

Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com

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04/23/2026

COA#0-278, Yoonhwak Kim, FL PE #86367
 Florida Certificate of Product Approval #FL 1999

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 26-3717
Job Description: STECKBECK	
Address: FL	

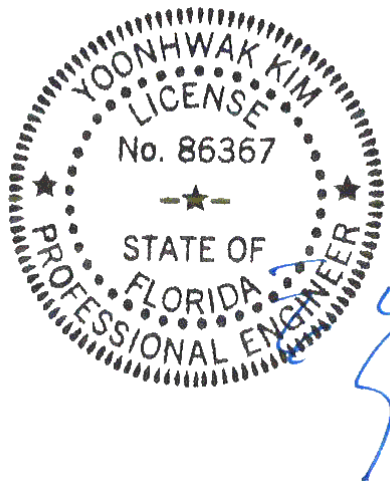
Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	IntelliVIEW Version: 24.02.00D through 25.02.01C JRef #: 1YJ12150001
Wind Standard: ASCE 7-22 Wind Speed (mph): 130	Design Loading (psf): 40
Building Type: Enclosed	

This package contains general notes pages, 48 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	112.26.0916.20647	A1
3	112.26.0918.55860	A2
5	112.26.0930.41937	B1
7	112.26.0919.26977	B2
9	112.26.0919.32987	C1
11	112.26.0919.36170	C3
13	112.26.0922.42720	D1E
15	112.26.0924.01730	D2
17	112.26.0923.48120	D3G
19	112.26.0921.16593	FG1
21	112.26.0923.29253	FG3
23	112.26.0924.09407	G1E
25	112.26.0924.12870	G3
27	112.26.0924.16360	H1E
29	112.26.0924.19097	J1
31	112.26.0924.21817	J3
33	112.26.0924.24213	J5
35	112.26.0924.28423	K1E
37	112.26.0923.06033	K3
39	112.26.0924.32740	M1
41	112.26.0924.35207	M2
43	112.26.0924.37780	M3A
45	112.26.0920.26260	P1E
47	112.26.0920.37140	V1
49	BRCLBSUB0119	

Item	Drawing Number	Truss
2	112.26.0918.43637	A1E
4	112.26.0919.17573	A2E
6	112.26.0919.24180	B1E
8	112.26.0919.29697	B3
10	112.26.0919.34643	C2
12	112.26.0919.38170	D1
14	112.26.0923.54133	D1G
16	112.26.0924.03650	D3
18	112.26.0924.06227	D4
20	112.26.0921.25547	FG2
22	112.26.0924.07647	G1
24	112.26.0924.11387	G2
26	112.26.0924.14270	H1
28	112.26.0924.17843	HJ1
30	112.26.0924.20490	J2
32	112.26.0924.23190	J4
34	112.26.0924.25810	K1
36	112.26.0924.29987	K2
38	112.26.0924.31447	L1
40	112.26.0924.33997	M1E
42	112.26.0924.36490	M3
44	112.26.0933.05090	P1
46	112.26.0920.35807	P2
48	112.26.0920.38323	V2
50	VAL180220723	

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Site Information:	Page 2:
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Address: FL	

Item	Drawing Number	Truss
51	VALTN220723	

Item	Drawing Number	Truss
52	160TL	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCDL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Borafire Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

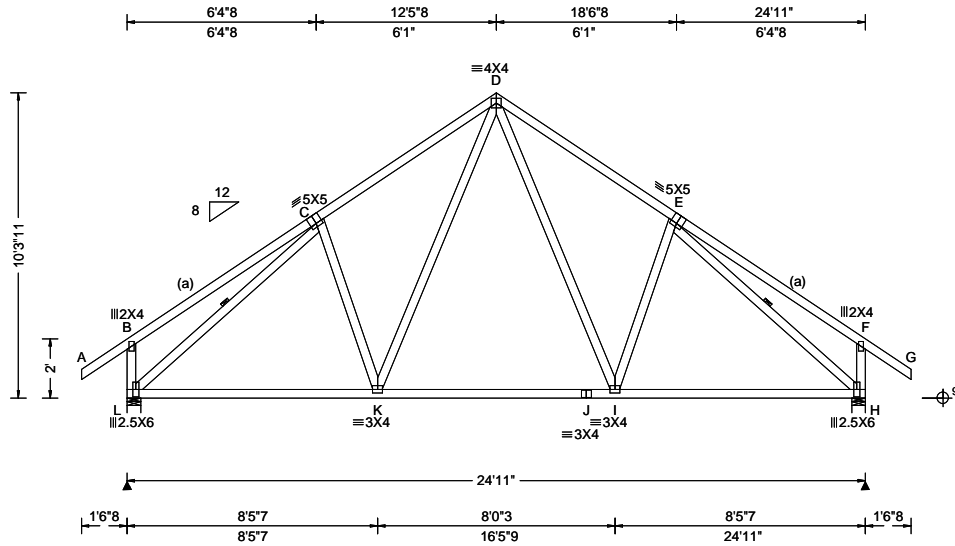
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catocin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.045 K 999 360 VERT(CL): 0.079 K 999 240 HORZ(LL): 0.027 F - - HORZ(TL): 0.048 F - - Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.795 Max Web CSI: 0.550 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>L</td> <td>1355</td> <td>-</td> <td>-</td> <td>/607</td> <td>/191</td> <td>/330</td> </tr> <tr> <td>H</td> <td>1355</td> <td>-</td> <td>-</td> <td>/607</td> <td>/191</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS L Brg Wid = 5.5 Min Req = 1.6 (Truss) H Brg Wid = 5.5 Min Req = 1.6 (Truss) Bearings L & H are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>490 - 1330</td> <td>D - E</td> <td>490 - 1330</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	L	1355	-	-	/607	/191	/330	H	1355	-	-	/607	/191	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	490 - 1330	D - E	490 - 1330
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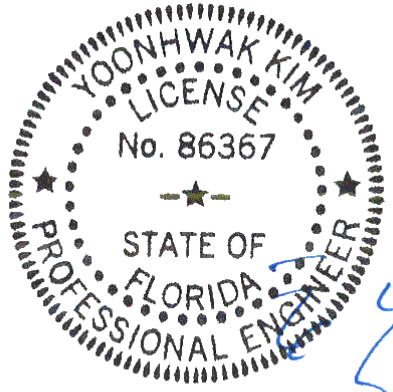
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Reinforcement
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/360.
 Wind loading based on both gable and hip roof types.

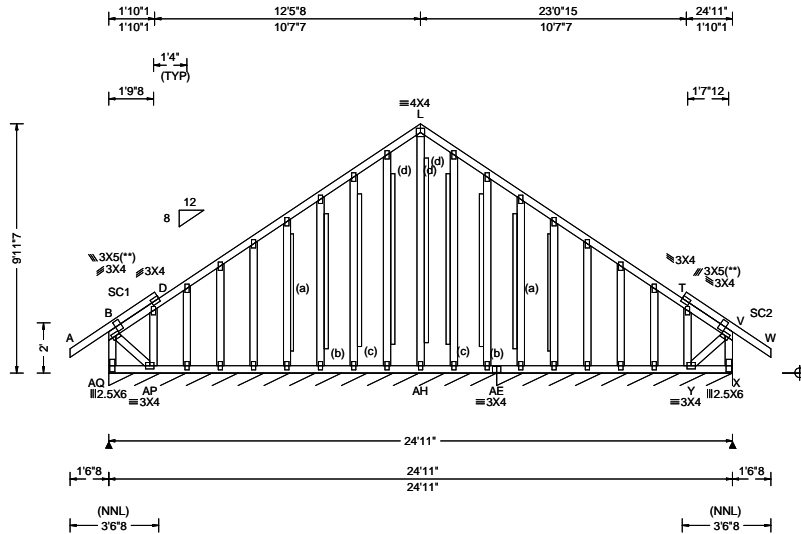
Additional Notes
 The overall height of this truss excluding overhang is 10-3-11.



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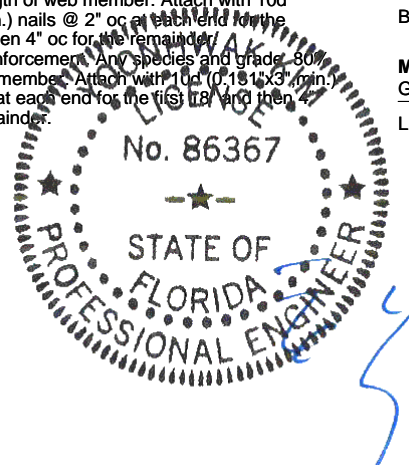
****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.003 M 999 360 VERT(CL): 0.005 L 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.012 I - - Creep Factor: 2.0 Max TC CSI: 0.320 Max BC CSI: 0.061 Max Web CSI: 0.979 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs), or *=PLF <table style="width:100%; border-collapse: collapse;"> <tr> <th colspan="2"></th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> <th></th> </tr> <tr> <td>AQ*152</td> <td>-</td> <td>-</td> <td>-</td> <td>/65</td> <td>/16</td> <td>/32</td> <td></td> </tr> <tr> <td>AE*142</td> <td>-</td> <td>-</td> <td>-</td> <td>/69</td> <td>/38</td> <td>-</td> <td></td> </tr> </table> <p>Wind reactions based on MWFRS AQ Brg Wid = 186 Min Req = - AE Brg Wid = 112 Min Req = - Bearings AQ & AE are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>D - L</td> <td>408</td> <td>T - V</td> <td>237</td> </tr> <tr> <td>L - T</td> <td>408</td> <td>-203</td> <td></td> </tr> </table>			Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL		AQ*152	-	-	-	/65	/16	/32		AE*142	-	-	-	/69	/38	-		Chords	Tens.Comp.	Chords	Tens. Comp.	D - L	408	T - V	237	L - T	408	-203	
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2; Plating Notes All plates are 2X4 except as noted. (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. Loading Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise. Wind Wind loads based on MWFRS with additional C&C member design. End verticals exposed to wind pressure. Deflection meets L/360. Wind loading based on both gable and hip roof types. Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/252.	Gable Reinforcement (a) 1x4 SP/DF #2 or better "L" reinforcement. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder. (b) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder. (c) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder. (d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.	Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>AP-AH</td> <td>455</td> <td>AE- Y</td> <td>476</td> </tr> <tr> <td>AH-AE</td> <td>455</td> <td>-257</td> <td></td> </tr> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> <tr> <td>B -AP</td> <td>471</td> <td>Y - V</td> <td>500</td> </tr> <tr> <td></td> <td>-267</td> <td>-307</td> <td></td> </tr> </table> Maximum Gable Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <tr> <th>Gables</th> <th>Tens.Comp.</th> <th>Gables</th> <th>Tens. Comp.</th> </tr> <tr> <td>L -AH</td> <td>20</td> <td>V - X</td> <td>271</td> </tr> <tr> <td></td> <td>-377</td> <td>-388</td> <td></td> </tr> </table>	Chords	Tens.Comp.	Chords	Tens. Comp.	AP-AH	455	AE- Y	476	AH-AE	455	-257		Webs	Tens.Comp.	Webs	Tens. Comp.	B -AP	471	Y - V	500		-267	-307		Gables	Tens.Comp.	Gables	Tens. Comp.	L -AH	20	V - X	271		-377	-388	
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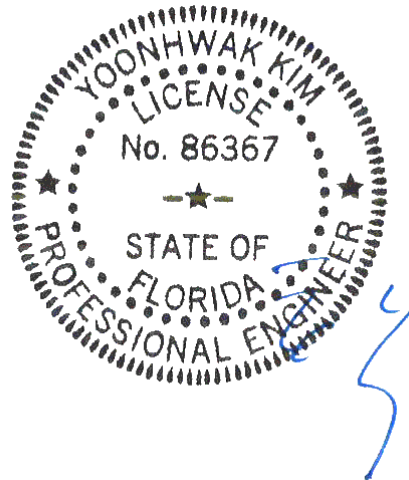
SEQN: 738910	GABL	Ply: 1	Job Number: 26-3717	Cust: R215 JRef: 1YJl2150001 T39
FROM: RFG		Qty: 1	STECKBECK	DrwNo: 112.26.0918.43637
Page 2 of 2			Truss Label: A1E	JKS / YK 04/22/2026

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

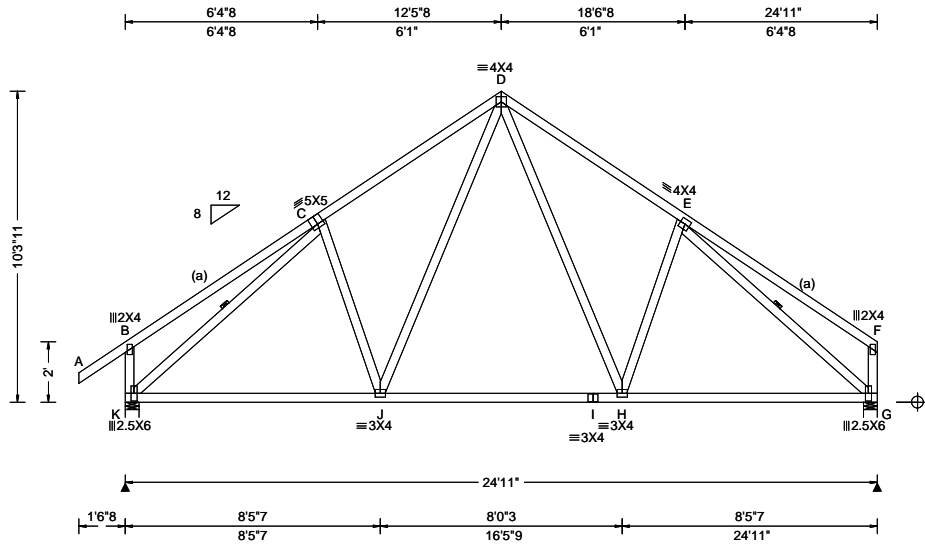
The overall height of this truss excluding overhang is 9-11-7.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.045 H 999 360 VERT(CL): 0.080 H 999 240 HORZ(LL): 0.027 F - - HORZ(TL): 0.049 F - - Creep Factor: 2.0 Max TC CSI: 0.501 Max BC CSI: 0.796 Max Web CSI: 0.552 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K 1358 -/- /- /609 /190 /338 G 1246 -/- /- /595 /166 -/ Wind reactions based on MWFRS K Brg Wid = 5.5 Min Req = 1.6 (Truss) G Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings K & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 489 -1335 D - E 490 -1343
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

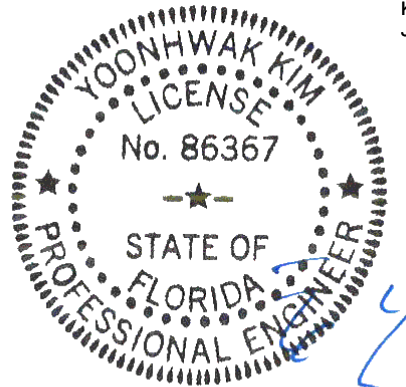
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

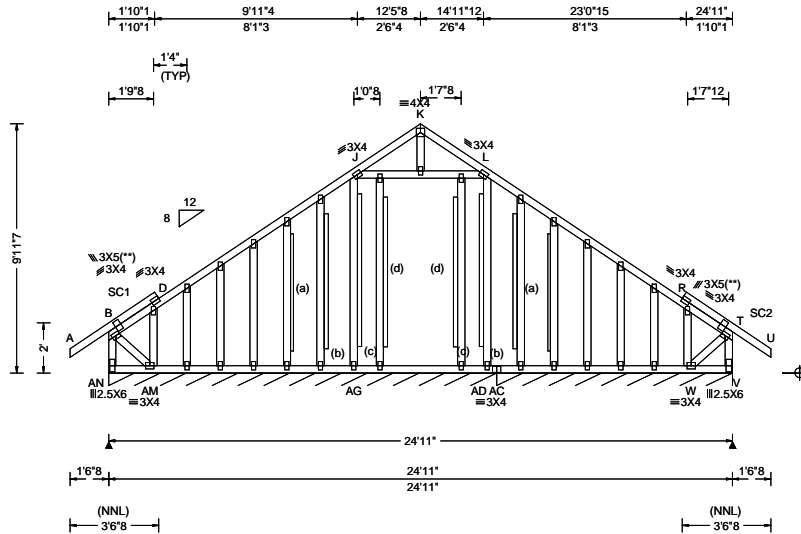
The overall height of this truss excluding overhang is 10-3-11.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 K 999 360 VERT(CL): 0.009 K 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.010 H - - Creep Factor: 2.0 Max TC CSI: 0.320 Max BC CSI: 0.087 Max Web CSI: 0.980 VIEW Ver: 24.02.00D.0114.10	Gravity Loc R+ / R- / Rh / Rw / U / RL AN*152 - / - / - / 64 / 14 / 32 AC*143 - / - / - / 69 / 41 / - Non-Gravity Wind reactions based on MWFRS AN Brg Wid = 186 Min Req = - AC Brg Wid = 112 Min Req = - Bearings AN & AC are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - J 376 -166 L - R 376 -211

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

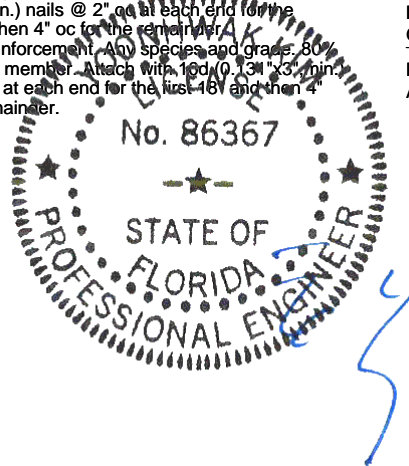
Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/252.

Gable Reinforcement
(a) 1x4 SP/DF #2 or better "L" reinforcement. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(b) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(c) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp.
AC - W 394 -282

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp.
W - T 395 -301

Maximum Gable Forces Per Ply (lbs)
Gables Tens.Comp. Gables Tens. Comp.
B - AN 209 -402 L - AD 37 -378
AG - J 18 -378 T - V 264 -402



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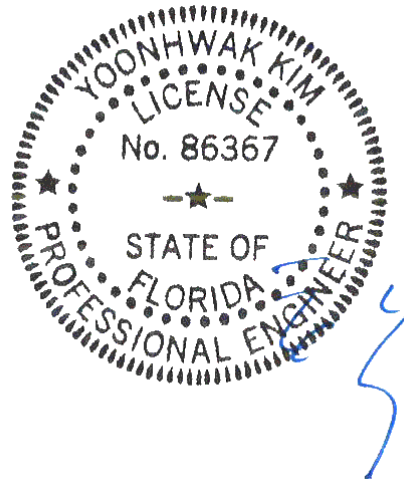
SEQN: 738851	GABL	Ply: 1	Job Number: 26-3717	Cust: R215 JRef: 1YJI2150001 T44
FROM: RFG		Qty: 1	STECKBECK	DrwNo: 112.26.0919.17573
Page 2 of 2			Truss Label: A2E	JKS / YK 04/22/2026

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

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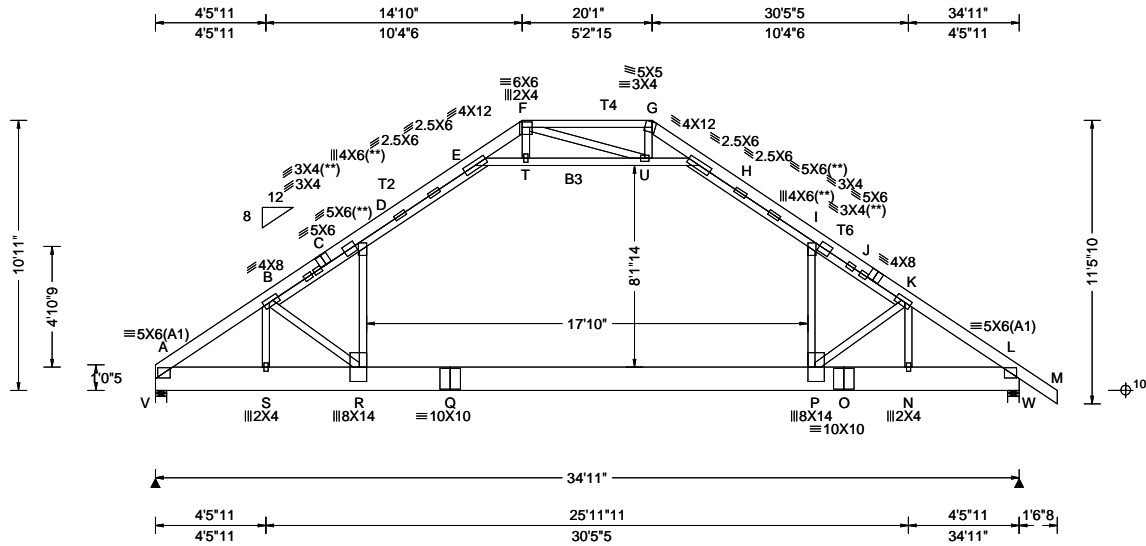
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.33 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.49 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.324 R 999 360 VERT(CL): 0.644 R 644 240 HORZ(LL): 0.195 D - - HORZ(TL): 0.391 D - - Creep Factor: 2.0 Max TC CSI: 0.966 Max BC CSI: 0.515 Max Web CSI: 0.946 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>V</td> <td>2554</td> <td>-</td> <td>-</td> <td>/870</td> <td>/240</td> <td>/303</td> </tr> <tr> <td>W</td> <td>2659</td> <td>-</td> <td>-</td> <td>/883</td> <td>/268</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS V Brg Wid = 5.5 Min Req = 2.1 (Truss) W Brg Wid = 5.5 Min Req = 2.2 (Truss) Bearings V & W are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>5428 -254</td> <td>F - G</td> <td>591 -37</td> </tr> <tr> <td>D - E</td> <td>1062 -2999</td> <td>I - J</td> <td>5291 -234</td> </tr> <tr> <td>H - I</td> <td>5916 -240</td> <td>J - K</td> <td>526 -5372</td> </tr> <tr> <td>A - B</td> <td>513 -3624</td> <td>K - L</td> <td>492 -3586</td> </tr> <tr> <td>B - C</td> <td>1990 -5377</td> <td>J - K</td> <td>1544 -73</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	V	2554	-	-	/870	/240	/303	W	2659	-	-	/883	/268	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	5428 -254	F - G	591 -37	D - E	1062 -2999	I - J	5291 -234	H - I	5916 -240	J - K	526 -5372	A - B	513 -3624	K - L	492 -3586	B - C	1990 -5377	J - K	1544 -73
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Lumber
 Top chord: 2x6 SP 2400f-2.0E; T2,T6 2x4 SP M-31;
 T4 2x4 SP #2;
 Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
 Webs: 2x4 SP #3;

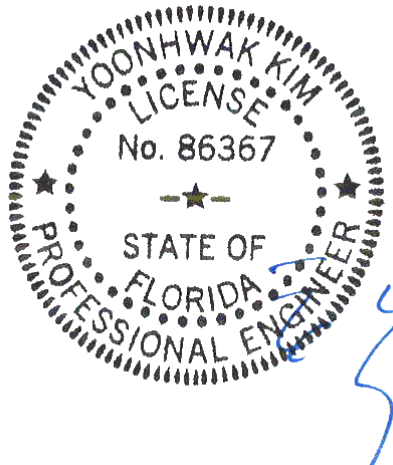
Plating Notes
 (**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Attic room loading from 8-6-8 to 26-4-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

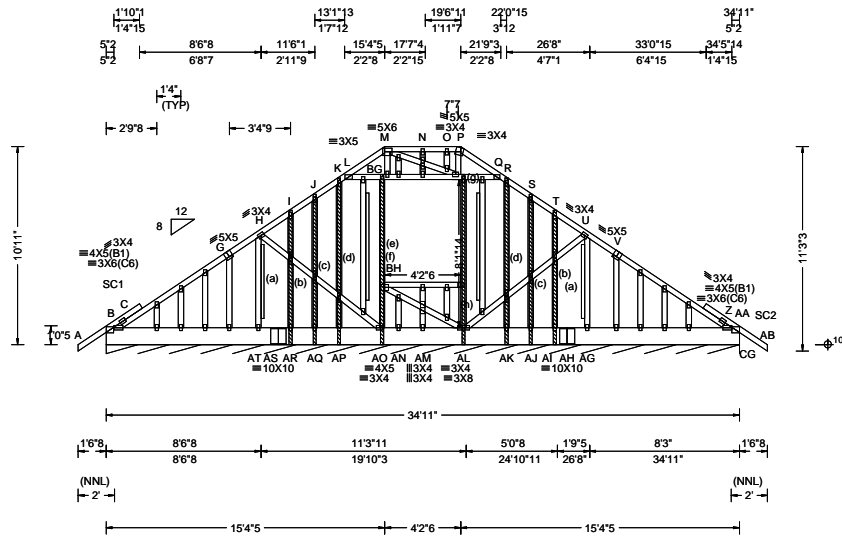
Additional Notes
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.49 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.044 N 999 360 VERT(CL): 0.049 N 999 240 HORZ(LL): 0.006 S - - HORZ(TL): 0.007 S - - Creep Factor: 2.0 Max TC CSI: 0.294 Max BC CSI: 0.013 Max Web CSI: 0.991 VIEW Ver: 24.02.00D.0114.10	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL CG*148 /- /- /57 /15 /11 Wind reactions based on MWFRS CG Brg Wid = 419 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. L - M 69 -411 P - Q 41 -462 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. AO-BH 0 -388 BG-BH 0 -396

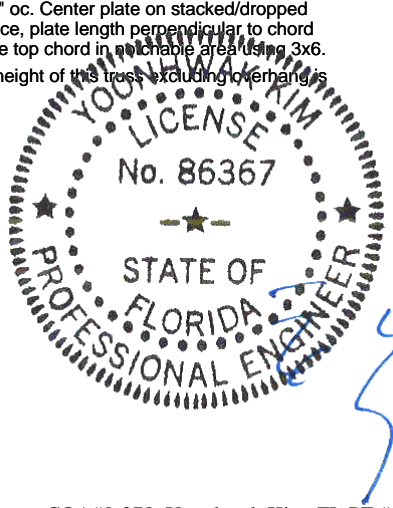
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x12 SP 2400F-2.0E;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes
 All plates are 2X4 except as noted.

Loading
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/198.

Additional Notes
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.
 The overall height of this truss, excluding overhangs is 10'-11-0".



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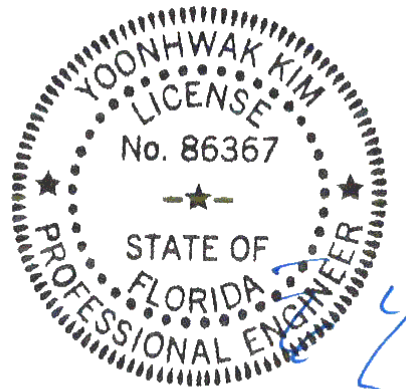
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SEQN: 738895	GABL	Ply: 1	Job Number: 26-3717	Cust: R215 JRef: 1YJ12150001 T21
FROM: RFG		Qty: 1	STECKBECK	DrwNo: 112.26.0919.24180
Page 2 of 2			Truss Label: B1E	JKS / YK 04/22/2026

Gable Reinforcement

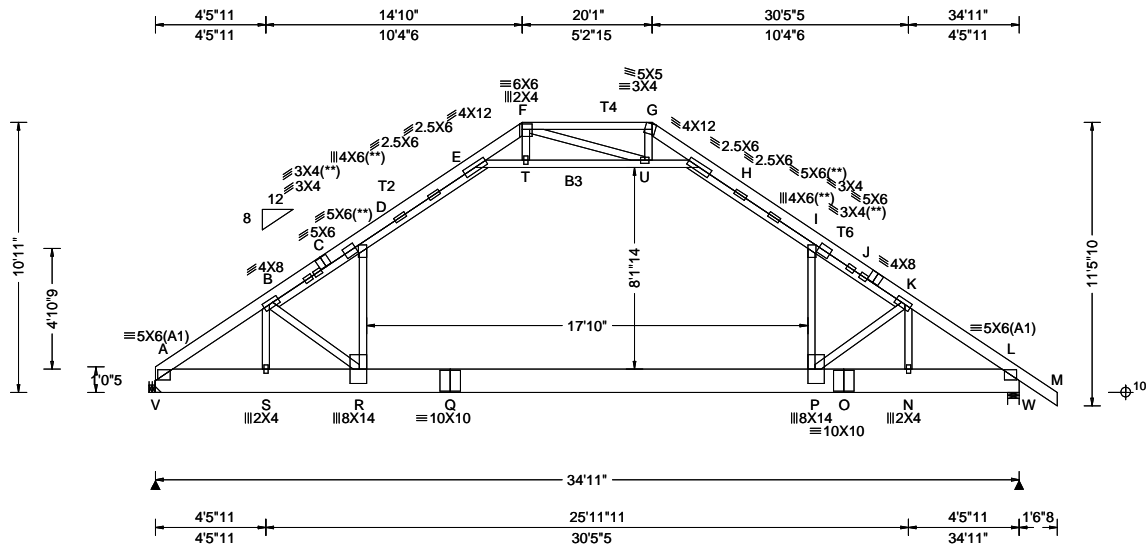
- (a) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (b) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (c) 2x3 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (d) 2x4 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (e) 2x4 SP/DF #2 or better "L" reinforcement. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (f) 2x6 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (g) 2x4 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (h) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.49 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.324 R 999 360 VERT(CL): 0.644 R 644 240 HORZ(LL): 0.195 D - - HORZ(TL): 0.391 D - - Creep Factor: 2.0 Max TC CSI: 0.966 Max BC CSI: 0.515 Max Web CSI: 0.946 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL V 2554 -/ - /813 -/ /303 W 2659 -/ - /825 -/ - Wind reactions based on MWFRS V Brg Wid = - Min Req = - W Brg Wid = 5.5 Min Req = 2.2 (Truss) Bearing W is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 5428 -326 F - G 591 -24 D - E 885 -3003 I - J 5291 -310 H - I 5916 -328 J - K 345 -5372 A - B 299 -3624 K - L 278 -3586 B - C 1808 -5418 J - K 1544 -119
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Lumber
Top chord: 2x6 SP 2400f-2.0E; T2, T6 2x4 SP M-31; T4 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Attic room loading from 8-6-8 to 26-4-8: Live Load: 40
PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - S	2952 -141	P - O	2938 -123
S - R	2975 -141	O - N	2938 -123
R - Q	2643 0	N - L	2914 -123
Q - P	2643 0		

Plating Notes
(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
S - B	107 -1014	U - H	391 -3094
B - R	224 -540	P - K	235 -494
R - D	2149 0	I - P	2139 0
E - T	387 -3022	K - N	132 -1029
T - U	386 -3003		

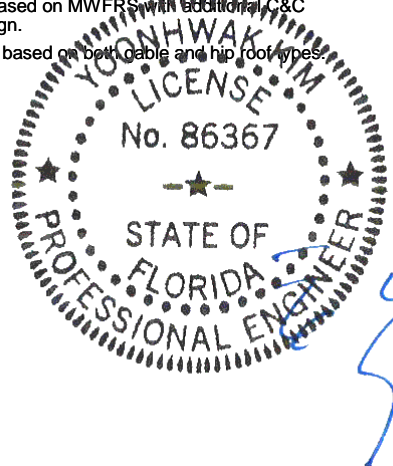
Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Wind
Wind loads based on MWFRS with structural C&C member design.
Wind loading based on both gable and hip roof types.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'
Bearing V (0', 10') HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(6) 0.148"x3" nails into supported member.



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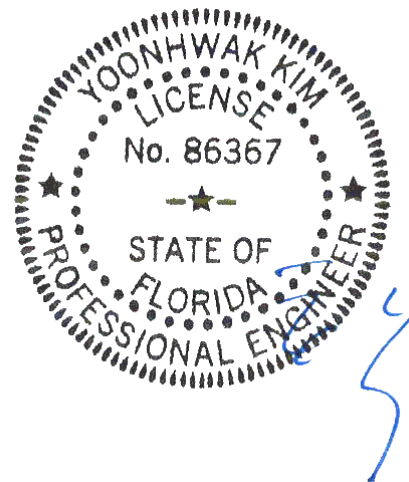
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SEQN: 738734	ATIC	Ply: 1	Job Number: 26-3717	Cust: R215	JRef: 1YJ12150001	T15
FROM: RFG		Qty: 4	STECKBECK	DrwNo: 112.26.0919.26977		
Page 2 of 2			Truss Label: B2	JKS / YK	04/22/2026	

Additional Notes

The overall height of this truss excluding overhang is 10-11-0.



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SEQN: 738886	ATIC	Ply: 2	Job Number: 26-3717	Cust: R215	JRef: 1YJI2150001	T22
FROM: RFG		Qty: 1	STECKBECK	DrwNo: 112.26.0919.29697		
Page 2 of 2			Truss Label: B3	JKS / YK	04/22/2026	

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

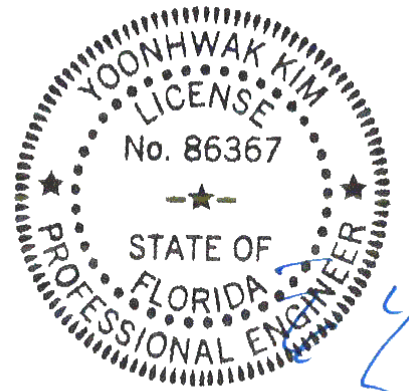
Bearing at location x=0' uses the following support conditions: 0'

Bearing Z (0', 10') HGUS26-2

Supporting Member: (2)2x6 SP 2400f-2.0E

(20) 0.148"x3" nails into supporting member,

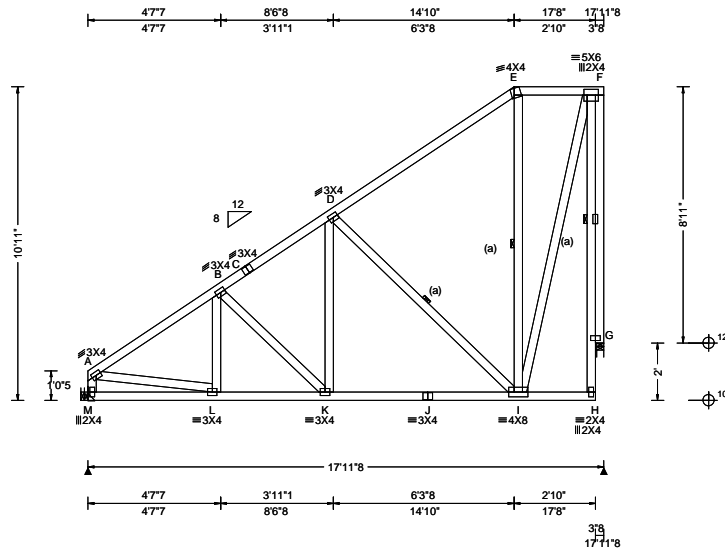
(6) 0.148"x3" nails into supported member.



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 17.01 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.019 K 999 360 VERT(CL): 0.041 K 999 240 HORZ(LL): 0.006 B - - HORZ(TL): 0.014 B - - Creep Factor: 2.0 Max TC CSI: 0.407 Max BC CSI: 0.362 Max Web CSI: 0.701 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>M</td> <td>748</td> <td>-</td> <td>-</td> <td>/467</td> <td>-</td> <td>/305</td> </tr> <tr> <td>G</td> <td>755</td> <td>-</td> <td>-</td> <td>/627</td> <td>/119</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS M Brg Wid = - Min Req = - G Brg Wid = 3.0 Min Req = 1.5 (Support) Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>0 -918</td> <td>C - D</td> <td>0 -703</td> </tr> <tr> <td>B - C</td> <td>0 -739</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>M - L</td> <td>208 -462</td> <td>K - J</td> <td>559 -242</td> </tr> <tr> <td>L - K</td> <td>705 -341</td> <td>J - I</td> <td>559 -242</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - M</td> <td>7 -707</td> <td>I - F</td> <td>734 -306</td> </tr> <tr> <td>A - L</td> <td>671 0</td> <td>F - G</td> <td>231 -583</td> </tr> <tr> <td>D - I</td> <td>233 -517</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	M	748	-	-	/467	-	/305	G	755	-	-	/627	/119	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	0 -918	C - D	0 -703	B - C	0 -739			Chords	Tens.Comp.	Chords	Tens. Comp.	M - L	208 -462	K - J	559 -242	L - K	705 -341	J - I	559 -242	Webs	Tens.Comp.	Webs	Tens. Comp.	A - M	7 -707	I - F	734 -306	A - L	671 0	F - G	231 -583	D - I	233 -517		
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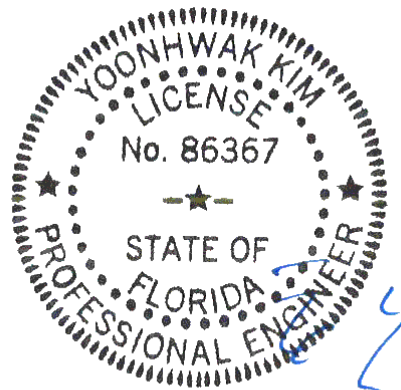
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Rt Bearing Leg: 2x4 SP #3;

Reinforcement
 (a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
 Hanger Support Required, by others

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 10-11-0.

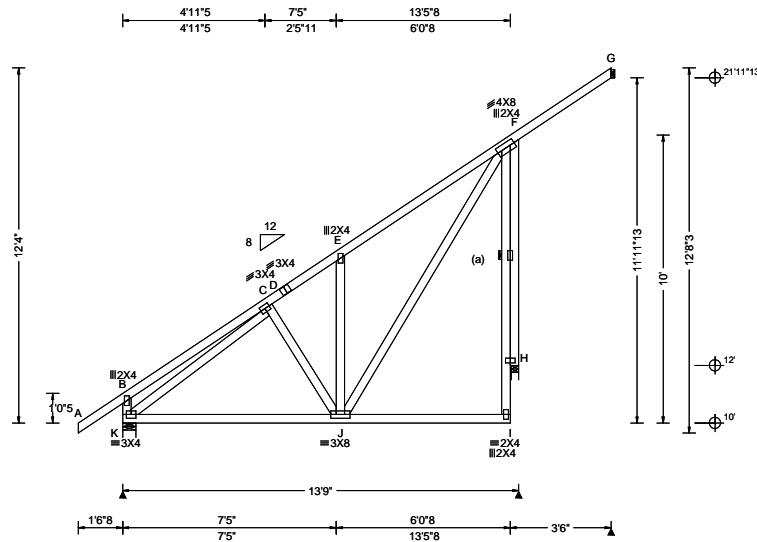


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SEQN: 738881 FROM: RFG	SPEC Ply: 1 Qty: 2	Job Number: 26-3717 STECKBECK Truss Label: C2	Cust: R215 JRRef: 1YJ12150001 T19 DrwNo: 112.26.0919.34643 JKS / YK 04/22/2026
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.17 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.028 E 999 360 VERT(CL): 0.037 E 999 240 HORZ(LL): 0.014 E - - HORZ(TL): 0.020 E - - Creep Factor: 2.0 Max TC CSI: 0.356 Max BC CSI: 0.520 Max Web CSI: 0.640 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K 692 /- /- /327 /- /316 H 844 /- /- /583 /193 /- G 76 /-2 /- /40 /25 /- Wind reactions based on MWFRS K Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = 3.0 Min Req = 1.5 (Support) G Brg Wid = 1.5 Min Req = - Bearings K & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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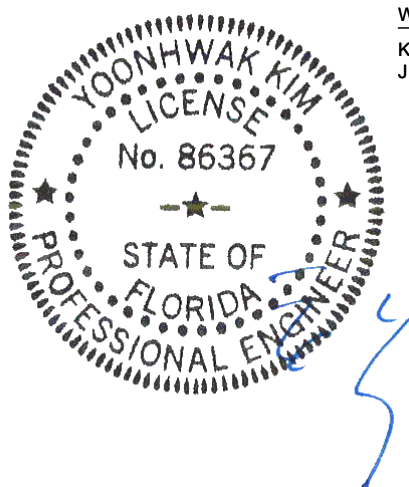
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Reinforcement
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

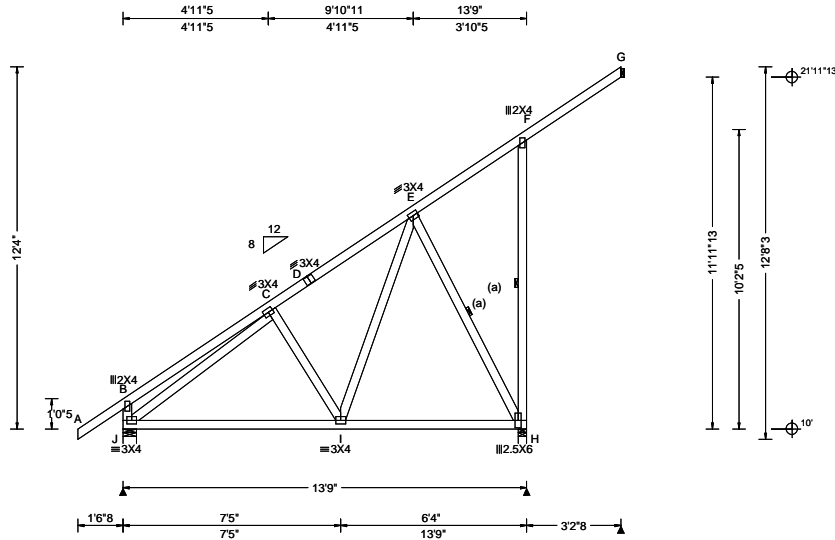
Additional Notes
The overall height of this truss excluding overhang is 12-4-0.



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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.17 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.017 C 999 360 VERT(CL): 0.023 I 999 240 HORZ(LL): -0.008 G - - HORZ(TL): 0.011 G - - Creep Factor: 2.0 Max TC CSI: 0.260 Max BC CSI: 0.594 Max Web CSI: 0.455 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity J 714 /- /- /337 /- /316 H 784 /- /- /561 /178 /- G 86 /- /- /57 /33 /- Wind reactions based on MWFRS J Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) G Brg Wid = 1.5 Min Req = - Bearings J & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 0 -560 D - E 0 -507 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. J - I 507 -253 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. J - C 0 -606 E - H 222 -524 I - E 449 -100
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Loading

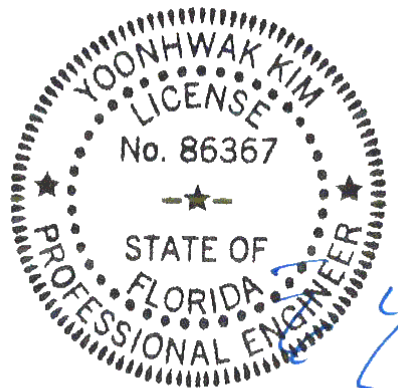
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

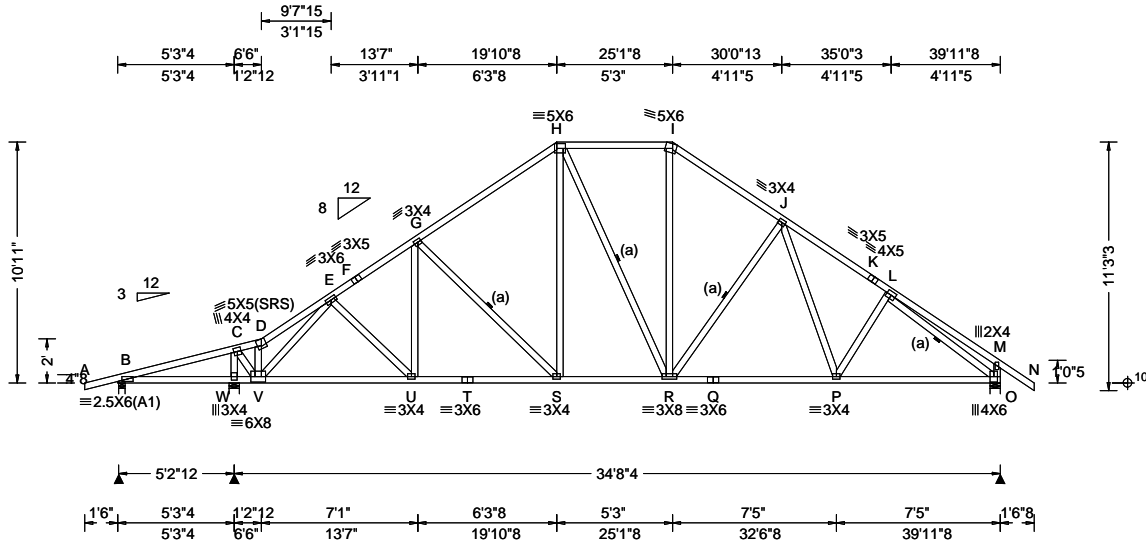
The overall height of this truss excluding overhang is 12-4-0.



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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.084 S 999 360 VERT(CL): 0.156 S 999 240 HORZ(LL): 0.048 M - - HORZ(TL): 0.090 M - - Creep Factor: 2.0 Max TC CSI: 0.507 Max BC CSI: 0.758 Max Web CSI: 0.705 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>203</td> <td>-/53</td> <td>-/</td> <td>/20</td> <td>/114</td> <td>/292</td> </tr> <tr> <td>W</td> <td>2048</td> <td>-/</td> <td>-/</td> <td>/1269</td> <td>/249</td> <td>-/</td> </tr> <tr> <td>O</td> <td>1702</td> <td>-/</td> <td>-/</td> <td>/936</td> <td>/233</td> <td>-/</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) W Brg Wid = 5.5 Min Req = 2.0 (Truss) O Brg Wid = 5.5 Min Req = 2.0 (Truss) Bearings B, W, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>671 -125</td> <td>G - H</td> <td>463 -1684</td> </tr> <tr> <td>C - D</td> <td>111 -500</td> <td>H - I</td> <td>455 -1325</td> </tr> <tr> <td>D - E</td> <td>174 -626</td> <td>I - J</td> <td>478 -1677</td> </tr> <tr> <td>E - F</td> <td>380 -1932</td> <td>J - K</td> <td>479 -2082</td> </tr> <tr> <td>F - G</td> <td>398 -1888</td> <td>K - L</td> <td>452 -2114</td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - W</td> <td>163 -633</td> <td>S - R</td> <td>1305 -20</td> </tr> <tr> <td>W - V</td> <td>140 -493</td> <td>R - Q</td> <td>1590 -162</td> </tr> <tr> <td>V - U</td> <td>1405 -175</td> <td>Q - P</td> <td>1590 -162</td> </tr> <tr> <td>U - T</td> <td>1565 -119</td> <td>P - O</td> <td>1720 -246</td> </tr> <tr> <td>T - S</td> <td>1565 -119</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>W - C</td> <td>390 -1884</td> <td>R - I</td> <td>577 -68</td> </tr> <tr> <td>C - V</td> <td>1674 -239</td> <td>R - J</td> <td>199 -470</td> </tr> <tr> <td>V - E</td> <td>229 -1427</td> <td>L - O</td> <td>314 -2127</td> </tr> <tr> <td>H - S</td> <td>459 -18</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	203	-/53	-/	/20	/114	/292	W	2048	-/	-/	/1269	/249	-/	O	1702	-/	-/	/936	/233	-/	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	671 -125	G - H	463 -1684	C - D	111 -500	H - I	455 -1325	D - E	174 -626	I - J	478 -1677	E - F	380 -1932	J - K	479 -2082	F - G	398 -1888	K - L	452 -2114	Chords	Tens.Comp.	Chords	Tens. Comp.	B - W	163 -633	S - R	1305 -20	W - V	140 -493	R - Q	1590 -162	V - U	1405 -175	Q - P	1590 -162	U - T	1565 -119	P - O	1720 -246	T - S	1565 -119			Webs	Tens.Comp.	Webs	Tens. Comp.	W - C	390 -1884	R - I	577 -68	C - V	1674 -239	R - J	199 -470	V - E	229 -1427	L - O	314 -2127	H - S	459 -18		
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Loading

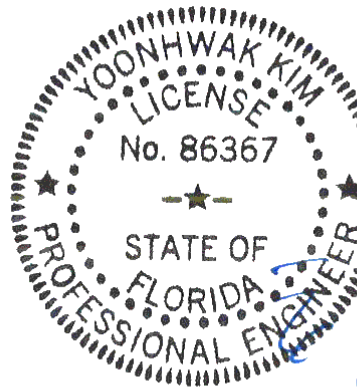
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

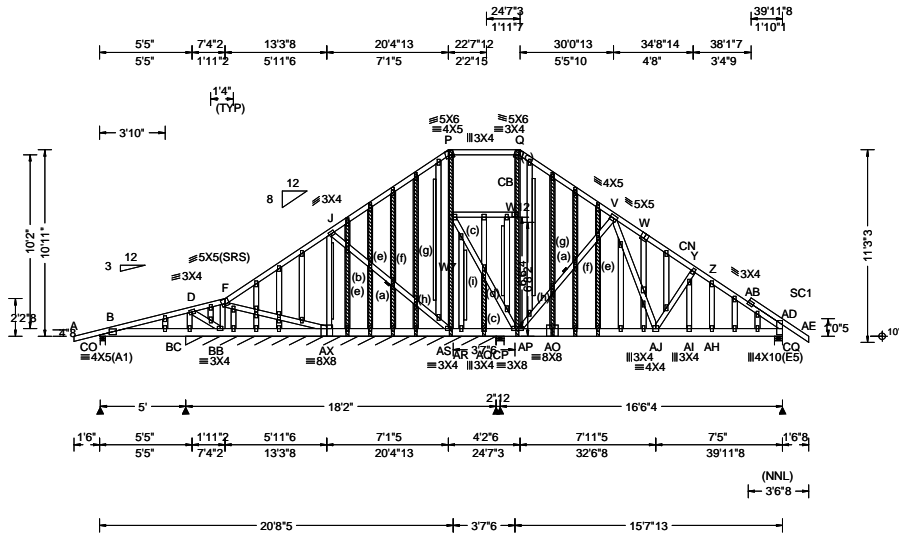
The overall height of this truss excluding overhang is 10-11-0.



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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W7,W12 2x4 SP #2;
 Stack Chord: SC1 2x4 SP #2;

Reinforcement
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 61 plf at -1.50 to 61 plf at 7.34
 TC: From 64 plf at 7.34 to 64 plf at 41.50
 BC: From 4 plf at -1.50 to 4 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 39.96
 BC: From 5 plf at 39.96 to 5 plf at 41.50
 BC: 631 lb Conc. Load at 6.74
 BC: 257 lb Conc. Load at 8.77,10.77,12.77,14.77
 BC: 669 lb Conc. Load at 16.15
 BC: 827 lb Conc. Load at 35.85
 BC: 177 lb Conc. Load at 37.90

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/127.
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point)

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
V - W	174 -1426	Z - AB	277 -2132
W - Y	198 -1608	AB-AD	220 -1692
Y - Z	236 -1860	AB-AD	68 -471

Maximum Bot Chord Forces Per Ply (lbs)

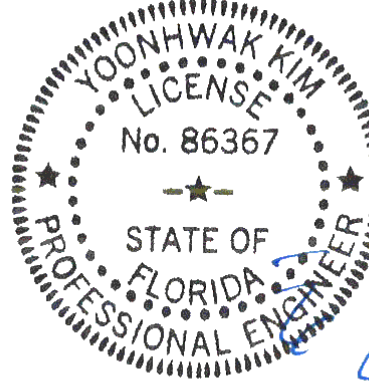
Chords	Tens.Comp.	Chords	Tens. Comp.
AP-AO	830 -93	AI-AH	1706 -218
AO-AJ	830 -93	AH-AD	1744 -225
AJ-AI	1689 -216		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
AP - V	153 -1318	V - AJ	1198 -146
CB-AP	92 -599	AJ - Y	159 -1067
CB - Q	103 -689		

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.	Gables	Tens. Comp.
CN-AI	544 -66	AH - Z	390 -44



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Gable Reinforcement

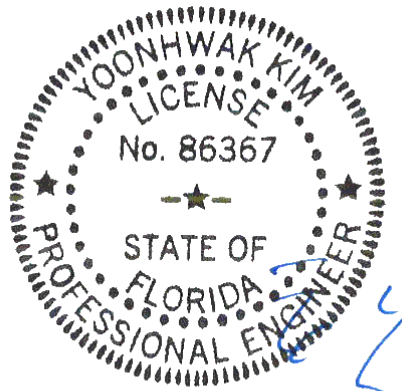
- (b) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (c) 2x6 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (d) 2x3 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (e) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (f) 2x3 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (g) 2x4 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (h) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (i) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 10-11-0.

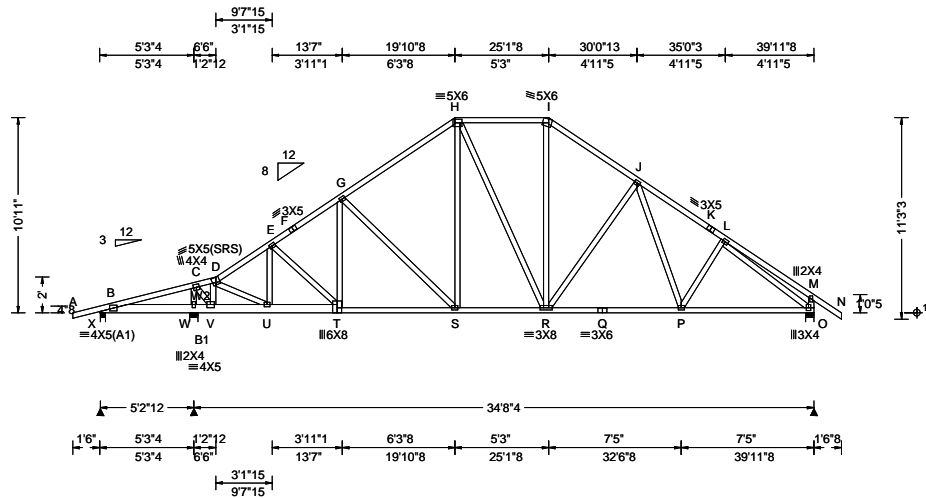


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2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.46 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.036 S 999 360 VERT(CL): 0.073 S 999 240 HORZ(LL): 0.018 M - - HORZ(TL): 0.037 M - - Creep Factor: 2.0 Max TC CSI: 0.268 Max BC CSI: 0.299 Max Web CSI: 0.373 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>X</td> <td>172</td> <td>-104</td> <td>-</td> <td>-</td> <td>42</td> <td>-</td> </tr> <tr> <td>W</td> <td>3667</td> <td>-</td> <td>-</td> <td>-</td> <td>345</td> <td>-</td> </tr> <tr> <td>O</td> <td>1562</td> <td>-</td> <td>-</td> <td>-</td> <td>274</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS X Brg Wid = 3.5 Min Req = 1.5 (Truss) W Brg Wid = 5.5 Min Req = 1.5 (Truss) O Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings X, W, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>616 -70</td> <td>H - I</td> <td>94 -587</td> </tr> <tr> <td>D - E</td> <td>129 -953</td> <td>I - J</td> <td>130 -748</td> </tr> <tr> <td>E - F</td> <td>143 -923</td> <td>J - K</td> <td>145 -924</td> </tr> <tr> <td>F - G</td> <td>134 -902</td> <td>K - L</td> <td>151 -940</td> </tr> <tr> <td>G - H</td> <td>134 -764</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	X	172	-104	-	-	42	-	W	3667	-	-	-	345	-	O	1562	-	-	-	274	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	616 -70	H - I	94 -587	D - E	129 -953	I - J	130 -748	E - F	143 -923	J - K	145 -924	F - G	134 -902	K - L	151 -940	G - H	134 -764		
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B1 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W2 2x4 SP #2;

Nailnote
 Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 61 plf at -1.50 to 61 plf at 6.50
 TC: From 64 plf at 6.50 to 64 plf at 19.88
 TC: From 61 plf at 19.88 to 61 plf at 25.12
 TC: From 64 plf at 25.12 to 64 plf at 41.50
 BC: From 4 plf at -1.50 to 4 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 39.96
 BC: From 5 plf at 39.96 to 5 plf at 41.50
 BC: 1640 lb Conc. Load at 6.62

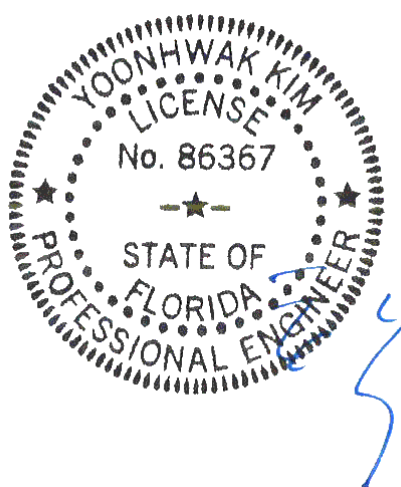
Additional Notes
 The overall height of this truss excluding overhang is 10-11-0.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	62 -582	S - R	588 -94
W - V	49 -449	R - Q	708 -117
V - U	442 -30	Q - P	708 -117
U - T	778 -103	P - O	769 -130
T - S	745 -112		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
W - C	158 -1620	D - U	498 -79
C - V	1477 -112	L - O	168 -943
V - D	117 -677		

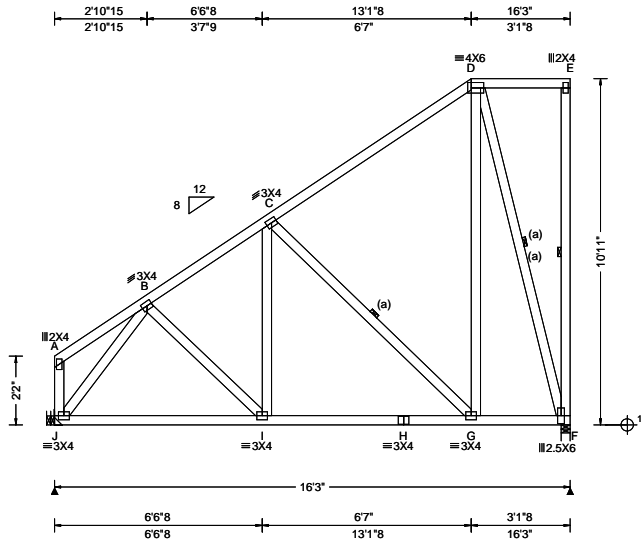


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SEQN: 738918 FROM: RFG	COMN Ply: 1 Qty: 3	Job Number: 26-3717 STECKBECK Truss Label: D2	Cust: R 215 JRRef: 1YJ12150001 T48 DrwNo: 112.26.0924.01730 JKS / YK 04/22/2026
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 17.42 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.017 I 999 360 VERT(CL): 0.031 I 999 240 HORZ(LL): 0.009 C - - HORZ(TL): 0.016 C - - Creep Factor: 2.0 Max TC CSI: 0.552 Max BC CSI: 0.426 Max Web CSI: 0.521 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>J</td> <td>732</td> <td>-</td> <td>-</td> <td>/416</td> <td>-</td> <td>/309</td> </tr> <tr> <td>F</td> <td>732</td> <td>-</td> <td>-</td> <td>/447</td> <td>/271</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	J	732	-	-	/416	-	/309	F	732	-	-	/447	/271	-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Hanger Support Required, by others

Loading

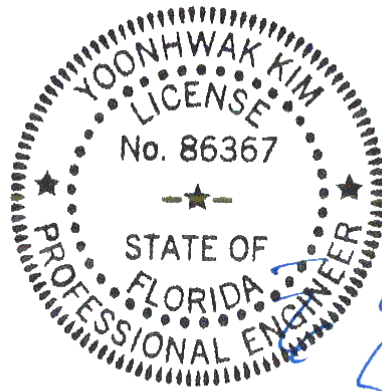
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

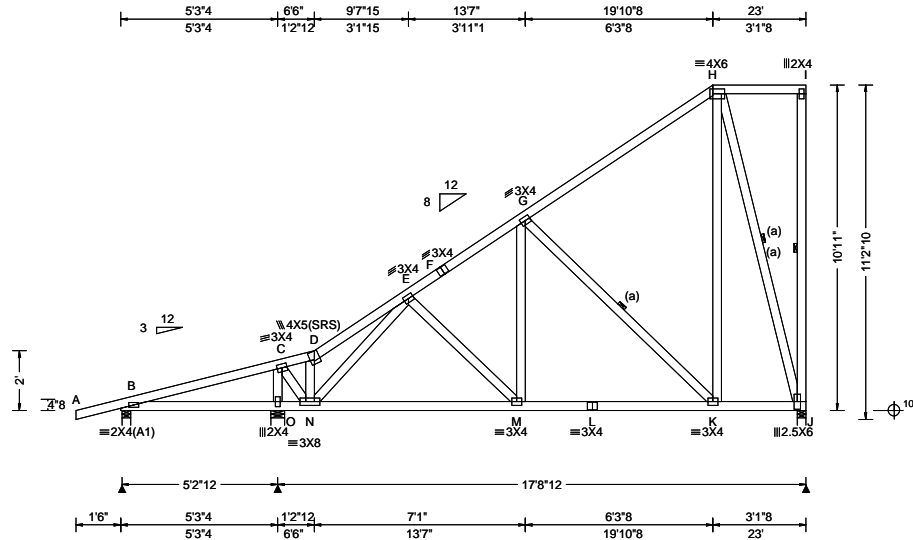
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Loading

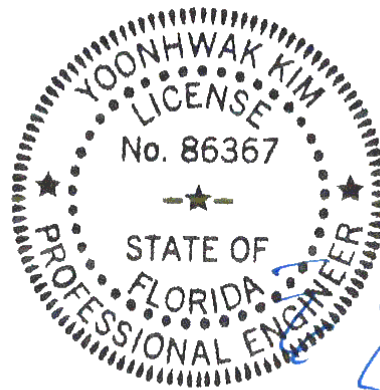
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Wind

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Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

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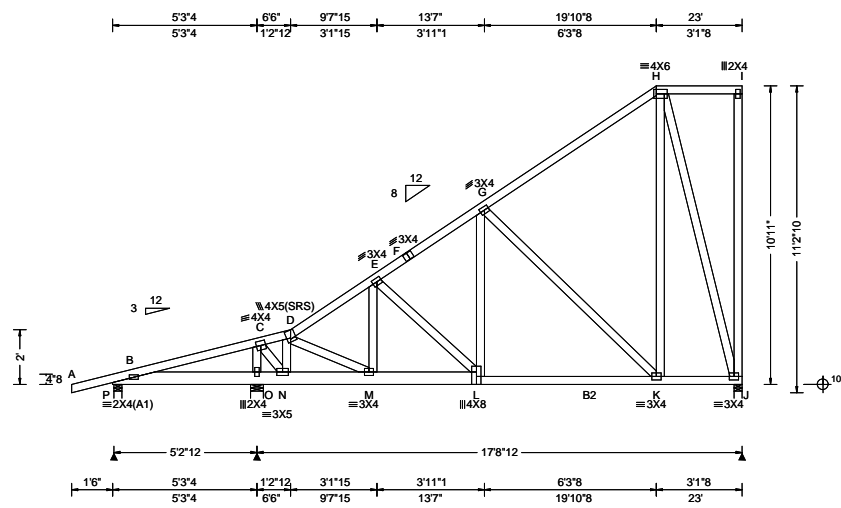


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2 Complete Trusses Required

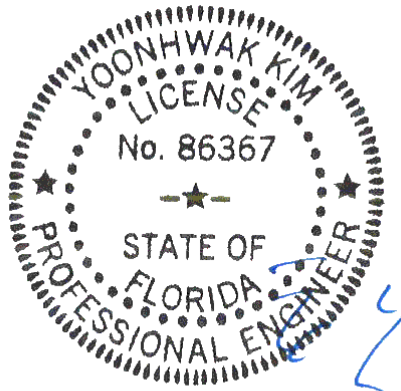


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.46 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.012 L 999 360 VERT(CL): 0.025 L 999 240 HORZ(LL): 0.003 E - - HORZ(TL): 0.007 E - - Creep Factor: 2.0 Max TC CSI: 0.318 Max BC CSI: 0.156 Max Web CSI: 0.426 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity P 251 /- /- /- /69 /- O 2805 /- /- /- /208 /- J 797 /- /- /- /127 /- Wind reactions based on MWFRS P Brg Wid = 3.5 Min Req = 1.5 (Truss) O Brg Wid = 5.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings P, O, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 22 -413 E - F 63 -422 D - E 69 -586 F - G 54 -400 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. N - M 436 -28 M - L 460 -51 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. O - C 98 -1246 C - N 1117 -54
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E; B2 2x4 SP #2;
Webs: 2x4 SP #3;

Nailnote
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 61 plf at -1.50 to 61 plf at 6.50
TC: From 64 plf at 6.50 to 64 plf at 19.88
TC: From 61 plf at 19.88 to 61 plf at 23.00
BC: From 4 plf at -1.50 to 4 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 23.00
BC: 1701 lb Conc. Load at 6.62



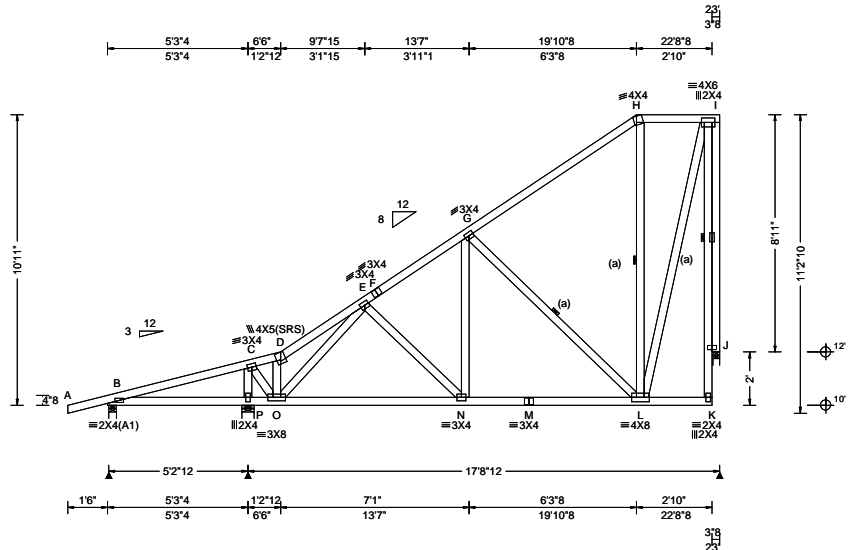
Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 10-11-0.

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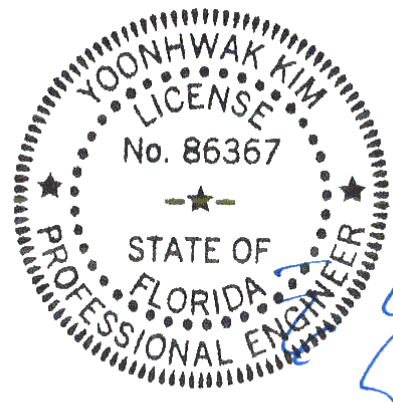
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.017 N 999 360 VERT(CL): 0.035 N 999 240 HORZ(LL): 0.006 E - - HORZ(TL): 0.013 E - - Creep Factor: 2.0 Max TC CSI: 0.410 Max BC CSI: 0.421 Max Web CSI: 0.631 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 282 - / - / - / 91 / 86 / 316 P 1030 - / - / - / 643 - / - J 720 - / - / - / 587 / 97 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) P Brg Wid = 5.5 Min Req = 1.5 (Truss) J Brg Wid = 3.0 Min Req = 1.5 (Support) Bearings B, P, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 0 -411 F - G 0 -659 E - F 0 -678 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. O - N 561 -271 M - L 518 -209 N - M 518 -209 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. P - C 152 -917 G - L 198 -472 C - O 717 -7 L - I 694 -276 O - E 92 -407 I - J 216 -565
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #2;

Reinforcement
(a) Continuous lateral restraint equally spaced on member.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 10-11-0.

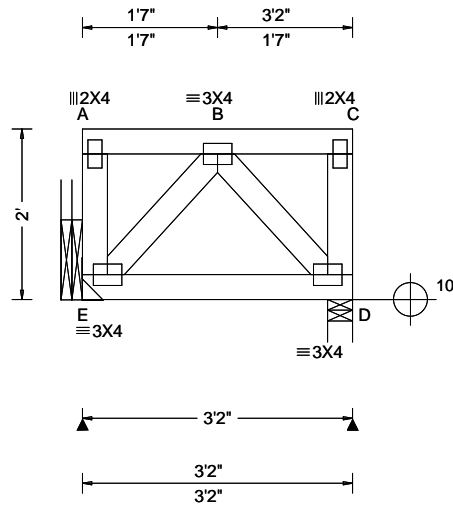


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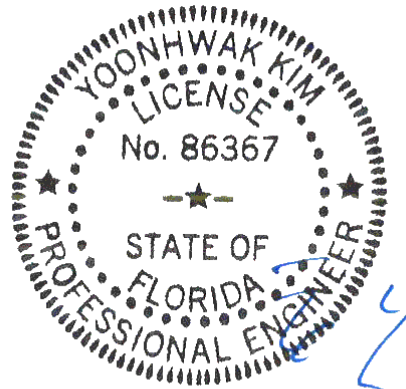


2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: NA Loc. from endwall: not in 10.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 360 VERT(CL): 0.002 B 999 240 HORZ(LL): 0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.262 Max BC CSI: 0.055 Max Web CSI: 0.051 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E 724 /- /- /- /114 /- D 877 /- /- /- /126 /- Wind reactions based on MWFRS E Brg Wid = - Min Req = - D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearing D is a rigid surface. Members not listed have forces less than 375#
				Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Nailnote Nail Schedule: 0.128"x3", min. nails Top Chord: 1 Row @ 3.25" o.c. Bot Chord: 1 Row @ 12.00" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting. Special Loads -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 30 plf at 0.00 to 30 plf at 3.17 BC: From 10 plf at 0.00 to 10 plf at 3.17 TC: 755 lb Conc. Load at 0.77 TC: 720 lb Conc. Load at 2.77 Purlins The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing. Wind Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure. Additional Notes Truss must be installed as shown with top chord up. The overall height of this truss excluding overhang is 2'-0".

A MIN. ROOF SLOPE OF 0.25" PER LINEAR FOOT MUST BE PROVIDED. If parapet walls or condition exist that restrict free runoff, roof drains and scuppers must be provided that will meet code requirements and allow complete drainage of roof at all times to avoid ponding. This truss is NOT designed for ponding. Provide for complete roof drainage.



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SEQN: 738770	FLAT	Ply: 2	Job Number: 26-3717	Cust: R215	JRef: 1YJI2150001	T24
FROM: RFG		Qty: 1	STECKBECK	DrwNo: 112.26.0921.16593		
Page 2 of 2			Truss Label: FG1	JKS / YK	04/22/2026	

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

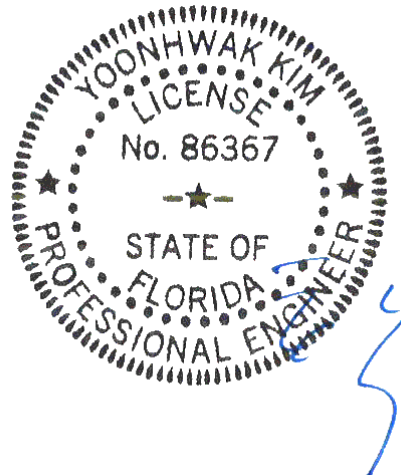
Bearing at location $x=0'$, $y=10'$ uses the following support conditions: 0'

Bearing E (0', 10') HGUS26-2

Supporting Member: (2)2x12 SP 2400f-2.0E

(20) 0.148"x3" nails into supporting member,

(6) 0.148"x3" nails into supported member.

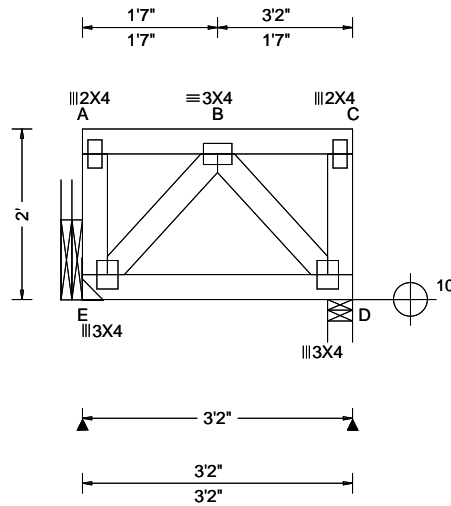


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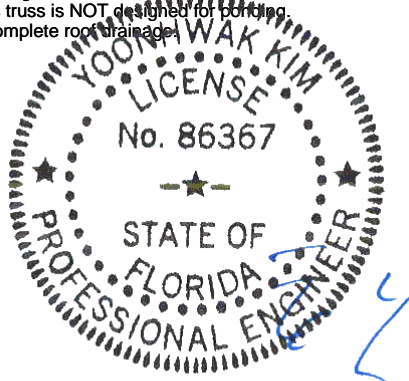
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2 Complete Trusses Required



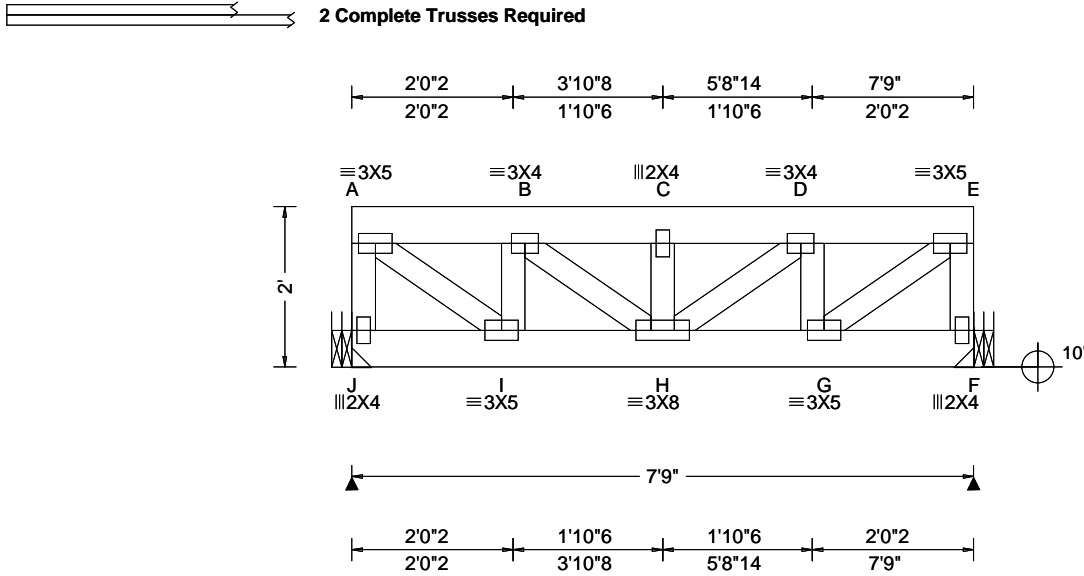
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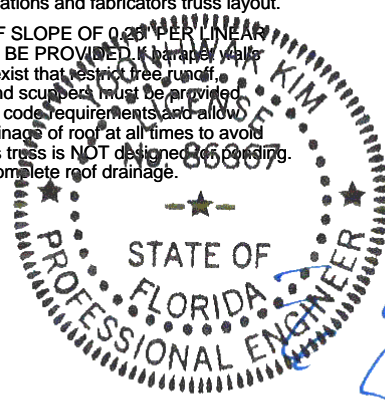
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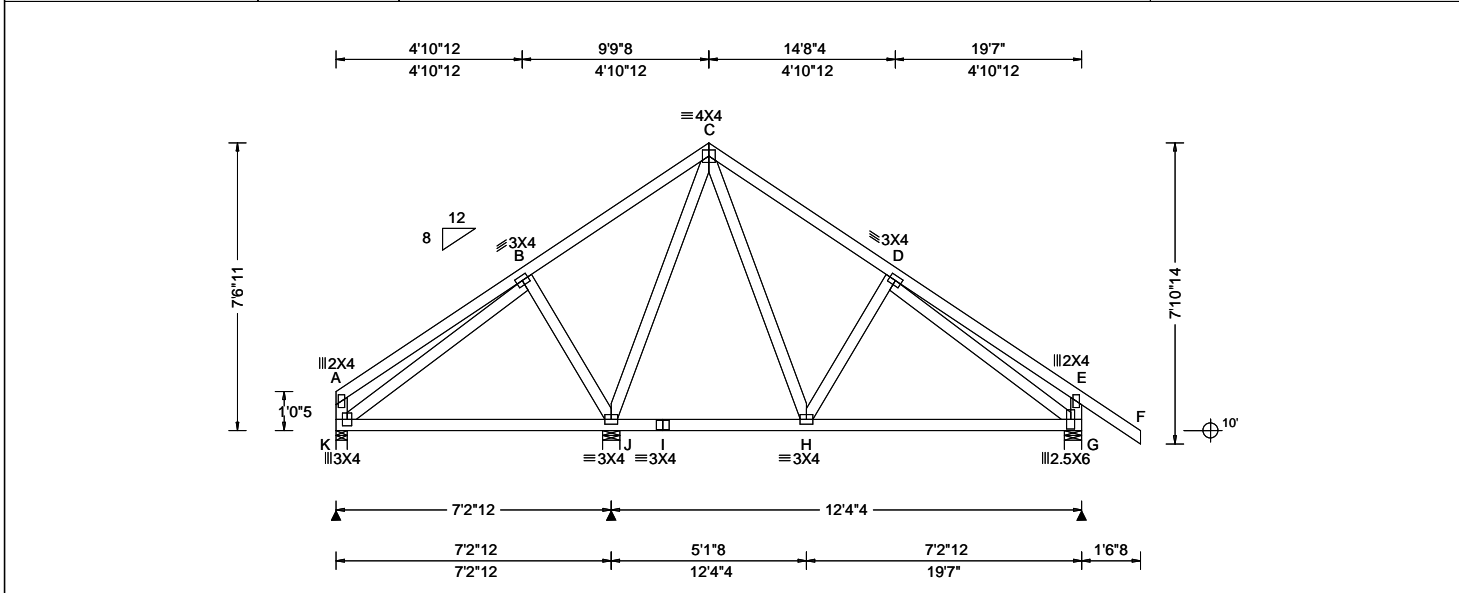
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Lumber Top chord: 2x6 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #3; Nailnote Nail Schedule: 0.128"x3", min. nails Top Chord: 1 Row @ 10.50" o.c. Bot Chord: 1 Row @ 4.75" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting. Special Loads -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 30 plf at 0.00 to 30 plf at 2.21 TC: From 154 plf at 2.21 to 154 plf at 4.54 TC: From 30 plf at 4.54 to 30 plf at 7.75 BC: From 10 plf at 0.00 to 10 plf at 7.75 TC: 144 lb Conc. Load at 2.21, 4.54 BC: 128 lb Conc. Load at 1.15 BC: 732 lb Conc. Load at 1.94, 3.94 BC: 860 lb Conc. Load at 5.94 Hangers / Ties Hanger Support Required, by others Purlins The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing. Wind Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure.				Additional Notes Truss must be installed as shown with top chord up. The overall height of this truss excluding overhang is 2'-0". It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout. A MIN. ROOF SLOPE OF 0.25" PER LINEAR FOOT MUST BE PROVIDED. If barrier walls or condition exist that restrict free runoff, roof drains and scuppers must be provided that will meet code requirements and allow complete drainage of roof at all times to avoid ponding. This truss is NOT designed for ponding. Provide for complete roof drainage.		Maximum Bot Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>I - H</td> <td>1000 -7</td> <td>H - G</td> <td>969 -8</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - J</td> <td>10 -798</td> <td>H - D</td> <td>417 0</td> </tr> <tr> <td>A - I</td> <td>1199 -6</td> <td>G - E</td> <td>1171 -8</td> </tr> <tr> <td>I - B</td> <td>11 -390</td> <td>E - F</td> <td>11 -778</td> </tr> <tr> <td>B - H</td> <td>375 0</td> <td></td> <td></td> </tr> </tbody> </table>		Chords	Tens.Comp.	Chords	Tens. Comp.	I - H	1000 -7	H - G	969 -8	Webs	Tens.Comp.	Webs	Tens. Comp.	A - J	10 -798	H - D	417 0	A - I	1199 -6	G - E	1171 -8	I - B	11 -390	E - F	11 -778	B - H	375 0															
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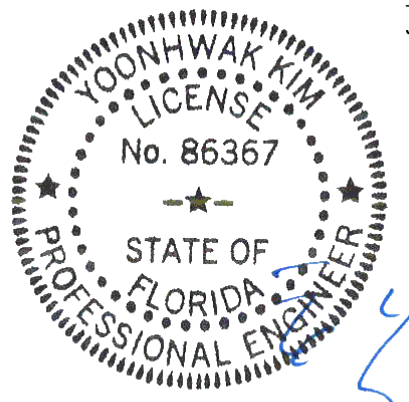


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.008 D 999 360 VERT(CL): 0.017 D 999 240 HORZ(LL): 0.004 E - - HORZ(TL): 0.009 E - - Creep Factor: 2.0 Max TC CSI: 0.356 Max BC CSI: 0.471 Max Web CSI: 0.600 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL K 260 /- /- /141 /33 /200 J 901 /- /- /562 /152 /- G 611 /- /- /328 /99 /- Wind reactions based on MWFRS K Brg Wid = 3.5 Min Req = 1.5 (Truss) J Brg Wid = 5.5 Min Req = 1.5 (Truss) G Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings K, J, & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. C - D 183 -396 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. H - G 377 -11 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. J - C 57 -527 D - G 25 -435
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

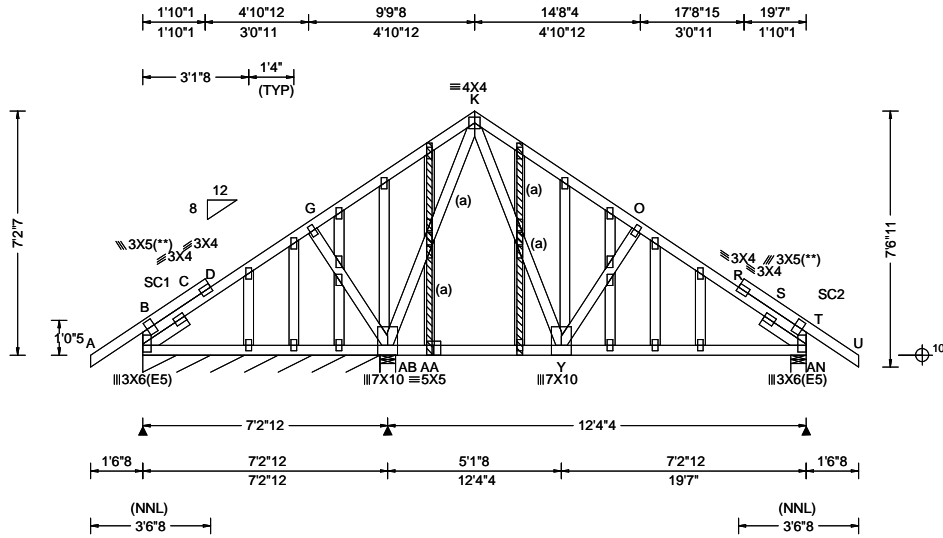
Additional Notes
The overall height of this truss excluding overhang is 7-6-11.



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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;
 Lt Slider: 2x4 SP #3; block length = 1.500'
 Rt Slider: 2x4 SP #3; block length = 1.500'

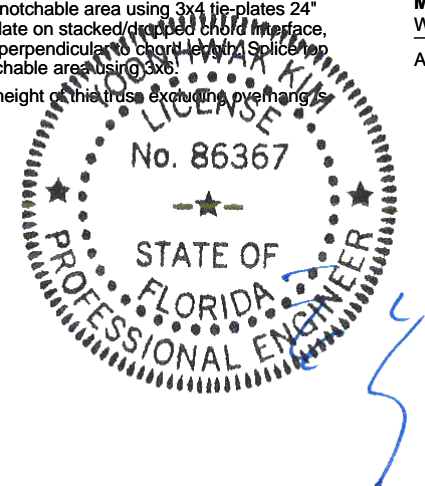
Plating Notes
 All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Additional Notes
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 7-2-7.

Loading
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/269.

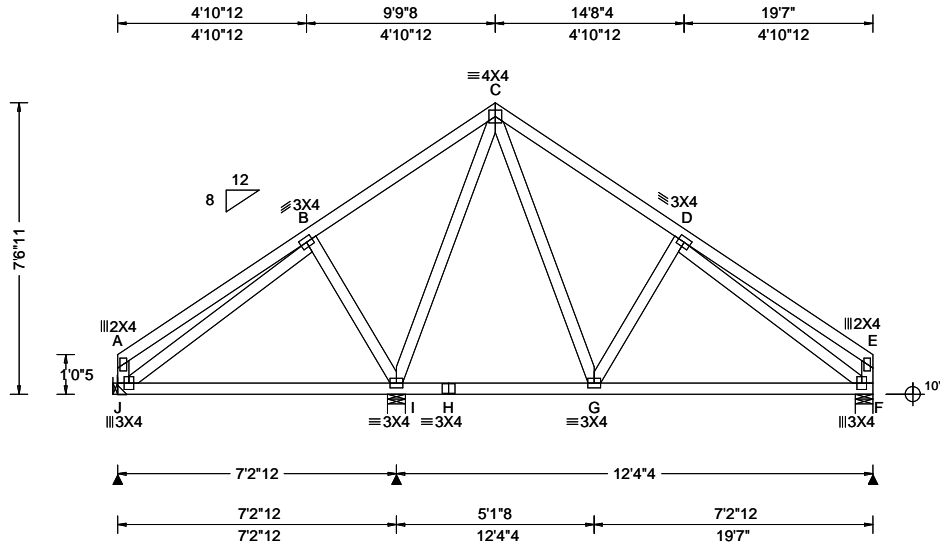
Gable Reinforcement
 (a) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.



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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

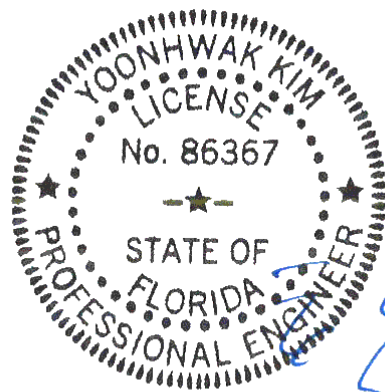
Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

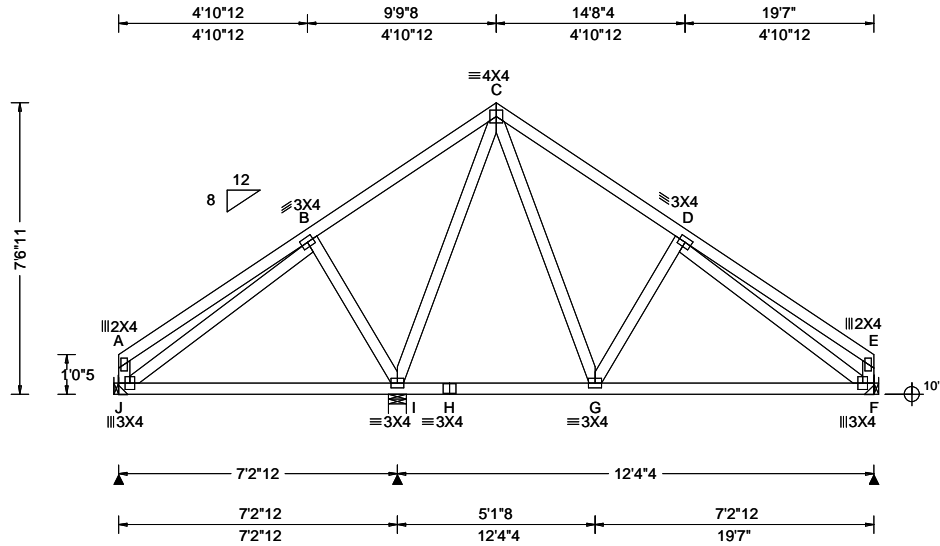
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

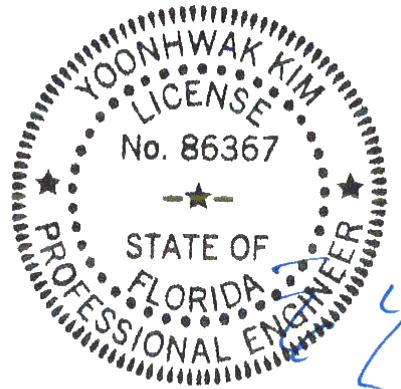
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

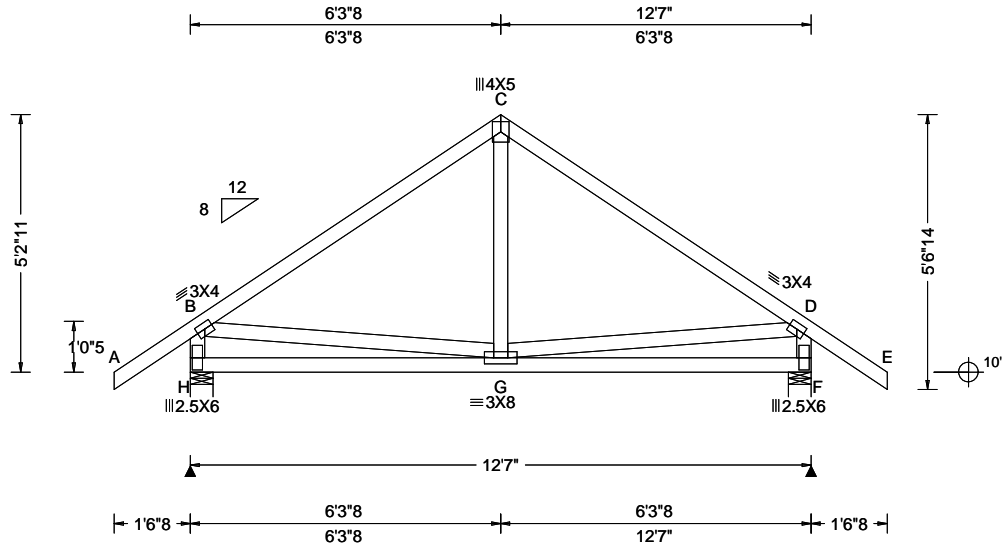
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.007 G 999 360 VERT(CL): 0.013 G 999 240 HORZ(LL): 0.001 D - - HORZ(TL): 0.004 D - - Creep Factor: 2.0 Max TC CSI: 0.452 Max BC CSI: 0.356 Max Web CSI: 0.200 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL H 635 /- /- /315 /108 /118 F 635 /- /- /315 /108 /- Wind reactions based on MWFRS H Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 266 -566 C - D 266 -566
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Lumber

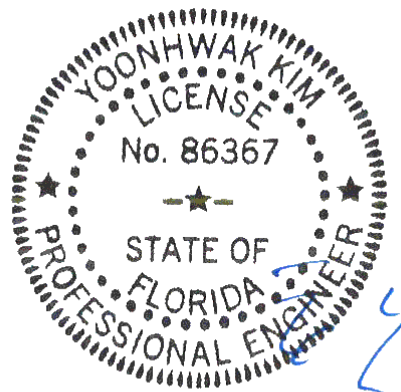
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

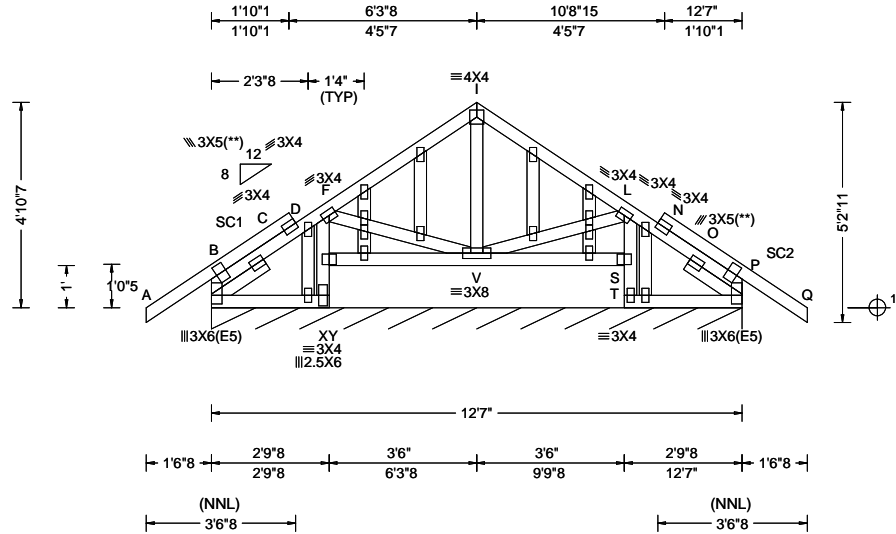
The overall height of this truss excluding overhang is 5-2-11.



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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;
 Lt Slider: 2x4 SP #3; block length = 1.50'
 Rt Slider: 2x4 SP #3; block length = 1.50'

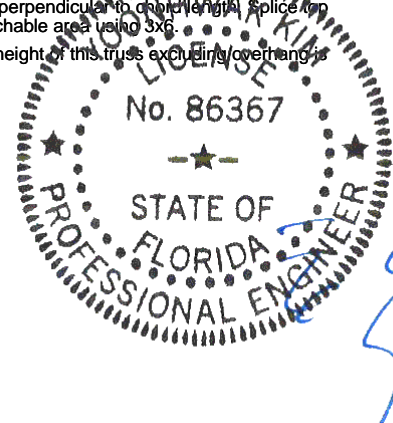
Plating Notes
 All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins
 In lieu of structural panels use purlins to brace TC @ 24" oc.

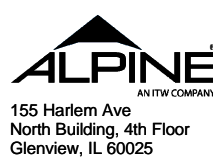
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 4-10-7.

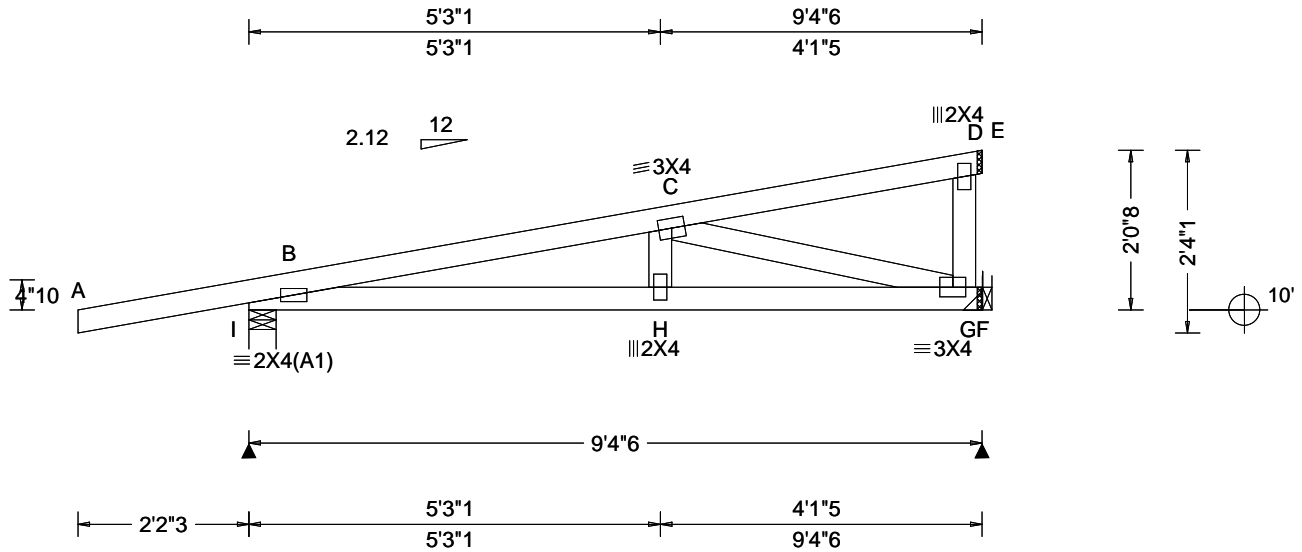


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SEQN: 738826 FROM: RFG	HIP_ Ply: 1 Qty: 1	Job Number: 26-3717 STECKBECK Truss Label: HJ1	Cust: R215 JRef: 1YJ12150001 T9 DrwNo: 112.26.0924.17843 JKS / YK 04/22/2026
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.034 H 999 360 VERT(CL): 0.070 H 999 240 HORZ(LL): 0.007 G - - HORZ(TL): 0.015 G - - Creep Factor: 2.0 Max TC CSI: 0.383 Max BC CSI: 0.543 Max Web CSI: 0.342 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>I</td> <td>358</td> <td>-</td> <td>-</td> <td>-</td> <td>/62</td> <td>-</td> </tr> <tr> <td>F</td> <td>374</td> <td>-</td> <td>-</td> <td>-</td> <td>/51</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	I	358	-	-	-	/62	-	F	374	-	-	-	/51	-
				Loc		Gravity			Non-Gravity																						
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 0 plf at -2.18 to 60 plf at 0.00
 TC: From 2 plf at 0.00 to 2 plf at 9.37
 BC: From 0 plf at -2.18 to 4 plf at 0.00
 BC: From 2 plf at 0.00 to 2 plf at 9.37
 TC: 8 lb Conc. Load at 1.75
 TC: 108 lb Conc. Load at 3.73
 TC: 233 lb Conc. Load at 6.56
 BC: 24 lb Conc. Load at 1.75
 BC: 85 lb Conc. Load at 3.73
 BC: 166 lb Conc. Load at 6.56

Hangers / Ties

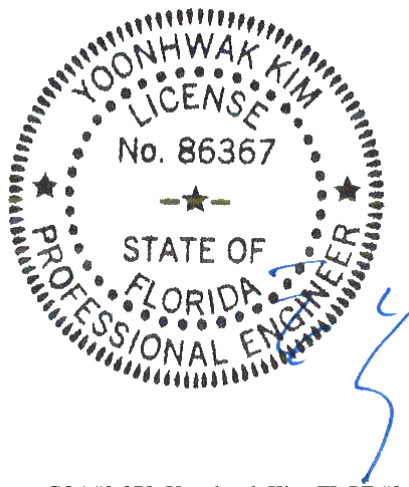
Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

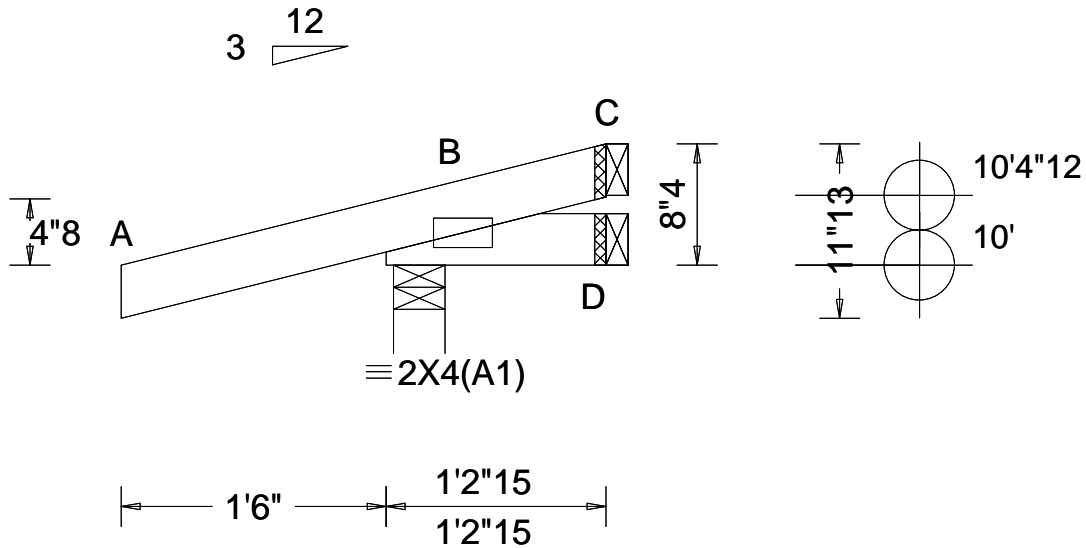
The overall height of this truss excluding overhang is 2-0-8.



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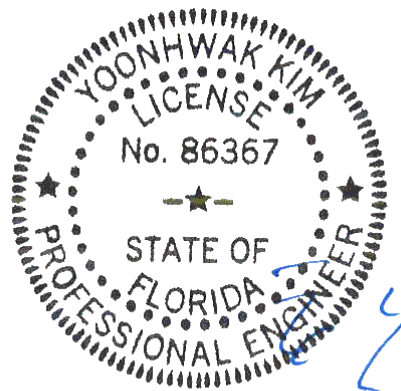
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

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 Wind loading based on both gable and hip roof types.

Additional Notes

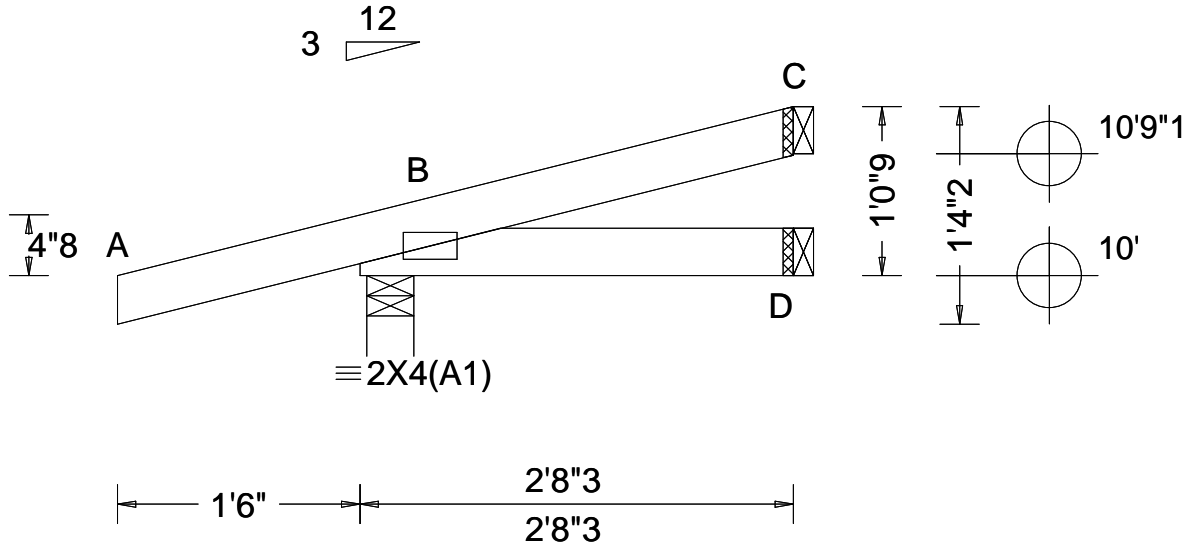
The overall height of this truss excluding overhang is 0-8-4.



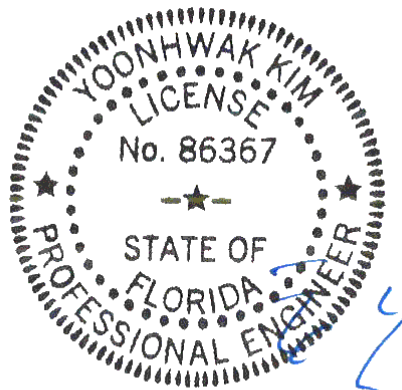
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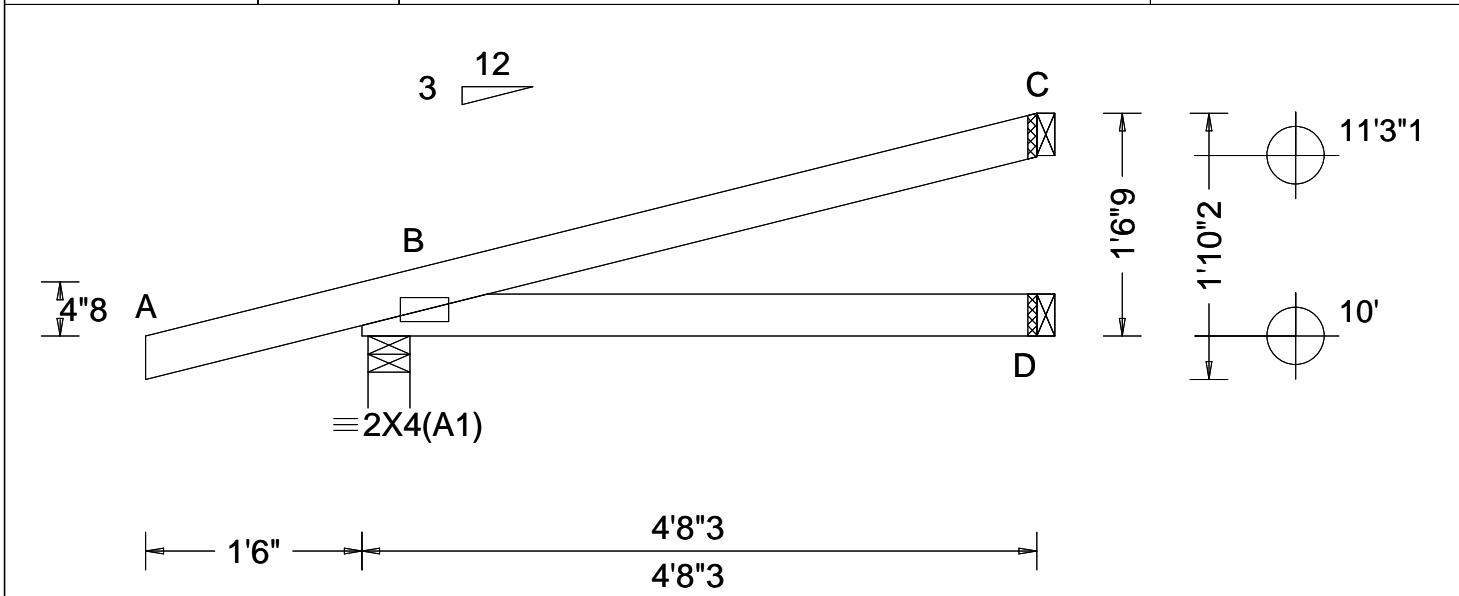


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Lumber

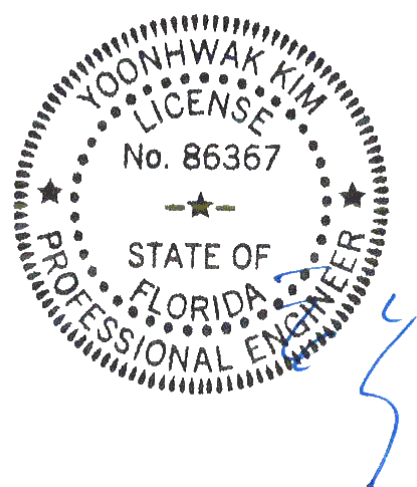
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

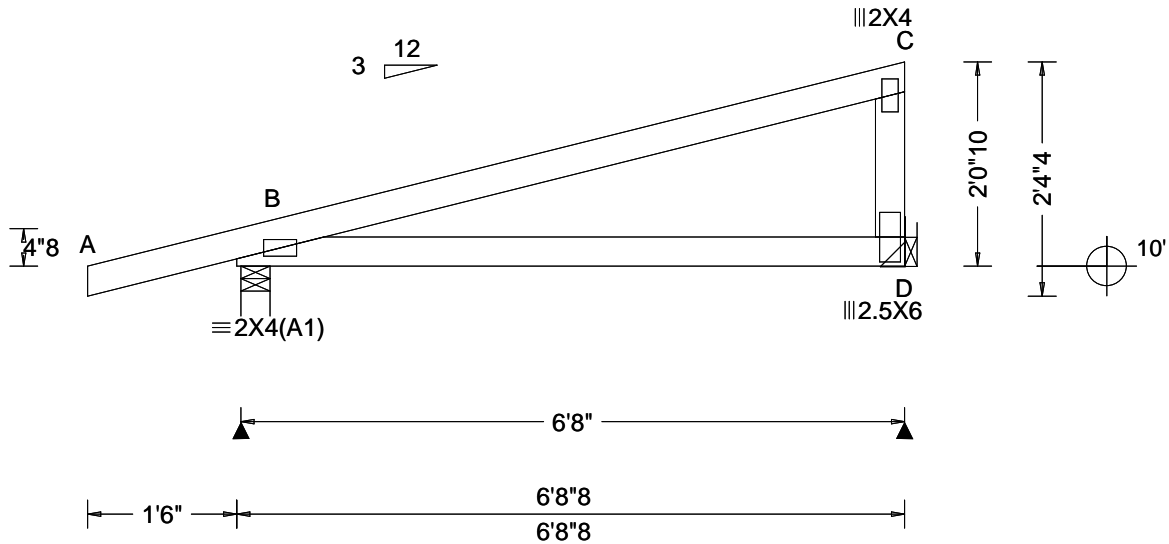
The overall height of this truss excluding overhang is 1-6-9.



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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

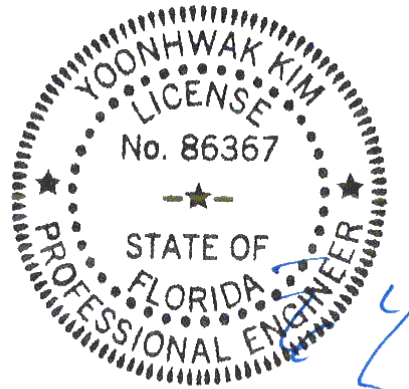
Hanger Support Required, by others
Bearing D (6'5"8, 10') HUS26
Supporting Member: (1)2x6 SP 2400F-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

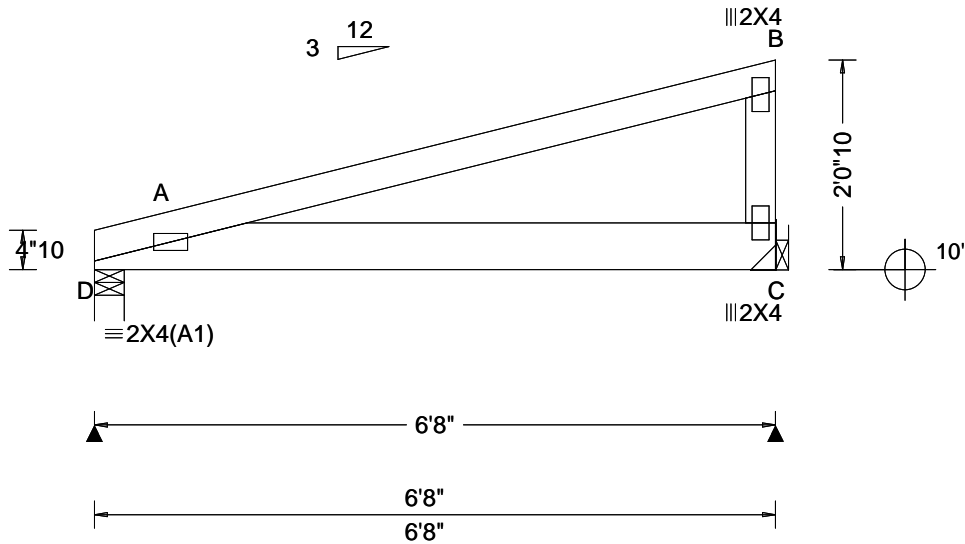
The overall height of this truss excluding overhang is 2-0-10.



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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 61 plf at 0.00 to 61 plf at 6.67
BC: From 10 plf at 0.00 to 10 plf at 6.67
BC: 260 lb Conc. Load at 1.90
BC: 253 lb Conc. Load at 3.90, 5.90

Hangers / Ties

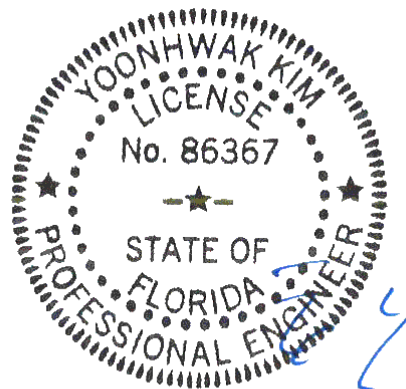
Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

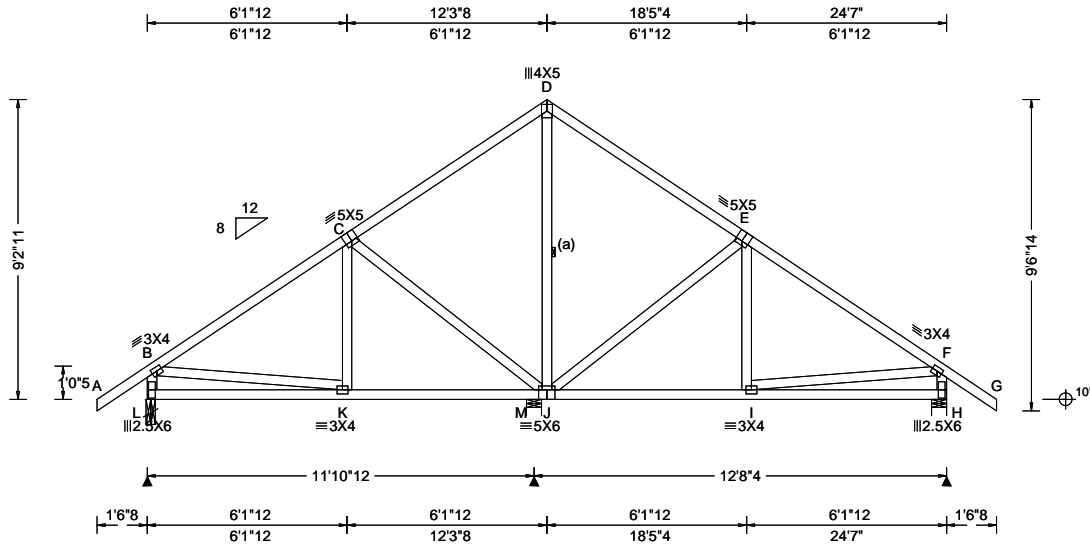
The overall height of this truss excluding overhang is 2-0-10.



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SEQN: 738802 FROM: RFG	SPEC Ply: 1 Qty: 1	Job Number: 26-3717 STECKBECK Truss Label: K1	Cust: R215 JRRef: 1YJ12150001 T5 DrwNo: 112.26.0924.25810 JKS / YK 04/22/2026
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.017 I 999 360 VERT(CL): 0.035 I 999 240 HORZ(LL): 0.007 F - - HORZ(TL): 0.014 F - - Creep Factor: 2.0 Max TC CSI: 0.458 Max BC CSI: 0.497 Max Web CSI: 0.538 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity L 753 /- /- /381 /128 /230 M 749 /- /- /447 /117 /- H 777 /- /- /411 /132 /- Wind reactions based on MWFRS L Brg Wid = 3.0 Min Req = 1.5 (Truss) M Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings L, M, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

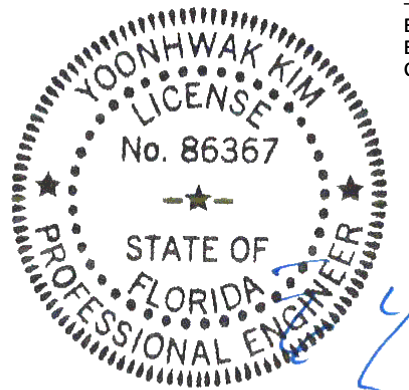
Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-2-11.

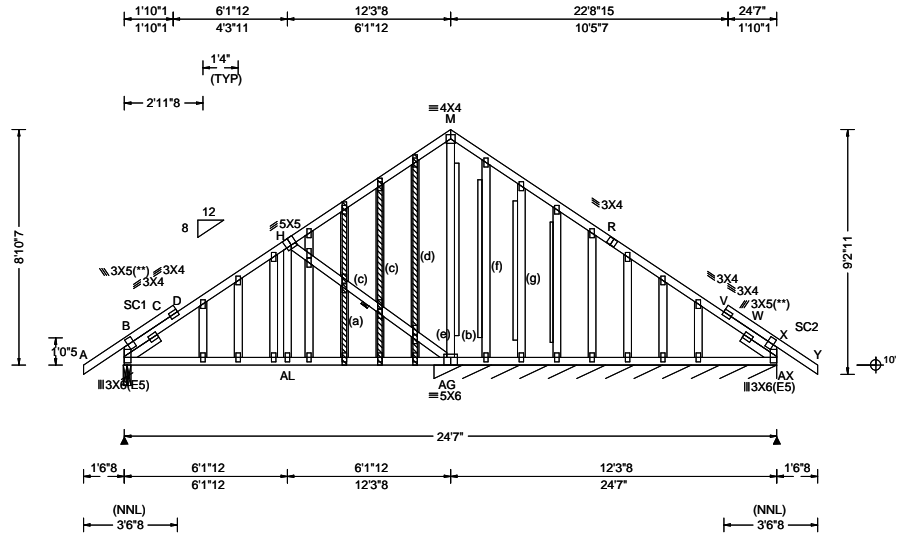
Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
K - J	1054 -230	J - I	571 -17
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
B - L	232 -694	J - E	186 -465
B - K	473 0	I - F	528 0
C - J	177 -423	F - H	240 -732



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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/285.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
B -AL 561 -171 AL-AG 546 -165

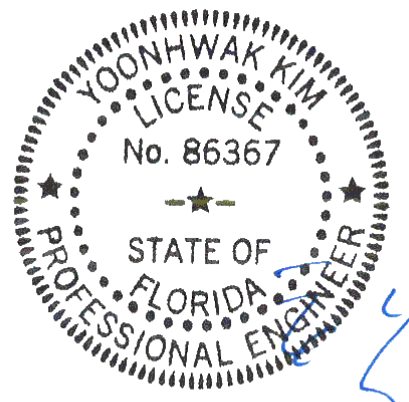
Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
AL- H 384 -181 H -AG 343 -782

Reinforcement
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.



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SEQN: 738807	GABL	Ply: 1	Job Number: 26-3717	Cust: R215 JRef: 1YJ12150001 T2
FROM: RFG		Qty: 1	STECKBECK	DrwNo: 112.26.0924.28423
Page 2 of 2			Truss Label: K1E	JKS / YK 04/22/2026

Gable Reinforcement

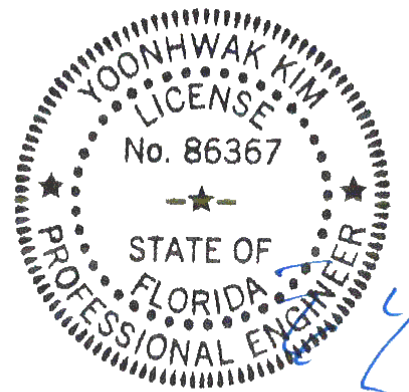
- (b) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (c) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (d) 2x3 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (e) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (f) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (g) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 8-10-7.

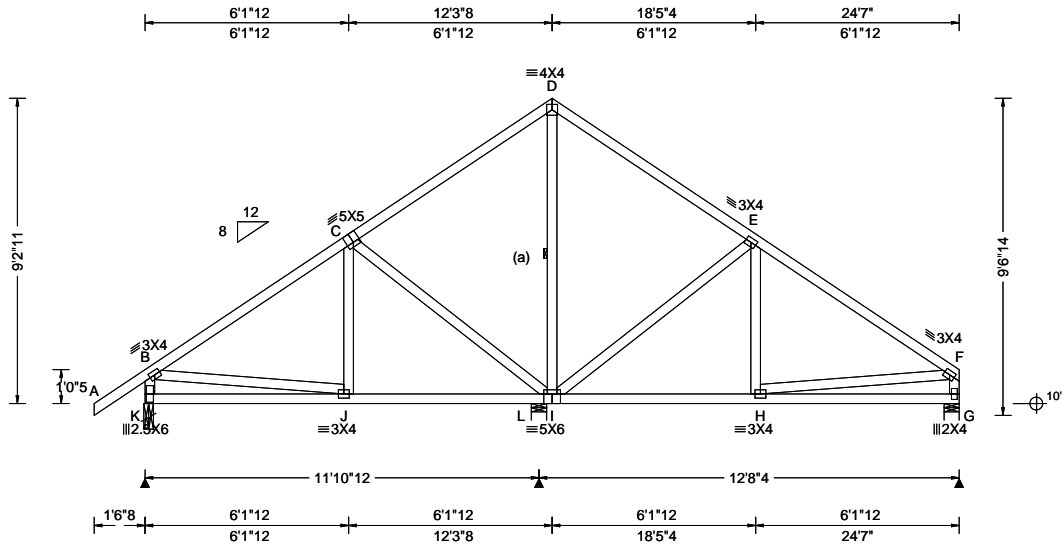


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SEQN: 738805 FROM: RFG	SPEC Ply: 1 Qty: 1	Job Number: 26-3717 STECKBECK Truss Label: K2	Cust: R215 JRRef: 1YJ12150001 T6 DrwNo: 112.26.0924.29987 JKS / YK 04/22/2026
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.026 H 999 360 VERT(CL): 0.035 H 999 240 HORZ(LL): 0.010 F - - HORZ(TL): 0.014 F - - Creep Factor: 2.0 Max TC CSI: 0.514 Max BC CSI: 0.500 Max Web CSI: 0.559 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>K</td> <td>754</td> <td>-</td> <td>-</td> <td>/383</td> <td>/10</td> <td>/247</td> </tr> <tr> <td>L</td> <td>753</td> <td>-</td> <td>-</td> <td>/444</td> <td>/5</td> <td>-</td> </tr> <tr> <td>G</td> <td>665</td> <td>-</td> <td>-</td> <td>/400</td> <td>/5</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	K	754	-	-	/383	/10	/247	L	753	-	-	/444	/5	-	G	665	-	-	/400	/5	-
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D - E	188 -375																																					

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Reinforcement

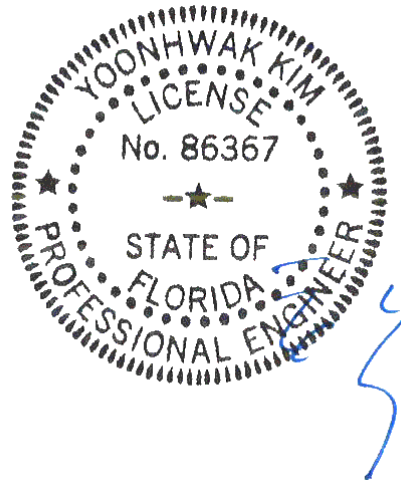
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-2-11.

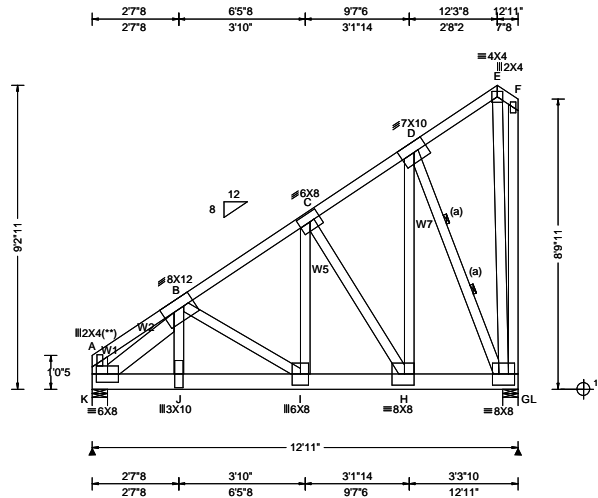


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2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.12 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.075 I 999 360 VERT(CL): 0.150 I 999 240 HORZ(LL): 0.033 B - - HORZ(TL): 0.066 B - - Creep Factor: 2.0 Max TC CSI: 0.384 Max BC CSI: 0.583 Max Web CSI: 0.954 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>K</td> <td>7577</td> <td>-</td> <td>-</td> <td>-</td> <td>322</td> <td>-</td> </tr> <tr> <td>L</td> <td>7269</td> <td>-</td> <td>-</td> <td>-</td> <td>144</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS K Brg Wid = 5.5 Min Req = 3.1 (Truss) L Brg Wid = 5.5 Min Req = 3.0 (Truss) Bearings K & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>100 - 3644</td> <td>C - D</td> <td>48 - 1970</td> </tr> </tbody> </table> </p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	K	7577	-	-	-	322	-	L	7269	-	-	-	144	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	100 - 3644	C - D	48 - 1970
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W1,W2 2x6 SP 2400f-2.0E; W5, W7 2x4 SP #2;

Reinforcement
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 2 Rows @ 3.00" o.c. (Each Row)
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 64 plf at 0.00 to 64 plf at 12.92
 BC: From 10 plf at 0.00 to 10 plf at 10.69
 BC: From 20 plf at 10.69 to 20 plf at 12.92
 BC: 748 lb Conc. Load at 1.73
 BC: 2903 lb Conc. Load at 2.62
 BC: 2554 lb Conc. Load at 4.69, 6.69, 8.69,10.69

Plating Notes
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind
 Wind loads and reactions based on MWFRS.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

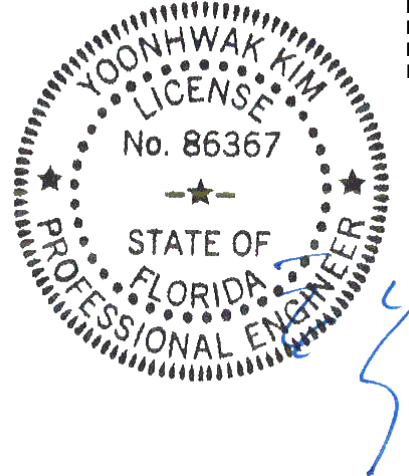
Additional Notes
 The overall height of this truss excluding overhang is 9-2-11.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
K - J	4011 -171	I - H	2881 -71
J - I	4087 -175	H - G	1492 -29

Maximum Web Forces Per Ply (lbs)

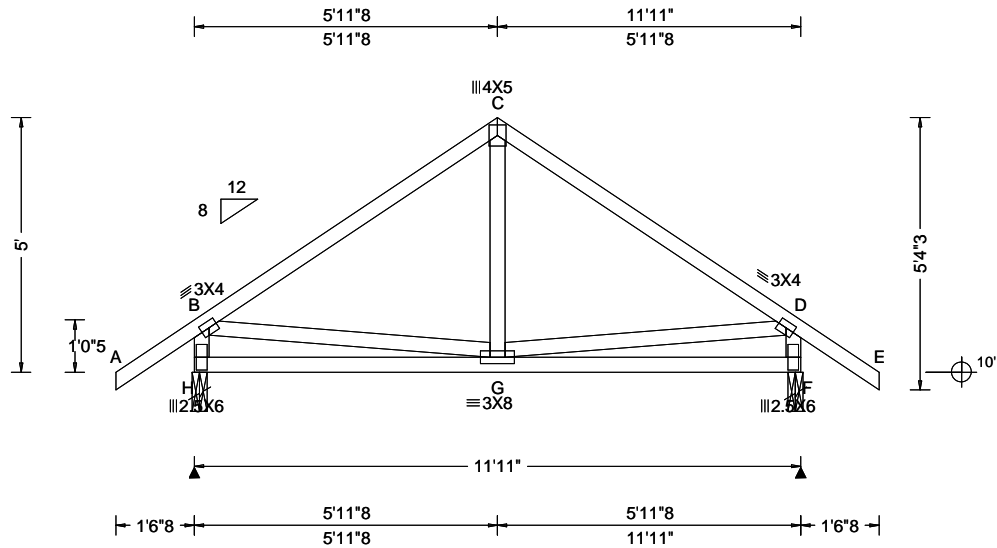
Webs	Tens.Comp.	Webs	Tens. Comp.
K - B	219 -5155	C - H	80 -2477
B - J	2198 -127	H - D	4227 -44
B - I	120 -1271	D - G	67 -3580
I - C	2943 -53		



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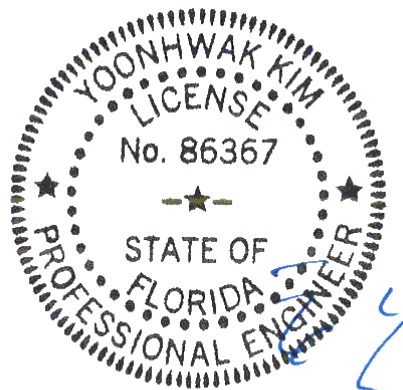


Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.006 G 999 360 VERT(CL): 0.012 G 999 240 HORZ(LL): 0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.395 Max BC CSI: 0.318 Max Web CSI: 0.133 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>607</td> <td>-</td> <td>-</td> <td>/298</td> <td>/8</td> <td>/112</td> </tr> <tr> <td>F</td> <td>607</td> <td>-</td> <td>-</td> <td>/298</td> <td>/8</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	607	-	-	/298	/8	/112	F	607	-	-	/298	/8	-
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

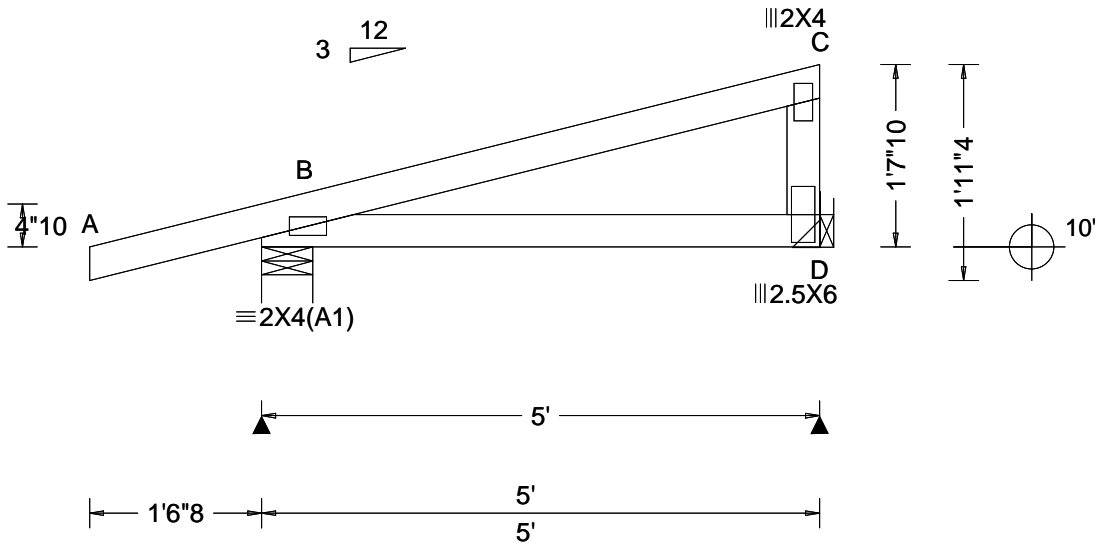
Additional Notes
 The overall height of this truss excluding overhang is 5-0-0.



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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

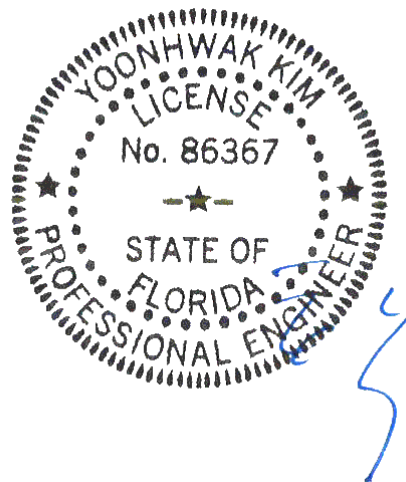
Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-7-10.

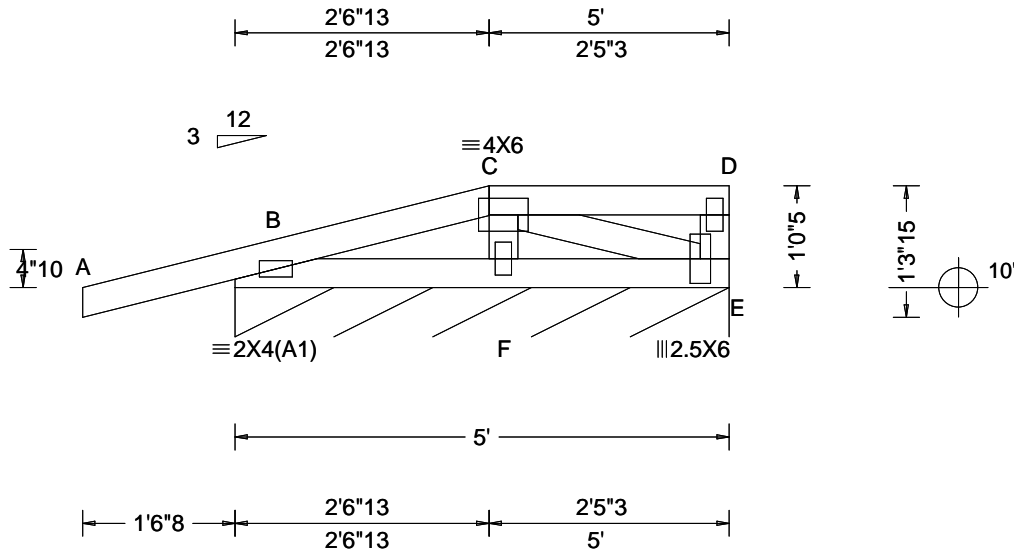


COA#0-278, Yoonhwak Kim, FL PE #86367
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SEQN: 738841 FROM: RFG	GABL Ply: 1 Qty: 1	Job Number: 26-3717 STECKBECK Truss Label: M1E	Cust: R215 JRef: 1YJ12150001 T33 DrwNo: 112.26.0924.33997 JKS / YK 04/22/2026
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.002 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.333 Max BC CSI: 0.049 Max Web CSI: 0.088 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E* 133 /- /- /56 /113 /22 Wind reactions based on MWFRS E Brg Wid = 60.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

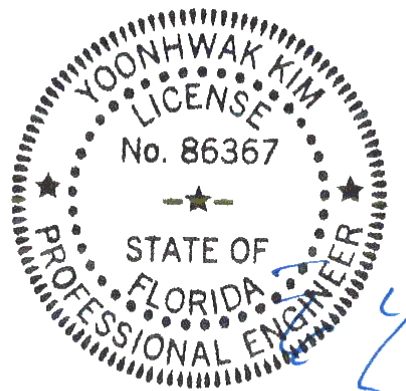
Wind loads based on MWFRS.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

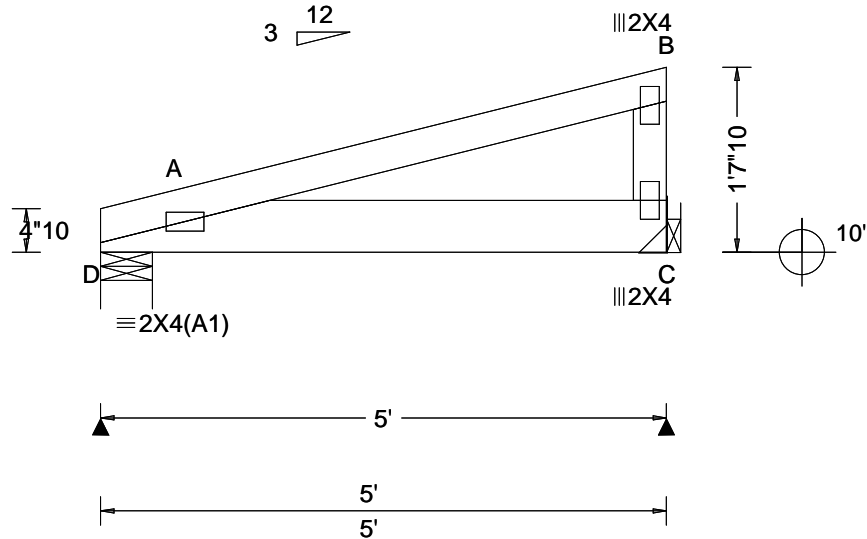
The overall height of this truss excluding overhang is 1-0-5.



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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.017 A - - HORZ(TL): 0.018 A - - Creep Factor: 2.0 Max TC CSI: 0.428 Max BC CSI: 0.417 Max Web CSI: 0.092 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>D</td> <td>574</td> <td>-</td> <td>-</td> <td>-</td> <td>/92</td> <td>-</td> </tr> <tr> <td>C</td> <td>827</td> <td>-</td> <td>-</td> <td>-</td> <td>/132</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	D	574	-	-	-	/92	-	C	827	-	-	-	/132	-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
D	574	-	-	-	/92	-																									
C	827	-	-	-	/132	-																									
Wind reactions based on MWFRS D Brg Wid = 5.5 Min Req = 1.5 (Truss) C Brg Wid = - Min Req = - Bearing D is a rigid surface. Members not listed have forces less than 375#																															

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 61 plf at 0.00 to 61 plf at 5.00
 BC: From 20 plf at 0.00 to 20 plf at 2.23
 BC: From 10 plf at 2.23 to 10 plf at 5.00
 BC: 512 lb Conc. Load at 2.23, 4.23

Hangers / Ties

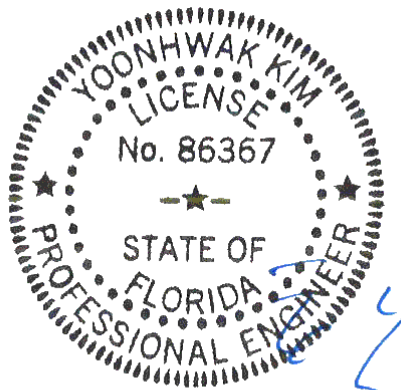
Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

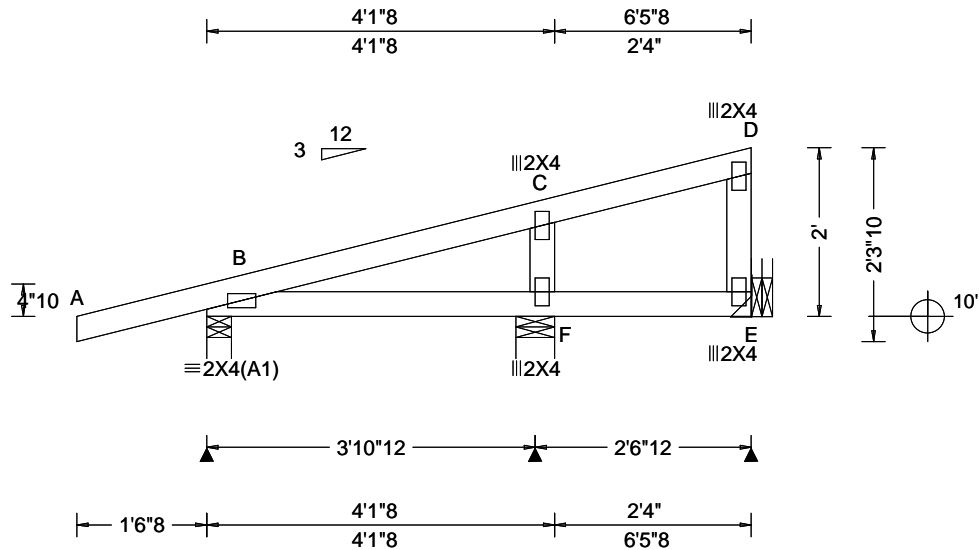
The overall height of this truss excluding overhang is 1-7-10.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 36.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.006 B 999 360 VERT(CL): 0.007 B 999 240 HORZ(LL): -0.002 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.387 Max BC CSI: 0.171 Max Web CSI: 0.126 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 408 /- /- /152 /126 /102 F 423 /- /- /261 /89 /- E 128 /- /- /48 /23 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - F 422 -306
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Hanger Support Required, by others
Bearing E (6'2"8, 10') HUS26
Supporting Member: (2)2x6 SP #2
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.

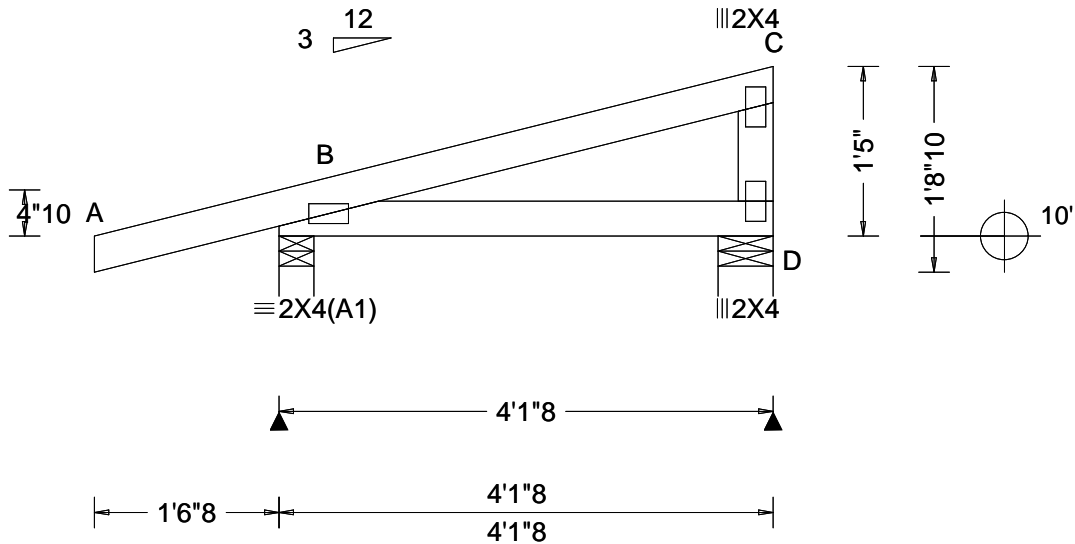
Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 2-0-0.

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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 36.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 B - - HORZ(TL): 0.005 B - - Creep Factor: 2.0 Max TC CSI: 0.392 Max BC CSI: 0.219 Max Web CSI: 0.096 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>443</td> <td>-</td> <td>-</td> <td>/176</td> <td>/134</td> <td>/75</td> </tr> <tr> <td>D</td> <td>206</td> <td>-</td> <td>-</td> <td>/130</td> <td>/43</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	443	-	-	/176	/134	/75	D	206	-	-	/130	/43	-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
B	443	-	-	/176	/134	/75																									
D	206	-	-	/130	/43	-																									
Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375#																															

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins

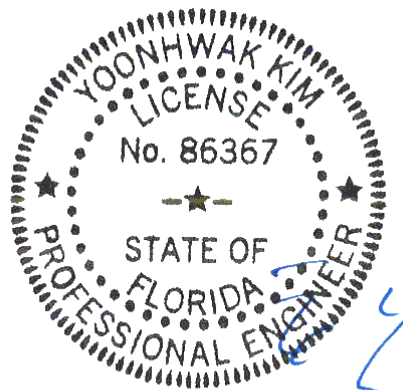
In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

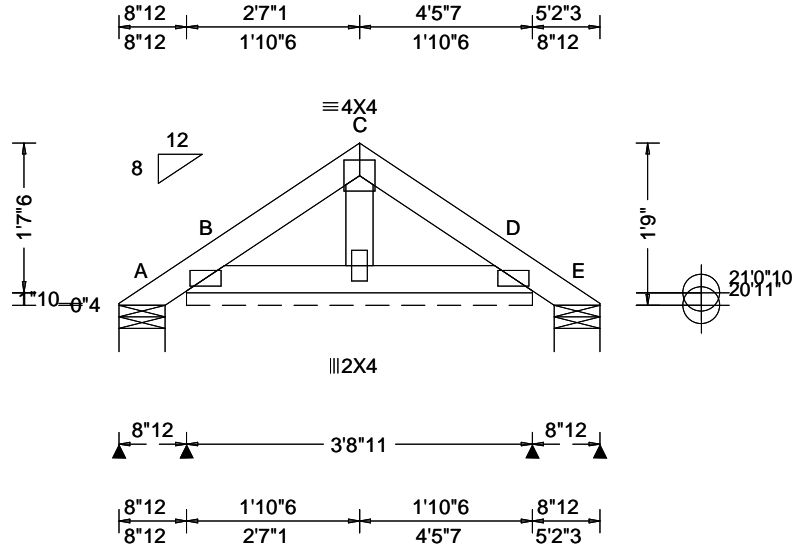
The overall height of this truss excluding overhang is 1'-5-0.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 "	Speed: 130 mph Risk Category: II Enclosure: Enclosed EXP: C Kzt: NA TCDL: 5.0 psf BCDL: 1.2 psf Mean Height: 21.80 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 - - 360 VERT(CL): 0.000 - - 240 HORZ(LL): 0.000 - - - HORZ(TL): 0.000 - - - Creep Factor: 2.0 Max TC CSI: 0.032 Max BC CSI: 0.015 Max Web CSI: 0.007 VIEW Ver: 25.02.01C.0416.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 5 /- /- /18 /17 /30 B* 55 /- /- /39 /25 /- E 5 /- /- /1 /2 /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 44.7 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

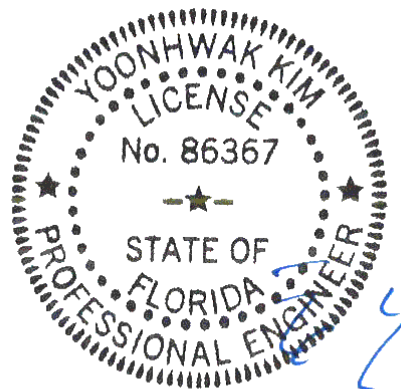
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

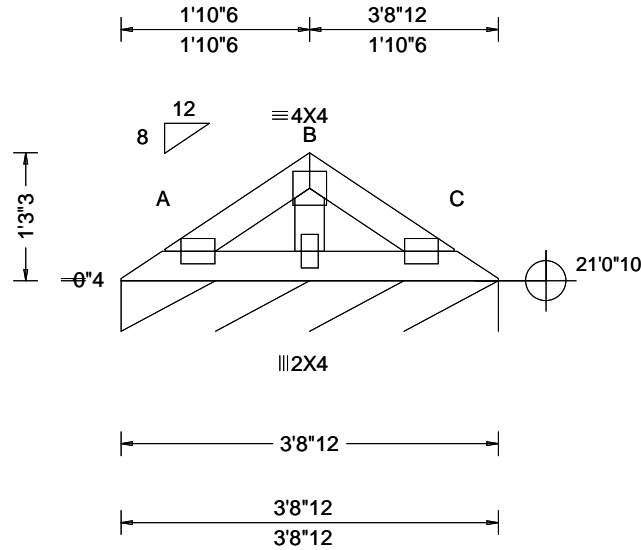
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Refer to DWG PB160220723 for piggyback details.



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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4(D1) except as noted.

Wind

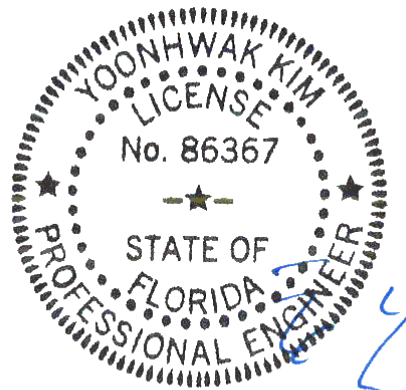
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-4-13.

Additional Notes

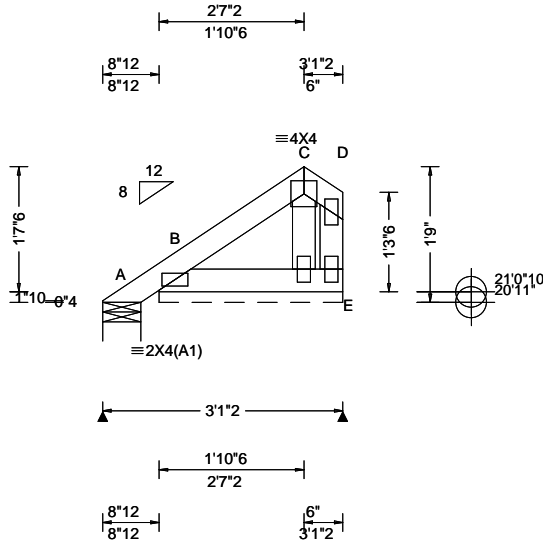
See DWGS VALTN220723 and VAL180220723 for valley details.



COA#0-278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 16.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 - - 360 VERT(CL): 0.000 - - 240 HORZ(LL): 0.000 - - - HORZ(TL): 0.000 - - - Creep Factor: 2.0 Max TC CSI: 0.032 Max BC CSI: 0.027 Max Web CSI: 0.011 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 5 /- /- /27 /15 /46 B* 80 /- /- /92 /30 /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 28.4 Min Req = - Bearings A & B are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

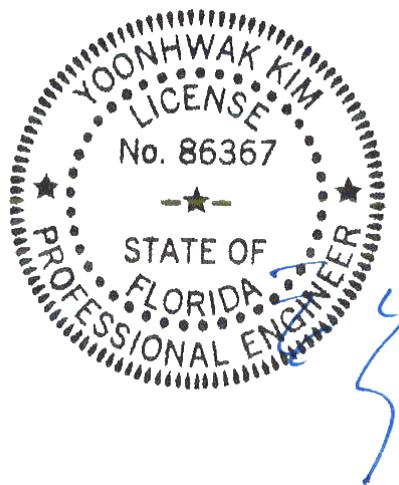
All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

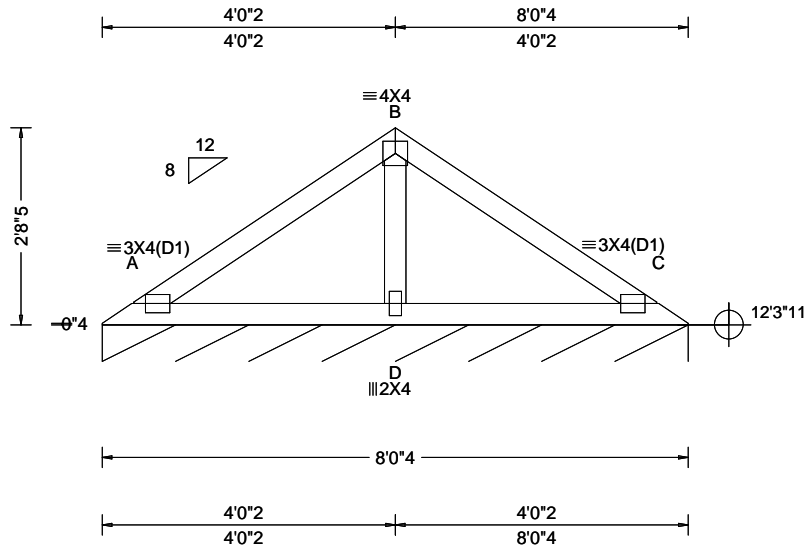
The overall height of this truss excluding overhang is 1-9-0.
Refer to DWG PB160220723 for piggyback details.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 A 999 360 VERT(CL): 0.013 A 999 240 HORZ(LL): -0.003 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.177 Max Web CSI: 0.085 VIEW Ver: 24.02.00D.0114.10	Gravity Loc R+ / R- / Rh / Rw / U / RL C* 84 /- /- /43 /- /8 Non-Gravity Wind reactions based on MWFRS C Brg Wid = 96.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 286 -384

Lumber

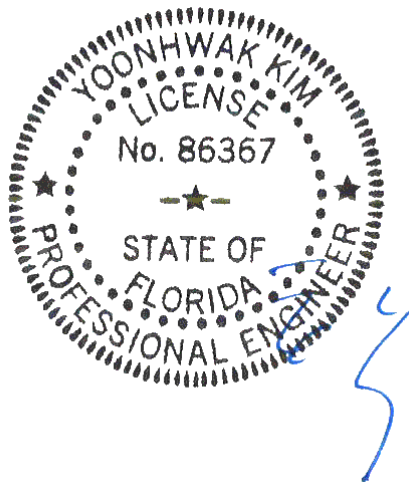
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

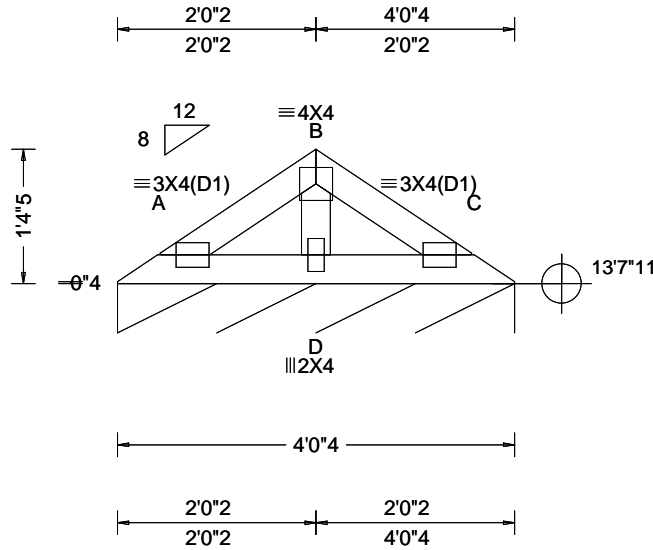
See DWGS VALTN220723 and VAL180220723 for valley details.
The overall height of this truss excluding overhang is 2-8-5.



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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Enclosed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 360 VERT(CL): 0.001 A 999 240 HORZ(LL): -0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.039 Max BC CSI: 0.031 Max Web CSI: 0.031 VIEW Ver: 24.02.00D.0114.10	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>83</td> <td>/-</td> <td>/-</td> <td>/40</td> <td>/-</td> <td>/7</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 48.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#	Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL	C*	83	/-	/-	/40	/-	/7
Gravity			Non-Gravity																					
Loc	R+	/R-	/Rh	/Rw	/U	/RL																		
C*	83	/-	/-	/40	/-	/7																		

Lumber

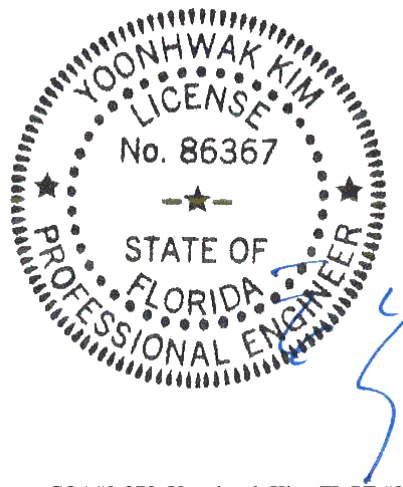
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.
 The overall height of this truss excluding overhang is 1-4-5.



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CLR Reinforcing Member Substitution

This detail is to be used for ANSI/TPI 1-2014 standards and older when a Continuous Lateral Restraint (CLR) is specified on a truss design and an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

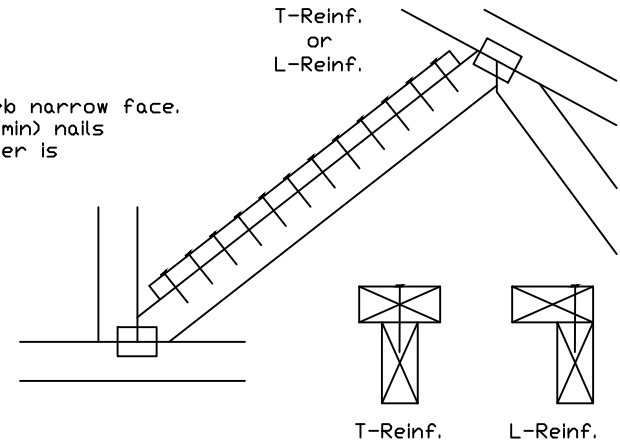
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

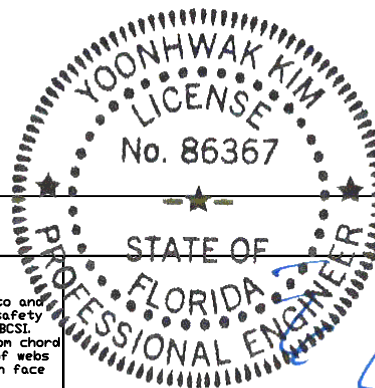
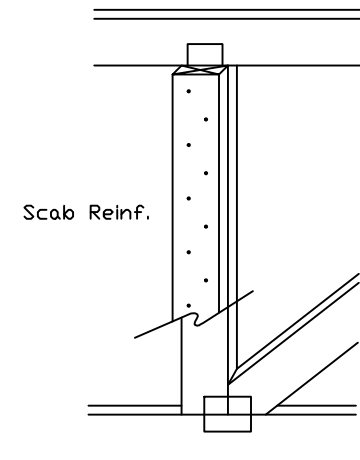
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.131"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.131"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Rev: 01/23/26



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-22 180 mph, 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-22 160 mph, 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

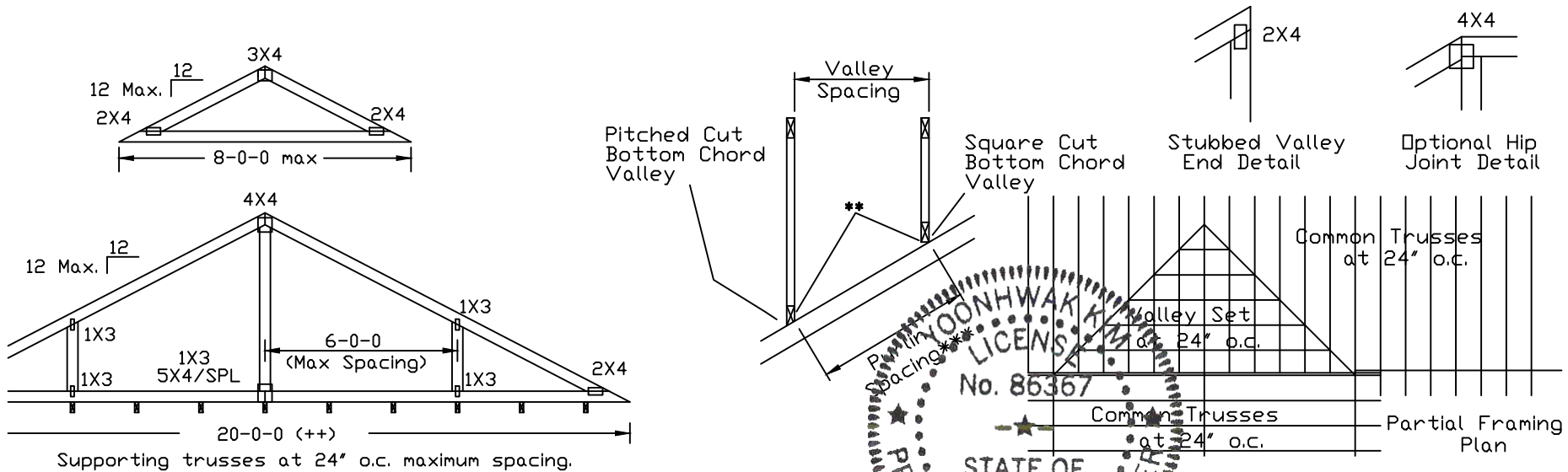
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.131" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

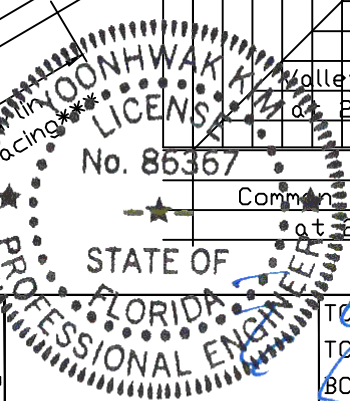
++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Rev: 01/23/26



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TC LL	30	30	40PSF
TC DL	20	15	7PSF
BC DL	10	10	10 PSF
BC LL	0	0	0PSF
TOT. LD.	60	55	57PSF
DUR.FAC.	1.25/1.33	1.15	1.15
SPACING			24.0"

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VAL180220723

04/23/2026
 COA#0-278, Yoonhwak Kim, No. 86367
 Florida Certificate of Product Approval #PL 1909

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

**** Attach each valley to every supporting truss with:**
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 140 mph for SP (G = 0.55, min.),
 125 mph for DF-L (G = 0.50, min.), or
 105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

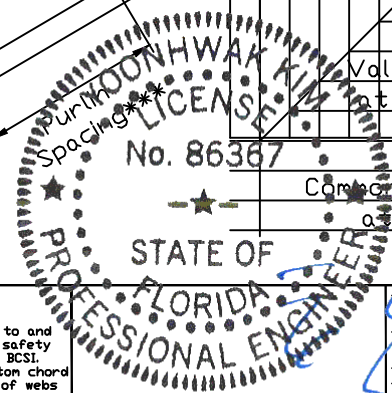
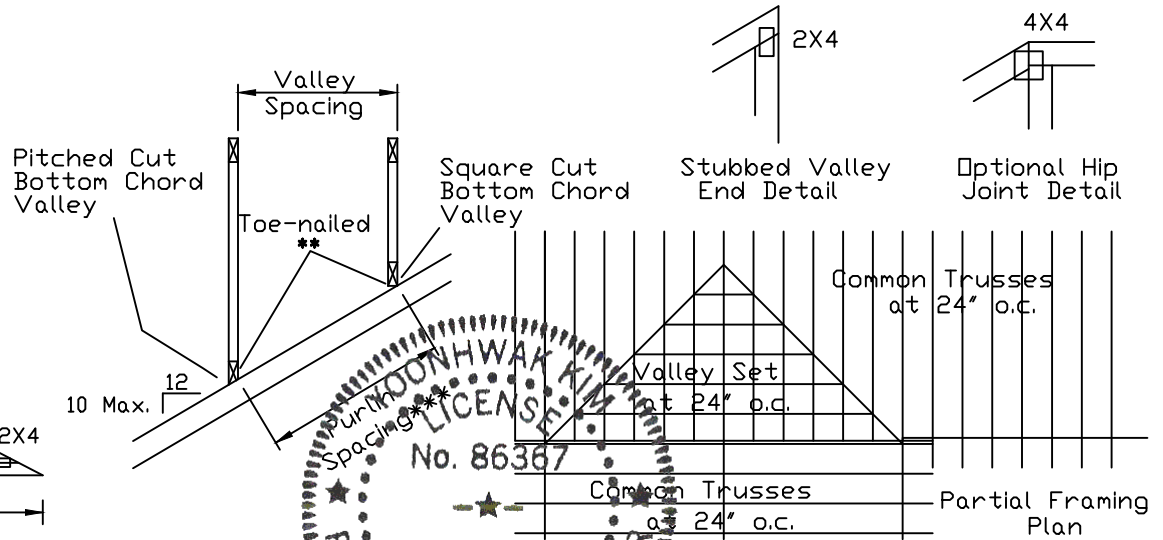
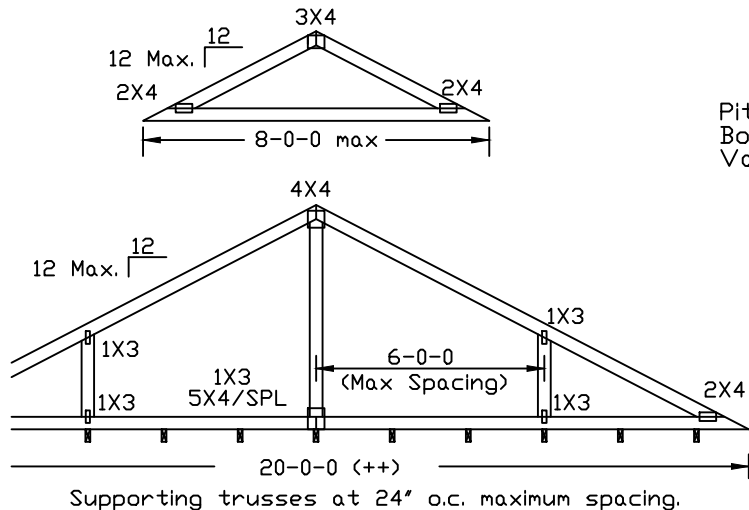
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Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

***** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.**

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

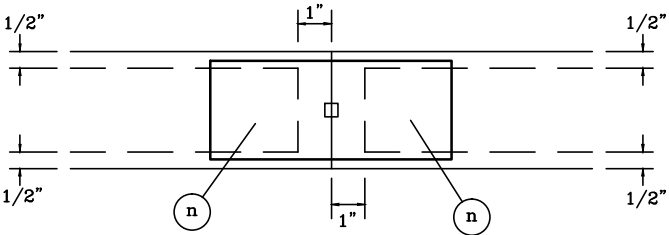
TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	07/03/2023
BC DL	10	10	10 PSF	DRWG	VALTN220723
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING			24.0"		

04/23/2026
 COA#0-278, Yoonhwak Kim, FL PE #86387
 Florida Certificate of Product Approval #PL 1999

Rev: 01/23/26

TRULOX INFORMATION DETAIL

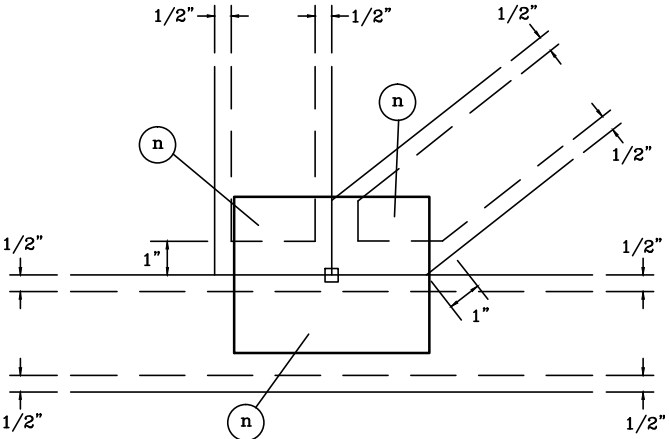
TYPICAL OFF PANEL SPLICE



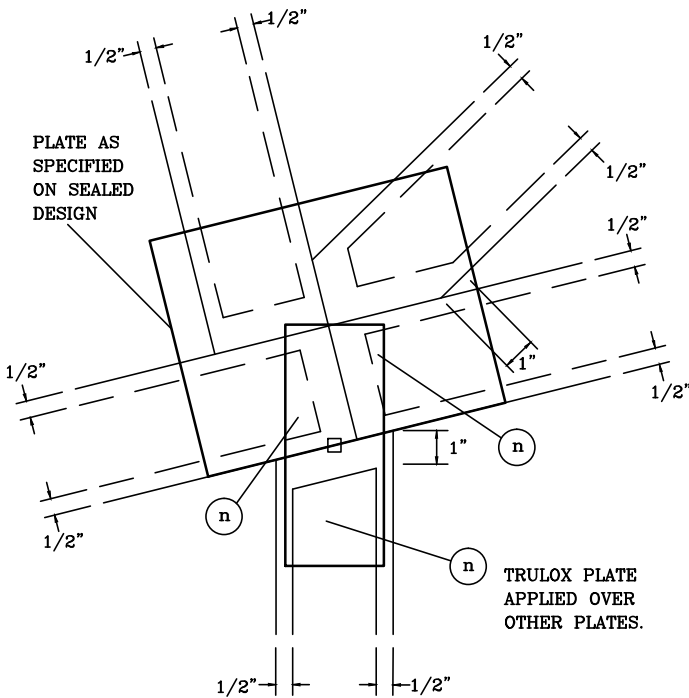
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

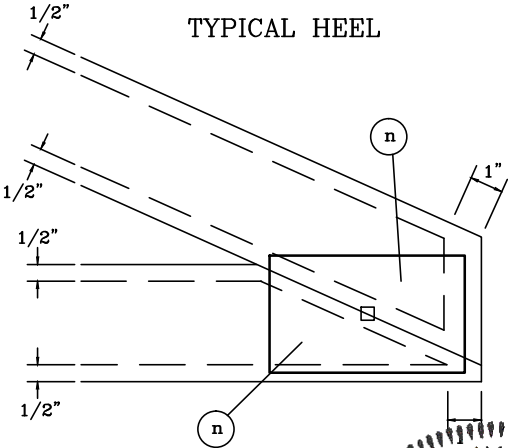
TYPICAL PANEL POINT WITHOUT SPLICE



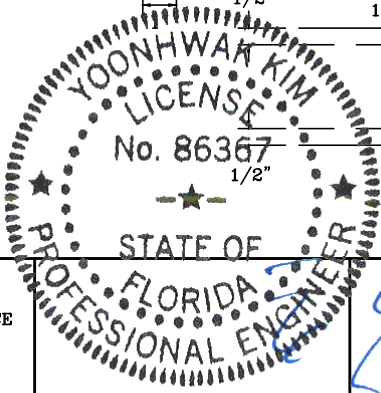
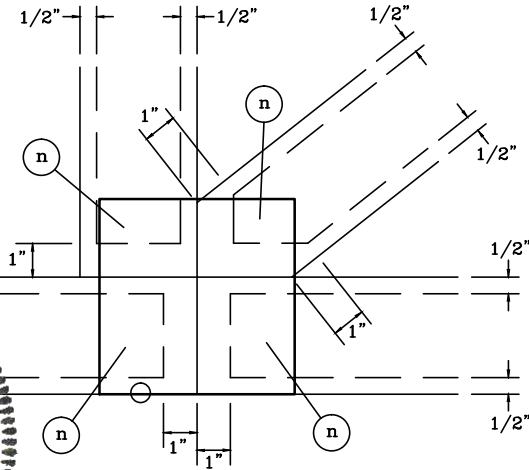
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

NOTES:
(n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
○ LOCATES PLATE CORNER OR FLUSH EDGE.
□ LOCATES PLATE CENTER.

TRULOX PLATING

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TL

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DATE 10/01/14