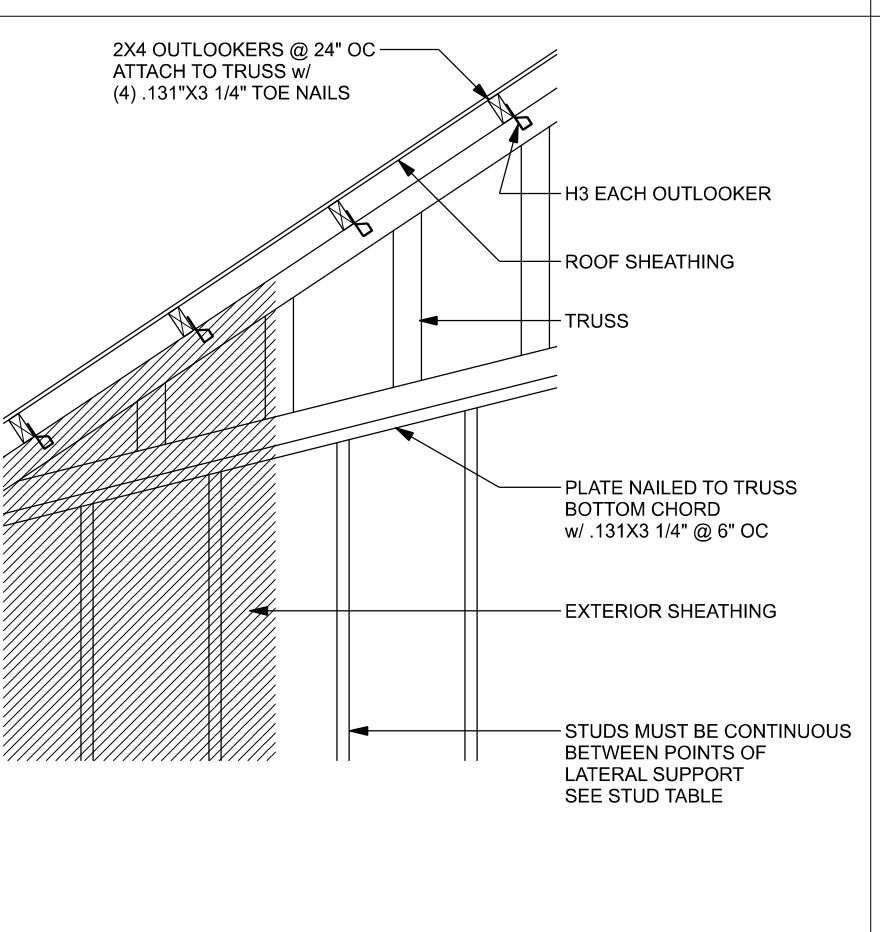
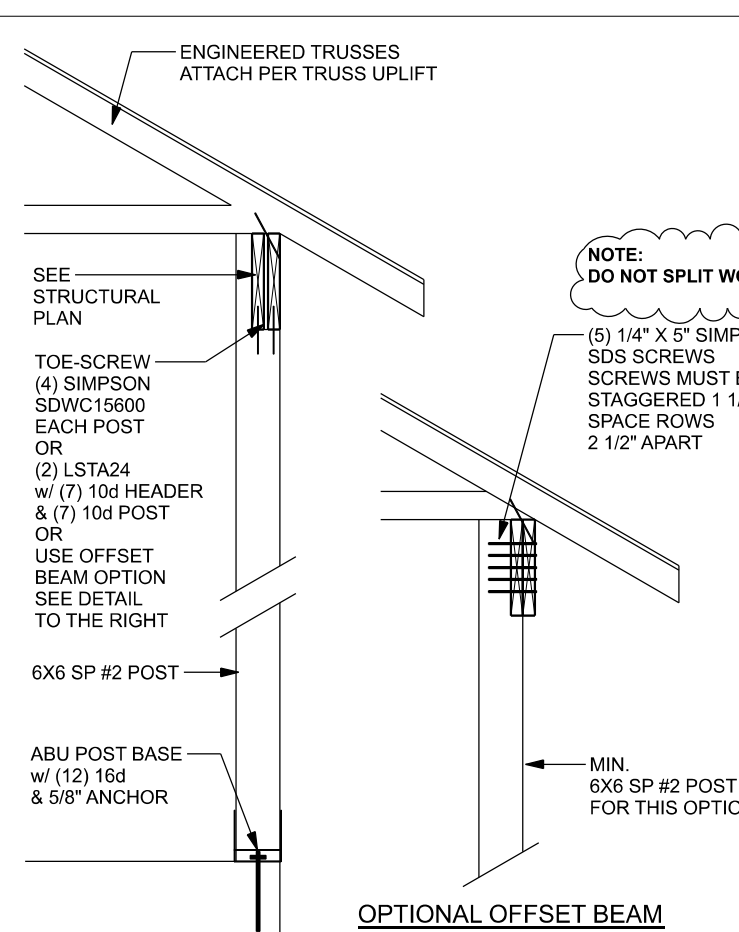


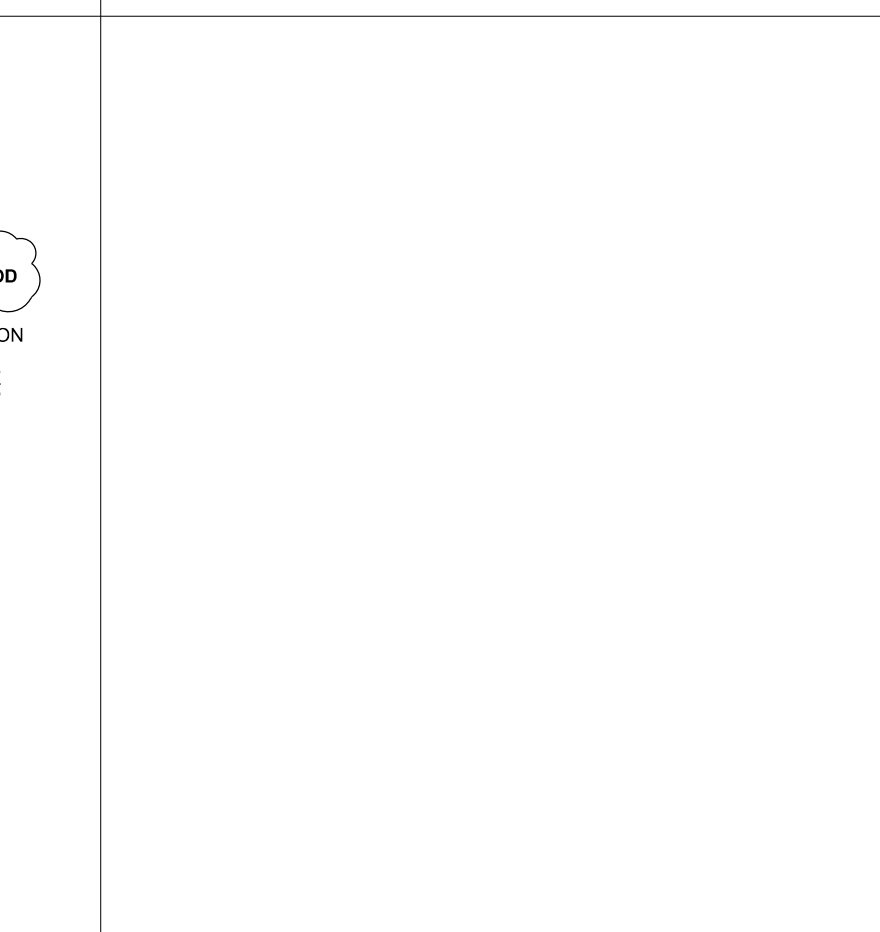
**(TYP.) INTERIOR BEARING WALL ATTACHMENT DETAILS**



**(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ RODS**



**(TYP.) INTERIOR BEARING WALL w/ SCREW OPTION**



**CONNECTOR TABLE**

| Uplift SP | Uplift SPF | Truss Connector       | To Plate               | To Truss/Rafter          |
|-----------|------------|-----------------------|------------------------|--------------------------|
| 605       | 505        | SDWC15600             | -                      | -                        |
| 600       | 290        | H3                    | 4-131"x1 1/2"          | 4-131"x1 1/2"            |
| 625       | 540        | H2 5A                 | 5-131"x1 1/2"          | 5-131"x1 1/2"            |
| 1040      | 1015       | H10A                  | 9-148"x1 1/2"          | 9-148"x1 1/2"            |
| 645       | 515        | LTS12-20              | 6-148"x1 1/2"          | 6-148"x1 1/2"            |
| 690       | 850        | MTS12-30              | 7-148"x1 1/2"          | 7-148"x1 1/2"            |
| 1415      | 1215       | HTS16-30              | 8-148"x1 1/2"          | 8-148"x1 1/2"            |
| 1040      | 1015       | H10A                  | 9-148"x1 1/2"          | 9-148"x1 1/2"            |
| 1235      | 1235       | LSTA21                | 8-148"x1 1/2"          | 8-148"x1 1/2"            |
| 1340      | 1460       | MSTA24                | 9-148"x1 1/2"          | 9-148"x1 1/2"            |
| 1030      | 1030       | CS20                  | 7-148"x1 1/2"          | 7-148"x1 1/2"            |
| 1515      | 1215       | HTS16-30              | 8-148"x1 1/2"          | 8-148"x1 1/2"            |
| 1010      | 535        | SP1                   | 4-148"x3"              | 4-148"x3"                |
| 1010      | 605        | SP2                   | 6-148"x3"              | 6-148"x3"                |
| 1280      | 1100       | SPH46                 | 12-148"x1 1/2"         | wrap under or over plate |
| 771       | 771        | LSTA24                | 10-148"x1 1/2"         | wrap under or over plate |
| 1235      | 1235       | LSTA24                | 14-148"x1 1/2"         | wrap under or over plate |
| Uplift SP | Uplift SPF | Holdowns @ Stewwall   | To Stud / Post         | Anchor                   |
| 2145      | 1835       | DTT22                 | 8-SDS 1/4"x1 1/2"      | 1/2"x12" Titen HD        |
| 4235      | 3640       | HTT4                  | 18-162"x2 1/2"         | 1/2"x12" Titen HD        |
| Uplift SP | Uplift SPF | Holdowns @ Mono       | To Stud / Post         | Anchor                   |
| 2145      | 1835       | DTT22                 | 8-SDS 1/4"x1 1/2"      | 1/2"x6" Titen HD         |
| 4235      | 3640       | HTT4                  | 18-162"x2 1/2"         | 1/2"x12" Titen HD        |
| Uplift SP | Uplift SPF | Post Bases @ Stewwall | To Post                | Anchor                   |
| 1900      | ABU442     | 12-162"x3 1/2"        | 5/8"x12" Drill & Epoxy |                          |
| 2475      | ABU662     | 12-162"x3 1/2"        | 5/8"x12" Drill & Epoxy |                          |
| Uplift SP | Uplift SPF | Post Bases @ Mono     | To Post                | Anchor                   |
| 1900      | ABU442     | 12-162"x3 1/2"        | 5/8"x12" Drill & Epoxy |                          |
| 2475      | ABU662     | 12-162"x3 1/2"        | 5/8"x12" Drill & Epoxy |                          |

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

**THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)**

| Uplift           | Uplift SP        | Uplift SPF       | Post             | Stud             | Anchor           |
|------------------|------------------|------------------|------------------|------------------|------------------|
| (1) 2x4 @ 16" OC | (1) 2x4 @ 16" OC | (1) 2x4 @ 16" OC | (1) 2x6 @ 16" OC | (1) 2x6 @ 16" OC | (1) 2x6 @ 12" OC |
| (1) 2x4 @ 12" OC | (1) 2x4 @ 12" OC | (1) 2x4 @ 12" OC | (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC |
| (1) 2x6 @ 16" OC | (1) 2x6 @ 16" OC | (1) 2x6 @ 16" OC | (1) 2x6 @ 16" OC | (1) 2x6 @ 16" OC | (1) 2x6 @ 12" OC |
| (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC | (1) 2x6 @ 12" OC |

**GRADE & SPECIES TABLE**

| Grade | Species      | Fd   | E   |
|-------|--------------|------|-----|
| 2x8   | SP #2        | 925  | 1.4 |
| 2x10  | SP #2        | 800  | 1.4 |
| 2x12  | SP #2        | 750  | 1.4 |
| GLB   | 24F-V3 SP    | 2600 | 1.9 |
| LVL   | TIMBERSTRAND | 1700 | 1.7 |
| LVL   | MICROLAM     | 2900 | 2.0 |
| PSL   | PARLAM       | 2900 | 2.0 |

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE TRUSS DESIGNER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED WITH ALL REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR DESIGN OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415L EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN IS A SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PREFERRED).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 2500$  PSI. WELDED WIRE REINFORCED SLAB 6" x 6" w/ #4 x w/ #4. FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC 3/8" x 18" CONFORMING TO ASTM A185. LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.075 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1119. SUPPLIER TO PROVIDE ASTM C 1119 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12'. DO NOT CUT WWF OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAP SPACINGS 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 308.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR ANOTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

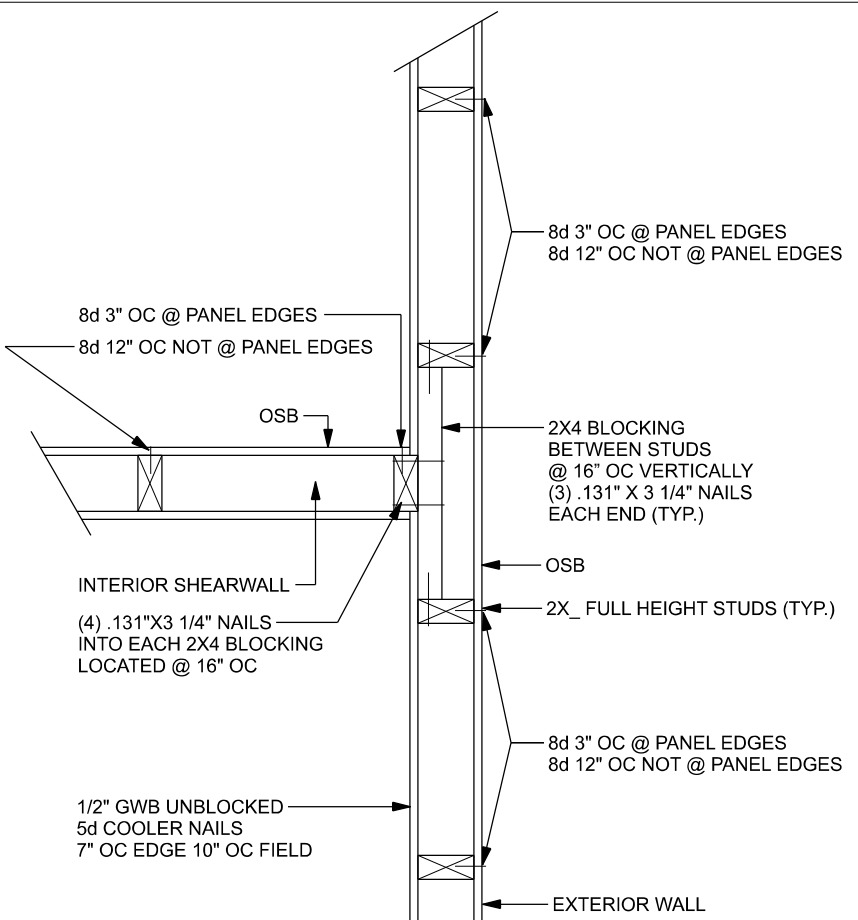
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

- CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
- PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FIBC REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
- PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
- VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

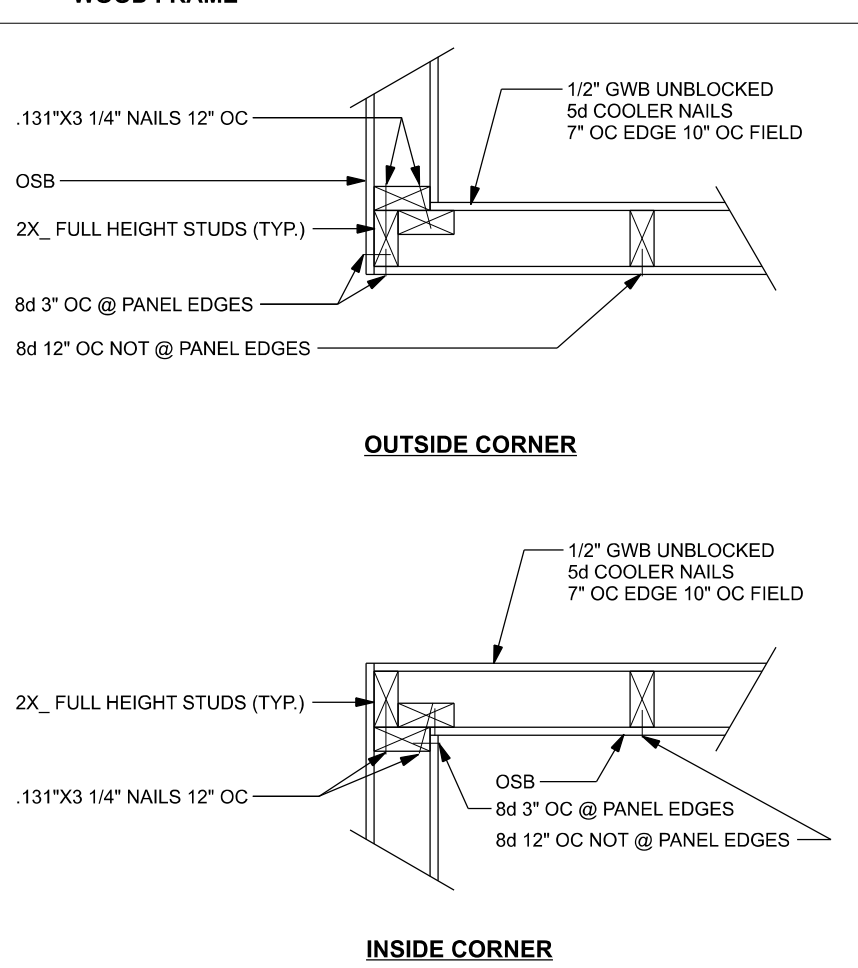
**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FIBC IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBC REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

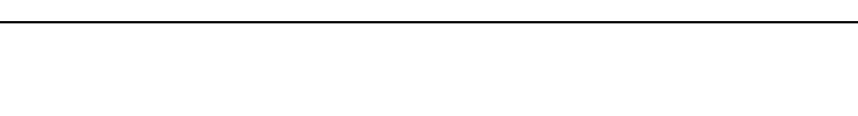
**ONE STORY WALL SECTION**



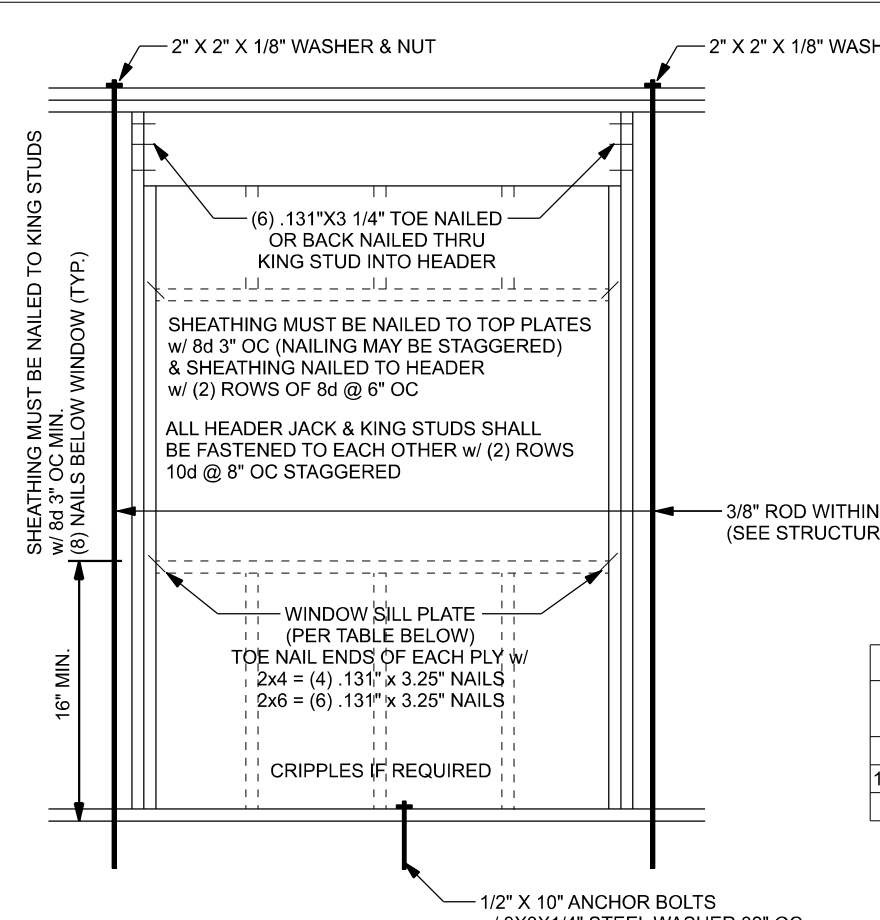
**(TYP.) INTERSECTING WALL FRAMING**



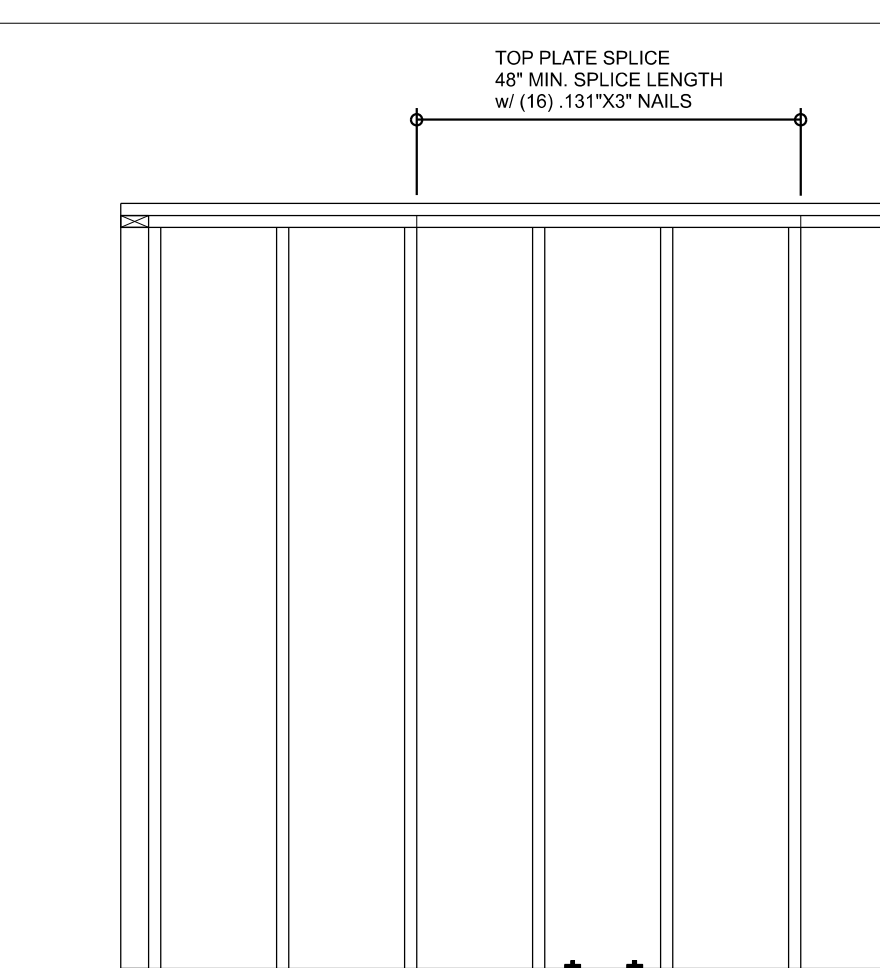
**(TYP.) CORNER FRAMING**



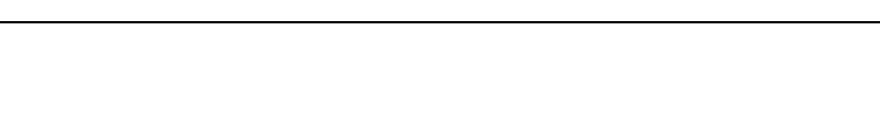
**(TYP.) GABLE BRACING DETAIL**



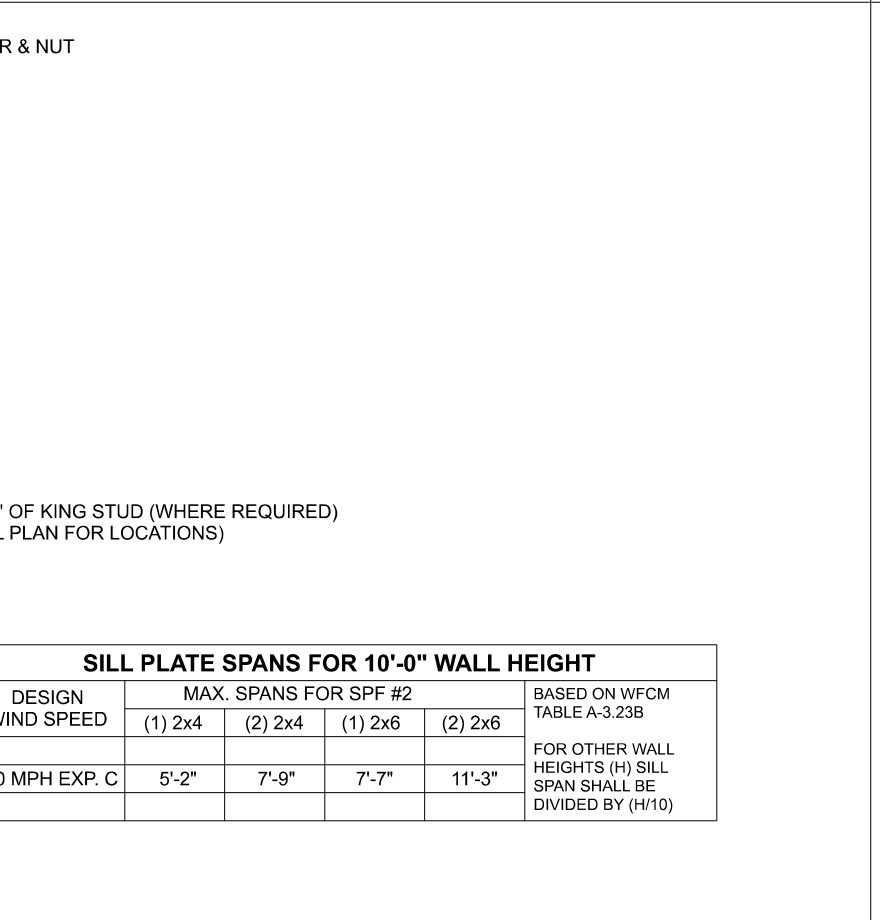
**TYPICAL HEADER STRAPPING DETAIL**



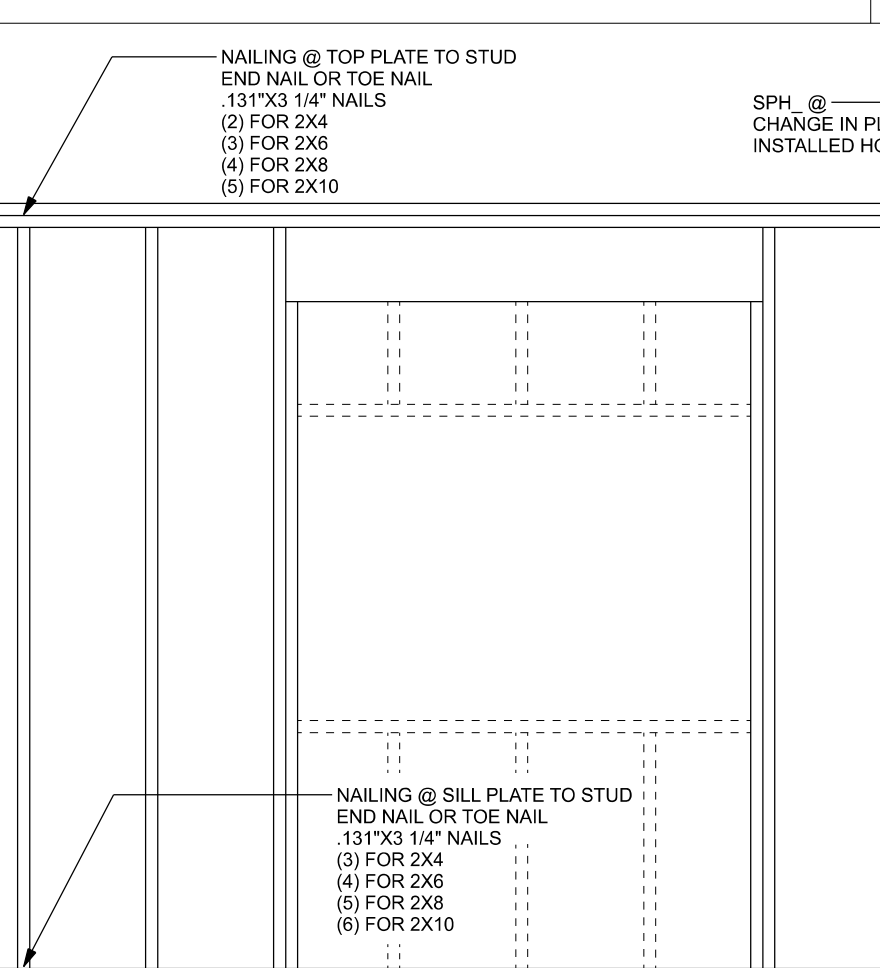
**(TYP.) WALL CONNECTIONS**



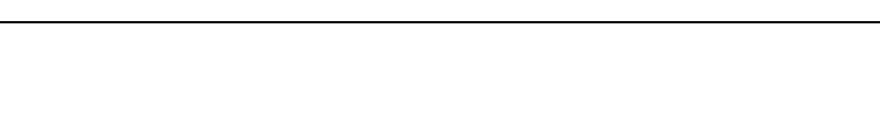
**(TYP.) GABLE WALL w/ VAULTED CEILING**



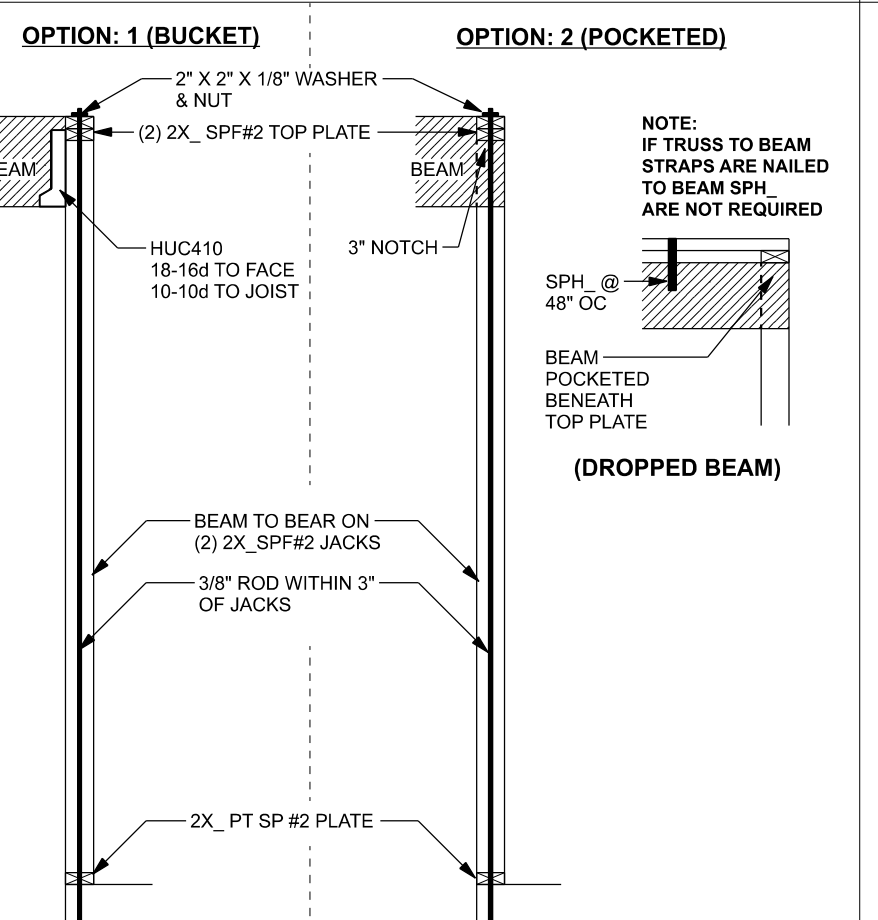
**(TYP.) PORCH POST**



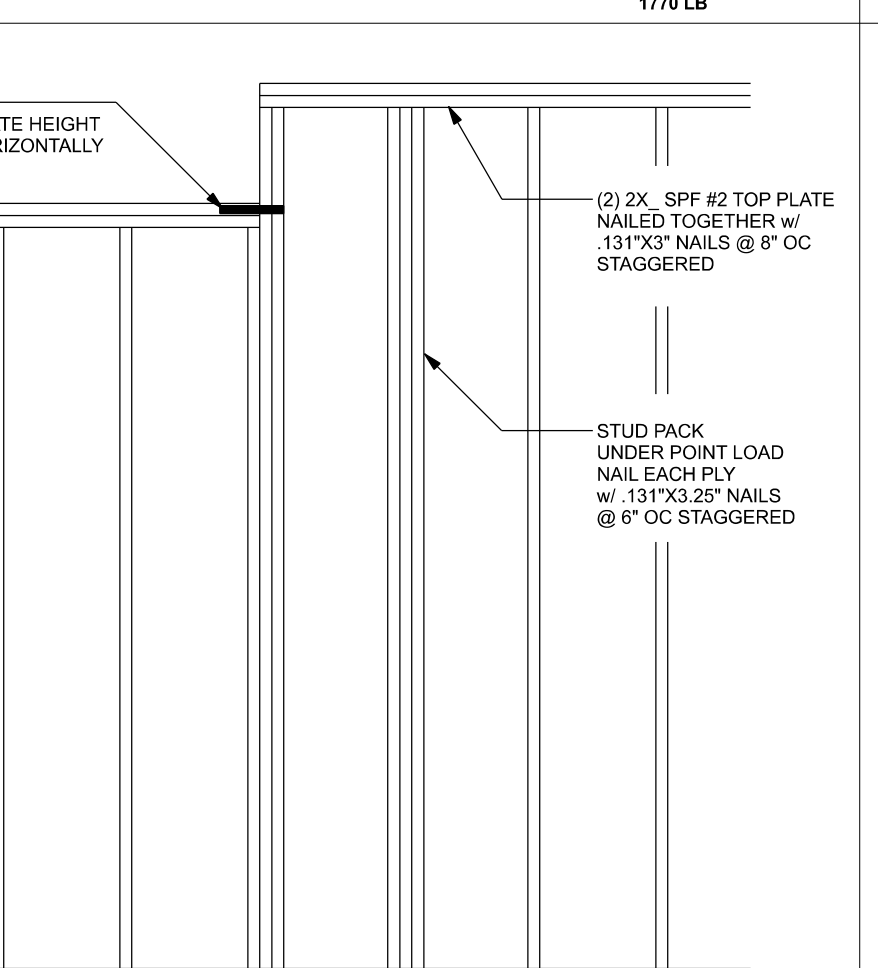
**(TYP.) BEAM TO WALL**



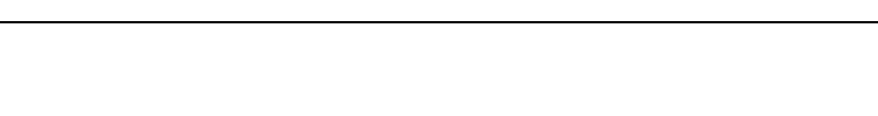
**(TYP.) BEAM TO WALL**



**(TYP.) GARAGE DOOR BUCK ATTACHMENT**



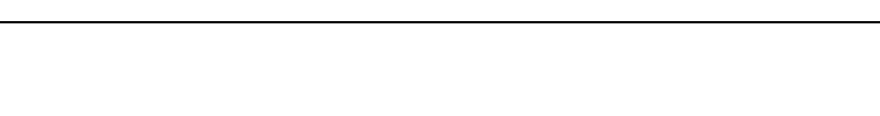
**(TYP.) GARAGE DOOR BUCK INSTALLATION**



**(TYP.) WALL CONNECTIONS**

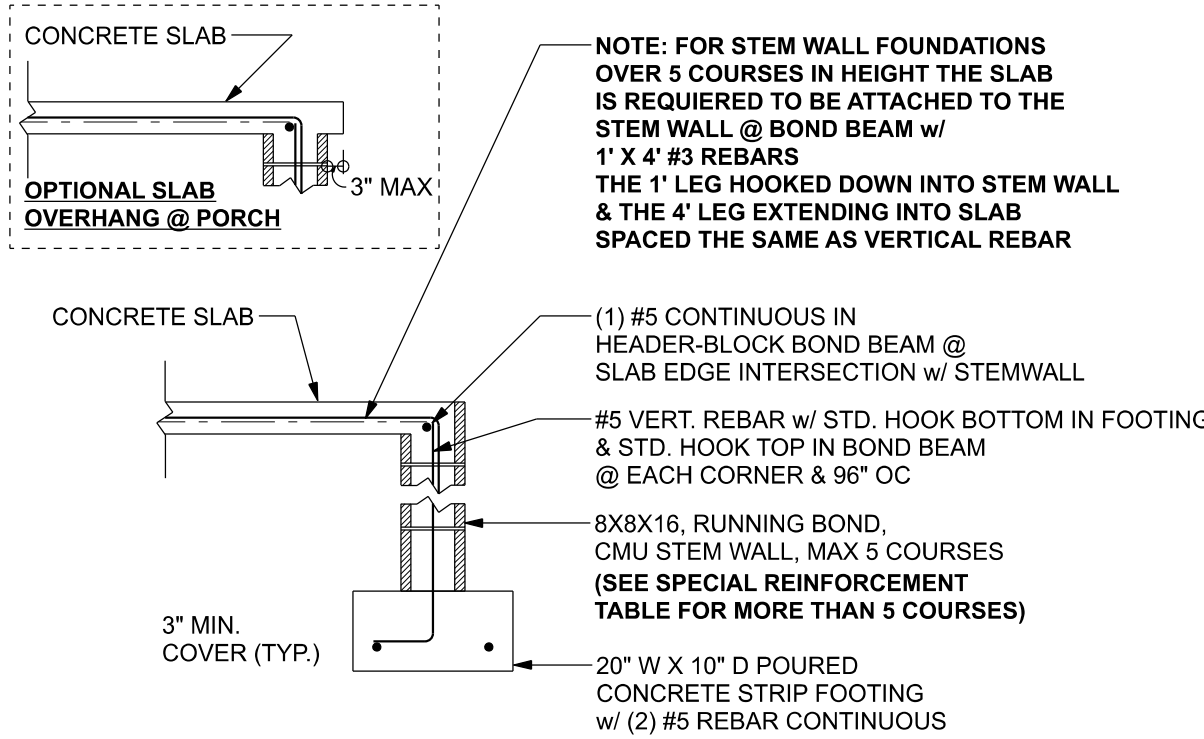


**(TYP.) GARAGE DOOR BUCK INSTALLATION**



**DESIGN CRITERIA & LOADS:**

|                                                |                                                      |
|------------------------------------------------|------------------------------------------------------|
| BUILDING CODE                                  | 8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023) |
| CODE FOR DESIGN LOADS                          | ASCE 7-22                                            |
| <b>WINDLOADS</b>                               |                                                      |
| BASIC WIND SPEED (ASCE 7-22, 3S GUST)          | 130 MPH                                              |
| WIND EXPOSURE (BUILDER MUST FIELD VERIFY)      | C                                                    |
| TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY) | I                                                    |
| RISK CATEGORY                                  | II                                                   |
| ENCLOSURE CLASSIFICATION                       | ENCLOSED                                             |
| INTERNAL PRESSURE COEFFICIENT                  | 0.18                                                 |
| ROOF ANGLE                                     | 7.45 DEGREES                                         |
| MEAN ROOF HEIGHT                               | 30 FT                                                |
| <b>C&amp;D DESIGN PRESSURES</b>                | SEE TABLE                                            |
| <b>FLOOR LOADING</b>                           |                                                      |
| ROOMS OTHER THAN SLEEPING ROOM                 | 40 PSF LIVE LOAD                                     |
| SLEEPING ROOM                                  | 30 PSF LIVE LOAD                                     |
| SLEEPING ROOMS                                 | 30 PSF LIVE LOAD                                     |
| <b>ROOF LOADING</b>                            |                                                      |
| FLAT OR < 4:12                                 | 20 PSF LIVE LOAD                                     |
| 4:12 TO < 12:12                                | 16 PSF LIVE LOAD                                     |
| 12:12 & GREATER                                | 12 PSF LIVE LOAD                                     |
| <b>SOIL BEARING CAPACITY</b>                   | 1500 PSF                                             |
| <b>FLOOD ZONE</b>                              | THIS BUILDING IS NOT IN THE FLOOD                    |



**TALL STEM WALL TABLE:**  
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

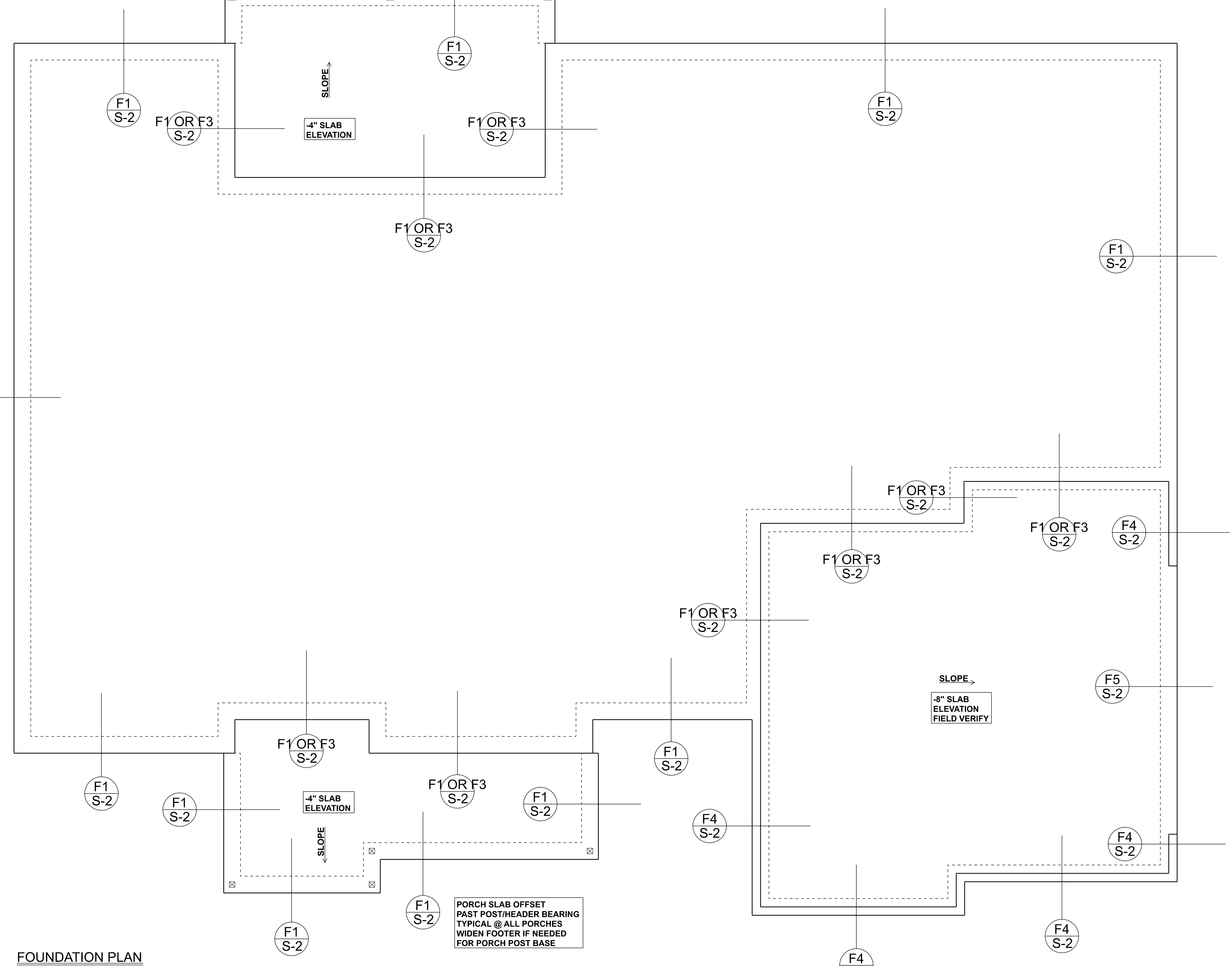
| STEM WALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.) |    |    | VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.) |    |    |
|-------------------------|----------------------------|-----------------------------------------------------------|----|----|------------------------------------------------------------|----|----|
|                         |                            | #5                                                        | #7 | #8 | #5                                                         | #7 | #8 |
| 3.3                     | 3.0                        | 96                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 4.0                     | 3.7                        | 96                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 4.7                     | 4.3                        | 88                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 5.3                     | 5.0                        | 56                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 6.0                     | 5.7                        | 40                                                        | 80 | 96 | 80                                                         | 96 | 96 |
| 6.7                     | 6.3                        | 32                                                        | 56 | 80 | 56                                                         | 96 | 96 |

**THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING**

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| ACI 530.1-02 Section | Specific Requirements                                                                                                                                                                                           |
|----------------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 1.4A                 | Compressive strength: 8" block bearing walls: F'm = 1500 psi                                                                                                                                                    |
| 2.1                  | Mortar: ASTM C 270, Type N, UNO                                                                                                                                                                                 |
| 2.2                  | Grout: ASTM C 476, admixtures require approval                                                                                                                                                                  |
| 2.3                  | CMU standard: ASTM C 90-02, Normal weight, hollow; medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block                                                                            |
| 2.3                  | Clay brick standard: ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"                                                                                                                                        |
| 2.4                  | Reinforcing bars, #3 - #11: ASTM A615, Grade 60, Fy = 60 ksi, Lap splices min 40 bar dia. (25" for #5)                                                                                                          |
| 2.4F                 | Coating for corrosion protection: Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 950, 0.50 oz/lb or 304SS                                                                   |
| 2.4F                 | Coating for corrosion protection: Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS |
| 3.3.2                | Pipes, conduits, and accessories: Any not shown on the project drawings require engineering approval.                                                                                                           |
| 3.3.2                | Pipes, conduits, and accessories: Any not shown on the project drawings require engineering approval.                                                                                                           |
| 3.3.2                | Pipes, conduits, and accessories: Any not shown on the project drawings require engineering approval.                                                                                                           |
| 3.3.7                | Movement joints: Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.                                                                                |

PORCH SLAB OFFSET PAST POST/HEADER BEARING TYPICAL @ ALL PORCHES WIDEN FOOTER IF NEEDED FOR PORCH POST BASE



**FOUNDATION PLAN**

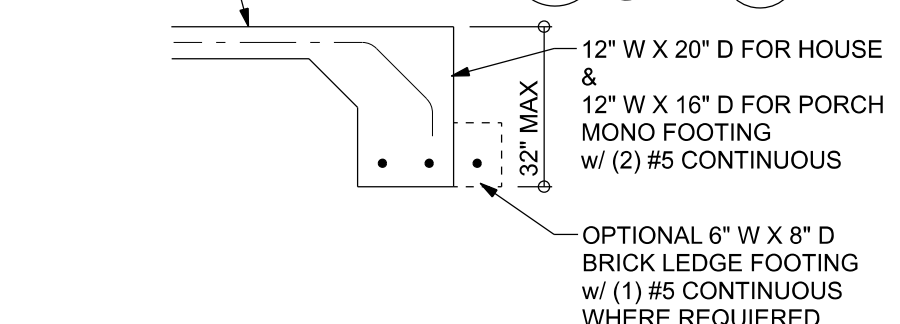
SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

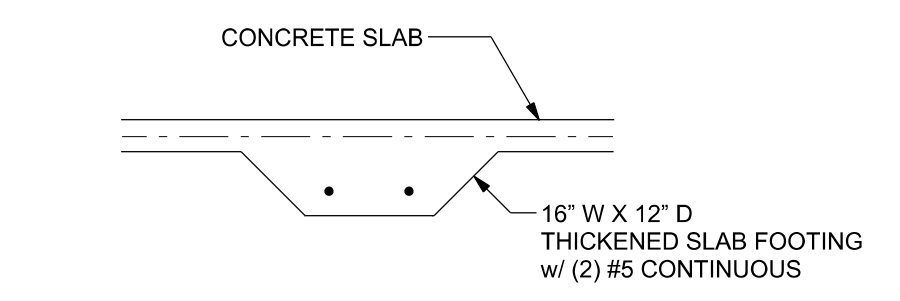
- DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISCREPANCY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
- THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6x6x4/4 WELDED WIRE MESH PLACED ON CHAIRS # 1/2" DEPTH OR FIBER MESH CONCRETE, w/ 1/2" POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

**F1 S-2 OPTIONAL STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"

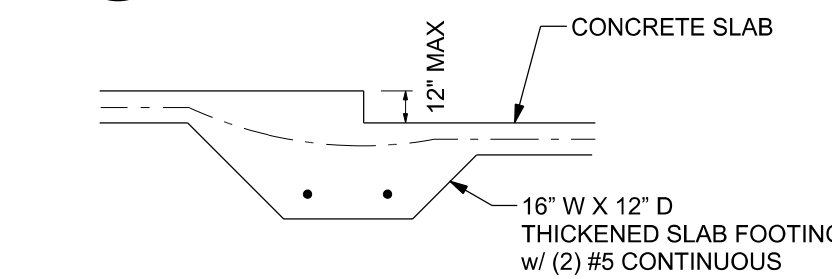
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



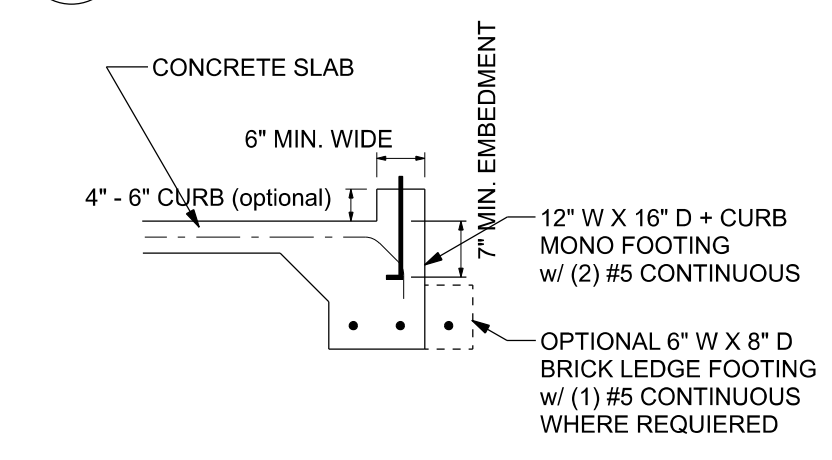
**F1 S-2 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



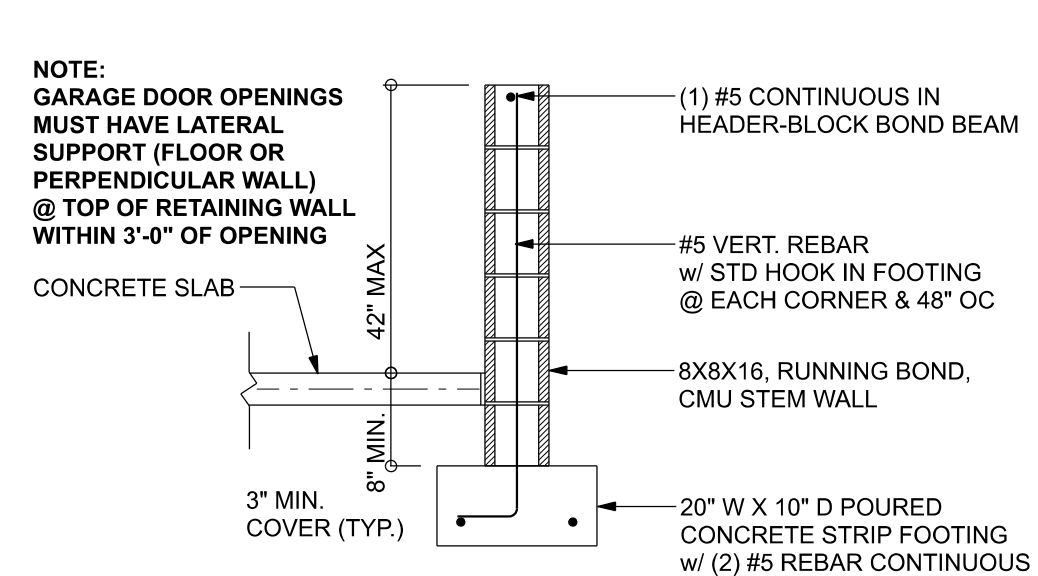
**F2 S-2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



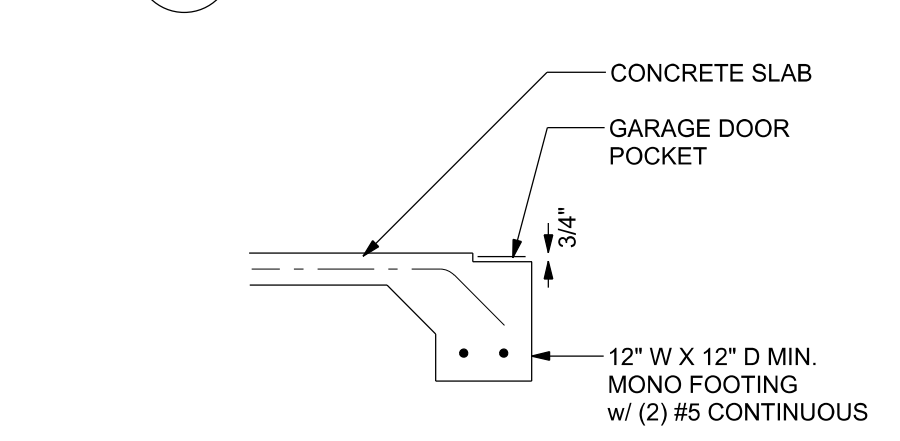
**F3 S-2 INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"



**F4 S-2 MONOLITHIC CURB FOOTING**  
SCALE: 1/2" = 1'-0"



**F4 S-2 OPTIONAL STEM WALL CURB FOOTING**  
SCALE: 1/2" = 1'-0"



**F5 S-2 GARAGE DOOR POCKET FOOTING**  
SCALE: 1/2" = 1'-0"

Gibraltar Contracting, LLC  
Gonzalez Res.  
PROJECT ADDRESS: Lot 6 Hills of Huntsville NW Milb Terr., Lake City, FL 32055

FL PE 53915  
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3/12/2026

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

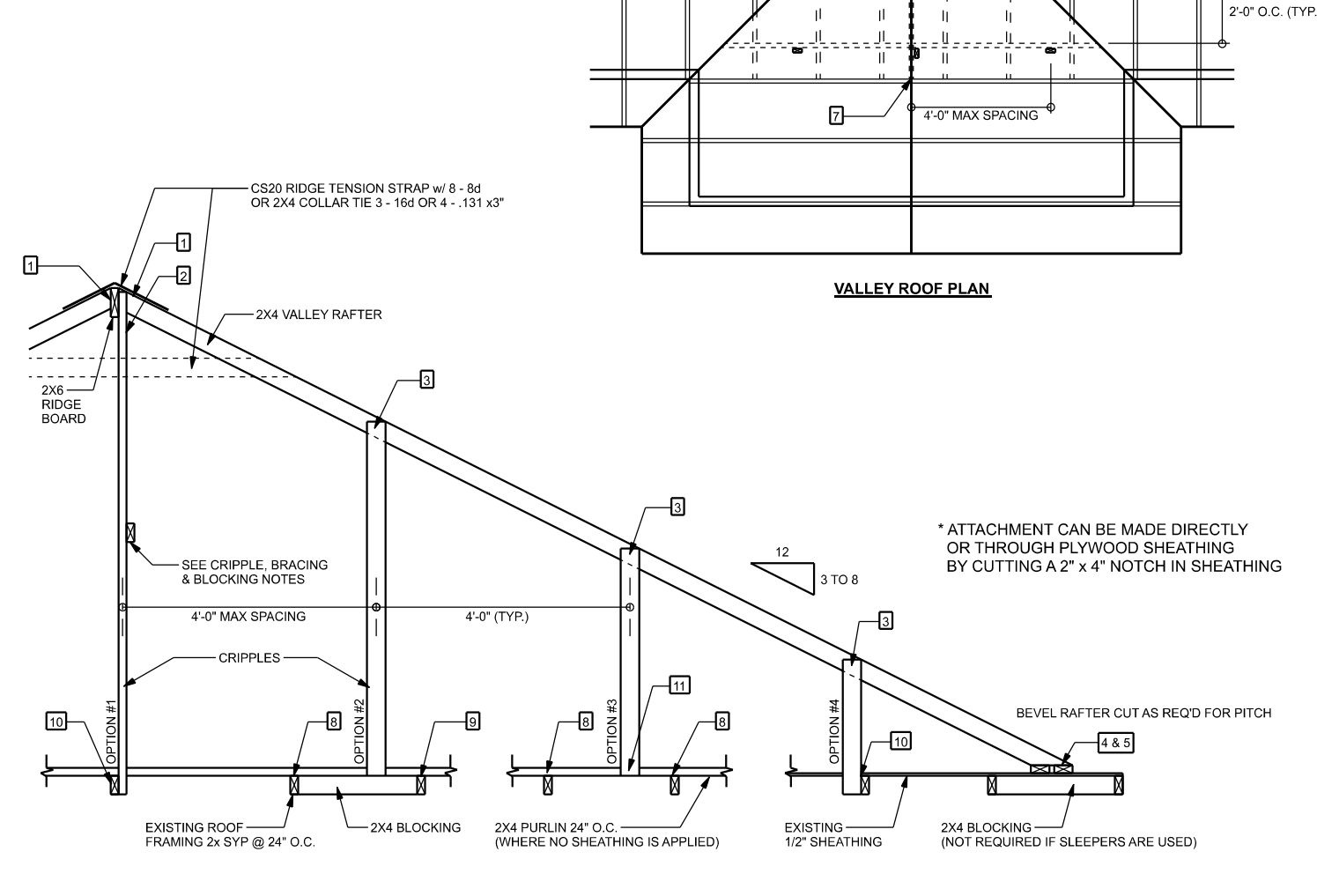
**LIMITATION:** This design is valid for one building, at specified location.

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JOB NUMBER:  
260205

**S-2**  
OF 3 SHEETS

|                             |                                                              |
|-----------------------------|--------------------------------------------------------------|
| RISE BOARD                  | 2X4 SYP #2                                                   |
| RAFTER SPANS 20'-0" OR LESS | 2X4 SYP #2                                                   |
| PURLIN - LATERAL BRACING    | 2X4 SYP #2                                                   |
| SLEEPERS                    | 2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2x INCHES 2X4 SYP #2 |
| CRIPPLES & BLOCKING         | 2X4 SYP #2 OR BETTER                                         |
| TRUSS BELOW                 | SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL                    |

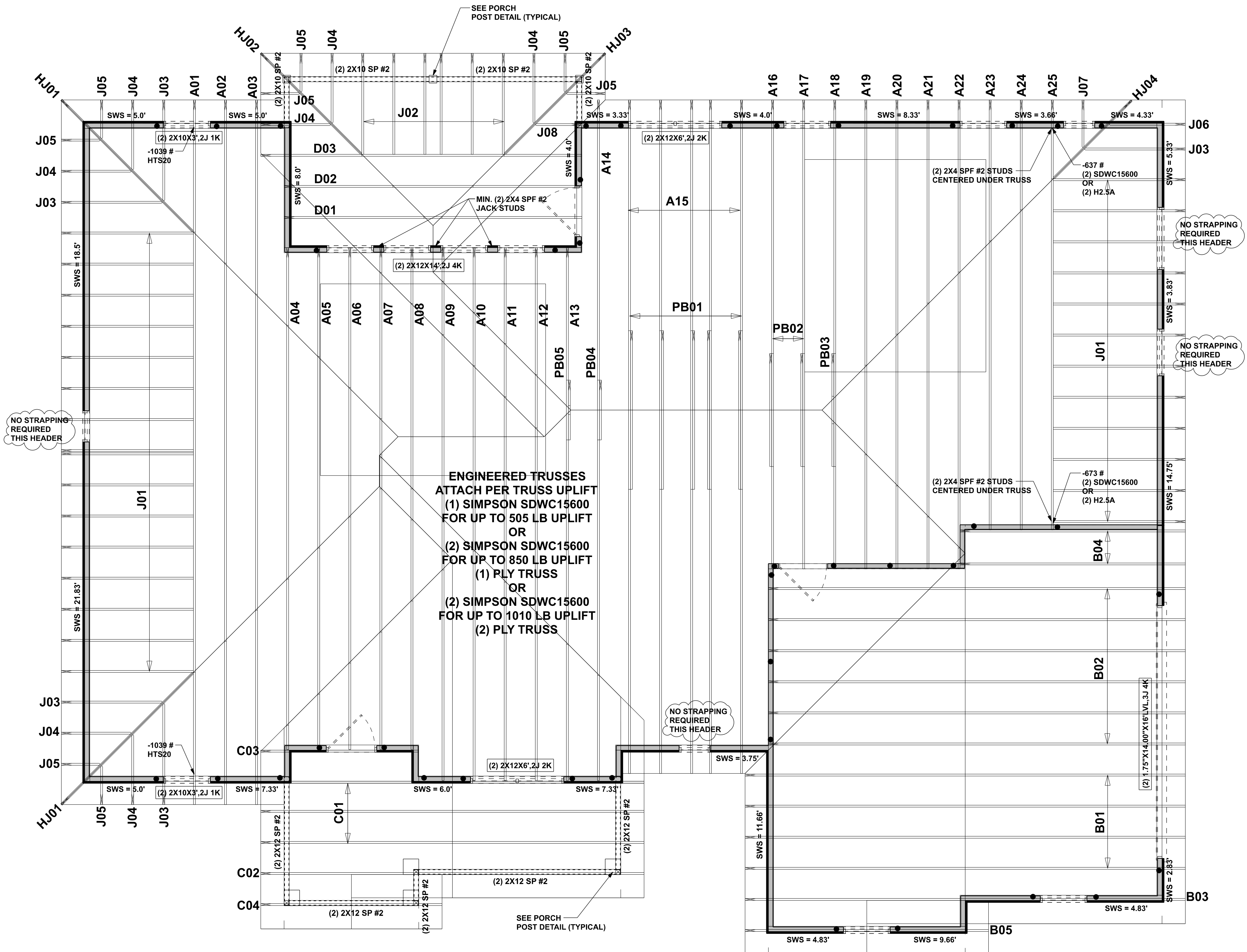


**GENERAL NOTES**

MAXIMUM RAFTER SPAN: 6'-0" OR 2X4 SYP OR 2X8 SYP #2 OR SYP #2  
 MAXIMUM ROOF AREA PER SUPPORT: 160 IN ZONE 2 & 3, 240 IN ZONE 1 (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN)  
 PURLIN REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED  
 PURLIN REQUIRED OVER LAP SHEATHING ONE TRUSS SPACING MINIMUM IN CASES THAT THIS IS IMPRACTICAL. OVER LAP SHEATHING MINIMUM OF 4" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-#6 COMMON WIRE NAILS  
 THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:  
 - SPRING DIVISIONS BE TWICE THE RISE 40:12 OR LESS  
 - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS  
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET  
 - MAXIMUM TOTAL LOADS: 40 PSF  
 - MEETS FBC / ASCE 7-16 WIND REQUIREMENTS  
 - EXPOSURE CATEGORY "C" 1, 1.1, 1.2, 1.3  
 - ENCLOSED BUILDING

**CRIPPLE BRACING & BLOCKING NOTES**

- 2X4 CONTINUOUS LATERAL BRACE (CLB) WITH 16 REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED #2-16d NAILS OR 2X4 "1" OR SCAB BRACE NAIL TO FLAT EDGE OF CRIPPLE WITH 16d NAILS @ 8" O.C. "1" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES w/ "1" OR SCAB. USE STRESS GRADED LUMBER & BOB OR COMMON NAILS  
 - NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER. ALSO CHECK THE PROPER NUMBER OF NAILS ARE INCLUDED INTO HEADER BRING  
 - INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.  
 - INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN SLEEPERS  
 - LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED  
 - NAILS ALL NAILING ACCORDANCE TO 16d-16d SECTION 12 NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

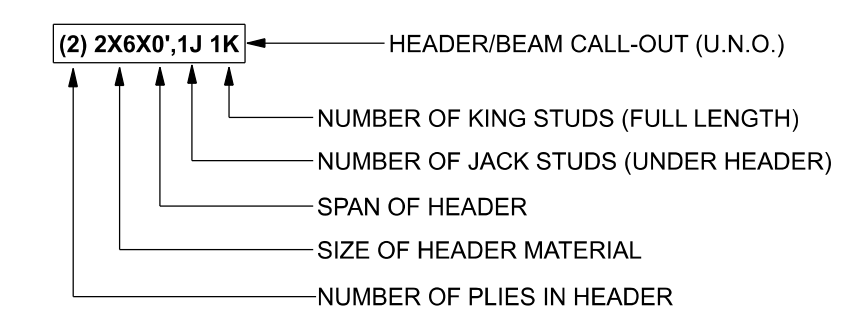


**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

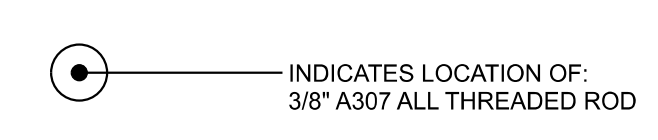
**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**HEADER LEGEND**



**THREADED ROD LEGEND**



|          | TRANSVERSE | LONGITUDINAL |
|----------|------------|--------------|
| ACTUAL   | 21775 LBF  | 19771 LBF    |
| REQUIRED | 16881 LBF  | 13252 LBF    |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND TRUSS CO. JOB #21-5392

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**S-3**  
 OF 3 SHEETS