



TowerCore

11500 Belmack Blvd. N.

Odessa, FL 33556

813-417-3747

Tony.sloan@towercorecontracting.com

Troy Crews
Columbia County
Building and Zoning
RE: Permit #000032030, New Foundation Design

Dear Mr. Crews,

I've enclosed the new foundation design along with the calculations for your review. There are no proposed location changes for the tower therefore we will utilize the same site plan. The rest of the drawings other than the proposed foundation design change are still in effect. Crews are ready to mobilize and our customer is eager to complete this site. If you could please review and let us know if everything is okay so we can re-start the development I would have appreciated. Thank you.

Tony Sloan

BOARD OF COUNTY COMMISSIONERS
OFFICE OF
BUILDING & ZONING
COLUMBIA COUNTY, FLORIDA

BUILDING PERMIT RECEIPT

RECEIPT NUMBER / PERMIT NUMBER 000032030 DATE 06/11/2014

APPLICANT TURK BEKTAS

OWNER NEW CINGULAR WIRELESS,PCS,LLC

CONTRACTOR TURK BEKTAS

PARCEL ID NUMBER 19-6S-16-03869-105 NUMBER OF EXISTING DWELLINGS 0

TYPE OF DEVELOPMENT COMMUNICATIONS TOWER

COMMENTS SE-0354

FEES:

BUILDING PERMIT 950 00 CERTIFICATION FEE 0 00

ZONING FEE 50 00 SURCHARGE FEE 0 00

FLOOD ZONE FEE _____ FLOOD DEVELOPMENT PERMIT _____

MOBILE HOME PERMIT _____ RELOCATION PERMIT _____

TRAVEL TRAILER PERMIT _____ RECONNECTION PERMIT _____

UTILITY POLE PERMIT _____ WASTE ASSESSMENT FEE _____

FIRE FEE (5 ACRES OR LESS) _____ CULVERT PERMIT _____

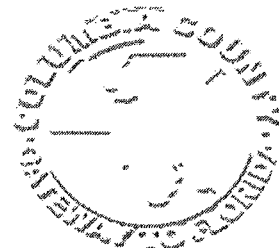
FIRE FEE (MORE THAN 5 ACRES) _____ RENEWAL FEE _____

CHECK NUMBER 2047 **TOTAL FEES CHARGES** 1000.00

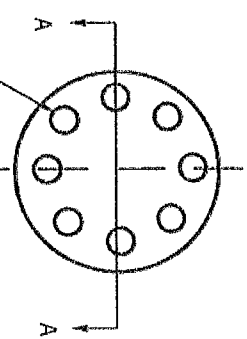
MAKE CHECKS PAYABLE TO BCC (Board of County Commissioners)

NOTE A SEPARATE CHECK IS REQUIRED FOR THE CULVERT WAIVER PERMITS

135 NE HERNANDO AVE.
SUITE B-21
LAKE CITY, FL 32055
Phone: 386-758-1008
Fax: 386-758-2160



Revised Shaft
 ok 11-18-14
 11-18-14



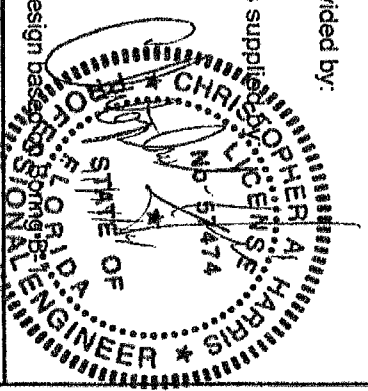
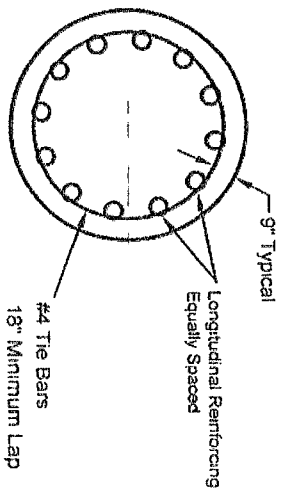
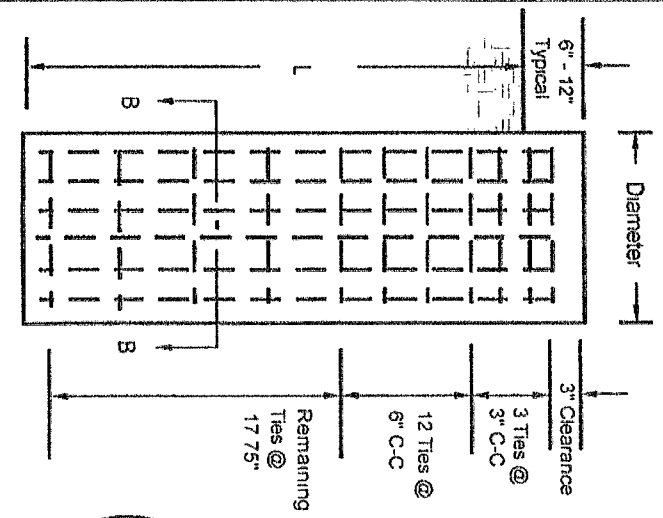
Bolt Circle = 71"
 Based on Tower Drawings

Foundation Layout		Drilled Shaft Information			Tower Loading			
Number of Shafts	"X" Dim	"Y" Dim	Shaft Diameter (IN)	Embedment Depth "L" (FT)	Long. Rebar	Axial Comp. (Kips)	OTM (FT-Kips)	Shear (Kips)
1	*	*	108"	36'	54 - #10	68.1 ^K	6,414.9 ^{FT-K}	46.3 ^K

** If needed for ground stability, permanent casing may be installed in the top 20'

Notes:

- 1 Drilled shafts shall be installed in accordance with ACI-336 (Latest Edition).
- 2 Concrete work shall be in accordance with ACI-318 (Latest Edition), and have a minimum 28 day strength of 4,000 psi
- 3 All reinforcing steel shall be deformed billet steel conforming to ASTM A-615, Grade 60.
- 4 All reinforcing steel shall have a minimum of three inches (3") coverage.
5. RWH Engineering will not be responsible for foundation integrity if tower is set within 72 hours of concrete placement, or without prior verification of concrete strength.
- 6 Final length of foundation may vary depending on actual in situ soil conditions relative to boring data supplied
- 7 Tower foundation design based on soil boring data provided by: EGSci Project #2014 10579344 Dated 9/12/14 Boring B-1
- 8 Tower layout and foundation design loading conditions supplied by Nello Drawing #227350 Dated 2/28/14 EIA/TIA-222-G



11/14/14 - Rev #1 - Design based on Boring B-1		Date		10/31/14	
American Tower Corporation		Scale		None	
195' Monopole Foundation		Approved By:		[Signature]	
Ft. White West Site		None		[Signature]	
Columbia County, Florida		None		None	

RWH
 ENGINEERING, INC.

Summary of Pile Response(s)

Definitions of Pile-head Loading Conditions:

- Load Type 1: Load 1 = Shear, lbs, and Load 2 = Moment, in-lbs
- Load Type 2: Load 1 = Shear, lbs, and Load 2 = Slope, radians
- Load Type 3: Load 1 = Shear, lbs, and Load 2 = Rotational Stiffness, in-lbs/radian
- Load Type 4: Load 1 = Top Deflection, inches, and Load 2 = Moment, in-lbs
- Load Type 5: Load 1 = Top Deflection, inches, and Load 2 = Slope, radians

Load Case No.	Load Type No.	Pile-head Condition		Axial Loading lbs	Pile-head Deflection inches	Maximum		Pile-head Rotation radians
		1 V(lbs) or Y(inches)	2 in-lb, rad., or in-lb/rad.			Moment in-lbs	Shear lbs	
1	1	V = 23150.	M = 38489400.	68100.	0.56962138	40858917.	-207070.	-0.00263310
2	1	V = 46300.	M = 76978800.	68100.	1.07059431	81355533.	-430321.	-0.00851663
3	1	V = 61733.	M = 1.026E+08	98800.	3.08277523	108370916	-610898.	-0.01413742

The analysis ended normally.

Fl. White West (83)-Fl. 11713
 bending properties relative to the magnitude of bending moment developed in the pile.

Output Summary for Load Case No. 3:

Pile-head deflection = 3.0827752 inches
 Computed slope at pile head = -0.0141374 radians
 Maximum bending moment = 1081701.6 Inch-lbs
 Maximum shear force = -610898.1 lbs
 Capth of maximum bending moment = 9.6200000 feet below pile head
 Capth of maximum shear force = 23.7100000 feet below pile head
 Number of iterations = 49
 Number of zero deflection points = 1

Pile-head Deflection vs. Pile Length for Load Case 3

Boundary Condition Type 1, Shear and Moment

Shear = 61733.1b
 Moment = 102538400. In-lb
 Axial Load = 90800.1b

Pile Length feet	Pile Head Deflection inches	Maximum Moment In-lbs	Maximum Shear lbs
37.0000	3.0827752	1081701.6	-610898.
35.1500	3.7997140	107818776.	-676484.
33.3000	4.4392053	107603864.	-713140.
31.4500	5.2369334	107408364.	-749197.
29.6000	5.9522051	107337220.	-773414.
27.7500	10.1183336	107458069.	-858038.

Pt. White West (B1) - FL 1p70										
24.428	-0.1139	44128552	-691875.	-0.008438	0.000	2.959E+13	1360.0403	51018.	0.000	0.000
24.796	-0.1513	41471043	-594773.	-0.008431	0.000	2.961E+13	1839.2778	51959.	0.000	0.000
25.160	-0.1888	38851780.	-531508.	-0.008425	0.000	2.964E+13	2334.0346	54899.	0.000	0.000
25.530	-0.2262	36278527.	-574917.	-0.008420	0.000	2.966E+13	2844.3155	51839.	0.000	0.000
25.900	-0.2635	33761544.	-550216.	-0.008415	0.000	2.968E+13	3370.1262	56779.	0.000	0.000
26.270	-0.3009	31318995.	-544951.	-0.008410	0.000	2.971E+13	3911.4734	57715.	0.000	0.000
26.640	-0.3382	28935955.	-525447.	-0.008405	0.000	2.973E+13	4468.3644	58660.	0.000	0.000
27.010	-0.3753	26651409.	-504337.	-0.008401	0.000	2.975E+13	5040.8075	59600.	0.000	0.000
27.380	-0.4128	24465216.	-480650.	-0.008397	0.000	2.977E+13	5628.8113	60540.	0.000	0.000
27.750	-0.4501	22389995.	-454319.	-0.008394	0.000	2.979E+13	6212.3847	61480.	0.000	0.000
28.120	-0.4874	20437635.	-425272.	-0.008391	0.000	2.981E+13	6851.5370	62421.	0.000	0.000
28.490	-0.5248	18628342.	-393442.	-0.008388	0.000	2.982E+13	7486.2773	63361.	0.000	0.000
28.860	-0.5618	16950630.	-358957.	-0.008385	0.000	2.984E+13	8047.8568	64300.	0.000	0.000
29.230	-0.5991	15439958.	-322723.	-0.008383	0.000	2.985E+13	8271.6053	64321.	0.000	0.000
29.600	-0.6363	14091689.	-298078.	-0.008380	0.000	2.986E+13	8627.8923	64973.	0.000	0.000
29.970	-0.6735	12799396.	-285432.	-0.008378	0.000	2.987E+13	8868.3538	65910.	0.000	0.000
30.340	-0.7107	11563728.	-272610.	-0.008377	0.000	2.988E+13	9077.1635	66163.	0.000	0.000
30.710	-0.7479	10388371.	-259620.	-0.008375	0.000	2.989E+13	9244.4707	66748.	0.000	0.000
31.080	-0.7850	9265058.	-246466.	-0.008374	0.000	2.990E+13	9380.4050	67185.	0.000	0.000
31.450	-0.8222	8203580.	-233156.	-0.008372	0.000	2.991E+13	9485.0790	67581.	0.000	0.000
31.820	-0.8594	7261379.	-219695.	-0.008371	0.000	2.991E+13	9548.5925	67950.	0.000	0.000
32.190	-0.8966	6259357.	-206087.	-0.008370	0.000	2.991E+13	9580.0301	68258.	0.000	0.000
32.560	-0.9337	5378072.	-192338.	-0.008369	0.000	2.991E+13	9582.4727	68500.	0.000	0.000
32.930	-0.9709	4558145.	-178451.	-0.008368	0.000	2.991E+13	9542.9854	68674.	0.000	0.000
33.300	-1.0080	3800178.	-164430.	-0.008368	0.000	2.991E+13	9472.6311	68794.	0.000	0.000
33.670	-1.0452	3104754.	-150280.	-0.008367	0.000	2.991E+13	9281.4703	68809.	0.000	0.000
34.040	-1.0823	2472440.	-136043.	-0.008367	0.000	2.991E+13	9029.5471	68748.	0.000	0.000
34.410	-1.1195	1903797.	-121643.	-0.008367	0.000	2.991E+13	8756.9840	68617.	0.000	0.000
34.780	-1.1566	1399356.	-105737.	-0.008366	0.000	2.991E+13	8480.6149	68437.	0.000	0.000
35.150	-1.1938	971417.	-88459.	-0.008366	0.000	2.991E+13	8211.4940	68249.	0.000	0.000
35.520	-1.2309	620385.	-71926.	-0.008366	0.000	2.991E+13	7941.4321	68127.	0.000	0.000
35.890	-1.2681	347454.	-53460.	-0.008366	0.000	2.991E+13	7670.8357	68094.	0.000	0.000
36.260	-1.3052	152503.	-35766.	-0.008366	0.000	2.991E+13	7399.6521	68095.	0.000	0.000
36.630	-1.3424	36300.	-17945.	-0.008366	0.000	2.991E+13	7027.8298	68022.	0.000	0.000
37.000	-1.3795	0.000	0.000	-0.008366	0.000	2.991E+13	6655.3990	68025.	0.000	0.000

* This analysis computed pile response using nonlinear moment-curvature relationships. Values of total stress due to combined axial and bending stresses are computed only for elastic sections only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for "nonlinear".

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9.670	1.5013	1.082E+08	-2145.9534	-0.0115	0.000	4.630E+12	-2865.0509	7543.8295	0.000
9.990	1.5505	1.081E+08	-15285.	-0.0114	0.000	4.638E+12	-3053.6198	8744.2920	0.000
10.350	1.5001	1.080E+08	-29253.	-0.0113	0.000	4.638E+12	-3238.1996	9584.2805	0.000
10.730	1.4502	1.079E+08	-44031.	-0.0112	0.000	4.638E+12	-3418.2607	10463.	0.000
11.100	1.4007	1.077E+08	-59596.	-0.0111	0.000	4.639E+12	-3593.2800	11390.	0.000
11.470	1.3517	1.074E+08	-75927.	-0.0110	0.000	4.639E+12	-3762.7395	12359.	0.000
11.840	1.3032	1.070E+08	-92966.	-0.0109	0.000	4.640E+12	-3926.1244	13376.	0.000
12.210	1.2551	1.065E+08	-110776.	-0.0108	0.000	4.641E+12	-4082.9219	14444.	0.000
12.580	1.2074	1.060E+08	-129237.	-0.0107	0.000	4.642E+12	-4232.9487	15563.	0.000
12.950	1.1602	1.054E+08	-148359.	-0.0106	0.000	4.643E+12	-4380.4257	16763.	0.000
13.320	1.1135	1.047E+08	-168119.	-0.0105	0.000	4.643E+12	-4520.4108	18025.	0.000
13.690	1.0672	1.039E+08	-188482.	-0.0104	0.000	4.643E+12	-4652.4034	19356.	0.000
14.060	1.0213	1.030E+08	-209413.	-0.0103	0.000	4.643E+12	-4775.3963	20762.	0.000
14.430	0.9759	1.021E+08	-230872.	-0.0102	0.000	4.643E+12	-4890.3972	22249.	0.000
14.800	0.9309	1.010E+08	-252819.	-0.0101	0.000	4.643E+12	-4995.3972	23825.	0.000
15.170	0.8864	99837344.	-275208.	-0.0099899	0.000	4.643E+12	-5090.3955	25498.	0.000
15.540	0.8422	98569255.	-297999.	-0.0098994	0.000	4.643E+12	-5174.3977	27278	0.000
15.910	0.7985	9715912.	-321132.	-0.0098001	0.000	4.643E+12	-5247.3298	29177	0.000
16.280	0.7552	95725599.	-344565.	-0.0097009	0.000	4.643E+12	-5308.4590	31210	0.000
16.650	0.7123	94147210.	-368200.	-0.0096119	0.000	4.643E+12	-5337.9271	33274	0.000
17.020	0.6698	92463645.	-391508.	-0.0095300	0.000	4.643E+12	-5361.1897	34214	0.000
17.390	0.6277	90678299.	-413999.	-0.0094413	0.000	4.643E+12	-4969.5368	35154	0.000
17.760	0.5859	88794951.	-435605.	-0.0093518	0.000	4.643E+12	-4763.1518	36094.	0.000
18.130	0.5446	86876670.	-456263.	-0.0092715	0.000	4.643E+12	-4542.2082	37035.	0.000
18.500	0.5036	84750813.	-475908.	-0.0092194	0.000	4.643E+12	-4306.0686	37975.	0.000
18.870	0.4629	82599001.	-494477.	-0.0091115	0.000	4.643E+12	-4057.2851	38915.	0.000
19.240	0.4226	80367210.	-511905.	-0.0089918	0.000	4.643E+12	-3793.5986	39855.	0.000
19.610	0.3827	78060585.	-528133.	-0.0088563	0.000	4.643E+12	-3515.9190	40795.	0.000
19.980	0.3430	75584619.	-543096.	-0.0088001	0.000	4.643E+12	-3224.4246	41735.	0.000
20.350	0.3037	73245059.	-556735.	-0.0088221	0.000	4.643E+12	-2919.1625	42675.	0.000
20.720	0.2647	70717924.	-568938.	-0.0088753	0.000	4.643E+12	-2600.2481	43615.	0.000
21.090	0.2260	68199503.	-579795.	-0.0088888	0.000	4.643E+12	-2267.7056	44555.	0.000
21.460	0.1875	655606349.	-589306.	-0.0088626	0.000	4.643E+12	-1921.7874	45497.	0.000
21.830	0.1494	62975236.	-596831.	-0.0088566	0.000	4.643E+12	-1562.3748	46437.	0.000
22.200	0.1115	60313399.	-6023940	-0.0088308	0.000	4.643E+12	-1189.5775	47377.	0.000
22.570	0.0738	57628038.	-607355	-0.0088476	0.000	2.944E+13	-803.4110	48317.	0.000
22.940	0.0362	54926836.	-610040	-0.0088468	0.000	2.944E+13	-401.7652	49257.	0.000
23.310	-0.001364	52217710.	-610898	-0.0088460	0.000	2.951E+13	15.4162	50198.	0.000
23.680	-0.0339	49508934.	-609869	-0.0088452	0.000	2.954E+13	448.1111	51138.	0.000
24.050	-0.0754	46808890.	-606884	-0.0088445	0.000	2.956E+13	896.3188	52078.	0.000

Ft White West (B1)-FL1p70

Ft White West (51)-Fl.1p70
 Computed Values of Pile Loading and Deflection
 for Lateral Loading for Load Case Number 3

Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 61773.3 lbs
 Applied moment at pile head = 102638400.0 in-lbs
 Axial thrust load on pile head = 908000.0 lbs

Depth X feet	Deflect.		Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness lb-in ²	Soil Res. p lb/in	Soil Spv. E _s ^m lb/inch	Disturb Lat. Load lb/inch
	Y	inches								
0.00	3.0828	1.026E+08	61773.	-0.0141	0.000	4.650E+12	0.000	0.000	0.000	
0.370	3.0202	1.029E+08	61773.	-0.0140	0.000	4.650E+12	0.000	0.000	0.000	
0.740	2.9581	1.032E+08	61773.	-0.0139	0.000	4.648E+12	0.000	0.000	0.000	
1.110	2.8964	1.035E+08	61773.	-0.0138	0.000	4.648E+12	0.000	0.000	0.000	
1.480	2.8352	1.038E+08	61773.	-0.0137	0.000	4.647E+12	0.000	0.000	0.000	
1.850	2.7744	1.040E+08	61773.	-0.0136	0.000	4.646E+12	0.000	0.000	0.000	
2.220	2.7140	1.043E+08	61773.	-0.0135	0.000	4.646E+12	0.000	0.000	0.000	
2.590	2.6541	1.046E+08	61773.	-0.0134	0.000	4.645E+12	0.000	0.000	0.000	
2.960	2.5946	1.049E+08	61773.	-0.0133	0.000	4.645E+12	0.000	0.000	0.000	
3.330	2.5356	1.052E+08	61511.	-0.0132	0.000	4.644E+12	-100.1054	175.2099	0.000	
3.700	2.4770	1.054E+08	60828.	-0.0131	0.000	4.643E+12	-207.4350	371.8270	0.000	
4.070	2.4189	1.057E+08	59680.	-0.0130	0.000	4.643E+12	-309.6439	568.3641	0.000	
4.440	2.3612	1.059E+08	58090.	-0.0129	0.000	4.642E+12	-406.7795	764.9012	0.000	
4.810	2.3040	1.052E+08	56079.	-0.0128	0.000	4.642E+12	-498.9059	961.4383	0.000	
5.180	2.2472	1.055E+08	53671.	-0.0127	0.000	4.641E+12	-586.0831	1157.9755	0.000	
5.550	2.1909	1.057E+08	50836.	-0.0126	0.000	4.641E+12	-668.3712	1354.5125	0.000	
5.920	2.1350	1.059E+08	47746.	-0.0125	0.000	4.640E+12	-745.8303	1551.0497	0.000	
6.290	2.0796	1.072E+08	44273.	-0.0124	0.000	4.640E+12	-818.5225	1747.5863	0.000	
6.660	2.0246	1.073E+08	40438.	-0.0123	0.000	4.639E+12	-886.5069	1944.1240	0.000	
7.030	1.9701	1.075E+08	36412.	-0.0122	0.000	4.639E+12	-949.8450	2140.6511	0.000	
7.400	1.9160	1.077E+08	32054.	-0.0121	0.000	4.639E+12	-1008.5977	2337.1982	0.000	
7.770	1.8624	1.078E+08	27455.	-0.0120	0.000	4.638E+12	-1062.8261	2533.7353	0.000	
8.140	1.8093	1.079E+08	22636.	-0.0119	0.000	4.638E+12	-1112.5915	2730.2724	0.000	
8.510	1.7566	1.080E+08	17595.	-0.0118	0.000	4.638E+12	-1157.9543	2926.8095	0.000	
8.880	1.7044	1.081E+08	12363.	-0.0117	0.000	4.638E+12	-1198.9775	3123.3467	0.000	
9.250	1.6526	1.081E+08	6957.7594	-0.0116	0.000	4.638E+12	-1235.7205	3319.8838	0.000	

Output Summary for Load Case No. 2:

Pile-head deflection = 1.8705943 inches
 Computed slope at pile head = -0.0089166 radians
 Maximum bending moment = 81356533. Inch-lbs
 Maximum shear force = -430321. lbs
 Depth of maximum bending moment = 9.9900000 feet below pile head
 Depth of maximum shear force = 23.6800000 feet below pile head
 Number of iterations = 52
 Number of zero deflection points = 1

Pile head Deflection vs. Pile Length for Load Case 2

Boundary Condition Type 1, Shear and Moment

Shear = 45300. lb
 Moment = 76978300. in-lb
 Axial Load = 68100. lb

Pile Length Feet	Pile Head Deflection Inches	Maximum Moment In-lbs	Maximum Shear lbs
37.0000	1.8705943	81356533.	-430321.
35.1500	2.2498857	81144708.	-475623.
33.3000	2.5553534	80980557.	-509757.
31.4500	2.9593293	80804510.	-542079.
29.6000	3.3390896	80677503.	-564702.
27.7500	4.9144083	80346127.	-619022.
25.9000	8.2598628	80317578.	-681442.

Pile White West (B1)-FL.1D70									
24.790	-0.0717	31055289.	-424190.	-0.004973	0.000	2.969E+13	871.1797	53959.	0.000
25.150	-0.0938	31182976.	-419682.	-0.004958	0.000	2.971E+13	1159.2205	54899.	0.000
25.530	-0.1158	29332515.	-413876.	-0.004953	0.000	2.973E+13	1456.3473	55839.	0.000
25.900	-0.1378	21512762.	-405730.	-0.004959	0.000	2.974E+13	1762.5629	56779.	0.000
26.270	-0.1598	25723755.	-398204.	-0.004955	0.000	2.976E+13	2077.8707	57719.	0.000
26.640	-0.1818	23977708.	-388258.	-0.004951	0.000	2.979E+13	2402.2749	58660.	0.000
27.010	-0.2038	22279018.	-376851.	-0.004948	0.000	2.982E+13	2735.7803	59600.	0.000
27.380	-0.2258	20634259.	-363944.	-0.004945	0.000	2.985E+13	3078.3923	60540.	0.000
27.750	-0.2477	19050186.	-349495.	-0.004942	0.000	2.988E+13	3430.1167	61480.	0.000
28.120	-0.2697	17533731.	-333464.	-0.004939	0.000	2.983E+13	3790.9595	62421.	0.000
28.490	-0.2916	16092009.	-315811.	-0.004934	0.000	2.986E+13	4160.9272	63361.	0.000
28.860	-0.3135	14732314.	-296455.	-0.004932	0.000	2.987E+13	4540.0264	64301.	0.000
29.230	-0.3354	13462117.	-278475.	-0.004930	0.000	2.988E+13	4928.2634	65241.	0.000
29.600	-0.3573	12289075.	-259100.	-0.004929	0.000	2.988E+13	2447.9496	30421.	0.000
29.970	-0.3792	11164289.	-248150.	-0.004929	0.000	2.988E+13	2484.6074	25094.	0.000
30.340	-0.4011	10088483.	-237040.	-0.004927	0.000	2.989E+13	2519.6993	27895.	0.000
30.710	-0.4229	9062150.	-225778.	-0.004926	0.000	2.990E+13	2553.3734	26806.	0.000
31.080	-0.4448	8086552.	-214469.	-0.004924	0.000	2.992E+13	2585.7560	26181.	0.000
31.450	-0.4667	7161728.	-202819.	-0.004923	0.000	2.993E+13	2616.9599	24999.	0.000
31.820	-0.4885	6288494.	-191133.	-0.004922	0.000	2.993E+13	2647.0780	24059.	0.000
32.190	-0.5104	5467443.	-179315.	-0.004921	0.000	2.993E+13	2676.1978	23282.	0.000
32.560	-0.5322	4699149.	-167371.	-0.004920	0.000	2.993E+13	2704.3917	22561.	0.000
32.930	-0.5541	3984168.	-155302.	-0.004919	0.000	2.993E+13	2731.7265	21891.	0.000
33.300	-0.5759	3323039.	-143115.	-0.004919	0.000	2.993E+13	2758.2615	21255.	0.000
33.670	-0.5977	2716286.	-130811.	-0.004919	0.000	2.993E+13	2784.0494	20680.	0.000
34.040	-0.6196	2164446.	-118394.	-0.004918	0.000	2.993E+13	2809.1380	20131.	0.000
34.410	-0.6414	1667923.	-105867.	-0.004918	0.000	2.993E+13	2833.5703	19614.	0.000
34.780	-0.6633	1227291.	-92080.	-0.004918	0.000	2.993E+13	3176.9098	22606.	0.000
35.150	-0.6851	853230.	-77825.	-0.004918	0.000	2.993E+13	3404.3671	22069.	0.000
35.520	-0.7069	546281.	-61850.	-0.004918	0.000	2.993E+13	3431.1750	21550.	0.000
35.890	-0.7288	306973.	-46558.	-0.004918	0.000	2.993E+13	3457.3683	21064.	0.000
35.260	-0.7506	135822.	-31150.	-0.004918	0.000	2.993E+13	3482.9795	20603.	0.000
35.630	-0.7724	33333.	-15630.	-0.004918	0.000	2.993E+13	3508.0375	20164.	0.000
37.000	-0.7943	0.000	0.000	-0.004918	0.000	2.993E+13	3532.5700	9873.6353	0.000

* This analysis computed pile response using nonlinear moment-curvature relationships. Values of total stress due to combined axial and bending stresses are computed only for elastic sections only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for nonlinear bending properties relative to the magnitude of bending moment developed in the pile.

FC White West (81) - FL 1970

9.990	0.9229	\$135533.	-4587.5433	-0.005873	0.000	4.663E+12	-2628.7272	12647.	0.000
10.360	0.8926	\$1310988.	-16998.	-0.005795	0.000	4.663E+12	-2785.9055	13858.	0.000
10.730	0.8626	\$1210499.	-73617.	-0.005718	0.000	4.664E+12	-2939.0052	15128.	0.000
11.100	0.8329	\$1052049.	-42996	-0.005641	0.000	4.664E+12	-3087.5885	16459.	0.000
11.470	0.8036	\$8832707.	-57024	-0.005564	0.000	4.665E+12	-3231.2444	17853.	0.000
11.840	0.7746	\$6549044.	-71678.	-0.005487	0.000	4.665E+12	-3369.4069	19314.	0.000
12.210	0.7460	\$4290132.	-85912.	-0.005411	0.000	4.666E+12	-3501.9550	20843.	0.000
12.580	0.7177	\$231562.	102752	-0.005335	0.000	4.667E+12	-3628.5318	22448.	0.000
12.950	0.6897	\$9291437.	-149050	-0.005259	0.000	4.668E+12	-3708.2881	23871.	0.000
13.320	0.6621	\$728185.	-135496	-0.005184	0.000	4.670E+12	-3799.9844	24812.	0.000
13.690	0.6348	\$4891971.	-151834	-0.005109	0.000	4.671E+12	-3881.9096	25752.	0.000
14.060	0.6079	\$2333150.	-168170	-0.005035	0.000	4.673E+12	-3954.2579	26592.	0.000
14.430	0.5812	\$662269.	184313	-0.004962	0.000	4.675E+12	-4017.2194	27532.	0.000
14.800	0.5549	\$7530058.	-200271	-0.004889	0.000	4.677E+12	-4070.9794	28573.	0.000
15.170	0.5289	\$4827428.	-216003	-0.004819	0.000	4.680E+12	-4115.7180	29513.	0.000
15.540	0.5032	\$3835468.	-231471	-0.004748	0.000	4.683E+12	-4151.6105	30453.	0.000
15.910	0.4779	\$2275444.	-24634.	-0.004679	0.000	4.685E+12	-4178.8261	31393.	0.000
16.280	0.4528	\$1648791.	-261456.	-0.004610	0.000	4.689E+12	-4297.5285	32333.	0.000
16.650	0.4281	\$945711.	-27898.	-0.004543	0.000	4.692E+12	-4387.8752	33274.	0.000
17.020	0.4036	\$6282171.	-28923.	-0.004477	0.000	4.696E+12	-4510.0172	34214.	0.000
17.390	0.3794	\$7885903.	-304497.	-0.004412	0.000	4.700E+12	-4604.0991	35154.	0.000
17.760	0.3535	\$6510193.	-316582.	-0.004349	0.000	4.704E+12	-4690.2586	36094.	0.000
18.130	0.3319	\$5077887.	-329145.	-0.004287	0.000	4.709E+12	-4768.6263	37035.	0.000
18.500	0.3086	\$3590783.	-344151.	-0.004226	0.000	4.713E+12	-4839.3256	37975.	0.000
18.870	0.2855	\$2051630.	-353565.	-0.004167	0.000	4.719E+12	-4902.4727	38915.	0.000
19.240	0.2627	\$9453127.	-363356.	-0.004109	0.000	4.725E+12	-4958.1761	39855.	0.000
19.610	0.2401	\$3828118.	-373490.	-0.004053	0.000	1.712E+13	-2206.5367	40795.	0.000
19.980	0.2177	\$749606.	-382930.	-0.004001	0.000	2.947E+13	-2045.9802	41736.	0.000
20.350	0.1952	\$5430758.	-391638.	-0.003953	0.000	2.948E+13	-1876.2858	42676.	0.000
20.720	0.1728	\$3674919.	-399571.	-0.003904	0.000	2.950E+13	-1697.4064	43616.	0.000
21.090	0.1504	\$1885616.	-40690.	-0.003857	0.000	2.952E+13	-1509.4135	44556.	0.000
21.460	0.1281	\$9066554.	-412955.	-0.003809	0.000	2.953E+13	-1312.2974	45497.	0.000
21.830	0.1058	\$4821621.	-418323.	-0.003762	0.000	2.955E+13	-1106.0673	46437.	0.000
22.200	0.0835	\$6354880.	-422756.	-0.003714	0.000	2.957E+13	-890.7310	47377.	0.000
22.570	0.0612	\$4470578.	-426213.	-0.003668	0.000	2.959E+13	-666.2953	48317.	0.000
22.940	0.0390	\$2573138.	-428653.	-0.003601	0.000	2.960E+13	-432.7656	49257.	0.000
23.310	0.0168	\$9667165.	-430036.	-0.003549	0.000	2.962E+13	-190.1462	50198.	0.000
23.680	-0.005345	\$3757443.	-430321.	-0.003485	0.000	2.964E+13	61.5596	51138.	0.000
24.050	-0.0275	\$684932.	-429469.	-0.003483	0.000	2.966E+13	322.3497	52078.	0.000
24.420	-0.0496	\$1946773.	-427438.	-0.003478	0.000	2.967E+13	592.2231	53018.	0.000

Ft. White West (E1)-Fl. 1p70
for Lateral Loading for Load Case Number 2

Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 46300.0 lbs
Applied moment at pile head = 76978800.0 in-lbs
Axial thrust load on pile head = 68100.0 lbs

Depth	Deflect.	Bending	Shear	Slope	Total	Bending	Soil Res.	Soil Spr.	Distrib
X	Y	Moment	Force	S	Stress	Stiffness	p	E ^s h	Lat. Load
feet	inches	in-lbs	lbs	radians	psi*	lb-in ²	lb/in	lb/inch	lb/inch
0.00	1.8706	76978800.	46300.	-0.008917	0.000	4.674E+12	0.000	0.000	0.000
0.370	1.8312	77187057.	46300.	-0.008843	0.000	4.674E+12	0.000	0.000	0.000
0.740	1.7921	77395292.	46300.	-0.008770	0.000	4.673E+12	0.000	0.000	0.000
1.110	1.7533	77603504.	46300.	-0.008696	0.000	4.673E+12	0.000	0.000	0.000
1.480	1.7148	77811695.	46300.	-0.008623	0.000	4.672E+12	0.000	0.000	0.000
1.850	1.6767	78019863.	46300.	-0.008548	0.000	4.672E+12	0.000	0.000	0.000
2.220	1.6385	78228008.	46300.	-0.008474	0.000	4.671E+12	0.000	0.000	0.000
2.590	1.6015	78436131.	46300.	-0.008400	0.000	4.670E+12	0.000	0.000	0.000
2.960	1.5643	78644232.	46300.	-0.008325	0.000	4.670E+12	0.000	0.000	0.000
3.330	1.5275	78852310.	46166.	-0.008250	0.000	4.669E+12	0.000	0.000	0.000
3.700	1.4911	79059176.	45755.	-0.008175	0.000	4.669E+12	-60.3070	175.2899	0.000
4.070	1.4549	79263558.	45064.	-0.008100	0.000	4.668E+12	-124.8702	371.8270	0.000
4.440	1.4192	79464246.	44108.	-0.008024	0.000	4.668E+12	-186.2479	568.3641	0.000
4.810	1.3837	79660091.	42900.	-0.007949	0.000	4.667E+12	-244.4847	764.9012	0.000
5.180	1.3486	79850006.	41454.	-0.007873	0.000	4.667E+12	-299.6251	961.4383	0.000
5.550	1.3138	80032965.	39784.	-0.007797	0.000	4.667E+12	-351.7139	1157.9755	0.000
5.920	1.2793	80208000.	37902.	-0.007720	0.000	4.666E+12	-400.7962	1354.5126	0.000
6.290	1.2452	80374201.	35822.	-0.007644	0.000	4.666E+12	-446.9168	1551.0497	0.000
6.660	1.2115	80530717.	33556.	-0.007568	0.000	4.665E+12	-490.1211	1747.5868	0.000
7.030	1.1780	80676753.	31117.	-0.007491	0.000	4.665E+12	-530.4544	1944.1240	0.000
7.400	1.1449	80811569.	28518.	-0.007414	0.000	4.665E+12	-567.9621	2140.6611	0.000
7.770	1.1122	809334481.	25772.	-0.007337	0.000	4.665E+12	-602.6897	2337.1982	0.000
8.140	1.0798	810444857.	22888.	-0.007260	0.000	4.664E+12	-634.6849	2535.7353	0.000
8.510	1.0477	81142121.	19881.	-0.007183	0.000	4.664E+12	-663.9873	2730.2724	0.000
8.880	1.0160	81225746.	16761.	-0.007105	0.000	4.664E+12	-690.6465	2926.8095	0.000
9.250	0.9846	81295258.	13540.	-0.007028	0.000	4.663E+12	-714.7127	3125.3467	0.000
9.620	0.9536	81350233.	6427.0051	-0.006951	0.000	4.663E+12	-736.2254	3319.8838	0.000

Ft White West (B1)-Fl. 1p70

Pile-head deflection = 0.5696214 inches
 Computed slope at pile head = -0.0020331 radians
 Maximum bending moment = 40858917. inch-lbs
 Maximum shear force = -207070. lbs
 Depth of maximum bending moment = 10.3600000 feet below pile head
 Depth of maximum shear force = 25.9000000 feet below pile head
 Number of iterations = 22
 Number of zero deflection points = 1

Pile-head Deflection vs. Pile length for load Case 1

Boundary Condition Type 1, Shear and Moment

Shear = 23150. lb
 Moment = 38489400. in-lb
 Axial load = 68100. lb

Pile Length feet	Pile Head Deflection inches	Maximum Moment In-lbs	Maximum Shear lbs
37.0000	0.5696214	40858917.	-207070.
35.1500	0.7442185	40756470.	-222215.
33.3000	0.8973385	40657925.	-236799.
31.4500	1.0576384	40573577.	-256375.
29.6000	1.1891559	40506291.	-267275.
27.7500	1.5264730	40375926.	-288854.
25.9000	2.1087076	40234818.	-320294.
24.0500	3.3362940	40081302.	-361973.
22.2000	5.9177757	39987835.	-404454.
20.3500	10.9020022	40336089.	-494109.

				- Ft White West (E1)-Fl.jp70					
26.270	-0.006651	16736605.	-206895.	-0.001658	0.000	2.984E+13	86.4589	57713.	0.000
26.640	-0.0140	15819329.	-206296.	-0.001655	0.000	2.985E+13	185.0436	58650.	0.000
27.010	-0.0214	14905700.	-205245.	-0.001653	0.000	2.986E+13	286.6033	59600.	0.000
27.380	-0.0287	13997720.	-203744.	-0.001651	0.000	2.987E+13	391.1396	60540.	0.000
27.750	-0.0360	13092450.	-201769.	-0.001649	0.000	2.987E+13	498.6544	61492.	0.000
28.120	-0.0433	12207010.	-199309.	-0.001647	0.000	2.988E+13	609.1504	62421.	0.000
28.490	-0.0506	11326578.	-196355.	-0.001645	0.000	2.988E+13	722.6305	63361.	0.000
28.860	-0.0579	10464391.	-192886.	-0.001644	0.000	2.989E+13	839.0979	64301.	0.000
29.230	-0.0652	9616745.	-188895.	-0.001642	0.000	2.990E+13	958.5563	65241.	0.000
29.600	-0.0725	8787996.	-183119.	-0.001641	0.000	2.991E+13	1083.1936	66181.	0.000
29.970	-0.0798	7991639.	-175735.	-0.001640	0.000	2.992E+13	1212.9726	67121.	0.000
30.340	-0.0871	7226460.	-168180.	-0.001639	0.000	2.993E+13	1347.9895	68061.	0.000
30.710	-0.0944	6496189.	-160466.	-0.001637	0.000	2.993E+13	1488.261	69001.	0.000
31.080	-0.1016	5804514.	-152601.	-0.001637	0.000	2.993E+13	1633.847	70041.	0.000
31.450	-0.1089	5145082.	-144594.	-0.001636	0.000	2.993E+13	1784.9094	71081.	0.000
31.820	-0.1161	4521507.	-136452.	-0.001635	0.000	2.993E+13	1941.7231	72121.	0.000
32.190	-0.1234	3934373.	-128182.	-0.001634	0.000	2.993E+13	2104.5779	73161.	0.000
32.560	-0.1307	3384236.	-119790.	-0.001634	0.000	2.993E+13	2272.7179	74201.	0.000
32.930	-0.1379	2871627.	-111280.	-0.001633	0.000	2.993E+13	2446.4368	75241.	0.000
33.300	-0.1452	2397058.	-102657.	-0.001633	0.000	2.993E+13	2625.9801	76281.	0.000
33.670	-0.1524	1961018.	-93926.	-0.001633	0.000	2.993E+13	2810.5495	77321.	0.000
34.040	-0.1597	1563981.	-85092.	-0.001632	0.000	2.993E+13	2999.2851	78361.	0.000
34.410	-0.1669	1206401.	-76154.	-0.001632	0.000	2.993E+13	3192.3851	79401.	0.000
34.780	-0.1742	888719.	-66294.	-0.001632	0.000	2.993E+13	3389.9807	80441.	0.000
35.150	-0.1814	618693.	-55906.	-0.001632	0.000	2.993E+13	3592.225	81481.	0.000
35.520	-0.1887	396811.	-44610.	-0.001632	0.000	2.993E+13	3800.1607	82521.	0.000
35.890	-0.1959	223547.	-33608.	-0.001632	0.000	2.993E+13	4013.8456	83561.	0.000
36.260	-0.2031	99360.	-22504.	-0.001632	0.000	2.993E+13	4233.3512	84601.	0.000
36.630	-0.2104	24700.	-11300.	-0.001632	0.000	2.993E+13	4457.7571	85641.	0.000
37.000	-0.2176	0.000	0.000	-0.001632	0.000	2.993E+13	4687.1630	86681.	0.000

* This analysis computed pile response using nonlinear moment-curvature relationships
 Values of total stress due to combined axial and bending stresses are computed only
 for elastic sections only and do not equal the actual stresses in concrete and steel.
 Stresses in concrete and steel may be interpolated from the output for nonlinear
 bending properties relative to the magnitude of bending moment developed in the pile.

11.470	0.3024	40714808.	-19894.	-0.001848	0.000	2.962E+13	-1369	7727	20111.	0.000
11.840	0.2942	40613535.	-26032.	-0.001841	0.000	2.962E+13	-1394	9847	21051.	0.000
12.210	0.2861	40484759.	-32274.	-0.001835	0.000	2.962E+13	-1416	8521	21991.	0.000
12.580	0.2779	40328051.	-38606.	-0.001829	0.000	2.962E+13	-1435	4217	22931.	0.000
12.950	0.2698	40143043.	-45013.	-0.001823	0.000	2.963E+13	-1450	6799	23871.	0.000
13.320	0.2617	39929435.	-51481.	-0.001817	0.000	2.963E+13	-1462	6532	24812.	0.000
13.690	0.2537	39686991.	-57994.	-0.001811	0.000	2.963E+13	-1471	3576	25752.	0.000
14.060	0.2457	39415540.	-64539.	-0.001805	0.000	2.963E+13	-1476	8091	26692.	0.000
14.430	0.2377	39114974.	-71101.	-0.001800	0.000	2.964E+13	-1479	0233	27632.	0.000
14.800	0.2297	38785249.	-77666.	-0.001794	0.000	2.964E+13	-1478	0155	28573.	0.000
15.170	0.2217	38426385.	-84219.	-0.001788	0.000	2.964E+13	-1473	8008	29513.	0.000
15.540	0.2138	38038466.	-90746.	-0.001782	0.000	2.965E+13	-1466	3940	30453.	0.000
15.910	0.2059	37621637.	-97233.	-0.001777	0.000	2.965E+13	-1455	8294	31393.	0.000
16.280	0.1980	37176107.	-103667.	-0.001771	0.000	2.966E+13	-1442	0510	32333.	0.000
16.650	0.1902	36702147.	-110032.	-0.001765	0.000	2.966E+13	-1425	1526	33274.	0.000
17.020	0.1823	36200091.	-116315.	-0.001760	0.000	2.966E+13	-1405	1272	34214.	0.000
17.390	0.1745	35670333.	-122503.	-0.001755	0.000	2.967E+13	-1381	9678	35154.	0.000
17.760	0.1668	35113329.	-128580.	-0.001749	0.000	2.967E+13	-1355	6968	36094.	0.000
18.130	0.1590	34529598.	-134534.	-0.001744	0.000	2.968E+13	-1326	3260	37035.	0.000
18.500	0.1513	33919720.	-140351.	-0.001739	0.000	2.968E+13	-1293	8671	37975.	0.000
18.870	0.1436	33284332.	-146017.	-0.001734	0.000	2.969E+13	-1258	3308	38915.	0.000
19.240	0.1359	32624137.	-151518.	-0.001729	0.000	2.969E+13	-1219	7279	39855.	0.000
19.610	0.1282	31939896.	-156841.	-0.001724	0.000	2.970E+13	-1178	0681	40795.	0.000
19.980	0.1206	31232428.	-161973.	-0.001719	0.000	2.971E+13	-1133	3609	41736.	0.000
20.350	0.1129	30502617.	-166899.	-0.001715	0.000	2.972E+13	-1085	6154	42676.	0.000
20.720	0.1053	29751403.	-171606.	-0.001710	0.000	2.972E+13	-1034	8397	43616.	0.000
21.090	0.0978	28979788.	-176082.	-0.001706	0.000	2.973E+13	-981	0418	44556.	0.000
21.460	0.0902	28188831.	-180311.	-0.001702	0.000	2.974E+13	-924	2288	45497.	0.000
21.830	0.0826	27379653.	-184282.	-0.001697	0.000	2.974E+13	-864	4075	46437.	0.000
22.200	0.0751	26553433.	-187981.	-0.001693	0.000	2.975E+13	-801	5839	47377.	0.000
22.570	0.0676	25711410.	-191393.	-0.001690	0.000	2.976E+13	-735	7636	48317.	0.000
22.940	0.0601	24854881.	-194507.	-0.001686	0.000	2.977E+13	-666	9516	49257.	0.000
23.310	0.0526	23985203.	-197309.	-0.001682	0.000	2.977E+13	-595	1521	50198.	0.000
23.680	0.0452	23103792.	-199786.	-0.001679	0.000	2.978E+13	-520	3689	51138.	0.000
24.050	0.0377	22212121.	-201924.	-0.001675	0.000	2.979E+13	-442	6053	52078.	0.000
24.420	0.0303	21311723.	-203710.	-0.001672	0.000	2.980E+13	-361	8638	53018.	0.000
24.790	0.0229	20404191.	-205130.	-0.001669	0.000	2.981E+13	-278	1465	53959.	0.000
25.160	0.0155	19491175.	-206173.	-0.001666	0.000	2.981E+13	-191	4550	54899.	0.000
25.530	0.008094	18574384.	-206824.	-0.001663	0.000	2.982E+13	-101	7900	55839.	0.000
25.900	0.000716	17655585.	-207070.	-0.001660	0.000	2.983E+13	-9	1521	56779.	0.000

Ft White West (B1)-Fl. 1P70

shear force at pile head
 applied moment at pile head
 axial thrust at pile head

White West ()-FL. 770
 1150.4 lbs
 384.1400.4 lb-ft
 1100.4 lbs

Def	Y	Height	Shear Force	Moment	axial	Stress	Strain	Stress	Strain	Stress	Strain	Stress	Strain	Stress	Strain	Stress	Strain	Stress	Strain
1	0.00	0.566	384.1	100.0	231.1	-0.0	0.22033	0	0	0	0	0	0	0	0	0	0	0	0
2	0.370	0.566	385.1	100.0	231.1	-0.0	0.22027	0	0	0	0	0	0	0	0	0	0	0	0
3	0.740	0.551	386.1	198.0	231.1	-0.0	0.22022	0	0	0	0	0	0	0	0	0	0	0	0
4	1.100	0.544	387.1	294.0	231.1	-0.0	0.22016	0	0	0	0	0	0	0	0	0	0	0	0
5	1.460	0.531	389.1	389.0	231.1	-0.0	0.22010	0	0	0	0	0	0	0	0	0	0	0	0
6	1.820	0.522	390.1	482.0	231.1	-0.0	0.22004	0	0	0	0	0	0	0	0	0	0	0	0
7	2.180	0.511	391.1	573.0	231.1	-0.0	0.21998	0	0	0	0	0	0	0	0	0	0	0	0
8	2.540	0.507	392.1	662.0	231.1	-0.0	0.21992	0	0	0	0	0	0	0	0	0	0	0	0
9	2.900	0.494	393.1	750.0	231.1	-0.0	0.21986	0	0	0	0	0	0	0	0	0	0	0	0
10	3.260	0.481	394.1	835.0	231.1	-0.0	0.21981	0	0	0	0	0	0	0	0	0	0	0	0
11	3.620	0.481	395.1	919.0	229.1	-0.0	0.21975	0	0	0	0	0	0	0	0	0	0	0	0
12	3.980	0.477	396.1	1002.0	227.1	-0.0	0.21969	0	0	0	0	0	0	0	0	0	0	0	0
13	4.340	0.466	397.1	1084.0	224.1	-0.0	0.21963	0	0	0	0	0	0	0	0	0	0	0	0
14	4.700	0.451	398.1	1165.0	220.1	-0.0	0.21957	0	0	0	0	0	0	0	0	0	0	0	0
15	5.060	0.441	399.1	1245.0	215.1	-0.0	0.21951	0	0	0	0	0	0	0	0	0	0	0	0
16	5.420	0.437	400.1	1324.0	210.1	-0.0	0.21945	0	0	0	0	0	0	0	0	0	0	0	0
17	5.780	0.421	401.1	1402.0	203.1	-0.0	0.21939	0	0	0	0	0	0	0	0	0	0	0	0
18	6.140	0.411	402.1	1479.0	196.1	-0.0	0.21933	0	0	0	0	0	0	0	0	0	0	0	0
19	6.500	0.41	402.1	1556.0	189.1	-0.0	0.21927	0	0	0	0	0	0	0	0	0	0	0	0
20	6.860	0.401	403.1	1632.0	180.1	-0.0	0.21921	0	0	0	0	0	0	0	0	0	0	0	0
21	7.220	0.391	404.1	1708.0	171.1	-0.0	0.21915	0	0	0	0	0	0	0	0	0	0	0	0
22	7.580	0.381	405.1	1784.0	162.1	-0.0	0.21909	0	0	0	0	0	0	0	0	0	0	0	0
23	7.940	0.371	405.1	1860.0	152.1	-0.0	0.21903	0	0	0	0	0	0	0	0	0	0	0	0
24	8.300	0.361	406.1	1936.0	141.1	-0.0	0.21896	0	0	0	0	0	0	0	0	0	0	0	0
25	8.660	0.361	407.1	2012.0	130.1	-0.0	0.21890	0	0	0	0	0	0	0	0	0	0	0	0
26	9.020	0.351	407.1	2088.0	119.1	-0.0	0.21884	0	0	0	0	0	0	0	0	0	0	0	0
27	9.380	0.341	408.1	2164.0	704.5	-0.0	0.21878	0	0	0	0	0	0	0	0	0	0	0	0
28	9.740	0.331	408.1	2240.0	313.6	-0.0	0.21872	0	0	0	0	0	0	0	0	0	0	0	0
29	10.100	0.321	408.1	2316.0	256.7	-0.0	0.21866	0	0	0	0	0	0	0	0	0	0	0	0
30	10.460	0.311	408.1	2392.0	991.5	-0.0	0.21860	0	0	0	0	0	0	0	0	0	0	0	0
31	10.820	0.311	407.1	2468.0	-138.1	-0.0	0.21854	0	0	0	0	0	0	0	0	0	0	0	0

1	3.100	18,137.91	0.130000
2	19.800	18,159.73	0.130000

Note: The values in moment capacity in the table above are factors of safety by reduction factor (phi-factor).

In ACI 318-08 the value of the strength reduction factor (phi) is 0.75 for spiral reinforcement (0.7).

The above values should be multiplied by the appropriate strength reduction factor to compute ultimate moment capacity in accordance with the following table presented for common practice and corresponding design stresses and concrete action.

Load No.	Reaction Moment	Normal Capacity kip	Utilite (Factored) kip	Ultimate Factor (d)	Design Moment (kip-in)	Design Stiffness (kip-in ²)
1	3.65	1,007.9	4.265	11.05.13	4596.11803.09	15834.14
2	3.65	1,859.7	5.020	11.58.82	4616.19211.13	12541.17
1	3.70	1,007.9	4.670	12.05.53	4576.115834.14	15834.14
2	3.70	1,859.7	6.560	12.01.80	4595.12541.17	12541.17
1	3.75	1,007.9	5.075	13.05.93	4454.118708.14	15834.14
2	3.75	1,859.7	6.100	13.44.75	4486.112620.14	12620.14

Computed values for lateral loading and base number 1

Primary	f Res	ts fo	Nomin	(Unf	tored	Momen	Capac	y for	ectic	1							
0.0000	7	1725	..	2	152711	1.	1.8693	16	0.001	11	3.006	19	..	0.0000	10	C	
0.0000	7	1734	1.	2	162415	1.	1.7386	13	0.001	109	0.006	131	1.8573	12	0.0000	10	C
0.0000	7	1735	1.	2	174865	1.	1.6111	11	0.001	125	0.006	145	1.8807	19	0.0000	10	C
0.0000	7	1744	1.	2	173206	1.	1.4921	18	0.001	112	0.006	138	1.9021	15	0.0000	10	C
0.0000	7	1745	1.	2	195521	1.	1.3801	16	0.001	131	0.007	29	1.9214	10	0.0000	10	C
0.0000	7	1751	1.	2	140088	1.	1.2756	15	0.001	130	0.007	70	1.9387	12	0.0000	10	C
0.0000	7	1755	1.	2	164781	1.	1.1756	15	0.001	121	0.007	109	1.9536	16	0.0000	10	C
0.0000	7	1761	1.	2	187055	1.	1.0806	139	0.001	121	0.007	149	1.9665	10	0.0000	10	C
0.0000	7	1764	1.	2	157709	1.	0.9811	135	0.001	134	0.007	196	1.9775	115	0.0000	10	C
0.0000	7	1766	1.	2	128278	1.	0.8781	70	0.001	139	0.008	51	1.9856	13	0.0000	10	C
0.0000	7	1771	1.	2	180470	1.	0.7811	143	0.001	145	0.008	105	1.9921	18	0.0000	10	C
0.0000	7	1771	1.	2	154200	1.	0.6881	75	0.001	152	0.008	158	1.9961	12	0.0000	10	C
0.0000	7	1771	1.	2	193731	1.	0.6001	16	0.001	150	0.008	110	1.9991	18	0.0000	10	C
0.0000	7	1781	1.	2	135896	1.	0.5181	139	0.001	159	0.008	161	1.9971	132	0.0000	10	C
0.0000	7	1781	1.	2	136811	1.	0.4401	135	0.001	180	0.009	110	1.9931	153	0.0000	10	C
0.0000	7	1781	1.	2	122583	1.	0.3641	56	0.002	191	0.009	159	1.9971	139	0.0000	10	C
0.0000	7	1781	1.	2	122583	1.	0.2901	37	0.002	100	0.009	110	1.9991	13	0.0000	10	C
0.0000	7	1791	1.	2	179111	1.	0.2201	94	0.002	110	0.009	160	1.9961	139	0.0000	10	C
0.0000	7	1791	1.	2	179111	1.	0.1491	78	0.002	117	0.009	113	1.9921	178	0.0000	10	C
0.0000	7	1791	1.	2	347571	1.	0.0774	94	0.002	158	0.010	132	1.9951	138	0.0000	10	C
0.0000	7	1801	1.	2	182554	1.	0.4651	143	0.002	98	0.011	152	1.9951	148	0.0000	10	C
0.0000	7	1811	1.	2	181115	1.	0.1941	144	0.002	118	0.013	192	1.9881	137	0.0000	10	C
0.0000	7	1811	1.	2	148515	1.	0.9811	199	0.002	166	0.014	104	1.9951	135	0.0000	10	C
0.0000	7	1821	1.	2	156707	1.	0.8151	87	0.003	141	0.015	189	1.9961	175	0.0000	10	C
0.0000	7	1821	1.	2	177568	1.	0.7637	190	0.003	157	0.016	33	1.9861	110	0.0000	10	C
0.0000	7	1831	1.	2	107910	1.	0.4821	91	0.003	173	0.017	77	1.9941	114	0.0000	10	C
0.0000	7	1831	1.	2	146455	1.	0.7352	177	0.003	103	0.018	07	1.9941	138	0.0000	10	C
0.0000	7	1831	1.	2	192051	1.	0.7245	191	0.003	153	0.019	117	1.9811	143	0.0000	10	C
0.0000	7	1831	1.	2	143544	1.	0.2841	91	0.003	112	0.021	118	1.9991	137	0.0000	10	C
0.0000	7	1831	1.	2	199485	1.	0.003	91	0.003	112	0.021	118	1.9991	137	0.0000	10	C

Maximum level of development at maximum impression velocity in = 003
 Primary Resists for Nomin (Unf tored Momen Capac y for ectic 1
 Minimum level of development at maximum impression velocity in = 003
 Lo A | al Th | ist Nc | nal M | l. Caf | | X. Ccl | |
 N Kips in-k | | Straj |
 Pa | 18

		F		White		rest		() - FL		() 70				
1.0000	17	96756	466	152238	24	126645	4	100515	-1	101721	351811	-4	72462	C
1.0000	12	98978	465	184055	24	129966	4	100527	-1	101761	389611	-5	320751	C
1.0000	17	01197	465	151336	24	133903	4	100544	-1	101801	327121	-5	116411	C
1.0000	12	03415	464	144326	24	138415	4	100551	-1	101851	364341	-5	311571	C
1.0000	17	05625	464	154147	24	143487	4	100561	-1	101891	301281	-5	50624	C
1.0000	12	07842	463	172688	24	149067	4	100577	-1	101931	337931	-5	70042	C
1.0000	17	10055	463	192494	24	155134	4	100594	-1	101971	374291	-5	89410	C
1.0000	12	12261	462	106721	24	161661	4	100601	-1	102011	110361	-5	38728	C
1.0000	17	14467	462	109095	24	168634	4	100611	-1	10205	14614	-5	27995	C
1.0000	12	16676	462	159379	24	176024	4	100621	-1	102091	18163	-6	00000	CY
1.0000	17	18871	461	135461	24	183811	4	100641	-1	10214	21683	-6	00000	CY
1.0000	12	21076	461	189151	24	191991	4	100651	-1	10218	25173	-6	00000	CY
1.0000	17	23266	461	190261	24	19053	4	100661	-1	10222	28634	-6	00000	CY
1.0000	12	25466	461	195454	24	199421	4	100671	-1	10226	32065	-6	00000	CY
1.0000	17	27651	461	178011	24	118641	4	10069	-1	10230	35467	-6	00000	CY
1.0000	12	29785	451	299851	24	124881	4	100701	-1	10234	38813	-6	00000	CY
1.0000	17	31681	451	184290	24	115501	4	100711	-1	10238	42015	-6	00000	CY
1.0000	12	33333	451	159170	24	139389	4	100721	-1	10243	45072	-6	00000	CY
1.0000	17	34884	451	110821	24	165781	4	100731	-1	10247	48051	-6	00000	CY
1.0000	12	36004	441	149451	24	170511	4	100781	-1	10264	59222	-6	00000	CY
1.0000	17	37801	421	133669	24	110441	4	100821	-1	10281	69533	-6	00000	CY
1.0000	12	39462	411	194621	24	130025	4	10086	-1	10299	79141	-6	00000	CY
1.0000	17	41770	391	174375	24	171821	4	10090	-1	10316	88039	-6	00000	CY
1.0000	12	45065	381	189571	24	134166	4	10094	-1	10334	96385	-6	00000	CY
1.0000	17	45451	371	148121	24	152611	4	10098	-1	10352	04313	-6	00000	CY
1.0000	12	47421	351	175271	24	140574	4	10102	-1	10370	11770	-6	00000	CY
1.0000	17	49241	341	170763	24	120017	4	101061	-1	10387	18857	-6	00000	CY
1.0000	12	51780	331	11780	24	138424	4	10109	-1	10405	25407	-6	00000	CY
1.0000	17	53161	321	180842	24	178796	4	10113	-1	10423	31701	-6	00000	CY
1.0000	12	54964	311	17161	24	158786	4	10116	-1	10442	37553	-6	00000	CY
1.0000	17	54964	301	154964	24	140283	4	10120	-1	10460	43145	-6	00000	CY
1.0000	12	54964	291	160382	24	123118	4	10123	-1	10478	48475	-6	00000	CY
1.0000	17	54964	281	161555	24	105423	4	10127	-1	10496	53399	-6	00000	CY
1.0000	12	54964	281	166576	24	187936	4	10130	-1	10514	57996	-6	00000	CY
1.0000	17	54964	271	170220	24	171770	4	10134	-1	10532	62378	-6	00000	CY
1.0000	12	54964	261	16972	24	156407	4	10140	-1	10551	66486	-6	00000	CY
1.0000	17	54964	251	17032	24	141222	4	10144	-1	10587	73879	-6	00000	CY
1.0000	12	54964	241	17168	24	141425	4	10147	-1	10605	77234	-6	00000	CY
1.0000	17	54964	241	17234	24	100660	4	10150	-1	10624	80336	-6	00000	CY

Br. ing Cul. ture rll/in.	Br. ing Mllment ll/in	Br. ing St k	Br. ing fness k-in2	D. th to Axis in	M. Comp rain /in	M. Tens rain /in	Max oncrel ress sl	Ma Steel ress sl	Run Msg
0.00107	78338	165	05223	15	02064	-0.009577	39891	-60.00000	CY
0.00109	78536	162	05935	15	02094	-0.009751	39135	-60.00000	CY
0.00121	79621	147	023097	15	00227	-0.01087	39470	-60.00000	CY
0.00133	80536	134	05655	15	00246	-0.01198	39369	-60.00000	CY
0.00145	81092	124	085794	15	00264	-0.01309	38666	-60.00000	CY
0.00157	81598	115	07425	15	00282	-0.01420	39864	-60.00000	CY
0.00166	82046	107	098396	15	00301	-0.01531	39531	-60.00000	CY
0.00181	82364	106	049832	15	00319	-0.01643	38996	-60.00000	CY
0.00192	82494	94	04898	15	00337	-0.01754	99276	-60.00000	CY
0.00206	82662	81	08866	15	00355	-0.01866	99836	-60.00000	CY
0.00217	82807	80	02674	15	00374	-0.01977	97720	-60.00000	CY
0.00225	82880	79	07768	15	00395	-0.02085	99844	-60.00000	CY

axial trust force = 90000 k

Br. ing Cul. ture rll/in.	Br. ing Mllment ll/in	Br. ing St k	Br. ing fness k-in2	D. th to Axis in	M. Comp rain /in	M. Tens rain /in	Max oncrel ress sl	Ma Steel ress sl	Run Msg	
0.00002	748	35307	299	41230	6	98295	00001	06593	45271	
0.00005	1493	298	00762	5	50443	-0.00002	12197	84048		
0.00007	2234	297	026076	5	01168	-0.00003	17761	22825		
0.00010	2972	297	052420	5	26535	-0.00005	23284	61603		
0.00012	3707	296	03129	5	81758	-0.00006	28768	00381		
0.00015	4438	295	06018	5	51910	-0.00007	34211	39159		
0.00017	5166	295	04432	5	30592	-0.00009	39614	77937		
0.00020	5890	294	00048	5	14606	-0.00011	44977	16715		
0.00022	5890	261	08932	2	21159	-0.00006	25976	17095		
0.00025	5890	235	08039	2	81041	-0.00006	28392	77455		
0.00027	5890	214	07308	2	48067	-0.00007	30804	37835		
0.00030	5890	196	06699	2	19657	-0.00008	33197	98291		
0.00032	5890	181	07722	2	95713	-0.00008	35584	58735		
0.00035	5890	168	05742	2	75279	-0.00005	37964	19178		
0.00037	5890	157	05359	2	57653	-0.00005	40337	79607		
0.00040	5890	147	03024	2	42133	-0.00016	42705	40046		
0.00042	5890	138	04728	2	28026	-0.00017	45051	00546		
0.00045	5890	130	04466	2	15558	-0.00017	47394	61021		
0.00047	5890	124	09494	2	04471	-0.00017	49736	21495		
0.00050	5890	117	04015	2	94556	-0.00017	52064	81956		

1.0000	12	32642	453	003835	24	788837	000720	-01	30243	44260	-6	00000	CY
1.0000	17	34173	451	02244	24	761286	000730	-01	30247	47237	-6	00000	CY
1.0000	7	139316	436	02911	24	505001	00078	-01	30264	58421	-6	00000	CY
1.0000	17	143471	425	095607	24	114081	00082	-01	30282	68744	-6	00000	CY
1.0000	17	146925	414	012764	24	201931	00086	-01	30299	78320	-6	00000	CY
1.0000	17	149878	397	175135	24	377930	00090	-01	30317	87242	-6	00000	CY
1.0000	17	152375	383	136891	24	751031	00094	-01	30334	95606	-6	00000	CY
1.0000	17	154651	374	154881	24	534890	00098	-01	30352	10351	-6	00000	CY
1.0000	17	156621	357	064621	24	32152	00102	-01	30370	11034	-6	00000	CY
1.0000	17	158421	344	349250	24	11234	00105	-01	30388	18088	-6	00000	CY
1.0000	17	159974	331	123752	24	399660	00109	-01	30406	24665	-6	00000	CY
1.0000	17	161510	324	143817	24	70634	00112	-01	30424	30987	-6	00000	CY
1.0000	17	162740	314	173603	24	507980	00116	-01	30442	36859	-6	00000	CY
1.0000	17	163941	301	194101	24	32555	00120	-01	30460	42479	-6	00000	CY
1.0000	17	165110	291	176391	24	15606	00123	-01	30478	47836	-6	00000	CY
1.0000	17	166081	28	091751	24	97481	00126	-01	30496	52733	-6	00000	CY
1.0000	17	166991	271	195414	24	80223	00130	-01	30515	57361	-6	00000	CY
1.0000	17	167901	27	06806	24	64270	00133	-01	30533	61765	-6	00000	CY
1.0000	17	168741	261	022171	24	49053	00137	-01	30551	69756	-6	00000	CY
1.0000	17	169471	251	159116	24	34004	00140	-01	30569	73360	-6	00000	CY
1.0000	17	170151	251	155801	24	19659	00143	-01	30588	79848	-6	00000	CY
1.0000	17	170831	241	123312	24	06299	00146	-01	30606	82697	-6	00000	CY
1.0000	17	171481	231	076451	24	92753	00150	-01	30624	85295	-6	00000	CY
1.0000	17	172091	231	153261	24	79703	00153	-01	30643	87672	-6	00000	CY
1.0000	17	172611	221	178674	24	66695	00156	-01	30661	91816	-6	00000	CY
1.0000	17	173091	221	134235	24	54162	00155	-01	30679	93586	-6	00000	CY
1.0000	17	173571	211	147135	24	42396	00162	-01	30698	95134	-6	00000	CY
1.0000	17	174041	211	08393	24	31341	00166	-01	30716	96466	-6	00000	CY
1.0000	17	174511	201	170839	24	209491	00165	-01	30735	97556	-6	00000	CY
1.0000	17	174961	201	142628	24	11123	00172	-01	30753	98426	-6	00000	CY
1.0000	17	175391	191	177643	24	01625	00175	-01	30772	99124	-6	00000	CY
1.0000	17	175721	191	090281	24	91217	00178	-01	30790	99606	-6	00000	CY
1.0000	17	176041	191	169312	24	81004	00181	-01	30809	99907	-6	00000	CY
1.0000	17	176351	181	103261	24	71334	00184	-01	30827	99981	-6	00000	CY
1.0000	17	176661	181	04251	24	62186	00187	-01	30846	99990	-6	00000	CY
1.0000	17	176961	181	141629	24	53536	00190	-01	30864	99991	-6	00000	CY
1.0000	17	177261	171	137341	24	45339	00194	-01	30883	99993	-6	00000	CY
1.0000	17	177561	171	110688	24	37622	00197	-01	30901	99993	-6	00000	CY
1.0000	137	17784	171	113626	24	30144	00200	-01	30920	99994	-6	00000	CY
1.0000	157	17809	161	108178	24	22788	00206	-01	30938	99993	-6	00000	CY

0.000009500	38906	32066	127.	14.812	169	0.001	157	0.006	103	1.9182	11	1.769	18	C						
0.000009750	38906	30416	177.	14.796	154	0.001	118	0.006	112	1.9402	19	1.372	12	C						
0.0000102	38906	37469	186.	14.772	136	0.001	139	0.006	131	1.9846	10	1.578	16	C						
0.0000107	38906	34796	147.	14.751	117	0.001	161	0.006	149	1.0276	12	1.784	14	C						
0.0000112	38906	32360	107.	14.731	117	0.001	182	0.006	168	1.0716	17	1.598	12	C						
0.0000117	38906	30132	100.	14.716	188	0.001	104	0.006	186	1.1146	16	1.394	18	C						
0.0000122	38906	18086	172.	14.701	141	0.001	126	0.006	104	1.1561	12	1.399	19	C						
0.0000127	30247	17252	192.	14.691	122	0.001	148	0.006	122	1.1994	15	1.603	17	C						
0.0000133	32503	17172	197.	14.681	108	0.001	171	0.006	139	1.2411	18	1.806	16	C						
0.0000138	54757	17096	107.	14.671	229	0.001	193	0.006	157	1.2836	10	1.010	18	C						
0.0000143	57099	17024	242.	14.671	106	0.001	115	0.006	175	1.3254	13	1.212	15	C						
0.0000148	59260	16955	157.	14.661	102	0.001	138	0.006	192	1.3661	10	1.415	11	C						
0.0000153	71508	16890	1504.	14.661	183	0.001	161	0.006	109	1.4081	19	1.617	15	C						
0.0000157	73754	16827	1390.	14.661	185	0.001	184	0.006	126	1.4481	13	1.818	10	C						
0.0000162	75997.	16767	1293.	14.651	187	0.001	107	0.006	143	1.4891	13	1.019	16	C						
0.0000167	78239.	16709	1496.	14.661	191	0.001	131	0.006	159	1.5291	19	1.220	14	C						
0.0000172	80479.	16654	1026.	14.661	106	0.001	154	0.006	176	1.5701	11	1.420	14	C						
0.0000177	82716.	16600	1265.	14.661	106	0.001	178	0.006	192	1.6091	12	1.619	14	C						
0.0000182	84952.	16548	1629.	14.661	130	0.001	101	0.006	209	1.6491	10	1.818	14	C						
0.0000187	87185.	16498	1294.	14.661	136	0.001	125	0.006	125	1.6881	15	1.017	15	C						
0.0000192	89416.	16450	1970.	14.671	107	0.001	150	0.006	140	1.7271	13	1.6215	17	C						
0.0000197	91645.	16402	1707.	14.671	135	0.001	174	0.006	156	1.7661	12	1.413	16	C						
0.0000202	93872.	16356	1726.	14.681	136	0.001	198	0.006	172	1.8041	10	1.610	14	C						
0.0000207	96097.	16311	1280.	14.681	147	0.001	123	0.006	187	1.8421	15	1.807	12	C						
0.0000212	98319.	16267	536.	14.691	139	0.001	148	0.006	170	1.8801	18	1.004	15	C						
0.0000217	100539.	16224	1462.	14.701	115	0.001	173	0.006	117	1.9181	15	1.219	12	C						
0.0000222	102757.	16183	1743.	14.701	103	0.001	198	0.006	132	1.9551	19	1.395	11	C						
0.0000227	104973.	16141	1700.	14.711	189	0.001	123	0.006	147	1.9921	16	1.490	15	C						
0.0000232	107186.	16101	1224.	14.721	107	0.001	148	0.006	162	2.0291	16	1.578	14	C						
0.0000237	109398.	16061	1709.	14.731	104	0.001	174	0.006	176	2.0651	16	1.978	12	C						
0.0000242	111607.	16021	1010.	14.741	181	0.001	100	0.006	190	2.1011	18	1.171	10	C						
0.0000247	113813.	15981	1386.	14.751	148	0.001	126	0.006	104	2.1371	18	1.364	14	C						
0.0000252	116017.	15941	1469.	14.761	137	0.001	152	0.006	118	2.1731	22	1.000	10	C						
0.0000257	118219.	15916	1221.	14.771	104	0.001	178	0.006	132	2.2081	10	1.000	10	C						
0.0000262	120419.	15887	1907.	14.771	116	0.001	150	0.006	145	2.2431	14	1.000	10	C						
0.0000267	122616.	15837	1063.	14.791	145	0.001	131	0.006	129	2.2781	18	1.000	10	C						
0.0000272	124811.	15801	1472.	14.801	189	0.001	175	0.006	157	2.3121	14	1.000	10	C						
0.0000277	127004.	15761	1145.	14.811	105	0.001	188	0.006	185	2.3461	18	1.000	10	C						
0.0000282	129133.	15716	1285.	14.811	139	0.001	111	0.006	199	2.3801	12	1.000	10	C						
0.0000287	131007.	15561	1710.	14.811	135	0.001	133	0.006	117	2.4121	16	1.000	10	C						

Curvati e	M:me	ti f	s	t	t	B	I	l	h	k	s	st	s
rad/ir	i - l	k ip				n	l		ln	k		k	
0.00004 2:0	81.1	118	5:7	4.	€1.	0.	M:12	3.	3:18	3.	5:26	3.	4:30
0.00004 5:0		31.	8:0	€1.	€1.	0.	M:17	3.	3:53	3.	5:25	3.	1:54
0.00004 7:0		41.	7:2	€1.	€1.	0.	M:12	3.	3:38	3.	2:14	1.	8:25
0.00004 8:0		21.	7:5	€1.	€1.	0.	M:13	3.	3:23	3.	5:34	1.	5:14
0.00004 2:0		71.	€1:7	€1.	€1.	3.	M:12	3.	3:58	3.	3:37	1.	3:38
0.00004 5:0		€1.	€1:9	€1.	€1.	3.	M:13	3.	3:32	3.	3:71	2.	3:16
0.00004 7:0		€1.	€1:0	€1.	€1.	3.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:0		€1.	€1:0	€1.	€1.	3.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 7:50		9:6	7:0	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:00		9:6	7:6	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:50		9:6	7:4	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:00		9:6	€1:3	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:50		9:6	€1:3	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:00		9:6	€1:4	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:50		9:6	€1:4	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:00		9:6	€1:6	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16
0.00004 8:50		9:6	€1:6	€1.	€1.	0.	M:13	3.	3:13	3.	3:71	2.	3:16

B: 1

Concrete Properties:

Compressive Strength of Concrete = 4000.00000 psi
 Modulus of Elasticity of Concrete = 3504997. psi
 Modulus of Rupture of Concrete = -474.34154 psi
 Compression Strain at Peak Stress = 0.00189
 Tensile Strain at Fracture of Concrete = -0.0001154
 Maximum Coarse Aggregate Size = 0.75000 in

Number of Axial Thrust Force Values: Determined from file-head loadings = 2

Number	Axial Thrust Force kips
1	68.100
2	90.800

Definitions of Run Messages and Notes:

C = concrete in section has cracked in tension.
 Y = stress in reinforcing steel has reached yield stress.
 T = ACI 318-08 criteria for tension-controlled section met, tensile strain in reinforcement exceeds 0.005 while simultaneously compressive strain in concrete more than 0.003. See ACI 318-08, Section 10.3.4.
 Z = depth of tensile zone in concrete section is less than 10 percent of section depth.
 Bending Stiffness (EI) = Computed Bending Moment / Curvature.
 Position of neutral axis is measured from edge of compression side of pile.
 Compressive stresses and strains are positive in sign.
 Tensile stresses and strains are negative in sign.

Axial Thrust Force = 68.100 kips

				F	Write	Size	()	FL	Size
23	1.27000	1.27000	1.27000	17.066	17.066	17.066	17.066	17.066	21.37	57
24	1.27000	1.27000	1.27000	17.646	17.646	17.646	17.646	17.646	19.91	55
25	1.27000	1.27000	1.27000	11.689	11.689	11.689	11.689	11.689	15.17	52
26	1.27000	1.27000	1.27000	13.169	13.169	13.169	13.169	13.169	19.23	57
27	1.27000	1.27000	1.27000	11.065	11.065	11.065	11.065	11.065	5.15	46
28	1.27000	1.27000	1.27000	11.365	11.365	11.365	11.365	11.365	9.00	48
29	1.27000	1.27000	1.27000	11.065	11.065	11.065	11.065	11.065	5.15	46
30	1.27000	1.27000	1.27000	13.169	13.169	13.169	13.169	13.169	19.23	57
31	1.27000	1.27000	1.27000	11.689	11.689	11.689	11.689	11.689	15.17	52
32	1.27000	1.27000	1.27000	17.646	17.646	17.646	17.646	17.646	19.91	55
33	1.27000	1.27000	1.27000	17.066	17.066	17.066	17.066	17.066	21.37	57
34	1.27000	1.27000	1.27000	11.985	11.985	11.985	11.985	11.985	14.37	52
35	1.27000	1.27000	1.27000	11.985	11.985	11.985	11.985	11.985	14.37	52
36	1.27000	1.27000	1.27000	11.445	11.445	11.445	11.445	11.445	12.26	50
37	1.27000	1.27000	1.27000	11.492	11.492	11.492	11.492	11.492	12.58	50
38	1.27000	1.27000	1.27000	11.182	11.182	11.182	11.182	11.182	11.84	50
39	1.27000	1.27000	1.27000	17.572	17.572	17.572	17.572	17.572	20.73	56
40	1.27000	1.27000	1.27000	11.724	11.724	11.724	11.724	11.724	12.56	50
41	1.27000	1.27000	1.27000	17.703	17.703	17.703	17.703	17.703	23.65	60
42	1.27000	1.27000	1.27000	11.579	11.579	11.579	11.579	11.579	14.28	50
43	1.27000	1.27000	1.27000	17.703	17.703	17.703	17.703	17.703	23.65	60
44	1.27000	1.27000	1.27000	11.724	11.724	11.724	11.724	11.724	12.56	50
45	1.27000	1.27000	1.27000	11.572	11.572	11.572	11.572	11.572	14.07	50
46	1.27000	1.27000	1.27000	11.182	11.182	11.182	11.182	11.182	11.84	50
47	1.27000	1.27000	1.27000	11.492	11.492	11.492	11.492	11.492	12.58	50
48	1.27000	1.27000	1.27000	11.445	11.445	11.445	11.445	11.445	12.26	50
49	1.27000	1.27000	1.27000	11.985	11.985	11.985	11.985	11.985	14.37	52
50	1.27000	1.27000	1.27000	11.066	11.066	11.066	11.066	11.066	14.37	52
51	1.27000	1.27000	1.27000	11.646	11.646	11.646	11.646	11.646	19.91	55
52	1.27000	1.27000	1.27000	11.689	11.689	11.689	11.689	11.689	15.17	52
53	1.27000	1.27000	1.27000	11.165	11.165	11.165	11.165	11.165	10.27	49
54	1.27000	1.27000	1.27000	11.066	11.066	11.066	11.066	11.066	14.37	52

NOTE: The positions of the above letters will be considered in the

Minimum spacing between any two bars is not less than 1/16 inch and 47

Spacing to aggregate size is 5.159

Edge-to-Edge Bar Spacing
 Maximum Concrete Aggregate Size
 Ratio of Bar Spacing to Aggregate Size
 Offset of Center of Rebar Cap from Center of Pier

Axial Structural Capacities

Nom. Axial Structural Capacity: $P_n = 0.85 f'_c A_g + A_s F_y$
 Tensile Load for Cracking of Concrete: T_c
 Nominal Axial Tensile Capacity: T_n

Reinforcing Bar Dimensions and Positions

Bar Number	Bar Diam. inches	Bar Area sq. in.	K inches	Y inches
1	1.27000	1.2700	14.365	0.00
2	1.27000	1.2700	14.065	5.15
3	1.27000	1.2700	13.169	10.23
4	1.27000	1.2700	11.689	15.17
5	1.27000	1.2700	10.646	19.91
6	1.27000	1.2700	9.066	24.37
7	1.27000	1.2700	7.985	28.51
8	1.27000	1.2700	6.445	32.26
9	1.27000	1.2700	5.492	35.58
10	1.27000	1.2700	4.182	38.42
11	1.27000	1.2700	2.572	40.73
12	1.27000	1.2700	2.724	42.56
13	1.27000	1.2700	7.703	43.65
14	1.27000	1.2700	2.575	44.28
15	1.27000	1.2700	2.575	44.28
16	1.27000	1.2700	7.703	43.65
17	1.27000	1.2700	2.724	42.56
18	1.27000	1.2700	7.572	40.73
19	1.27000	1.2700	2.182	38.42
20	1.27000	1.2700	6.492	35.58
21	1.27000	1.2700	6.445	32.26
22	1.27000	1.2700	3.985	28.51

Edge

Number of loads specified = 3

Ft White West (B1)-FL.1p7o

Load No.	Load Type	Condition 1	Condition 2	Axial Thrust		Compute Top y vs. Pile Length	
				Force, lbs	lbs		
1	1	V =	23150. lbs	M =	38489400. in-lbs	68100. lbs	Yes
2	1	V =	46300. lbs	M =	76978800. in-lbs	68100. lbs	Yes
3	1	V =	61733. lbs	M =	102638400. in-lbs	90800. lbs	Yes

V = perpendicular shear force applied to pile head
M = bending moment applied to pile head
y = lateral deflection relative to pile axis
S = pile slope relative to original pile batter angle
R = rotational stiffness applied to pile head
Axial thrust is assumed to be acting axially for all pile batter angles.

Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness

Axial thrust force values were determined from pile-head loading conditions

Number of pile Sections Analyzed = 1

Pile Section No. 1:

Dimensions and Properties of Drilled Shaft (Bored Pile):

Length of Section = 37.00000 ft
Shaft Diameter = 108.00000 in
Concrete Cover Thickness = 9.00000 in
Number of Reinforcing Bars = 54 bars
Yield Stress of Reinforcing Bars = 60000. psi
Modulus of Elasticity of Reinforcing Bars = 29000000. psi
Gross Area of Shaft = 9160.88418 sq. in.
Total Area of Reinforcing Steel = 68.58000 sq. in.
Area Ratio of Steel Reinforcement = 0.75 percent

Ft White West (81)-FL.1p70

1	Sand (Reese, et al.)	3.000	95.000	--	27.000	--	default
		9.500	95.000	--	27.000	--	default
2	Sand (Reese, et al.)	9.500	110.000	--	30.000	--	default
		29.500	110.000	--	30.000	--	default
3	Stiff Clay w/o Free Water	29.500	110.000	1100.000	--	default	--
		34.500	110.000	1100.000	--	default	--
4	Stiff Clay w/o Free Water	34.500	110.000	1300.000	--	default	--
		39.500	110.000	1300.000	--	default	--
5	Stiff Clay w/o Free Water	39.500	37.600	700.000	--	default	--
		43.500	37.600	700.000	--	default	--
6	Sand (Reese, et al.)	43.500	52.600	--	35.000	--	default
		51.000	52.600	--	35.000	--	default

Loading Type

Static loading criteria were used when computing p-y curves for all analyses.

Pile-head loading and Pile-head Fixity Conditions

Ft White West (B1)-FL.1p70

Distance from top of pile to top of Layer = 39.50000 ft
 Distance from top of pile to bottom of Layer = 43.50000 ft
 Effective unit weight at top of Layer = 37.60000 pcf
 Effective unit weight at bottom of Layer = 37.60000 pcf
 Undrained cohesion at top of Layer = 700.00000 psf
 Undrained cohesion at bottom of Layer = 700.00000 psf
 Epsilon-50 at top of Layer = 0.0000
 Epsilon-50 at bottom of Layer = 0.0000

NOTE: Internal default values for Epsilon-50 will be computed for this soil layer.

Layer 6 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of Layer = 43.50000 ft
 Distance from top of pile to bottom of Layer = 51.00000 ft
 Effective unit weight at top of Layer = 52.60000 pcf
 Effective unit weight at bottom of Layer = 52.60000 pcf
 Friction angle at top of Layer = 35.00000 deg.
 Friction angle at bottom of Layer = 35.00000 deg.
 Subgrade k at top of Layer = 0.0000 pci
 Subgrade k at bottom of Layer = 0.0000 pci

NOTE: Internal default values for subgrade k will be computed for this soil layer.

(Depth of lowest soil layer extends 14.00 ft below pile tip)

Summary of Soil Properties

Layer	Layer	Layer	Effective	Undrained	Angle of	Strain	
Layer	Soil Type	Depth	Unit Wt.	Cohesion	Friction	Factor	
Num.	(p-y Curve Criteria)	ft	pcf	psf	deg.	Epsilon 50	kpy

Ft White West (B1)-FL. 1p70
 Layer 2 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of Layer	=	9.50000 ft
Distance from top of pile to bottom of Layer	=	29.50000 ft
Effective unit weight at top of Layer	=	110.00000 pcf
Effective unit weight at bottom of Layer	=	110.00000 pcf
Friction angle at top of Layer	=	30.00000 deg.
Friction angle at bottom of Layer	=	30.00000 deg.
Subgrade k at top of Layer	=	0.0000 pci
Subgrade k at bottom of Layer	=	0.0000 pci

NOTE: Internal default values for subgrade k will be computed for this soil layer.

Layer 3 is stiff clay without free water

Distance from top of pile to top of Layer	=	29.50000 ft
Distance from top of pile to bottom of Layer	=	34.50000 ft
Effective unit weight at top of Layer	=	110.00000 pcf
Effective unit weight at bottom of Layer	=	110.00000 pcf
Undrained cohesion at top of Layer	=	1100.00000 psf
Undrained cohesion at bottom of Layer	=	1100.00000 psf
Epsilon-50 at top of Layer	=	0.0000
Epsilon-50 at bottom of Layer	=	0.0000

NOTE: Internal default values for Epsilon-50 will be computed for this soil layer.

Layer 4 is stiff clay without free water

Distance from top of pile to top of Layer	=	34.50000 ft
Distance from top of pile to bottom of Layer	=	39.50000 ft
Effective unit weight at top of Layer	=	110.00000 pcf
Effective unit weight at bottom of Layer	=	110.00000 pcf
Undrained cohesion at top of Layer	=	1300.00000 psf
Undrained cohesion at bottom of Layer	=	1300.00000 psf
Epsilon-50 at top of Layer	=	0.0000
Epsilon-50 at bottom of Layer	=	0.0000

NOTE: Internal default values for Epsilon-50 will be computed for this soil layer.

Layer 5 is stiff clay without free water

Input Structural Properties:

Pile Section No. 1:

Section Type = Drilled Shaft (Bored Pile)
 Section Length = 37.00000 ft
 Section Diameter = 108.00000 in

Ground Slope and Pile Batter Angles

Ground Slope Angle = 0.000 degrees
 = 0.000 radians
 Pile Batter Angle = 0.000 degrees
 = 0.000 radians

Soil and Rock Layering Information

The soil profile is modelled using 6 layers

Layer 1 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 3.00000 ft
 Distance from top of pile to bottom of layer = 9.50000 ft
 Effective unit weight at top of layer = 95.00000 pcf
 Effective unit weight at bottom of layer = 95.00000 pcf
 Friction angle at top of layer = 27.00000 deg.
 Friction angle at bottom of layer = 27.00000 deg.
 Subgrade k at top of layer = 0.0000 pci
 Subgrade k at bottom of layer = 0.0000 pci

NOTE: Internal default values for subgrade k will be computed for this soil layer.

Ft White West (B1)-Fl.1p7o

- Use unfactored loads in computations (conventional analysis)
- Compute pile response under loading and nonlinear bending properties of pile (only if nonlinear pile properties are input)
- Use of p-y modification factors for p-y curves not selected
- Loading by lateral soil movements acting on pile not selected
- Input of shear resistance at the pile tip not selected
- Computation of pile-head foundation stiffness matrix not selected
- Push-over analysis of pile not selected
- Buckling analysis of pile not selected

Output Options:

- No p-y curves to be computed and reported for user-specified depths
- Values of pile-head deflection, bending moment, shear force, and soil reaction are printed for full length of pile.
- Printing Increment (nodal spacing of output points) = 1

Pile Structural Properties and Geometry

Total number of pile sections = 1

Total length of pile = 37.00 ft

Depth of ground surface below top of pile = 3.00 ft

Pile diameter values used for p-y curve computations are defined using 2 points.

p-y curves are computed using pile diameter values interpolated with depth over the length of the pile.

Point	Depth X ft	Pile Diameter in
1	0.00000	108.0000000
2	37.00000	108.0000000

Problem Title

Project Name: Ft. White West Site

Location: Columbia County, FL

Type: 195' Monopole

Description: Nello

Engineer: Christopher A. Harris, PE

Program Options and Settings

Engineering Units of Input Data and Computations:

- Engineering units are US Customary Units (pounds, feet, inches)

Analysis Control Options:

- Maximum number of iterations allowed = 500
- Deflection tolerance for convergence = 1.0000E-05 in
- Maximum allowable deflection = 100.0000 in
- Number of pile increments = 100

Loading Type and Number of Cycles of Loading:

- Static loading specified

Computational Options:

LPILE Plus for Windows, Version 2013-07.005

Analysis of Individual Piles and Drilled Shafts
Subjected to Lateral Loading Using the p-y Method

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This copy of LPILE is used by:

Christopher A. Harris, P.E.
R.W. Harris, Inc.

Serial Number of Security Device: 228746911

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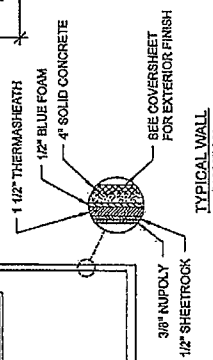
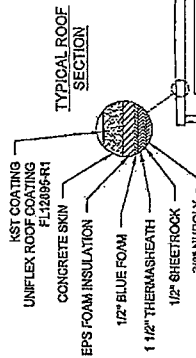
Files Used for Analysis

Path to file locations: C:\Users\Chris Harris\Documents\Ensoft\Lpile2013\DATA\2014\
Name of input data file: Ft White West (B1)-FL.1p7d
Name of output report file: Ft White West (B1)-FL.1p7o
Name of plot output file: Ft White West (B1)-FL.1p7p
Name of runtime message file: Ft White West (B1)-FL.1p7r

Date and Time of Analysis

Date: November 14, 2014 Time: 11:34:33
Page 1

- NOTE:**
- ALL HELIX WARRIED FIBERBOND SHALL BE INSTALLED, TESTED, REMOVED & PACKED IN BUILDING PRIOR TO SHIPMENT.
 - ITEM 15, 24, 25 & 47 ARE SHOWN IN ALL OPTIONAL LOCATIONS. SEE BUILDING COVERSHEET FOR SELECTED LOCATIONS.
 - SECURE WITH LOCKWASERS AND REDUCING WASHER. CAP OFF PENETRATION. DO NOT GLUE.
 - CONNECT #2 SOLID TINNED WITH TWO-BOLT HOLE NON-COMPRESSION CORNER #2 SOLID TINNED WITH TWO-BOLT HOLE NON-COMPRESSION CORNER #2 SOLID TINNED WITH TWO-BOLT HOLE NON-COMPRESSION CORNER TYPE LUG. TO FLOOR FRAMING AND CONCEPT.
 - SEE ALSO INTERIOR DETAILS SHEET FOR EXTERIOR GROUND BAR MOUNTING HARDWARE AND DETAIL.
 - CUT RIGID CAST-IN CONDUITS 10" ABOVE FLOOR ASSEMBLY CONNECTORS AND SEALTIGHT.



SECTION "A-A"

SCALE: NTS

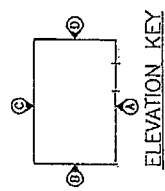
Const. Type	VB
Occupancy	SZ
Wind Velocity	1
Fire Rating of	180 LPS-C
Ext. Walls	0
Plan No.	MF77-D-577
Allow. Floor Load	250 PSF
Manufacturer	17/62014
Manufacturer	ZFRANCIS

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

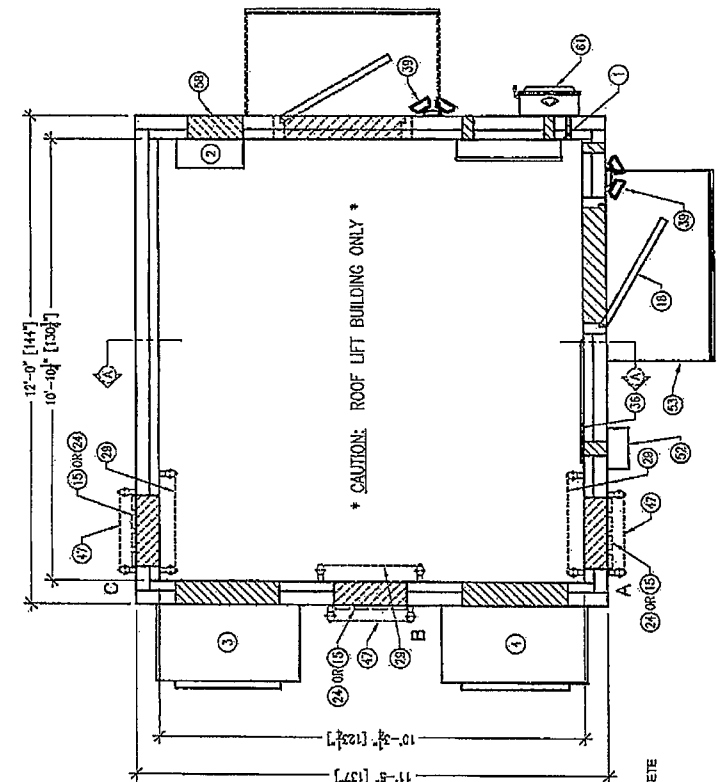
These prints comply with the Florida Manufacture Building Act and adopted Codes and all required components shall comply with SB-72

APPROVED BY

NIA INC.

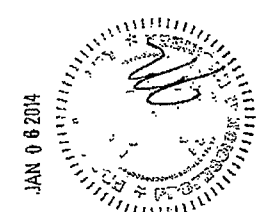


ELEVATION KEY



FLOOR PLAN

138.0 sq. ft.



1800 DAVENPORT DRIVE MINDEN, LA 71056
ph. (800) 924-2814 www.fiberbond.com

FIBERBOND

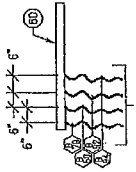
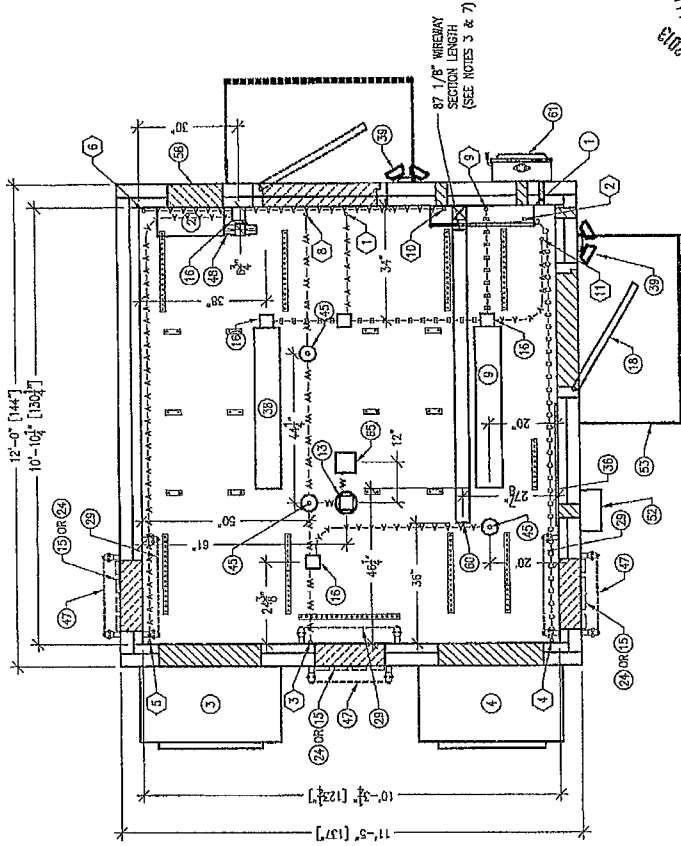
AT&T MOBILITY
EQUIPMENT SHELTER

11'-5" X 12'-0" EQUIPMENT SHELTER

SCALE: 3/8"=1'-0" 1-1

FLOOR PLAN

REV.	DATE	BY	CHK	DATE	REVISION



- NOTES:**
1. ALL MEASUREMENTS SHOWN ARE ACQUIRE WITHIN $\pm 1/4"$ ALL ADJACENT BOXES SHALL BE THE SAME DIMENSION.
 2. THE LOCATIONS OF 4, X, Z, AND 4 11/16" X 4 11/16" JUNCTION BOXES AND SMALLER THAN CAN BE ACQUIRED AS REQUIRED TO ACCOMMODATE THE DEVICES. INTERFERENCE WITH OTHER COMPONENTS WILL REQUIRE THEM TO BE CUT.
 3. MOUNT ON UNRESTRICT.
 4. CONDUIT SHOULD BE INSTALLED AS SHOWN IF POSSIBLE. ANY CHANGES TO 80° BENDS, OFFSETS AND SADDLES MAY BE DONE WHEN NEEDED AND SHOULD BE UNDER THE GUIDANCE OF A QUALIFIED SENIOR CREW LEADER. ANY CHANGE MUST BE WITHIN THE GUIDELINES OF STANDARD OPERATING PROCEDURE FOR GENERAL CONDUIT RULES.
 5. ALL CONDUIT TO HAVE A MINIMUM OF 3/4" BETWEEN THEM.
 6. ALL CONDUIT TO BE 1/2" UNLESS OTHERWISE NOTED.
 7. PULL 8'-0" OF FLEXIBLE METALLIC CONDUIT TO FLOOR. CONNECT TO FLEXIBLE METALLIC CONDUIT THROUGH A 50° CONNECTOR TO EACH FLOOR.
 8. ITEMS 15, 24, 28, 37, 47 AND 58 ARE SHOWN IN ALL OPTIONAL LOCATIONS. INSTALL ONLY AT INSTALLED WAGESHEET, LOCATE AS SHOWN ON ELEVATION VIEW. SEE BUILDING COVERSHEET FOR SELECTED LOCATIONS.
 9. REMOVED NOTE.
 10. ALL LIGHT SWITCHES TO BE MOUNTED AT 54" A.F.F. UNLESS NOTED OTHERWISE.
 11. ALL RECEPTACLES TO BE MOUNTED AT 18" A.F.F. UNLESS NOTED OTHERWISE.
 12. ALL CONDUIT SHALL BE EMT UNLESS OTHERWISE NOTED.
 13. ALL CONDUIT THROUGH A WALL SHALL BE RIGID UNLESS OTHERWISE NOTED.

A.C.	CONDUIT KEY	P.V.C.
A = 1 1/2"	AA = 1 1/2"	XA = 1 1/2"
B = 3/4"	BB = 3/4"	XB = 3/4"
C = 1"	CC = 1"	XC = 1"
D = 1 1/4"	DD = 1 1/4"	XD = 1 1/4"
E = 1 1/2"	EE = 1 1/2"	XE = 1 1/2"
F = 2"	FF = 2"	XF = 2"
G = 2 1/2"	GG = 2 1/2"	YG = 2 1/2"
H = 3"	HH = 3"	XH = 3"
I = 3 1/2"	II = 3 1/2"	XI = 3 1/2"
J = 4"	JJ = 4"	XJ = 4"

OOOOOO = FLEXIBLE NONMETALLIC CONDUIT

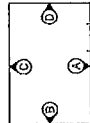
These prices comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with 9B-72

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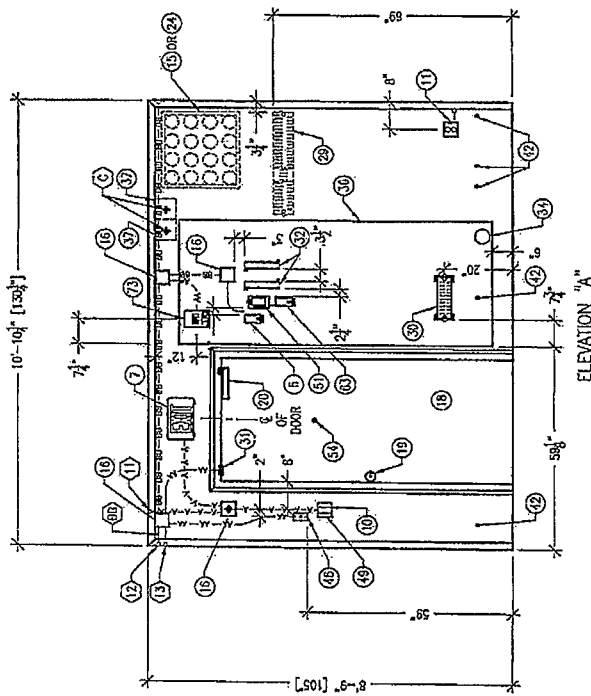
MAINC.

Approval of this document does not constitute or approve any deviation or deviations from the requirements of applicable State Laws.

Condit. Type: VB
 Occurrence: S2
 Allowable No.: 1
 Wind Velocity: 150 mph G
 Fire Rating of Allow. Floor Load: 0
 Box Weights: 250 PSF
 Approval Date: 11/27/13-9577
 Manufacturer: Fibrebond LLC
 Manufacturer or approve any deviation or deviations from the requirements of applicable State Laws.



ELEVATION KEY



ELEVATION "A"

These prints comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with 99-72

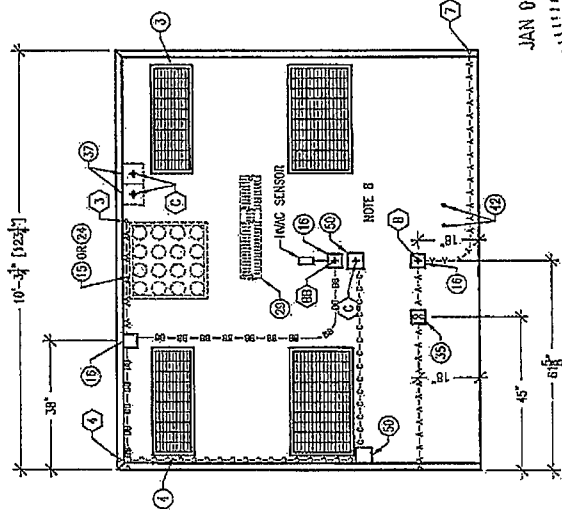
APPROVED BY



Approval of this document does not authorize or approve any variation or deviation from the requirements of applicable State Laws.

Const. Type:	VB
Occupancy:	SZ
Allowable No. of Floors:	1
Wind Velocity:	150 MSF G
Fire Rating of Floor:	0
Plan No.:	MF727-D-9577
Allow. Floor Load:	250 PSF
Approval Date:	1/16/2014
Manufacturer:	FIBREBOND

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ELEVATION "B"

JAN 0 6 2014



1900 DAVENPORT DRIVE MINDEN, LA 71055 ph: (504) 824-2814 www.fibrebond.com	
FIBREBOND	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
INTERIOR ELEVATIONS "A" & "B"	
DATE	DATE
BY	BY
APP.	DATE
REVISION	DATE



ELEVATION KEY

These prints comply with the Florida Manufacture Act and all required components shall comply with 9877-APPROVED BY

NIA INC.

Approval of this document does not constitute approval of any deviation or deviation from the requirements of applicable State Laws.

Const. Type: _____
 Occupancy: _____
 Accessible No. _____
 Wind Velocity: _____
 Fire Rating of _____
 Ekt. Walls: _____
 Plot No. _____
 Allow. Floor Load: _____
 Max. Wind Speed: _____
 Max. Seismic: _____
 Material: _____
 Finish: _____
 Fiberglass: _____
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NOTE 6

LOAD	LOAD PER PHASE (VA)			WIRE COLOR	LOADS CONTINUOUS	LOADS NON-CONTINUOUS	LOADS 3WIRE/4WIRE	WIRE SIZE	TYPE	LOAD PER PHASE (VA)			WIRE COLOR	LOADS CONTINUOUS	LOADS NON-CONTINUOUS	LOADS 3WIRE/4WIRE	WIRE SIZE	TYPE	LOAD PER PHASE (VA)			WIRE COLOR	LOADS CONTINUOUS	LOADS NON-CONTINUOUS	LOADS 3WIRE/4WIRE	WIRE SIZE	TYPE	DESCRIPTION																			
	UNIT	VA	%							UNIT	VA	%							UNIT	VA	%								UNIT	VA	%	UNIT	VA	%	UNIT	VA	%	UNIT	VA	%	UNIT	VA	%	UNIT	VA	%	DESCRIPTION
	A	B								A	B								A	B									A	B		A	B		A	B		A	B		A	B		A	B		
1	1	3324	1	3324	1	3324	3324	100	40	40	40	BLK	X			8	(10)	8	40	40	40	BLK	3324	3324	3324	1	3324	1	HVAC #2																		
3	1	3324	1	3324	1	3324	3324	100	21	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	BLK BEARS & BATT. WOKER																		
5	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	INTERIOR RECEPTACLES & CORN REEL																		
7	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	INTERIOR LIGHTS																		
9	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
11	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
13	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
15	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
17	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
19	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
21	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
23	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
25	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
27	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
29	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
31	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
33	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
35	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
37	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
39	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		
41	1	500	1	500	1	500	500	100	20	12	12	RED	X			10	10	10	30	30	30	RED	500	500	500	1	500	1	EXTERIOR LIGHTS																		

- NOTES:
- ALL WIRE TO BE #12 THRU #14 UNLESS NOTED OTHERWISE.
 - CONDUCTOR COLOR CODES:
 - PH = PHASE
 - BL = BLACK
 - BR = BROWN
 - BU = BLUE
 - GN = GREEN
 - GR = GREY
 - OR = ORANGE
 - RD = RED
 - WH = WHITE
 - ALL WORK TO CONFORM TO N.E.C. LATEST STATE ADOPTED EDITION.
 - LABEL SERVICE DISCONNECT WITH RED TAG.
 - LETTER LABEL ON ALL CONDUCTORS WITH COLOR AS CIRCUIT CONDUCTORS.
 - LETTER LABEL ON ALL CONDUCTORS WITH COLOR AS CIRCUIT CONDUCTORS.
 - PULL ONE GROUND CONDUCTOR PER FLEXIBLE METALLIC CONDUIT FOR ALL OTHER CIRCUITS PULL A SEPARATE CONDUCTOR.
 - ALL EFCI RECEPTACLES TO HAVE A DEDICATED GROUND WIRE.
 - EQUIPMENT TERMINATION LUGS AND CONDUCTORS ARE RATED AT A MINIMUM OF 75°C.



1500 DAVENPORT DRIVE, MOBILE, AL 36688
 PH: (904) 686-2814 www.niainc.com

AT&T MOBILITY
 11'-5" X 12'-0" EQUIPMENT SHELTER
 ELECTRICAL SCHEMATIC #1

DATE: NONE
 SCALE: 3"-1" 1/2
 SHEET: B OF 2
 PROJECT: B OF 2

DESIGNED BY: JMS
 CHECKED BY: JMS
 DATE: 11/27/15
 PROJECT NO: 15-001

REVISION

Const. Type: This code comply with the
 Occupancy: Mobile Manufactured Building
 Allowable No. of Floors: 1
 Wind Velocity: 150 mph
 Ed. Code: 2015
 Plan No. MF725-5977
 Allow. Floor Load: 250 PSF
 Approval Date: 11/27/15
 Manufacturer: FibreBond
 Approval of this document does not authorize or approve any deviation or conditions
 for the requirements of applicable State Laws.

NOTES:

1. DASHED LINES DENOTE FIELD WIRE.
2. SERVICE SHALL BE PROVIDED AT 22,000 A.L.C. IF HIGHER BUILDING IS REQUIRED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR/OWNER TO MEET SUCH REQUIREMENTS.
3. SERVICE BOND IS TO BE MADE BY DEVICES (STRAPS, SCREWS, ETC.) SUPPLIED BY EQUIPMENT MANUFACTURER. IF NO SUCH DEVICE IS SUPPLIED, BOND IS TO BE MADE IN ACCORDANCE WITH REC. ARTICLE 250. WHEN SERVICE OVERCURRENT DISCONNECT IS FIELD INSTALLED AND HAS A NEUTRAL TO GROUND CONNECTION ESTABLISHED, REMAKE NEUTRAL TO GROUND CONNECTION IN TRANSFER SWITCH.
4. CONDUCTOR OVERCURRENT PROTECTION DEVICES ARE SELECTED IN ACCORDANCE WITH REC. (ARTICLE 240.3).
5. CONDUCTOR SIZING IS SELECTED FROM REC. (ARTICLE 214.3).
6. ALL CONDUCTORS SHALL BE COPPER.
7. LABEL SERVICE DISCONNECT WITH A RED TAG.
8. EQUIPMENT SERVICE ENTRANCE CONDUCTOR SIZE: (3) 600 KCMIL & (1) 1/0 GROUND.
9. EQUIPMENT TERMINATION LUGS & CONDUCTORS ARE RATED AT A MINIMUM OF 75 °C.

These plans comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with 88-72.

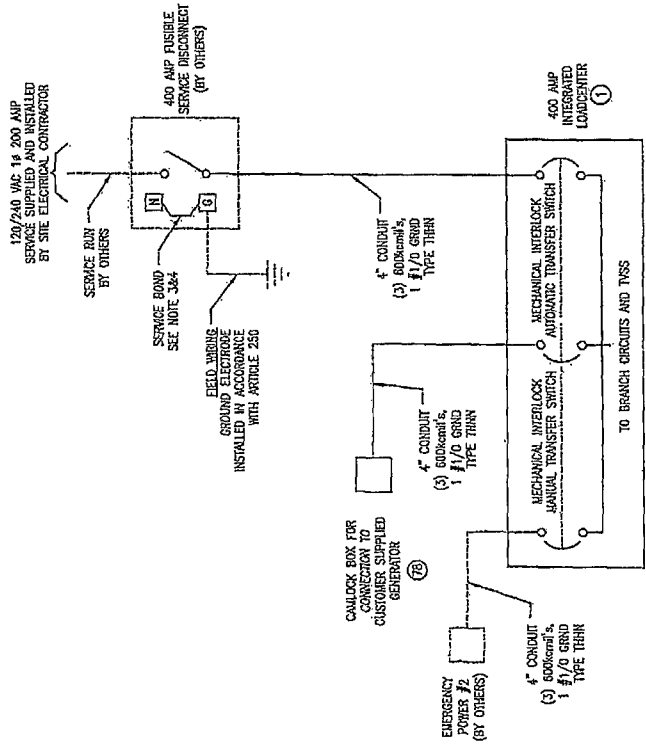
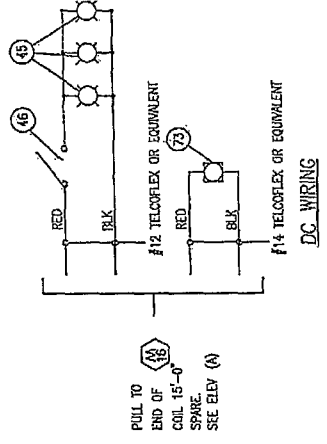
APPROVED BY

NFA INC.

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

Contract Type	SB
Occupancy	1
Allowable No. of Floors	0
Wind Velocity	150 mph
Fire Rating of Int. Wall	0
Int. Wall	MFT2-0-9577
Allow. Floor Load	250 PSF
Approval Date	1/16/2014
Manufacturer	Fibrebond

JAN 0 8 2014

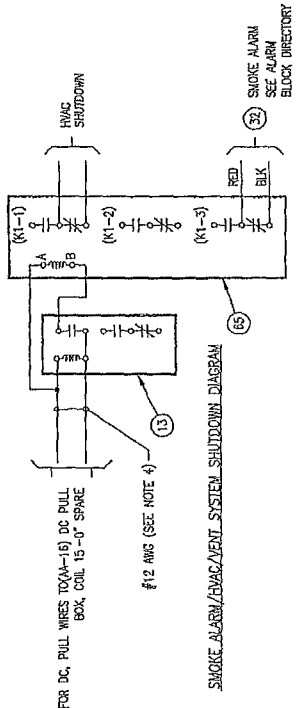


POWER RISER BLOCK DIAGRAM

1300 DAVENPORT DRIVE MINNEBA, LA 71055 PH: (504) 824-6264 www.fibrebond.com	
FIBREBOND	
AT&T MOBILITY	DATE: 3-2-1
11'-5" X 12'-0" EQUIPMENT SHELTER	NO. B-9577
ELECTRICAL SCHEMATIC #2	
REV. 1	DATE
REV. 2	DATE
REV. 3	DATE
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REV. 96	DATE
REV. 97	DATE
REV. 98	DATE
REV. 99	DATE
REV. 100	DATE

ALARM TERMINAL BLOCK (2)

ALARM BLOCK #1	ALARM BLOCK #2
INTRUSION DOOR ALARM	1
COMMERCIAL POWER FAIL	2
SURGE ARRESTOR ALARM	3
EQUIP ROOM SMOKE ALARM	4
HYDROGEN GAS ALARM	5
HIGH HUMIDITY ALARM	6
HIGH TEMPERATURE ALARM	7
LOW TEMPERATURE ALARM	8
HVAC FAIL #1 ALARM	9
HVAC FAIL #2 ALARM	10
GEN. RUM ALARM	11
GEN. LOW FUEL ALARM	12
GEN. SHUTDOWN ALARM	13
GEN. MAJOR ALARM	14
FUEL BASIN RUPTURE ALARM	15
TRANSFER SWITCH FAIL	16
PORTABLE GEN. LOW FUEL	17
PORTABLE GEN. MAJOR	18
PORTABLE GEN. RUNNING	19
PORTABLE GEN. THEFT ALARM	20
GEN. FAIL ALARM	21
FUEL BASIN RUPTURE ALARM	22
TRANSFER SWITCH FAIL	23
PORTABLE GEN. LOW FUEL	24
PORTABLE GEN. MAJOR	25
PORTABLE GEN. RUNNING	26
PORTABLE GEN. THEFT ALARM	27
GEN. FAIL ALARM	28
FUEL BASIN RUPTURE ALARM	29
TRANSFER SWITCH FAIL	30
PORTABLE GEN. LOW FUEL	31
PORTABLE GEN. MAJOR	32
PORTABLE GEN. RUNNING	33
PORTABLE GEN. THEFT ALARM	34
GEN. FAIL ALARM	35
FUEL BASIN RUPTURE ALARM	36
TRANSFER SWITCH FAIL	37
PORTABLE GEN. LOW FUEL	38
PORTABLE GEN. MAJOR	39
PORTABLE GEN. RUNNING	40
PORTABLE GEN. THEFT ALARM	41
GEN. FAIL ALARM	42
FUEL BASIN RUPTURE ALARM	43
TRANSFER SWITCH FAIL	44
PORTABLE GEN. LOW FUEL	45
PORTABLE GEN. MAJOR	46
PORTABLE GEN. RUNNING	47
PORTABLE GEN. THEFT ALARM	48
GEN. FAIL ALARM	49
FUEL BASIN RUPTURE ALARM	50
TRANSFER SWITCH FAIL	51
PORTABLE GEN. LOW FUEL	52
PORTABLE GEN. MAJOR	53
PORTABLE GEN. RUNNING	54
PORTABLE GEN. THEFT ALARM	55



These plans comply with the Florida Manufactured Building Act and adopted Codes and Regulations. The manufacturer shall comply with 9B-172.

APPROVED BY **NIA INC.**

Approval of this document does not constitute or approve any violation of the requirements of applicable State Laws.

Cons. Type: _____
 Occupancy: _____
 Allowable No. of Units: _____
 Max. Occupancy: _____
 Fire Rating of Floor: _____
 Ext. Walls: _____
 Plan No.: _____
 Allow. Floor Load: _____
 Approval Date: _____
 Signature: _____
 Title: _____

THESE PLANS ARE THE PROPERTY OF NIA INC. ANY REVISIONS TO THESE PLANS MUST BE MADE IN WRITING AND BE APPROVED BY NIA INC. BEFORE ANY CONSTRUCTION BEGINS. NIA INC. SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION PROVIDED HEREON. NIA INC. SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION PROVIDED HEREON.

1930 DAVENPORT DRIVE MANDEN, LA 71055
 Ph. (800) 824-2614 www.fibrebond.com

AT&T MOBILITY
 11'-5" X 12'-0" EQUIPMENT SHELTER #2
 ALARM WIRING SHEET #2

STATE OF MISSISSIPPI
 DEC 2 2013

REV. BY: _____ DATE: _____
 REV. BY: _____ DATE: _____
 REV. BY: _____ DATE: _____

DATE: NONE
 SHEET NO. 4-2
 E BE D-9577

ITEM LIST

Table with columns: ITEM NO., REFERENCE PART NO., MANUFACTURER, DESCRIPTION. Lists various parts and materials for a mobile shelter.

ITEM LIST

Table with columns: ITEM NO., REFERENCE PART NO., MANUFACTURER, DESCRIPTION. Continuation of the item list.

NOTE: THESE PRICES ARE SUBJECT TO CHANGE DUE TO AVAILABILITY AND EQUAL REQUIREMENTS MAY CHANGE DUE TO MANUFACTURER SPECIFICATIONS.

These prices comply with the National Manufactured Building Code and all required components shall comply with 98-72.

APPROVED BY NIA INC.

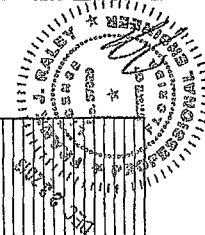
Approval of this document does not authorize or approve any deviation or conditions from the requirements of applicable State Laws.

Const. Type: V9
Occupancy: S2
Alternative No. 1
Wind Velocity: 130 MPH
Fire Rating of Ex. Walls: 0
Plan No. MT172-D-9577
Allow. Floor Load: 200 PSF
Approval Date: 11/6/2014
Manufacturer: Fibrebond

PACKING LIST

Table with columns: ITEM NO., QTY, REFERENCE PART NO., MANUFACTURER, DESCRIPTION. Lists items for packing.

* VARIES WITH SIZE OF BLDG. SEE SEQUENCE 103



Administrative forms including Fibrebond logo, AT&T Mobility logo, and a revision table with columns: REV, BY, DATE, REASON.

ITEM LIST

Table with columns: ITEM, PIREBOND PART NO., MANUFACTURER PART NO., MANUFACTURER, DESCRIPTION. Lists various components like bolts, nuts, washers, and structural members.

ITEM LIST

Table with columns: ITEM, PIREBOND PART NO., MANUFACTURER PART NO., MANUFACTURER, DESCRIPTION. Lists various components like bolts, nuts, washers, and structural members.

NOTES: 1. THIS IS SUBJECT TO CHANGE DUE TO AMALGAMY AND EQUAL REQUIREMENTS MAY CHANGE DUE TO MANUFACTURER SPECIFICATIONS.

These prints comply with the Florida Manufactured Building and other adopted Codes and all other applicable codes and requirements shall comply with BS-72.

APPROVED BY NIA INC. Approval of this document does not authorize or approve any deviation or deviation from the requirements of applicable State Laws.

Coast Type: V9 Allowable No. of Floors: 2 Wind Velocity: 130 mph c. Pile Rating of: 0 Allow. Floor Load: 250 PSF Approval Date: 07/25/2014 Manufacturer: PIREBOND

PACKING LIST

Table with columns: ITEM NO., QTY., PIREBOND PART NO., MANUFACTURER PART NO., MANUFACTURER. Lists items for packing, including bolts, nuts, and washers.

*VARIES WITH SIZE OF BLDG. - SEE SEQUENCE 508



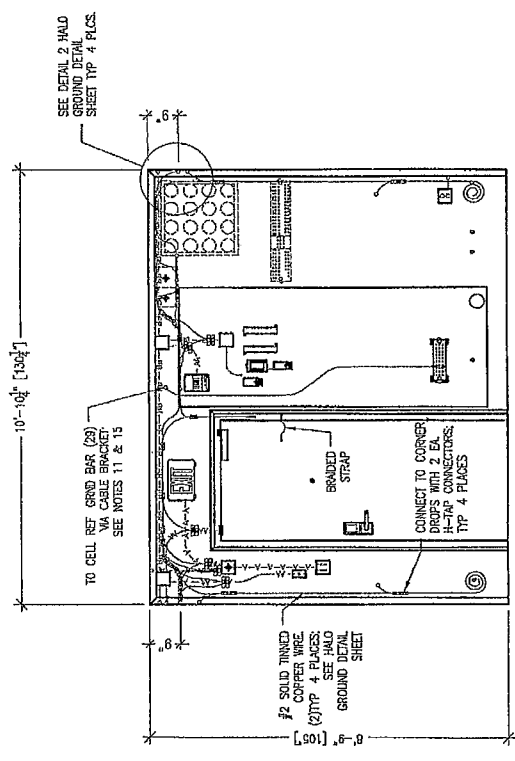
1500 DAVENPORT DRIVE WINDEN, LA. 71095
ph: 601-624-8214 www.pirebond.com

AT&T MOBILITY
11'-5" X 12'-0" EQUIPMENT SHELTER

Table with columns: REV, DATE, BY, APP, REASON. Revision table showing changes to the drawing.

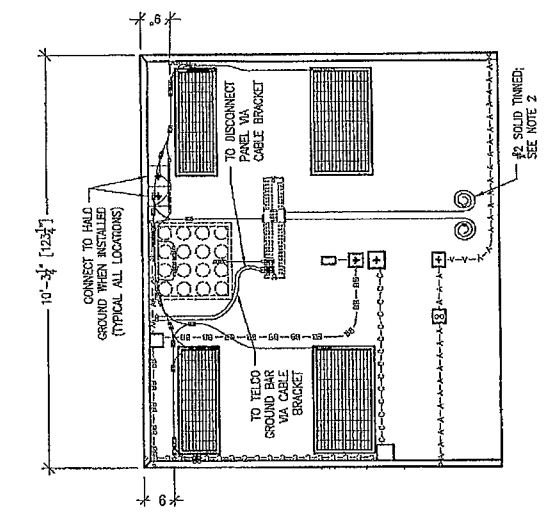
THESE PLANS ARE THE PROPERTY OF PIREBOND. NO PART OF THESE PLANS MAY BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT THE WRITTEN PERMISSION OF PIREBOND.

DATE: NONE
SCALE: 3.1
B REE D-9577



NOTES:
 1. HALO GROUND CEILING HIGH SHEET FOR NOTES.
 2. SEE HALO GROUND DETAILS SHEET FOR DETAILS.

ELEVATION "A"

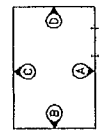
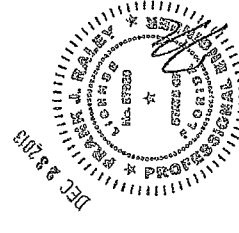


These prints comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with SB-72

APPROVED BY
NIA INC.
 Approval of this document does not authorize or approve any deviation or variations from the requirements of applicable State Laws.

Const. Type: _____
 Occupancy: _____
 Allowable No. of Floors: _____
 Wind Velocity: _____
 Fire Rating of Ex. Wall: _____
 Allow. Floor Load: _____
 Approval Date: _____
 Manufacturer: _____

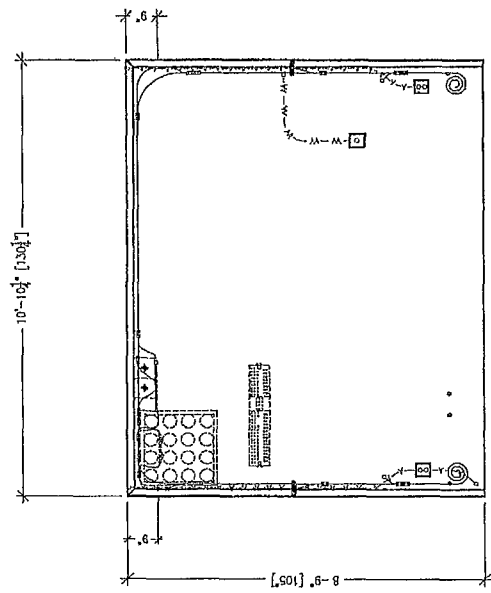
ELEVATION "B"



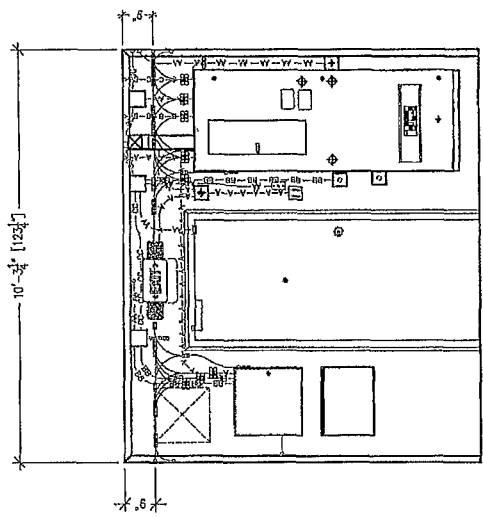
ELEVATION KEY

1300 DANFORTH DRIVE MIAMI FL 33156 PH: (305) 825-2314 www.niainc.com	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
HALO GROUND ELEVATIONS "A" & "B"	
DATE: 3/6-1-07	SCALE: 7-2
REV. 1	DATE
REV. 2	DATE
REV. 3	DATE
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FIBREBOND
 3/6-1-07 7-2
 B Rev D-9577



ELEVATION "C"



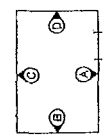
ELEVATION "D"

NOTES:
 SEE HALO GROUND CEILING VIEW SHEET FOR NOTES.
 SEE HALO GROUND DETAILS SHEET FOR DETAILS.

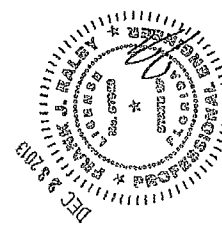
These prints comply with the
 requirements of the Building
 Code and all required components shall
 comply with 89-27

APPROVED BY
NIFA INC.
 Approval of this document does not authorize or approve any deviation or variations
 from the requirements of applicable State Laws.

Const. Type: _____
 Occupancy: _____
 Allowable No. of Persons: _____
 Wind Velocity: _____
 Fire Rating of _____
 Ekt. Walling: _____
 Plan No. MF27-D-9577
 Allow. Floor Load: 250 PSF
 Max. Allow. Data: 7/18/2014
 Manufacturer: Fibreboard

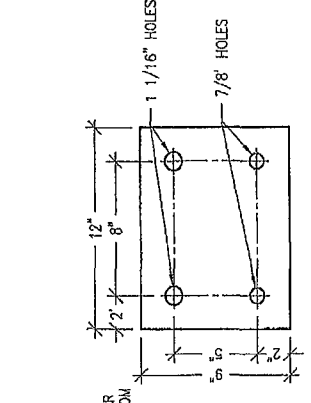
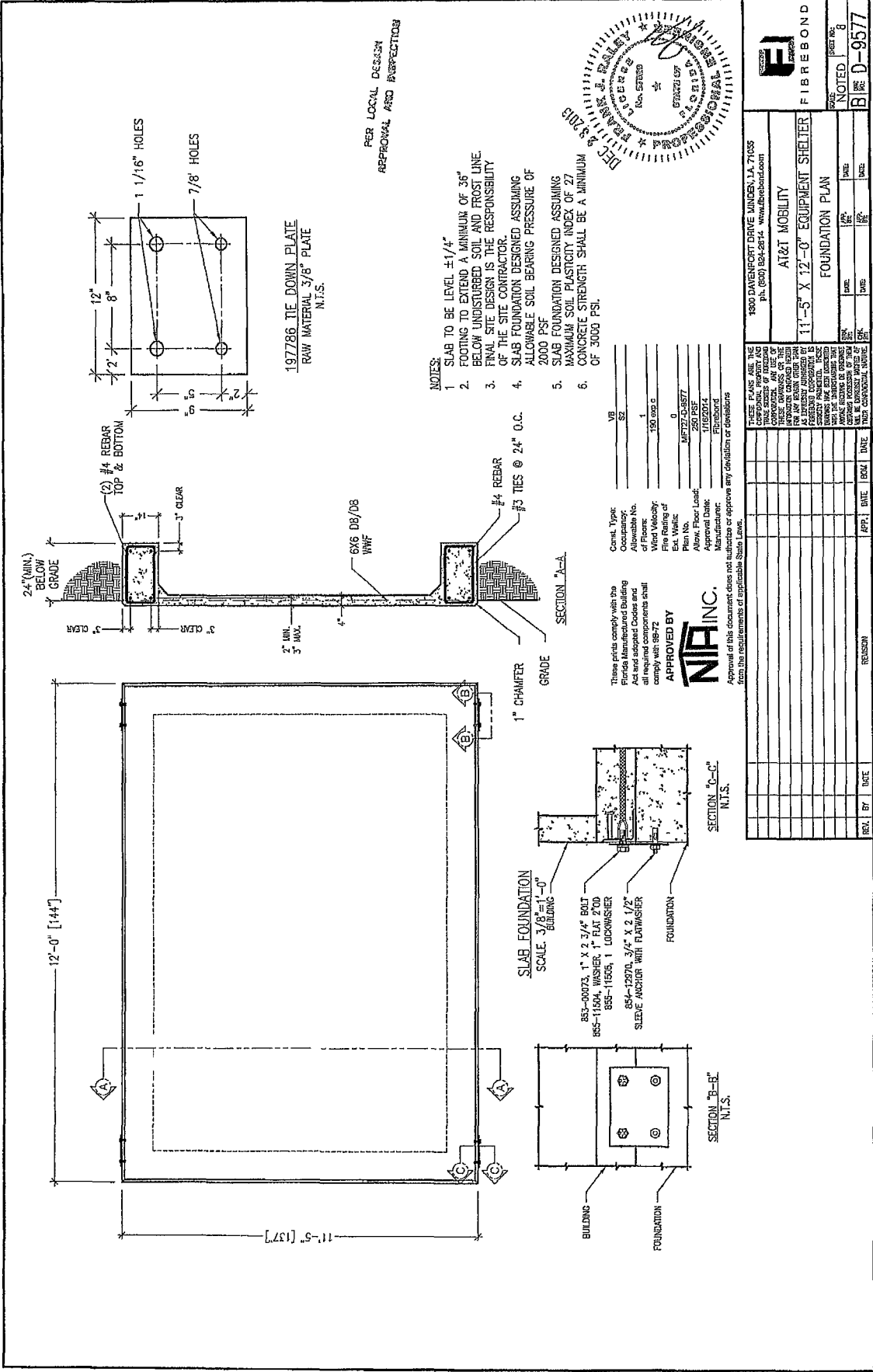


ELEVATION KEY



1500 DIXONPORT DRIVE MONROE, LA 71055 PH: (504) 835-2377 www.nifa.com	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
HALO GROUND ELEVATIONS "C" & "D"	
DATE	DATE
3/18-1-07	7-3
REV	DATE
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FIBREBOARD
 3/18-1-07
 7-3
 B SEE D-9577



197786 TIE DOWN PLATE
RAW MATERIAL 3/8" PLATE
N.T.S.

PER LOCAL DESIGN
APPROVAL AND INSPECTIONS

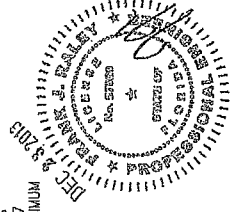
NOTES:

1. SLAB TO BE LEVEL $\pm 1/4"$
2. FOOTING TO EXTEND A MINIMUM OF 36" BELOW UNDISTURBED SOIL AND FROST LINE. FINAL SITE DESIGN IS THE RESPONSIBILITY OF THE SITE CONTRACTOR.
3. SLAB FOUNDATION DESIGNED ASSUMING ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF
4. SLAB FOUNDATION DESIGNED ASSUMING MAXIMUM SOIL PLASTICITY INDEX OF 27
5. CONCRETE STRENGTH SHALL BE A MINIMUM OF 3000 PSI.

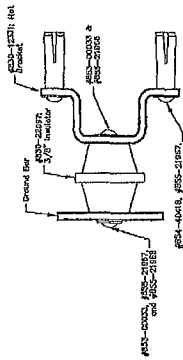
Comp. Type:	VS
Occupancy:	1
Number of Floors:	1
Wind Velocity:	150 mph c
Fire Rating of Ext. Wall:	0
Plan No.:	11/19/2014
Approval Date:	11/19/2014
Manufacturer:	Fluorobond

APPROVED BY
NIA INC.

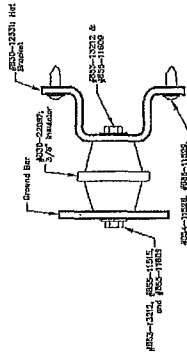
Approval of this document does not constitute or approve any deviation or deviations from the requirements of applicable State Laws.



1930 DAVENPORT DRIVE MONROE, LA 71055 ph. (800) 894-8814 www.fibrebond.com					
FIBREBOND					
AT&T MOBILITY					
11'-5" X 12'-0" EQUIPMENT SHELTER					
FOUNDATION PLAN					
REV	DATE	BY	CHK	DATE	BY
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2					
3					
4					
5					
6					
7					
8					
NOTED		PAGE NO. 8		REV D-9577	



Exterior



Interior

GROUND BAR MOUNTING PROCEDURES

General Notes:

1. For Waveguides, Door Canopy, and Exterior Vent Hood installations, use 1/4" x 1 5/8" Hilti HPS Impact anchors, #854-22486
2. HVAC mounting, use 5/16" x 1 1/2" sleeve anchors, #854-11135.
3. All interior boxes, panels, straps, etc. shall use #853-10848 screws. Large boxes and lights need to have washer #855-11513 with screw
4. For exterior boxes, use #854-11528, #855-11529, and #853-11533 for anchors.
5. On furred walls using hot channel, sheetrock and FRP shall be fastened at perimeter at 12" o.c. using screws. On furred walls using THERMASHEATH insulation attach 3/8" Nupoly to wall over THERMASHEATH insulation with 3/16" x 3/4" PNF topcoat #854-12917. Use Chemrexx #850-11235 glue on 3/8" Nupoly.
6. On Concrete walls, mount sheetrock 16" o.c using 3/16" x 1 1/2" HPS anchors, #854-15111 Mount FRP over sheetrock using 1 1/2" coil nails, #854-11841 12" o.c. for Light Weight Bldgs For C2 Bldgs, use 1/4" x 2 1/16" HPS anchors, #854-11501

These prints comply with the Florida Manufactured Building Act and all required components shall comply with SB-72

APPROVED BY

NIA INC.

Approval of this document does not authorize or approve any deviation or variations from the requirements of applicable State Laws.

Const. Type:	VB
Approval No.:	S2
Wind Velocity:	1
Fire Rating of:	150 Temp c
Ex. Wall:	D
Plan No.:	MF12-D-8577
Drawn, Error Log:	250 PSF
Approved:	17/8/2014
Manufacturer:	20/20/20



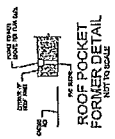
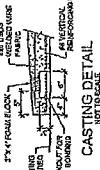
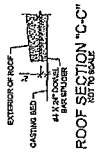
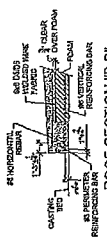
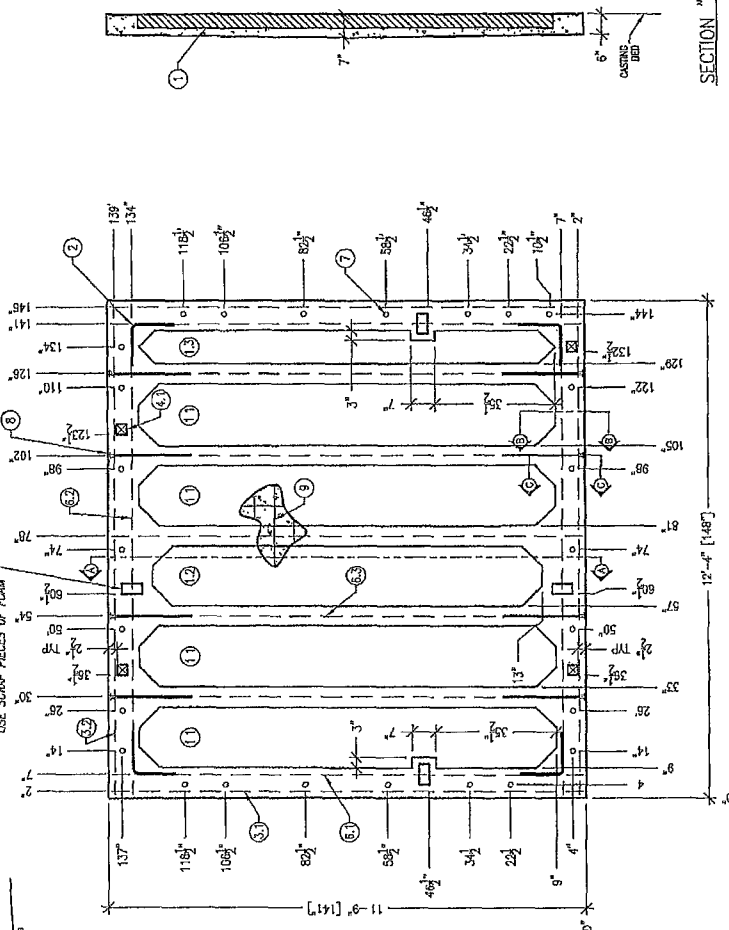
1500 DAMENPORT DRIVE MINNEN LA 71055 Ph: (810) 284-8614 www.fibresond.com	
FIBRESOND	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
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REV.:	100
DATE:	11/20/14

REBAR CUT LIST	FORM CUT LIST	DESCRIPTION	DWG. NO.		
ITEM	QTY	LENGTH	WIDTH		
1	137	137"	18"	11'-5" X 12'-0" EPS FOM INSULATION REBAR BEND #3 X 24" 60° REBAR #3 (3/8") GRADE 60 6" EPS FOM INSULATION REBAR #6 (3/4") GRADE 60 PRODUCT FORMER SPLICER #4 X 24" DIODEL BAR 11'-5" X 12'-0" DS-05	DWS-11
2	144	144"	18"		
3	129	129"	18"		
4	110	110"	18"		
5	102	102"	18"		
6	98	98"	18"		
7	74	74"	18"		
8	57	57"	18"		
9	4	4"	3"		

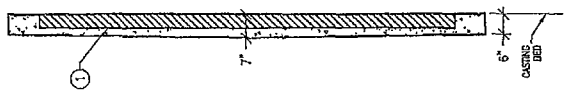
REBAR CUT LIST	FORM CUT LIST	DESCRIPTION
ITEM	QTY	LENGTH
1	137	137"
2	144	144"
3	129	129"
4	110	110"
5	102	102"
6	98	98"
7	74	74"
8	57	57"
9	4	4"

ALL FORM CORNERS TO BE CHAMFERED 6" AS SHOWN

3" X 6" BLOCKOUT FOR REBAR BONDING
4 PLACES-SEE NOTE 3 & CASTING DETAIL
USE SCRAP PIECES OF FOAM



SECTION "A-A"



* CAUTION ROOF LIFT BUILDING ONLY *

ROOF
1443 sq. ft.

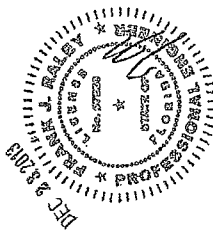
1800 DAVENPORT DRIVE WINSTON, LA. 71055
ph. 850 254-2314 www.fibrebond.com

FIBREBOND

AT&T MOBILITY
STRUCTURAL LAYOUT
ROOF

DATE: 11/15/05
DRAWN BY: [Signature]
CHECKED BY: [Signature]
SCALE: 3/8"=1'-0"

REV. NO. D-9577

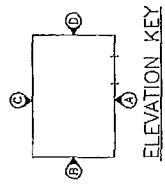


These prints comply with the Florida Manufactured Building Occupancy Code, Chapter 630, Part 1, Section 630.01(1) and (2) as amended.

Approved By: **NIA INC.**

Approval of this document does not constitute or approve any violation or deviation from the requirements of applicable State laws.

- NOTES:
1. ALL REBAR GRADE 60 TYPICAL. SEE MISC. DETAIL SHEET FOR REBAR SIZE & SPLICE LENGTHS.
 2. DIAGONAL DIMENSION-204.7/15"
 3. ALL REBAR TO BE TIED AT EACH CROSS OVER.



ELEVATION KEY