 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R116*	1	MONO	40101	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 6	SP-#2
BC	0.09	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	4- 1- 1
BC	Cont.	0- 0- 0	4- 1- 1

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	356	112	3- 8	1- 8
			Hz =	69
C	164	70	3- 8	1- 8
			Hz =	37

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.13	88	C 0.00 0.13
Bottom Chords			
A -C	0.09	37	T 0.00 0.09
Webs			
C -B	0.05	237	T

TL Defl -0.01" in A -C L/999
LL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

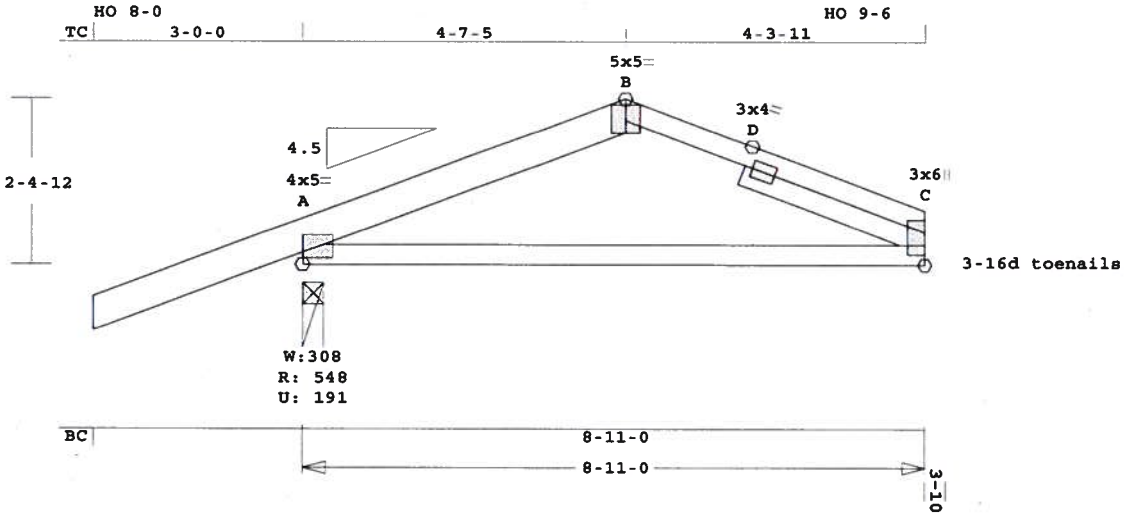
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 123 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R115*	1	TR	81100	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 54.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 6	SP-#2
EX B -C		2x 4	SP-#2
BC	0.36	2x 4	SP-#2
SL	0.07	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8-11- 0
BC	Cont.	0- 0- 0	8-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	549	191	3- 8	1- 8
C	357	112	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.12	390 T	0.03	0.09
B -D	0.12	412 T	0.05	0.07
D -C	0.10	475 C	0.00	0.10
--------Bottom Chords---------				
A -C	0.36	354 T	0.03	0.33
----------Sliders-----------				
D -C	0.07	314 T		

TL Defl -0.14" in A -C L/702
LL Defl -0.07" in A -C L/999
Shear // Grain in A -C 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	5.0x 5.0	Ctr-0.5		0.37
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 6.0	-1.5	0.3	0.81

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

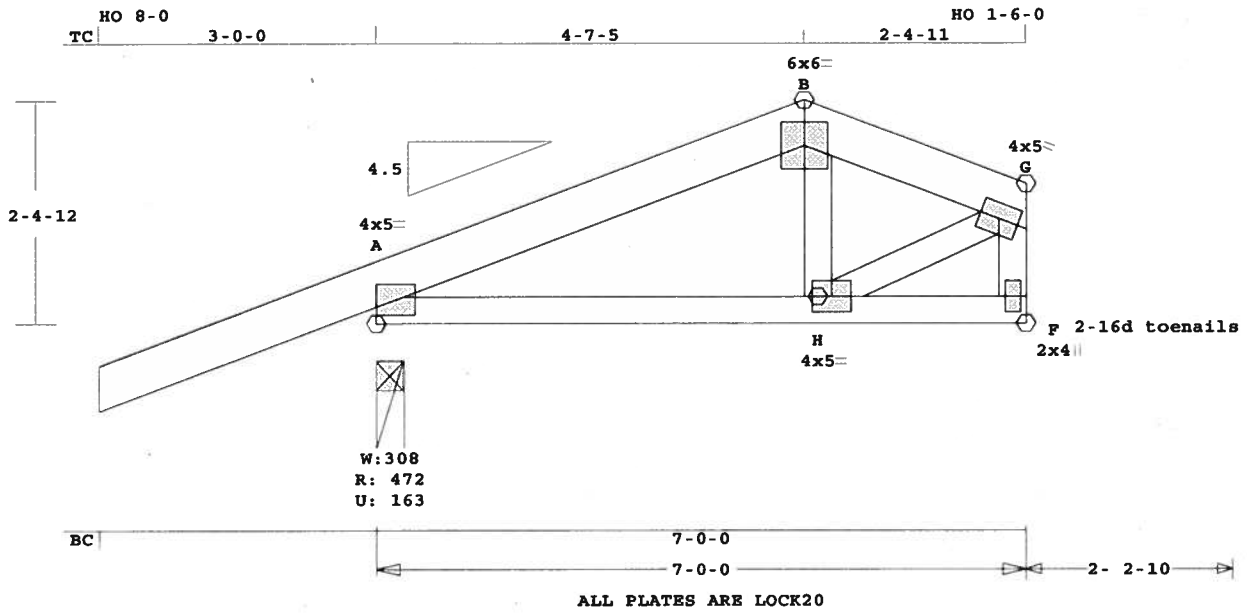
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 475 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R114	2	TR	70000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 52.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.14	2x 6	SP-#2
BC	0.10	2x 4	SP-#2
WB	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	472	164	3- 8	1- 8
			Hz =	37
F	280	91	3- 8	1- 8
			Hz =	18

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.14	242 C	0.02 0.12
B -G	0.04	265 T	0.02 0.02
--------Bottom Chords---------			
A -H	0.10	224 T	0.02 0.08
H -F	0.08	18 T	0.00 0.08
------------Webs------------			
H -B	0.03	147 T	
H -G	0.08	256 T	
F -G	0.07	305 T	

TL Defl -0.02" in A -H L/999
LL Defl -0.01" in A -H L/999
Shear // Grain in A -B 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
G	LOCK	4.0x 5.0	Ctr	Ctr	0.47
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

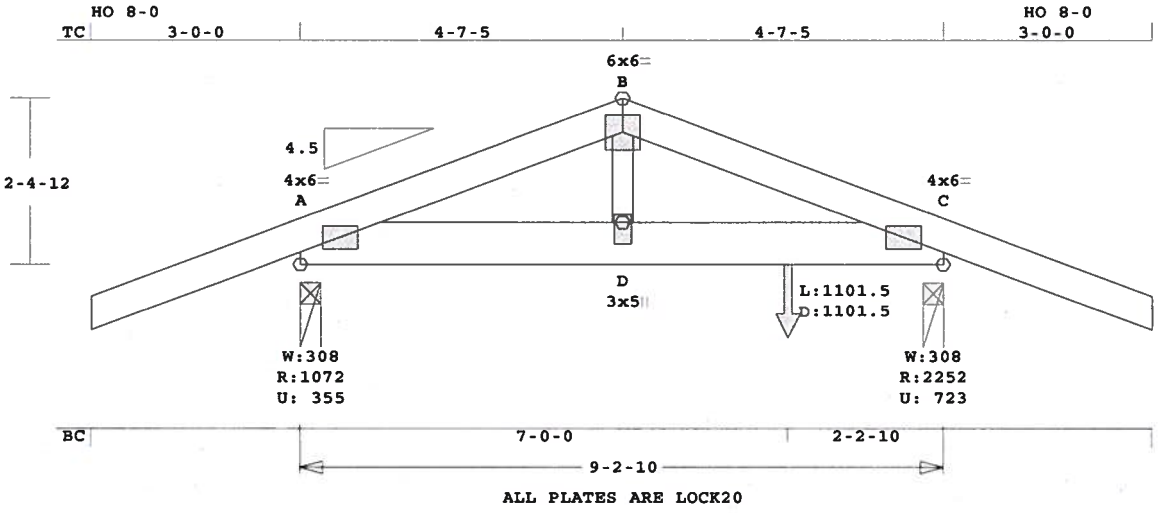
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 270 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R113A*	1	TR	90210	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 6	SP-#2
BC	0.59	2x 8	SP-2400
WB	0.35	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 2-10
BC	Cont.	0- 0- 0	9- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.2'
BC V	0	20	0.0'	9.2'
BC V	1102	1102	7.0'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1072	355	3- 8	1- 8
C	2252	724	3- 8	1-14

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.24	2066 C	0.17	0.07
B -C	0.45	2008 C	0.17	0.28
--------Bottom Chords---------				
A -D	0.19	1929 T	0.07	0.12
D -C	0.59	1929 T	0.07	0.52
-------------Webs-------------				
D -B	0.35	1155 T		

TL Defl -0.05" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in D -C 0.75

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.77
B	LOCK	6.0x 6.0	Ctr	Ctr	0.47
C	LOCK	4.0x 6.0	Ctr	Ctr	0.77
D	LOCK	3.0x 5.0	Ctr	-1.1	0.83

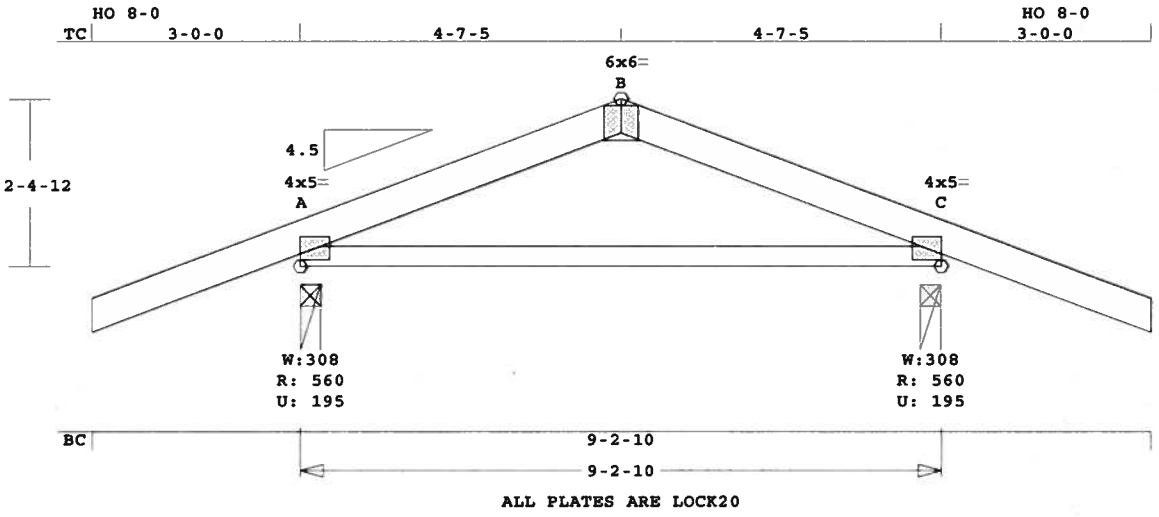
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2066 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R113*	16	TR	90210	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 64.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 6	SP-#2
BC	0.39	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 2-10
BC Cont.	0- 0- 0	9- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	561	195	3- 8	1- 8
C	561	195	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.11	415 T	0.00	0.11
B -C	0.11	415 T	0.00	0.11
--------Bottom Chords---------				
A -C	0.39	382 T	0.03	0.36

TL Defl -0.17" in A -C L/617
LL Defl -0.08" in A -C L/999
Shear // Grain in A -C 0.19

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	6.0x 6.0	Ctr-1.2	0.3	0.33
C	LOCK	4.0x 5.0	-0.7	0.3	0.65

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 405 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R112*	1*4P	SP	450404	4.5	0	0	

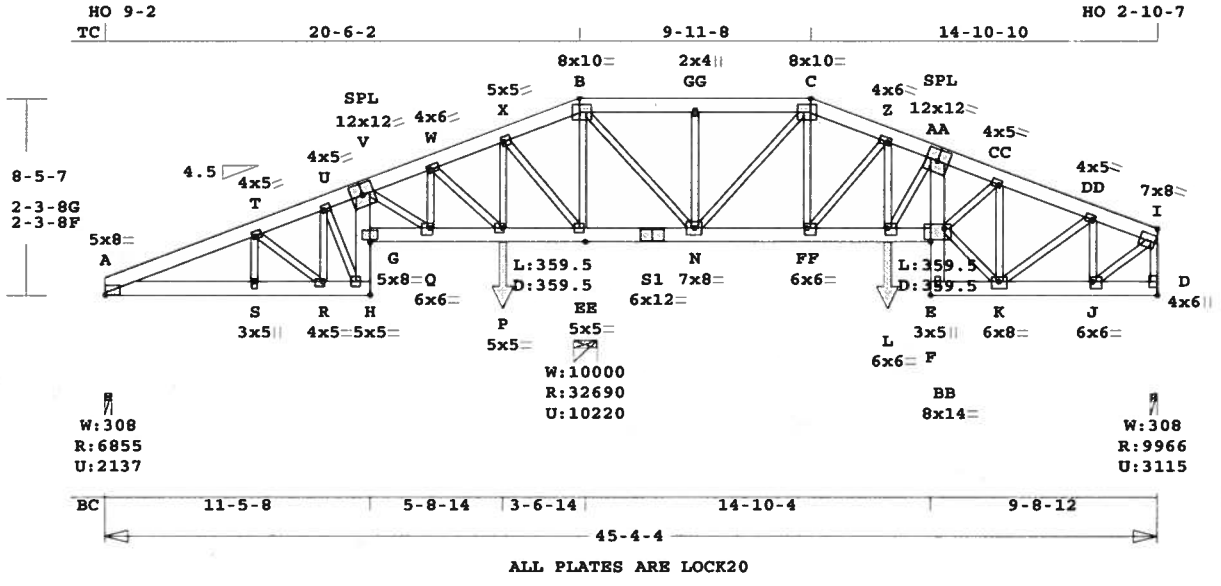
+Office and Care Center, Hospice, M.M. Parrish

Plus use 1/2 In (ASTM-A307)
thru bolts at each panel
point and on each side of
splices in 2x6 or larger
chords only.
Design checked for 10 psf non-
concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
bearing locations.
Use properly rated hangers for
loads framing into girder
truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 17903 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R112	1*4P	SP	450404	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 562.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 4-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.32	2x 8	SP-2400
BC	0.36	2x 8	SP-2400
CW	0.37	2x 8	SP-2400
WB	0.82	2x 4	SP-#3
EX B -N		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	45- 4- 4
BC Cont.		0- 0- 0	45- 4- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	45.4'
BC V	490	510	0.0'	45.4'
BC V	360	360	17.2'	CL-LB
BC V	360	360	33.8'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	6856	2138	3- 8	1- 8
			Hz =	-116
EE	32691	10220	12- 0	6- 9
D	9967	3115	3- 8	2- 1
			Hz =	127

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -T	0.08	9371	C	0.00	0.08
T -U	0.02	4596	C	0.00	0.02
U -V	0.10	2212	C	0.00	0.10
V -W	0.12	3232	T	0.02	0.10
W -X	0.27	9573	T	0.08	0.19
X -B	0.32	15755	T	0.13	0.19
B -GG	0.17	3243	T	0.02	0.15
GG-C	0.11	3243	T	0.02	0.09
C -Z	0.10	5138	C	0.00	0.10
Z -AA	0.10	10359	C	0.00	0.10
AA-CC	0.09	12905	C	0.01	0.08
CC-DD	0.03	9463	C	0.00	0.03
DD-I	0.02	7281	C	0.02	0.00
Bottom Chords					
A -S	0.24	8862	T	0.07	0.17
S -R	0.18	8862	T	0.07	0.11
R -H	0.06	4276	T	0.03	0.03
G -Q	0.17	2792	T	0.02	0.15
Q -P	0.06	2888	C	0.00	0.06
P -EE	0.35	8614	C	0.00	0.35
EE-S1	0.36	14969	C	0.01	0.35
S1-N	0.07	14969	C	0.01	0.06
N -FF	0.17	4924	T	0.04	0.13
FF-L	0.15	9649	T	0.08	0.07
L -BB	0.22	12158	T	0.10	0.12
E -K	0.06	1024	T	0.00	0.06
K -J	0.13	6800	T	0.05	0.08
J -D	0.03	126	T	0.00	0.03
Chord-Webs					
H -G	0.37	6368	T	0.06	0.31
G -V	0.21	6793	T	0.06	0.15
E -BB	0.17	992	T	0.00	0.17
BB-AA	0.08	4949	T	0.04	0.04
Webs					
S -T	0.40	5033	T		
T -R	0.24	6026	C		
R -U	0.43	5424	T		
U -H	0.23	5867	C		
V -Q	0.27	6935	C		
Q -W	0.66	8275	T		
W -P	0.32	8056	C		
P -X	0.63	7925	T		
X -EE	0.44	10614	C		
EE-B	0.74	17903	C		
B -N	0.80	17436	T		
N -GG	0.01	216	T		
N -C	0.54	12165	C		
FF-C	0.79	9965	T		
FF-Z	0.30	7235	C		
L -Z	0.66	8240	T		
L -AA	0.19	4788	C		
BB-CC	0.34	4278	T		
BB-K	0.82	10248	T		
K -CC	0.18	4493	C		
K -DD	0.21	2628	T		
J -DD	0.10	2647	C		
J -I	0.73	9103	T		
D -I	0.34	8743	C		

TL Defl -0.24" in FF-L L/999
LL Defl -0.12" in FF-L L/999
Shear // Grain in P -EE 0.47

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	4.0	1.4	0.86
T	LOCK	4.0x 5.0-0.5	0.2		0.72
U	LOCK	4.0x 5.0-0.5	0.2		0.75
V	LOCK	12.0x12.0 Ctr	Ctr		0.91
W	LOCK	4.0x 6.0-0.5	0.2		0.87
X	LOCK	5.0x 5.0 Ctr	Ctr		0.95
B	LOCK	8.0x10.0 Ctr	Ctr		0.81
GG	LOCK	2.0x 4.0 Ctr	Ctr		0.51
C	LOCK	8.0x10.0 Ctr	Ctr		0.80
Z	LOCK	4.0x 6.0 0.5	0.2		0.85
AA	LOCK	12.0x12.0 Ctr	Ctr		0.91
CC	LOCK	4.0x 5.0 0.5	0.2		0.56
DD	LOCK	4.0x 5.0 0.5	0.2		0.56
I	LOCK	7.0x 8.0-1.5	0.6		0.81
S	LOCK	3.0x 5.0 Ctr	-1.1		0.90
R	LOCK	4.0x 5.0 Ctr	-0.5		0.85
H	LOCK	5.0x 5.0 Ctr	Ctr		0.96
G	LOCK	5.0x 8.0 Ctr	Ctr		0.79
Q	LOCK	6.0x 6.0 Ctr	-0.6		0.77
P	LOCK	5.0x 5.0 Ctr	Ctr		0.89
EE	LOCK	5.0x 5.0 Ctr	Ctr		0.91
S1	LOCK	6.0x12.0 Ctr	Ctr		0.91
N	LOCK	7.0x 8.0-0.5	0.1		0.94
FF	LOCK	6.0x 6.0 Ctr	-0.6		0.93
L	LOCK	6.0x 6.0 Ctr	-0.6		0.77
BB	LOCK	8.0x14.0 Ctr	Ctr		0.82
E	LOCK	3.0x 5.0 Ctr	Ctr		0.96
K	LOCK	6.0x 8.0 Ctr	-0.6		0.91
J	LOCK	6.0x 6.0 Ctr	-0.6		0.73
D	LOCK	4.0x 6.0-0.2	Ctr		0.91

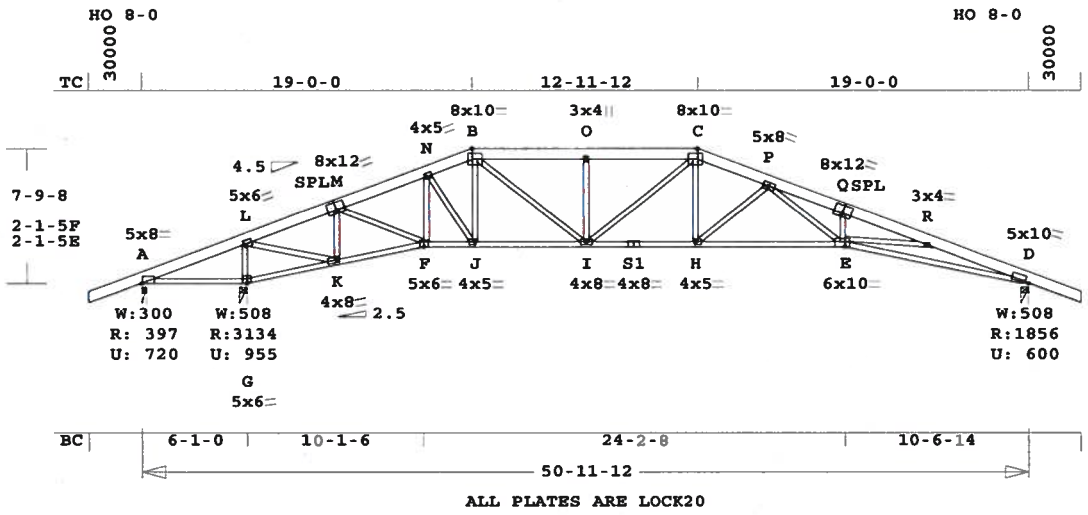
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 51- 0- 0
4 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS6 screws -OR- 16d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
	----Spacing (In)----			
TC	2	12	24	24
BC	2	6	9.5	14.5
WB	1	8	8	

Web Connection Exception -- Use 4" spacing for screws or nails on the following webs
X- E E- B B- N N- C
No bolts in 2x4s or smaller.
Plus clusters of nails where shown.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R110	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 459.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.32	2x 8	SP-2400
BC	0.89	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.96	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	398	720G	3- 0	1- 8
		Hz =	-132	
G	3135	955	5- 8	3- 7
D	1856	601	5- 8	1- 9
		Hz =	133	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.27	2367 T	0.08		0.19
L -M	0.19	896 C	0.00		0.19
M -N	0.05	2410 C	0.00		0.05
N -B	0.03	2478 C	0.03		0.00
B -O	0.05	3137 C	0.01		0.04
O -C	0.07	3137 C	0.01		0.06
C -P	0.06	3476 C	0.04		0.02
P -Q	0.11	6730 C	0.05		0.06
Q -R	0.32	6690 C	0.05		0.27
R -D	0.29	5771 C	0.03		0.26
Bottom Chords					
A -G	0.71	2128 C	0.04		0.67
G -K	0.88	2193 C	0.05		0.83
K -F	0.36	806 T	0.13		0.23
F -J	0.68	2265 T	0.38		0.30
J -I	0.49	2320 T	0.38		0.11
I -S1	0.65	3253 T	0.54		0.11
S1-H	0.73	3253 T	0.54		0.19
H -E	0.89	4217 T	0.70		0.19
E -D	0.52	5352 T	0.38		0.14
Webs					
G -L	0.44	2573 C			
L -K	0.96	3013 T			
K -M	0.27	1340 C			
M -F	0.51	1592 T			
F -N	0.09	338 C			
N -J	0.03	127 T			
J -B	0.02	88 T			
B -I	0.40	1038 T			
I -O	0.16	423 C			
I -C	0.13	148 C			
H -C	0.31	970 T			
H -P	0.55	1244 C			
P -E	0.87	2722 T			
E -Q	0.08	427 C			
E -R	0.34	1070 T			

TL Defl -0.15" in A -G L/417
LL Defl -0.07" in A -G L/918

Hz Disp	LL	DL	TL
Jt D	0.21"	0.21"	0.42"

Shear // Grain in R -D 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

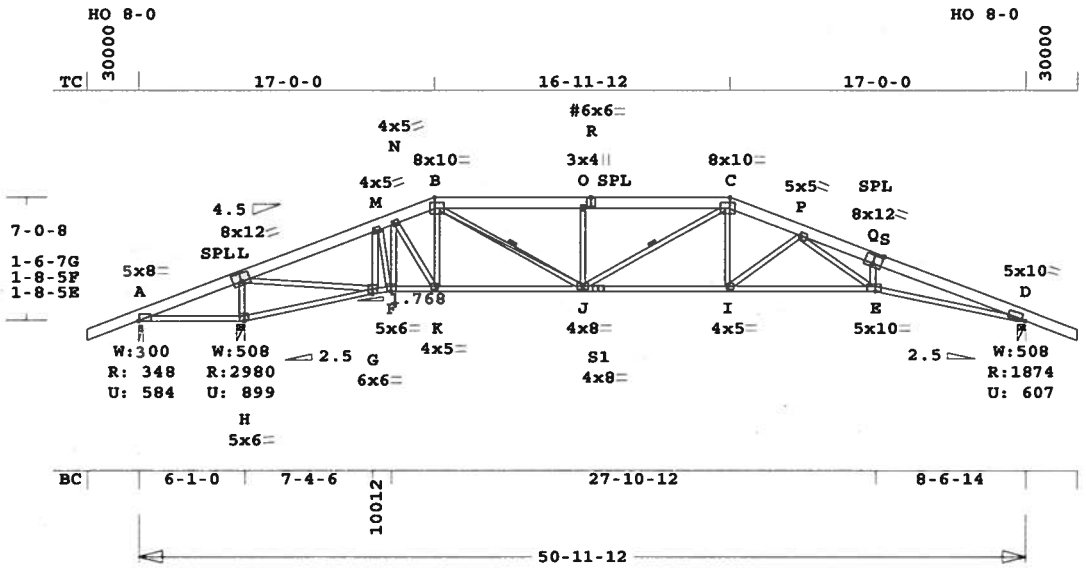
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
L	LOCK	5.0x 6.0	0.5	0.2	0.97
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	3.0x 4.0	Ctr	Ctr	0.36
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
P	LOCK	5.0x 8.0	0.4	-0.2	0.98
Q	LOCK	8.0x12.0	0.6	1.5	0.97
R	LOCK	3.0x 4.0	Ctr	Ctr	0.58
D	LOCK	5.0x10.0	Ctr	1.1	1.00
G	LOCK	5.0x 6.0	-0.3	2.8	0.75
K	LOCK	4.0x 8.0	-0.5	-0.1	0.97
F	LOCK	5.0x 6.0	Ctr	-1.0	0.75
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S1	LOCK	4.0x 8.0	Ctr	0.2	0.75
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x10.0	Ctr	-0.5	0.87

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6730 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R109*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 451.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.33	2x 8	SP-2400
BC	0.95	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.63	2x 4	SP-#3
EX L -G		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

WB 1 rows CLB on B -J
WB 1 rows CLB on J -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	348	585G	3- 0	1- 8
			Hz =	-118
H	2980	899	5- 8	3- 5
D	1875	608	5- 8	1- 9
			Hz =	119

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.28	1989 T	0.07		0.21
L -M	0.21	1891 C	0.00		0.21
M -N	0.03	2131 C	0.00		0.03
N -B	0.03	2365 C	0.00		0.03
B -O	0.09	3487 C	0.04		0.05
O -R	0.09	3487 C	0.04		0.05
R -C	0.11	3487 C	0.01		0.10
C -P	0.06	3698 C	0.04		0.02
P -Q	0.30	6262 C	0.04		0.26
Q -D	0.33	6432 C	0.05		0.28
Bottom Chords					
A -H	0.66	1774 C	0.03		0.63
H -G	0.85	1825 C	0.04		0.81
G -F	0.44	1801 T	0.30		0.14
F -K	0.50	2008 T	0.33		0.17
K -J	0.59	2218 T	0.23		0.36
J -S1	0.78	3454 T	0.57		0.21
S1-I	0.79	3454 T	0.57		0.22
I -E	0.95	4389 T	0.73		0.22
E -D	0.62	6053 T	0.43		0.19
Webs					
H -L	0.43	2497 C			
L -G	0.91	3532 T			
G -M	0.25	1074 C			
M -F	0.26	835 T			
F -N	0.12	496 C			
N -K	0.12	390 T			
K -B	0.06	192 C			
B -J	0.46	1451 T			1 Br
J -O	0.19	556 C			
J -C	0.04	134 C			1 Br
I -C	0.28	903 T			
I -P	0.50	1167 C			
P -E	0.63	1967 T			
E -Q	0.07	271 T			

TL Defl -0.15" in A -H L/442
LL Defl -0.07" in A -H L/938

Hz Disp	LL	DL	TL
Jt D	0.21"	0.21"	0.42"

Shear // Grain in Q -D 0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
L	LOCK	8.0x12.0	-0.6	1.5	0.97
M	LOCK	4.0x 5.0	-0.5	0.2	0.62
N	LOCK	4.0x 5.0	-0.5	0.2	0.56
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
O	LOCK	3.0x 4.0	Ctr	Ctr	0.36
R#	LOCK	6.0x 6.0	Ctr	0.3	0.91
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
P	LOCK	5.0x 5.0	Ctr	Ctr	0.78
Q	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	5.0x10.0	Ctr	1.1	1.00
H	LOCK	5.0x 6.0	-0.3	2.8	0.75
G	LOCK	6.0x 6.0	Ctr	Ctr	0.89
F	LOCK	5.0x 6.0	Ctr	-1.0	0.75
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
J	LOCK	4.0x 8.0	Ctr	Ctr	0.57
S1	LOCK	4.0x 8.0	Ctr	0.2	0.75
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	5.0x10.0	-1.0	-1.0	0.95

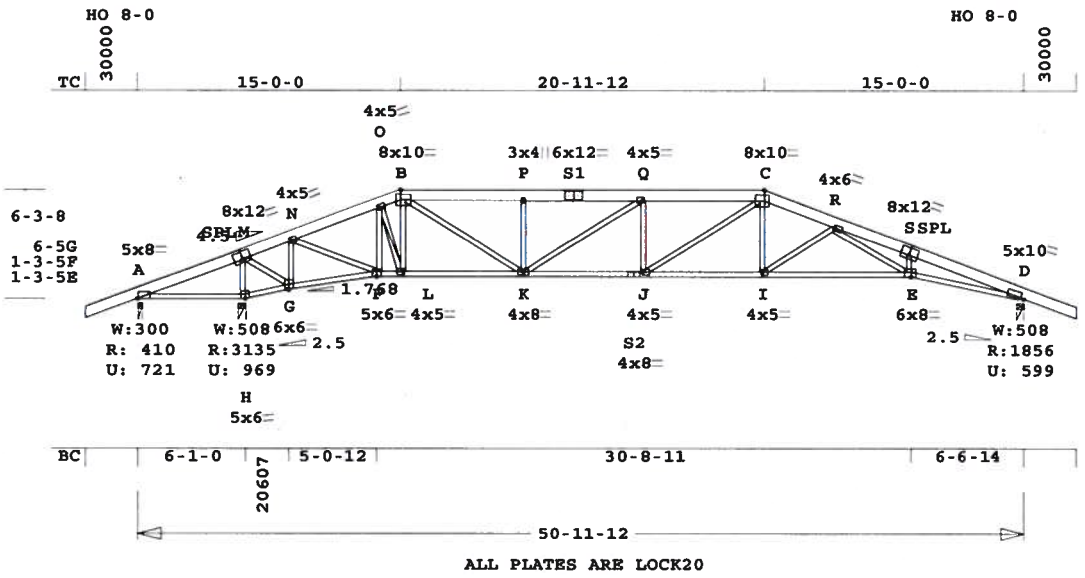
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6432 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R108*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 456.8 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.35	2x 8	SP-2400
BC	0.91	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.69	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	410	721G	3- 0	1- 8
		Hz =	-104	
H	3135	969	5- 8	3- 7
D	1857	599	5- 8	1- 9
		Hz =	105	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.26	2366 T	0.08		0.18
M -N	0.19	441 T	0.01		0.18
N -O	0.04	1722 C	0.02		0.02
O -B	0.03	1911 C	0.02		0.01
B -P	0.06	3195 C	0.01		0.05
P -S1	0.06	3195 C	0.01		0.05
S1-Q	0.06	3195 C	0.01		0.05
Q -C	0.08	3772 C	0.01		0.07
C -R	0.06	3713 C	0.04		0.02
R -S	0.35	5896 C	0.04		0.31
S -D	0.35	6100 C	0.04		0.31
Bottom Chords					
A -H	0.61	2131 C	0.04		0.57
H -G	0.89	2216 C	0.03		0.86
G -F	0.23	404 C	0.00		0.23
F -L	0.67	1631 T	0.27		0.40
L -K	0.44	1732 T	0.18		0.26
K -S2	0.77	3772 T	0.63		0.14
S2-J	0.74	3772 T	0.63		0.11
J -I	0.77	3462 T	0.58		0.19
I -E	0.91	4335 T	0.72		0.19
E -D	0.55	5696 T	0.40		0.15
Webs					
H -M	0.43	2485 C			
M -G	0.64	2022 T			
G -N	0.31	1657 C			
N -F	0.69	2174 T			
F -O	0.18	711 C			
O -L	0.09	302 T			
L -B	0.10	338 C			
B -K	0.62	1725 T			
K -P	0.11	387 C			
K -Q	0.65	692 C			
J -Q	0.04	162 T			
J -C	0.15	364 T			
I -C	0.24	778 T			
I -R	0.42	1050 C			
R -E	0.50	1560 T			
E -S	0.11	418 T			

TL Defl -0.14" in A -H L/467
LL Defl -0.06" in A -H L/999

Hz Disp	LL	DL	TL
Jt D	0.18"	0.18"	0.35"

Shear // Grain in S -D 0.30

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

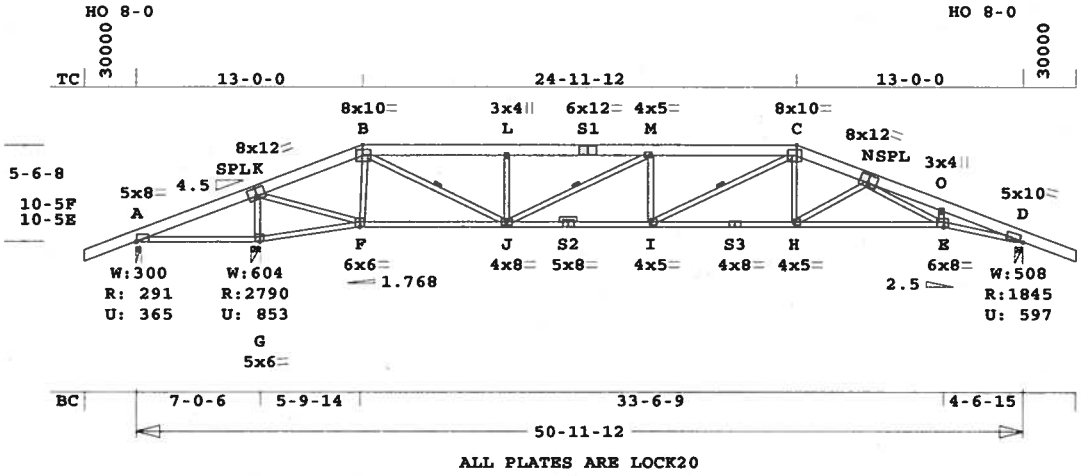
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
M	LOCK	8.0x12.0	-0.6	1.5	0.97
N	LOCK	4.0x 5.0	-0.5	0.2	0.99
O	LOCK	4.0x 5.0	-0.5	0.2	0.67
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
P	LOCK	3.0x 4.0	Ctr	Ctr	0.36
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.60
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
R	LOCK	4.0x 6.0	0.5	0.2	0.94
S	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	5.0x10.0	Ctr	1.1	1.00
H	LOCK	5.0x 6.0	-0.3	2.8	0.75
G	LOCK	6.0x 6.0	Ctr	Ctr	0.88
F	LOCK	5.0x 6.0	Ctr	-1.0	0.86
L	LOCK	4.0x 5.0	Ctr	Ctr	0.66
K	LOCK	4.0x 8.0	Ctr	Ctr	0.69
S2	LOCK	4.0x 8.0	Ctr	0.3	0.75
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x 8.0	-1.0	-0.5	0.96

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6100 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R107*	1	SP	501112	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 437.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.35	2x 8	SP-2400
BC	0.91	2x 4	SP-#2
EX E -D		2x 4	SP-2400
WB	0.82	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

WB 1 rows CLB on B -J
WB 1 rows CLB on J -M
WB 1 rows CLB on I -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	292	365G	3- 0	1- 8
			Hz =	-90
G	2790	853	6- 4	3- 2
D	1845	597	5- 8	1- 8
			Hz =	91

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.26	1540 T	0.05		0.21
K -B	0.21	1245 C	0.00		0.21
B -L	0.09	3254 C	0.01		0.08
L -S1	0.09	3254 C	0.01		0.08
S1-M	0.08	3254 C	0.01		0.07
M -C	0.10	4055 C	0.01		0.09
C -N	0.06	3697 C	0.04		0.02
N -O	0.35	5296 C	0.03		0.32
O -D	0.35	5512 C	0.03		0.32
Bottom Chords					
A -G	0.56	1370 C	0.02		0.54
G -F	0.81	1401 C	0.01		0.80
F -J	0.54	1198 T	0.20		0.34
J -S2	0.83	4056 T	0.68		0.15
S2-I	0.87	4056 T	0.68		0.19
I -S3	0.76	3448 T	0.57		0.19
S3-H	0.78	3448 T	0.57		0.21
H -E	0.91	4186 T	0.70		0.21
E -D	0.49	5081 T	0.36		0.13
Webs					
G -K	0.44	2415 C			
K -F	0.82	2584 T			
F -B	0.24	891 C			
B -J	0.73	2288 T			1 Br
J -L	0.12	443 C			
J -M	0.30	899 C			1 Br
I -M	0.05	209 T			
I -C	0.21	674 T			1 Br
H -C	0.20	650 T			
H -N	0.33	853 C			
N -E	0.32	1020 T			
E -O	0.17	559 T			

TL Defl -0.18" in A -G L/426
LL Defl -0.07" in A -G L/999

Hz Disp	LL	DL	TL
Jt D	0.15"	0.15"	0.29"

Shear // Grain in O -D 0.34

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

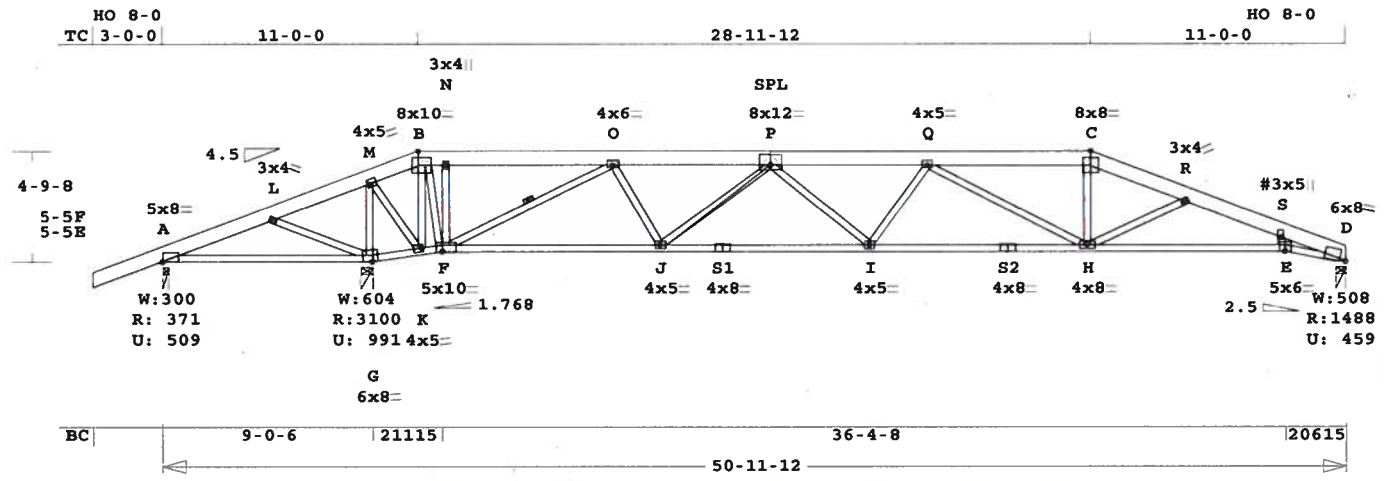
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
K	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	8.0x10.0	0.5	Ctr	0.98
L	LOCK	3.0x 4.0	Ctr	Ctr	0.36
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
M	LOCK	4.0x 5.0	Ctr	0.5	0.62
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
N	LOCK	8.0x12.0	0.6	1.5	0.97
O	LOCK	3.0x 4.0	Ctr	Ctr	0.36
D	LOCK	5.0x10.0	Ctr	1.1	1.00
G	LOCK	5.0x 6.0	-0.2	2.7	0.75
F	LOCK	6.0x 6.0	Ctr	-0.5	0.99
J	LOCK	4.0x 8.0	Ctr	Ctr	0.94
S2	LOCK	5.0x 8.0	Ctr	0.8	0.75
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	4.0x 8.0	Ctr	0.2	0.75
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	6.0x 8.0	-1.0	-0.5	0.86

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 5512 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R106*	1	SP	501112	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 431.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.25	2x 8	SP-2400
BC	0.84	2x 4	SP-#2
EX G -F		2x 4	SP-2400
WB	0.80	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

WB 1 rows CLB on F -0
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	372	510G	3- 0	1- 8
		Hz =	-76	
G	3100	991	6- 4	3- 7
D	1488	459	5- 8	1-12
		Hz =	77	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.23	1471 T	0.05	0.18	
L -M	0.25	2342 T	0.07	0.18	
M -B	0.06	1203 T	0.04	0.02	
B -N	0.05	713 T	0.02	0.03	
N -O	0.08	711 T	0.02	0.06	
O -P	0.06	2225 C	0.00	0.06	
P -Q	0.07	3434 C	0.01	0.06	
Q -C	0.08	2963 C	0.01	0.07	
C -R	0.06	3152 C	0.03	0.03	
R -S	0.24	3615 C	0.01	0.23	
S -D	0.24	3922 C	0.01	0.23	
Bottom Chords					
A -G	0.66	1265 C	0.04	0.62	
G -K	0.70	2242 C	0.02	0.68	
K -F	0.25	1101 C	0.00	0.25	
F -J	0.63	1782 T	0.29	0.34	
J -S1	0.76	3100 T	0.32	0.44	
S1-I	0.77	3100 T	0.52	0.25	
I -S2	0.83	3506 T	0.58	0.25	
S2-H	0.84	3506 T	0.58	0.26	
H -E	0.84	3454 T	0.58	0.26	
E -D	0.72	3507 T	0.58	0.14	
Webs					
L -G	0.35	1035 C			
G -M	0.43	1997 C			
M -K	0.56	1746 T			
K -B	0.48	1907 C			
B -F	0.49	1535 T			
F -N	0.06	278 C			
F -O	0.80	2810 C			1 Br
O -J	0.31	986 T			
J -P	0.63	1127 C			
P -I	0.14	449 T			
I -Q	0.05	216 T			
Q -H	0.56	623 C			
H -C	0.23	733 T			
H -R	0.20	544 C			
E -S	0.22	710 T			

TL Defl -0.31" in A -G L/316
LL Defl -0.13" in A -G L/757

Hz Disp	LL	DL	TL
Jt D	0.10"	0.11"	0.21"

Shear // Grain in G -K 0.48

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.78
L	LOCK	3.0x 4.0	Ctr	Ctr	0.36
M	LOCK	4.0x 5.0-0.5	0.2		0.98
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
N	LOCK	3.0x 4.0	Ctr	Ctr	0.36
O	LOCK	4.0x 6.0-0.1	0.5		0.97
P	LOCK	8.0x12.0	Ctr	1.5	0.99
Q	LOCK	4.0x 5.0	Ctr	0.5	0.61
C	LOCK	8.0x 8.0	Ctr	Ctr	0.84
R	LOCK	3.0x 4.0	Ctr	Ctr	0.36
S#	LOCK	3.0x 5.0-0.6	1.4		0.41
D	LOCK	6.0x 8.0	Ctr	1.1	1.00
G	LOCK	6.0x 8.0-1.2	3.3		0.86
K	LOCK	4.0x 5.0	Ctr	Ctr	0.72
F	LOCK	5.0x10.0	Ctr	-1.0	0.97
J	LOCK	4.0x 5.0	Ctr	Ctr	0.55
S1	LOCK	4.0x 8.0	Ctr	0.2	0.75
I	LOCK	4.0x 5.0	Ctr	Ctr	0.51
S2	LOCK	4.0x 8.0	Ctr	0.2	0.83
H	LOCK	4.0x 8.0	Ctr	Ctr	0.56
E	LOCK	5.0x 6.0	Ctr	-1.0	0.75

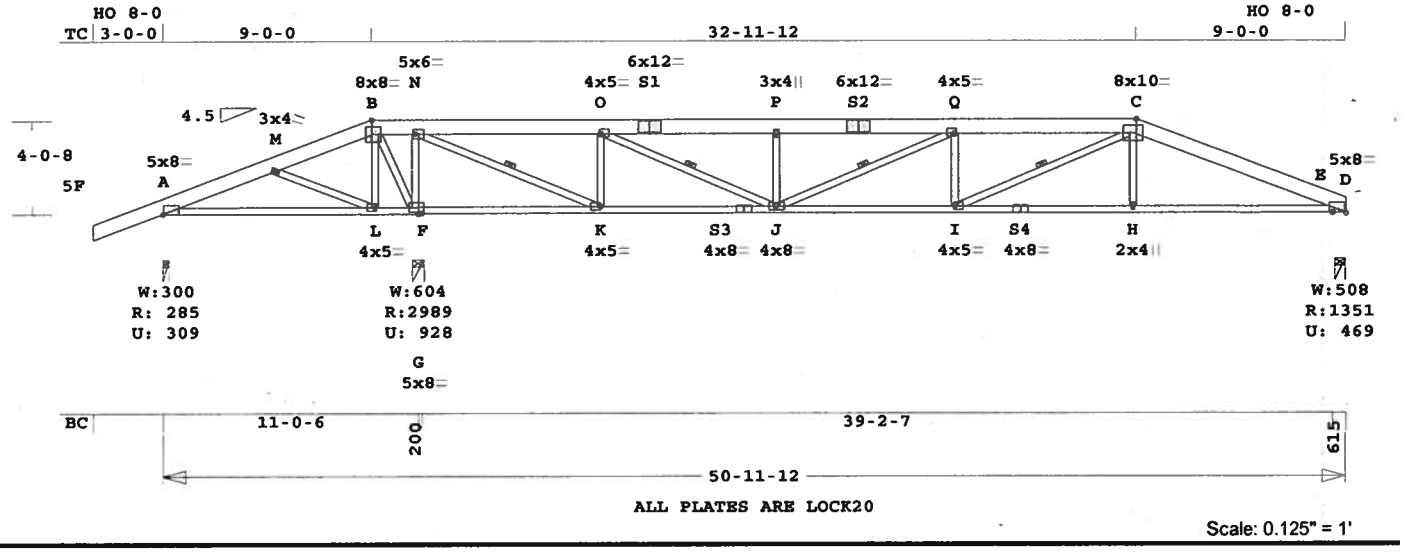
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3922 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R105	1	SP	501112	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish

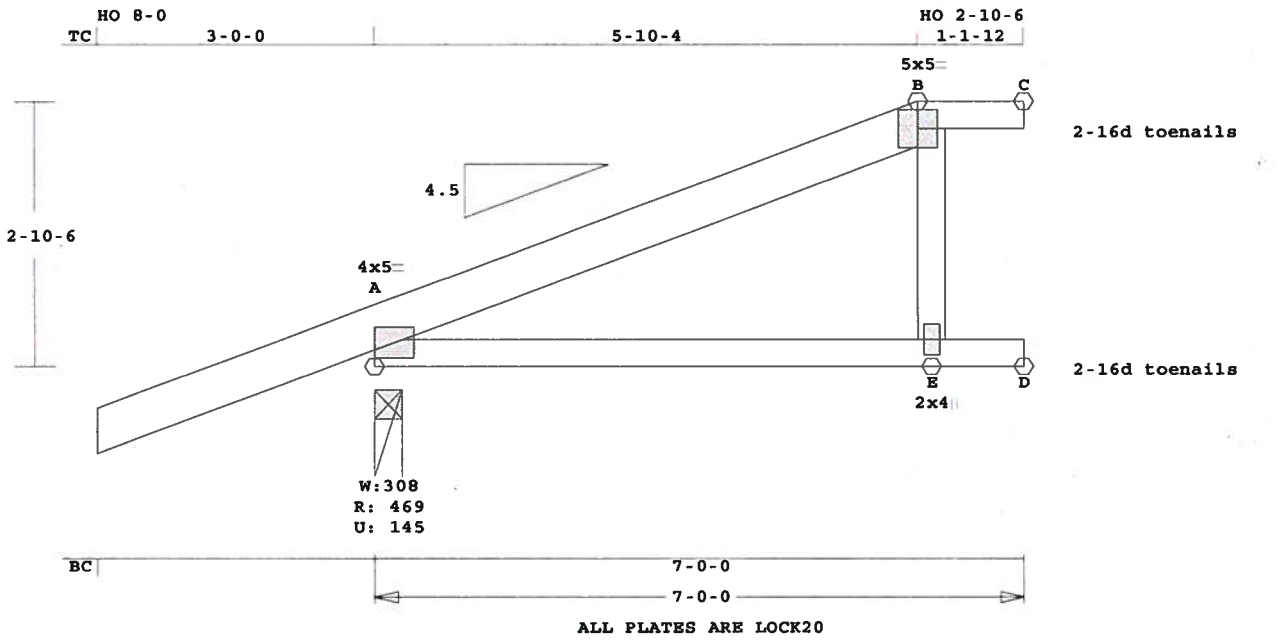


Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 419.0 LBS

```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

     CSI -Size- ----Lumber----
TC  0.21  2x 8  SP-2400
BC  0.74  2x 4  SP-#2
WB  0.65  2x 4  SP-#3
EX N -K   2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 50- 4-13
 BC Cont.      0- 0- 0 50- 4-13
 WB 1 rows CLB on N -K
 WB 1 rows CLB on O -J
 WB 1 rows CLB on J -Q
 WB 1 rows CLB on I -C
Attach CLB with (2)-10d nails
  at each web.

Loading   Live   Dead  (psf)
TC         20.0   10.0
BC          0.0   10.0
Total      20.0   20.0   40.0
Spacing                  24.0"
Lumber Duration Factor   1.25
Plate  Duration Factor   1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A     286   309G 3- 0  1- 8
                 Hz =   -62
G    2990   928  6- 4  3- 5
D    1351   470  5- 8  1-10
                 Hz =    63
G = Gravity Uplift

Membr  CSI   P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -M  0.12  1133 T  0.04  0.08
M -B  0.13  1745 T  0.05  0.08
B -N  0.21  2095 T  0.07  0.14
N -O  0.14  1029 C  0.00  0.14
O -S1 0.06  2883 C  0.03  0.03
S1-P  0.07  2883 C  0.01  0.06
P -S2 0.08  2883 C  0.01  0.07
S2-Q  0.08  2883 C  0.01  0.07
Q -C  0.09  3407 C  0.01  0.08
C -D  0.11  2789 C  0.01  0.10
--------Bottom Chords---------
A -L  0.30  1000 C  0.00  0.30
L -G  0.30  1669 C  0.00  0.30
G -K  0.32  2181 C  0.01  0.31
K -S3 0.42  1029 T  0.11  0.31
S3-J  0.40  1029 T  0.11  0.29
J -I  0.74  3407 T  0.57  0.17
I -S4 0.57  2593 T  0.43  0.14
S4-H  0.62  2593 T  0.43  0.19
H -D  0.62  2599 T  0.43  0.19
------------Webs------------
M -L  0.25   708 C
L -B  0.11   377 T
B -G  0.21   877 C
G -N  0.46  2119 C
N -K  0.64  3516 T        1 Br
K -O  0.28  1288 C
O -J  0.65  2049 T        1 Br
J -P  0.10   477 C
J -Q  0.17   578 C        1 Br
I -Q  0.05   210 T
I -C  0.28   885 T        1 Br
H -C  0.08   324 T

TL Defl  -0.17" in A -L  L/761
LL Defl  -0.22" in J -I  L/999
Shear // Grain  in B -N   0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  5.0x 8.0 Ctr 0.6 0.78
M  LOCK  3.0x 4.0 Ctr Ctr 0.36
B  LOCK  8.0x 8.0-1.0 Ctr 0.84
N  LOCK  5.0x 6.0 Ctr Ctr 0.86
O  LOCK  4.0x 5.0 Ctr 0.5 0.97
S1 LOCK  6.0x12.0 Ctr Ctr 0.91
P  LOCK  3.0x 4.0 Ctr Ctr 0.36
S2 LOCK  6.0x12.0 Ctr Ctr 0.91
Q  LOCK  4.0x 5.0 Ctr 0.5 0.64
C  LOCK  8.0x10.0 Ctr Ctr 0.84
D  LOCK  5.0x 8.0 Ctr 0.8 0.81
L  LOCK  4.0x 5.0 Ctr Ctr 0.60
G  LOCK  5.0x 8.0-1.0 Ctr 0.81
K  LOCK  4.0x 5.0-1.0 Ctr 1.00
S3 LOCK  4.0x 8.0 Ctr 0.2 0.75
J  LOCK  4.0x 8.0 Ctr Ctr 0.87
I  LOCK  4.0x 5.0 Ctr Ctr 0.60
S4 LOCK  4.0x 8.0 Ctr 0.2 0.75
H  LOCK  2.0x 4.0 Ctr Ctr 0.54


NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
Prevent truss rotation at all
  bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:    B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  TC Dead Load:        4.0 psf
  BC Dead Load:        6.0 psf
Max comp. force     3407 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
    Designer)
    Gary Dounson, PE 35054
    Gary Dounson & Associates,
    Inc.
    2830 NW 41st Street Suite D
    Gainesville, FL 32606
    (352)375-8593
    CA 5201
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R104*	1	HHIP	70000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 47.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 6	SP-#2
EX B -C		2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.08	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	470	146	3- 8	1- 8
			Hz =	103
D	182	110	3- 8	1- 8
C	120	1	3- 8	1- 8
			Hz =	55

Membr	CSI	P Lbs	Axl-CSI-Bnd	OH Loading

Top Chords

A -B	0.45	89	C 0.00	0.45
B -C	0.13	55	T 0.00	0.13

Bottom Chords

A -E	0.36	18	C 0.00	0.36
E -D	0.35	0	T 0.00	0.35

Webs

E -B	0.08	327	T

TL Defl	-0.19" in A -E		L/397
LL Defl	-0.09" in A -E		L/883
Hz Disp	LL	DL	TL
Jt C	0.09"	0.02"	0.07"
Shear // Grain		in E -D	0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
B	LOCK	5.0x 5.0	Ctr	Ctr	0.49
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

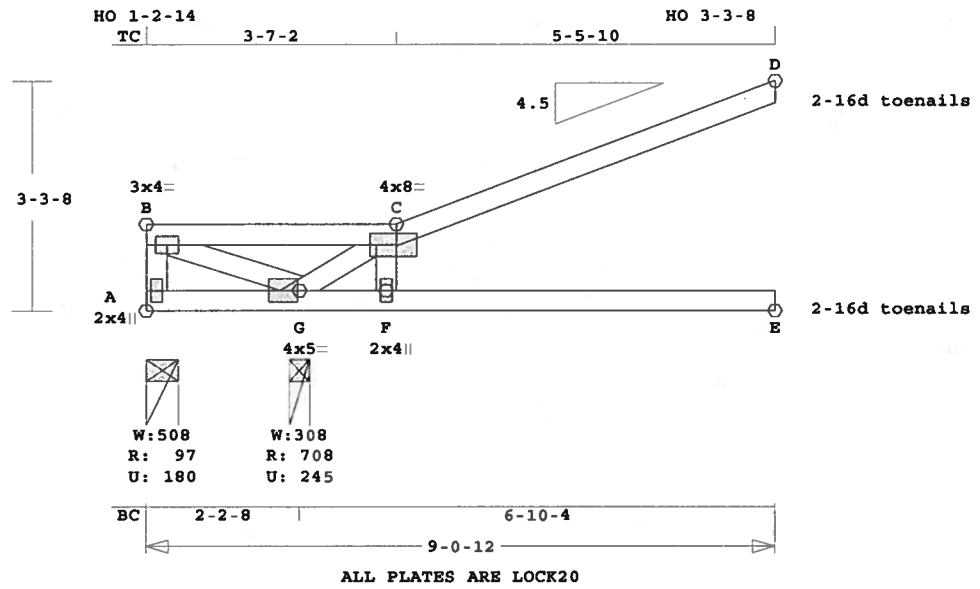
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 118 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R102*	1	SP	90012	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.19	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	97	180G	5- 8	1- 0
			Hz =	114
G	709	245	3- 8	1- 0
D	145	105	3- 8	1- 0
			Hz =	66
E	97	0	3- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
B -C	0.35	674 C	0.09	0.26
C -D	0.34	98 C	0.00	0.34
--------Bottom Chords---------				
A -G	0.07	220 C	0.00	0.07
G -F	0.15	31 T	0.00	0.15
F -E	0.16	0 T	0.00	0.16
-------------Webs-------------				
A -B	0.05	243 C	WindLd	
B -G	0.19	759 T		
G -C	0.13	700 C		
F -C	0.05	206 T		

LL Defl -0.03" in F -E L/999
TL Defl -0.06" in F -E L/999
Shear // Grain in C -D 0.30

Hz Disp	LL	DL	TL
Jt D	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.52
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	4.0x 5.0	0.5	Ctr	0.97
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

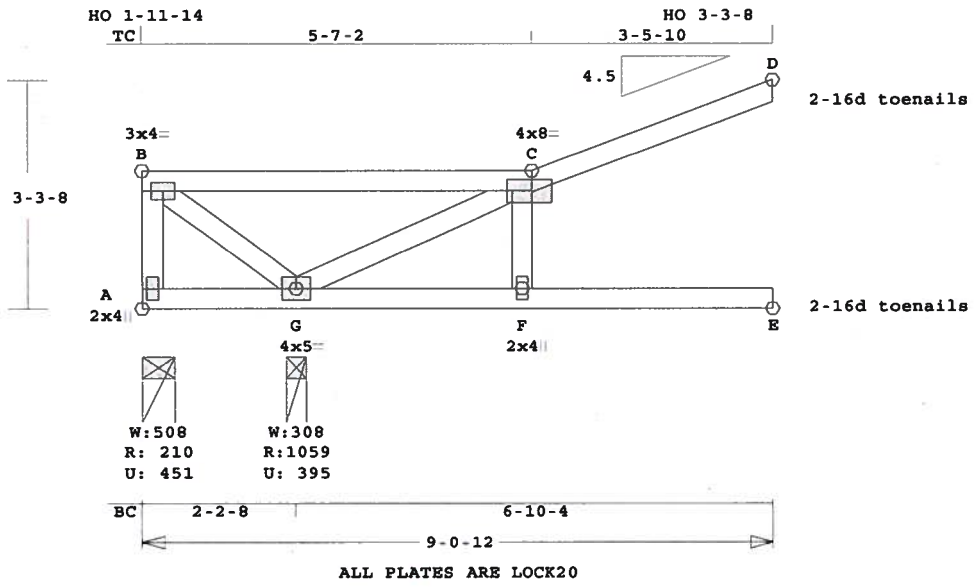
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 700 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R101*	1	SP	90012	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.1 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.41	2x 4	SP-#2
BC	0.10	2x 4	SP-#2
WB	0.26	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	210	452G	5- 8	1- 0
			Hz =	100
G	1059	395	3- 8	1- 2
D	72	54	3- 8	1- 0
			Hz =	64
E	71	0	3- 8	1- 0

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.41	779 C	0.12 0.29
C -D	0.26	59 T	0.00 0.26
--------Bottom Chords---------			
A -G	0.10	165 C	0.00 0.10
G -F	0.10	11 T	0.00 0.10
F -E	0.10	0 T	0.00 0.10
------------Webs------------			
A -B	0.14	498 C	WindLd
B -G	0.26	1063 T	
G -C	0.19	804 C	
F -C	0.03	127 T	

LL Defl -0.01" in F -E L/999
TL Defl -0.03" in F -E L/999
Shear // Grain in B -C 0.24

Hz Disp	LL	DL	TL
Jt D	0.00"	0.00"	0.00"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.60
C	LOCK	4.0x 8.0-0.5	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.48
G	LOCK	4.0x 5.0	Ctr	Ctr	0.54
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

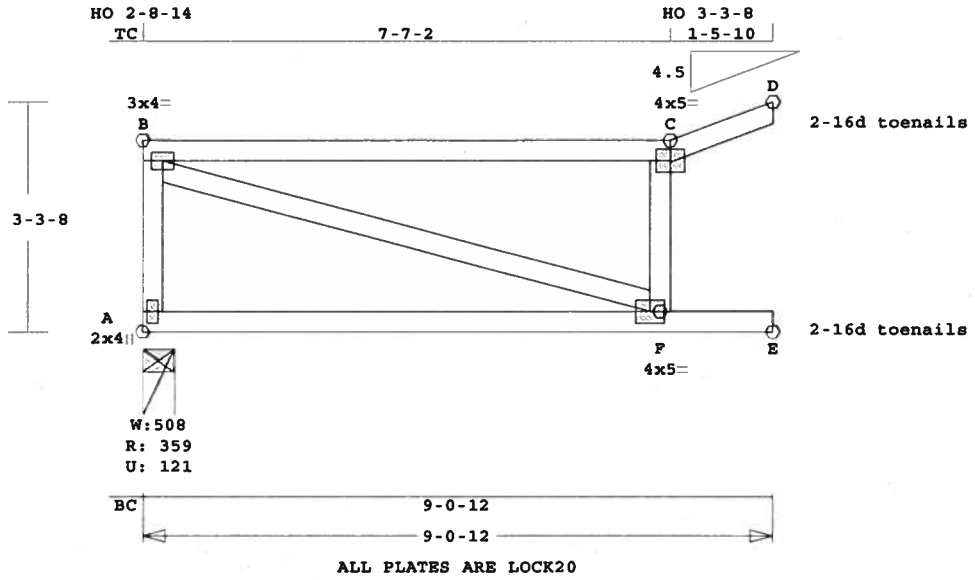
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 933 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R100*	1	SP	90012	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 56.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.87	2x 4	SP-#2
BC	0.57	2x 4	SP-#2
WB	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	360	122	5- 8	1- 0
			Hz =	87
D	203	3	3- 8	1- 0
			Hz =	62
E	203	126	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.87	71 C	0.00 0.87
C -D	0.32	71 T	0.00 0.32
--------Bottom Chords---------			
A -F	0.57	110 C	0.00 0.57
F -E	0.51	0 T	0.00 0.51
------------Webs------------			
A -B	0.10	246 C	WindLd
B -F	0.04	135 T	
F -C	0.07	294 T	

LL Defl -0.21" in A -F L/482
TL Defl -0.48" in A -F L/209
Shear // Grain in B -C 0.31

Hz Disp	LL	DL	TL
Jt D	0.01"	0.01"	0.01"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.78

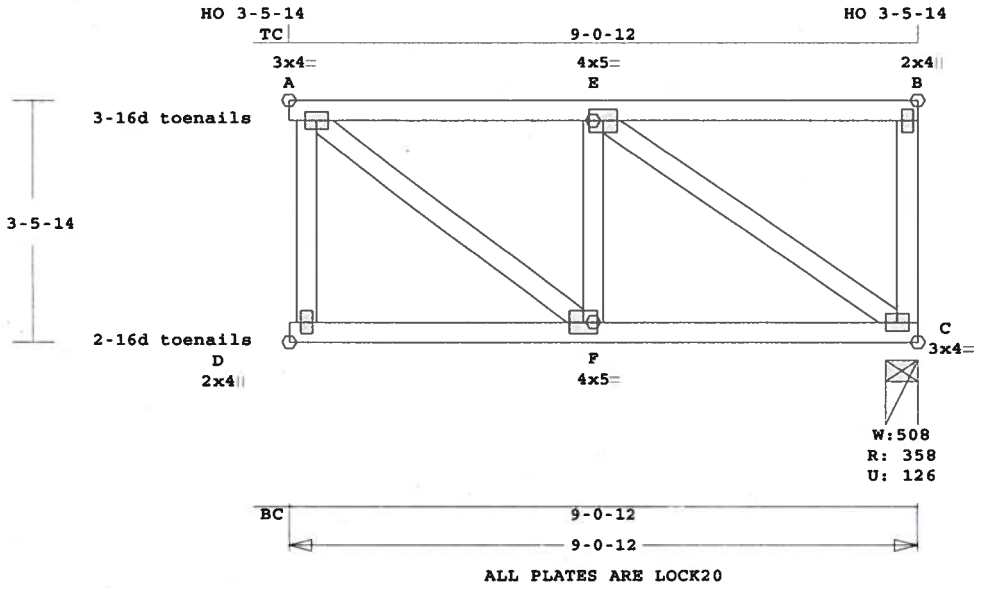
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 246 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R99*	1	FLAT	90012	30514	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.15	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	324	141	3- 8	1- 0
D	67	0	3- 8	1- 0
			Hz =	-91
C	358	127	5- 8	1- 0
			Hz =	65

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.24	288 C	0.00 0.24
E -B	0.24	0 T	0.00 0.24
--------Bottom Chords---------			
D -F	0.13	76 C	0.00 0.13
F -C	0.16	288 T	0.03 0.13
------------Webs------------			
D -A	0.10	0 T	WindLd
A -F	0.11	364 T	
F -E	0.04	167 T	
E -C	0.15	352 C	
C -B	0.03	143 T	

LL Defl -0.01" in F -C L/999
TL Defl -0.02" in F -C L/999
Shear // Grain in E -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.52
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	3.0x 4.0	Ctr	Ctr	0.52

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

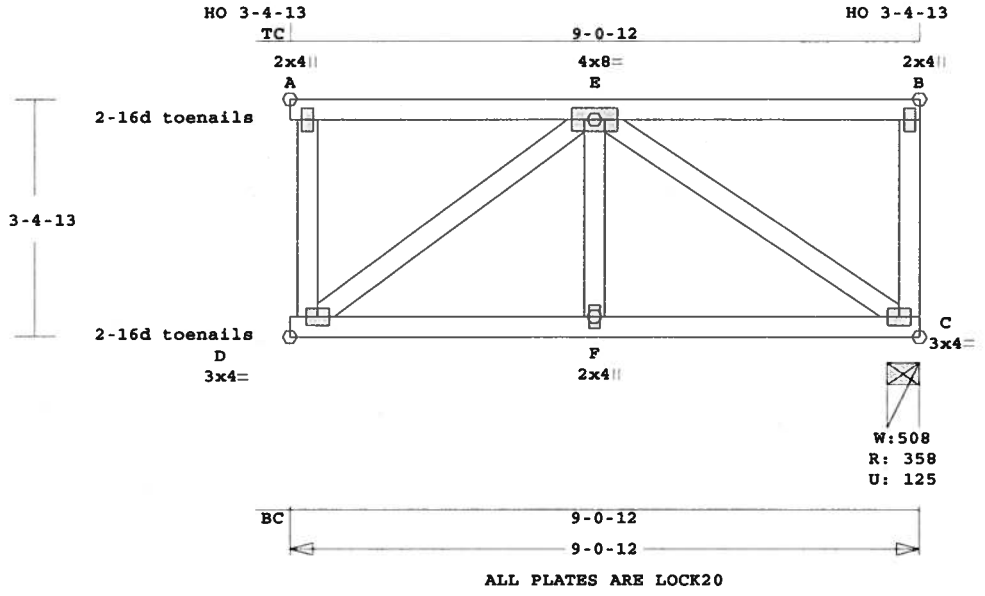
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 352 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R98	1	FLAT	90012	30413	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.15	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	101	62	3- 8	1- 0
D	257	59	3- 8	1- 0
			Hz = -88	
C	358	126	5- 8	1- 0
			Hz = 63	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.24	74 T	0.00 0.24
E -B	0.24	0 T	0.00 0.24
--------Bottom Chords---------			
D -F	0.16	296 T	0.03 0.13
F -C	0.16	296 T	0.03 0.13
------------Webs------------			
D -A	0.09	0 T	WindLd
D -E	0.14	370 C	
F -E	0.05	213 T	
E -C	0.15	359 C	
C -B	0.03	143 T	

LL Defl -0.01" in F -C L/999
TL Defl -0.03" in F -C L/999
Shear // Grain in E -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	4.0x 8.0	Ctr	Ctr	0.52
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	3.0x 4.0	Ctr	Ctr	0.52
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	3.0x 4.0	Ctr	Ctr	0.52

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

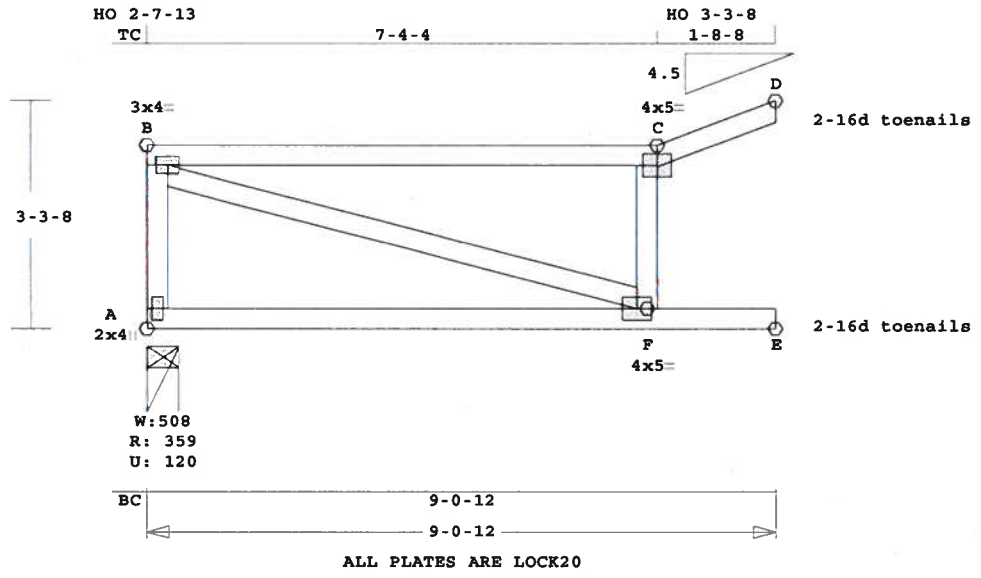
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To: FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Provide drainage to prevent water ponding.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 370 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R97*	1	SP	90012	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.8 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.85	2x 4	SP-#2
BC	0.60	2x 4	SP-#2
WB	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	360	121	5- 8	1- 0
			Hz =	88
D	184	26	3- 8	1- 0
			Hz =	62
E	193	104	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.85	79 C	0.00 0.85
C -D	0.35	65 T	0.00 0.35
--------Bottom Chords---------			
A -F	0.60	117 C	0.00 0.60
F -E	0.56	0 T	0.00 0.56
------------Webs------------			
A -B	0.10	244 C	WindLd
B -F	0.04	141 T	
F -C	0.06	268 T	

LL Defl -0.21" in A -F L/467
TL Defl -0.48" in A -F L/206
Shear // Grain in B -C 0.31

Hz Disp	LL	DL	TL
Jt D	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.78

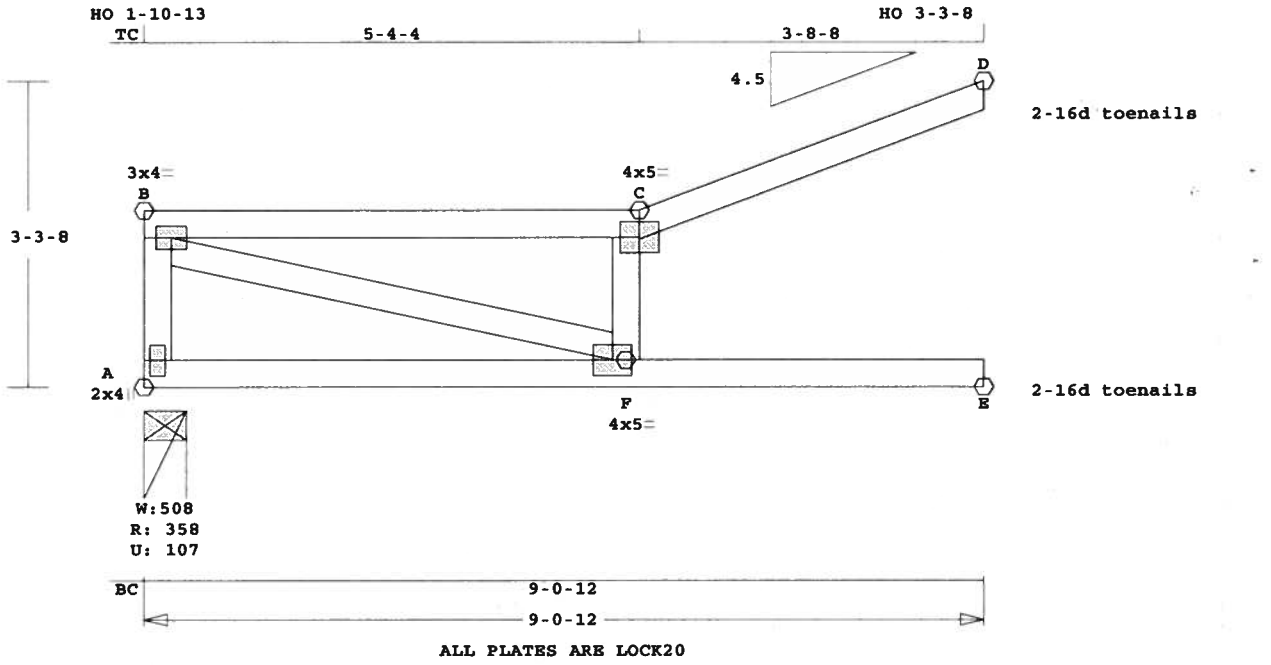
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 244 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R96*	1	SP	90012	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.2 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.72	2x 4	SP-#2
BC	0.76	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0-12
BC Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	359	108	5- 8	1- 0
			Hz =	102
D	217	108	3- 8	1- 0
			Hz =	64
E	149	27	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.72	156 C	0.00 0.72
C -D	0.59	104 C	0.00 0.59
--------Bottom Chords---------			
A -F	0.76	172 C	0.00 0.76
F -E	0.76	0 T	0.00 0.76
------------Webs------------			
A -B	0.06	225 C	WindLd
B -F	0.05	200 T	
F -C	0.05	204 T	

LL Defl -0.22" in A -F L/450
TL Defl -0.46" in A -F L/216
Shear // Grain in B -C 0.28
Hz Disp LL DL TL
Jt D 0.02" 0.02" 0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	4.0x 5.0	Ctr	Ctr	0.77

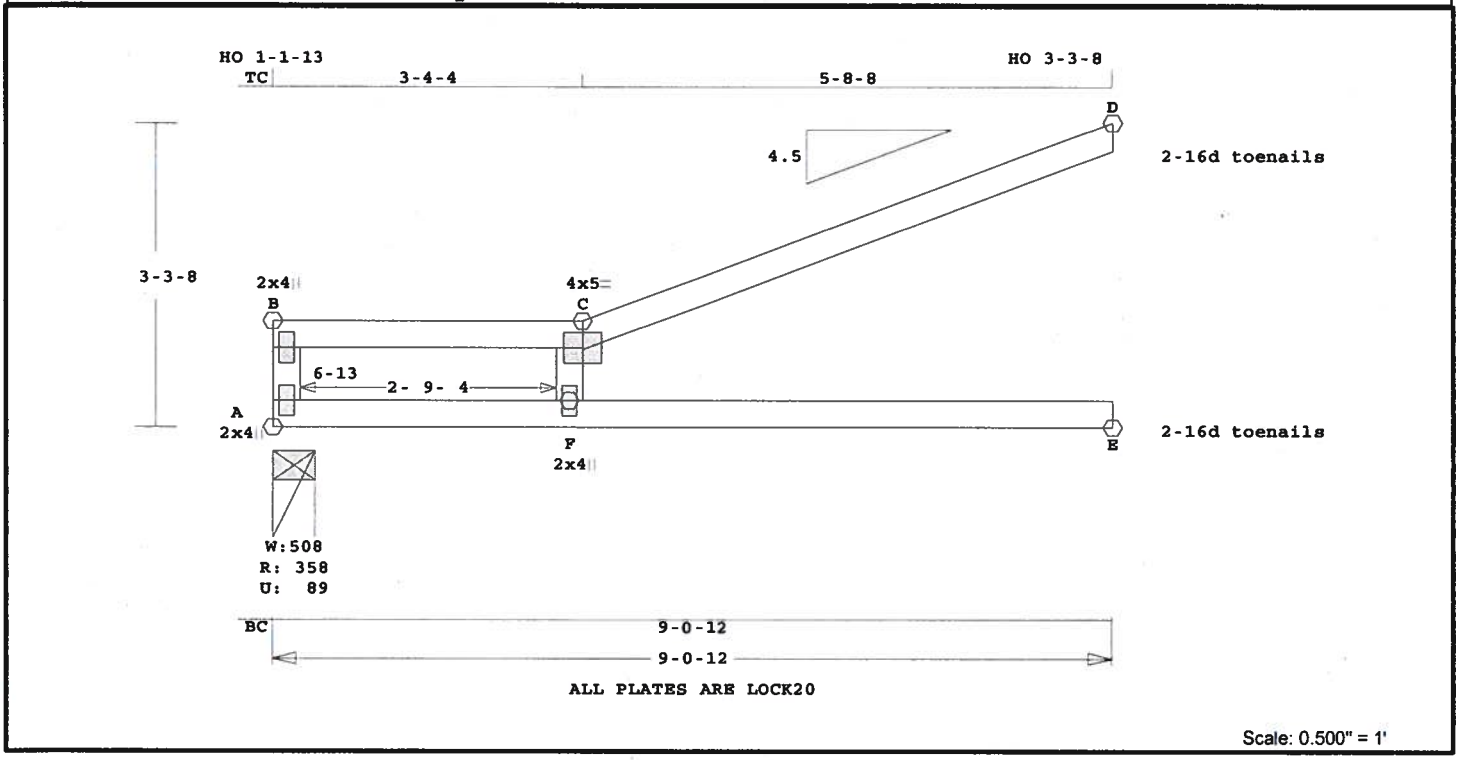
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 225 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R95*	1	SP	90012	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.71	2x 4	SP-#2
BC	0.78	2x 4	SP-#2
WB	0.04	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0-12
BC	Cont.	0- 0- 0	9- 0-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	359	89	5- 8	1- 0
			Hz = 115	
D	235	136	3- 8	1- 0
			Hz = 66	
E	158	1	3- 8	1- 0

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
B -C	0.28	346 C	0.00 0.28
C -D	0.71	129 C	0.00 0.71
--------Bottom Chords---------			
A -F	0.49	432 C	0.05 0.44
F -E	0.78	0 T	0.00 0.78
------------Webs------------			
A -B	0.04	201 C	0.04 0.00
F -C	0.04	171 T	0.04 0.00

LL Defl -0.21" in F -E L/464
TL Defl -0.46" in F -E L/218
Shear // Grain in C -D 0.30

Hz Disp	LL	DL	TL
Jt D	0.03"	0.04"	0.07"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	4.0x 5.0	Ctr	Ctr	0.66
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

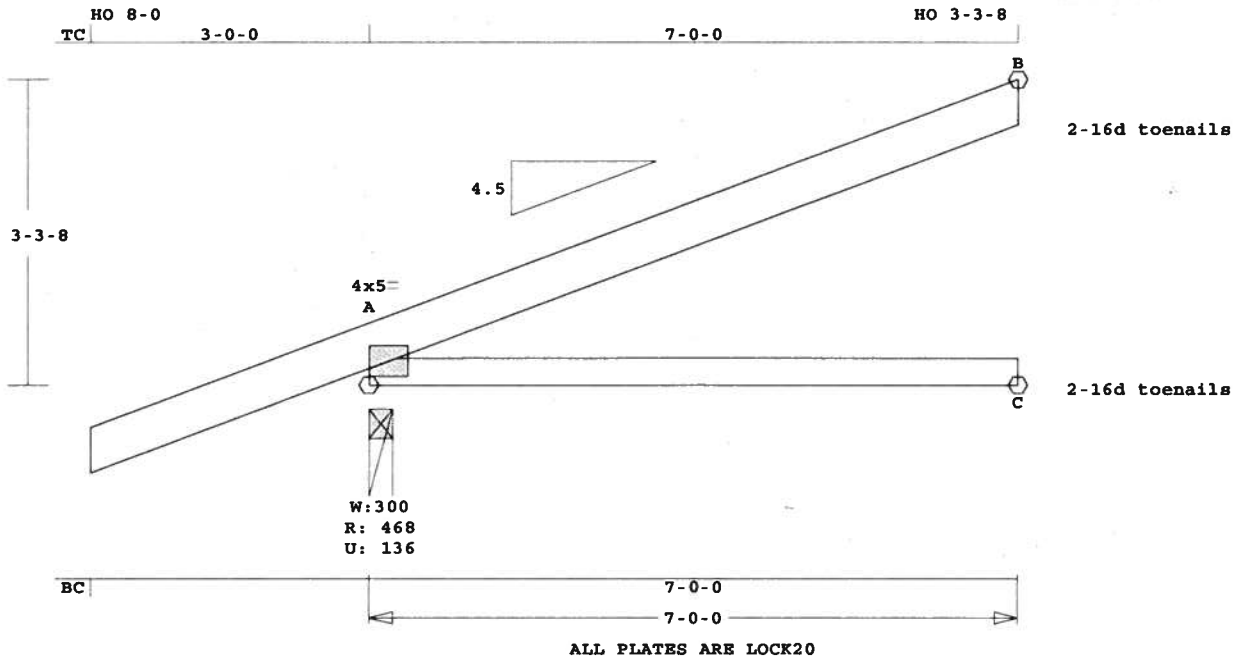
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 432 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R94*	11	JACK	70000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 6	SP-#2
BC	0.24	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	468	137	3- 0	1- 8
			Hz =	118
C	117	0	3- 8	1- 8
B	217	144	3- 8	1- 8
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.29	127	C 0.00 0.29
--------Bottom Chords---------			
A -C	0.24	0	T 0.00 0.24
TL Defl	-0.13"	in A -C	L/600
LL Defl	-0.06"	in A -C	L/999
Shear // Grain		in A -B	0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 127 Lbs
Quality Control Factor 1.25

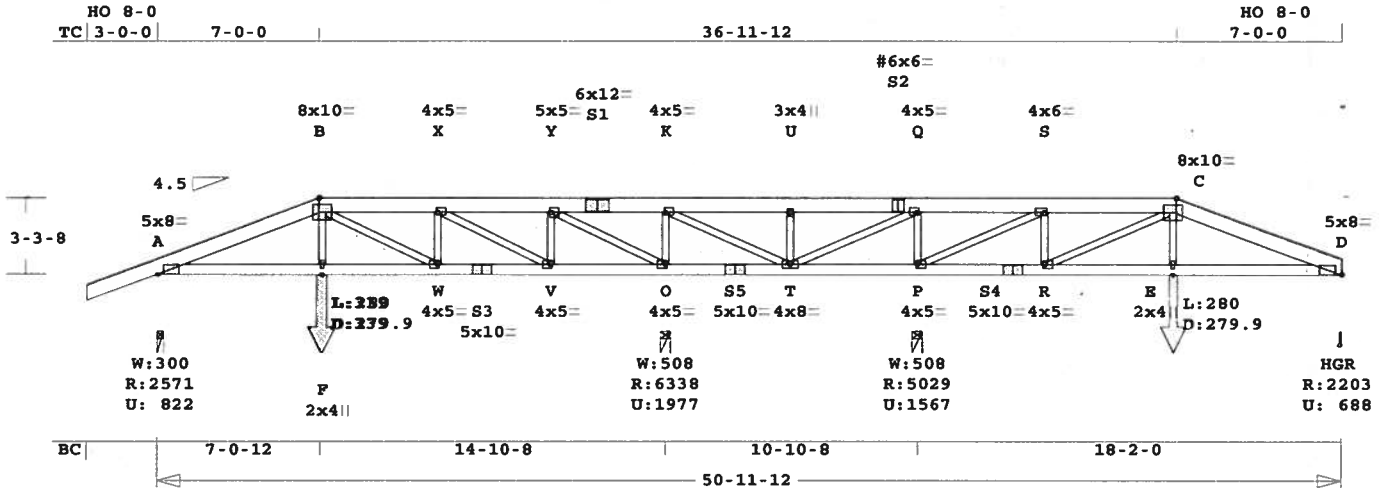
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R93*	1*2P	HIPP	501112	4.5	3- 0- 0	0	
+Office and Care Center, Hospice, M.M. Parrish								

Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R93	1*2P	HIPP	501112	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
+LOAD ASSIST USED TO CALCULATE LOADS+

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 459.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.19	2x 8	SP-2400
BC	0.55	2x 6	SP-#2
WB	0.57	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	50-11-12
BC Cont.		0- 0- 0	50-11-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	51.0'
BC V	0	20	0.0'	51.0'
BC V	70	70	8.1'	22.1'
BC V	91	91	22.1'	34.1'
BC V	70	70	34.1'	50.1'
TC V	50	25	7.0'	44.0'
BC V	0	25	7.1'	43.8'
BC V	339	339	7.1'	CL-LB
BC V	280	280	7.1'	CL-LB
BC V	280	280	43.8'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2571	823	3- 0	1- 8
		Hz =	-48	
O	6339	1977	5- 8	3- 8
P	5029	1568	5- 8	2-12
D	2204	688	3- 8	1- 8
		Hz =	49	

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -B	0.04	5636	C	0.01 0.03
B -X	0.07	5274	C	0.01 0.06
X -Y	0.05	2160	C	0.00 0.05
Y -S1	0.05	3089	T	0.05 0.00
S1-K	0.19	3089	T	0.05 0.14
K -U	0.15	1016	T	0.01 0.14
U -S2	0.06	1016	T	0.01 0.05
S2-Q	0.13	1016	T	0.01 0.12
Q -S	0.15	1870	T	0.03 0.12
S -C	0.08	2740	C	0.00 0.08
C -D	0.05	4300	C	0.00 0.05
--------Bottom Chords---------				
A -F	0.44	5271	T	0.35 0.09
F -W	0.55	5201	T	0.34 0.21
W -S3	0.55	5274	T	0.35 0.20
S3-V	0.48	5274	T	0.35 0.13
V -O	0.54	2160	T	0.14 0.40
O -S5	0.40	3089	C	0.00 0.40
S5-T	0.22	3089	C	0.00 0.22
T -P	0.37	1870	C	0.00 0.37
P -S4	0.55	2740	T	0.18 0.37
S4-R	0.38	2740	T	0.18 0.20
R -E	0.46	3987	T	0.26 0.20
E -D	0.47	4061	T	0.27 0.20
------------Webs------------				
F -B	0.23	1517	T	
B -W	0.01	81	T	
W -X	0.16	1234	T	
X -V	0.34	3568	C	
V -Y	0.39	2488	T	
Y -O	0.57	6015	C	
O -K	0.17	2134	C	
K -T	0.37	2324	T	
T -U	0.04	390	C	
T -Q	0.15	958	T	
P -Q	0.11	1419	C	
P -S	0.52	5157	C	
R -S	0.24	1735	T	
R -C	0.13	1368	C	
E -C	0.24	1571	T	

TL Defl -0.17" in F -W L/999
LL Defl -0.08" in F -W L/999
Shear // Grain in O -S5 0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	Ctr	0.90
B	LOCK	8.0x10.0	Ctr	Ctr	0.84
X	LOCK	4.0x 5.0	Ctr	0.5	0.77
Y	LOCK	5.0x 5.0	Ctr	Ctr	0.82
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
K	LOCK	4.0x 5.0	Ctr	0.5	0.63
U	LOCK	3.0x 4.0	Ctr	Ctr	0.36
S2#	LOCK	6.0x 6.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	Ctr	0.5	0.63
S	LOCK	4.0x 6.0	Ctr	0.5	0.93
C	LOCK	8.0x10.0	Ctr	Ctr	0.84
D	LOCK	5.0x 8.0	Ctr	Ctr	0.90
F	LOCK	2.0x 4.0	Ctr	Ctr	0.81
W	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
V	LOCK	4.0x 5.0	Ctr	Ctr	0.60
O	LOCK	4.0x 5.0	Ctr	Ctr	0.97
S5	LOCK	5.0x10.0	Ctr	Ctr	0.82
T	LOCK	4.0x 8.0	Ctr	Ctr	0.57
P	LOCK	4.0x 5.0	Ctr	Ctr	0.84
S4	LOCK	5.0x10.0	Ctr	Ctr	0.82
R	LOCK	4.0x 5.0	Ctr	Ctr	0.60
E	LOCK	2.0x 4.0	Ctr	Ctr	0.84

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

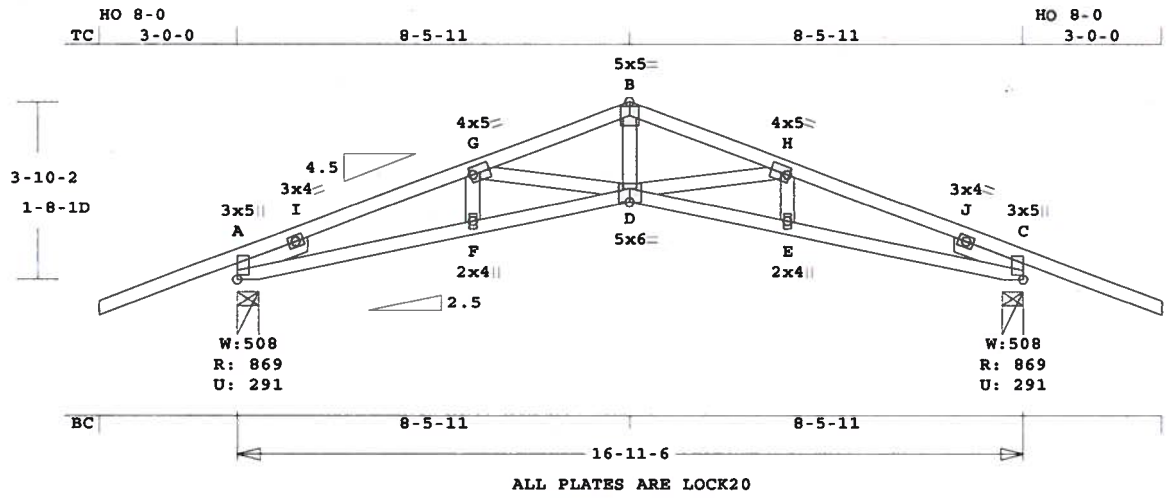
	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	2	12	24	0
BC	2	12	24	0
WB	1	8	8	

Web Connection Exception --
Use 4" spacing for screws or nails on the following webs
Y- O P- S
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 6015 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R92*	5	SCISSOR	161106	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 104.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
BC	0.66	2x 4	SP-#2
WB	0.27	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16-11- 6
BC	Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	870	292	5- 8	1- 8
C	870	292	5- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.43	1314 C	0.01	0.42
I -G	0.44	1777 C	0.02	0.42
G -B	0.22	1623 C	0.10	0.12
B -H	0.22	1623 C	0.10	0.12
H -J	0.44	1777 C	0.02	0.42
J -C	0.43	1314 C	0.01	0.42
--------Bottom Chords---------				
A -F	0.66	1657 T	0.27	0.39
F -D	0.48	1681 T	0.28	0.20
D -E	0.48	1681 T	0.28	0.20
E -C	0.66	1657 T	0.27	0.39
------------Webs------------				
F -G	0.02	107 T		
G -D	0.03	152 T		
D -B	0.27	861 T		
D -H	0.03	152 T		
E -H	0.02	107 T		
----------Sliders----------				
A -I	0.11	698 C		
J -C	0.11	698 C		

TL Defl -0.24" in D -E L/791
LL Defl -0.12" in D -E L/999

Hz Disp	LL	DL	TL
Jt C	0.07"	0.07"	0.14"

Shear // Grain in I -I 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

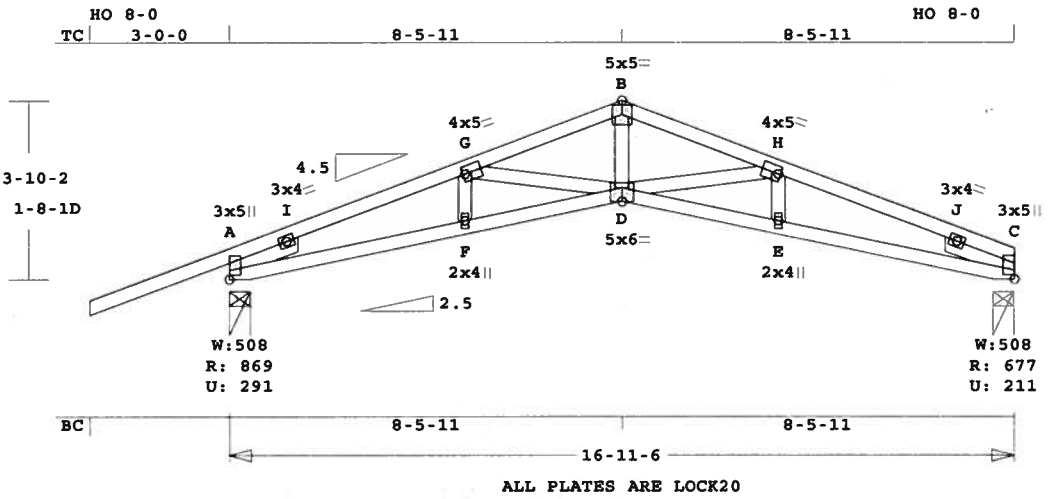
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.94
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0-1.5	0.4		0.94
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 6.0	Ctr-1.1		0.55
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1777 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R91*	2	SCISSOR	161106	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 98.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
BC	0.66	2x 4	SP-#2
WB	0.27	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	870	292	5- 8	1- 8
C	678	212	5- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -I	0.43	1314	C	0.01	0.42
I -G	0.44	1777	C	0.02	0.42
G -B	0.22	1623	C	0.10	0.12
B -H	0.22	1623	C	0.10	0.12
H -J	0.44	1777	C	0.02	0.42
J -C	0.43	1314	C	0.01	0.42
Bottom Chords					
A -F	0.66	1657	T	0.27	0.39
F -D	0.48	1681	T	0.28	0.20
D -E	0.48	1681	T	0.28	0.20
E -C	0.66	1657	T	0.27	0.39
Webs					
F -G	0.02	107	T		
G -D	0.03	152	T		
D -B	0.27	861	T		
D -H	0.03	152	T		
E -H	0.02	107	T		
Sliders					
A -I	0.11	698	C		
J -C	0.11	698	C		

TL Defl -0.24" in D -E L/791
LL Defl -0.12" in D -E L/999

Hz Disp	LL	DL	TL
Jt C	0.07"	0.07"	0.14"

Shear // Grain in I -I 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

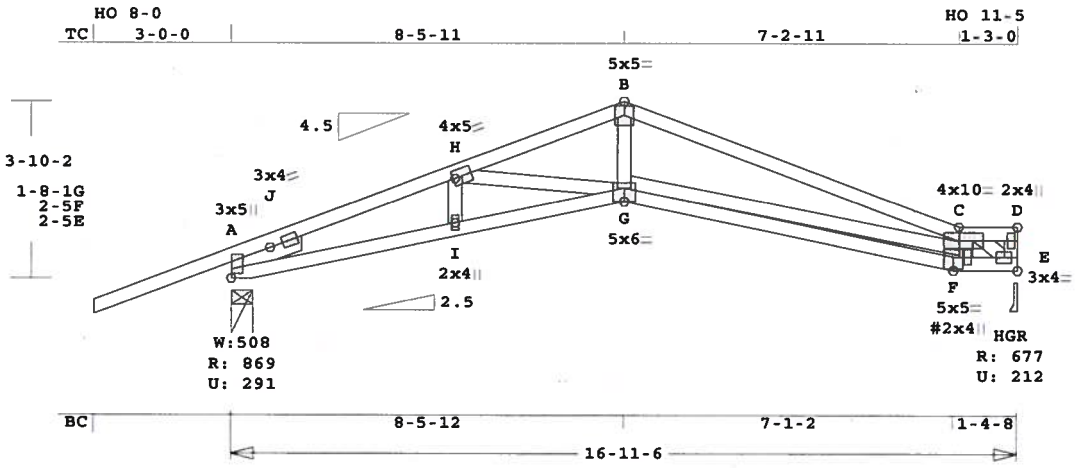
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.94
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
G	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 5.0	-1.5	0.4	0.94
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	5.0x 6.0	Ctr	-1.1	0.55
E	LOCK	2.0x 4.0	Ctr	Ctr	0.38

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1777 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R90*	1	SP	161106	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 104.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.59	2x 4	SP-#2
BC	0.65	2x 4	SP-#2
WB	0.26	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	16-11- 6
BC	Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	870	291	5- 8	1- 8
			Hz =	22
E	678	213	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -J	0.41	1297 C	0.01	0.40
J -H	0.41	1747 C	0.01	0.40
H -B	0.34	1716 C	0.12	0.22
B -C	0.59	1714 C	0.06	0.53
C -D	0.10	0 T	0.00	0.10
--------Bottom Chords---------				
A -I	0.65	1625 T	0.27	0.38
I -G	0.50	1653 T	0.27	0.23
G -F	0.40	1304 T	0.14	0.26
F -E	0.27	1306 T	0.14	0.13
------------Webs------------				
I -H	0.02	108 T		
H -G	0.02	79 T		
G -B	0.26	814 T		
G -C	0.16	331 T		
F -C	0.06	248 T		
C -E	0.23	1466 C		
E -D	0.00	75 C		
----------Sliders----------				
A -J	0.11	678 C		

TL Defl -0.26" in G -F L/760
LL Defl -0.11" in G -F L/999

Hz Disp	LL	DL	TL
Jt E	0.05"	0.05"	0.11"

Shear // Grain in J -J 0.37

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.92
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x10.0	1.0	Ctr	0.88
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
I	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	5.0x 6.0	Ctr	-1.1	0.60
F	LOCK	5.0x 5.0	Ctr	1.3	0.54
F#	LOCK	2.0x 4.0	0.4	0.1	0.40
E	LOCK	3.0x 4.0	Ctr	Ctr	0.75

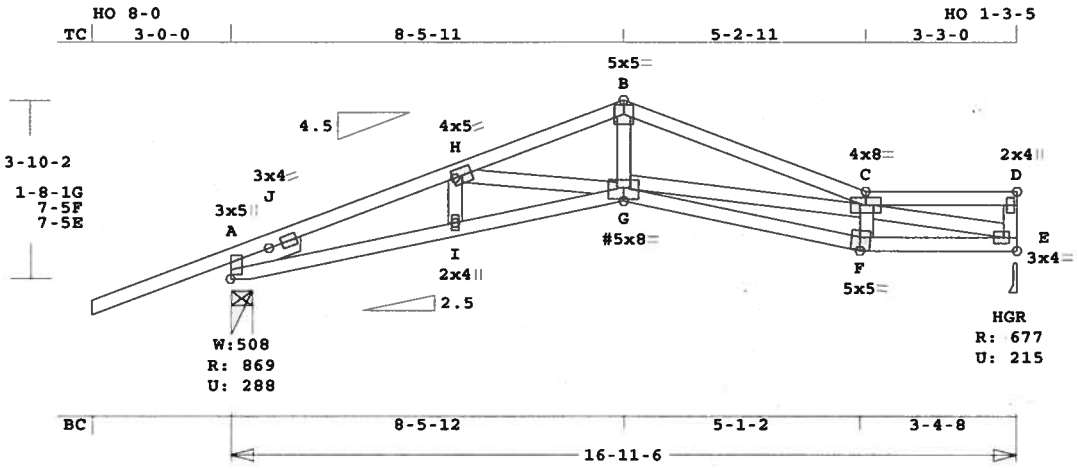
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1747 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R89*	1	SP	161106	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 104.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
BC	0.67	2x 4	SP-#2
WB	0.41	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	870	289	5- 8	1- 8
			Hz =	55
E	678	215	3- 8	1- 8
			Hz =	25

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -J	0.43	1319 C	0.01	0.42
J -H	0.44	1781 C	0.02	0.42
H -B	0.24	1615 C	0.11	0.13
B -C	0.31	1609 C	0.11	0.20
C -D	0.31	0 T	0.00	0.31
Bottom Chords				
A -I	0.67	1659 T	0.27	0.40
I -G	0.47	1681 T	0.28	0.19
G -F	0.40	2012 T	0.33	0.07
F -E	0.41	1969 T	0.33	0.08
Webs				
I -H	0.02	108 T		
H -G	0.03	137 T		
G -B	0.27	843 T		
G -C	0.19	470 C		
F -C	0.06	341 C		
C -E	0.41	2027 C		
E -D	0.02	154 C		
Sliders				
A -J	0.11	694 C		

TL Defl -0.25" in I -G L/773
LL Defl -0.12" in I -G L/999

Hz Disp	LL	DL	TL
Jt E	0.05"	0.05"	0.11"

Shear // Grain in J -J 0.38

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.94
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x 8.0	Ctr	Ctr	0.88
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
I	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G#	LOCK	5.0x 8.0-0.1	-0.5		0.55
F	LOCK	5.0x 5.0	0.3	2.7	0.46
E	LOCK	3.0x 4.0-0.5	Ctr		0.99

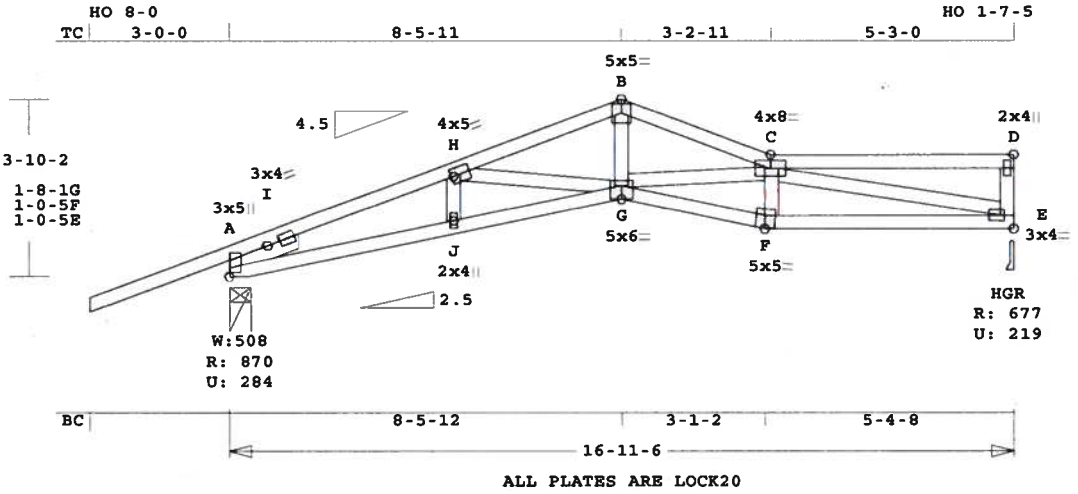
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2027 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R88*	1	SP	161106	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 105.7 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 4	SP-#2
BC	0.68	2x 4	SP-#2
WB	0.85	2x 4	SP-#3
SL	0.11	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16-11- 6
BC Cont.	0- 0- 0	16-11- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	870	284	5- 8	1- 8
			Hz =	89
E	678	219	3- 8	1- 8
			Hz =	45

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -I	0.44	1330 C	0.01	0.43
I -H	0.45	1795 C	0.02	0.43
H -B	0.26	1571 C	0.11	0.15
B -C	0.35	1571 C	0.01	0.34
C -D	0.45	0 T	0.00	0.45
--------Bottom Chords---------				
A -J	0.68	1672 T	0.27	0.41
J -G	0.47	1693 T	0.28	0.19
G -F	0.40	2100 T	0.35	0.05
F -E	0.48	2056 T	0.34	0.14
------------Webs------------				
J -H	0.02	111 T		
H -G	0.04	176 C		
G -B	0.29	925 T		
G -C	0.12	605 C		
F -C	0.07	345 C		
C -E	0.85	2102 C		
E -D	0.02	182 C		
-----------Sliders-----------				
A -I	0.11	698 C		

TL Defl -0.29" in J -G L/669
LL Defl -0.14" in J -G L/999

Hz Disp	LL	DL	TL
Jt E	0.06"	0.06"	0.11"

Shear // Grain in I -I 0.39

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

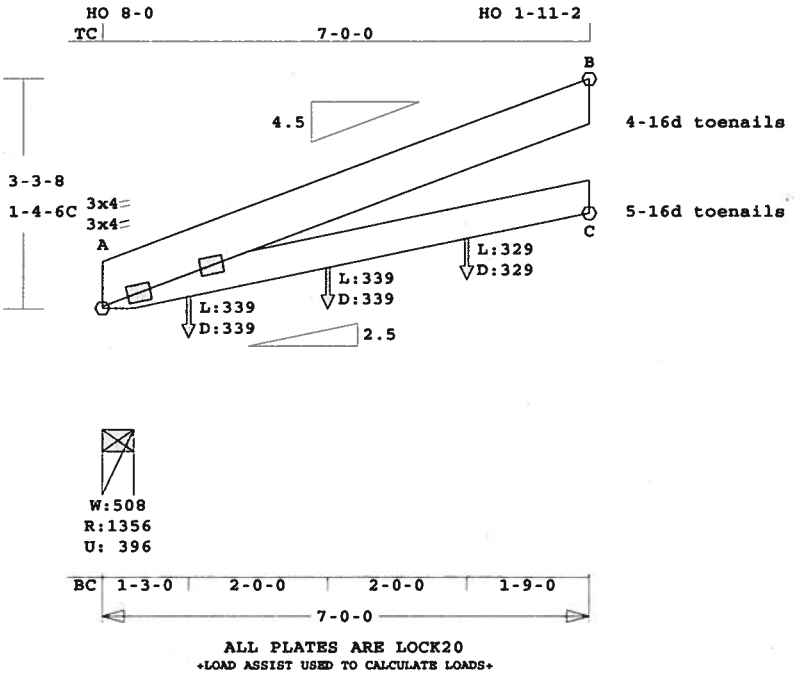
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.95
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
B	LOCK	5.0x 5.0	Ctr	Ctr	0.50
C	LOCK	4.0x 8.0	Ctr	Ctr	0.92
D	LOCK	2.0x 4.0	Ctr	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	5.0x 6.0	Ctr	-1.1	0.67
F	LOCK	5.0x 5.0	0.3	2.7	0.46
E	LOCK	3.0x 4.0	-1.0	Ctr	0.96

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2102 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R87*	1	VJACK	70000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.6 LBS

Online Plus -- Version 18.0.020
RUN DATE: 14-FEB-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 8	SP-2400
BC	0.56	2x 6	SP-2400

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	7.0'
BC V	0	20	0.0'	7.0'
BC V	339	339	1.2'	CL-LB
BC V	339	339	3.2'	CL-LB
BC V	329	329	5.2'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd		Lbs	Lbs	In-Sx	In-Sx
A	1356	396	5- 8	1- 2				
			Hz =	118				
B	533	229	3- 8	1- 0				
C	719	179	3- 8	1- 0				
			Hz =	63				

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.53	203 C	0.00 0.53
--------Bottom Chords---------			
A -C	0.56	63 T	0.00 0.56

LL Defl -0.05" in A -C L/999
TL Defl -0.10" in A -C L/716
Shear // Grain in A -C 0.56

Hz Disp	LL	DL	TL
Jt B	0.01"	0.01"	0.02"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.67
A	LOCK	3.0x 4.0	Ctr	Ctr	0.17

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

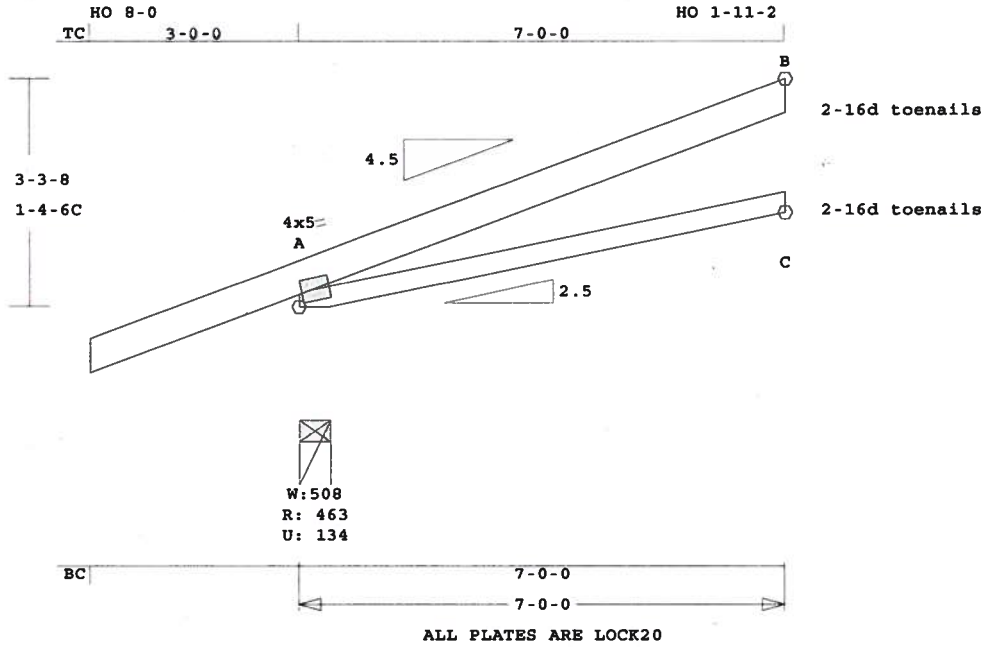
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 203 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R86*	14	VJACK	70000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 6	SP-#2
BC	0.25	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	463	134	5- 8	1- 8
			Hz = 118	
C	119	0	3- 8	1- 8
B	221	147	3- 8	1- 8
			Hz = 63	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.30	130 C	0.00 0.30
--------Bottom Chords---------			
A -C	0.25	4 C	0.00 0.25

TL Defl -0.14" in A -C L/539
LL Defl -0.06" in A -C L/999

Hz Disp	LL	DL	TL
Jt B	0.02"	0.00"	0.02"

Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	1.9	0.7	0.79

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

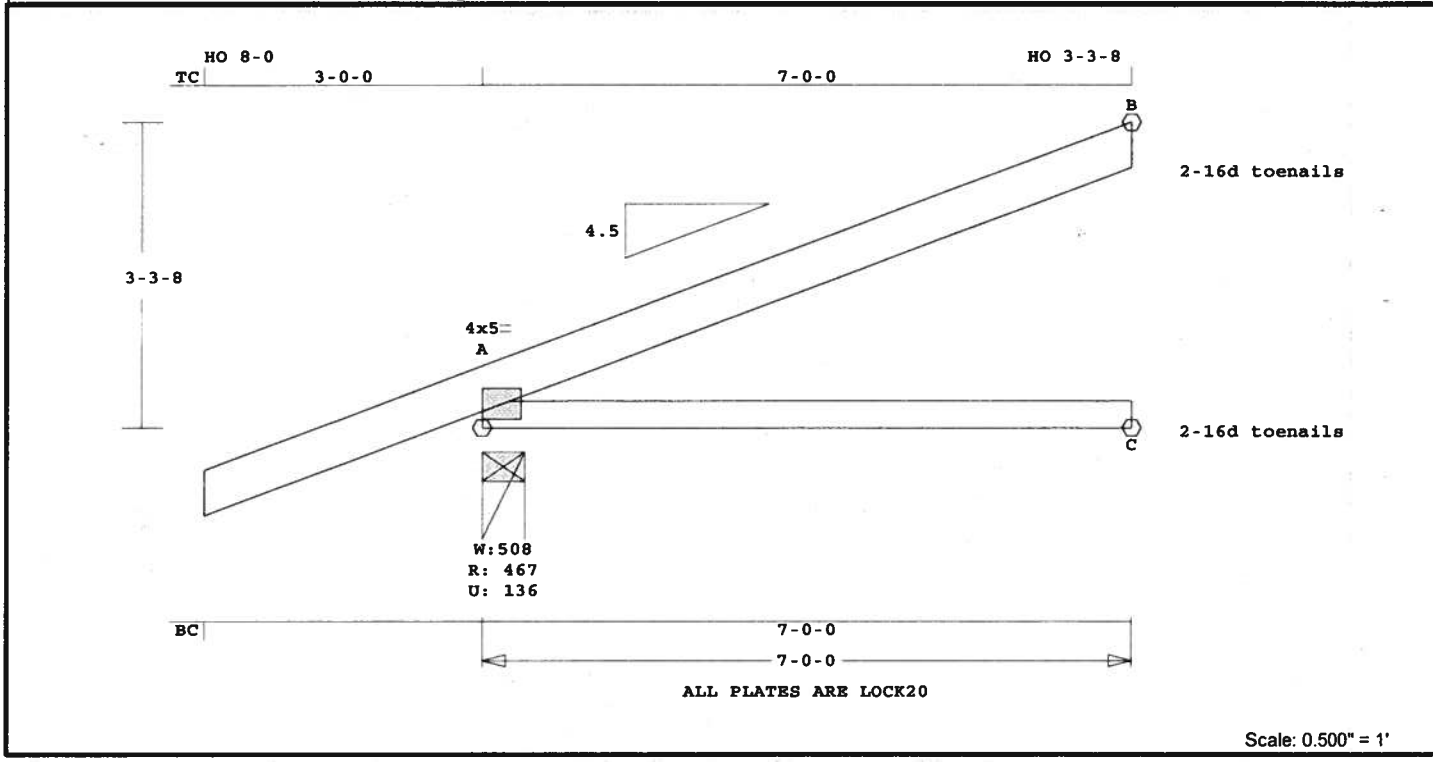
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph

Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 130 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R84	1	MONO	70000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 6	SP-#2
BC	0.24	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 0
BC	Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	467	136	5- 8	1- 8
			Hz = 118	
C	117	0	3- 8	1- 8
B	218	144	3- 8	1- 8
			Hz = 63	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.29	128 C	0.00 0.29
--------Bottom Chords---------			
A -C	0.24	0 T	0.00 0.24

TL Defl -0.13" in A -C L/588
LL Defl -0.06" in A -C L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

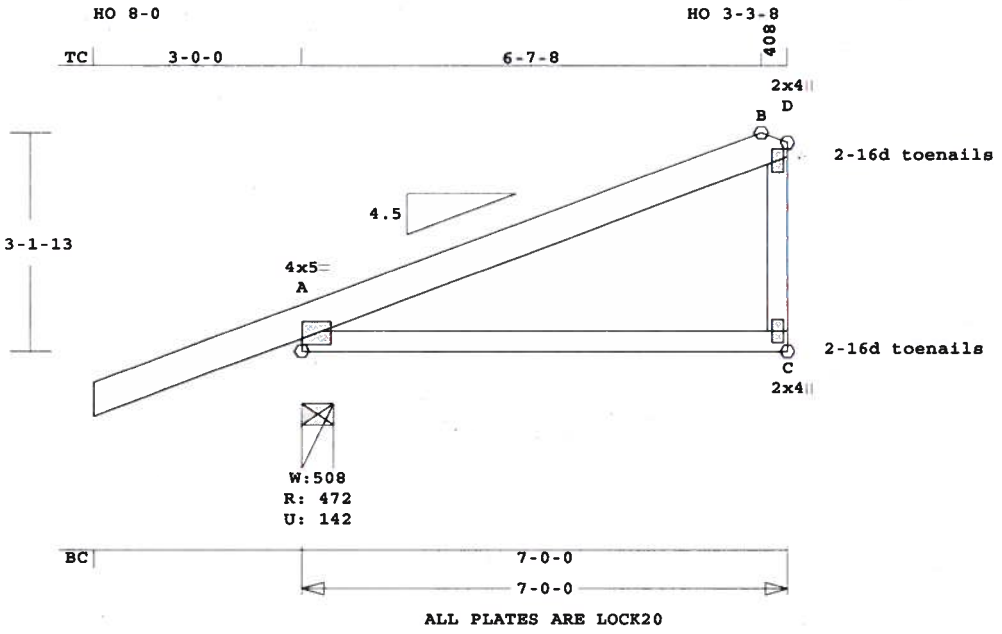
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 128 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R83*	3	SP	70000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 6	SP-#2
BC	0.24	2x 4	SP-#2
WB	0.00	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 0
BC	Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	472	142	5- 8	1- 8
			Hz =	112
C	118	0	3- 8	1- 8
D	212	106	3- 8	1- 8
			Hz =	60

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.31	110	C 0.00 0.31
--------Bottom Chords---------			
A -C	0.24	0	T 0.00 0.24
------------Webs------------			
C -D	0.00	0	T

TL Defl -0.13" in A -C L/582
LL Defl -0.06" in A -C L/999
Shear // Grain in A -D 0.17

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

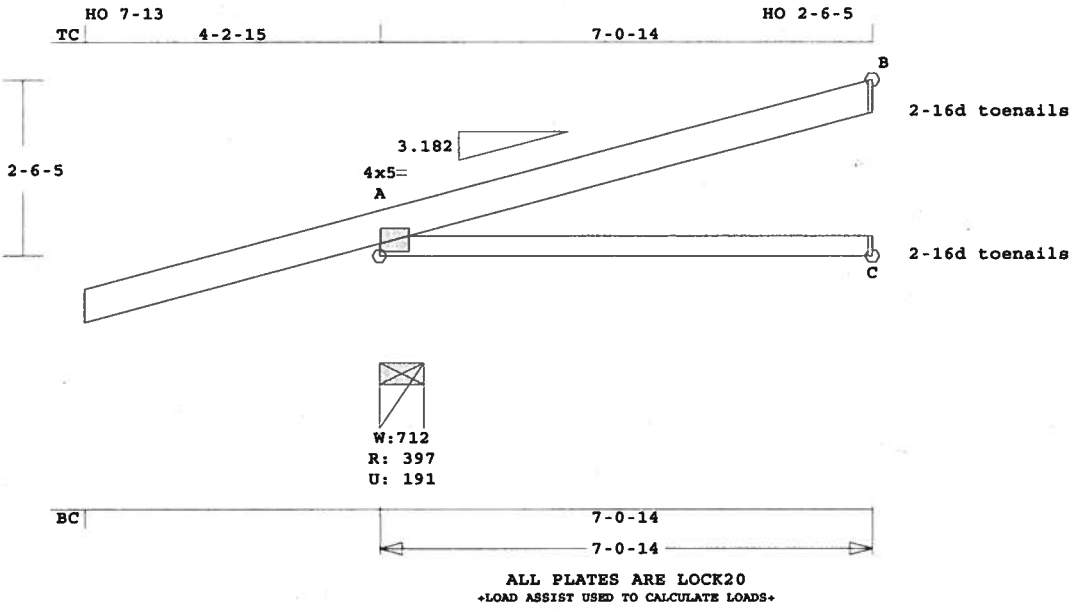
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 110 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R81	2	KING	70014	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 6	SP-#2
BC	0.21	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0-14
BC Cont.	0- 0- 0	7- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	7.1'
BC V 0	20	0.0'	7.1'
TC V -40	-20	0.0'	
17	8		7.1'
BC V 0	-20	0.0'	
0	8		7.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	398	191	7-12	1- 8
			Hz =	60
C	119	0	3- 8	1- 8
B	209	130	3- 8	1- 8
			Hz =	32

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.24	83 C	0.00 0.24
--------Bottom Chords---------			
A -C	0.21	0 T	0.00 0.21

TL Defl -0.10" in A -C L/771
LL Defl -0.04" in A -C L/999
Shear // Grain in A -B 0.16

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	Ctr	Ctr	0.71

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

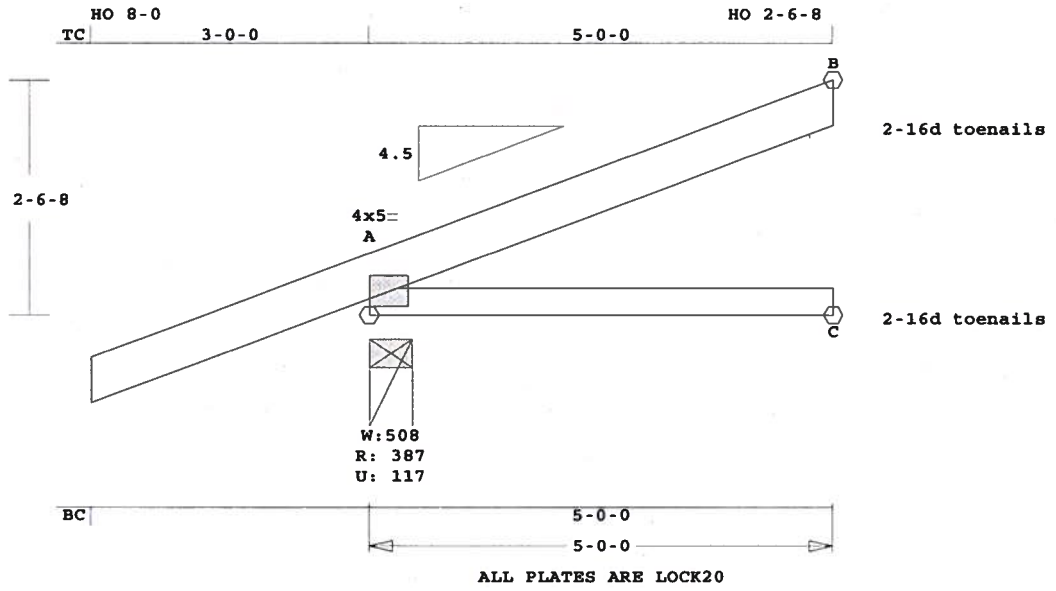
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder King Jack
Loading TC and BC
Setback 5- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 83 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R80*	3	JACK	50000	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.17	2x 6	SP-#2
BC	0.12	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	5- 0- 0
BC	Cont.	0- 0- 0	5- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	387	118	5- 8	1- 8
			Hz =	84
C	84	0	3- 8	1- 8
B	157	106	3- 8	1- 8
			Hz =	45

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.17	107	C 0.00 0.17
--------Bottom Chords---------			
A -C	0.12	0	T 0.00 0.12

TL Defl -0.03" in A -C L/999
LL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65

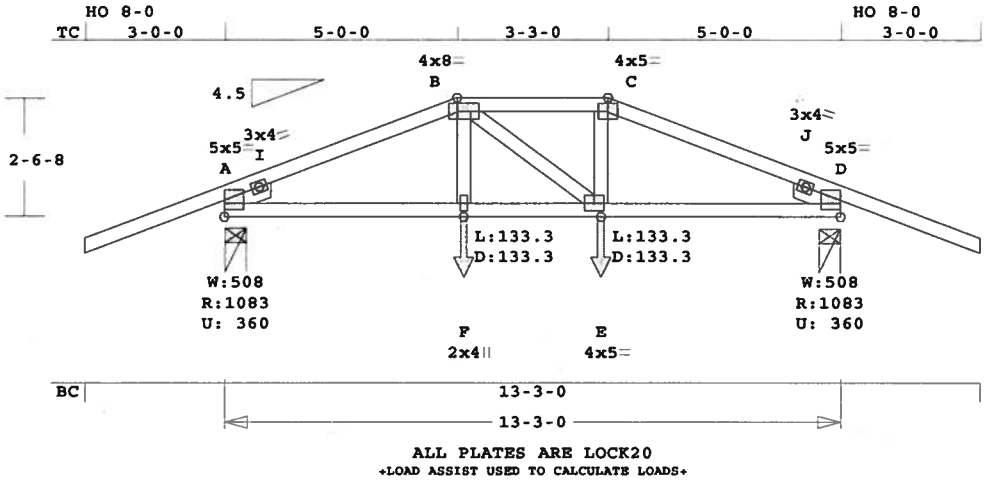
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 107 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R79	1	HIPP	130300	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.75	2x 4	SP-#2
WB	0.09	2x 4	SP-#3
SL	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	13- 3- 0
BC Cont.	0- 0- 0	13- 3- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	13.2'
BC V 0	20	0.0'	13.2'
TC V 30	15	5.0'	8.2'
BC V 0	15	5.1'	8.1'
BC V 133	133	5.1'	CL-LB
BC V 133	133	8.1'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1084	361	5- 8	1- 8
D	1084	361	5- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
I -B	0.49	1600 C	0.02	0.47
B -C	0.27	1486 C	0.01	0.26
C -J	0.51	1604 C	0.02	0.49
Bottom Chords				
A -F	0.75	1484 T	0.27	0.48
F -E	0.48	1469 T	0.27	0.21
E -D	0.73	1486 T	0.27	0.46
Webs				
F -B	0.09	335 T		
B -E	0.00	34 C		
E -C	0.09	339 T		
Sliders				
A -I	0.09	404 C		
J -D	0.10	422 C		

TL Defl -0.14" in F -E L/999
LL Defl -0.07" in F -E L/999
Shear // Grain in I -B 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

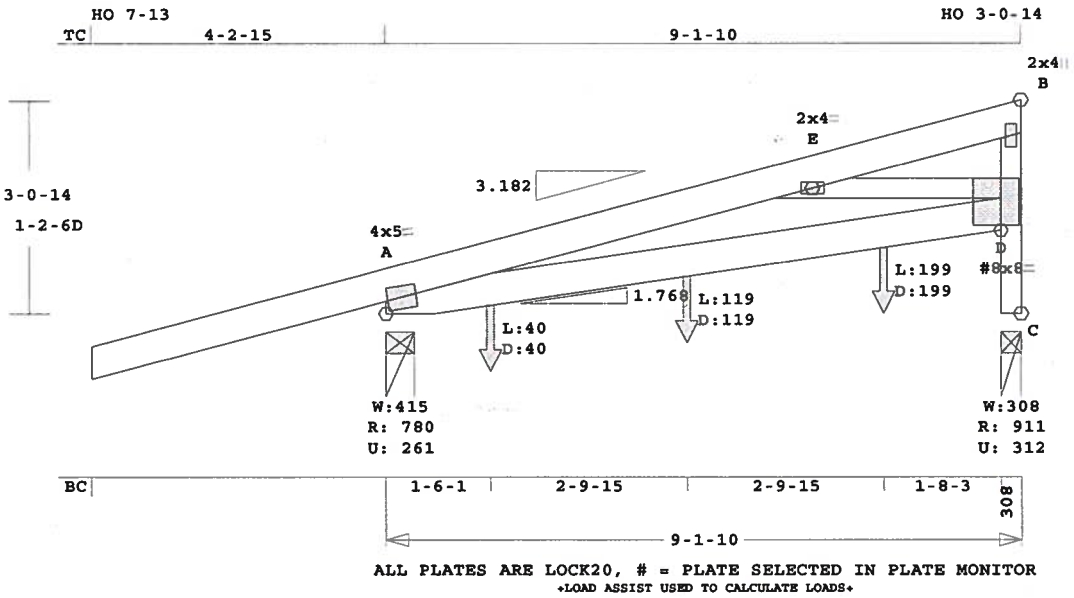
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 5.0	2.5	0.5	0.90
I	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 8.0	Ctr	Ctr	0.73
C	LOCK	4.0x 5.0	Ctr	Ctr	0.73
J	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	5.0x 5.0-2.5	0.5		0.90
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 5- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1604 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R78*	1	HFSC	90110	3.182	4- 2-15	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 76.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.28	2x 6	SP-#2
BC	0.69	2x 6	SP-#2
WB	0.22	2x 4	SP-#3
EX D -B		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 1-10
BC Cont.	0- 0- 0	9- 1-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.1'
BC V	0	20	0.0'	9.1'
TC V	-40	-20	0.0'	
	37	19		9.1'
BC V	0	-20	0.0'	
	0	19		8.8'
BC V	40	40	1.5'	CL-LB
BC V	119	119	4.3'	CL-LB
BC V	199	199	7.2'	CL-LB

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	781	262	4-15	1- 8
			Hz =	67
C	911	312	3- 8	1- 8
			Hz =	-68

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -E	0.28	1402	C	0.00	0.28
E -B	0.12	470	C	0.00	0.12
--------Bottom Chords---------					
A -D	0.69	1417	T	0.18	0.51
------------Webs------------					
E -D	0.22	966	C		
D -B	0.93	262	T	0.00	0.93
C -D	0.35	911	C	0.09	0.26

TL Defl 0.02" in A -D L/999
LL Defl 0.02" in A -D L/999

Hz Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.10"

Shear // Grain in A -D 0.42

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	2.8	0.7	0.81
E	LOCK	2.0x 4.0	Ctr	Ctr	0.49
B	LOCK	2.0x 4.0	Ctr	Ctr	0.39
D	LOCK	8.0x 8.028.4-3.2			0.27

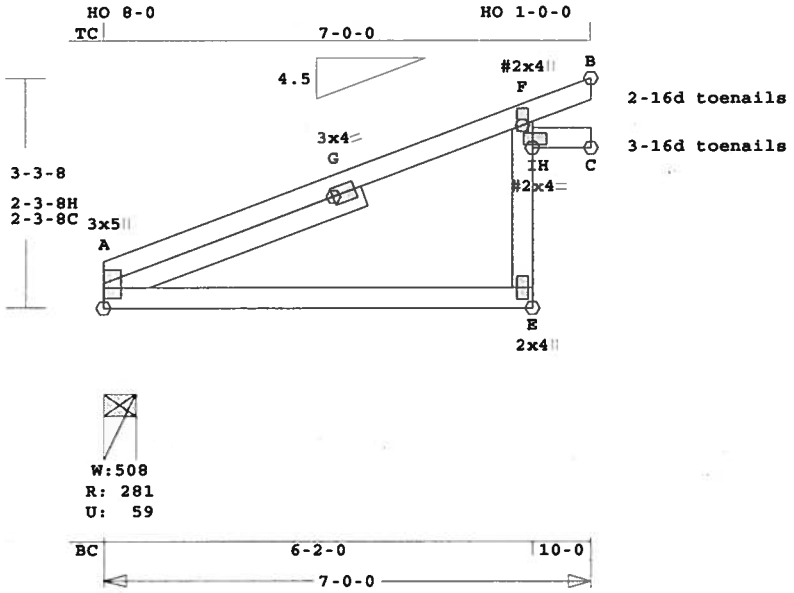
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack

Loading TC and BC
Setback 6-10- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Use properly rated hangers for loads framing into girder truss.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1402 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R77A*	1	SP	70000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 39.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	-Size-	----Lumber----
TC	0.26	2x 4	SP-#2
BC	0.56	2x 4	SP-#2
CW	0.10	2x 4	SP-#3
SL	0.03	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	282	60	5- 8	1- 0
			Hz = 118	
B	76	88G	3- 8	1- 0
C	367	191	3- 8	1- 0
			Hz = 63	

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -G	0.26	268 C	0.00	0.26
G -F	0.26	136 C	0.00	0.26
F -B	0.18	44 C	0.00	0.18
--------Bottom Chords---------				
A -E	0.24	157 T	0.02	0.22
E -I	0.11	75 T	0.00	0.11
I -C	0.56	63 T	0.00	0.56
----------Chord-Webs----------				
E -I	0.00	137 T		
I -F	0.10	424 T	0.10	0.00
-----------Sliders-----------				
A -G	0.03	137 T		

LL Defl -0.02" in A -E L/999
TL Defl -0.05" in A -E L/999
Shear // Grain in F -C 0.47

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.3	0.69
G	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	2.0x 4.0	Ctr	0.9	0.30
E	LOCK	2.0x 4.0	Ctr	Ctr	0.58
I	LOCK	2.0x 4.0	0.4-0.2		0.71

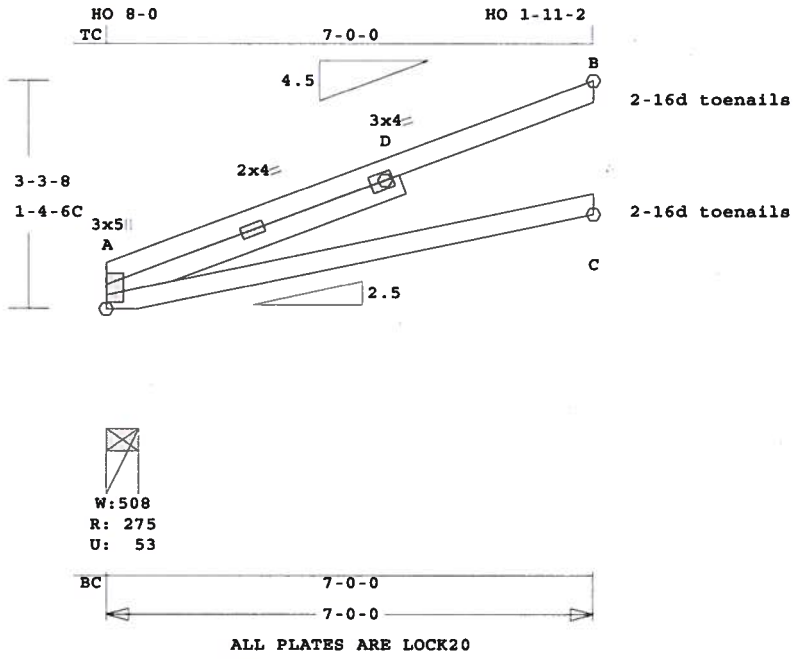
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 268 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R77*	8	VJACK	70000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 36.5 LBS

Online Plus -- Version 18.0.020
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
SL	0.13	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	275	54	5- 8	1- 0
			Hz =	118
B	207	135	3- 8	1- 0
C	116	0	3- 8	1- 0
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.53	443 C	0.00 0.53
D -B	0.53	116 C	0.00 0.53
--------Bottom Chords---------			
A -C	0.35	62 T	0.00 0.35
----------Sliders------------			
A -D	0.13	451 T	

LL Defl -0.06" in A -C L/999
TL Defl -0.13" in A -C L/567
Shear // Grain in A -D 0.28

Hz Disp	LL	DL	TL
Jt B	0.04"	-0.01"	0.04"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	1.5	0.4	0.69
A	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50

Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 443 Lbs
Quality Control Factor 1.25

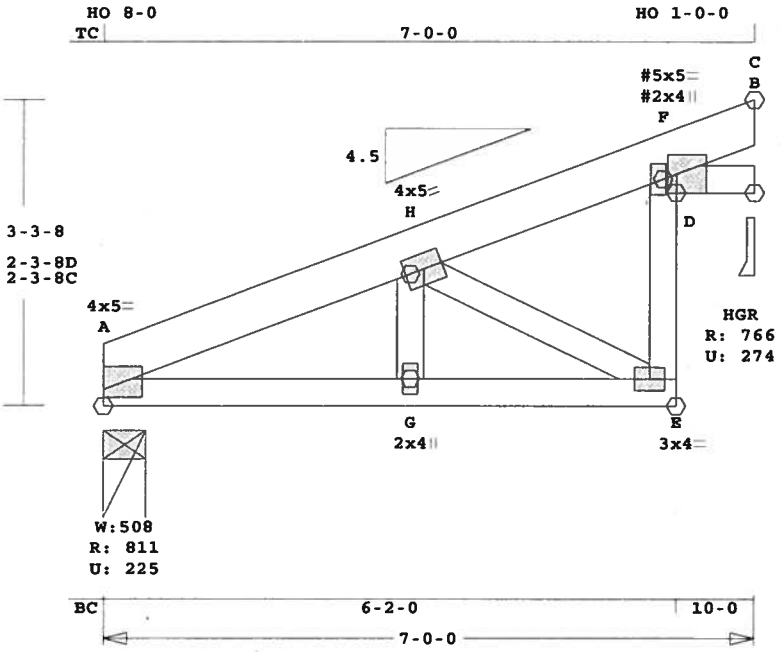
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R76*	1	SP	70000	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.4 LBS

Online Plus -- Version 18.0.020
RUN DATE: 16-FEB-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 6	SP-#2
BC	0.43	2x 4	SP-#2
EX F -C		2x 4	SP-2400
CW	0.80	2x 4	SP-#3
WB	0.14	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	7- 0- 0
BC Cont.		0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	7.0'
BC V 0	20	0.0'	7.0'
BC V 77	77	0.0'	6.2'

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	812	226	5- 8	1- 0
			Hz =	118
C	766	275	3- 8	1- 0
			Hz =	63

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -H	0.10	949 C	0.06	0.04
H -F	0.07	282 C	0.00	0.07
F -B	0.04	10 C	0.00	0.04
Bottom Chords				
A -G	0.40	873 T	0.16	0.24
G -E	0.43	873 T	0.16	0.27
E -F	0.41	328 T	0.02	0.39
F -C	0.74	63 T	0.00	0.74
Chord-Webs				
E -F	0.80	255 T	0.08	0.72
Webs				
G -H	0.14	530 T		
H -E	0.13	684 C		

LL Defl -0.03" in E -F L/999
TL Defl -0.06" in E -F L/999
Shear // Grain in F -C 0.83

Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.03"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 5.0	0.7	0.3	0.65
H	LOCK	4.0x 5.0	Ctr	Ctr	0.64
F	LOCK	2.0x 4.0	-0.6	Ctr	0.14
G	LOCK	2.0x 4.0	Ctr	Ctr	0.57
E	LOCK	3.0x 4.0	Ctr	Ctr	0.77
F	LOCK	5.0x 5.0	1.3	0.1	0.97

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 949 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R75*	1*3P	SP	610902	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish

No bolts in 2x4s or smaller.
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 12850 Lbs
Quality Control Factor 1.25

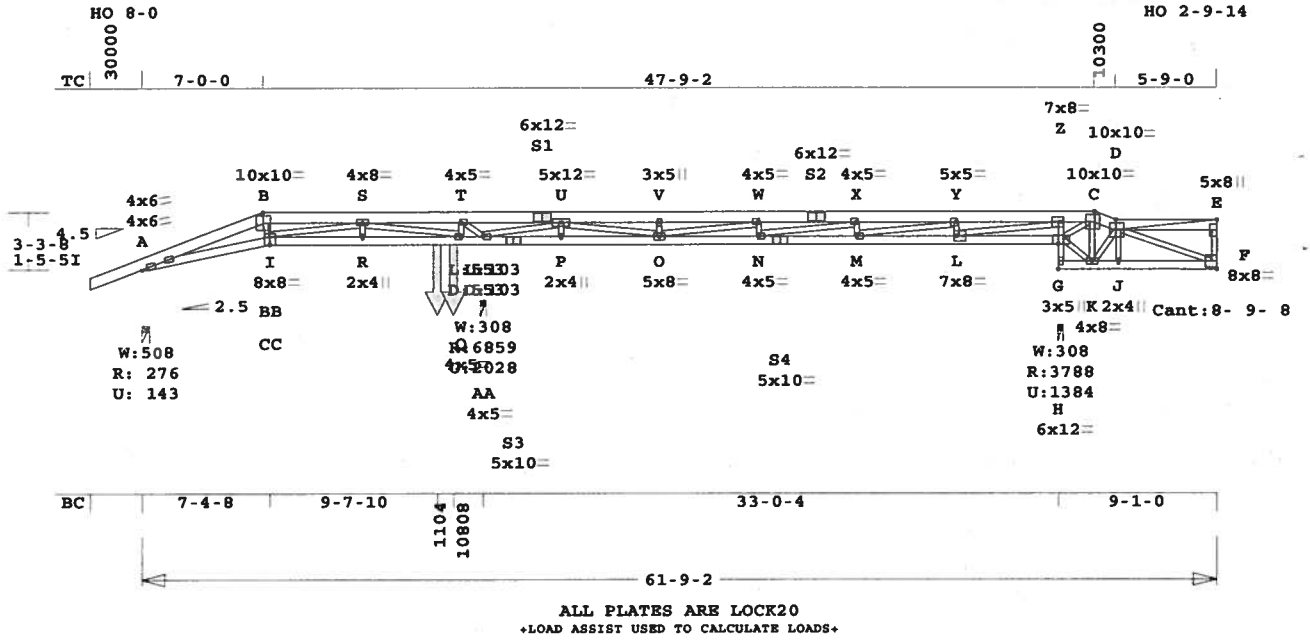
4. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R75	1*3P	SP	610902	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 556.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.27	2x 8	SP-2400
BC	0.69	2x 6	SP-#2
CW	0.54	2x 4	SP-#3
WB	0.92	2x 4	SP-#3
EX U -O		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	61.8'
BC V	0	20	0.0'	61.8'
BC V	70	70	18.9'	38.9'
BC V	70	70	40.9'	52.9'
BC V	553	553	17.0'	CL-LB
BC V	103	103	17.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	276	144	5- 8	1- 8
		Hz =		96
AA	6859	2028	3- 8	2- 7
G	3789	1385	3- 8	1- 8
		Hz =		55

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.03	1141 T	0.01	0.02
B -S	0.03	1159 T	0.01	0.02
S -T	0.24	8163 T	0.09	0.15
T -S1	0.27	11134 T	0.12	0.15
S1-U	0.16	11134 T	0.12	0.04
U -V	0.05	8551 C	0.00	0.05
V -W	0.09	8551 C	0.00	0.09
W -S2	0.12	12850 C	0.02	0.10
S2-X	0.12	12850 C	0.02	0.10
X -Y	0.11	12590 C	0.01	0.10
Y -Z	0.13	7818 C	0.00	0.13
Z -C	0.14	1067 C	0.01	0.13
C -D	0.00	792 T		
D -E	0.01	0 T	0.00	0.01
--------Bottom Chords---------				
A -I	0.09	1060 C	0.00	0.09
I -R	0.15	3470 C	0.00	0.15
R -Q	0.32	3470 C	0.00	0.32
Q -AA	0.36	8163 C	0.02	0.34
AA-S3	0.35	890 C	0.00	0.35
S3-P	0.11	890 C	0.00	0.11
P -O	0.12	890 C	0.00	0.12
O -N	0.69	12850 T	0.52	0.17
N -S4	0.62	12591 T	0.51	0.11
S4-M	0.68	12591 T	0.51	0.17
M -L	0.50	7818 T	0.31	0.19
L -H	0.25	1006 T	0.00	0.25
G -K	0.00	284 C		
K -J	0.03	536 C	0.00	0.03
J -F	0.03	538 C	0.00	0.03
----------Chord-Webs----------				
G -H	0.53	3763 C	0.03	0.50
H -Z	0.54	3410 C	0.02	0.52
------------Webs------------				
B -I	0.03	619 C		
I -S	0.27	2585 T		
R -S	0.05	578 T		
S -Q	0.27	4817 C		
Q -T	0.23	2230 T		
T -AA	0.22	4274 C		
AA-U	0.58	10691 C		
P -U	0.06	740 T		
U -O	0.59	9692 T		
O -V	0.01	241 C		
O -W	0.24	4413 C		
N -W	0.07	871 T		
N -X	0.02	280 C		
M -X	0.02	438 C		
M -Y	0.52	4899 T		
L -Y	0.05	1035 C		
L -Z	0.92	8675 T		
H -C	0.05	657 T		
H -K	0.04	609 C		
K -C	0.06	626 T		
K -D	0.01	309 C		
J -D	0.01	218 T		
D -F	0.06	572 T		
F -E	0.00	149 C		

TL Defl	-0.76" in N -S4	L/515	
LL Defl	-0.38" in N -S4	L/999	
LL Cant	0.21" in J -F	L/496	
Hz Disp	LL	DL	TL
Jt G	0.06"	0.06"	0.11"
Shear // Grain	in R -Q	0.32	

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.78
A	LOCK	4.0x 6.0	Ctr	Ctr	0.19
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
S	LOCK	4.0x 8.0	Ctr	0.5	1.00
T	LOCK	4.0x 5.0	Ctr	0.5	0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
U	LOCK	5.0x12.0	Ctr	Ctr	0.84
V	LOCK	3.0x 5.0	Ctr	Ctr	0.41
W	LOCK	4.0x 5.0	Ctr	0.5	1.00
S2	LOCK	6.0x12.0	Ctr	Ctr	0.91
X	LOCK	4.0x 5.0	Ctr	0.5	1.00
Y	LOCK	5.0x 5.0	Ctr	Ctr	0.74
Z	LOCK	7.0x 8.0-0.5	Ctr		0.93
C	LOCK	10.0x10.0	0.5	Ctr	0.76
D	LOCK	10.0x10.0	0.5	Ctr	0.96
E	LOCK	5.0x 8.0-0.8	Ctr		0.91
I	LOCK	8.0x 8.0	Ctr-1.5		0.91
R	LOCK	2.0x 4.0	Ctr	Ctr	0.61
Q	LOCK	4.0x 5.0	Ctr	Ctr	0.75
AA	LOCK	4.0x 5.0-0.5	Ctr		0.87
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
P	LOCK	2.0x 4.0	Ctr	Ctr	0.61
O	LOCK	5.0x 8.0	Ctr	Ctr	0.97
N	LOCK	4.0x 5.0	Ctr	Ctr	0.75
S4	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	Ctr	0.92
L	LOCK	7.0x 8.0	0.5	Ctr	0.93
H	LOCK	6.0x12.0	Ctr	0.2	0.87
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
K	LOCK	4.0x 8.0	Ctr	Ctr	0.51
J	LOCK	2.0x 4.0	Ctr	Ctr	0.61
F	LOCK	8.0x 8.0-0.5	Ctr		0.79

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 2	12	24	0
BC 2	12	24	0
WB 1	8	8	

Web Connection Exception -- Use 4" spacing for screws or nails on the following webs
A- U

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R74*	1*2P	SP	610902	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish

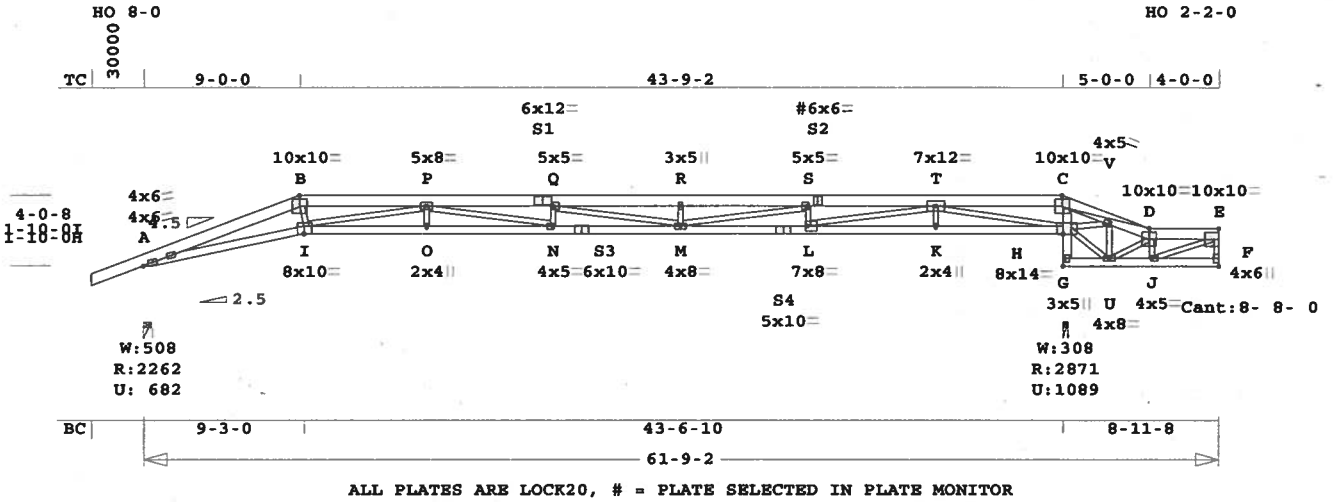
ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R74	1*2P	SP	610902	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 556.2 LBS

```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.23  2x 8  SP-2400
BC  0.84  2x 6  SP-#2
EX I -S3  2x 6  SP-2400
EX S3-S4  2x 6  SP-2400
CW  0.23  2x 6  SP-#2
WB  1.00  2x 4  SP-#3

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 61- 9- 2
 BC Cont.      0- 0- 0 61- 9- 2

Loading   Live   Dead  (psf)
TC        20.0   10.0
BC         0.0   10.0
Total     20.0   20.0   40.0
Spacing                 24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A    2262   682  5- 8  1- 8
                Hz =    67
G    2872  1089  3- 8  1- 8
                Hz =    55

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -B  0.06  9110 C  0.04  0.02
B -P  0.15  9278 C  0.02  0.13
P -S1 0.23 15262 C  0.07  0.16
S1-Q  0.15 15262 C  0.07  0.08
Q -R  0.17 14796 C  0.06  0.11
R -S  0.17 14796 C  0.06  0.11
S -S2 0.10 11824 C  0.04  0.06
S2-T  0.13 11824 C  0.04  0.09
T -C  0.08  1283 T  0.02  0.06
C -V  0.04  1784 T  0.03  0.01
V -D  0.01   669 T  0.01  0.00
D -E  0.01   326 T  0.00  0.01
--------Bottom Chords---------
A -I  0.84  8663 T  0.52  0.32
I -O  0.48 12584 T  0.28  0.20
O -N  0.44 12584 T  0.28  0.16
N -S3 0.40 15260 T  0.34  0.06
S3-M  0.42 15260 T  0.34  0.08
M -S4 0.34 11823 T  0.27  0.07
S4-L  0.81 11823 T  0.71  0.10
L -K  0.54  6291 T  0.38  0.16
K -H  0.73  6291 T  0.38  0.35
G -U  0.00   165 C
U -J  0.01   343 C  0.00  0.01
J -F  0.01     0 T  0.00  0.01
----------Chord-Webs----------
G -H  0.15  2848 C  0.00  0.15
H -C  0.23  1007 C  0.00  0.23
------------Webs-------------
B -I  0.44  2755 T
I -P  0.39  3373 C
O -P  0.02   194 T
P -N  0.44  2746 T
N -Q  0.03   386 C
Q -M  0.06   477 C
M -R  0.03   453 C
M -S  0.48  3049 T
L -S  0.08  1090 C
L -T  0.91  5676 T
K -T  0.02   184 T
T -H  1.00  7762 C
H -V  0.12  1080 C
H -U  0.06   562 C
U -V  0.08   531 T
U -D  0.02   310 C
J -D  0.03   247 T
J -E  0.03   357 C
F -E  0.00    68 T

TL Defl  -2.44" in N -S3 L/257
LL Defl  -1.21" in N -S3 L/518
LL Cant   0.44" in J -F  L/235
Hz Disp   LL      DL      TL
  Jt G   0.34"   0.35"   0.69"
Shear // Grain  in I -O   0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  LOCK  4.0x 6.0 Ctr Ctr 0.78
A  LOCK  4.0x 6.0 Ctr Ctr 0.28
B  LOCK 10.0x10.0 Ctr Ctr 0.76
P  LOCK  5.0x 8.0 Ctr Ctr 0.62
S1 LOCK  6.0x12.0 Ctr Ctr 0.91
Q  LOCK  5.0x 5.0 Ctr Ctr 0.60
R  LOCK  3.0x 5.0 Ctr Ctr 0.41
S  LOCK  5.0x 5.0 Ctr Ctr 0.69
S2#LOCK  6.0x 6.0 Ctr Ctr 0.91
T  LOCK  7.0x12.0 Ctr Ctr 0.92
C  LOCK 10.0x10.0 Ctr Ctr 0.77
V  LOCK  4.0x 5.0 0.5 0.2 0.56
D  LOCK 10.0x10.0 Ctr Ctr 0.76
E  LOCK 10.0x10.0-1.5 Ctr 0.75
I  LOCK  8.0x10.0 0.5-1.5 0.90
O  LOCK  2.0x 4.0 Ctr Ctr 0.61
N  LOCK  4.0x 5.0 Ctr Ctr 0.78
S3 LOCK  6.0x10.0 Ctr 0.2 0.95
M  LOCK  4.0x 8.0 Ctr Ctr 0.86
S4 LOCK  5.0x10.0 Ctr Ctr 0.88
L  LOCK  7.0x 8.0 0.5 Ctr 0.92
K  LOCK  2.0x 4.0 Ctr Ctr 0.61
H  LOCK  8.0x14.0 Ctr 1.2 0.98
G  LOCK  3.0x 5.0 Ctr Ctr 0.73
U  LOCK  4.0x 8.0 Ctr Ctr 0.55
J  LOCK  4.0x 5.0 Ctr Ctr 0.60
F  LOCK  4.0x 6.0-0.2 Ctr 0.90

# = Plate Monitor used

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3  screws -OR- 10d nails
  as each layer is applied.)
          ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  2     12      24       0
  BC  2     12      24       0
  WB  1      8       8
Web Connection Exception --
Use 4" spacing for screws or
nails on the following webs
 I- P   T- H
OH Loading
  Soffit psf 2.0
CAUTION: Use extra care during
  fabrication, shipping,
  handling and erection of
  this truss.  Temporary and
  permanent bracing are req'd
  and are designed by others.
  Seek advice of registered
  engineer.
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
  bearing locations.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          110 mph
  Mean Roof Height:  25-0
  Exposure  Category:    B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  TC Dead Load:        4.0 psf
  BC Dead Load:        6.0 psf
Max comp. force    15262 Lbs
Quality Control Factor 1.25
 3. THIS TRUSS HAS SPAN GREATER
    THAN 60-0-0 AND HAS BEEN
    DESIGNED BASED ON INFORMA-
    TION FURNISHED TO ROBBINS
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R73*	1*2P	SP	610902	4.5	3- 0- 0	0	

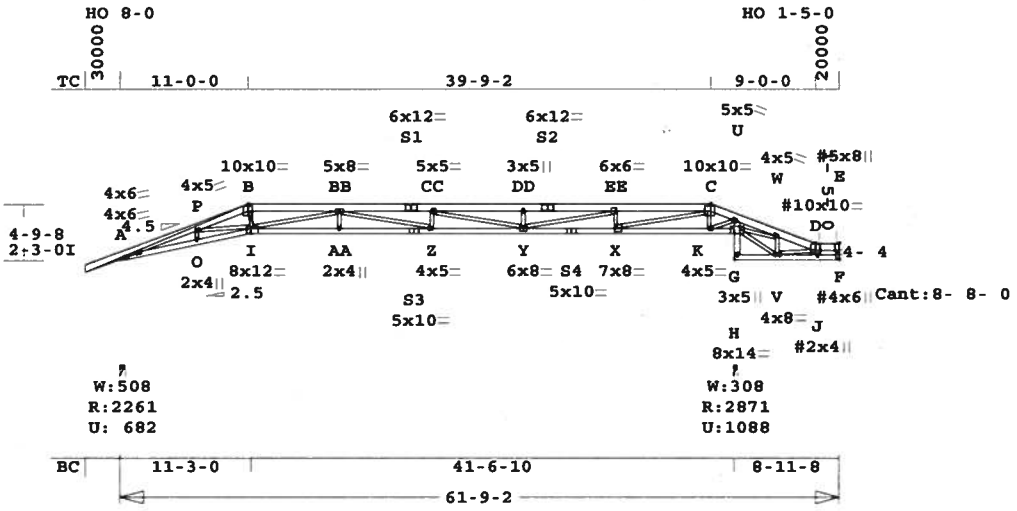
+Office and Care Center, Hospice, M.M. Parrish

selected through a plate
monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 12949 Lbs
Quality Control Factor 1.25
3. THIS TRUSS HAS SPAN GREATER THAN 60-0-0 AND HAS BEEN DESIGNED BASED ON INFORMATION FURNISHED TO ROBBINS ENGINEERING BY THE TRUSS FABRICATOR. PLEASE REFER TO THE ACCOMPANYING LETTER FOR FURTHER INFORMATION.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R73*	1*2P	SP	610902	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.063" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 563.7 LBS

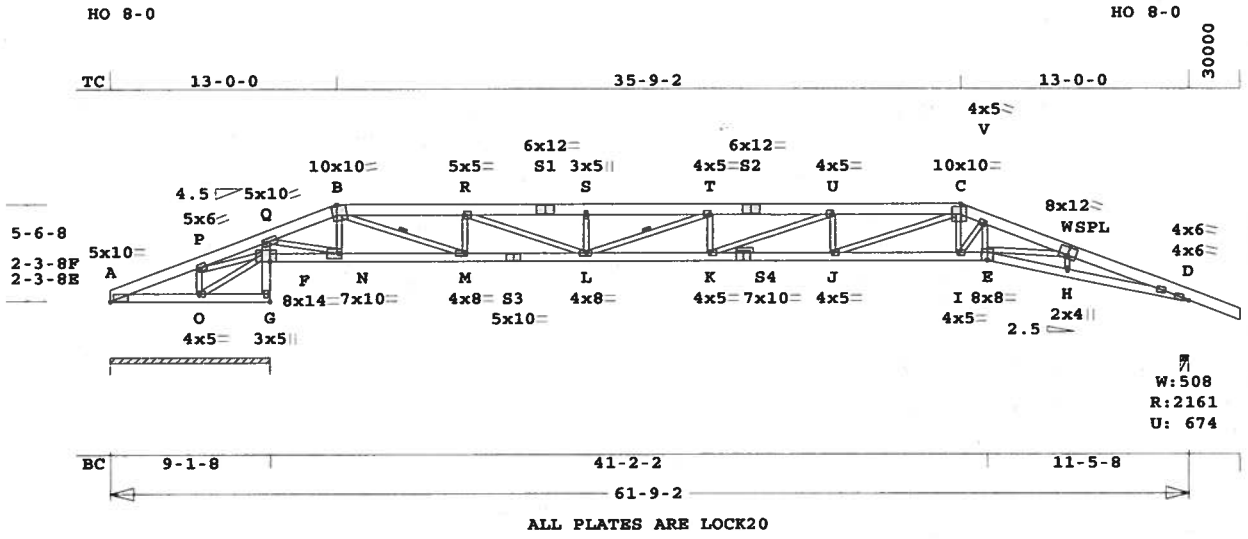
```
Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.19  2x 8  SP-2400
BC  0.87  2x 6  SP-#2
EX I -S3  2x 6  SP-2400
CW  0.14  2x 6  SP-#2
WB  0.98  2x 4  SP-#3

Brace truss as follows:
      O.C.      From       To
 TC Cont.      0- 0- 0 61- 9- 2
 BC Cont.      0- 0- 0 61- 9- 2

Loading   Live   Dead  (psf)
TC        20.0   10.0
BC         0.0   10.0
Total     20.0   20.0   40.0
Spacing                 24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10


Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)

Jt  React Uplft  Size Req'd
      Lbs   Lbs In-Sx In-Sx
A    2261   683  5- 8  1- 8
                 Hz =   -74
G    2872  1089  3- 8  1- 8
                 Hz =    78

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -P  0.10  8999 C  0.02  0.08
P -B  0.10  8975 C  0.02  0.08
B -BB 0.13  8891 C  0.02  0.11
BB-S1 0.18 12949 C  0.05  0.13
S1-CC 0.14 12949 C  0.05  0.09
CC-DD 0.12 11607 C  0.04  0.08
DD-S2 0.12 11607 C  0.04  0.08
S2-EE 0.18 11607 C  0.04  0.14
EE-C  0.15  6908 C  0.01  0.14
C -U  0.17  1256 C  0.00  0.17
U -W  0.19  1597 T  0.02  0.17
W -D  0.01   559 T  0.01  0.00
D -E  0.01     0 T  0.00  0.01
--------Bottom Chords---------
A -O  0.58  8501 T  0.51  0.07
O -I  0.78  8488 T  0.51  0.27
I -AA 0.42 11617 T  0.26  0.16
AA-S3 0.38 11616 T  0.26  0.12
S3-Z  0.81 11616 T  0.70  0.11
Z -Y  0.87 12948 T  0.78  0.09
Y -S4 0.58  6908 T  0.41  0.17
S4-X  0.62  6908 T  0.41  0.21
X -K  0.42   879 T  0.05  0.37
K -H  0.37  1423 C  0.00  0.37
G -V  0.02    84 C  0.00  0.02
V -J  0.02    25 C  0.00  0.02
J -F  0.02     0 T  0.00  0.02
----------Chord-Webs----------
G -H  0.14  2847 C  0.07  0.07
H -U  0.10  3428 C  0.10  0.00
------------Webs-------------
O -P  0.03   243 T
P -I  0.02   144 T
B -I  0.40  2549 T
I -BB 0.36  2794 C
AA-BB 0.02   169 T
BB-Z  0.22  1373 T
Z -CC 0.02   165 T
CC-Y  0.19  1384 C
Y -DD 0.04   628 C
Y -EE 0.77  4848 T
X -EE 0.10  1295 C
X -C  0.98  6154 T
K -C  0.05   529 C
K -U  0.40  2520 T
H -W  0.12   995 T
H -V  0.04   519 C
V -W  0.07   443 T
V -D  0.04   508 C
J -D  0.01   125 T
F -E  0.00    17 T

TL Defl  -1.96" in S3-Z  L/319
LL Defl  -0.98" in S3-Z  L/642
LL Cant   0.26" in J -F  L/398
Hz Disp    LL      DL      TL
  Jt G   0.29"   0.29"   0.58"
Shear // Grain  in C -U   0.28

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS  20 Ga, Gross Area
Jt Type  Plt Size  X    Y   JSI
A  LOCK  4.0x 6.0 Ctr Ctr 0.78
A  LOCK  4.0x 6.0 Ctr Ctr 0.28
P  LOCK  4.0x 5.0-0.5 0.2 0.64
B  LOCK 10.0x10.0 Ctr Ctr 0.76
BB LOCK  5.0x 8.0 Ctr Ctr 0.61
S1 LOCK  6.0x12.0 Ctr Ctr 0.91
CC LOCK  5.0x 5.0 Ctr Ctr 0.61
DD LOCK  3.0x 5.0 Ctr Ctr 0.41
S2 LOCK  6.0x12.0 Ctr Ctr 0.91
EE LOCK  6.0x 6.0 Ctr Ctr 0.92
C  LOCK 10.0x10.0 0.5 Ctr 0.97
U  LOCK  5.0x 5.0 Ctr Ctr 0.71
W  LOCK  4.0x 5.0 0.5 0.2 0.86
D# LOCK 10.0x10.0-0.4 1.9 0.65
E  LOCK  5.0x 8.0-0.8 3.1 0.51
O  LOCK  2.0x 4.0 Ctr Ctr 0.61
I  LOCK  8.0x12.0 Ctr-1.5 0.90
AA LOCK  2.0x 4.0 Ctr Ctr 0.61
S3 LOCK  5.0x10.0 Ctr Ctr 0.87
Z  LOCK  4.0x 5.0 Ctr Ctr 0.76
Y  LOCK  6.0x 8.0 Ctr Ctr 0.92
S4 LOCK  5.0x10.0 Ctr Ctr 0.82
X  LOCK  7.0x 8.0 0.5 Ctr 0.98
K  LOCK  4.0x 5.0 Ctr Ctr 0.60
H  LOCK  8.0x14.0 Ctr 1.3 0.66
G  LOCK  3.0x 5.0 Ctr Ctr 0.73
V  LOCK  4.0x 8.0 Ctr Ctr 0.75
J# LOCK  2.0x 4.0 Ctr-0.9 0.61
F# LOCK  4.0x 6.0-0.2-1.1 0.66

# = Plate Monitor used

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3   screws -OR- 10d nails
  as each layer is applied.)
          ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  2     12      24       0
  BC  2     12      24       0
  WB  1      8       8
Web Connection Exception --
Use 4" spacing for screws or
nails on the following webs
 I- B   X- C
OH Loading
  Soffit psf 2.0
CAUTION: Use extra care during
  fabrication, shipping,
  handling and erection of
  this truss.  Temporary and
  permanent bracing are req'd
  and are designed by others.
  Seek advice of registered
  engineer.
Design checked for 10 psf non-
  concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all
  bearing locations.
NOTE: USER MODIFIED PLATES
  This design may have plates
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R72*	1	SP	610902	4.5	0	3- 0- 0	
+Office and Care Center, Hospice, M.M. Parrish								

Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R72	1	SP	610902	4.5	0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 574.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.62	2x 8	SP-2400
BC	0.53	2x 6	SP-2400
EX A -G		2x 6	SP-#2
EX F -S3		2x 6	SP-#2
CW	0.76	2x 6	SP-#2
WB	0.78	2x 4	SP-#3
EX Q -N		2x 4	SP-#2
EX B -M		2x 4	SP-#2
EX R -L		2x 4	SP-#2
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -M
WB 1 rows CLB on L -T
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		9- 1- 8	
	2971	891	Hz =	100
D	2161	675	5- 8	1-13

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -P	0.14	1915 T	0.06	0.08
P -Q	0.62	5294 T	0.18	0.44
Q -B	0.44	387 C	0.00	0.44
B -R	0.19	4469 C	0.02	0.17
R -S1	0.20	7453 C	0.06	0.14
S1-S	0.16	7453 C	0.06	0.10
S -T	0.15	7453 C	0.06	0.09
T -S2	0.20	8665 C	0.09	0.11
S2-U	0.21	8665 C	0.09	0.12
U -C	0.19	8466 C	0.09	0.10
C -V	0.16	7208 C	0.05	0.11
V -W	0.29	8324 C	0.08	0.21
W -D	0.26	8160 C	0.07	0.19
Bottom Chords				
A -O	0.15	62 C	0.00	0.15
O -G	0.08	0 T	0.00	0.08
F -N	0.63	4901 C	0.07	0.56
N -M	0.21	257 T	0.03	0.18
M -S3	0.71	4470 T	0.54	0.17
S3-L	0.28	4470 T	0.20	0.08
L -K	0.45	8666 T	0.39	0.06
K -S4	0.44	8467 T	0.38	0.06
S4-J	0.46	8467 T	0.38	0.08
J -I	0.37	6803 T	0.31	0.06
I -E	0.40	7748 T	0.35	0.05
E -H	0.53	7741 T	0.35	0.18
H -D	0.53	7703 T	0.35	0.18
Chord-Webs				
G -F	0.76	3527 C	0.04	0.72
F -Q	0.27	4479 C	0.27	0.00
Webs				
O -P	0.33	1053 T		
O -F	0.51	1697 C		
P -F	0.78	3052 C		
Q -N	0.97	5271 T		
N -B	0.23	1326 C		
B -M	0.82	4449 T		1 Br
M -R	0.24	1355 C		
R -L	0.78	3199 T		
L -S	0.09	514 C		
L -T	0.32	1299 C		1 Br
K -T	0.05	216 T		
K -U	0.15	213 T		
J -U	0.07	448 C		
J -C	0.56	1757 T		
I -C	0.42	1317 T		
I -V	0.26	1509 C		
E -V	0.55	1724 T		
E -W	0.08	250 T		
H -W	0.03	146 T		

TL Defl -1.91" in S4-J L/327
LL Defl -0.95" in S4-J L/660

Hz Disp	LL	DL	TL
Jt D	0.26"	0.26"	0.52"

Shear // Grain in Q -B 0.36

Plates for each ply each face.
Plate - LOCK 20 Ga, Gross Area

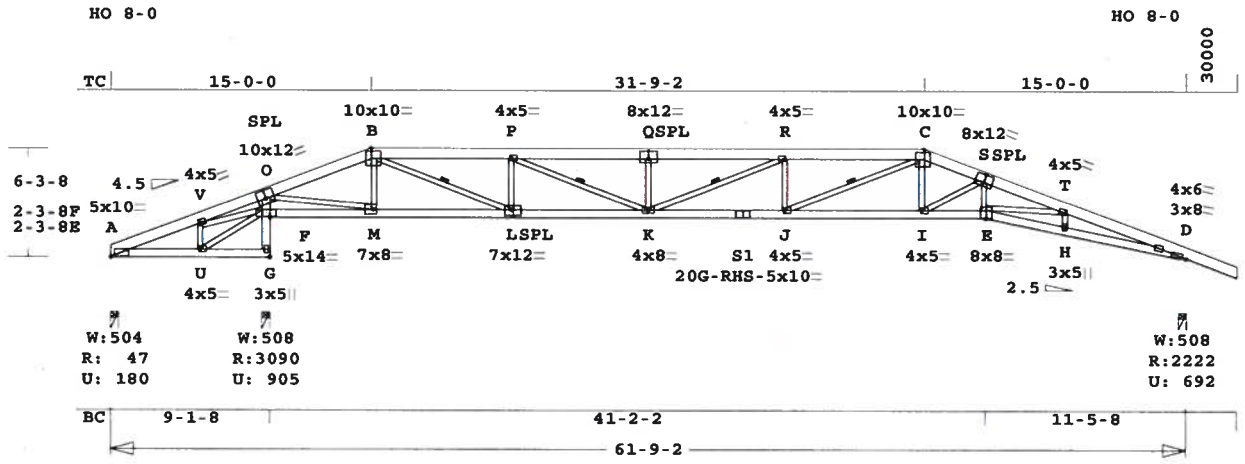
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
P	LOCK	5.0x 6.0	-0.5	0.2	0.84
Q	LOCK	5.0x10.0	0.6	2.3	0.97
B	LOCK	10.0x10.0	-0.5	0.2	0.98
R	LOCK	5.0x 5.0	-0.6	1.5	0.97
S1	LOCK	6.0x12.0	-0.5	Ctr	0.91
S	LOCK	3.0x 5.0	Ctr	Ctr	0.41
T	LOCK	4.0x 5.0	0.6	1.5	0.68
S2	LOCK	6.0x12.0	0.6	1.5	0.91
U	LOCK	4.0x 5.0	0.5	0.2	0.68
C	LOCK	10.0x10.0	Ctr	Ctr	0.89
V	LOCK	4.0x 5.0	Ctr	Ctr	0.85
W	LOCK	8.0x12.0	Ctr	Ctr	0.97
D	LOCK	4.0x 6.0	Ctr	Ctr	1.00
D					0.78
O	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.74
F	LOCK	8.0x14.0	Ctr	Ctr	0.72
N	LOCK	7.0x10.0	Ctr	Ctr	0.98
M	LOCK	4.0x 8.0	Ctr	Ctr	0.92
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 8.0	0.5	Ctr	0.99
K	LOCK	4.0x 5.0	Ctr	-1.5	0.60
S4	LOCK	7.0x10.0	Ctr	Ctr	0.97
J	LOCK	4.0x 5.0	Ctr	Ctr	0.79
I	LOCK	4.0x 5.0	Ctr	Ctr	0.62
E	LOCK	8.0x 8.0	Ctr	Ctr	0.88
H	LOCK	2.0x 4.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8665 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R71*	1	SP	610902	4.5	0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



EXCEPT AS SHOWN ALL PLATES ARE LOCK20

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 577.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 8	SP-2400
BC	0.99	2x 6	SP-#2
EX	S1-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.50	2x 6	SP-#2
WB	0.63	2x 4	SP-#3
EX	O -M	2x 4	SP-#2
EX	B -L	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -L
WB 1 rows CLB on P -K
WB 1 rows CLB on K -R
WB 1 rows CLB on J -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	47	180G	5- 4	1- 8
			Hz =	-114
G	3091	905	5- 8	3- 7
D	2223	692	5- 8	1-13
			Hz =	116

G = Gravity Uplift

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -V	0.08	985 T	0.03 0.05
V -O	0.43	2905 T	0.10 0.33
O -B	0.33	2269 C	0.00 0.33
B -P	0.14	5258 C	0.02 0.12
P -Q	0.17	7065 C	0.06 0.11
Q -R	0.15	7065 C	0.06 0.09
R -C	0.16	7301 C	0.06 0.10
C -S	0.16	6493 C	0.04 0.12
S -T	0.31	8647 C	0.09 0.22
T -D	0.28	8474 C	0.08 0.20
--------Bottom Chords---------			
A -U	0.08	908 C	0.00 0.08
U -G	0.07	202 C	0.00 0.07
F -M	0.48	2573 C	0.01 0.47
M -L	0.36	2067 T	0.25 0.11
L -K	0.75	5258 T	0.63 0.12
K -S1	0.99	7301 T	0.88 0.11
S1-J	0.38	7301 T	0.33 0.05
J -I	0.33	6088 T	0.27 0.06
I -E	0.40	8046 T	0.36 0.04
E -H	0.55	8037 T	0.36 0.19
H -D	0.55	7999 T	0.36 0.19
----------Chord-Webs----------			
G -F	0.50	3071 C	0.03 0.47
F -O	0.21	3481 C	0.21 0.00
------------Webs------------			
U -V	0.18	565 T	
U -F	0.25	824 C	
V -F	0.43	1709 C	
O -M	0.86	4689 T	
M -B	0.16	779 C	
B -L	0.63	3443 T	1 Br
L -P	0.24	1153 C	
P -K	0.63	1974 T	1 Br
K -Q	0.10	523 C	
K -R	0.07	258 C	1 Br
J -R	0.07	345 C	
J -C	0.42	1309 T	1 Br
I -C	0.40	1275 T	
I -S	0.58	2239 C	
E -S	0.57	1793 T	
E -T	0.08	260 T	
H -T	0.03	142 T	

TL Defl -1.50" in S1-J L/416
LL Defl -0.75" in S1-J L/839

Hz Disp	LL	DL	TL
Jt D	0.24"	0.24"	0.48"

Shear // Grain in O -B 0.24

Plates for each ply each face.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

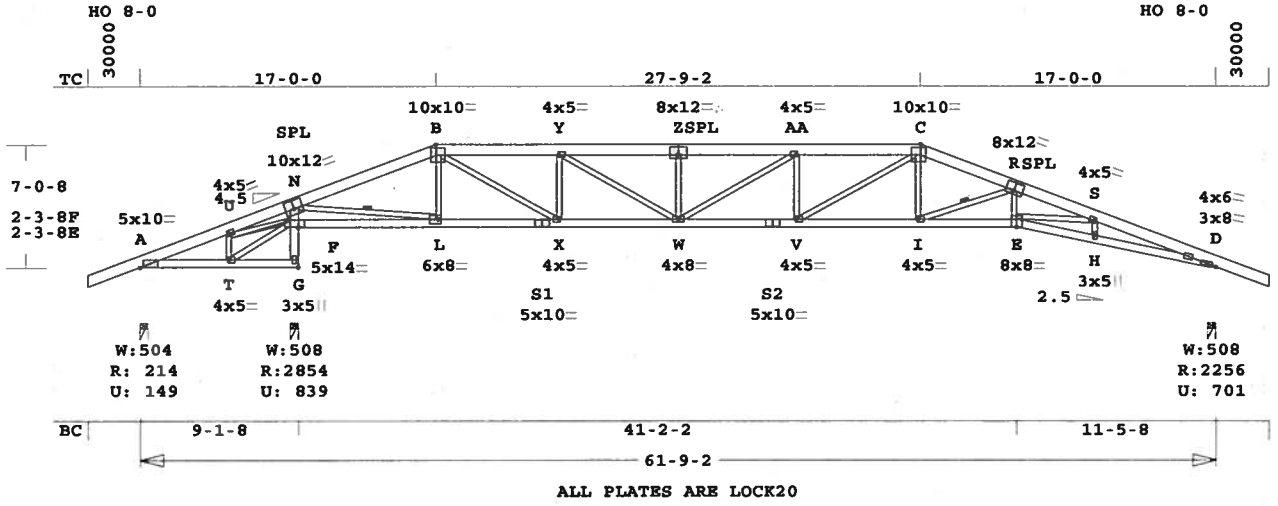
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
V	LOCK	4.0x 5.0-0.5	0.2	0.87	
O	LOCK	10.0x12.0	0.6	2.3	0.98
B	LOCK	10.0x10.0-0.5	0.2	0.89	
P	LOCK	4.0x 5.0-0.6	1.5	0.96	
Q	LOCK	8.0x12.0-0.5	Ctr	0.99	
R	LOCK	4.0x 5.0	Ctr	Ctr	0.65
C	LOCK	10.0x10.0	0.6	1.5	0.83
S	LOCK	8.0x12.0	0.6	1.5	0.97
T	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	3.0x 8.0	Ctr	Ctr	0.56
D					0.59
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	5.0x14.0	Ctr	1.2	0.78
M	LOCK	7.0x 8.0	Ctr	Ctr	0.88
L	LOCK	7.0x12.0	Ctr	Ctr	0.97
K	LOCK	4.0x 8.0	Ctr	Ctr	0.86
S1	RHS	5.0x10.0	Ctr	Ctr	0.98
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	Ctr	Ctr	0.72
E	LOCK	8.0x 8.0	0.5	Ctr	0.88
H	LOCK	3.0x 5.0	Ctr-1.5		0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8647 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R70	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 599.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 8	SP-2400
BC	0.83	2x 6	SP-#2
EX	S2-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.36	2x 6	SP-#2
WB	0.79	2x 4	SP-#3
EX	N -L	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on N -L
WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	215	149	5- 4	1- 8
		Hz =	-130	
G	2855	840	5- 8	3- 2
D	2256	702	5- 8	1-14
		Hz =	131	

Membr	CSI	P Lbs	Ax	l	CSI	Bnd
----------Top Chords----------						
A -U	0.05	471	T	0.01	0.04	
U -N	0.33	1534	T	0.05	0.28	
N -B	0.29	3071	C	0.01	0.28	
B -Y	0.09	4900	C	0.02	0.07	
Y -Z	0.12	6036	C	0.04	0.08	
Z -AA	0.10	6036	C	0.04	0.06	
AA-C	0.11	6238	C	0.04	0.07	
C -R	0.17	5877	C	0.04	0.13	
R -S	0.31	8827	C	0.09	0.22	
S -D	0.28	8648	C	0.08	0.20	
--------Bottom Chords---------						
A -T	0.06	431	C	0.00	0.06	
T -G	0.06	146	C	0.00	0.06	
F -L	0.44	1216	C	0.00	0.44	
L -S1	0.44	2837	T	0.34	0.10	
S1-X	0.36	2837	T	0.34	0.02	
X -W	0.67	4900	T	0.59	0.08	
W -S2	0.83	6237	T	0.75	0.08	
S2-V	0.30	6237	T	0.28	0.02	
V -I	0.29	5508	T	0.25	0.04	
I -E	0.41	8213	T	0.37	0.04	
E -H	0.56	8201	T	0.37	0.19	
H -D	0.56	8164	T	0.37	0.19	
----------Chord-Webs----------						
G -F	0.36	2832	C	0.02	0.34	
F -N	0.18	2926	C	0.03	0.15	
------------Webs------------						
T -U	0.09	312	T			
T -F	0.10	333	C			
U -F	0.23	914	C			
N -L	0.75	4079	T			1 Br
L -B	0.12	477	C			
B -X	0.79	2375	T			
X -Y	0.27	1038	C			
Y -W	0.45	1336	T			
W -Z	0.11	440	C			
W -AA	0.20	237	C			
V -AA	0.07	282	C			
V -C	0.27	839	T			
I -C	0.37	1166	T			
I -R	0.57	2870	C			1 Br
E -R	0.58	1836	T			
E -S	0.08	267	T			
H -S	0.03	137	T			

TL Defl -1.27" in V -I L/493
LL Defl -0.63" in V -I L/998

Hz Disp	LL	DL	TL
Jt D	0.23"	0.24"	0.47"

Shear // Grain in U -N 0.21

Plates for each ply each face.
Plate - LOCK 20 Ga, Gross Area

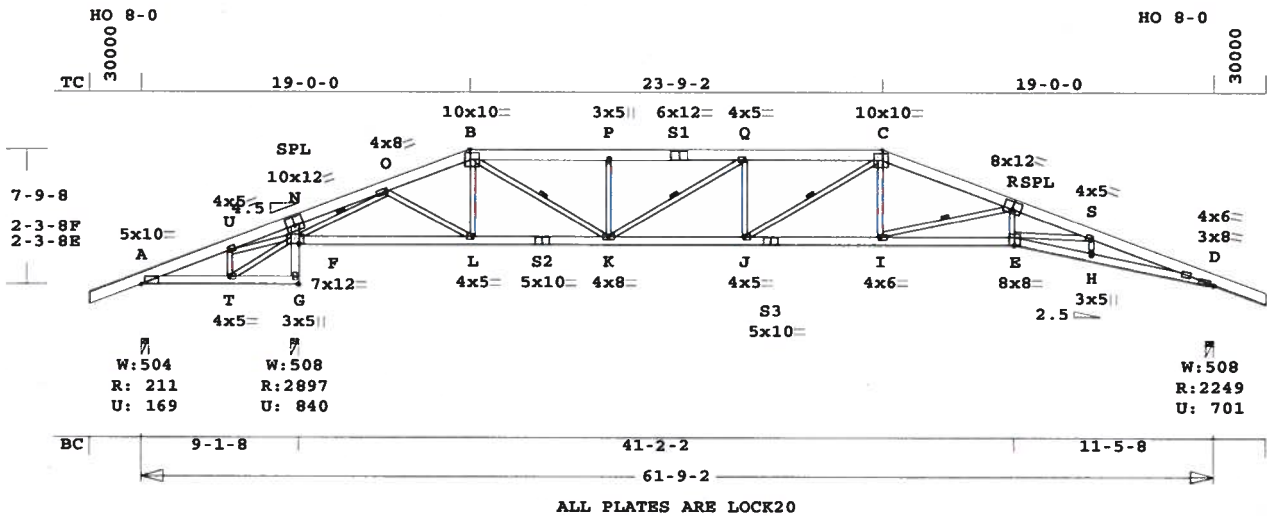
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
U	LOCK	4.0x 5.0	-0.5	0.2	0.87
N	LOCK	10.0x12.0	0.6	2.3	0.89
B	LOCK	10.0x10.0	-0.5	0.2	0.86
Y	LOCK	4.0x 5.0	-0.6	1.5	0.63
Z	LOCK	8.0x12.0	-0.5	Ctr	0.99
AA	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	10.0x10.0	0.6	1.5	0.76
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	4.0x 5.0	0.5	0.2	0.63
D	LOCK	3.0x 8.0	Ctr	Ctr	0.59
D					0.59
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	5.0x14.0	Ctr	1.2	0.78
L	LOCK	6.0x 8.0	Ctr	Ctr	0.88
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
X	LOCK	4.0x 5.0	Ctr	Ctr	0.90
W	LOCK	4.0x 8.0	Ctr	Ctr	0.53
S2	LOCK	5.0x10.0	Ctr	Ctr	0.93
V	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	0.5	Ctr	0.94
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8827 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R69	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 602.2 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 8	SP-2400
BC	0.72	2x 6	SP-#2
EX	S3-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.35	2x 6	SP-#2
WB	0.90	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on F -O
WB 1 rows CLB on B -K
WB 1 rows CLB on K -Q
WB 1 rows CLB on J -C
WB 1 rows CLB on I -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	212	170	5- 4	1- 8
			Hz =	-145
G	2898	840	5- 8	3- 3
D	2250	702	5- 8	1-14
			Hz =	146

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -U	0.06	566	T	0.01	0.05
U -N	0.33	1859	T	0.06	0.27
N -O	0.32	1586	T	0.05	0.27
O -B	0.10	3139	C	0.01	0.09
B -P	0.08	4642	C	0.02	0.06
P -S1	0.09	4642	C	0.02	0.07
S1-Q	0.09	4642	C	0.02	0.07
Q -C	0.11	5315	C	0.03	0.08
C -R	0.19	5259	C	0.03	0.16
R -S	0.31	8810	C	0.09	0.22
S -D	0.28	8631	C	0.08	0.20
Bottom Chords					
A -T	0.07	519	C	0.00	0.07
T -G	0.06	149	C	0.00	0.06
F -L	0.65	2164	T	0.26	0.39
L -S2	0.43	2956	T	0.35	0.08
S2-K	0.43	2956	T	0.35	0.08
K -J	0.72	5314	T	0.64	0.08
J -S3	0.64	4925	T	0.59	0.05
S3-I	0.26	4925	T	0.22	0.04
I -E	0.41	8201	T	0.37	0.04
E -H	0.55	8185	T	0.37	0.18
H -D	0.55	8149	T	0.37	0.18
Chord-Webs					
G -F	0.35	2875	C	0.02	0.33
F -N	0.16	824	C	0.00	0.16
Webs					
T -U	0.11	372	T		
T -F	0.13	438	C		
U -F	0.28	1133	C		
F -O	0.88	4340	C		1 Br
O -L	0.29	927	T		
L -B	0.12	350	C		
B -K	0.63	1968	T		1 Br
K -P	0.14	432	C		
K -Q	0.26	797	C		1 Br
J -Q	0.04	197	T		
J -C	0.14	455	T		1 Br
I -C	0.35	1097	T		
I -R	0.90	3383	C		1 Br
E -R	0.58	1836	T		
E -S	0.08	271	T		
H -S	0.03	127	T		

TL Defl	-1.20" in I -E	L/523	
LL Defl	-0.59" in I -E	L/999	
Hz Disp	LL	DL	TL
Jt D	0.24"	0.24"	0.49"
Shear // Grain		in U -N	0.21

Plates for each ply each face.

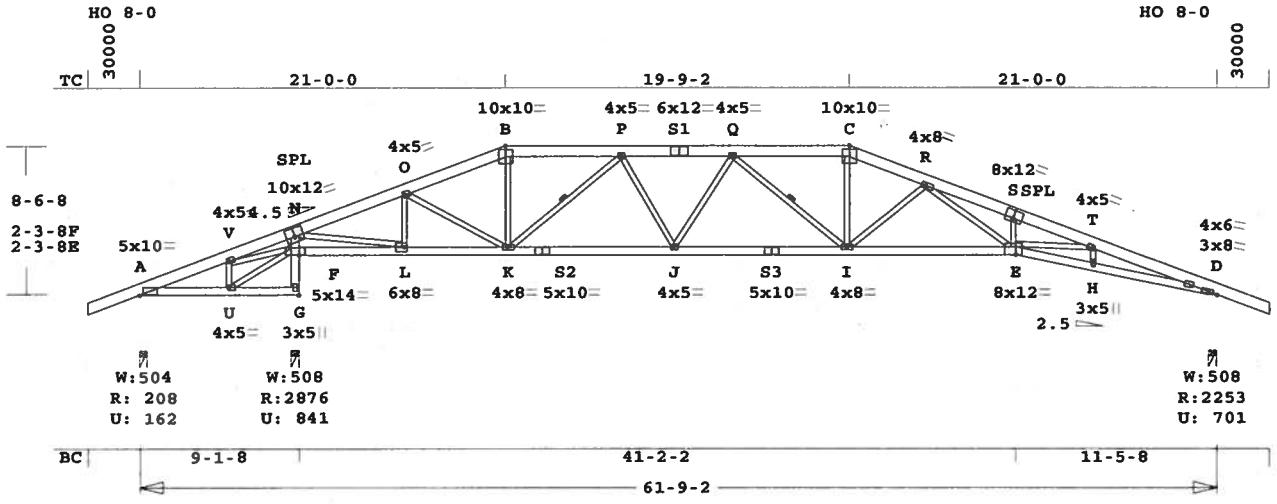
Plate - LOCK 20 Ga, Gross Area					
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
U	LOCK	4.0x 5.0-0.5	0.2	0.2	0.87
N	LOCK	10.0x12.0	0.6	2.3	0.89
O	LOCK	4.0x 8.0-0.5	0.2	0.2	0.92
B	LOCK	10.0x10.0-0.6	1.5	0.5	0.76
P	LOCK	3.0x 5.0-0.5	Ctr	0.4	0.41
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	0.6	1.5	0.60
C	LOCK	10.0x10.0	0.6	1.5	0.76
R	LOCK	8.0x12.0	0.5	0.2	0.97
S	LOCK	4.0x 5.0	Ctr	Ctr	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.59
D					0.54
T	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	1.2	0.73
F	LOCK	7.0x12.0	Ctr	Ctr	0.97
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.78
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	5.0x10.0	Ctr	Ctr	0.82
I					0.82
E	LOCK	8.0x 8.0	Ctr	-1.5	0.89
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8810 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R68	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 609.5 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 8	SP-2400
BC	0.87	2x 6	SP-#2
EX E -D		2x 6	SP-2400
CW	0.38	2x 6	SP-#2
WB	0.81	2x 4	SP-#3
EX N -L		2x 4	SP-#2
EX R -E		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on K -P
WB 1 rows CLB on Q -I
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	208	162	5- 4	1- 8
			Hz =	-161
G	2877	841	5- 8	3- 3
D	2253	702	5- 8	1-14
			Hz =	162

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -V	0.05	513	T	0.01	0.04
V -N	0.29	1813	T	0.06	0.23
N -O	0.23	2683	C	0.00	0.23
O -B	0.09	3271	C	0.01	0.08
B -P	0.07	3077	C	0.01	0.06
P -S1	0.06	4381	C	0.02	0.04
S1-Q	0.06	4381	C	0.02	0.04
Q -C	0.08	4478	C	0.02	0.06
C -R	0.07	4784	C	0.05	0.02
R -S	0.21	9013	C	0.09	0.12
S -T	0.34	9024	C	0.09	0.25
T -D	0.32	8405	C	0.08	0.24
--------Bottom Chords---------					
A -U	0.06	468	C	0.00	0.06
U -G	0.06	155	C	0.00	0.06
F -L	0.43	1518	C	0.00	0.43
L -K	0.41	2456	T	0.29	0.12
K -S2	0.62	4113	T	0.50	0.12
S2-J	0.59	4113	T	0.50	0.09
J -S3	0.64	4572	T	0.55	0.09
S3-I	0.74	4572	T	0.55	0.19
I -E	0.87	5661	T	0.68	0.19
E -H	0.63	7989	T	0.36	0.27
H -D	0.63	7925	T	0.36	0.27
----------Chord-Webs----------					
G -F	0.38	2853	C	0.02	0.36
F -N	0.17	2963	C	0.17	0.00
------------Webs-------------					
U -V	0.10	338	T		
U -F	0.11	366	C		
V -F	0.29	1155	C		
N -L	0.74	4042	T		
L -O	0.15	747	C		
O -K	0.23	720	T		
K -B	0.23	743	T		
K -P	0.42	1377	C	1 Br	
P -J	0.18	574	T		
J -Q	0.23	376	C		
Q -I	0.04	181	T	1 Br	
I -C	0.43	1361	T		
I -R	0.81	1574	C		
R -E	0.66	3535	T		
E -S	0.05	264	C		
E -T	0.22	693	T		
H -T	0.04	224	C		

TL Defl	-1.16" in I -E	L/538
LL Defl	-0.55" in I -E	L/999
Hz Disp	LL	DL TL
Jt D	0.22"	0.22" 0.43"
Shear // Grain		in T -D 0.21

Plates for each ply each face.
Plate - LOCK 20 Ga, Gross Area

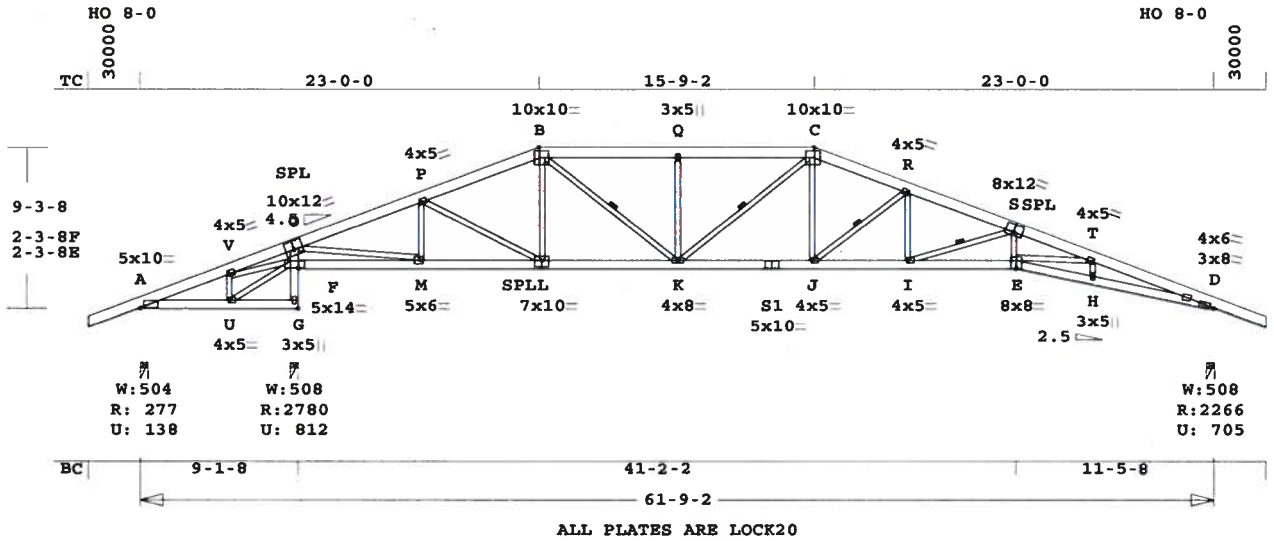
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
V	LOCK	4.0x 5.0-0.5	0.2		0.87
N	LOCK	10.0x12.0	0.6	2.3	0.89
O	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	10.0x10.0-0.6	1.5		0.76
P	LOCK	4.0x 5.0-0.5	Ctr		0.60
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
Q	LOCK	4.0x 5.0	0.6	1.5	0.59
C	LOCK	10.0x10.0	0.6	1.5	0.76
R	LOCK	4.0x 8.0	0.5	0.2	0.82
S	LOCK	8.0x12.0	Ctr	Ctr	0.97
T	LOCK	4.0x 5.0	Ctr	Ctr	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.54
D					0.73
U	LOCK	4.0x 5.0	Ctr	1.2	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	5.0x14.0	Ctr	Ctr	0.78
L	LOCK	6.0x 8.0	Ctr	Ctr	0.87
K	LOCK	4.0x 8.0	Ctr	Ctr	0.53
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 5.0	Ctr	Ctr	0.54
S3	LOCK	5.0x10.0	0.5	Ctr	0.82
I	LOCK	4.0x 8.0	Ctr-1.5		0.54
E	LOCK	8.0x12.0	Ctr	Ctr	0.89
H	LOCK	3.0x 5.0	Ctr	Ctr	0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9024 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R67	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 620.1 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.56	2x 6	SP-#2
EX	S1-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.32	2x 6	SP-#2
WB	0.65	2x 4	SP-#3
EX	O -M	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -R
WB 1 rows CLB on I -S
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	278	139	5- 4	1- 8
			Hz =	-176
G	2780	813	5- 8	3- 1
D	2267	706	5- 8	1-14
			Hz =	178

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -V	0.05	304 T	0.01 0.04
V -O	0.26	1241 T	0.04 0.22
O -P	0.23	3079 C	0.01 0.22
P -B	0.10	3339 C	0.01 0.09
B -Q	0.09	4018 C	0.01 0.08
Q -C	0.10	4018 C	0.01 0.09
C -R	0.08	4361 C	0.05 0.03
R -S	0.17	5731 C	0.04 0.13
S -T	0.31	8909 C	0.09 0.22
T -D	0.29	8707 C	0.09 0.20
Bottom Chords			
A -U	0.06	275 C	0.00 0.06
U -G	0.05	129 C	0.00 0.05
F -M	0.41	958 C	0.00 0.41
M -L	0.45	2843 T	0.34 0.11
L -K	0.49	3126 T	0.38 0.11
K -S1	0.56	4066 T	0.49 0.07
S1-J	0.21	4066 T	0.18 0.03
J -I	0.27	5396 T	0.24 0.03
I -E	0.41	8294 T	0.37 0.04
E -H	0.56	8258 T	0.37 0.19
H -D	0.56	8221 T	0.37 0.19
Chord-Webs			
G -F	0.32	2755 C	0.02 0.30
F -O	0.18	2752 C	0.02 0.16
Webs			
U -V	0.06	257 T	
U -F	0.05	170 C	
V -F	0.20	816 C	
O -M	0.93	3849 T	
M -P	0.13	562 C	
P -L	0.10	325 T	
L -B	0.04	183 T	
B -K	0.36	1143 T	1 Br
K -Q	0.30	527 C	
K -C	0.05	146 C	1 Br
J -C	0.38	1202 T	
J -R	0.38	1683 C	1 Br
I -R	0.36	1127 T	
I -S	0.65	3080 C	1 Br
E -S	0.59	1857 T	
E -T	0.09	294 T	
H -T	0.03	137 T	

TL Defl -1.11" in I -E L/564
LL Defl -0.55" in I -E L/999
Hz Disp LL DL TL
Jt D 0.22" 0.22" 0.44"
Shear // Grain in T -D 0.21

Plates for each ply each face.
Plate - LOCK 20 Ga, Gross Area

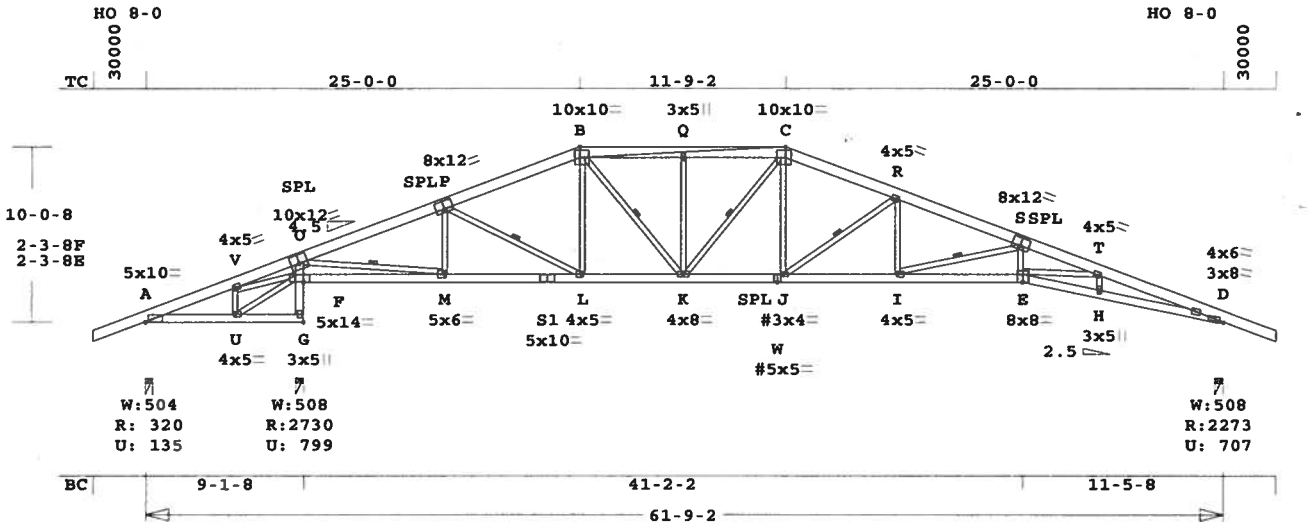
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
V	LOCK	4.0x 5.0-0.5	0.2		0.87
O	LOCK	10.0x12.0	0.6	2.3	0.89
P	LOCK	4.0x 5.0-0.5	0.2		0.56
B	LOCK	10.0x10.0-0.6	1.5		0.76
Q	LOCK	3.0x 5.0-0.5	Ctr		0.41
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
R	LOCK	4.0x 5.0	0.6	1.5	0.80
S	LOCK	8.0x12.0	0.6	1.5	0.97
T	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.60
D					0.79
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	5.0x14.0	Ctr	1.2	0.78
M	LOCK	5.0x 6.0	Ctr	Ctr	1.00
L	LOCK	7.0x10.0	Ctr	Ctr	0.97
K	LOCK	4.0x 8.0	Ctr	Ctr	0.51
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I					0.94
E	LOCK	8.0x 8.0	0.5	Ctr	0.90
H	LOCK	3.0x 5.0	Ctr-1.5		0.41

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8909 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R66*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 631.0 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.49	2x 6	SP-#2
EX W -E		2x 6	SP-2400
EX E -D		2x 6	SP-2400
CW	0.29	2x 6	SP-#2
WB	0.81	2x 4	SP-#3
EX O -M		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	61- 9- 2
BC Cont.		0- 0- 0	61- 9- 2

WB 1 rows CLB on O -M
WB 1 rows CLB on P -L
WB 1 rows CLB on B -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -R
WB 1 rows CLB on I -S
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	320	135	5- 4	1- 8
			Hz =	-192
G	2731	800	5- 8	3- 0
D	2274	707	5- 8	1-14
			Hz =	193

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -V	0.04	197	T	0.00	0.04
V -O	0.26	913	T	0.03	0.23
O -P	0.24	3299	C	0.01	0.23
P -B	0.11	3316	C	0.01	0.10
B -Q	0.05	3602	C	0.04	0.01
Q -C	0.06	3602	C	0.01	0.05
C -R	0.09	3981	C	0.04	0.05
R -S	0.19	5481	C	0.04	0.15
S -T	0.31	8941	C	0.09	0.22
T -D	0.30	8747	C	0.09	0.21
Bottom Chords					
A -U	0.06	176	C	0.00	0.06
U -G	0.05	116	C	0.00	0.05
F -M	0.41	628	C	0.00	0.41
M -S1	0.49	3056	T	0.37	0.12
S1-L	0.47	3056	T	0.37	0.10
L -K	0.47	3100	T	0.37	0.10
K -W	0.48	3704	T	0.45	0.03
W -J	0.18	3704	T	0.16	0.02
J -I	0.26	5163	T	0.23	0.03
I -E	0.42	8324	T	0.38	0.04
E -H	0.56	8295	T	0.37	0.19
H -D	0.56	8258	T	0.37	0.19
Chord-Webs					
G -F	0.29	2705	C	0.02	0.27
F -O	0.20	2640	C	0.02	0.18
Webs					
U -V	0.05	213	T		
U -F	0.03	109	T		
V -F	0.15	605	C		
O -M	0.68	3720	T		1 Br
M -P	0.11	429	C		
P -L	0.04	178	T		1 Br
L -B	0.07	268	T		
B -K	0.25	789	T		1 Br
K -Q	0.27	386	C		
K -C	0.05	168	C		1 Br
J -C	0.38	1199	T		
J -R	0.48	1797	C		1 Br
I -R	0.35	1098	T		
I -S	0.81	3306	C		1 Br
E -S	0.59	1862	T		
E -T	0.09	287	T		
H -T	0.03	135	T		

TL Defl -1.13" in I -E L/554
LL Defl -0.55" in I -E L/999

Hz Disp	LL	DL	TL
Jt D	0.22"	0.22"	0.45"

Shear // Grain in T -D 0.21

Plates for each ply each face.
Plate - LOCK 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
V	LOCK	4.0x 5.0	-0.5	0.2	0.87
O	LOCK	10.0x12.0	0.6	2.3	0.89
P	LOCK	8.0x12.0	-0.5	0.2	0.97
B	LOCK	10.0x10.0	-0.6	1.5	0.76
Q	LOCK	3.0x 5.0	-0.5	Ctr	0.41
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
R	LOCK	4.0x 5.0	0.6	1.5	0.85
S					0.97
T	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.61
D					0.79
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	5.0x14.0	Ctr	1.2	0.77
M	LOCK	5.0x 6.0	Ctr	Ctr	0.96
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	4.0x 8.0	Ctr	Ctr	0.50

W# LOCK 5.0x 5.0 Ctr Ctr 0.82
J# LOCK 3.0x 4.0 Ctr Ctr 0.94

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8941 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

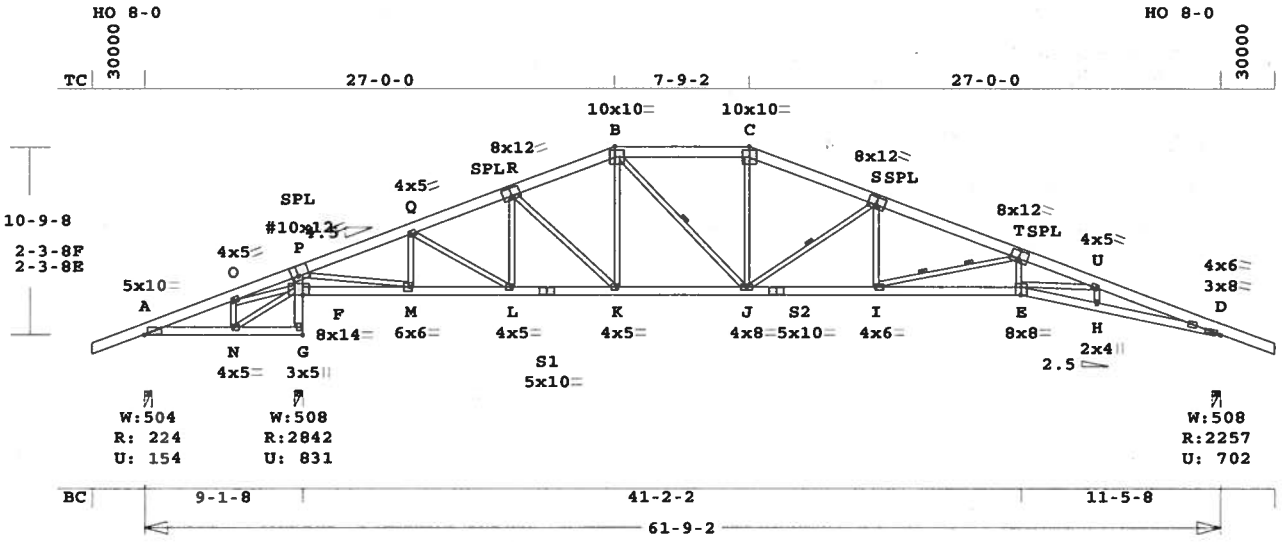
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R65*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	
+Office and Care Center, Hospice, M.M. Parrish								

(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R65	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 629.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

CSI	-Size-	----Lumber----
TC 0.31	2x 8	SP-2400
BC 0.67	2x 6	SP-#2
EX S2-E	2x 6	SP-2400
EX E -D	2x 6	SP-2400
CW 0.35	2x 6	SP-#2
WB 0.68	2x 4	SP-#3
EX P -M	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on B -J
WB 1 rows CLB on J -S
WB 2 rows CLB on I -T
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	225	155	5- 4	1- 8
			Hz =	-208
G	2842	832	5- 8	3- 2
D	2258	703	5- 8	1-14
			Hz =	209

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -O	0.05	438 T	0.01	0.04
O -P	0.28	1624 T	0.05	0.23
P -Q	0.23	2783 C	0.00	0.23
Q -R	0.08	3292 C	0.01	0.07
R -B	0.06	3135 C	0.03	0.03
B -C	0.10	3317 C	0.01	0.09
C -S	0.12	3559 C	0.01	0.11
S -T	0.19	5141 C	0.03	0.16
T -U	0.31	8857 C	0.09	0.22
U -D	0.29	8677 C	0.09	0.20
--------Bottom Chords---------				
A -N	0.06	398 C	0.00	0.06
N -G	0.05	144 C	0.00	0.05
F -M	0.41	1340 C	0.00	0.41
M -L	0.42	2555 T	0.31	0.11
L -S1	0.42	3098 T	0.37	0.05
S1-K	0.42	3098 T	0.37	0.05
K -J	0.44	2927 T	0.35	0.09
J -S2	0.67	4849 T	0.58	0.09
S2-I	0.25	4849 T	0.22	0.03
I -E	0.41	8247 T	0.37	0.04
E -H	0.55	8230 T	0.37	0.18
H -D	0.55	8194 T	0.37	0.18
----------Chord-Webs----------				
G -F	0.35	2818 C	0.02	0.33
F -P	0.17	2892 C	0.17	0.00
------------Webs-------------				
N -O	0.08	311 T		
N -F	0.08	297 C		
O -F	0.26	1051 C		
P -M	0.74	3959 T		
M -Q	0.15	697 C		
Q -L	0.20	635 T		
L -R	0.08	185 C		
R -K	0.21	231 C		
K -B	0.10	377 T		
B -J	0.18	561 T		1 Br
J -C	0.27	842 T		
J -S	0.60	1879 C		1 Br
I -S	0.33	1053 T		
I -T	0.68	3516 C		2 Br
E -T	0.59	1853 T		
E -U	0.08	273 T		
H -U	0.03	130 T		

TL Defl -1.14" in I -E L/551
LL Defl -0.55" in I -E L/999

Hz Disp	LL	DL	TL
Jt D	0.22"	0.22"	0.45"

Shear // Grain in U -D 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
O	LOCK	4.0x 5.0	-0.5	0.2	0.87
P#	LOCK	10.0x12.0	0.6	2.3	0.78
Q	LOCK	4.0x 5.0	-0.5	0.2	0.56
R	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	-0.5	Ctr	0.76
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
S	LOCK	8.0x12.0	0.6	1.5	0.97
T	LOCK	8.0x12.0	0.6	1.5	0.97
U	LOCK	4.0x 5.0	0.5	0.2	0.62
D	LOCK	3.0x 8.0	Ctr	Ctr	0.91
D	LOCK	4.0x 6.0	Ctr	Ctr	0.79
N	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	8.0x14.0	Ctr	1.2	0.70
M	LOCK	6.0x 6.0	Ctr	Ctr	0.99
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
J	LOCK	4.0x 8.0	Ctr	Ctr	0.66
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
I	LOCK	4.0x 6.0	0.5	Ctr	0.98
E	LOCK	8.0x 8.0	Ctr	-1.5	0.90
H	LOCK	2.0x 4.0	Ctr	Ctr	0.61

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8857 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R64*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish

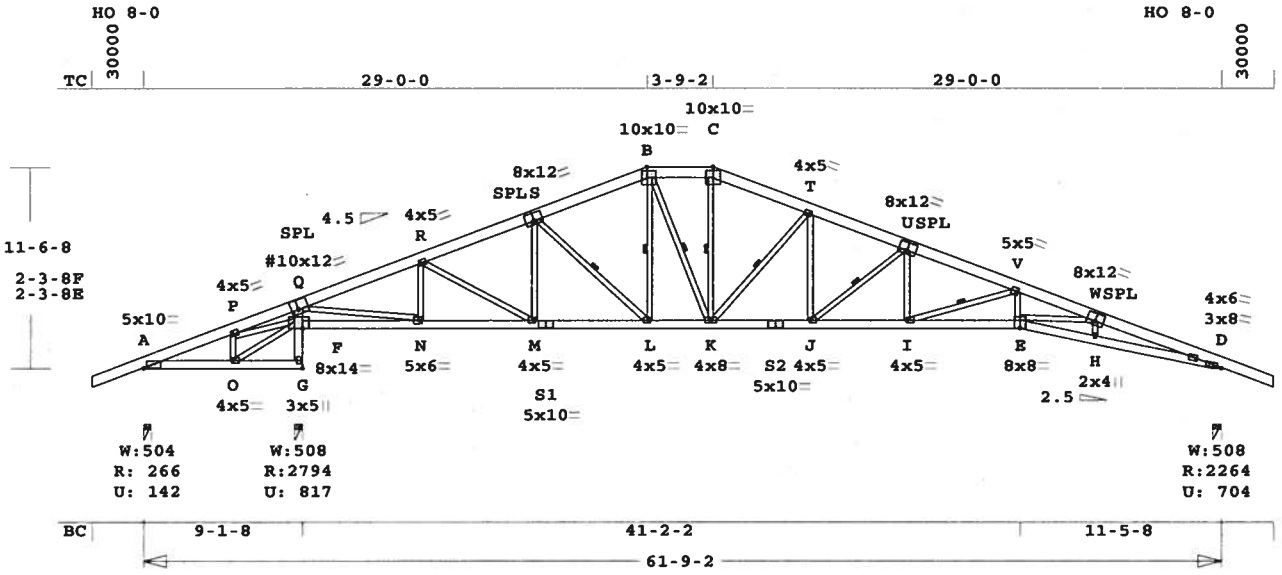
Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R64	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 654.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.53	2x 6	SP-#2
EX	S2-E	2x 6	SP-2400
EX	E -D	2x 6	SP-2400
CW	0.32	2x 6	SP-#2
WB	0.68	2x 4	SP-#3
EX	Q -N	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on S -L
WB 1 rows CLB on L -B
WB 1 rows CLB on B -K
WB 1 rows CLB on K -C
WB 1 rows CLB on K -T
WB 1 rows CLB on J -U
WB 1 rows CLB on I -V
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	266	142	5- 4	1- 8
			Hz =	-223
G	2794	818	5- 8	3- 1
D	2265	705	5- 8	1-14
			Hz =	224

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -P	0.05	334	T	0.01	0.04
P -Q	0.26	1322	T	0.04	0.22
Q -R	0.23	3006	C	0.01	0.22
R -S	0.08	3345	C	0.01	0.07
S -B	0.07	3045	C	0.03	0.04
B -C	0.04	3005	C	0.04	0.00
C -T	0.07	3208	C	0.04	0.03
T -U	0.08	4266	C	0.05	0.03
U -V	0.18	5658	C	0.04	0.14
V -W	0.31	8888	C	0.09	0.22
W -D	0.29	8703	C	0.09	0.20
Bottom Chords					
A -O	0.06	302	C	0.00	0.06
O -G	0.05	132	C	0.00	0.05
F -N	0.41	1040	C	0.00	0.41
N -M	0.44	2770	T	0.33	0.11
M -S1	0.45	3147	T	0.38	0.07
S1-L	0.42	3147	T	0.38	0.04
L -K	0.39	2840	T	0.34	0.05
K -S2	0.53	3983	T	0.48	0.05
S2-J	0.19	3983	T	0.18	0.01
J -I	0.27	5333	T	0.24	0.03
I -E	0.41	8274	T	0.37	0.04
E -H	0.56	8255	T	0.37	0.19
H -D	0.56	8218	T	0.37	0.19
Chord-Webs					
G -F	0.32	2769	C	0.02	0.30
F -Q	0.17	2784	C	0.02	0.15
Webs					
O -P	0.07	270	T		
O -F	0.05	200	C		
P -F	0.21	863	C		
Q -N	0.89	3861	T		
N -R	0.13	589	C		
R -M	0.13	436	T		
M -S	0.03	127	T		
S -L	0.13	414	C		1 Br
L -B	0.11	394	T		1 Br
B -K	0.14	440	T		1 Br
K -C	0.26	838	T		1 Br
K -T	0.45	1502	C		1 Br
J -T	0.38	1213	T		
J -U	0.40	1745	C		1 Br
I -U	0.35	1114	T		
I -V	0.68	3110	C		1 Br
E -V	0.59	1854	T		
E -W	0.08	276	T		
H -W	0.03	136	T		

TL Defl -1.09" in I -E L/573
LL Defl -0.54" in I -E L/999

Hz Disp	LL	DL	TL
Jt D	0.21"	0.22"	0.43"

Shear // Grain in W -D 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
P	LOCK	4.0x 5.0-0.5	0.2		0.87
Q#	LOCK	10.0x12.0	0.4	2.2	0.79
R	LOCK	4.0x 5.0-0.5	0.2		0.56
S	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0-0.5	Ctr		0.76
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
T	LOCK	4.0x 5.0	0.5	0.2	0.73
U	LOCK	8.0x12.0	0.6	1.5	0.97
V	LOCK	5.0x 5.0	Ctr	Ctr	0.95
W	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	3.0x 8.0	Ctr	Ctr	0.91
D	LOCK	4.0x 6.0	Ctr	Ctr	0.80
O	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	8.0x14.0	Ctr	1.2	0.70
N	LOCK	5.0x 6.0	Ctr	Ctr	1.00
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	4.0x 8.0	Ctr	Ctr	0.61
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 5.0	0.5	Ctr	0.89
E	LOCK	8.0x 8.0	Ctr	-1.5	0.90
H	LOCK	2.0x 4.0	Ctr	Ctr	0.61

= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8888 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins

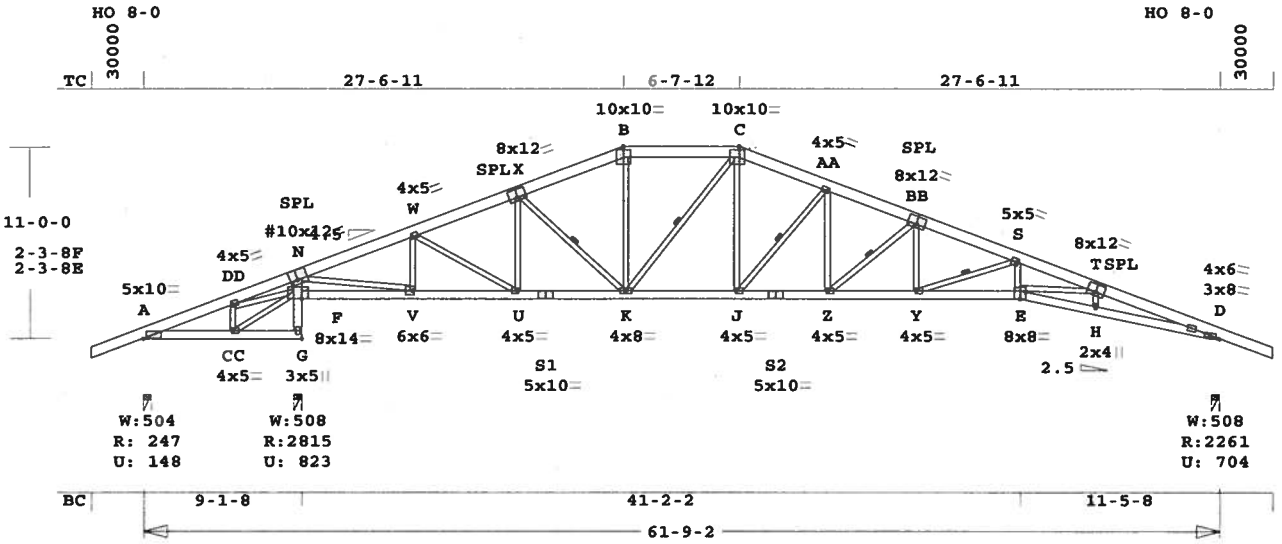
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R63*	3	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish

1. Delegated Engineer (Truss Designer)
   Gary Dounson, PE 35054
   Gary Dounson & Associates, Inc.
   2830 NW 41st Street Suite D
   Gainesville, FL 32606
   (352)375-8593
   CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R63	3	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 645.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.31	2x 8	SP-2400
BC	0.53	2x 6	SP-#2
EX S2-E		2x 6	SP-2400
EX E -D		2x 6	SP-2400
CW	0.33	2x 6	SP-#2
WB	0.61	2x 4	SP-#3
EX N -V		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	61- 9- 2
BC	Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on X -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -AA
WB 1 rows CLB on Z -BB
WB 1 rows CLB on Y -S
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	248	148	5- 4	1- 8
			Hz =	-212
G	2815	824	5- 8	3- 2
D	2262	704	5- 8	1-14
			Hz =	213

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -DD	0.05	379	T	0.01 0.04
DD-N	0.27	1467	T	0.05 0.22
N -W	0.22	2877	C	0.00 0.22
W -X	0.08	3322	C	0.01 0.07
X -B	0.07	3126	C	0.03 0.04
B -C	0.09	2923	C	0.01 0.08
C -AA	0.07	3443	C	0.04 0.03
AA-BB	0.08	4451	C	0.05 0.03
BB-S	0.17	5794	C	0.04 0.13
S -T	0.31	8874	C	0.09 0.22
T -D	0.29	8686	C	0.09 0.20
Bottom Chords				
A -CC	0.06	344	C	0.00 0.06
CC-G	0.05	137	C	0.00 0.05
F -V	0.41	1189	C	0.00 0.41
V -U	0.43	2645	T	0.32 0.11
U -S1	0.42	3124	T	0.37 0.05
S1-K	0.44	3124	T	0.37 0.07
K -J	0.46	3224	T	0.39 0.07
J -S2	0.53	4151	T	0.50 0.03
S2-Z	0.21	4151	T	0.19 0.02
Z -Y	0.28	5462	T	0.25 0.03
Y -E	0.41	8261	T	0.37 0.04
E -H	0.56	8240	T	0.37 0.19
H -D	0.56	8202	T	0.37 0.19
Chord-Webs				
G -F	0.33	2791	C	0.02 0.31
F -N	0.17	2833	C	0.17 0.00
Webs				
CC-DD	0.07	287	T	
CC-F	0.07	242	C	
DD-F	0.24	959	C	
N -V	0.77	3894	T	
V -W	0.14	655	C	
W -U	0.17	558	T	
U -X	0.06	147	T	
X -K	0.08	277	C	1 Br
K -B	0.23	706	T	
K -C	0.19	481	C	1 Br
J -C	0.40	1207	T	
J -AA	0.38	1425	C	1 Br
Z -AA	0.39	1232	T	
Z -BB	0.37	1730	C	1 Br
Y -BB	0.36	1129	T	
Y -S	0.61	2987	C	1 Br
E -S	0.59	1852	T	
E -T	0.08	278	T	
H -T	0.03	138	T	

TL Defl -1.09" in Y -E L/575
LL Defl -0.54" in Y -E L/999

Hz Disp	LL	DL	TL
Jt D	0.21"	0.22"	0.43"

Shear // Grain in T -D 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
DD	LOCK	4.0x 5.0	-0.5	0.2	0.87
N	LOCK	10.0x12.0	0.2	2.2	0.78
W	LOCK	4.0x 5.0	-0.5	0.2	0.56
X	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
C	LOCK	10.0x10.0	0.5	Ctr	0.76
AA	LOCK	4.0x 5.0	0.5	0.2	0.69
BB	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	5.0x 5.0	Ctr	Ctr	0.90
T	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	3.0x 8.0	Ctr	Ctr	0.91
D	LOCK	4.0x 6.0	Ctr	Ctr	0.79
CC	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
F	LOCK	8.0x14.0	Ctr	1.3	0.70
V	LOCK	6.0x 6.0	Ctr	Ctr	0.97
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
K	LOCK	4.0x 8.0	Ctr	Ctr	0.50
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	5.0x10.0	Ctr	Ctr	0.82
Z	LOCK	4.0x 5.0	Ctr	Ctr	0.60
Y	LOCK	4.0x 5.0	Ctr	Ctr	0.98
E	LOCK	8.0x 8.0	Ctr	-1.5	0.90
H	LOCK	2.0x 4.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8874 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:

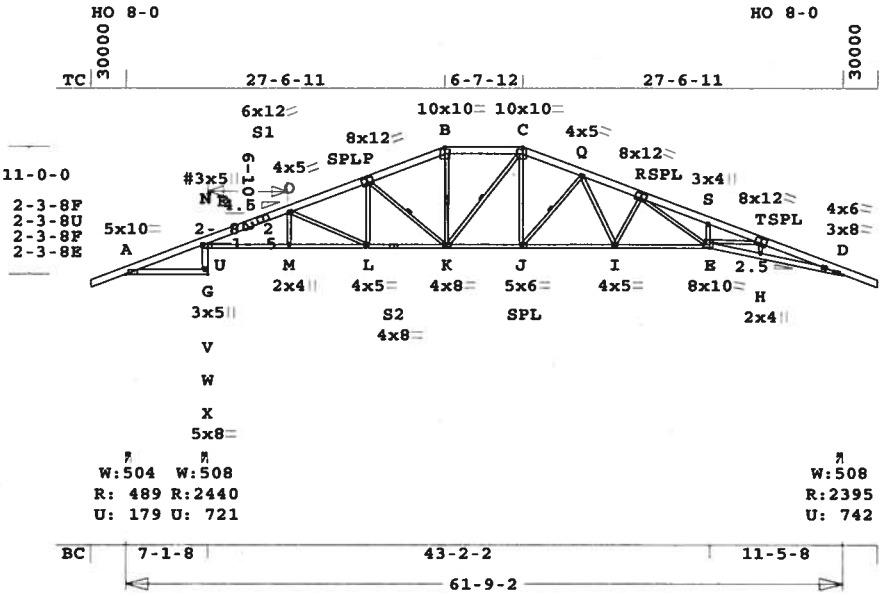
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R62*	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish

Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R62	1	SP	610902	4.5	3- 0- 0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.063" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 569.3 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.40	2x 8	SP-2400
BC	0.84	2x 4	SP-2400
EX A -G		2x 6	SP-#2
EX S2-J		2x 4	SP-#2
EX E -D		2x 6	SP-2400
CW	0.92	2x 6	SP-2400
WB	0.53	2x 4	SP-#3
EX R -E		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on P -K
WB 1 rows CLB on K -C
WB 1 rows CLB on J -Q
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	490	179	5- 4	1- 8
		Hz =	-212	
G	2441	721	5- 8	2-11
D	2395	742	5- 8	2- 0
		Hz =	213	

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -N	0.38	519	C	0.00 0.38
N -S1	0.40	4492	C	0.02 0.38
S1-O	0.18	4416	C	0.02 0.16
O -P	0.20	4280	C	0.02 0.18
P -B	0.09	3679	C	0.04 0.05
B -C	0.09	3435	C	0.01 0.08
C -Q	0.07	3883	C	0.04 0.03
Q -R	0.08	5574	C	0.06 0.02
R -S	0.24	9582	C	0.11 0.13
S -T	0.35	9587	C	0.10 0.25
T -D	0.34	9281	C	0.10 0.24
Bottom Chords				
A -G	0.16	572	T	0.06 0.10
X -M	0.84	4089	T	0.29 0.55
M -L	0.44	4089	T	0.29 0.15
L -S2	0.41	4050	T	0.29 0.12
S2-K	0.80	4050	T	0.68 0.12
K -J	0.76	3637	T	0.61 0.15
J -I	0.42	4550	T	0.20 0.22
I -E	0.58	5866	T	0.42 0.16
E -H	0.61	8795	T	0.40 0.21
H -D	0.61	8757	T	0.40 0.21
Chord-Webs				
G -X	0.67	2409	C	0.01 0.66
X -N	0.92	2259	C	0.01 0.91
Webs				
M -O	0.05	201	T	
O -L	0.06	98	T	
L -P	0.08	313	T	
P -K	0.25	807	C	1 Br
K -B	0.30	900	T	
K -C	0.13	327	C	1 Br
J -C	0.43	1233	T	
J -Q	0.39	1409	C	1 Br
Q -I	0.51	1607	T	
I -R	0.53	1474	C	
R -E	0.97	3868	T	
E -S	0.06	303	C	
E -T	0.13	410	T	
H -T	0.03	133	T	

TL Defl -1.37" in J -I L/473
LL Defl -0.66" in J -I L/980

Hz Disp	LL	DL	TL
Jt D	0.33"	0.33"	0.66"

Shear // Grain in G -N 0.43

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	Ctr	Ctr	0.93
N	LOCK	3.0x 5.0-2.9-1.1			0.97
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
O	LOCK	4.0x 5.0-0.5	0.2		0.56
P	LOCK	8.0x12.0-0.6	1.5		0.97
B	LOCK	10.0x10.0	Ctr	Ctr	0.76
C	LOCK	10.0x10.0	0.5	Ctr	0.76
Q	LOCK	4.0x 5.0	0.5	0.2	0.71
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	3.0x 4.0	Ctr	Ctr	0.41
T	LOCK	8.0x12.0	0.6	1.5	0.97
D	LOCK	3.0x 8.0	Ctr	Ctr	0.91
D	LOCK	4.0x 6.0	Ctr	Ctr	0.94
G	LOCK	3.0x 5.0	Ctr	Ctr	0.73
X	LOCK	5.0x 8.0	Ctr	0.8	0.86
M	LOCK	2.0x 4.0	Ctr	Ctr	0.61
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	4.0x 8.0	Ctr	0.2	0.76
K	LOCK	4.0x 8.0	Ctr	Ctr	0.50
J	LOCK	5.0x 6.0	Ctr	-0.5	0.88
I	LOCK	4.0x 5.0	Ctr	Ctr	0.71
E	LOCK	8.0x10.0	0.2	0.9	0.90
H	LOCK	2.0x 4.0	Ctr	Ctr	0.61

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9587 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054

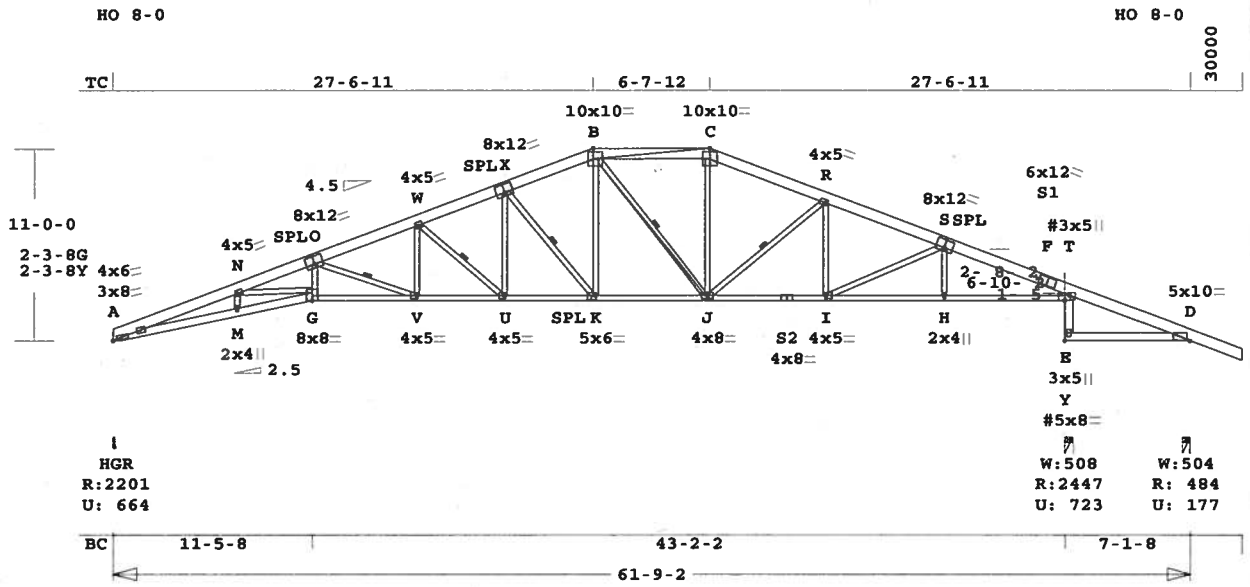
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
060039	*R61*	1	SP	610902	4.5	0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish

Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R61	1	SP	610902	4.5	0	3- 0- 0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 564.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	Size	Lumber
TC	0.40	2x 8	SP-2400
BC	0.95	2x 4	SP-2400
EX A -G		2x 6	SP-2400
EX K -S2		2x 4	SP-#2
EX E -D		2x 6	SP-#2
CW	0.92	2x 6	SP-2400
WB	0.62	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	61- 9- 2
BC Cont.	0- 0- 0	61- 9- 2

WB 1 rows CLB on O -V
WB 1 rows CLB on W -U
WB 1 rows CLB on X -K
WB 1 rows CLB on B -J
WB 1 rows CLB on J -R
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	2202	664	3- 8	1-13
		Hz =	-212	
E	2447	723	5- 8	2-11
D	484	178	5- 4	1- 8
		Hz =	213	

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -N	0.30	9488	C	0.10 0.20
N -O	0.35	9240	C	0.10 0.25
O -W	0.24	6213	C	0.04 0.20
W -X	0.08	4893	C	0.02 0.06
X -B	0.07	3875	C	0.04 0.03
B -C	0.09	3433	C	0.01 0.08
C -R	0.09	3677	C	0.04 0.05
R -S	0.20	4273	C	0.02 0.18
S -S1	0.25	4443	C	0.02 0.23
S1-T	0.40	4519	C	0.02 0.38
T -D	0.38	513	C	0.00 0.38
Bottom Chords				
A -M	0.55	8964	T	0.41 0.14
M -G	0.66	8976	T	0.41 0.25
G -V	0.95	8578	T	0.61 0.34
V -U	0.49	5872	T	0.42 0.07
U -K	0.37	4560	T	0.32 0.05
K -J	0.73	3630	T	0.60 0.13
J -S2	0.80	4045	T	0.67 0.13
S2-I	0.41	4045	T	0.29 0.12
I -H	0.44	4075	T	0.29 0.15
H -Y	0.85	4075	T	0.29 0.56
E -D	0.17	567	T	0.06 0.11
Chord-Webs				
E -Y	0.67	2416	C	0.01 0.66
Y -T	0.92	2265	C	0.01 0.91
Webs				
M -N	0.04	150	T	
N -G	0.04	189	T	
G -O	0.62	1950	T	
O -V	0.60	2901	C	1 Br
V -W	0.34	1085	T	
W -U	0.38	1745	C	1 Br
U -X	0.40	1269	T	
X -K	0.40	1441	C	1 Br
K -B	0.43	1225	T	
B -J	0.13	319	C	1 Br
J -C	0.30	899	T	
J -R	0.25	802	C	1 Br
I -R	0.08	307	T	
I -S	0.06	101	T	
H -S	0.05	200	T	

TL Defl -1.47" in V -U L/443
LL Defl -0.73" in V -U L/893

Hz Disp	LL	DL	TL
Jt D	0.36"	0.36"	0.72"

Shear // Grain in E -T 0.43

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.91
A	LOCK	4.0x 6.0	Ctr	Ctr	0.99
N	LOCK	4.0x 5.0	-0.5	0.2	0.62
O	LOCK	8.0x12.0	-0.6	1.5	0.97
W	LOCK	4.0x 5.0	-0.5	0.2	0.83
X	LOCK	8.0x12.0	-0.6	1.5	0.97
B	LOCK	10.0x10.0	-0.5	Ctr	0.76
C	LOCK	10.0x10.0	Ctr	Ctr	0.76
R	LOCK	4.0x 5.0	0.5	0.2	0.56
S	LOCK	8.0x12.0	0.6	1.5	0.97
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
T	LOCK	3.0x 5.0	2.6	-0.7	0.89
D	LOCK	5.0x10.0	Ctr	Ctr	0.93
M	LOCK	2.0x 4.0	Ctr	Ctr	0.61
G	LOCK	8.0x 8.0	Ctr	0.5	0.97
V	LOCK	4.0x 5.0	Ctr	Ctr	0.95
U	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	5.0x 6.0	Ctr	-0.5	0.88
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	4.0x 8.0	Ctr	0.2	0.76
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
H	LOCK	2.0x 4.0	Ctr	Ctr	0.61
Y#	LOCK	5.0x 8.0	-0.4	0.9	0.97
E	LOCK	3.0x 5.0	Ctr	Ctr	0.73

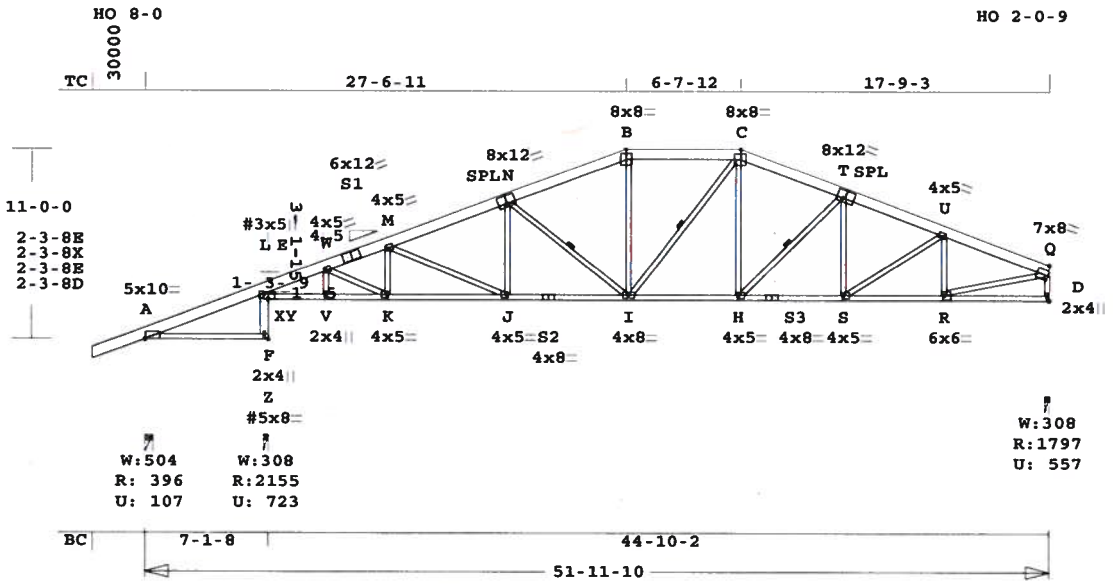
= Plate Monitor used

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
CAUTION: Use extra care during fabrication, shipping, handling and erection of this truss. Temporary and permanent bracing are req'd and are designed by others. Seek advice of registered engineer.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 9488 Lbs
Quality Control Factor 1.25
This truss has span greater than 60-0-0 and has been designed based on information furnished to Robbins Engineering by the truss fabricator. Please refer to the accompanying letter for further information.

FABRICATOR NOTES:
1. Delegated Engineer (Truss

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060039	R60	1	SP	511110	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 486.6 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 8	SP-2400
BC	0.64	2x 4	SP-#2
EX Z -S2		2x 4	SP-2400
CW	0.55	2x 6	SP-2400
WB	0.80	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51-11-10
BC Cont.	0- 0- 0	51-11-10

WB 1 rows CLB on N -I
WB 1 rows CLB on I -C
WB 1 rows CLB on H -T
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	397	107	5- 4	1- 8
			Hz =	165
F	2156	724	3- 8	2- 5
D	1797	558	3- 8	2- 2
			Hz =	168

Membr	CSI	P Lbs	Axl-CSI-Bnd
	Top Chords		
A -L	0.31	188 C	0.00 0.31
L -W	0.31	2722 C	0.00 0.31
W -S1	0.24	3463 C	0.01 0.23
S1-M	0.15	3426 C	0.01 0.14
M -N	0.07	3206 C	0.03 0.04
N -B	0.07	2580 C	0.03 0.04
B -C	0.07	2410 C	0.00 0.07
C -T	0.06	2483 C	0.03 0.03
T -U	0.06	2756 C	0.03 0.03
U -Q	0.07	2577 C	0.03 0.04

	Bottom Chords			
A -F	0.24	243 T	0.02	0.22
Z -V	0.73	2353 T	0.16	0.57
V -K	0.39	2353 T	0.16	0.23
K -J	0.31	3316 T	0.23	0.08
J -S2	0.29	3006 T	0.21	0.08
S2-I	0.64	3006 T	0.50	0.14
I -H	0.52	2319 T	0.38	0.14
H -S3	0.54	2582 T	0.43	0.11
S3-S	0.52	2582 T	0.43	0.09
S -R	0.50	2416 T	0.40	0.10
R -D	0.21	168 T	0.00	0.21
	Chord-Webs			
F -Z	0.30	2109 C	0.01	0.29
Z -L	0.55	1882 C	0.00	0.55
	Webs			
V -W	0.03	198 C		
W -K	0.34	1088 T		
K -M	0.06	326 C		
M -J	0.27	340 C		
J -N	0.10	391 T		
N -I	0.24	773 C		1 Br
I -B	0.16	538 T		
I -C	0.04	148 T		1 Br
H -C	0.12	432 T		
H -T	0.10	368 C		1 Br
S -T	0.03	146 T		
S -U	0.06	199 T		
R -U	0.12	532 C		
R -Q	0.80	2505 T		
D -Q	0.28	1744 C		

TL Defl -0.38" in S2-I L/999
LL Defl -0.18" in J -S2 L/999
Shear // Grain in L -W 0.37

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

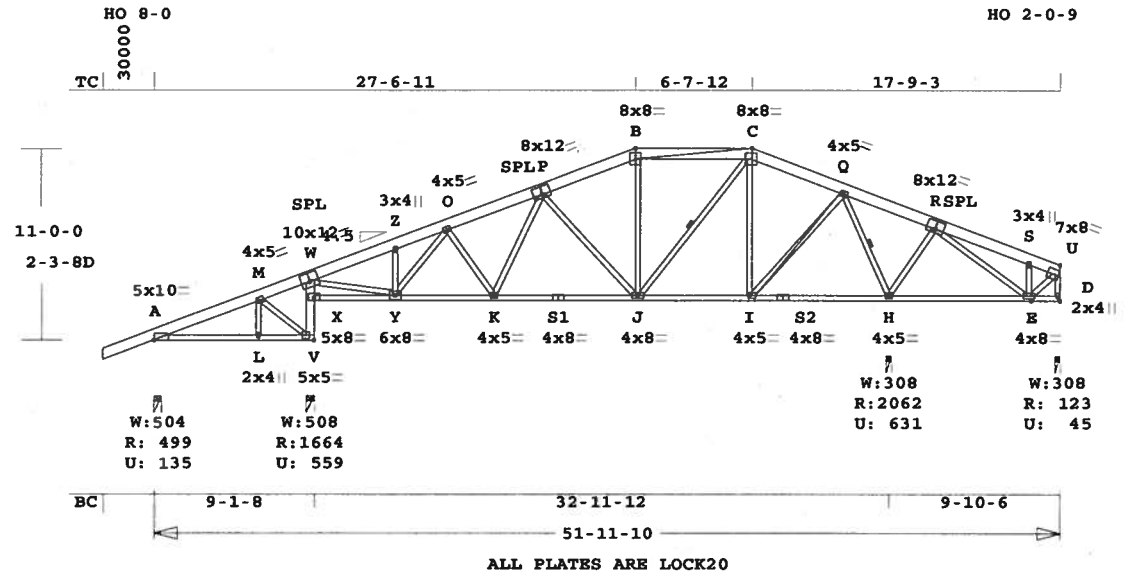
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	0.7	0.6	0.77
L	LOCK	3.0x 5.0-2.1-0.7			0.55
W	LOCK	4.0x 5.0-0.5	0.2	0.5	0.56
S1	LOCK	6.0x12.0	Ctr	Ctr	0.91
M	LOCK	4.0x 5.0-0.5	0.2	0.5	0.56
N	LOCK	8.0x12.0-0.6	1.5	0.9	0.97
B	LOCK	8.0x 8.0	Ctr	Ctr	0.85
C	LOCK	8.0x 8.0 1.0	Ctr		0.85
T	LOCK	8.0x12.0	0.6	1.5	0.97
U	LOCK	4.0x 5.0	0.5	0.2	0.56
Q	LOCK	7.0x 8.0-1.5	0.6	0.9	0.92
F	LOCK	2.0x 4.0	Ctr	Ctr	0.91
Z	LOCK	5.0x 8.0	0.8	0.8	0.95
V	LOCK	2.0x 4.0	Ctr	Ctr	0.55
K	LOCK	4.0x 5.0	Ctr	Ctr	0.60
J	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	4.0x 8.0	Ctr	0.2	0.75
I	LOCK	4.0x 8.0	Ctr	Ctr	0.51
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	4.0x 8.0	Ctr	0.2	0.75
S	LOCK	4.0x 5.0	Ctr	Ctr	0.60
R	LOCK	6.0x 6.0	Ctr	Ctr	0.95
D	LOCK	2.0x 4.0	Ctr	Ctr	0.70

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3463 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R57*	3	SP	511110	4.5	3- 0- 0	0	

+Office and Care Center, Hospice, M.M. Parrish



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 502.4 LBS

Online Plus -- Version 19.0.009
RUN DATE: 15-MAR-06

	CSI	-Size-	----Lumber----
TC	0.09	2x 8	SP-2400
BC	0.47	2x 4	SP-#2
CW	0.08	2x 6	SP-#2
WB	0.54	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	51-11-10
BC Cont.	0- 0- 0	51-11-10

WB 1 rows CLB on J -C
WB 1 rows CLB on Q -H
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	499	135	5- 4	1- 8
		Hz =		165
V	1665	559	5- 8	1-13
H	2062	631	3- 8	2- 3
D	123	45	3- 8	1- 8
		Hz =		168

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -M	0.04	226 C	0.00 0.04
M -W	0.09	159 T	0.00 0.09
W -Z	0.09	1656 C	0.00 0.09
Z -O	0.03	1614 C	0.02 0.01
O -P	0.05	1704 C	0.02 0.03
P -B	0.04	1196 C	0.01 0.03
B -C	0.06	1114 C	0.00 0.06
C -Q	0.04	786 C	0.00 0.04
Q -R	0.07	517 T	0.01 0.06
R -S	0.06	163 T	0.00 0.06
S -U	0.01	82 C	0.00 0.01
Bottom Chords			
A -L	0.14	224 T	0.02 0.12
L -V	0.14	224 T	0.02 0.12
X -Y	0.18	37 C	0.00 0.18
Y -K	0.47	1664 T	0.17 0.30
K -S1	0.45	1490 T	0.15 0.30
S1-J	0.44	1490 T	0.15 0.29
J -I	0.36	729 T	0.07 0.29
I -S2	0.25	127 T	0.00 0.25
S2-H	0.39	127 T	0.00 0.39
H -E	0.39	236 C	0.00 0.39
E -D	0.25	168 T	0.00 0.25
Chord-Webs			
V -X	0.08	1434 C	0.08 0.00
X -W	0.08	1370 C	0.08 0.00
Webs			
L -M	0.05	192 T	
M -V	0.07	339 C	
W -Y	0.51	1610 T	
Y -Z	0.03	146 T	
Y -O	0.09	244 C	
O -K	0.04	156 T	
K -P	0.09	339 T	
P -J	0.54	566 C	
J -B	0.05	200 T	
J -C	0.19	621 T	1 Br
I -C	0.52	550 C	
I -Q	0.29	907 T	
Q -H	0.35	1606 C	1 Br
H -R	0.17	475 C	
R -E	0.12	397 T	
E -S	0.06	248 T	
E -U	0.03	105 T	
D -U	0.02	148 C	

TL Defl -0.11" in H -E L/999
LL Defl -0.05" in H -E L/999
Shear // Grain in K -S1 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

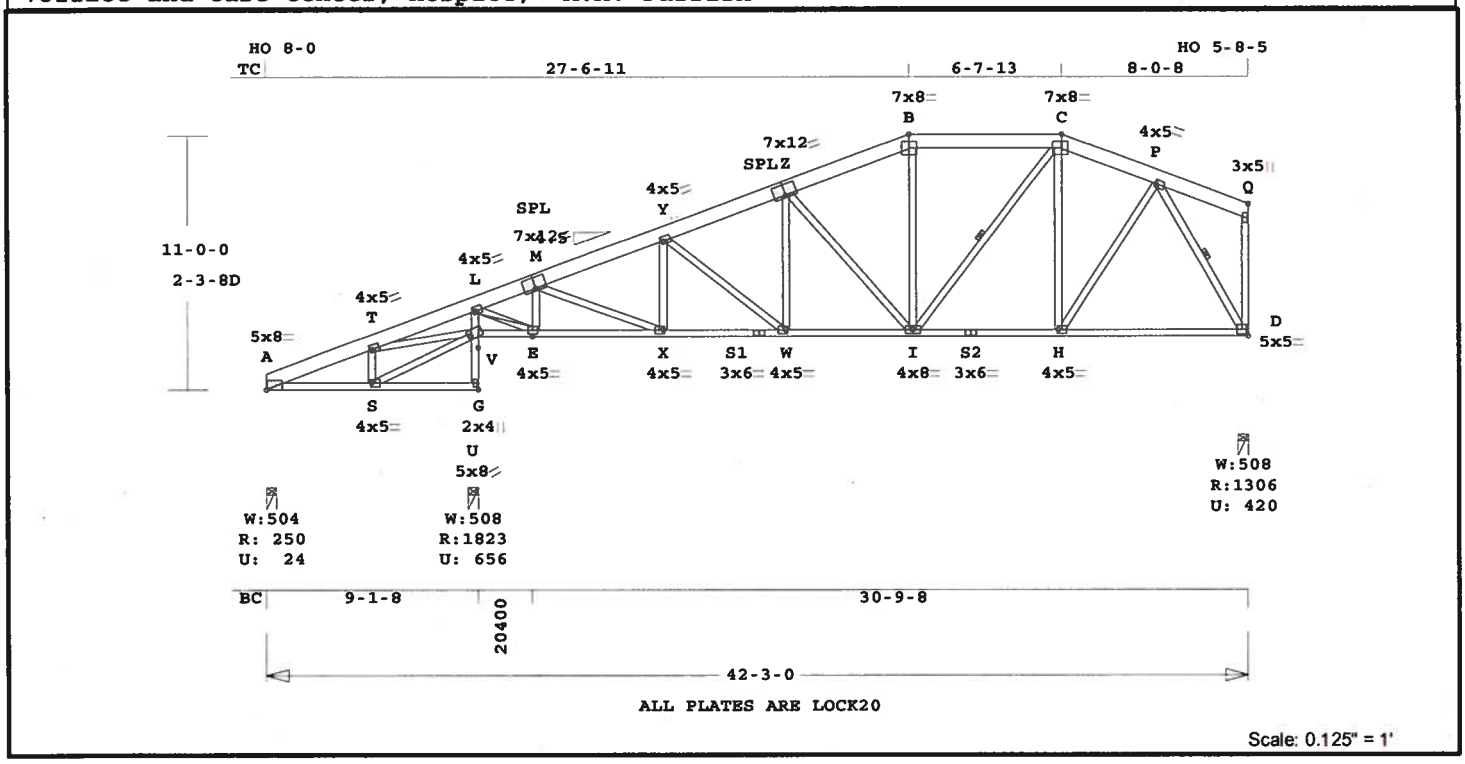
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x10.0	0.7	0.6	0.77
M	LOCK	4.0x 5.0-0.5	0.2	0.5	0.56
W	LOCK	10.0x12.0-0.4	1.0	0.8	0.89
Z	LOCK	3.0x 4.0 Ctr	Ctr	0.3	0.37
O	LOCK	4.0x 5.0-0.5	0.2	0.5	0.58
P	LOCK	8.0x12.0-0.6	1.5	0.9	0.97
B	LOCK	8.0x 8.0	Ctr	Ctr	0.85
C	LOCK	8.0x 8.0	1.0	Ctr	0.85
Q	LOCK	4.0x 5.0	0.5	0.2	0.60
R	LOCK	8.0x12.0	0.6	1.5	0.97
S	LOCK	3.0x 4.0	Ctr	Ctr	0.37
U	LOCK	7.0x 8.0-1.5	0.6	0.9	0.92
L	LOCK	2.0x 4.0	Ctr	Ctr	0.55
V	LOCK	5.0x 5.0	Ctr	Ctr	0.82
X	LOCK	5.0x 8.0	Ctr	0.8	0.78
Y	LOCK	6.0x 8.0	Ctr	Ctr	0.79
K	LOCK	4.0x 5.0	Ctr	Ctr	0.57
S1	LOCK	4.0x 8.0	Ctr	0.2	0.75
J	LOCK	4.0x 8.0	Ctr	Ctr	0.50
I	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	4.0x 8.0	Ctr	0.2	0.75
H	LOCK	4.0x 5.0	Ctr	Ctr	0.58
E	LOCK	4.0x 8.0	Ctr	Ctr	0.51
D	LOCK	2.0x 4.0	Ctr	Ctr	0.70

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1704 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
060039	*R56*	1	SP	420300	4.5	0	0	

+Office and Care Center, Hospice, M.M. Parrish



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 424.3 LBS

Online Plus -- Version 18.0.020
RUN DATE: 13-FEB-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 8	SP-2400
BC	0.49	2x 4	SP-#2
CW	0.27	2x 4	SP-#3
WB	0.53	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	42- 3- 0
BC Cont.	0- 0- 0	42- 3- 0

WB 1 rows CLB on I -C
WB 1 rows CLB on P -D
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	251	25	5- 4	1- 0
		Hz =	329	
G	1823	656	5- 8	2- 0
D	1306	420	5- 8	1- 9
		Hz =	191	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -T	0.03	200 C	0.00		0.03
T -L	0.11	555 T	0.02		0.09
L -M	0.09	1135 C	0.00		0.09
M -Y	0.04	1845 C	0.00		0.04
Y -Z	0.04	1718 C	0.02		0.02
Z -B	0.04	1352 C	0.01		0.03
B -C	0.06	1261 C	0.00		0.06
C -P	0.03	1032 C	0.01		0.02
P -Q	0.02	57 T	0.00		0.02
Bottom Chords					
A -S	0.12	191 C	0.01		0.11
S -G	0.11	16 C	0.00		0.11
U -E	0.10	486 C	0.00		0.10
E -X	0.25	1022 T	0.17		0.08
X -S1	0.37	1732 T	0.29		0.08
S1-W	0.37	1732 T	0.29		0.08
W -I	0.35	1610 T	0.27		0.08
I -S2	0.24	965 T	0.16		0.08
S2-H	0.49	965 T	0.10		0.39
H -D	0.45	645 T	0.06		0.39
Chord-Webs					
G -U	0.27	1786 C	0.27		0.00
U -L	0.27	1710 C	0.27		0.00
Webs					
S -T	0.03	155 T			
S -U	0.08	222 C			
T -U	0.21	670 C			
L -E	0.53	1676 T			
E -M	0.11	666 C			
M -X	0.24	769 T			
X -Y	0.05	188 T			
Y -W	0.09	156 C			
W -Z	0.07	288 T			
Z -I	0.50	533 C			
I -B	0.06	238 T			
I -C	0.15	478 T			1 Br
H -C	0.31	332 C			
H -P	0.19	594 T			
P -D	0.33	1301 C			1 Br
D -Q	0.04	146 T			

LL Defl -0.13" in H -D L/999
TL Defl -0.27" in H -D L/999
Shear // Grain in H -D 0.20

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 8.0	Ctr	0.6	0.72
T	LOCK	4.0x 5.0-0.5	0.2		0.69
L	LOCK	4.0x 5.0-0.5	0.2		0.84
M	LOCK	7.0x12.0-0.7	2.0		0.97
Y	LOCK	4.0x 5.0-0.5	0.2		0.56
Z	LOCK	7.0x12.0-0.7	2.0		0.97
B	LOCK	7.0x 8.0	Ctr	Ctr	0.87
C	LOCK	7.0x 8.0	1.0	Ctr	0.87
P	LOCK	4.0x 5.0	0.5	0.2	0.72
Q	LOCK	3.0x 5.0	Ctr	Ctr	0.89
S	LOCK	4.0x 5.0	Ctr	Ctr	0.60
G	LOCK	2.0x 4.0	Ctr	Ctr	0.71
U	LOCK	5.0x 8.0	Ctr	1.0	0.94
E	LOCK	4.0x 5.0	Ctr	Ctr	0.72
X	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
W	LOCK	4.0x 5.0	Ctr	Ctr	0.60
I	LOCK	4.0x 8.0	Ctr	Ctr	0.50
S2	LOCK	3.0x 6.0	Ctr	Ctr	0.88
H	LOCK	4.0x 5.0	Ctr	Ctr	0.60
D	LOCK	5.0x 5.0	Ctr	Ctr	0.69

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Provide connection to bearing for 329 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1845 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

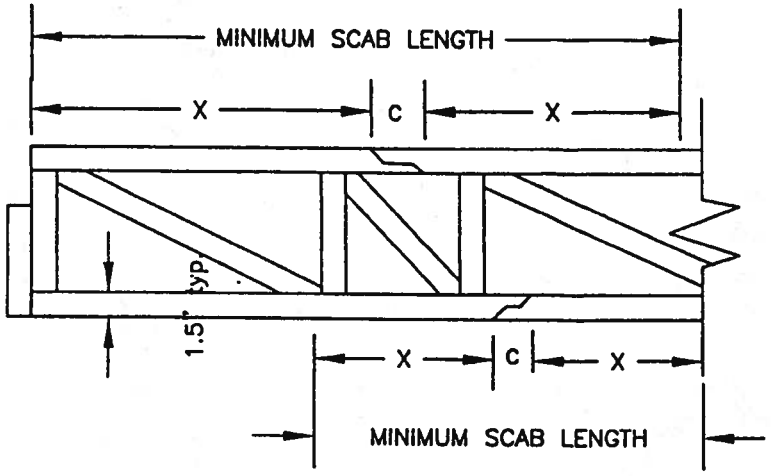
General detail for repair of broken, damaged or cut chords of PC42 floor trusses that meet the following conditions.

* Chord size must be maximum 4x2 and southern pine species.
* Scab must be at minimum the same size and grade as broken chord. Wide face of scab must be attached to narrow face of chord member.
* Truss must be single ply.
* No more than two broken or cracked chords per truss.
* Perimeter of break area must be minimum of X distance from end of truss. and minimum of 4" away from any plates at panel points. All plates must be intact and fully embedded.

C = Maximum length of damaged area or cut section not to exceed 12".
S = Overall length of scab member must be equal or greater than 2(X)+C.
X = Minimum length of scab member at each end of break area.
Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.
For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Attach the scab with one row of 10d common nails at 3" on center into 4x2 chord members. Axial force and number of nails may be doubled or X distance divided by 2, if scabs applied to each face of truss at the same location. Minimum end distance of 3" must be provided for all members.

THOMAS ALBANI
CERTIFICATE
No. 39350
STATE OF
FLORIDA
PROFESSIONAL ENGINEER



TOTAL NUMBER OF NAILS AT EACH END OF	X (INCHES)	MAXIMUM AXIAL FORCE IN CHORD MEMBER (LBS.)
3	16	384
6	24	768
8	30	1024
10	36	1280
12	42	1536
14	48	1792
16	54	2048
18	60	2304
20	66	2560
22	72	2816

Truss manufacturer must be notified if field conditions do not match this general detail.

DEC 10 2003

ROBBINS
Engineering Inc.
P.O.Box 280055, Tampa, FL 33682

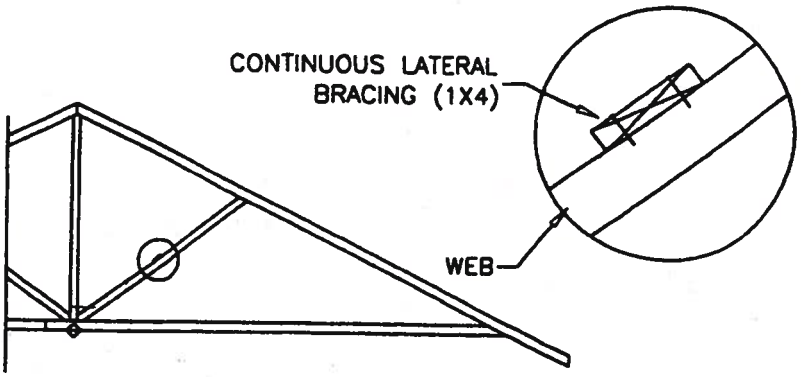
ROBBINS LOCK connector plates (20 ga. galv. steel–ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "V", all slots in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or voids in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces easy-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg., Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "domincing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

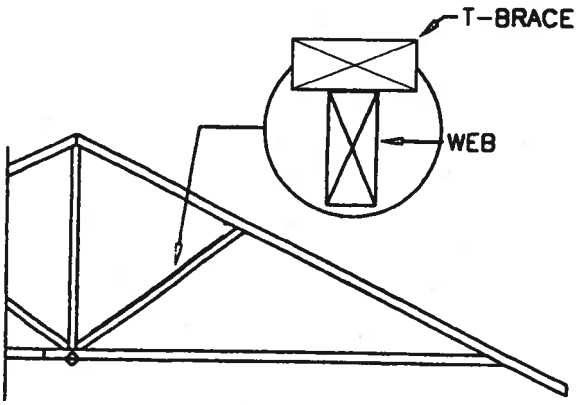
Designed By: MG
Checked By: TAA
Rev. Date: 12/09/03
Dwg. No: GD-62-FL

PERMANENT WEB BRACING



CONTINUOUS LATERAL BRACING

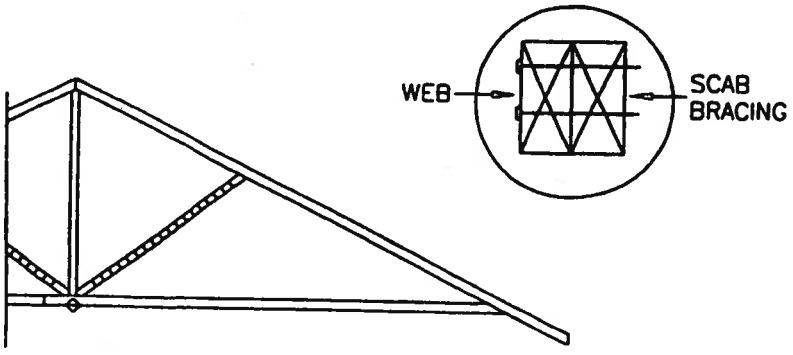
1x4 #3 HEM–FIR OR BETTER CONTINUOUS LATERAL BRACING TO BE EQUALLY SPACED. ATTACH WITH (2) 8d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.



T-BRACE

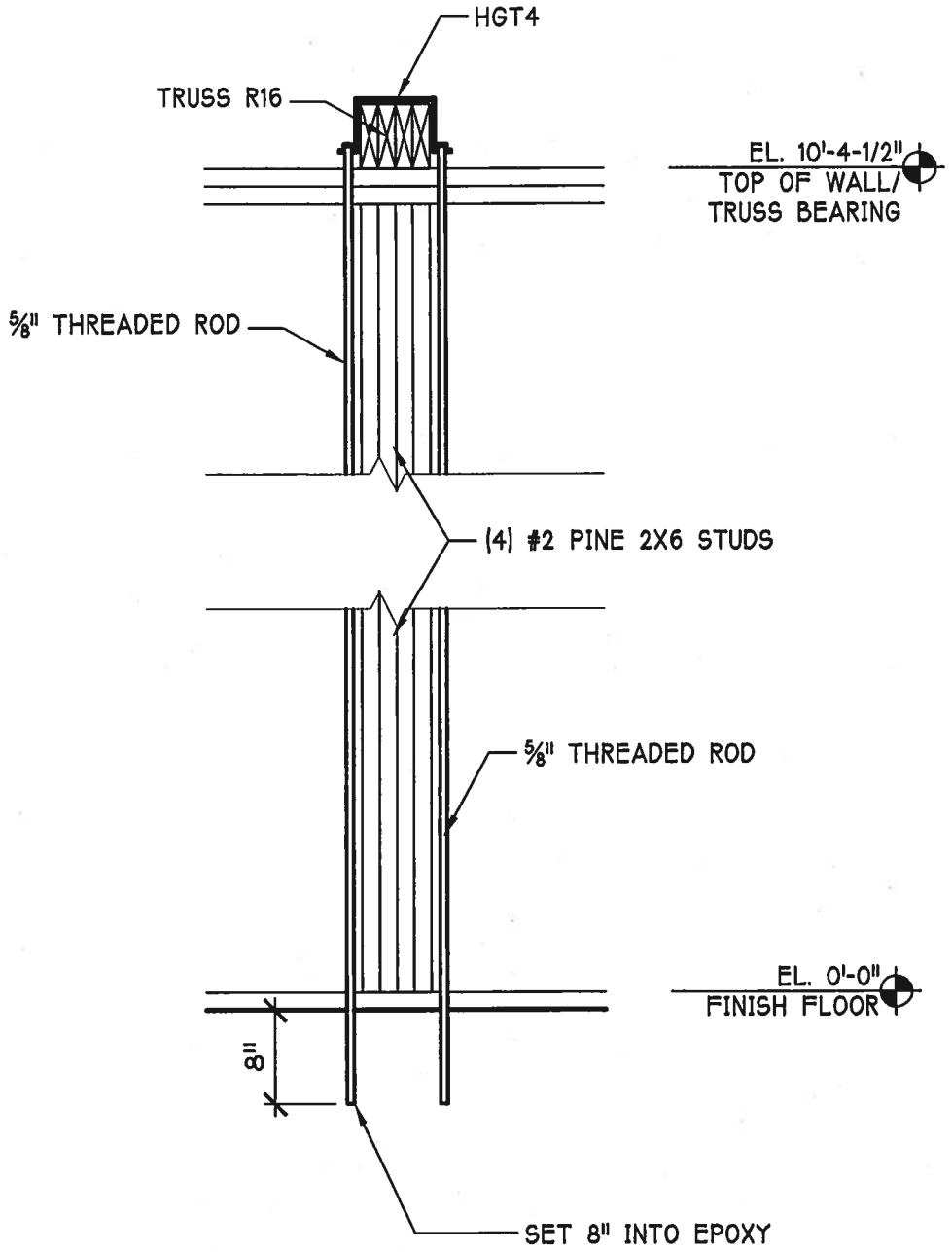
THESE DETAILS APPLY TO 1.5" WIDE WOOD TRUSSES.

- USE A 2x4 T-BRACE IF THE TRUSS DESIGN SPECIFIES ONE LATERAL BRACE (MID POINT OF WEB).
- USE A 2x6 T-BRACE IF THE TRUSS DESIGN SPECIFIES TWO LATERAL BRACES (AT THE THIRD POINTS OF THE WEB).
- USE A CONTINUOUS PIECE FOR THE T-BRACE, OF THE SAME GRADE AS THE WEB AND COVERING AT LEAST 90% OF THE WEB LENGTH.
- CENTER THE T-BRACE ON THE WEB AND FASTEN WITH 10d COMMON NAILS SPACED 4" ON CENTER.



SCAB BRACE

SCAB BRACE SAME SIZE, GRADE, AND LENGTH AS WEB MEMBER. ATTACH WITH 10d NAILS @ 4" O.C. BRACING MATERIAL TO BE SUPPLIED BY ERECTION CONTRACTOR.



DETAIL @ R16-C

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT
ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0448
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 6855

ADD DETAIL TO TRUSS DRAWING 060039A

Brame Architects
606 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

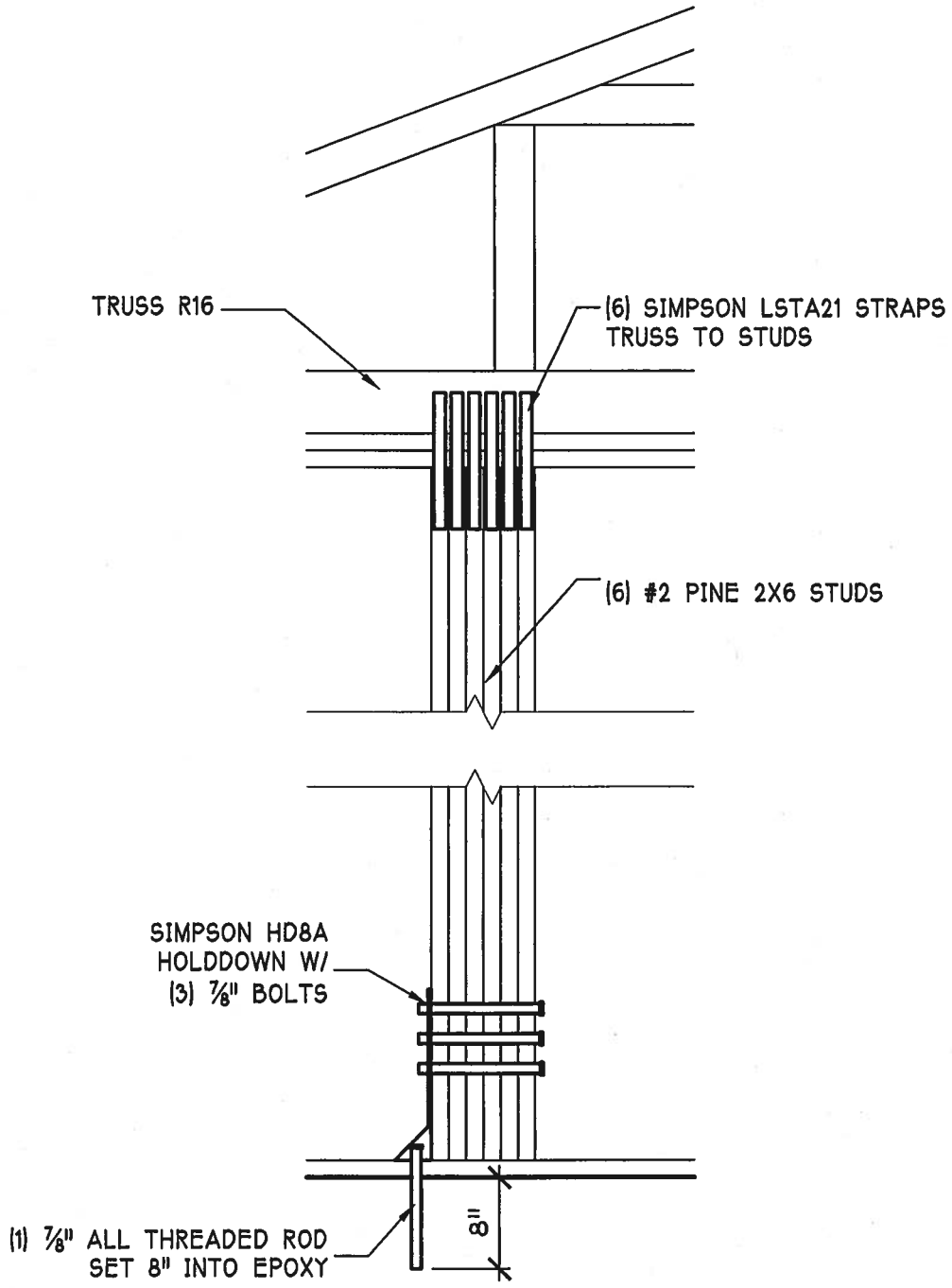
HAVEN HOSPICE OF THE SUWANNEE VALLEY

6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-12-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO. 5138.010

DRAWING SK-12



DETAIL @ R16-K

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT
ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0448
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 6855

ADD DETAIL TO TRUSS DRAWING 060039A

Brame Architects
606 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

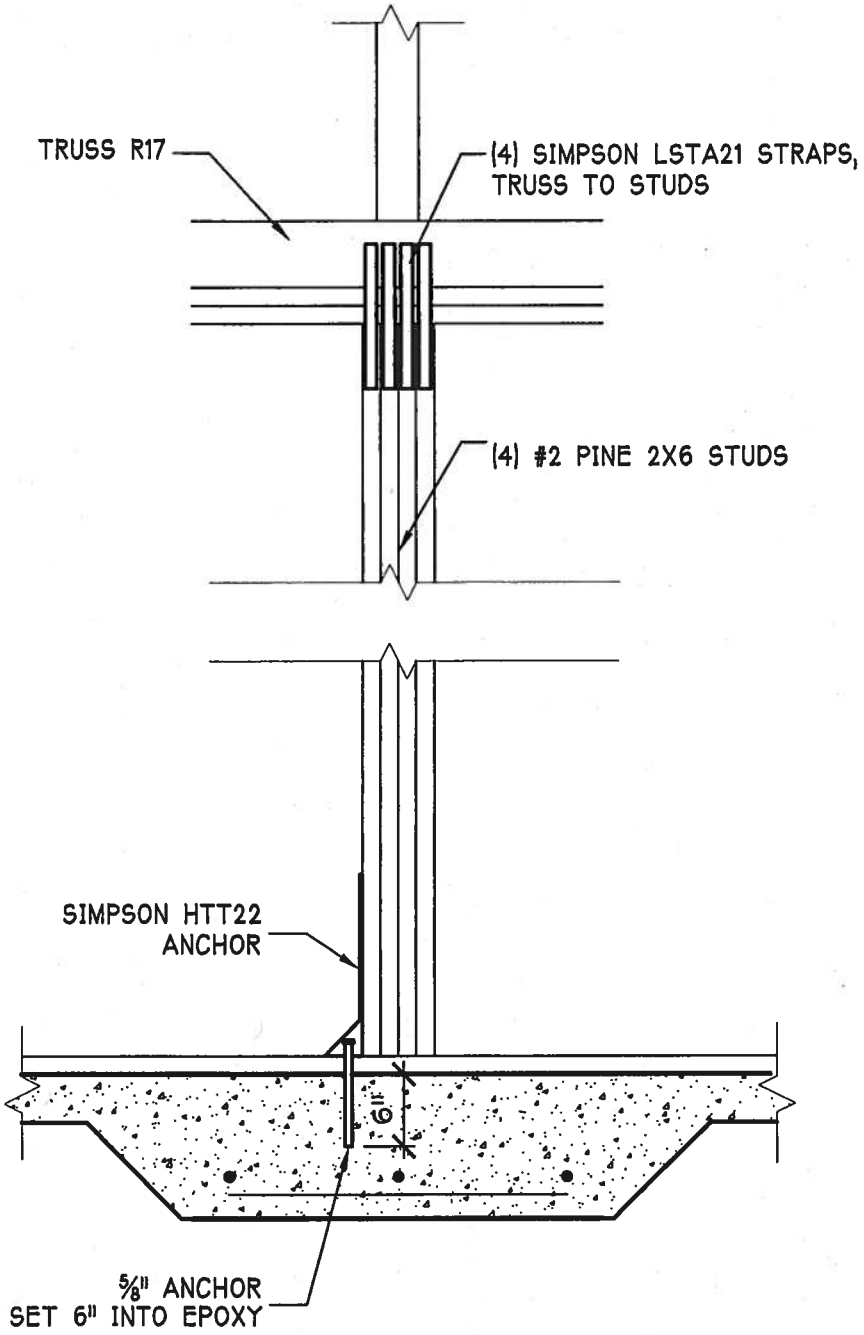
HAVEN HOSPICE OF THE SUWANNEE VALLEY

6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-13-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO. 5138.010

DRAWING SK-13



DETAIL @ R17-S3

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1st AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0446
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 6655

ADD DETAIL TO TRUSS DRAWING 060039A

Brame Architects
608 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

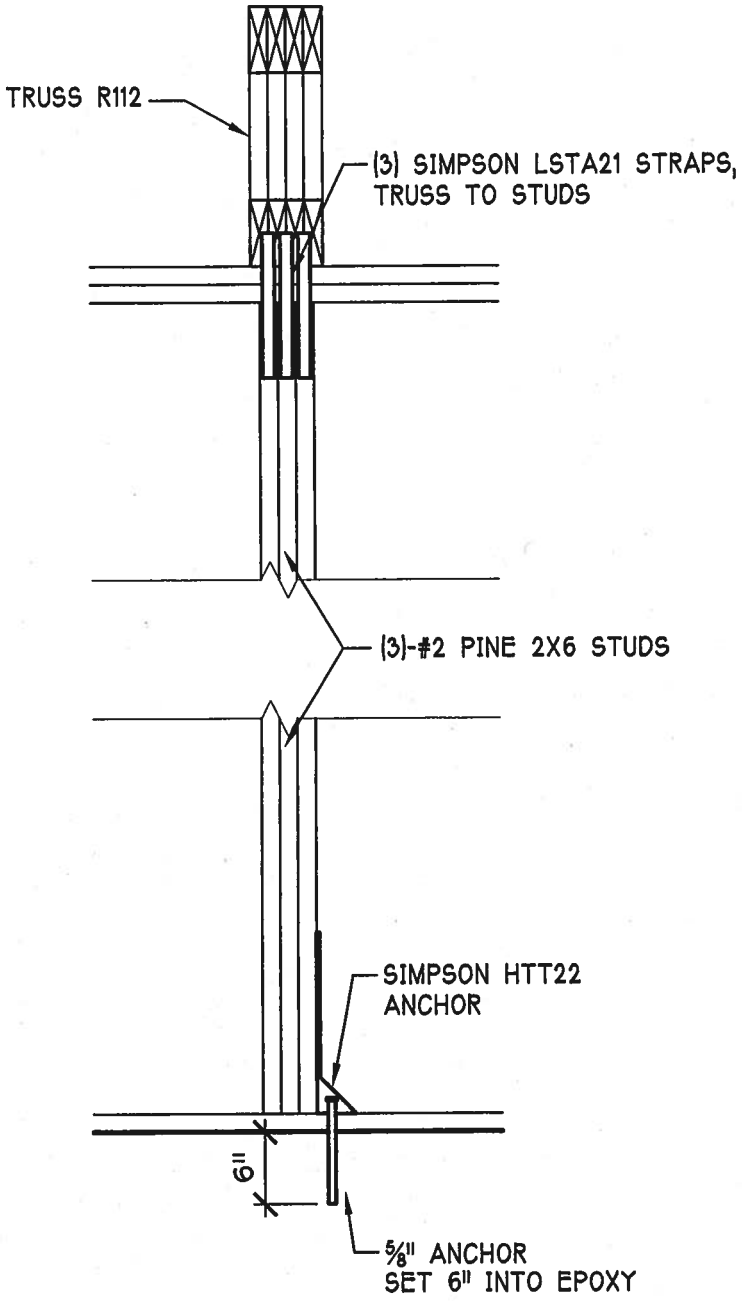
HAVEN HOSPICE OF THE SUWANNEE VALLEY

6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-14-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO. 5138.010

DRAWING SK-14



DETAIL @ R112-D

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0448
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 8855

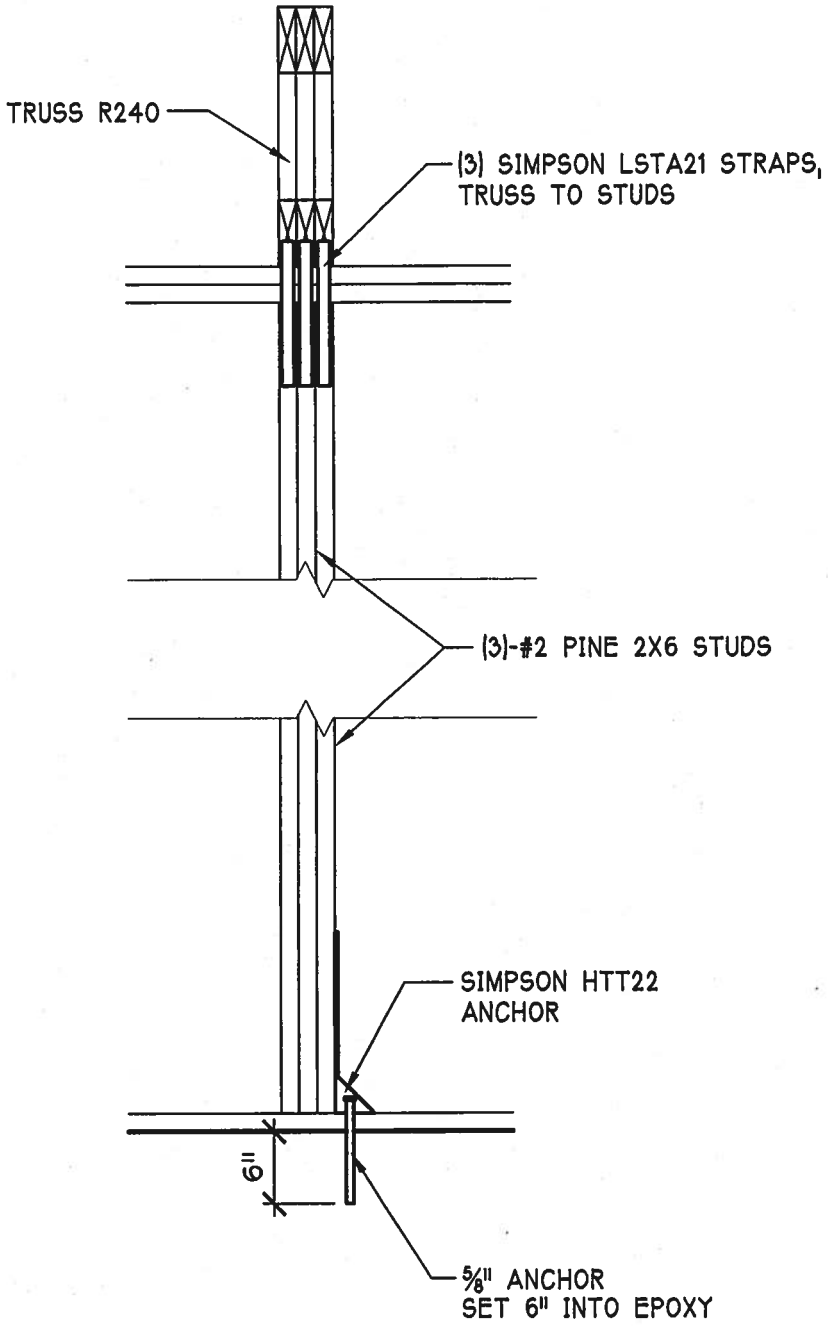
ADD DETAIL TO TRUSS DRAWING 060039B

Brame Architects
606 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

HAVEN HOSPICE OF THE SUWANNEE VALLEY
6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-15-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO. 5138.010
DRAWING SK-15



DETAIL @ R240-A

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0448
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 6855

ADD DETAIL TO TRUSS DRAWING 060039B

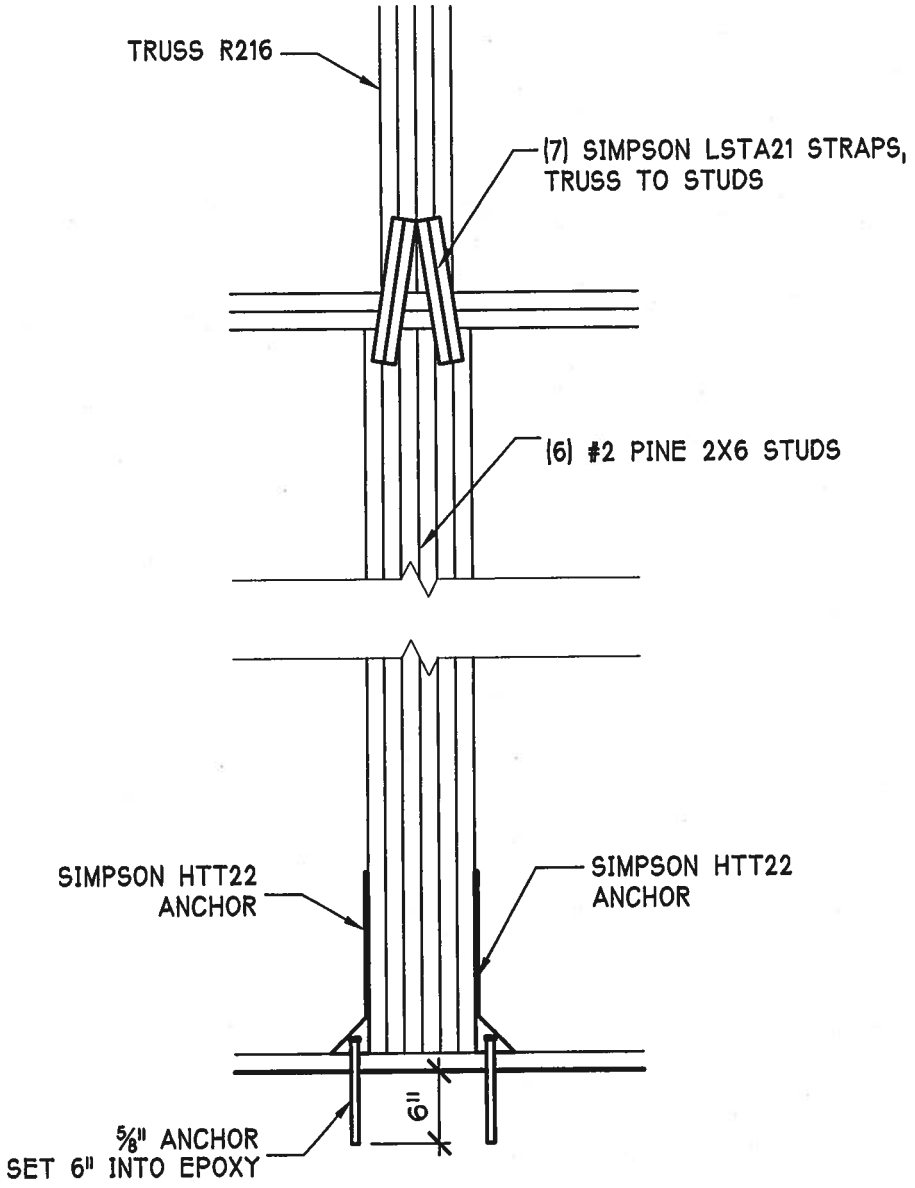
Brame Architects
606 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

HAVEN HOSPICE OF THE SUWANNEE VALLEY

6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-16-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO.	DRAWING
5138.010	SK-16



DETAIL @ R216-A4 & V

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0448
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 6855

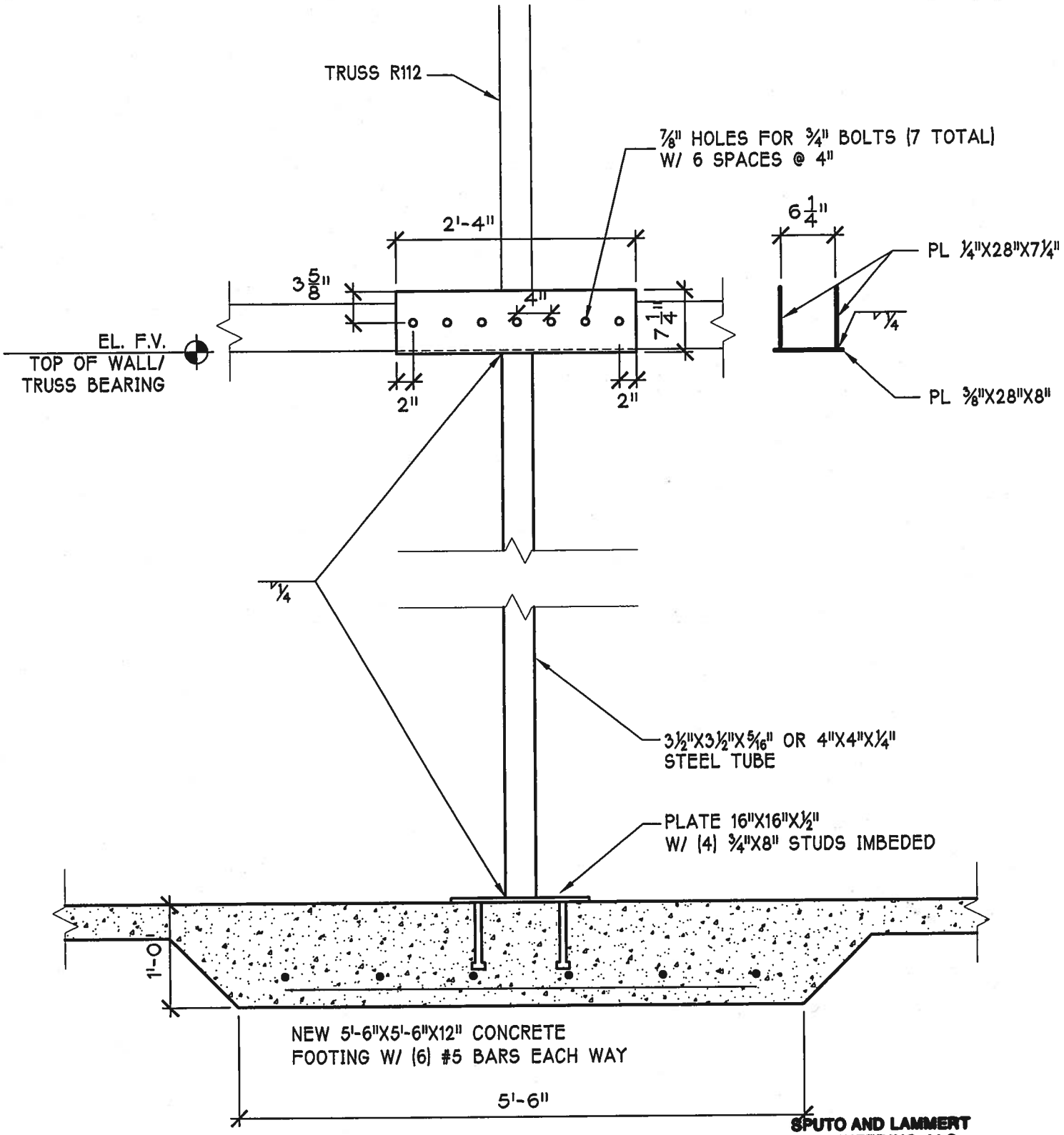
ADD DETAIL TO TRUSS DRAWING 060039B

Brame Architects
606 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

HAVEN HOSPICE OF THE SUWANNEE VALLEY
6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-17-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO. 5138.010
DRAWING SK-17



DETAIL @ R112-EE

SCALE: 3/4"=1'-0"

SPUTO AND LAMMERT
ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVE, GAINESVILLE, FL 32601
PHONE: 352-378-0448
THOMAS SPUTO, PH.D., P.E.
FLORIDA PE # 39142 CA # 6855

ADD DETAIL TO TRUSS DRAWING 060039B

Brame Architects
606 N.E. First Street
Gainesville, FL 32601
(352) 372-0425
FAX (352) 372-0427
CERT. NO. AA C000071

HAVEN HOSPICE OF THE SUWANNEE VALLEY

6037 West US highway 90 Lake City, Florida

DRAWN	CAD
SPL	5138010\CD SK-18-HosSV
CHECKED	REVISION
BB	
DATE	
3/20/06	

FILE NO. 5138.010

DRAWING SK-18