

STEEL ROOF DECK SHALL BE A MINIMUM YIELD STRENGTH OF 33 KSI, UNLESS NOTED OTHERWISE. ALL INTERIOR STEEL ROOF DECK SHALL CONFORM TO ASTM A1008; FACTORY PRIMED FOR PAINT. ALL EXPOSED STEEL ROOF DECK SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR EXTENDED NOTES.

STEEL ROOF DECK SHALL BE ATTACHED TO THE SUPPORTS WITH 5/8 INCH DIAMETER WELD BOLDS AND TO COLD-FORMED METAL FRAMING WITH #12 SELF-DRILLING SCREWS UNLESS NOTED OTHERWISE. WHEN DECK THICKNESS IS LESS THAN 0.2 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS SPECIFIED IN THE DECK ATTACHMENT SCHEDULE.

WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD DECK TO STEEL WITH SAME WELDS AS REQUIRED FOR SIDE BOUNDARIES.

ERECT STEEL DECK CLOSURES AND OTHER LIGHT GAGE MATERIAL REQUIRED TO PRODUCE A COMPLETED INSTALLATION.

FLAT, RIDGE, AND VALLEY PLATES:

UNLESS NOTED OTHERWISE, CONTRACTOR SHALL PROVIDE FLAT PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF DECK CHANGES DIRECTION AND RIDGE OR VALLEY PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF SLOPE EXCEEDS 1/4 INCH PER FOOT.

DO NOT HANG CEILINGS, DUCTS, LIGHT FIXTURES, EQUIPMENT, OR OTHER ITEMS FROM THE ROOF DECK WITHOUT PRIOR APPROVAL FROM THE DECK SUPPLIER AND REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.

SEE ARCHITECTURAL DRAWINGS FOR FINAL ROOF SLOPES. WHERE STRUCTURAL FRAMING DOES NOT CREATE THE SPECIFIED ROOF SLOPE, IT SHALL BE CREATED WITH RIGID INSULATION ABOVE THE DECK.

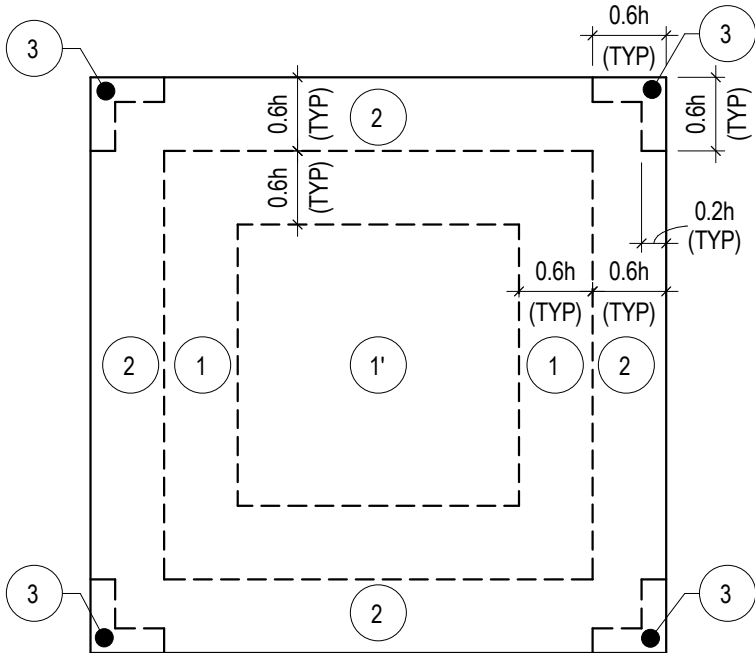
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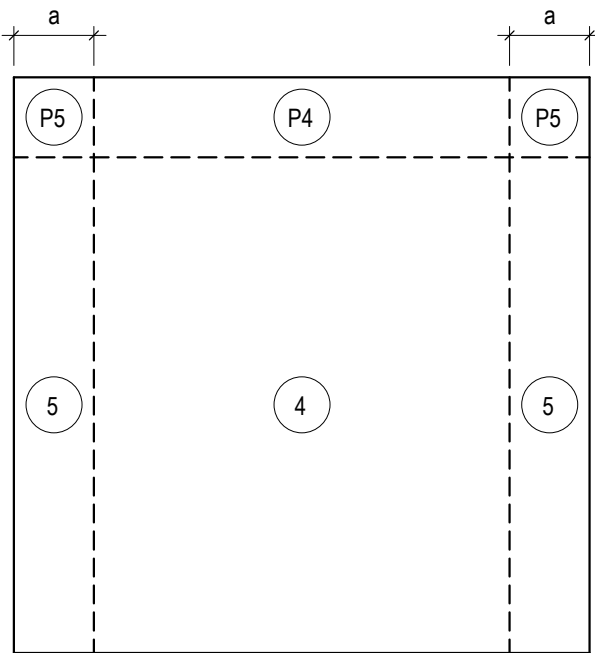
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ROOF PLAN



WALL ELEVATION

CONCRETE MIXTURE REQUIREMENTS								
APPLICATION	EXPOSURE CLASS	f _c	TEST AGE	MODULUS OF ELASTICITY	MAXIMUM W/CM	AIR CONTENT	NOMINAL MAXIMUM AGGREGATE	MAXIMUM CONCRETE WEIGHT
FOOTINGS	F0	3000 PSI	28 DAYS	3122 KSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
EXTERIOR SLAB-ON-GROUND	F1	4500 PSI	28 DAYS	3824 KSI	0.55	4.5% ± 1.5%	1"	150 PCF
SLAB-ON-GROUND	F0	3000 PSI	28 DAYS	3122 KSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
ELEVATED SLABS ON METAL DECK	F0	4000 PSI	28 DAYS	3605 KSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
TILT-WALLS	F0	4000 PSI (MIN)	28 DAYS	3605 KSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
NOTES: 1. EXPOSURE CATEGORIES AND CLASSES FOR SULFATES, PERMEABILITY, AND CORROSION PROTECTION OF REINFORCEMENT IS CLASS ZERO UNLESS NOTED OTHERWISE. 2. WATER/CEMENT RATIO SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX DESIGN. THERE IS NO MAXIMUM WATER/CEMENT RATIO REQUIREMENT FOR THE EXPOSURE CLASSIFICATION ASSOCIATED WITH THIS APPLICATION. MAXIMUM WATER/CEMENT RATIO IS NOT APPLICABLE FOR DURABILITY REQUIREMENTS IN LIGHTWEIGHT CONCRETE. 3. THERE IS NO MANDATORY TARGET AIR CONTENT FOR THIS APPLICATION. THE CONTRACTOR MAY CHOOSE TO ADD AIR ENTRAINMENT TO IMPROVE THE WORKABILITY AND FINISHING PROPERTIES OF THE MIX. AIR CONTENT SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX.								

WALL REINFORCEMENT - HORIZONTAL BAR LAP SPlice LENGTH SCHEDULE (INCHES)					
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)			COMPRESSION (LCS)
		f _c = 4,000 PSI	f _c = 5,000 PSI	f _c = 6,000 PSI	
#4	1,500	33	30	27	15
#5	1,875	40	36	34	19
#6	2,250	48	44	40	23
#7	2,625	105	96	87	27
#8	3,000	121	108	99	30
#9	3,375	137	122	112	34
#10	3,875	153	138	125	39
#11	4,250	170	152	139	43

WALL REINFORCEMENT - VERTICAL BAR LAP SPlice LENGTH SCHEDULE (INCHES)					
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)			COMPRESSION (LCS)
		f _c = 4000 PSI	f _c = 5000 PSI	f _c = 6000 PSI	
#4	2,000	25	23	21	15
#5	2,125	31	28	26	19
#6	2,250	37	34	31	23
#7	2,375	54	49	45	27
#8	2,500	62	56	51	30
#9	2,875	70	63	57	34
#10	3,250	118	106	96	39
#11	3,625	131	117	107	43

NOTES:

- FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
- DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.
- 0.6h = 15'-0". SEE WALL ELEVATION MAP BELOW FOR LOCATION OF ROOF ZONES.
- a = 9'-7". SEE WALL ELEVATION MAP BELOW FOR LOCATION OF WALL ZONES.
- POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE. NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
- EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
- PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
 - POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
 - POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - PA/S SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.
- A DESIGN WIND PRESSURE HORIZONTAL VALUE OF 193 PSF AND VERTICAL VALUE OF 153 PSF SHALL BE APPLIED TO COMPONENTS WHICH ARE EITHER ROOFTOP STRUCTURES OR ROOFTOP APPURTENANCES AND THEIR CONNECTION. EXAMPLES OF THIS ARE RTUS, AHUS, AND SCREEN WALLS.
- ROH# : DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.
- ALL DOORS TO BE RATED TO RESIST DESIGN WIND PRESSURES SPECIFIED.

CAST-IN-PLACE CONCRETE (NON-PRESTRESSED) CLEAR COVER SCHEDULE			
APPLICATION	BOTTOM	TOP	SIDES
FOUNDATIONS	3"	2"	3"
SLAB-ON-GROUND	SEE DETAILS	SEE DETAILS	3"
RETAINING WALLS	N/A	N/A	2"
WALLS	N/A	N/A	1/2" (2")
COLUMNS	N/A	N/A	1 1/2" (2")
ELEVATED SLABS	3/4" (2")	1/2" (2")	1/2" (2")
BEAMS	1 1/2" (2")	1 1/2" (2")	1 1/2" (2")

LAP SPlice AND DEVELOPMENT LENGTH SCHEDULE																								
BAR SIZE	MINIMUM BAR SPACING (NOTE 3)		LAP SPlice LENGTHS										DEVELOPMENT LENGTHS											
			f_c 28-DAYS (PSI)		3,000		4,000		5,000		6,000		STRAIGHT (L_d)		L_{dh}		HOOKED (L_{ch})							
	STRAIGHT	HOOKED	BAR LOCATION	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	3,000	4,000	5,000	6,000	
				TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	3,000	4,000	5,000	6,000	
#3	1,375	2,250			26	21	24	19	21	16	19	15	20	16	18	14	16	12	14	11	6	6	6	6
#4	1,500	1,375			36	28	30	24	28	21	25	20	27	21	23	18	21	16	19	15	8	7	7	7
#5	1,625	3,750			43	34	38	29	34	26	32	24	33	26	29	22	26	20	24	18	10	9	9	9
#6	1,750	4,500			52	41	46	36	41	32	37	29	40	21	36	27	31	24	28	22	13	13	12	12
#7	1,875	5,250			65	50	52	43	73	39	67	36	50	38	63	33	56	30	51	27	17	16	15	15
#8	2,000	6,000			94	73	93	63	84	56	76	51	72	56	71	48	64	43	58	39	20	19	18	18
#9	2,375	6,768			107	82	104	72	94	64	86	59	82	63	80	55	72	49	66	45	24	23	22	22
#10	2,625	7,625			121	94	119	81	106	73	97	67	93	72	91	62	81	56	74	51	28	27	26	26
#11	2,875	8,500			136	104	130	91	117	81	107	75	104	80	100	70	90	62	82	57	34	32	31	30
NOTES																								
1. VALUES LISTED CORRELATE TO AN F_c VALUE OF 60 KSI. LENGTHS ARE IN INCHES. REQUEST ALTERNATE SCHEDULE FOR DIFFERENT REBAR STRENGTHS.																								
2. WHERE BARS OF DIFFERENT SIZES ARE LAP SPliced, THE LAP SPlice LENGTH SHALL BE THE LARGER OF THE DEVELOPMENT LENGTH OF THE LARGER BAR AND THE LAP SPlice OF THE SMALLER BAR.																								
3. VALUES LISTED ASSUME MINIMUM SPACING AS SHOWN. REQUEST ALTERNATE VALUES FOR BARS WITH TIGHTER SPACING.																								
4. IF APPLICABLE, MODIFY VALUES LISTED AS FOLLOWS:																								
A. LIGHT WEIGHT CONCRETE = MULTIPLY ALL VALUES BY 0.75																								
B. EPOXY COATED REBAR = MULTIPLY L_d & LAP SPlice LENGTH VALUES BY 1.5 & L_{ch} VALUES BY 1.2																								

ABBREVIATIONS		ABBREVIATIONS	
ADDL	ADDITIONAL	KSI	KIPS PER SQUARE INCH
ADJ	ADJACENT	L	LENGTH
AFF	ABOVE FINISHED FLOOR	LB(S)	POUND(S)
ALT	ALTERNATE	LL	LIVE LOAD
APPROX	APPROXIMATE	LLH	LONG LEG HORIZONTAL
ARCH	ARCHITECT OR ARCHITECTURAL	LLV	LONG LEG VERTICAL
ASD	ALLOWABLE STRESS DESIGN	LONG	LONGITUDINAL
B/	BOTTOM OF	LRFD	LOAD RESISTANCE FACTORED DESIGN
B/B	BACK-TO-BACK	LSH	LONG SIDE HORIZONTAL
BLDG	BUILDING	LSV	LONG SIDE VERTICAL
BLKG	BLOCKING	LTS	LAP TENSION SPlice
BP	BASE PLATE	LW	LIGHT WEIGHT
BRG	BEARING	LWC	LIGHT WEIGHT CONCRETE
BOT	BOTTOM	M	MOMENT
BTWN	BETWEEN	MAX	MAXIMUM
C	COMPRESSION	MC	MOMENT CONNECTION(S)
CFS	COLD-FORMED STEEL	MECH	MECHANICAL
CIP	CAST-IN-PLACE	MEP	MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION
CJ	CONTROL JOINT	MFR	MANUFACTURER
CJP	COMPLETE JOINT PENETRATION	MID	MIDDLE
CL	CENTER LINE	MIN	MINIMUM
CLR	CLEAR OR CLEARANCE	MISC	MISCELLANEOUS
CMU	CONCRETE MASONRY UNIT	NIC	NOT IN CONTRACT
COL	COLUMN	NS	NEAR SIDE
CONC	CONCRETE	NTS	NOT TO SCALE
CONN(S)	CONNECTION(S)	NWC	NORMAL WEIGHT CONCRETE
CONST	CONSTRUCTION	OC	ON CENTER
CONT	CONTINUOUS	OD	OUTSIDE DIAMETER
COORD	COORDINATE	OF	OUTSIDE FACE
D&E	DRILL & EPOXY	OH	OPPOSITE HAND
Ø	REINFORCING BAR DIAMETER	OPNG(S)	OPENING(S)
DBA	DEFORMED BAR ANCHOR	OPP	OPPOSITE
DOW	DEMAND CRITICAL WELD	OSL	OUTSTANDING LEG
°	DEGREE(S)	PAF	POWDER ACTUATED FASTENER
Ø	DIAMETER	PERP	PERPENDICULAR
DIAG	DIAGONAL	PJF	PREFORMED JOINT FILLER
DIM(S)	DIMENSIONS(S)	PJP	PARTIAL JOINT PENETRATION
DL	DEAD LOAD	PL	PLATE
DWG(S)	DRAWING(S)	PLF	POUNDS PER LINEAL FOOT
EA	EACH	PCST	PRECAST
EF	EACH FACE	PREFAB	PRE-FABRICATED
EJ	EXPANSION JOINT	PSF	POUNDS PER SQUARE FOOT
EL	ELEVATION	PSI	POUNDS PER SQUARE INCH
ELEV	ELEVATOR	PT	POST-TENSIONED
EOS	EDGE-OF-SLAB	REF	REFERENCE
EQ	EQUAL	REINF	REINFORCE(D) (ING) OR (MENT)
EQUIP	EQUIPMENT	REQ(D)	REQUIRE(D)
EW	EACH WAY	REV	REVISION
EXIST	EXISTING	RTU	ROOF TOP UNIT
EXP	EXPANSION	SCHED	SCHEDULE(D)
EXT	EXTERIOR	SDL	SUPERIMPOSED DEAD LOAD
F/F	FACE-TO-FACE	SE	STRUCTURAL ENGINEER OF RECORD
FD	FLOOR DRAIN	SF	SQUARE FOOT (FEET)
FF	FINISH FLOOR	SIM	SIMILAR
FND	FOUNDATION	SLRS	SEISMIC LOAD RESISTING SYSTEM
FS	FAR SIDE	SOG	SLAB-ON-GROUND
FT	FEET	SP	SPACE
FTG	FOOTING	SPEC(S)	SPECIFICATIONS(S)
GA	GAGE, GAUGE	SS	STAINLESS STEEL
GALV	GALVANIZED	STD	STANDARD
GB	GRADE BEAM	STIFF	STIFFENER
GC	GENERAL CONTRACTOR	STR	STRUCTURE OR STRUCTURAL
GDR	GIRDER	SYM	SYMMETRICAL
GEN	GENERAL	T	TENSION
GYP	GYPSON	T&B	TOP AND BOTTOM
HCA	HEADED CONCRETE ANCHORS	T&G	TONGUE & GROOVE
HORIZ	HORIZONTAL	T/	TOP OF
HSS	HOLLOW STRUCTURAL SECTION	TEMP	TEMPERATURE OR TEMPORARY
ID	INSIDE DIAMETER	TYP	TYPICAL
IF	INSIDE FACE	UNO	UNLESS NOTED OTHERWISE
IN	INCH	V	SHEAR
INFO	INFORMATION	VERT	VERTICAL
INT	INTERIOR	VIF	VERIFY IN FIELD
ICC-ES	INTERNATION CODE COUNCIL - EVALUATION SERVICE	W/	WITH
JST(S)	JOIST(S)	WO	WITHOUT
K	KIPS (1,000 POUNDS)	WP	WORK POINT
KLF	KIP PER LINEAL FOOT	WWR	WELDED WIRE REINFORCEMENT
KSF	KIP PER SQUARE FOOT		



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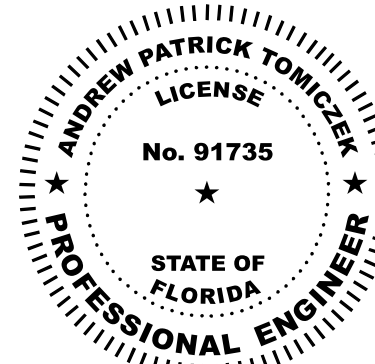
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COLUMBIA COUNTY, FLORIDA



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PROJECT NUMBER
C09414.003

DATE
03/13/2026

TITLE

GENERAL
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e-S101

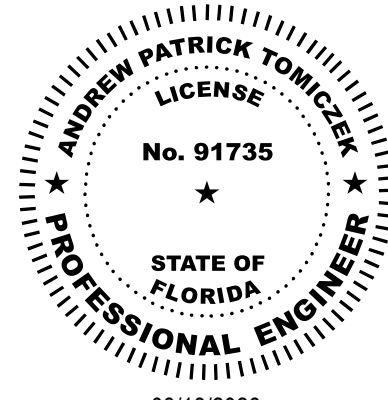


1. F#; DENOTES ISOLATED FOOTING MARK, SEE SCHEDULE ON S-303
2. F#; DENOTES CONTINUOUS FOOTING MARK, SEE SCHEDULE ON S-302
3. C#; DENOTES HSS COLUMN WITH BASE PLATE, SEE SCHEDULE ON S-303
4. BP#; DENOTES BASE PLATE, SEE SCHEDULE ON S-303
5. C#; DENOTES SLAB-ON-GIRDER CONTROL JOINT, SPACING SHALL NOT EXCEED 15' - 0"
- UNLESS NOTED OTHERWISE
6. FOR RE-ENTRANT CORNER BARS, SEE 6/e-S301
7. PLACE MIN 10 MIL VAPOR BARRIER OVER TERMITE TREATED, WELL COMPACTED FILL, SEE ARCH.
8. FOR CONCRETE FINISH, SEE ARCH.
9. C#; DENOTES 7' HIGH CONCRETE WALL
10. PIPING MUST PASS OVER COLUMN FOOTINGS, SEE DETAILS
- FOR STANDARD DETAILS OF PIPING PASSING UNDER WALL FOOTINGS AND TRENCHES ADJACENT TO FOOTINGS. PIPING PASSING UNDER FOOTINGS MUST BE IN PLACE AND INSPECTED BEFORE FOOTINGS ARE PREPARED OR FOOTING MUST STEP BELOW PIPE.
11. GC SHALL COORDINATE PLUMBING AND UTILITIES LOCATIONS WITH FOUNDATION AS NEEDED. ADDITIONALLY, GC SHALL COORDINATE FOUNDATION ELEVATIONS WITH PLUMBING AND UTILITIES AS NEEDED. RECORD ANY CHANGES REQUIRED TO FOUNDATION LOCATIONS TO STRUCTURAL ENGINEER OR RECORD FOR REVIEW AND APPROVAL.
12. GC SHALL PROVIDE HOUSEKEEPING PADS PER DETAIL 1/e-S303 AND COORDINATE LOCATIONS WITH MECHANICAL/ELECTRICAL EQUIPMENT DRAWINGS.

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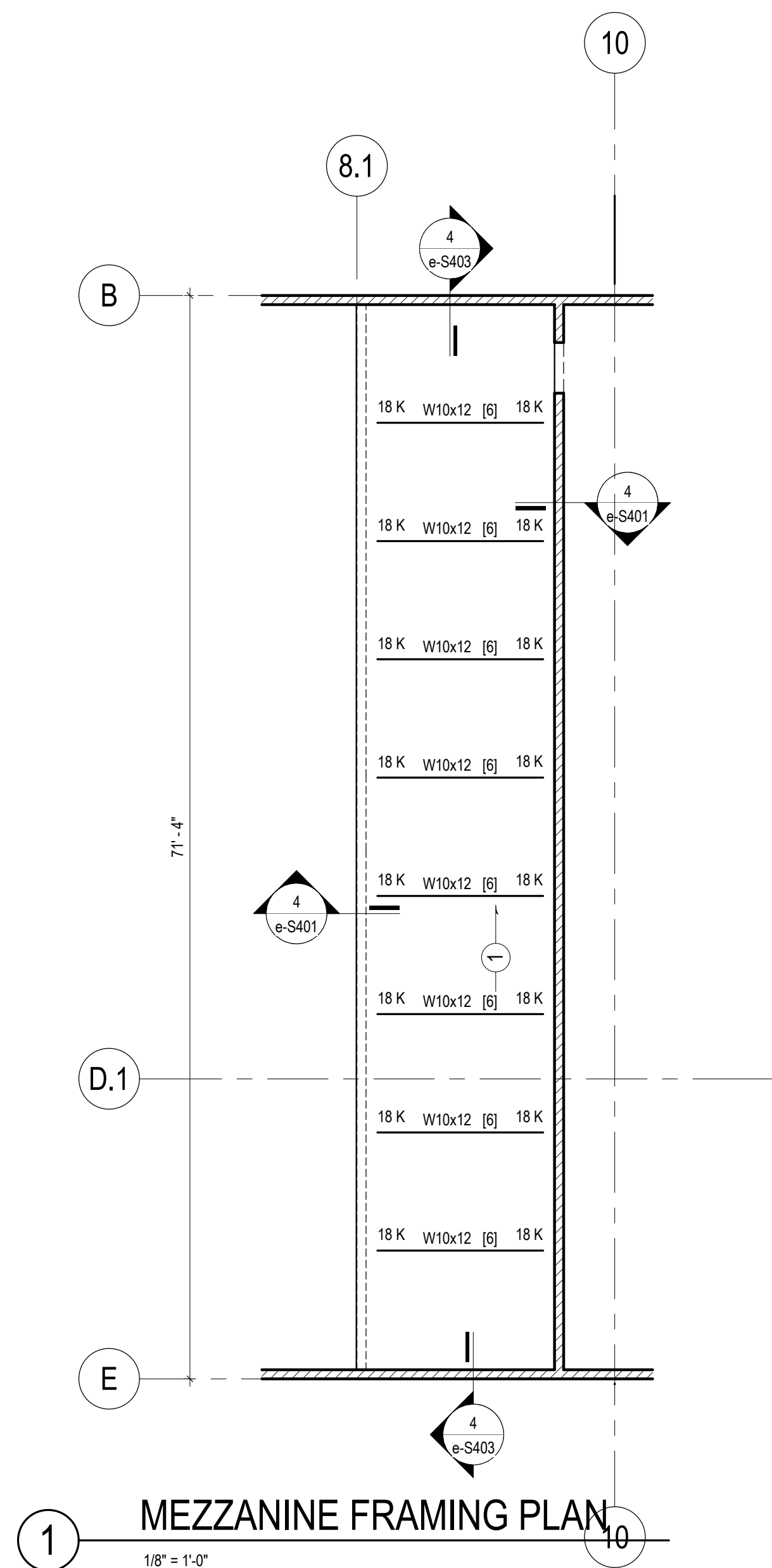
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MEZZANINE FRAMING PLAN

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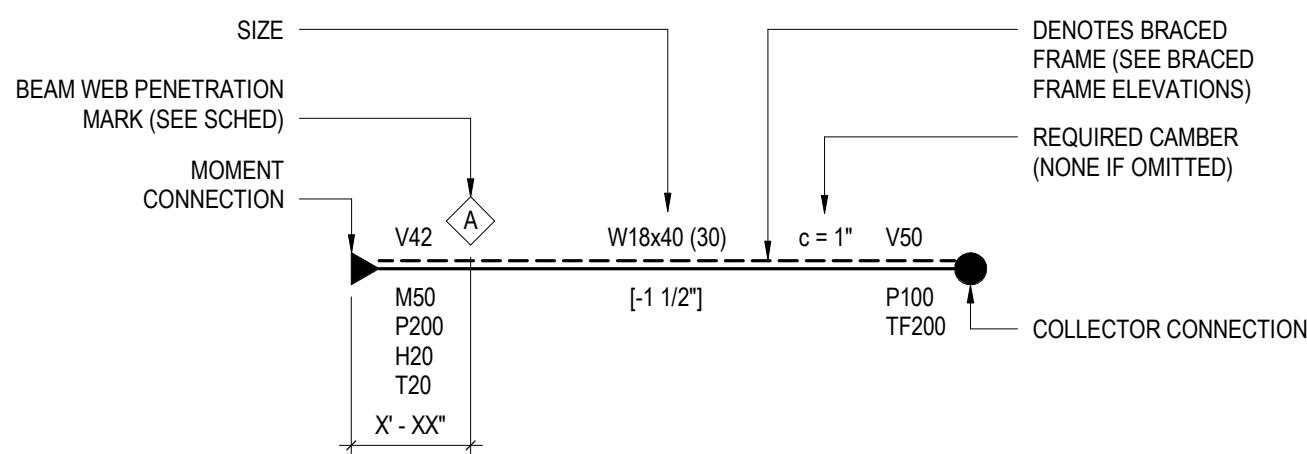
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NOTES:

1.  DENOTES 4 1/2" OF NORMALWEIGHT CONCRETE ON 1 1/2" 20 GAGE CORRUGATED COMPOSITE DECK REINFORCED W/ WWR 6x6 W2. 1xW2.
2. T/STEEL = (+)11' - 6"
3. PROVIDE AFTERSET CONCRETE HOUSEKEEPING PAD FOR MECHANICAL EQUIPMENT AS REQUIRED (SEE MECHANICAL DRAWINGS). SEE 3/ e-S303

STEEL BEAM LEGEND



- V INDICATES FACTORED VERTICAL SHEAR IN KIPS (SEE SCHED IF OMITTED)
M INDICATES FACTORED END MOMENT IN KIP-FT (SEE SCHED IF OMITTED)
P INDICATES FACTORED END AXIAL FORCE IN KIPS
H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS
T INDICATES TORSIONAL MOMENT IN KIP-FT
TF INDICATES AXIAL FORCE TRANSFERRED THROUGH THE CONNECTION JOINT IN KIPS



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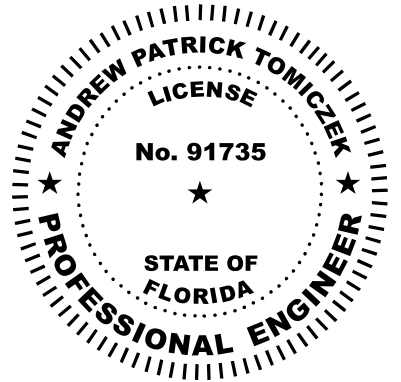
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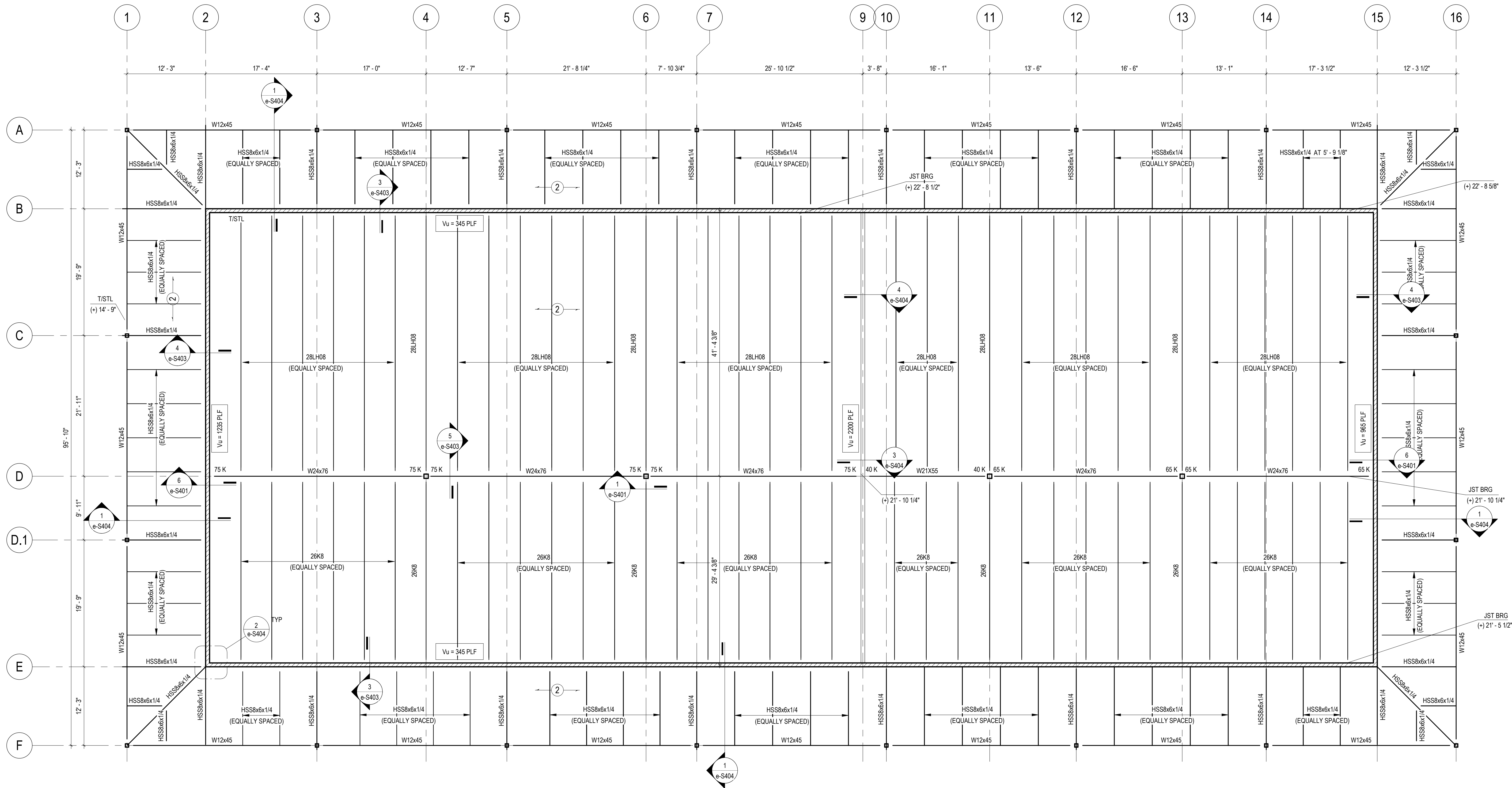
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ROOF FRAMING PLAN

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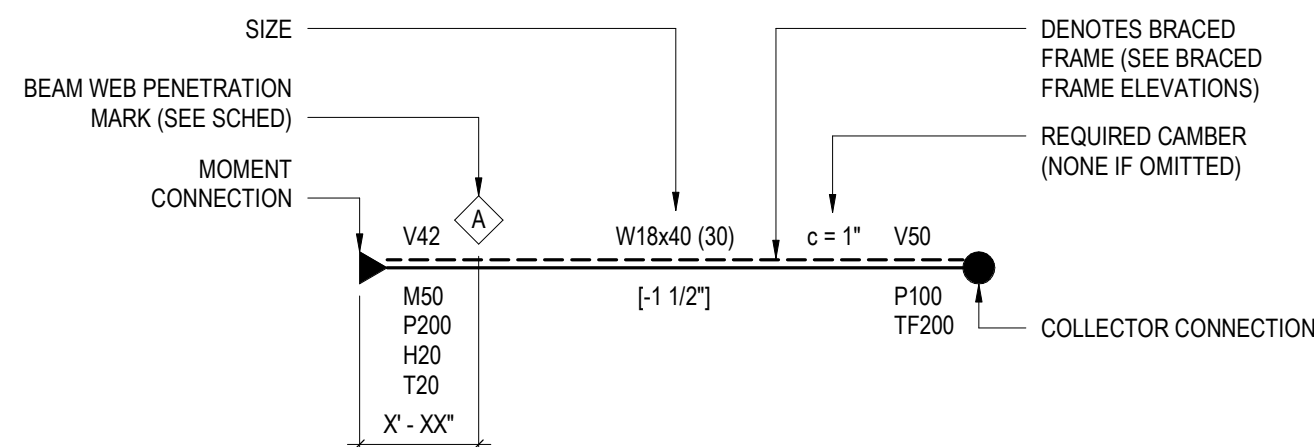
② ROOF FRAMING PLAN

$$1/8'' = 1'-0''$$

NOTES:

- | | | |
|----|--|--|
| 1. |  | DENOTES 4 1/2" OF NORMALWEIGHT CONCRETE ON 1 1/2" 20 GAGE CORRUGATED COMPOSITE DECK REINFORCED W/ WWR #6-W1.2xW2.1 |
| 2. |  | DENOTES 3" NORMALWEIGHT CONCRETE ON 1 1/2" 22 GAGE CORRUGATED COMPOSITE DECK REINFORCED WITH WWR #6-W1.4xW1.4 |
| 3. | GC TO COORDINATE OPEN WEB STEEL JOIST BRIDGING AND BRACING REQUIREMENTS PER OPEN WEB STEEL JOIST SUPPLIER AND SJI. | |
| 4. | 1/8" STEEL = VARIES (SEE PERIPLAN) | |
| 5. | HSS SPACING SHALL NOT EXCEED 6' - 0" UNLESS NOTED OTHERWISE. | |
| 6. | OPEN WEB STEEL JOIST SPACING SHALL NOT EXCEED 5' - 0" UNLESS NOTED OTHERWISE. | |
| 7. | PROVIDE 5' JOIST SEAT FOR 20Ks AT GRIDLINE D. | |
| 8. |  | DENOTES DIAPHRAGM SHEAR FORCE. |

STEEL BEAM LEGEND



V INDICATES FACTORED VERTICAL SHEAR IN KIPS (SEE SCHED IF OMITTED)
M INDICATES FACTORED END MOMENT IN KIP-FT (SEE SCHED IF OMITTED)
P INDICATES FACTORED END AXIAL FORCE IN KIPS
H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS
T INDICATES TORSIONAL MOMENT IN KIP-FT
TF INDICATES AXIAL FORCE TRANSFERRED THROUGH THE CONNECTION JOINT IN KIPS



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COLUMBIA STORM PROCESSING FACILITY
ISC STORES / PGD WAREHOUSE
BUILDING E
COLUMBIA COUNTY, FLORIDA

ANDREW PATRICK TOMCZEK
LICENSE
No. 91735
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
03/18/2026

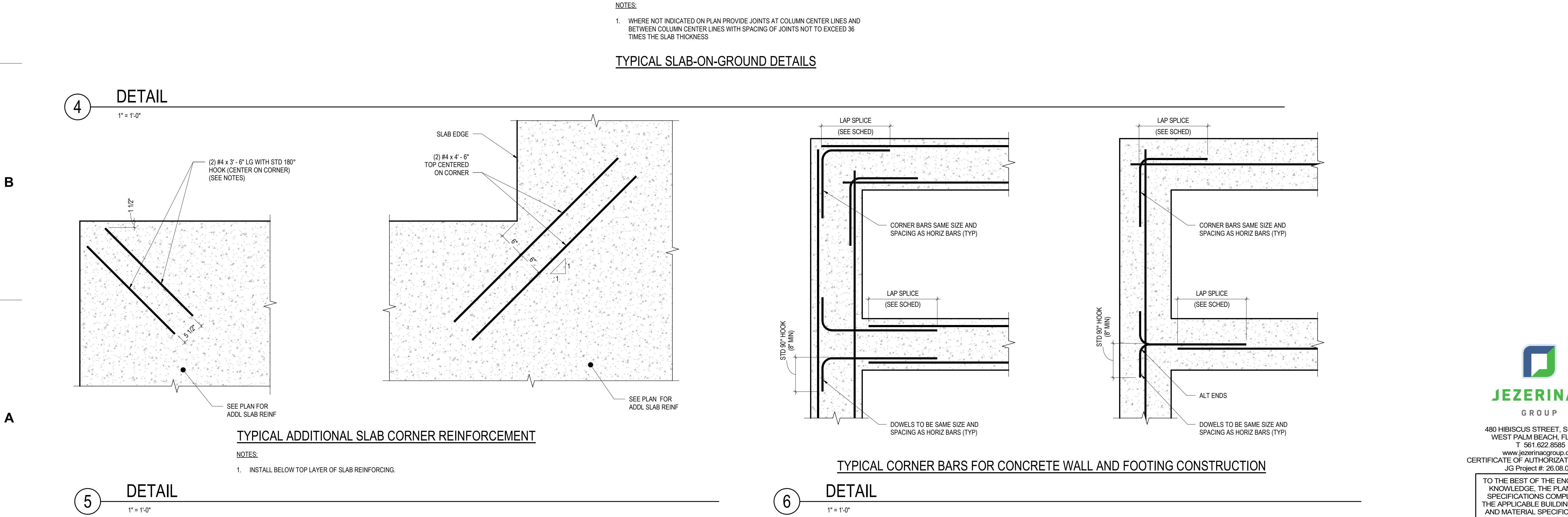
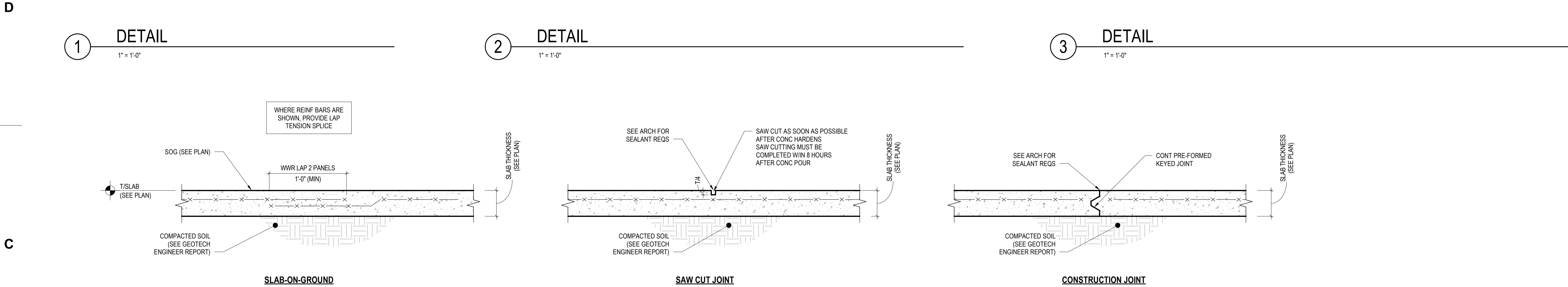
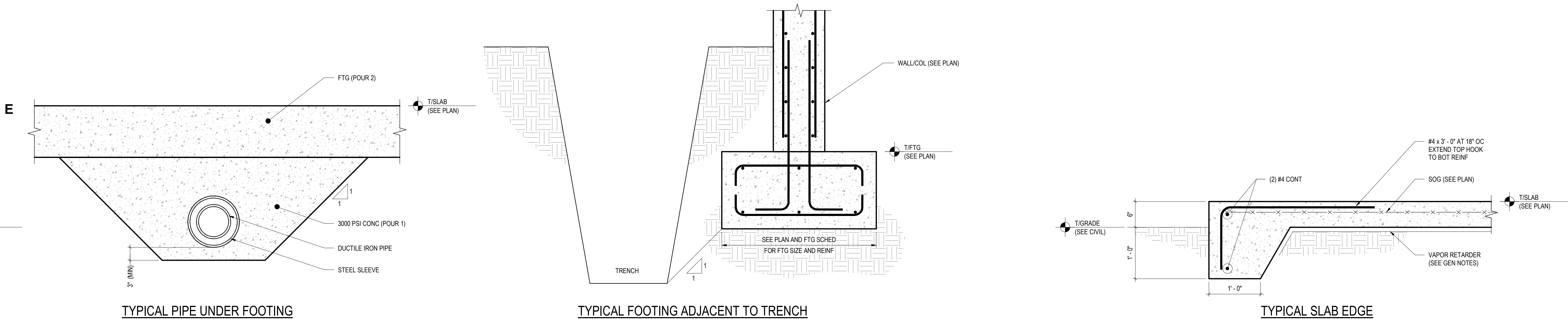
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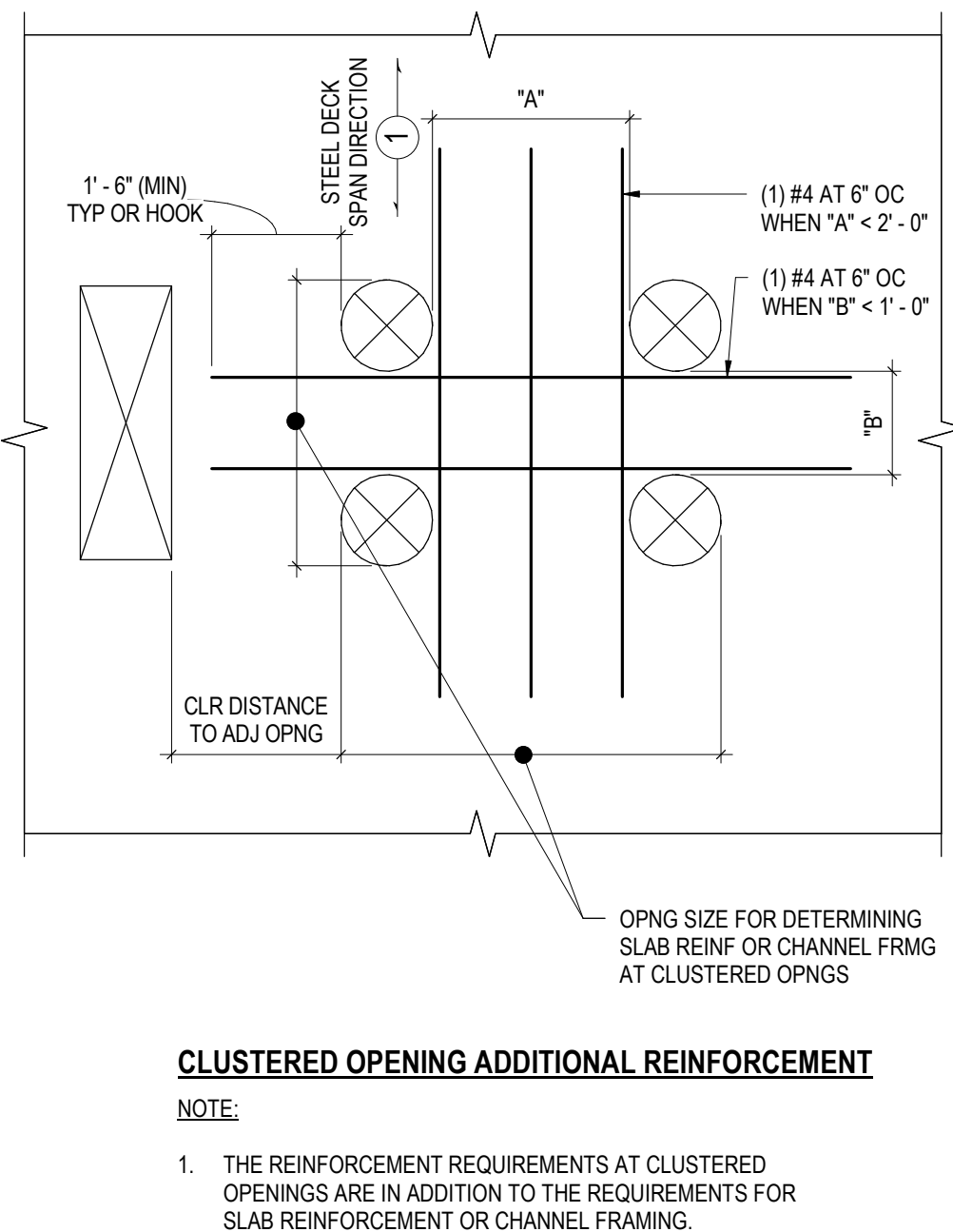
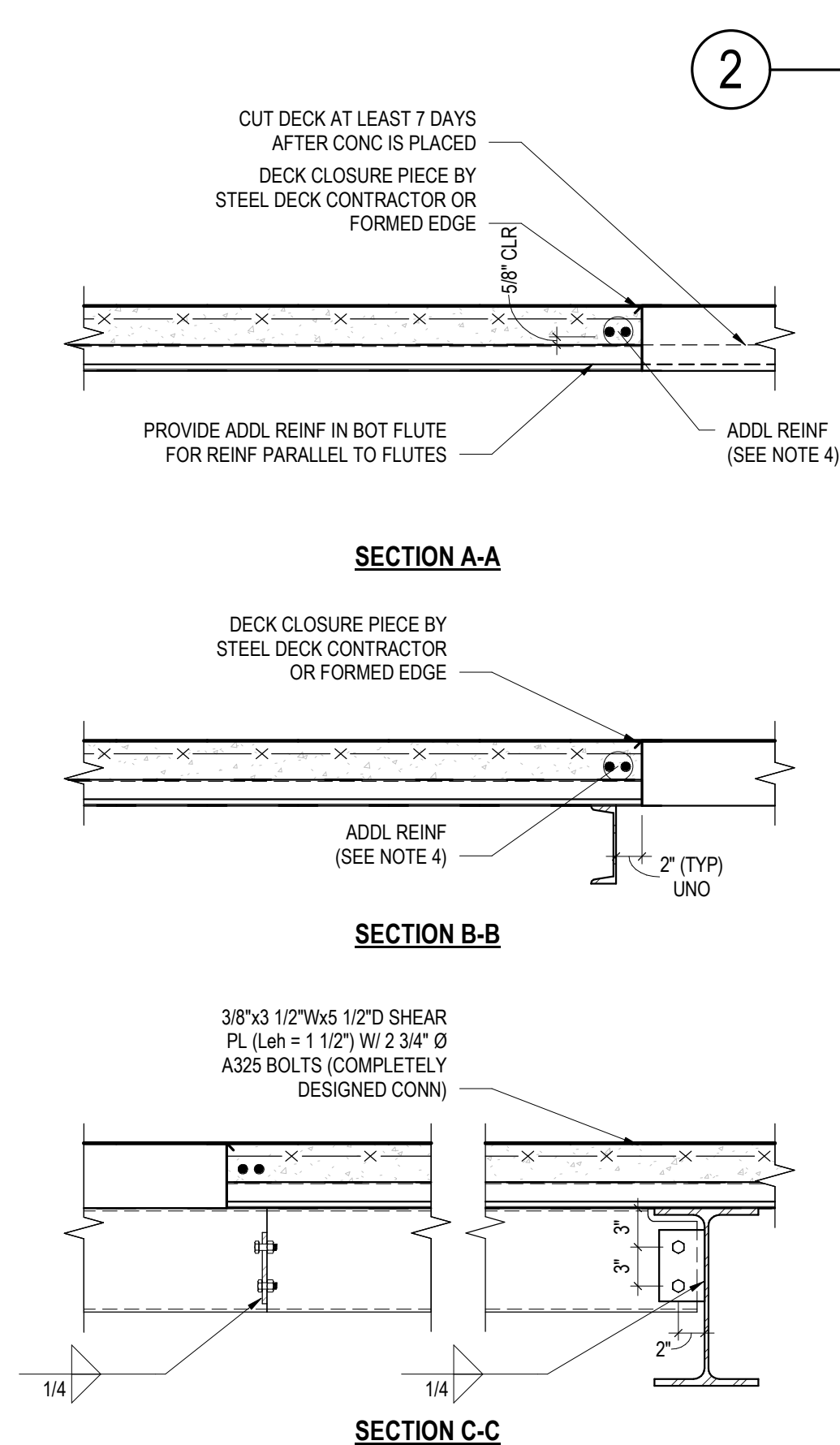
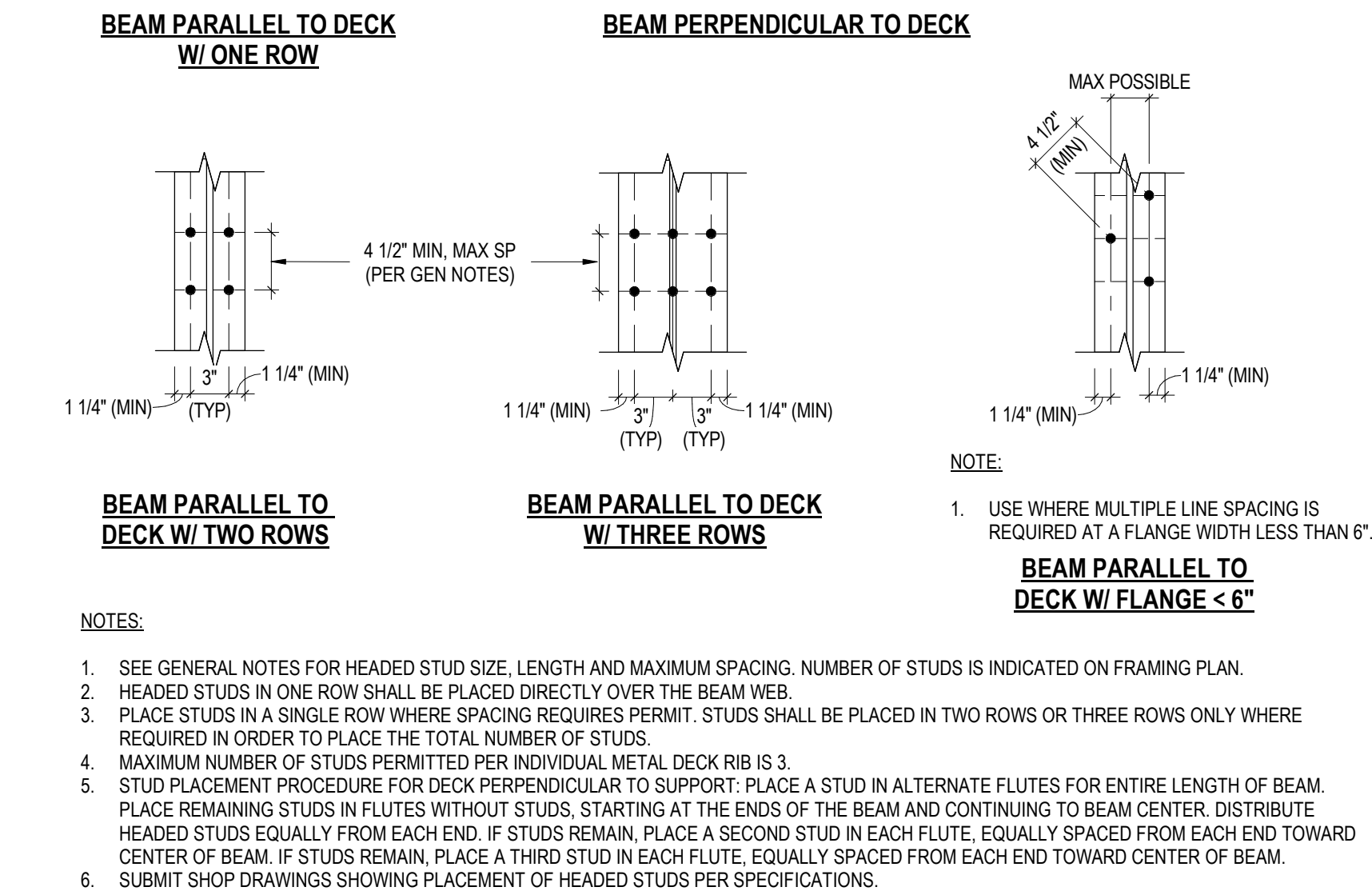
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NOTES:

1. SEE ADDITIONAL DETAILS FOR REINFORCEMENT AT CURTAIN WALL SUPPORT AND AT CORNERS (AS REQUIRED).
2. POUR STOP TO BE DESIGNED AND PROVIDED BY STEEL DECK CONTRACTOR (14 GA MINIMUM).
3. CANTILEVER REINFORCEMENT IS IN ADDITION TO ANY REINFORCEMENT SHOWN IN NOTES, ON PLAN, OR ON SLAB SCHEDULES.
4. SLAB EDGE SERVICE LOADS NOT TO EXCEED 400 PLF.



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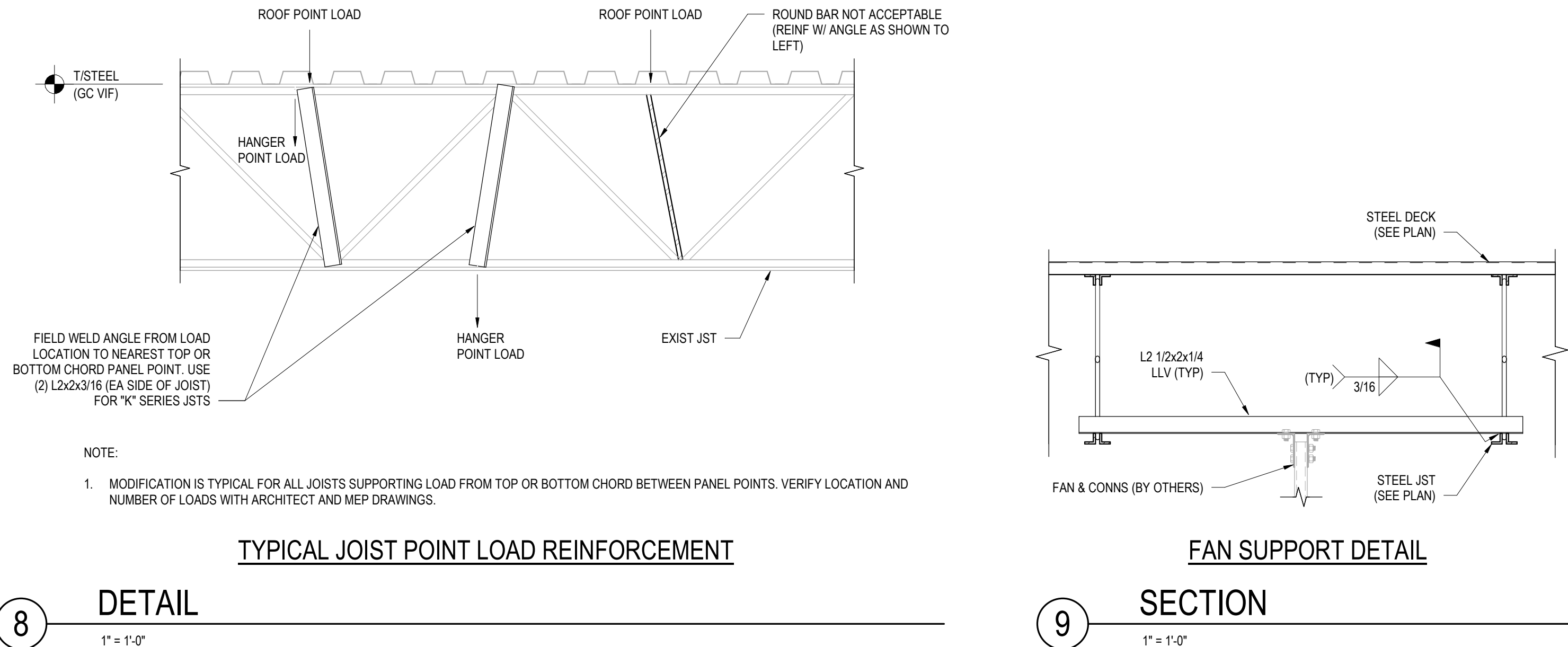
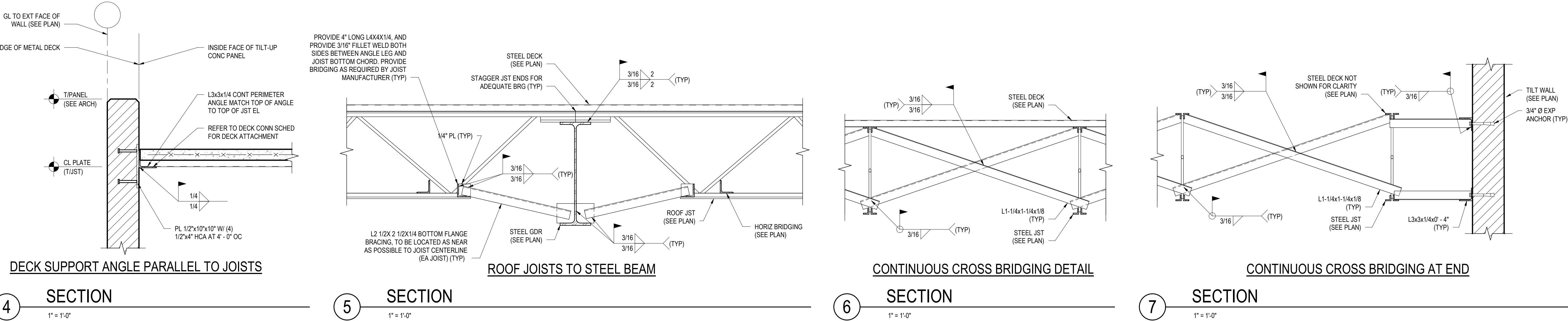
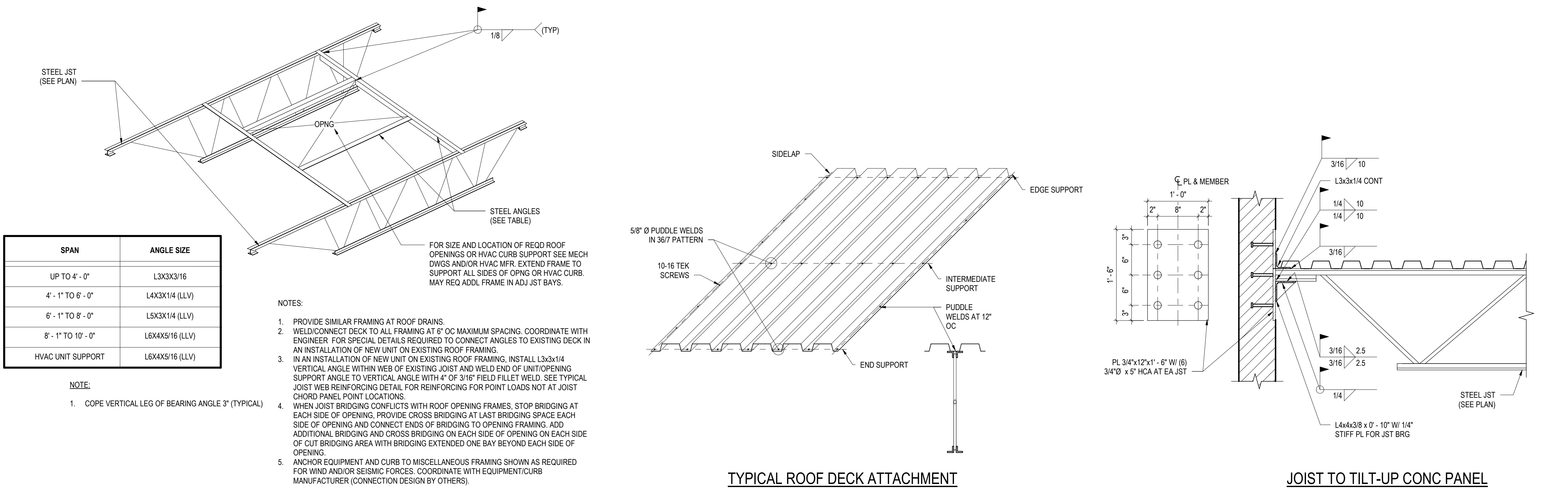
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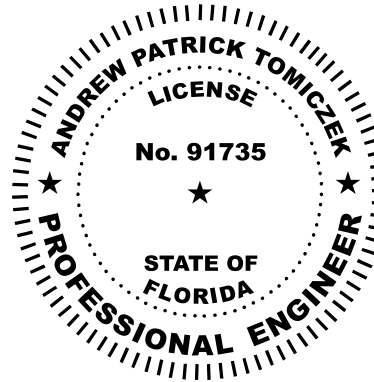
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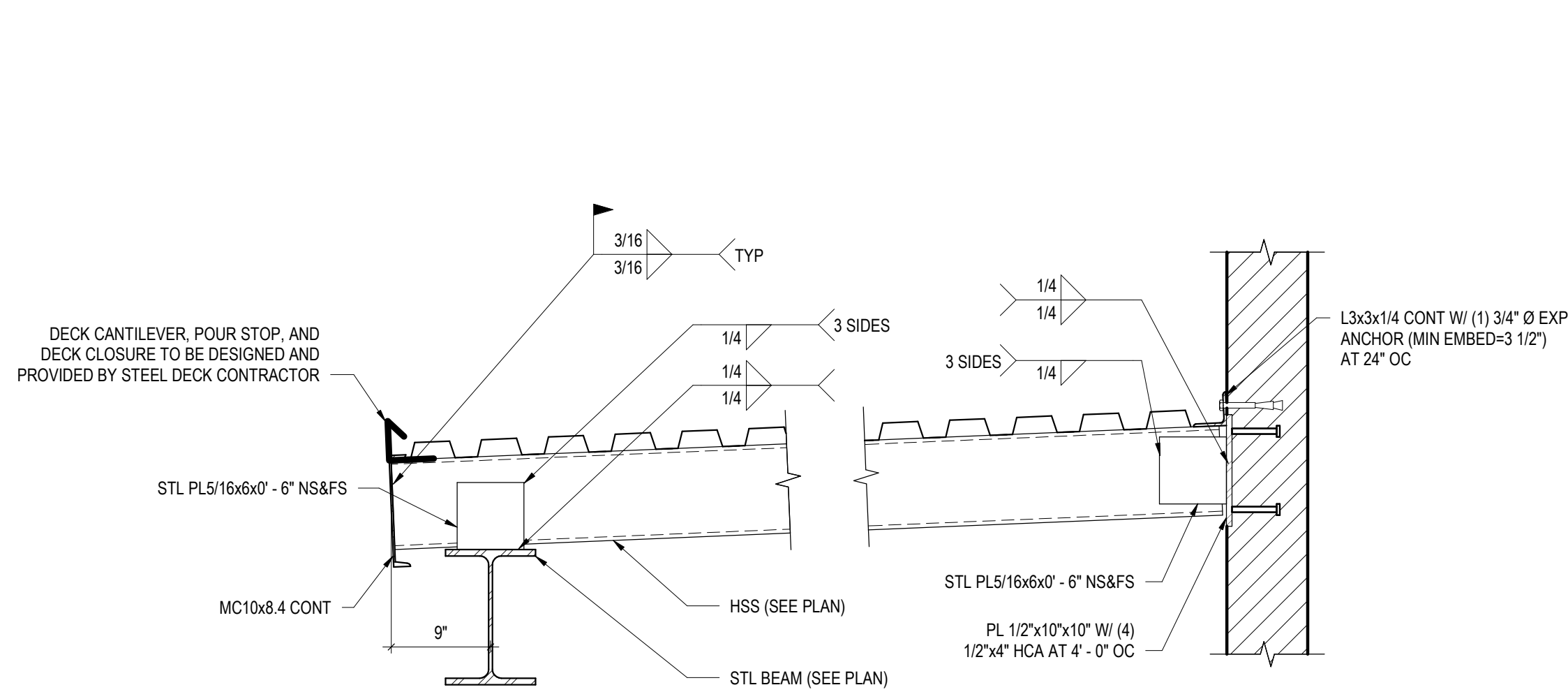
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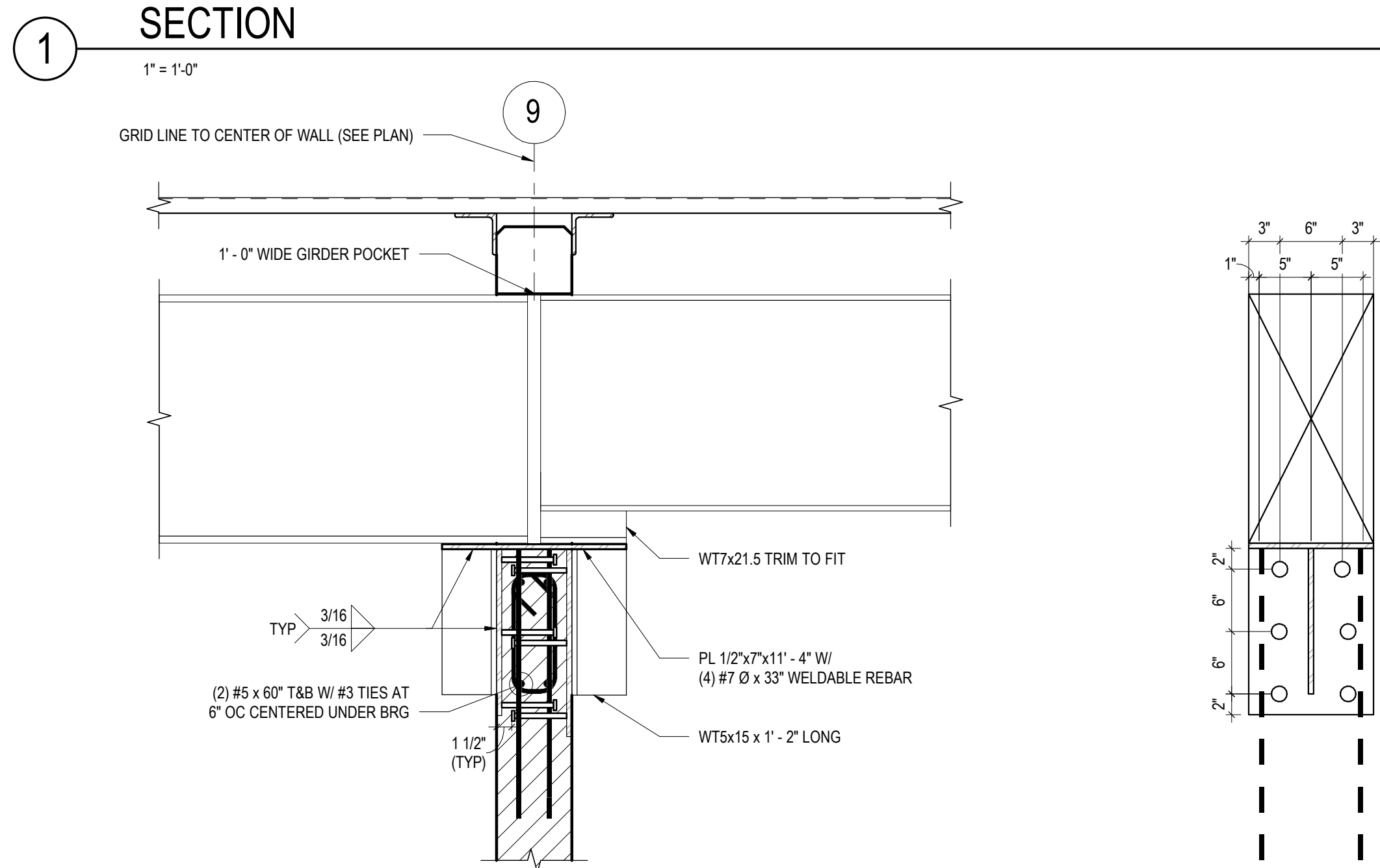


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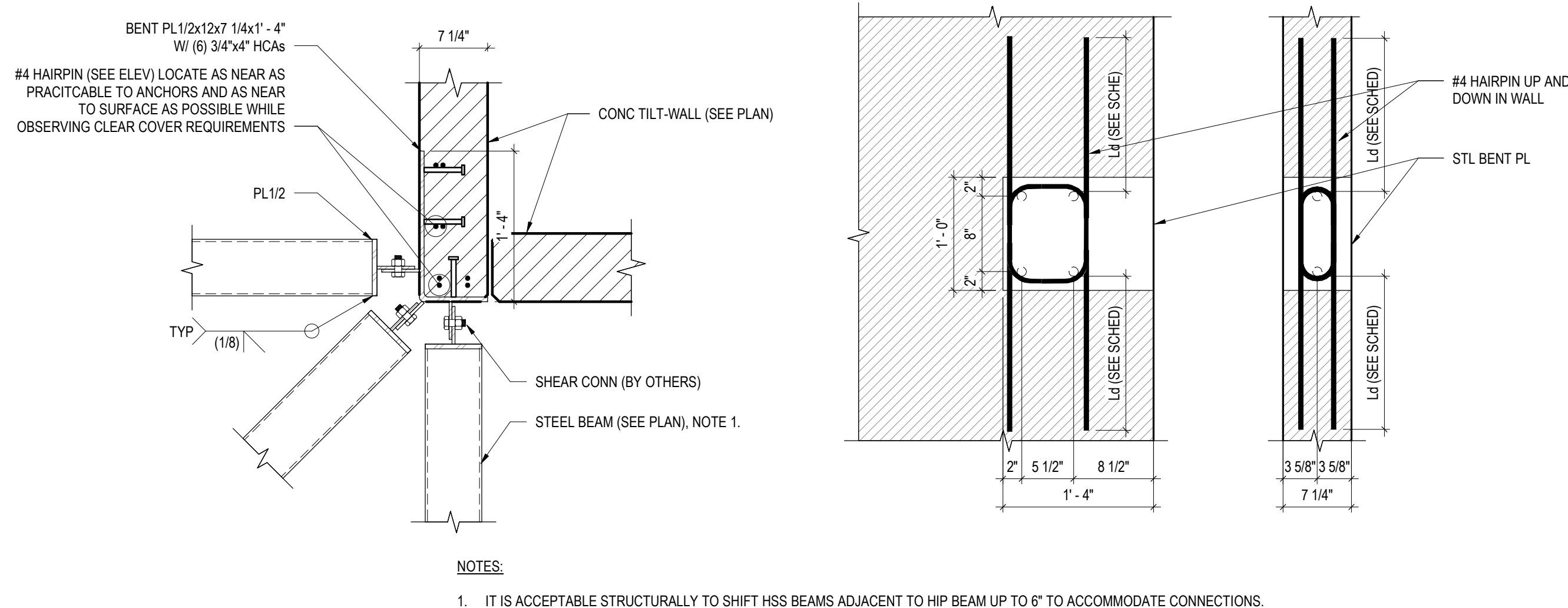
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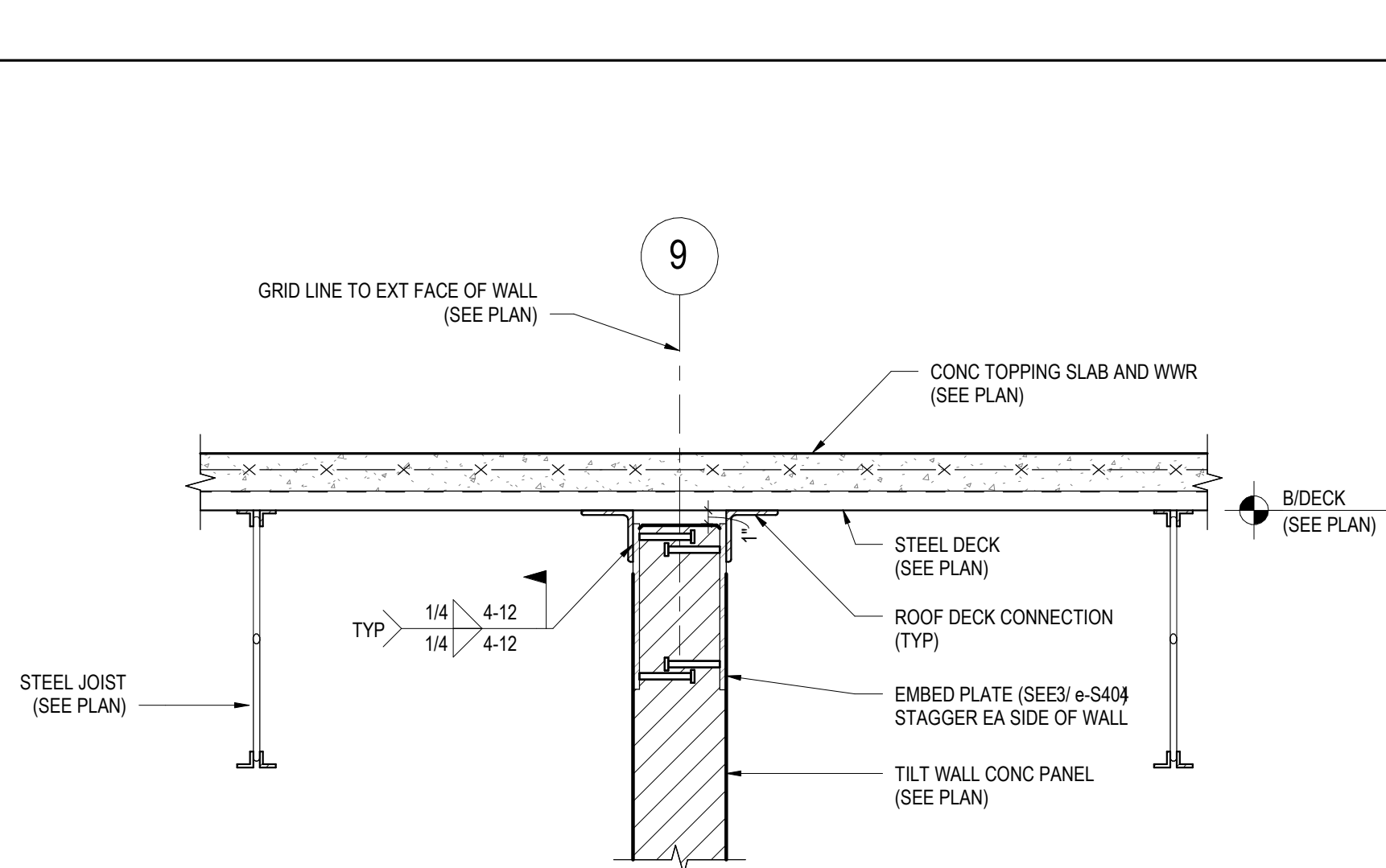
CANOPY FRAMING



ROOF GIRDER CONNECTION TO INTERIOR WALL



CANOPY CORNER CONNECTION



DECK SUPPORT ANGLE AT PARALLEL JOISTS

