(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B53 2-PLY)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2 :B3 2x8 SP #2:
Webs 2x4 SP #3

120 mph wind, 22.53 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf.
Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

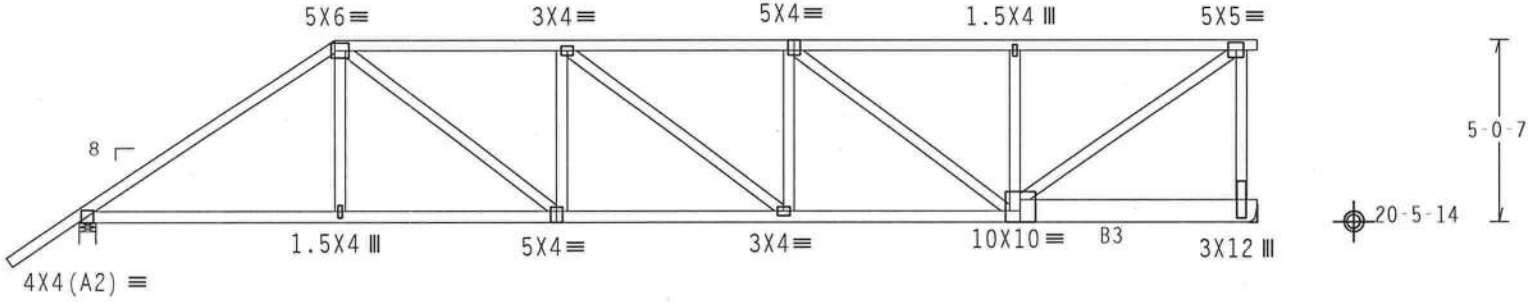
Hanger = Simpson HGUS26-2 or equivalent, fill all nail holes.

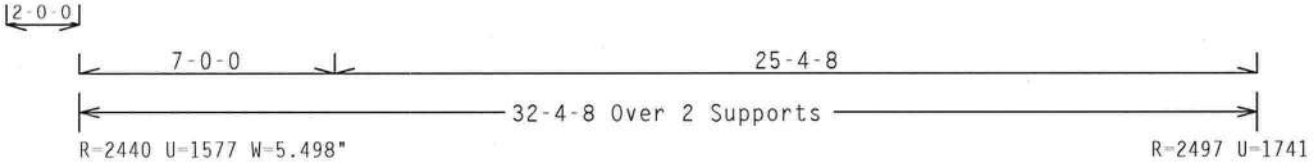
2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-2.04	6.88
TC	65	6.88	32.25
BC	91	0.02	32.25





Design Crit: FBC2007Res/TPI-2002(STD)
PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94289
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006020
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273483
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C54 3-PLY)

Top chord 2x4 SP #2
Bot chord 2x8 SP #2
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 57 plf at -2.00 to 57 plf at 12.38
TC - From 57 plf at 12.38 to 57 plf at 15.17
TC - From 54 plf at 15.17 to 54 plf at 20.27
TC - From 54 plf at 20.27 to 54 plf at 24.35
BC - From 20 plf at 0.00 to 20 plf at 15.79
BC - From 20 plf at 15.79 to 20 plf at 22.46
BC - From 20 plf at 22.46 to 20 plf at 24.35
PLB- 2497 LB Conc. Load at (7.13,20.53)
PLB- 1409 LB Conc. Load at (9.06,20.53)
PLB- 1579 LB Conc. Load at (10.73,20.53)
PLB- 1052 LB Conc. Load at (12.73,20.53) , (20.73,21.53), (22.73,20.53)
PLB- 1185 LB Conc. Load at (14.73,20.53)
PLB- 1240 LB Conc. Load at (16.73,21.53) , (18.73,21.53)

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	79	-2.13	12.25
TC	40	12.25	15.05
TC	63	15.05	20.15
TC	49	20.15	24.23
BC	120	0.02	15.67
BC	87	15.20	22.48
BC	21	22.48	24.27

3 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts MAY BE USED FOR (2) 0.131"x3" nails ON THE BOTTOM CHORD ONLY.
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

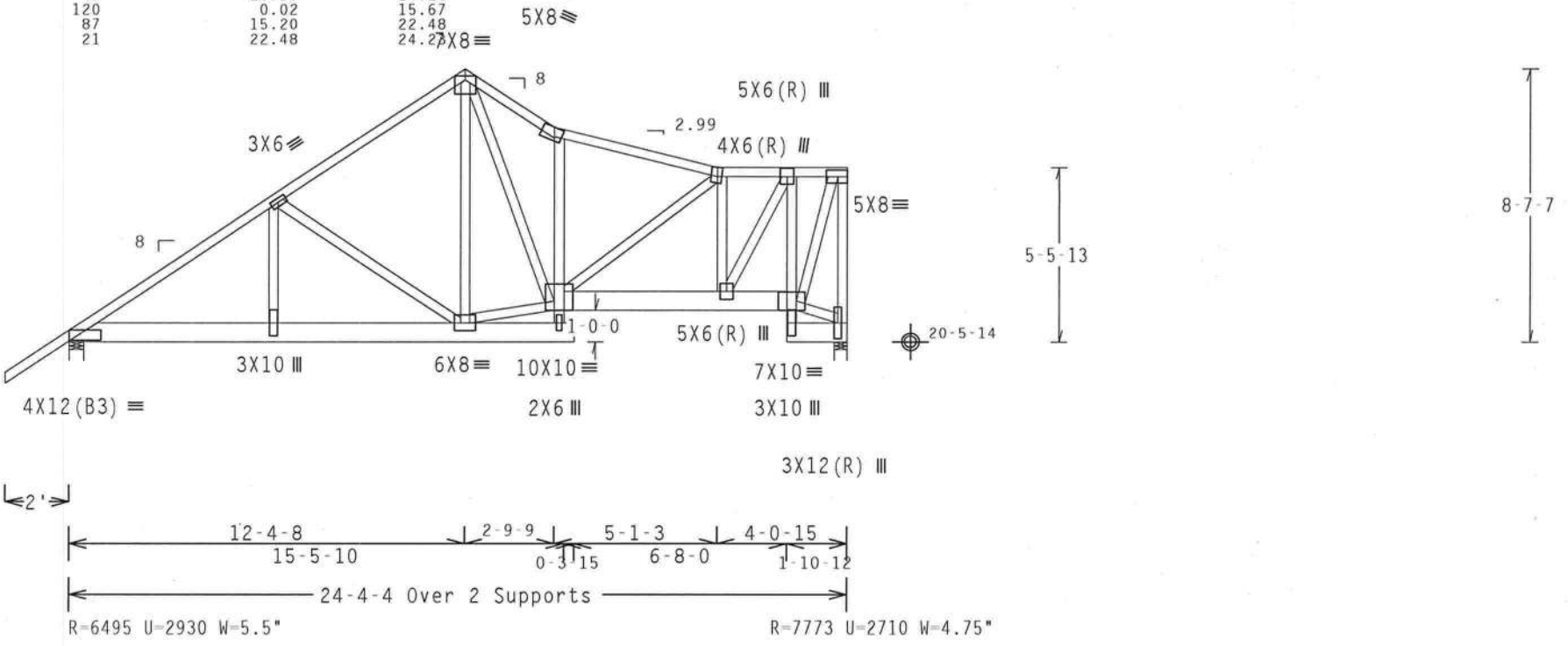
120 mph wind, 24.32 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2) 9.01.00.01

PLT TYP. Wave QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. BOLING, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94290
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006021
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273737
DUR.FAC.	1.25	FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C55)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (1)2x6 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.16 ft. from roof edge.

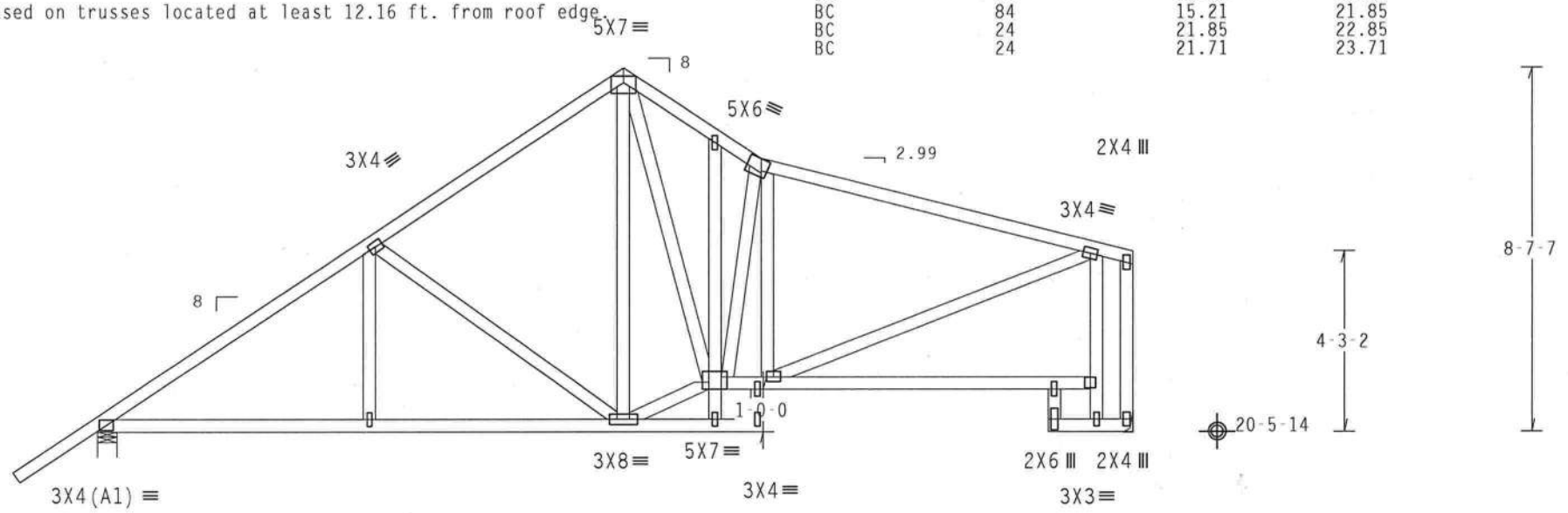
120 mph wind, 24.32 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

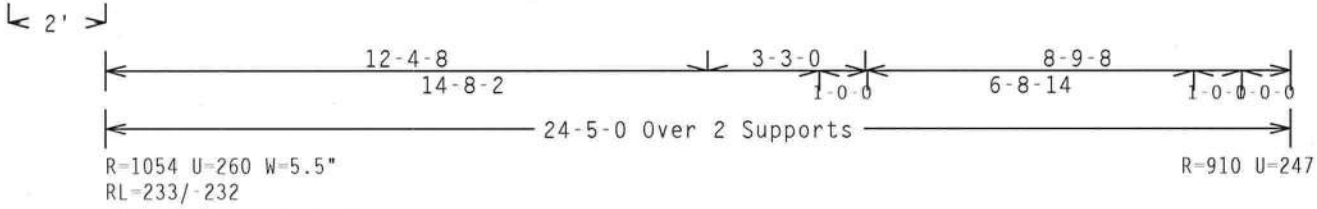
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	62	-2.63	11.67
TC	47	11.67	14.91
TC	51	14.91	23.71
BC	75	-0.56	14.97
BC	75	13.82	15.21
BC	84	15.21	21.85
BC	24	21.85	22.85
BC	24	21.71	23.71





Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES T. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94291
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006040
BC LL	0.0 PSF		HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223325
DUR.FAC.	1.25	FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C56)

Top chord 2x4 SP #2 :T1 2x6 SP #1 Dense:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

Calculated horizontal deflection is 0.15" due to live load and 0.13" due to dead load.

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (1)2x6 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.49 ft. from roof edge.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

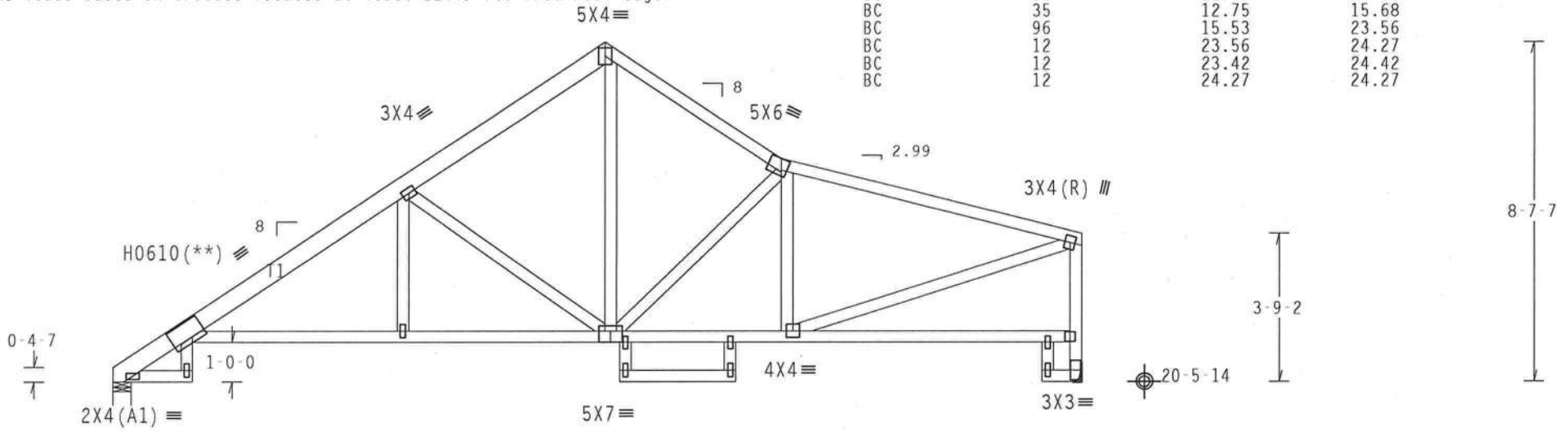
120 mph wind, 24.99 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	49	0.00	12.38
TC	64	12.38	16.82
TC	55	16.82	24.42
BC	18	0.36	1.87
BC	120	1.87	12.90
BC	35	12.90	15.53
BC	35	12.75	15.68
BC	96	15.53	23.56
BC	12	23.56	24.27
BC	12	23.42	24.42
BC	12	24.27	24.27



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/5(2) 9.01.00.021 QTY:1 FL/-/4/-/E/-/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.

JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94292
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006050
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223372
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C57)

Top chord 2x4 SP #2 :T1 2x6 SP #2:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 24.99 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

(H1) = (J) Hanger not calculated (1)2x4 SP #2 supporting member.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	18	-0.00	12.38
TC	68	12.38	18.02
TC	59	18.02	24.42
BC	20	0.36	2.00
BC	20	1.64	1.64
BC	120	1.64	12.90
BC	35	12.90	15.53
BC	35	12.75	15.68
BC	84	15.53	22.56
BC	24	22.56	24.27
BC	24	22.42	24.42
BC	24	24.27	24.27

(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 2 degrees.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

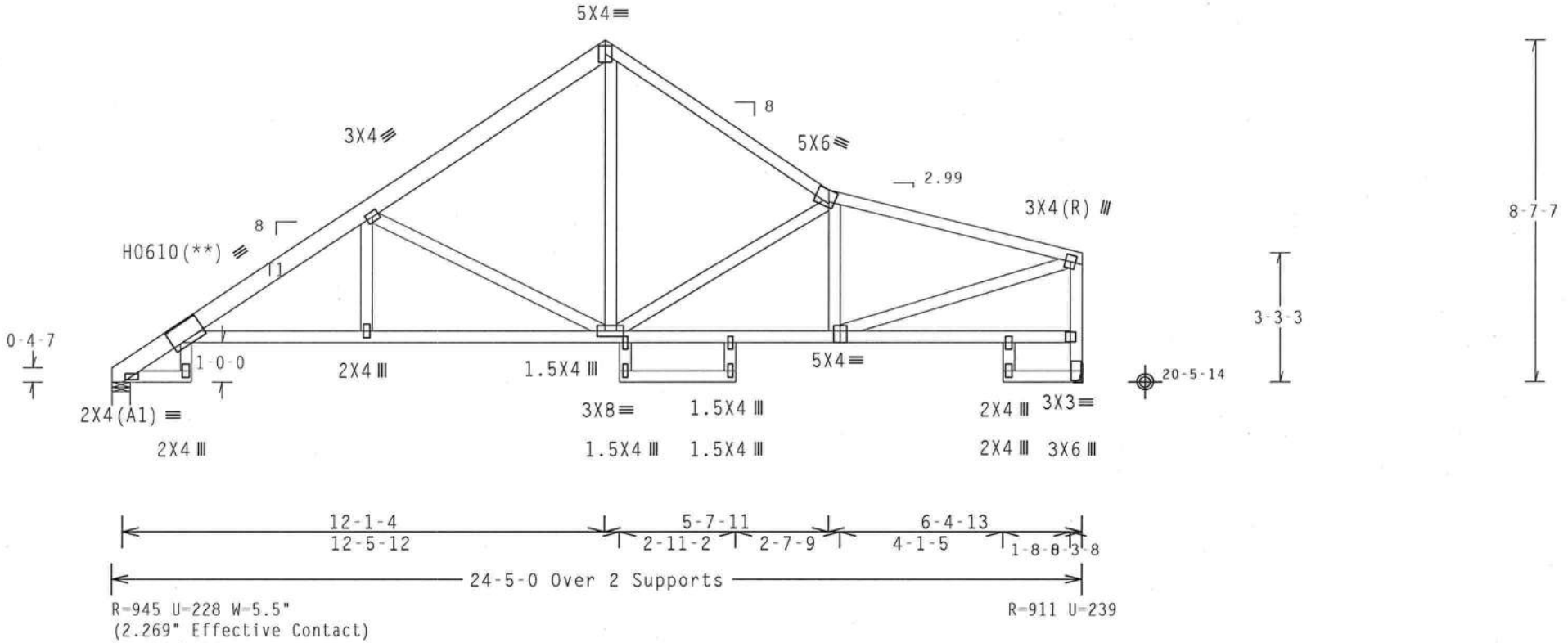
Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.49 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.

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TC LL	20.0 PSF	REF R235-- 94293
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006051
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223400
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2 :T1 2x6 SP #1 Dense:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 24.99 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.13" due to live load and 0.12" due to dead load.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	45	0.74	11.63
TC	65	11.63	18.47
TC	64	18.47	23.67
BC	18	0.38	1.13
BC	120	1.13	12.15
BC	75	12.15	14.15
BC	75	12.01	

23.67

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

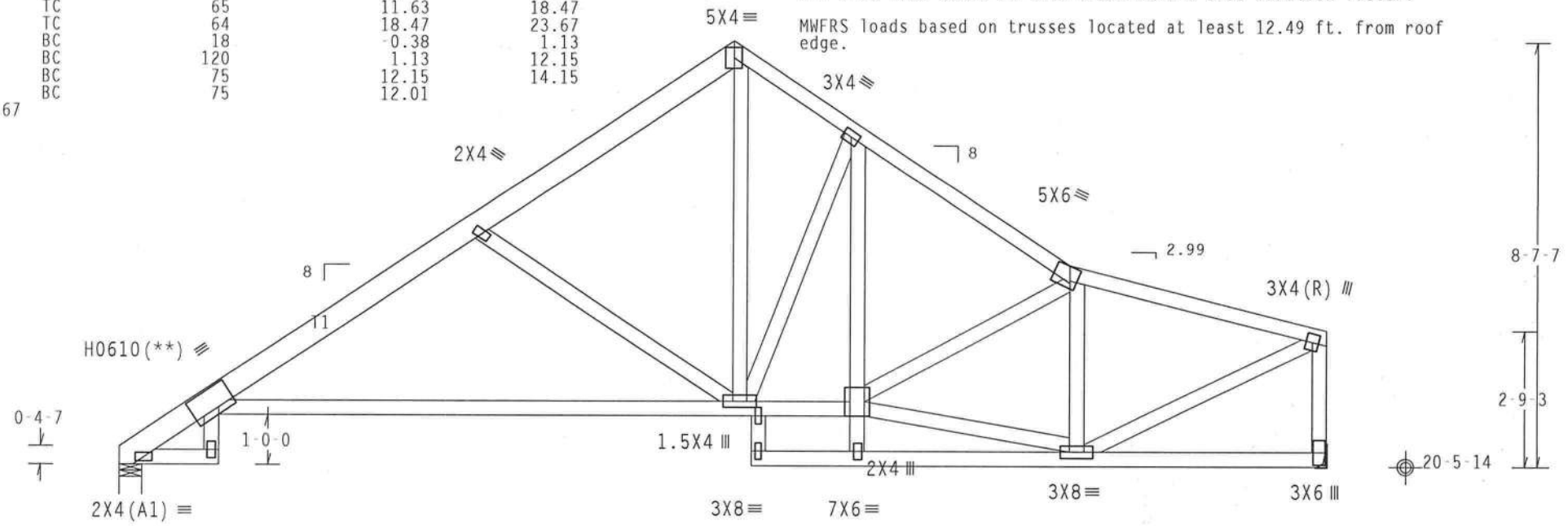
(H1) = (J) Hanger not calculated (1)2x4 SP #2 supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.49 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/5(2) 9.01.00.023 QTY:1 FL/-/4/-/E/-/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94294
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006057
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223419
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C59)

Top chord 2x6 SP #2 :T2 2x4 SP #2:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (2)2x8 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

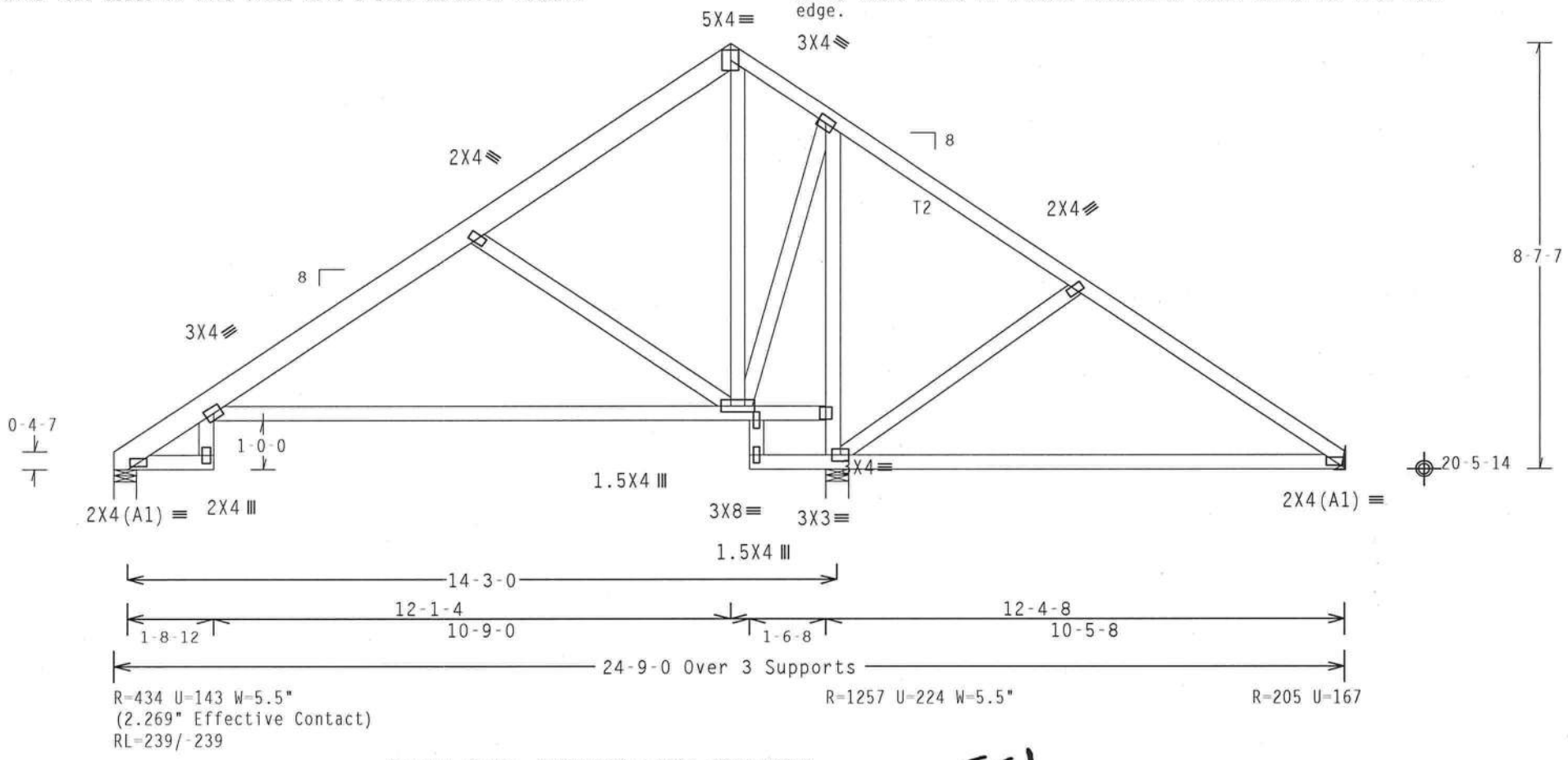
120 mph wind, 24.99 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	0.00	24.75
BC	18	0.36	1.87
BC	75	1.87	14.44
BC	75	12.75	24.63

MWFRS loads based on trusses located at least 12.49 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.13 QTY:1 FL/-/4/-/E/-/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.

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IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

JAMES F. COELING JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94295
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006044
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223438
DUR.FAC.	1.25	FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C60)

Top chord 2x4 SP #2 :T1 2x6 SP #2:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (2)2x8 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

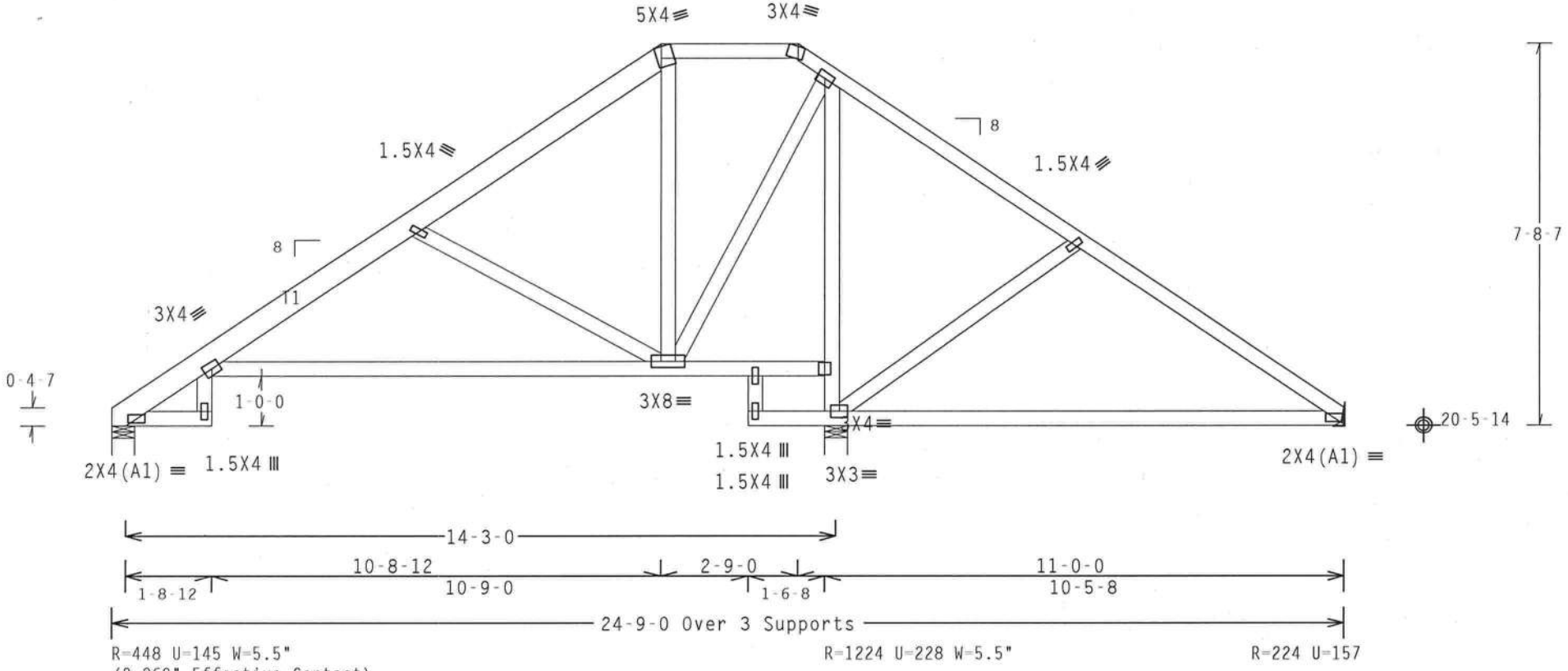
MWFRS loads based on trusses located at least 12.26 ft. from roof edge.

120 mph wind, 24.53 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-0.74	10.26
TC	33	10.26	13.01
TC	75	13.01	24.01
BC	18	-0.38	1.13
BC	75	1.13	13.69
BC	75	12.01	23.88



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 12 QTY:1 FL/-/4/-/E/-/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.

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DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

JAMES R. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94296
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006058
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223476
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C61)

Top chord 2x4 SP #2 :T1 2x6 SP #2:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (2)2x8 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

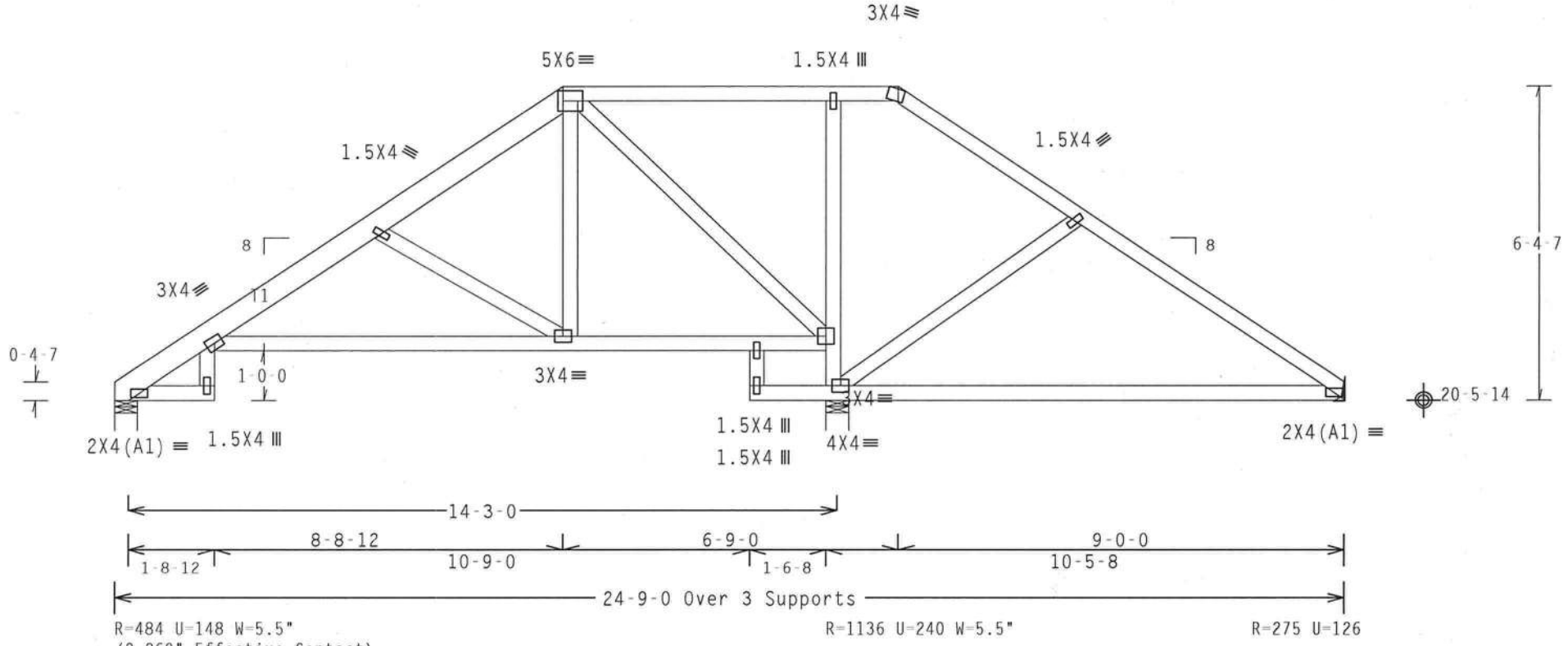
MWFRS loads based on trusses located at least 11.93 ft. from roof edge.

120 mph wind, 23.86 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-0.74	8.26
TC	81	8.26	15.01
TC	75	15.01	24.01
BC	18	-0.38	1.13
BC	120	1.13	12.15
BC	75	12.15	13.69
BC	75	12.01	23.88



R=484 U=148 W=5.5"
(2.269" Effective Contact)
RL=172/-173

R=1136 U=240 W=5.5"

R=275 U=126

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.0810 QTY:1 FL/-/4/-/E/-/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94297
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006054
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223470
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - C62)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	64	-1.92	7.00
TC	75	7.00	26.67
BC	120	0.15	24.60

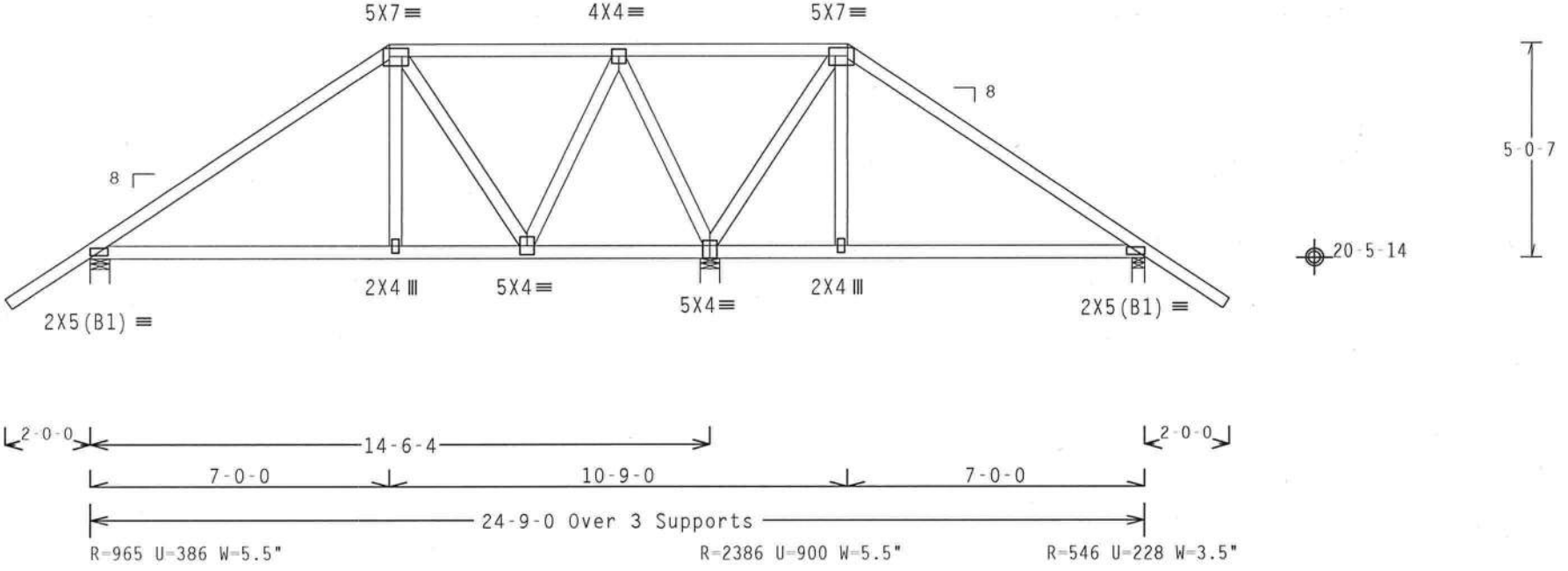
All wind load cases on this truss have a 1.33 duration factor.

120 mph wind, 22.53 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2) 9.01.00.021 QTY:1 FL/-/4/-/E/-/- Scale =.25"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.

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JAMES K. EVANS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94298
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006064
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223488
DUR.FAC.	1.25	FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - D63 2-PLY)

Top chord 2x4 SP #2
Bot chord 2x8 SP #2
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 56 plf at 0.00 to 56 plf at 2.12
TC - From 54 plf at 2.12 to 54 plf at 11.26
TC - From 57 plf at 11.26 to 57 plf at 16.92
BC - From 74 plf at 0.00 to 74 plf at 0.12
BC - From 20 plf at 0.12 to 20 plf at 16.92
PLB- 63 LB Conc. Load at (0.77,20.53)
PLB- -82 LB Conc. Load at (1.92,20.53)
PLB- 210 LB Conc. Load at (2.77,20.53)
PLB- 259 LB Conc. Load at (4.77,20.53)
PLB- 634 LB Conc. Load at (6.40,20.53)
PLB- 2117 LB Conc. Load at (6.73,20.53)

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	1199998	0.15	1.27
TC	12	1.27	2.12
TC	52	2.12	16.92
BC	120	0.00	16.92

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 7.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts MAY BE USED FOR (2) 0.131"x3" nails ON THE BOTTOM CHORD ONLY.

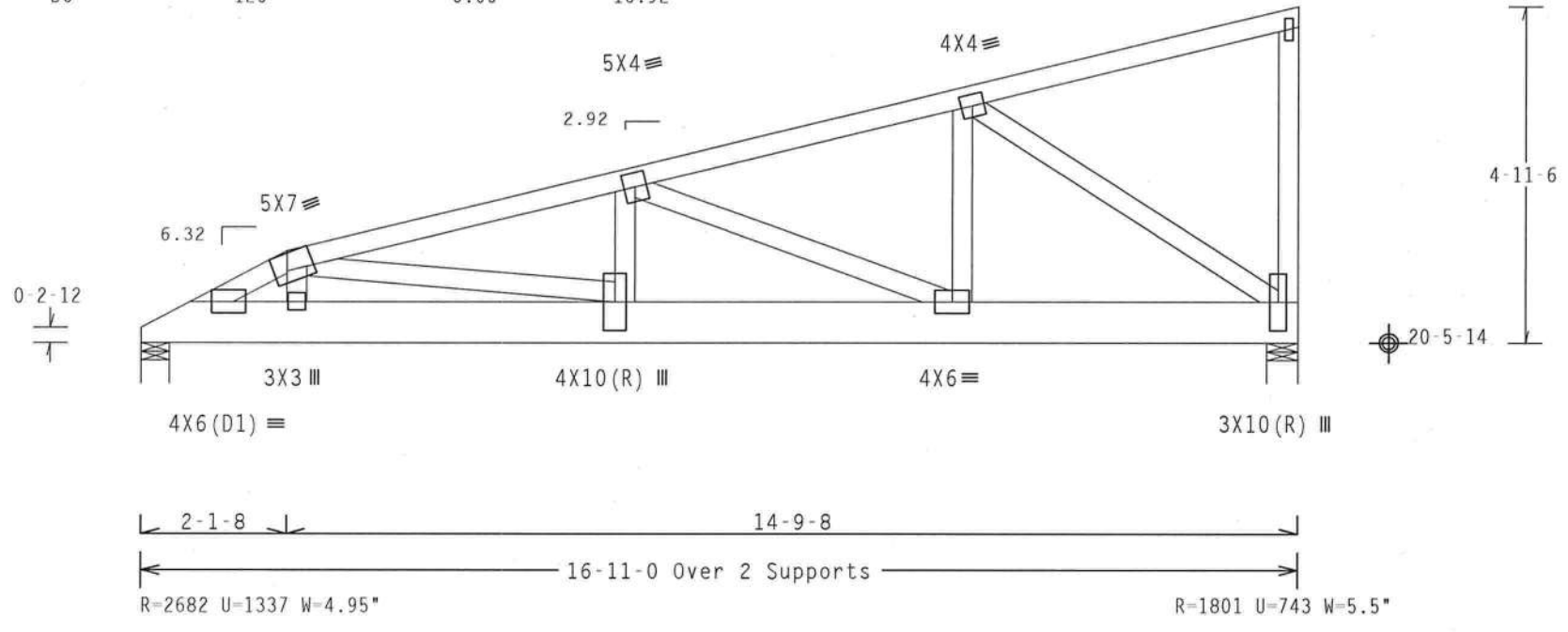
120 mph wind, 24.31 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)
PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94299
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006039
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223735
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2
Bot chord 2x8 SP #2
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 56 plf at 0.00 to 56 plf at 2.12
TC - From 54 plf at 2.12 to 54 plf at 6.21
BC - From 20 plf at 0.00 to 20 plf at 6.21
BC - 275 lb Conc. Load at 1.44
BC - 82 lb Conc. Load at 1.92
BC - 224 lb Conc. Load at 3.44
BC - 97 lb Conc. Load at 4.75
BC - 205 lb Conc. Load at 5.44

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	26	0.56	1.42
TC	50	1.42	5.50
BC	75	0.71	5.50

2 COMPLETE TRUSSES REQUIRED

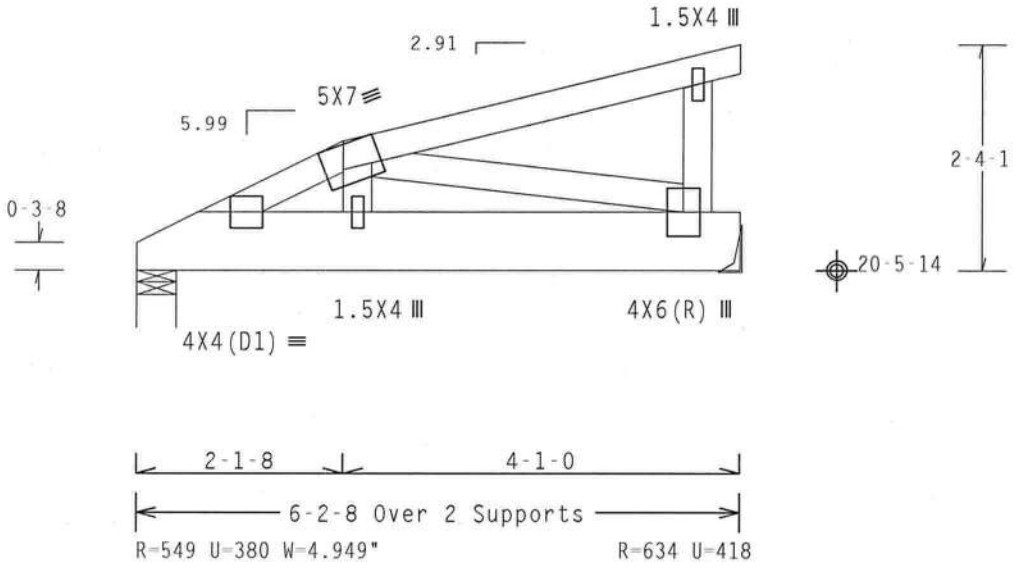
Nail Schedule:0.131"x3"_nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2"_bolts MAY BE USED FOR (2) 0.131"x3"_nails ON THE BOTTOM CHORD ONLY.

120 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

These hangers and support conditions used at bearings indicated
(H1) = Simpson HGUS28-2 w/ (2)2x8 SP #2 supporting member.
(36) 16d Common, 0.162"x3.5" nails into supporting member,
(6) 16d Common, 0.162"x3.5" nails into supported member.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)
PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.[illegible] QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

JAMES F. BOLLINGS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94300
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006041
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223504
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2
Bot chord 2x8 SP #2
Webs 2x4 SP #3
:Rt Bearing Leg 2x8 SP #2:

120 mph wind, 23.56 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf.
Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

This girder designed to support 26-4-8 span framing to the bottom chord from one side. Opposite side supports 5-1-0 jacks. Jacks have no webs.

Deflection meets L/240 live and L/180 total load.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

Hanger = Simpson HGUS28-2 or equivalent, fill all nail holes.

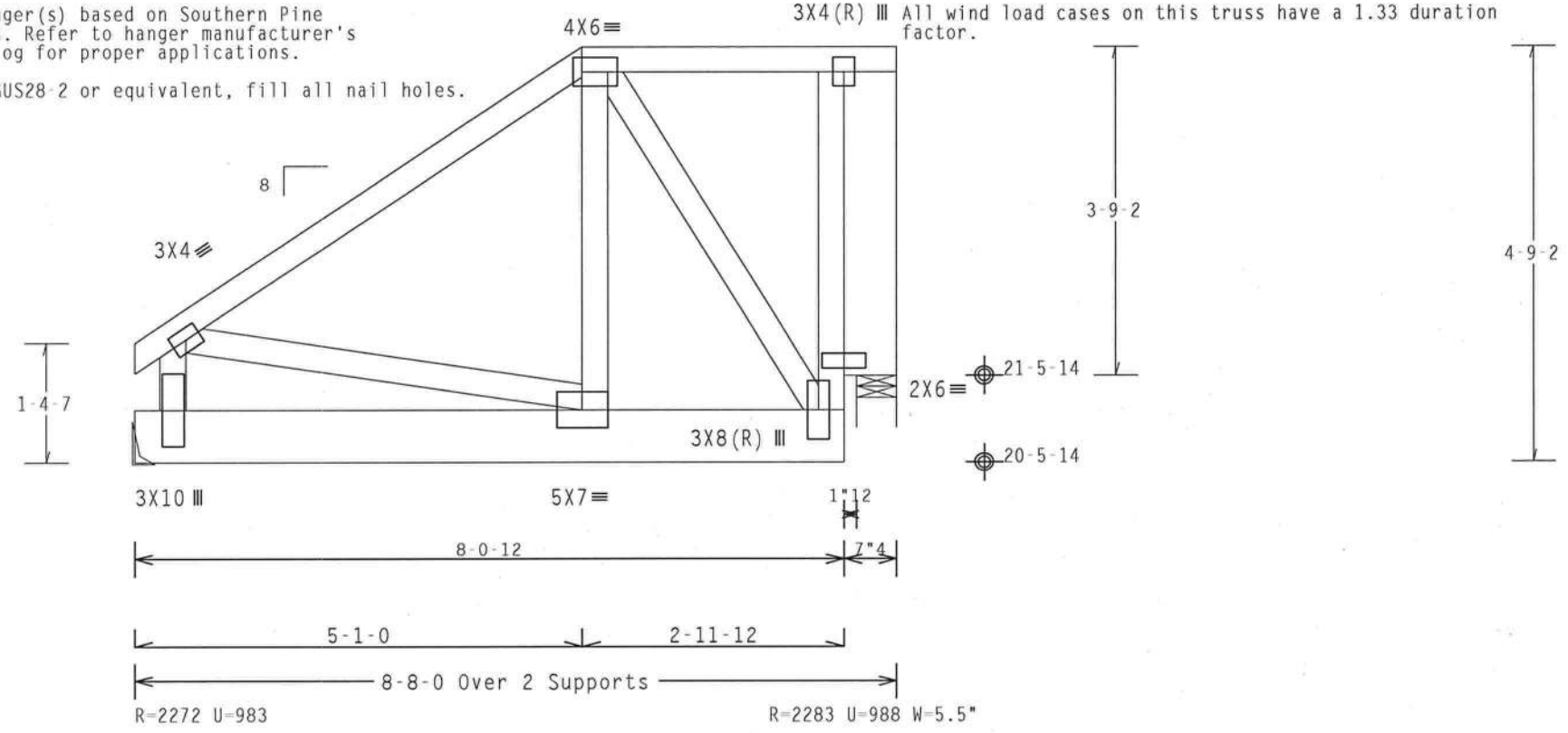
2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3"_nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 6.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2"_bolts MAY BE USED FOR (2) 0.131"x3"_nails ON THE BOTTOM CHORD ONLY.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	73	-0.74	4.34
TC	43	4.34	7.92
BC	97	-0.74	7.32

3X4(R) III All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2)

PLT TYP. Wave

9.01.00.00 QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

JAMES R. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94301
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006055
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223166
DUR.FAC.	1.25	FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - G66)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	74	-0.74	9.59
BC	120	-0.74	9.59

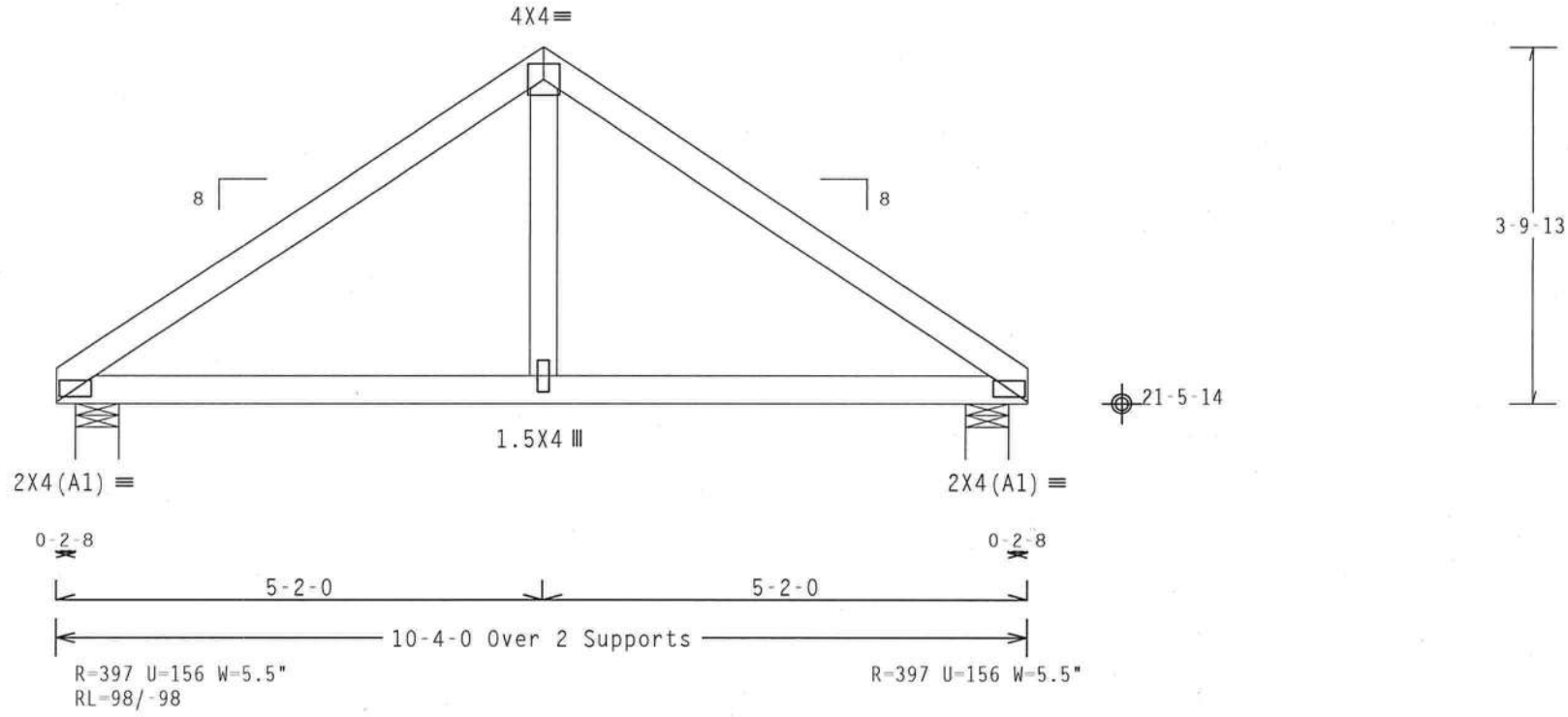
All wind load cases on this truss have a 1.33 duration factor.

120 mph wind, 23.58 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:2 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES R. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94302
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006060
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223740
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - G67)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 11.50 PSF. Top chord must not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	72	0.15	10.19
BC	109	0.00	10.33

120 mph wind, 23.37 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

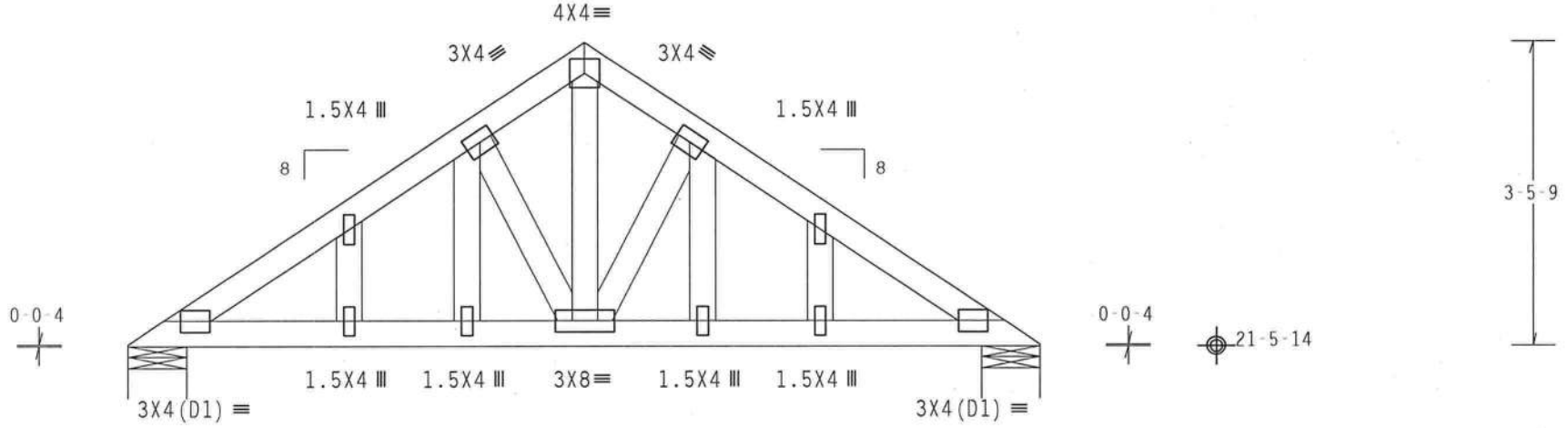
Wind reactions based on MWFRS pressures.

See DWGS A12030050109 & GBLLETIN0109 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R235-- 94303
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006047
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223751
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - EJ61)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-2.69	6.22
BC	22	-0.63	1.22
BC	22	0.83	1.08
BC	62	1.08	6.22

Deflection meets L/240 live and L/180 total load.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

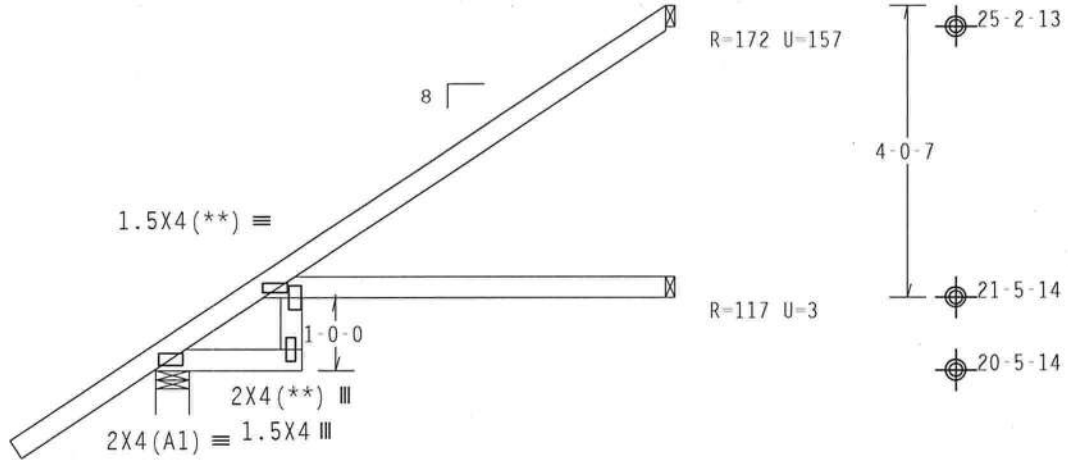
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

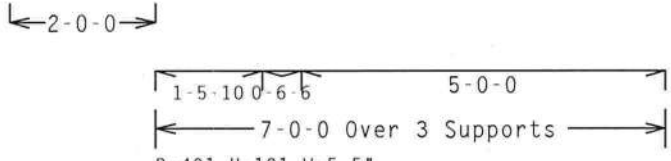
120 mph wind, 22.53 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.





R=401 U=101 W=5.5"
RL=259/-119

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2) 9.01.00

PLT TYP. Wave QTY:2 FL/-/4/-/E/-/- Scale =.375"/Ft.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94304
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006022
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273755
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - EJ62)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	49	1.92	1.50
TC	21	1.50	3.15
BC	36	0.15	3.15

All wind load cases on this truss have a 1.33 duration factor.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

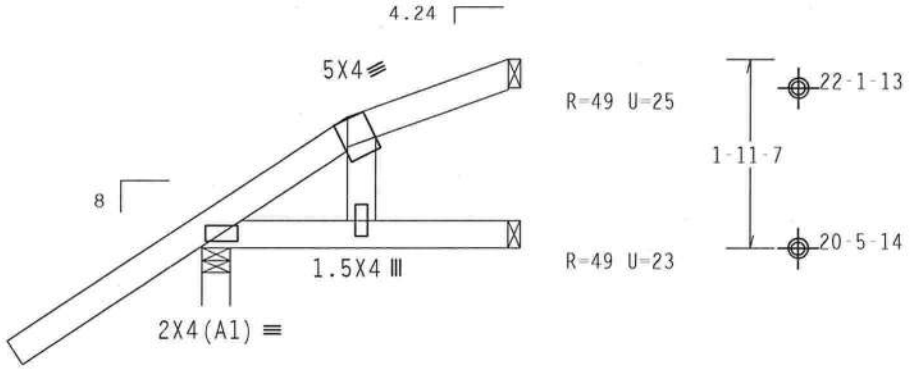
120 mph wind, 20.99 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 10.49 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=20%(0%)/5(2) | 9.01.00 | QTY:2 | FL/-/4/-/E/-/- | Scale =.5"/Ft.

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TC LL	20.0 PSF	REF R235-- 94305
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006042
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223496
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - EJ63)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	44	-1.92	1.15
BC	12	0.15	1.15

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 10.29 ft. from roof edge.

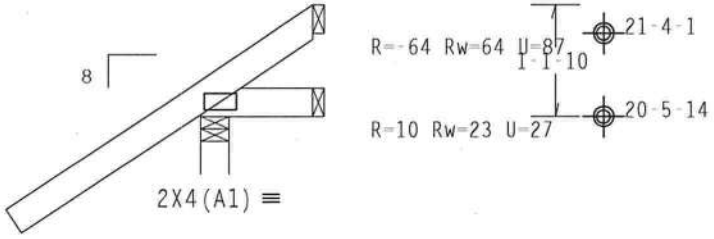
120 mph wind, 20.58 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

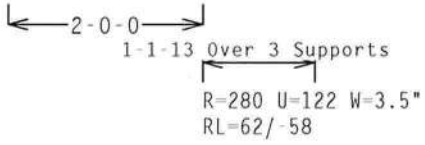
Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 02 [illegible] QTY:2 FL/-/4/-/E/-/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF R235-- 94306
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006043
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223491
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2
Bot chord 2x4 SP #2

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	2.04	6.87
BC	75	0.02	6.88

Deflection meets L/240 live and L/180 total load.

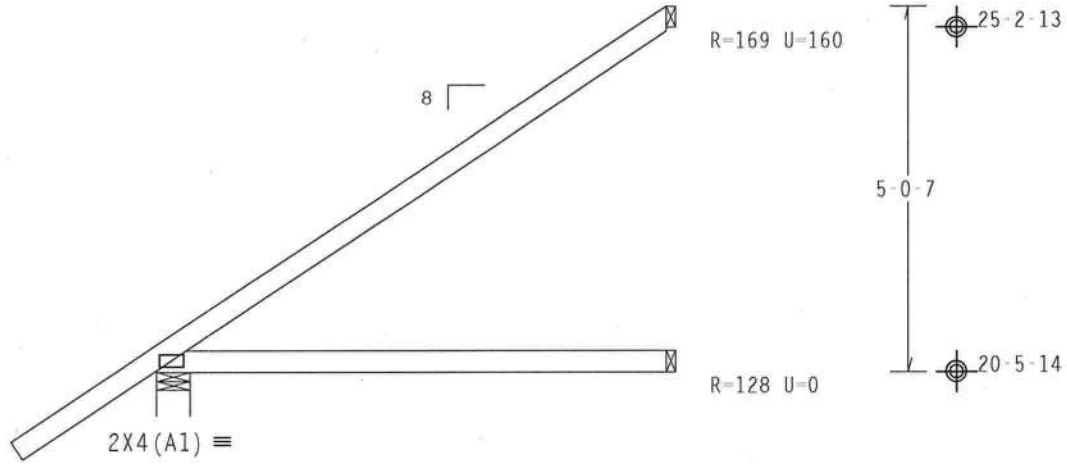
Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

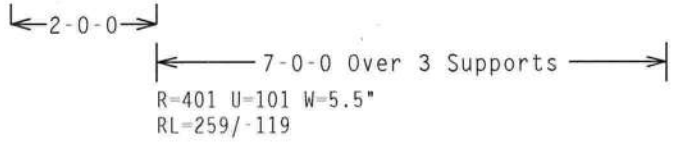
120 mph wind, 22.53 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.021 QTY:33 FL/-/4/-/E/-/- Scale =.375"/Ft.

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TC LL	20.0 PSF	REF R235-- 94307
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006038
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223783
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - EJ74)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-2.25	4.75
BC	57	0.00	4.75

***** BEARING ANALOG MODIFIED! *****

MWFRS loads based on trusses located at least 11.44 ft. from roof edge.

120 mph wind, 22.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

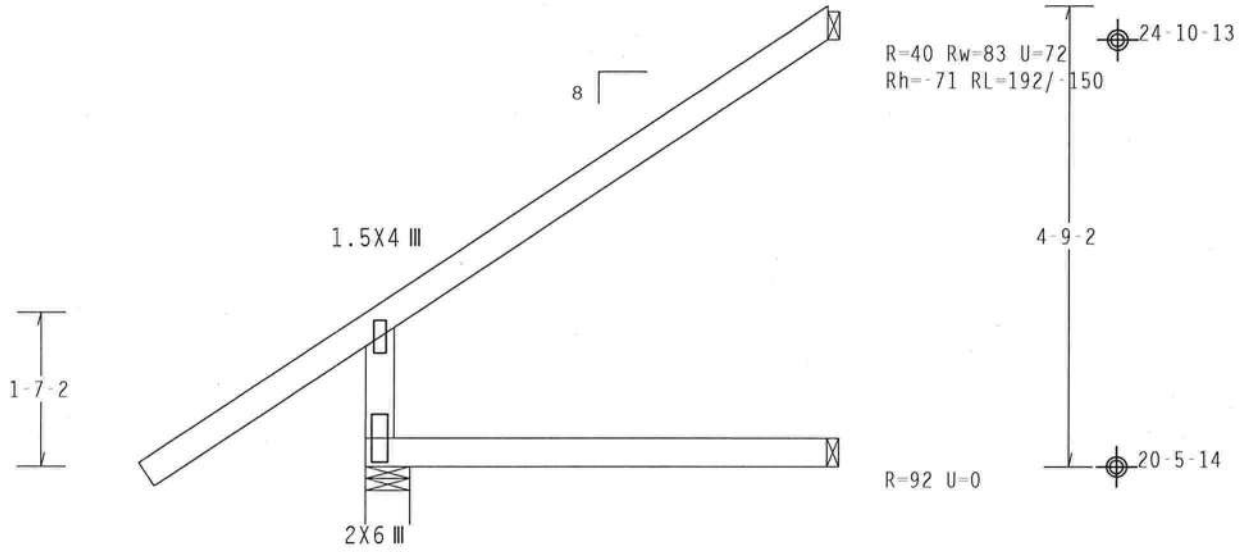
Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.0210 QTY:2 FL/-/4/-/E/-/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF R235-- 94308
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006049
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223755
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ75)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-2.04	4.91
BC	59	0.02	4.91

Deflection meets L/240 live and L/180 total load.

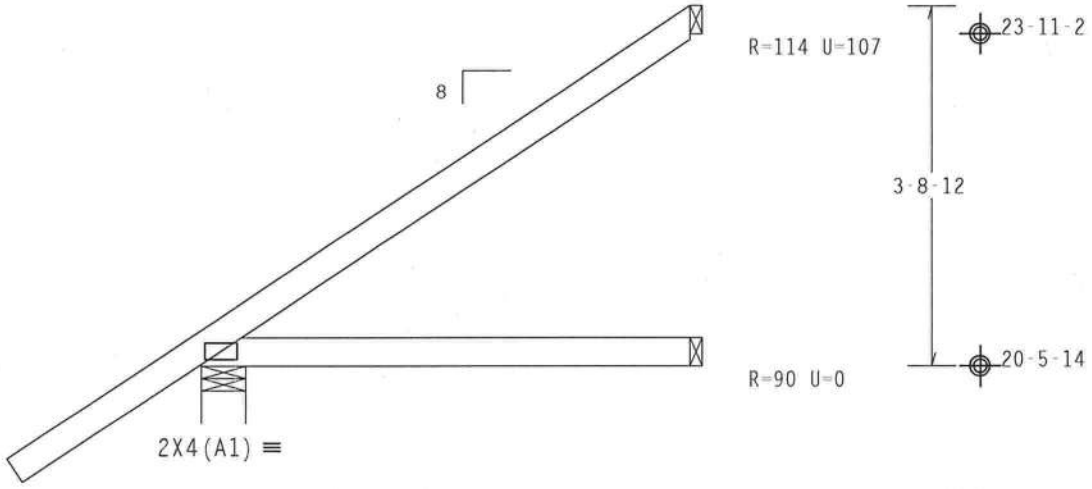
Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

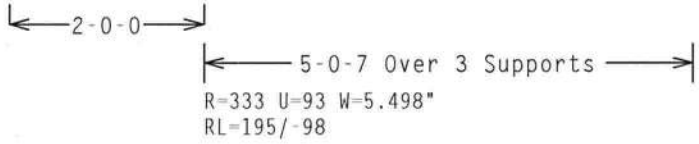
120 mph wind, 21.87 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:8 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0	PSF
TC DL	7.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	37.0	PSF
DUR.FAC.	1.25	

REF	R235-- 94309
DATE	01/06/10
DRW	HCUSR235 10006035
HC-ENG	DLJ/DLJ
SEQN-	223785
FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ76)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	71	-2.04	2.91
BC	35	0.02	2.91

Deflection meets L/240 live and L/180 total load.

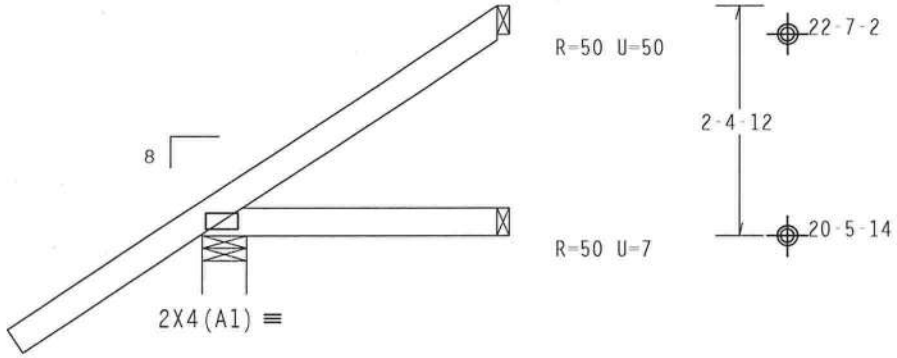
Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

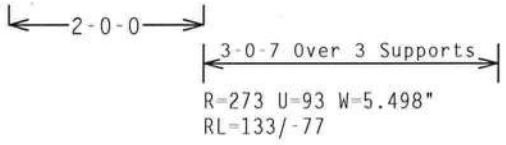
120 mph wind, 21.21 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01 00.0210 0 QTY:8 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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TC LL	20.0 PSF	REF R235-- 94310
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006036
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223787
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ77)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	43	2.04	0.91
BC	11	0.02	0.91

Deflection meets L/240 live and L/180 total load.

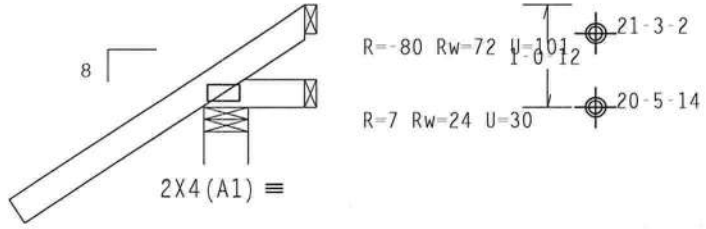
Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

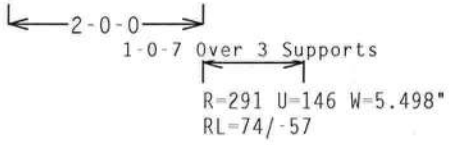
120 mph wind, 20.54 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.021 QTY:10 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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TC LL	20.0 PSF	REF	R235-- 94311
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006037
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223789
DUR.FAC.	1.25	FROM	RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ78)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	1.92	5.04
BC	22	0.15	2.00
BC	22	1.85	1.85
BC	38	1.85	5.04

Deflection meets L/240 live and L/180 total load.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

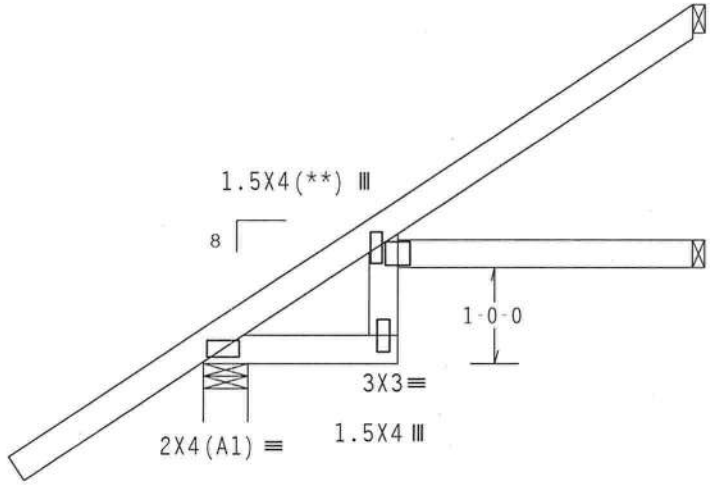
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

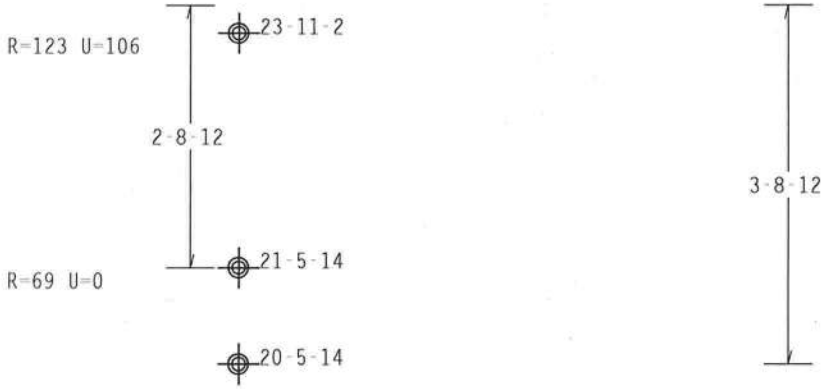
120 mph wind, 21.87 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

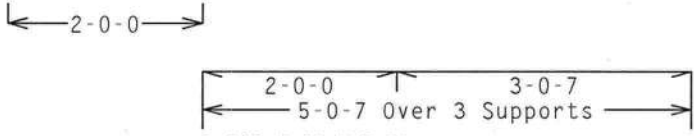
Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.







Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 [illegible] QTY:2 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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TC LL	20.0 PSF	REF R235-- 94312
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006062
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223777
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ79)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	71	-1.92	3.04
BC	22	0.15	2.00
BC	22	1.85	1.85
BC	14	1.85	3.04

Deflection meets L/240 live and L/180 total load.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.

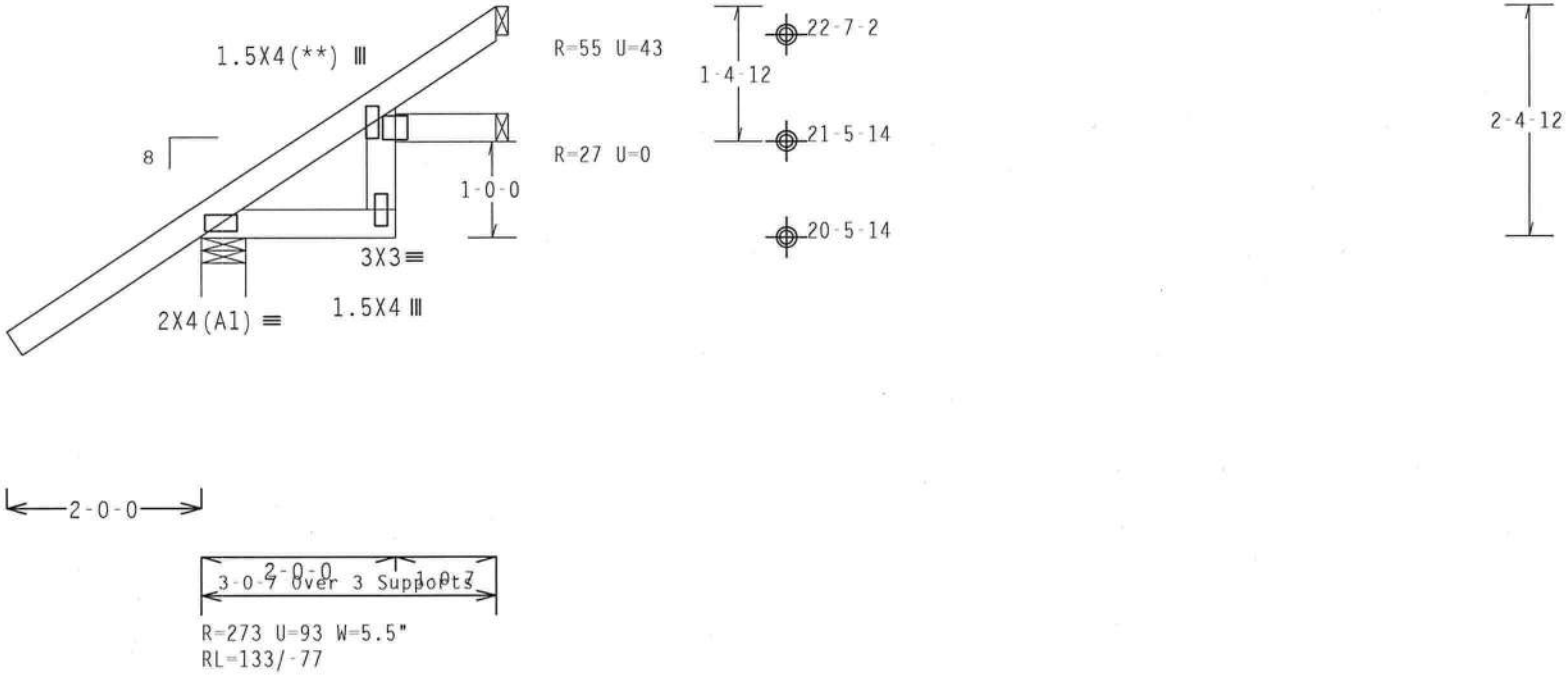
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 21.21 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.02 QTY:2 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. COLLINS, JR. LICENSE
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94313
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006063
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223781
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2 :T1 2x6 SP #1 Dense:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 25.19 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.16" due to live load and 0.15" due to dead load.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	26	-0.00	13.00
TC	73	13.00	27.42
TC	75	27.42	40.42
BC	18	0.36	1.87
BC	62	1.87	7.03
BC	75	7.03	8.03
BC	75	6.88	40.27

Negative reaction(s) of -404# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

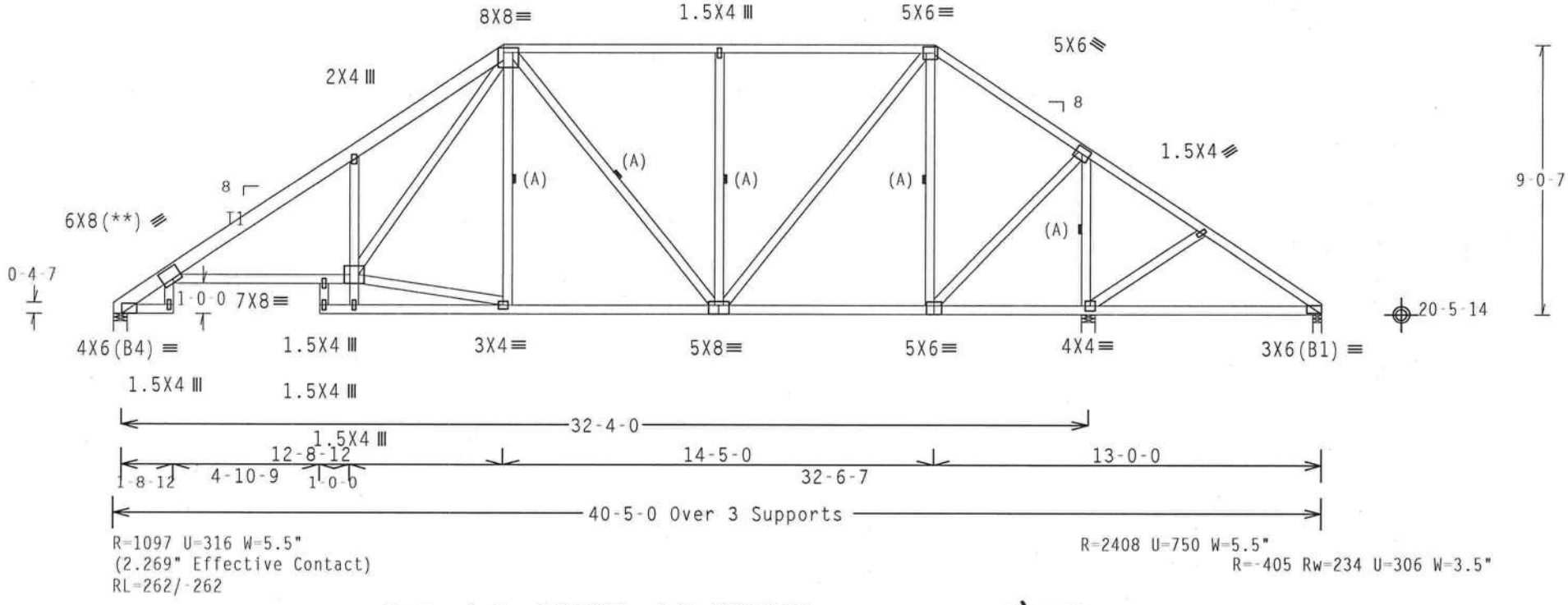
(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.60 ft. from roof edge.



R=1097 U=316 W=5.5"
(2.269" Effective Contact)
RL=262/-262

R=2408 U=750 W=5.5"
R=-405 Rw=234 U=306 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2)

PLT TYP. Wave

9.01.00.02 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94276
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006008
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273656
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - A40)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 :T1 2x6 SP #1 Dense:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 24.53 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.14" due to live load and 0.13" due to dead load.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	23	-0.13	10.88
TC	64	10.88	29.29
TC	120	29.29	40.29
BC	18	0.24	1.75
BC	108	1.75	10.88
BC	75	10.88	12.90
BC	75	10.76	40.15

Negative reaction(s) of -399# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

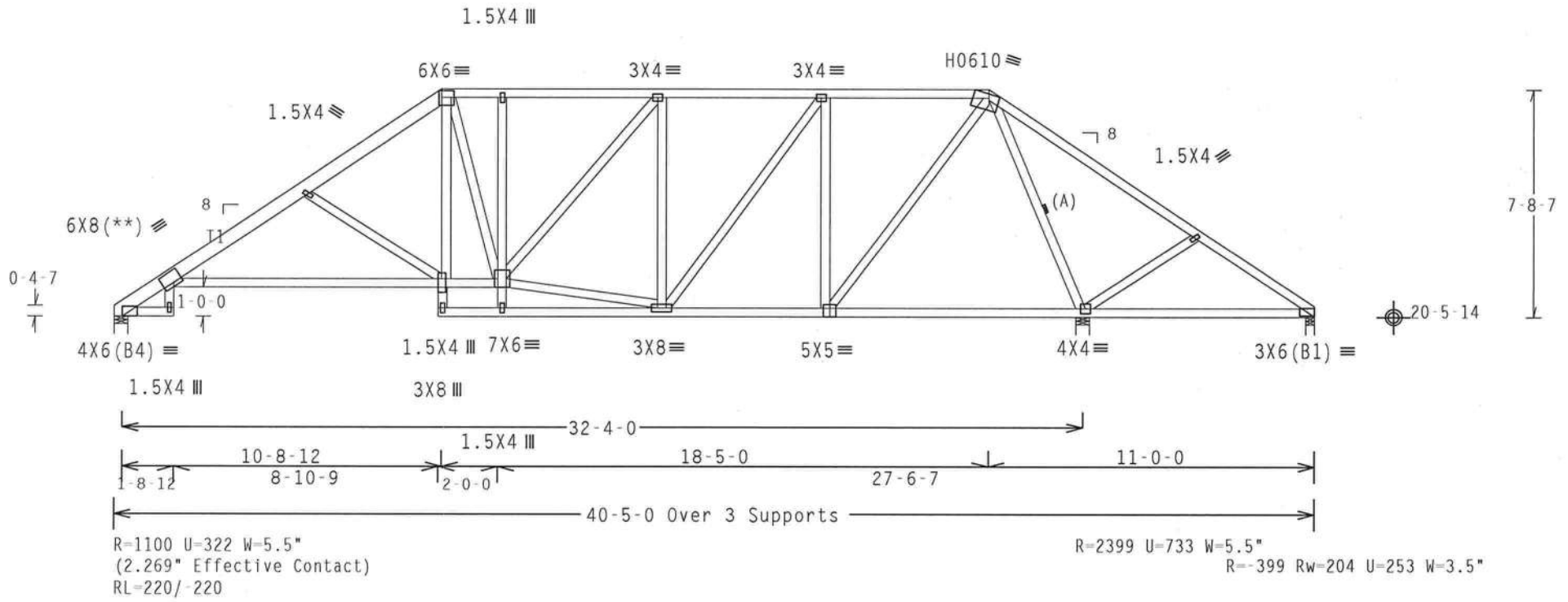
(A) Continuous lateral bracing equally spaced on member. Or 2x4 "T" brace. 80% length of web member. Same species & grade or better, attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.26 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES R COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94277
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006009
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273668
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - A41)

Top chord 2x4 SP #2 :T1 2x6 SP #1 Dense:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 23.19 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

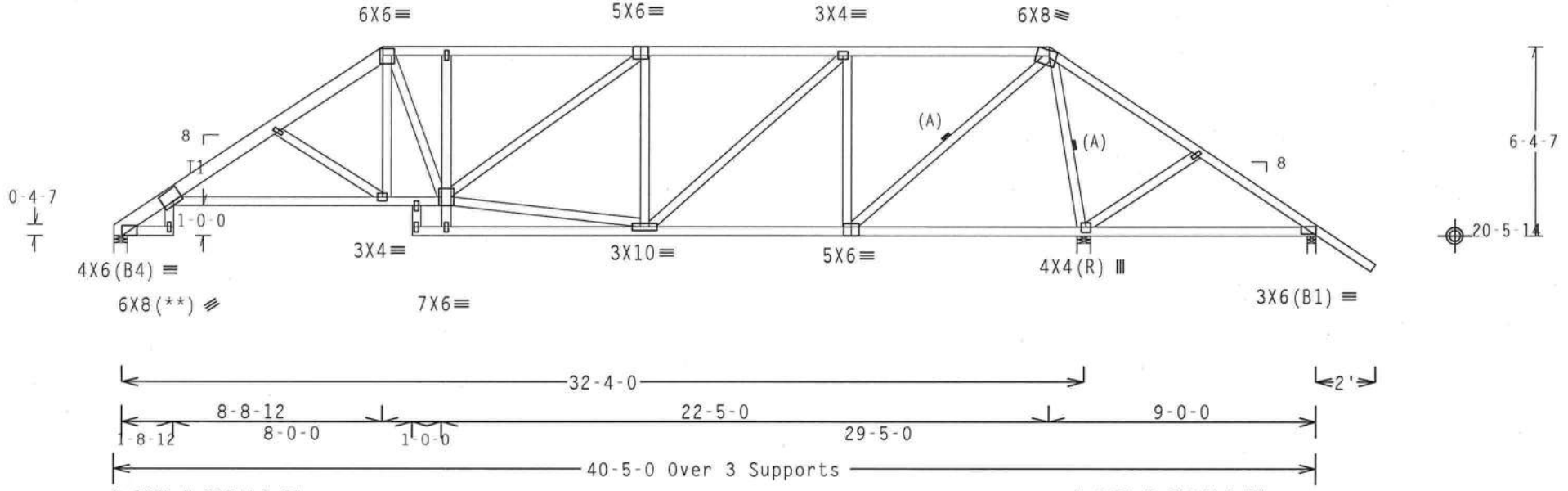
Negative reaction(s) of -371# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	28	0.13	8.88
TC	57	8.88	31.29
TC	120	31.29	42.21
BC	18	0.24	1.75
BC	99	1.75	10.02
BC	74	10.02	11.02
BC	74	9.88	40.15



Note: All Plates Are 1.5X4 Except As Shown.
RL=227/206

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.0210 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES R. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94278
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006010
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273680
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - A42 2-PLY)

Top chord 2x4 SP #2 :T1 2x6 SP #2:
Bot chord 2x4 SP #2
Webs 2x4 SP #3

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.15" due to live load and 0.14" due to dead load.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load.

(2) 2x6X4-10-4 SP #2 Top chord scabs centered 2-6-8 from left end. Attach one to each outer face of chord with (3) rows of 0.131"x3" nails @ 12" O.C., staggered 6".

2 COMPLETE TRUSSES REQUIRED

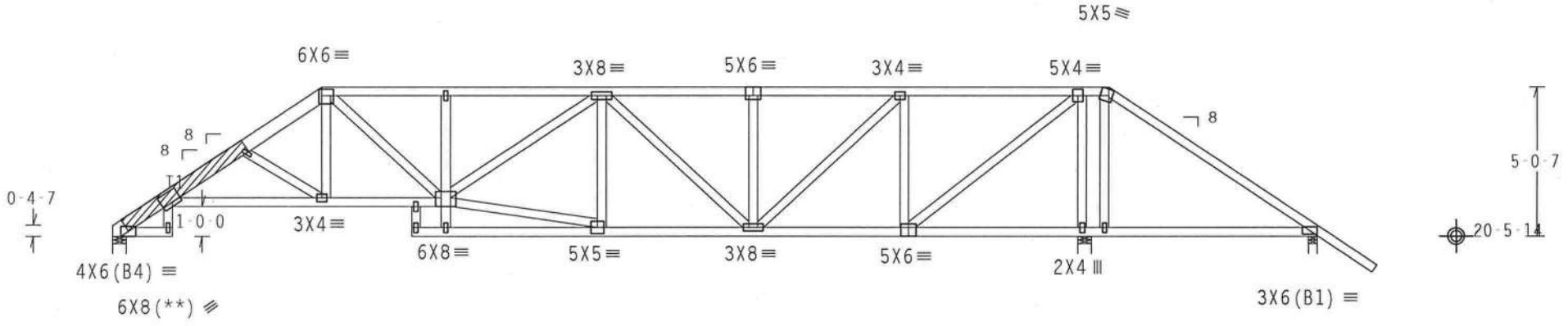
Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

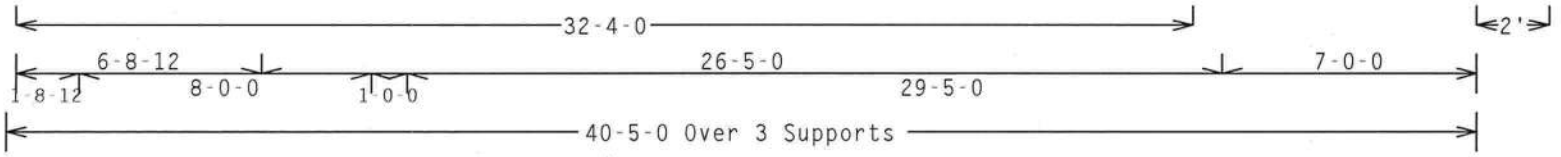
120 mph wind, 22.53 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	48	0.13	6.88
TC	63	6.88	33.29
TC	120	33.29	42.21
BC	18	0.24	1.75
BC	99	1.75	10.02
BC	120	10.02	11.02
BC	120	9.88	40.15

All wind load cases on this truss have a 1.33 duration factor.





R=2317 U=663 W=5.5"
(2.269" Effective Contact)

R=3942 U=1389 W=5.5"
R=0 Rw=138 U=0 W=3.5"

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES E. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94279
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006011
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273688
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B43)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (2)2x8 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 12.87 ft. from roof edge.

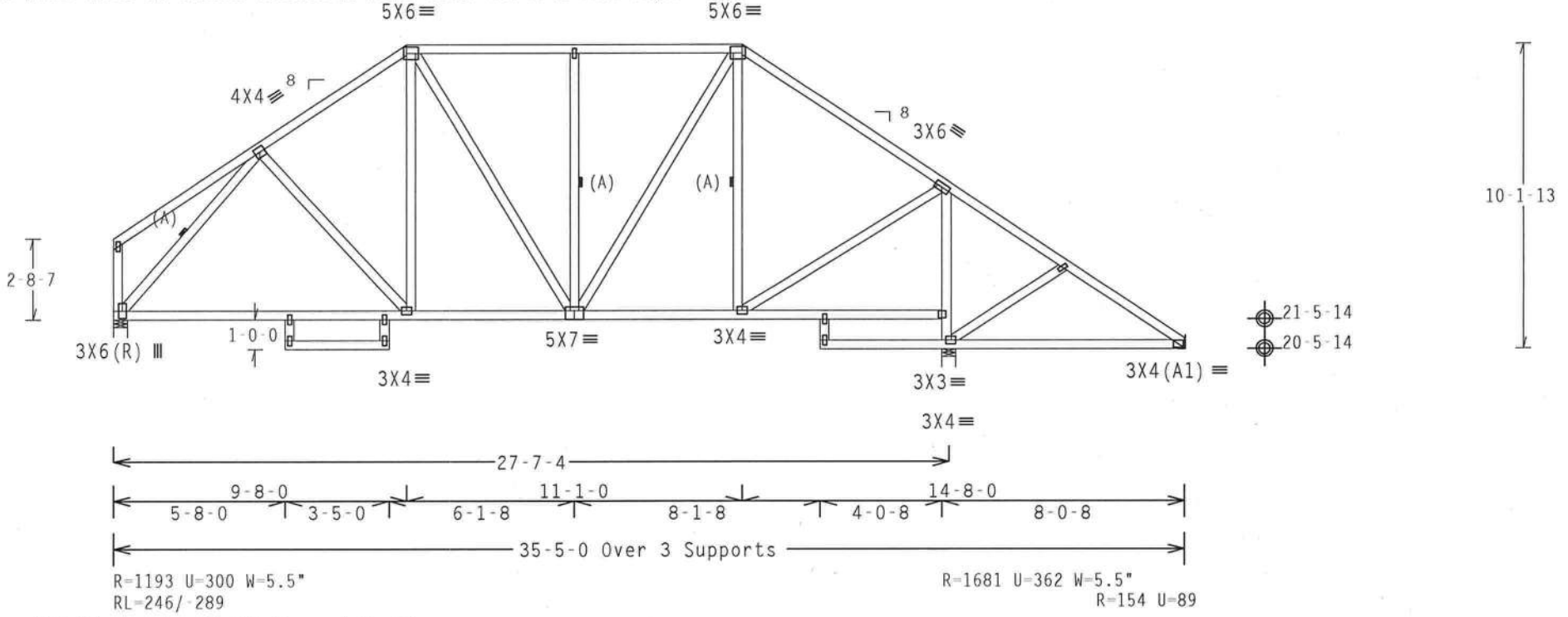
120 mph wind, 25.75 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	67	0.00	9.67
TC	73	9.67	35.42
BC	70	0.00	5.81
BC	41	5.81	8.94
BC	41	5.67	9.08
BC	75	8.94	27.52
BC	75	23.33	35.29



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 [illegible] QTY:2 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES R. DULLES, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94280
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006025
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273700
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B44)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2 :B4 2x6 SP #2:
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC -	From	57 plf at	0.00 to	57 plf at	9.67
TC -	From	57 plf at	9.67 to	57 plf at	20.75
TC -	From	57 plf at	20.75 to	57 plf at	35.42
BC -	From	20 plf at	0.00 to	20 plf at	5.67
BC -	From	20 plf at	5.67 to	20 plf at	9.08
BC -	From	20 plf at	9.08 to	20 plf at	23.33
BC -	From	20 plf at	23.33 to	20 plf at	35.42

BC - 910 lb Conc. Load at 28.44 , 32.44
BC - 908 lb Conc. Load at 30.44
BC - 913 lb Conc. Load at 34.44

Wind reactions based on MWFRS pressures.

Left end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

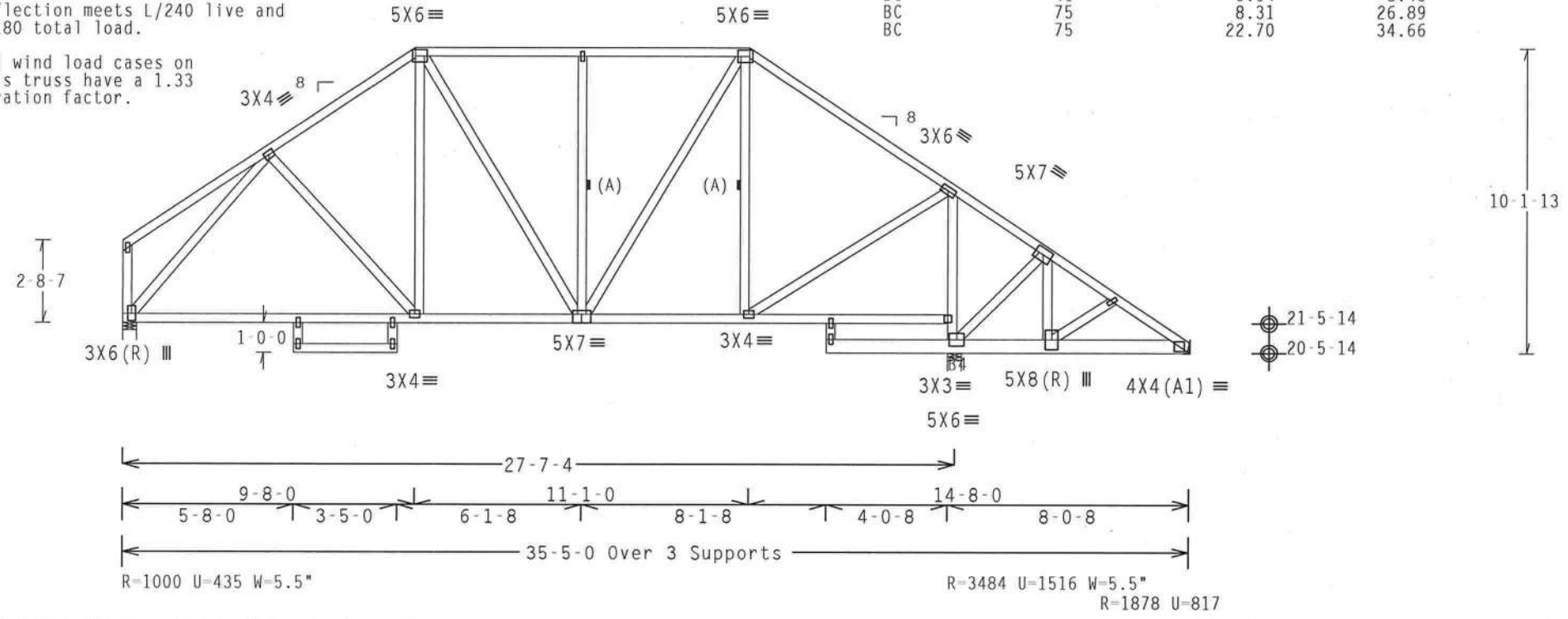
120 mph wind, 25.75 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (2)2x8 SP #2 supporting member.
(14) 10d, 0.148"x1.5" nails into supporting member,
(6) 10d Common, 0.148"x3.0" nails into supported member.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	70	-0.63	9.04
TC	75	9.04	20.12
TC	48	20.12	34.79
BC	70	-0.63	5.18
BC	41	5.18	8.31
BC	41	5.04	8.45
BC	75	8.31	26.89
BC	75	22.70	34.66



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES L. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94281
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006012
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273713
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B45)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 13.71 ft. from roof edge.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

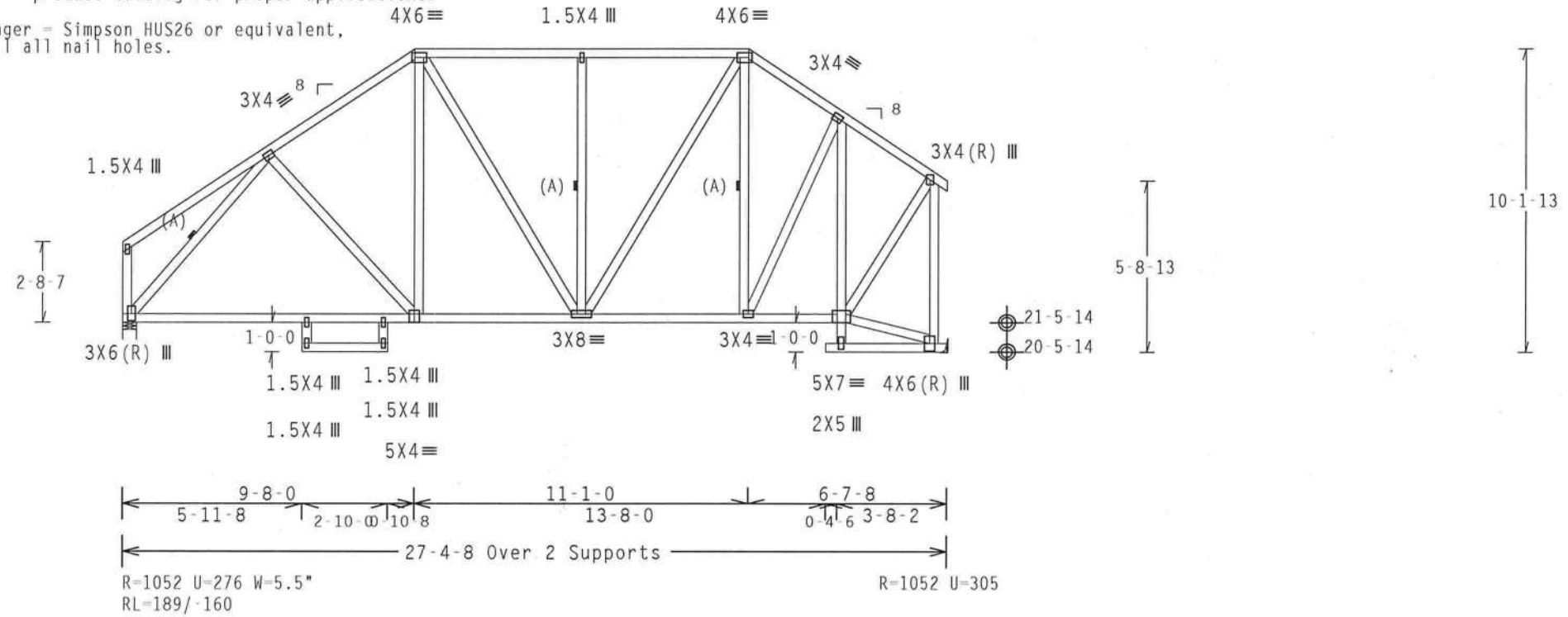
Hanger = Simpson HUS26 or equivalent, fill all nail holes.

120 mph wind, 27.42 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	72	-0.13	9.54
TC	75	9.54	27.25
BC	73	-0.13	5.98
BC	34	5.98	8.52
BC	34	5.83	8.67
BC	120	8.52	23.72
BC	120	23.21	23.72
BC	48	23.72	27.25



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.0 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94282
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006013
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273632
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 13.71 ft. from roof edge.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

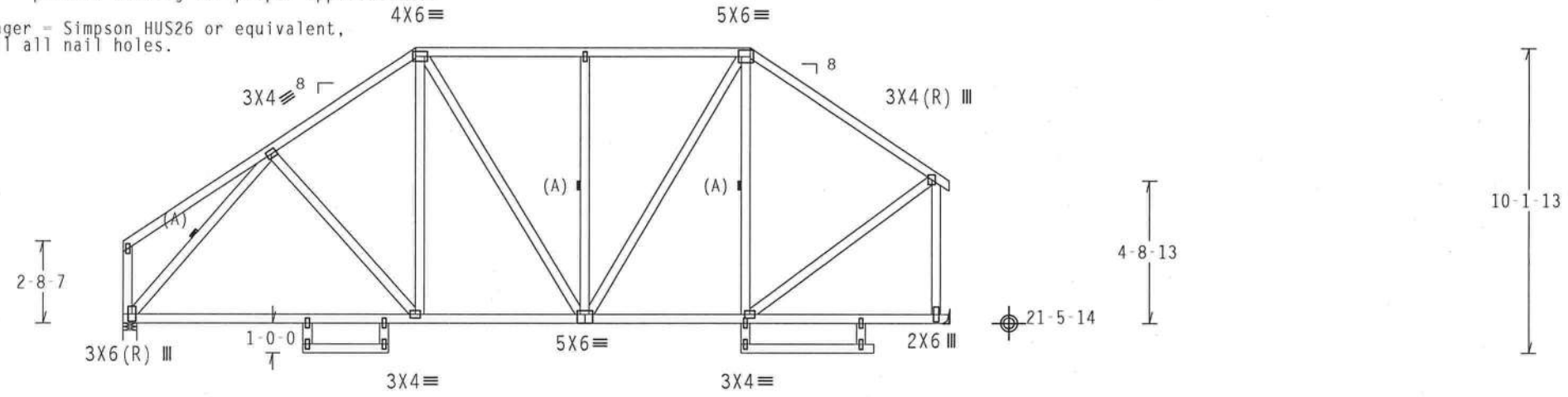
Hanger = Simpson HUS26 or equivalent, fill all nail holes.

120 mph wind, 27.42 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	72	-0.78	8.89
TC	75	8.89	26.60
BC	73	-0.78	5.33
BC	34	5.33	7.87
BC	34	5.18	8.02
BC	120	7.87	19.97
BC	53	19.68	23.66
BC	53	19.97	24.10
BC	35	23.66	26.60



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group Inc.

JAMES D. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94283
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006014
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273605
DUR.FAC.	1.25	FROM RCT

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

Hanger - Simpson HUS26 or equivalent, fill all nail holes.

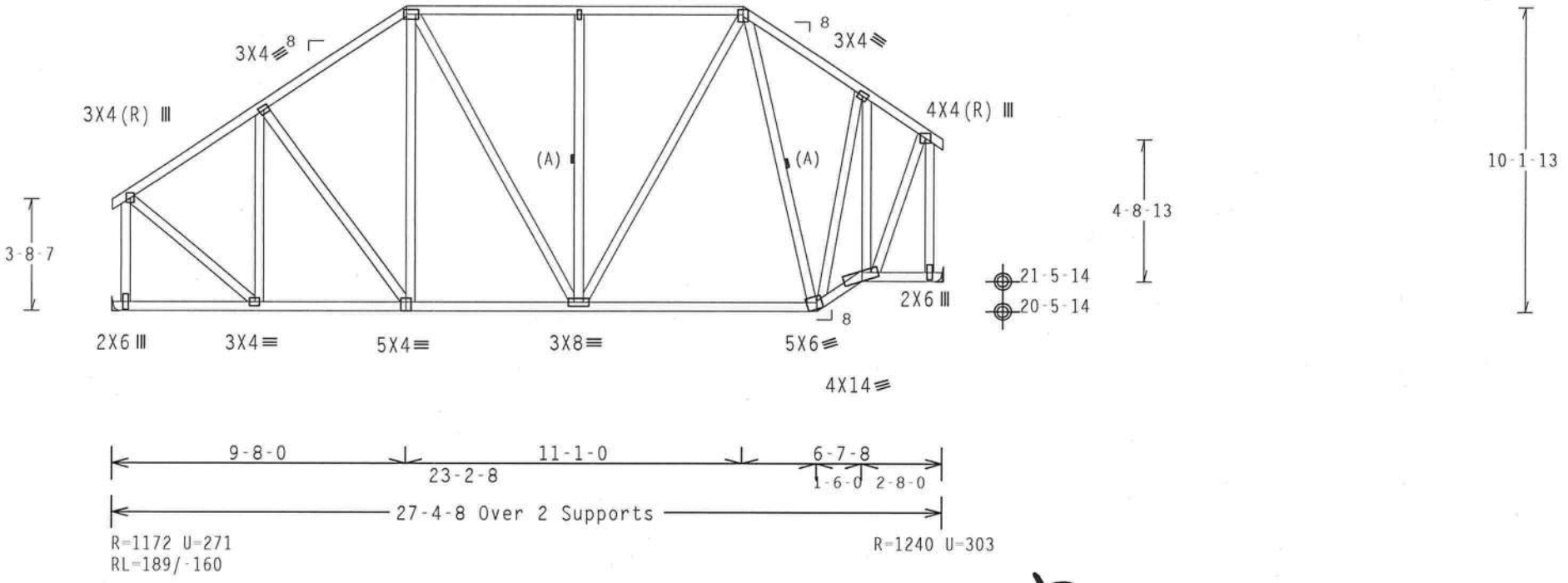
120 mph wind, 27.42 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	71	0.00	9.67
TC	74	9.67	20.75
TC	75	20.75	27.38
BC	120	0.00	23.21
BC	22	23.21	24.71
BC	32	24.71	27.38

MWFRS loads based on trusses located at least 13.71 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 [illegible] QTY:2 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94284
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006015
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273563
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B49)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

Hanger = Simpson HUS26 or equivalent, fill all nail holes.

120 mph wind, 27.42 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

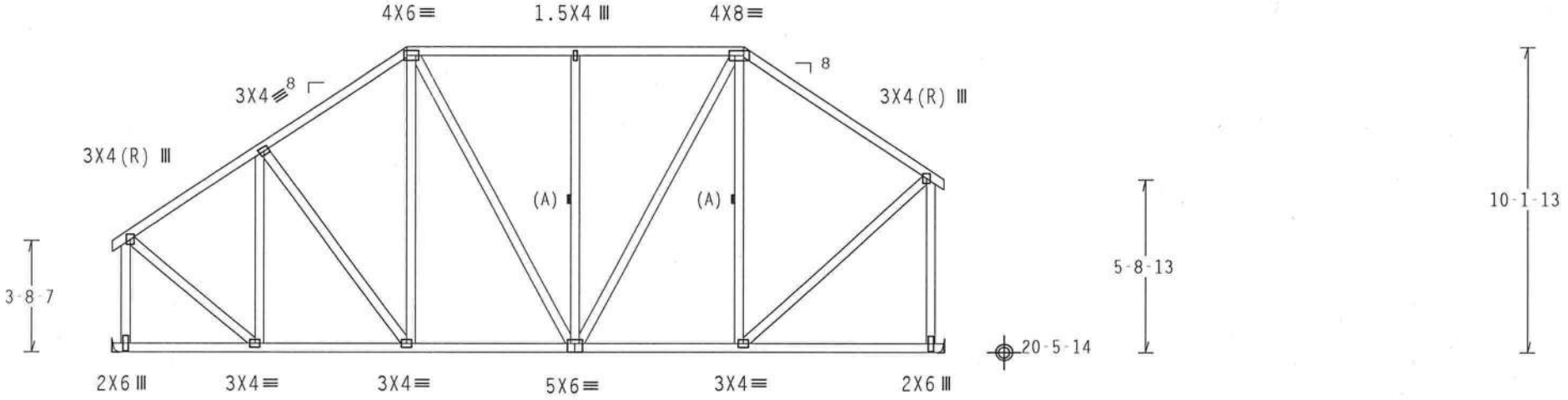
Wind reactions based on MWFRS pressures.

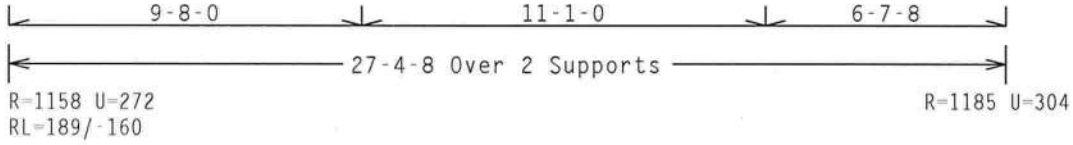
In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	71	-0.78	8.89
TC	74	8.89	26.60
BC	120	-0.78	26.60

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 13.71 ft. from roof edge.





Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2)

PLT TYP. Wave

9.01.00 [illegible] QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES R COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94285
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006016
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273538
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B50)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

Hanger = Simpson HUS26 or equivalent, fill all nail holes.

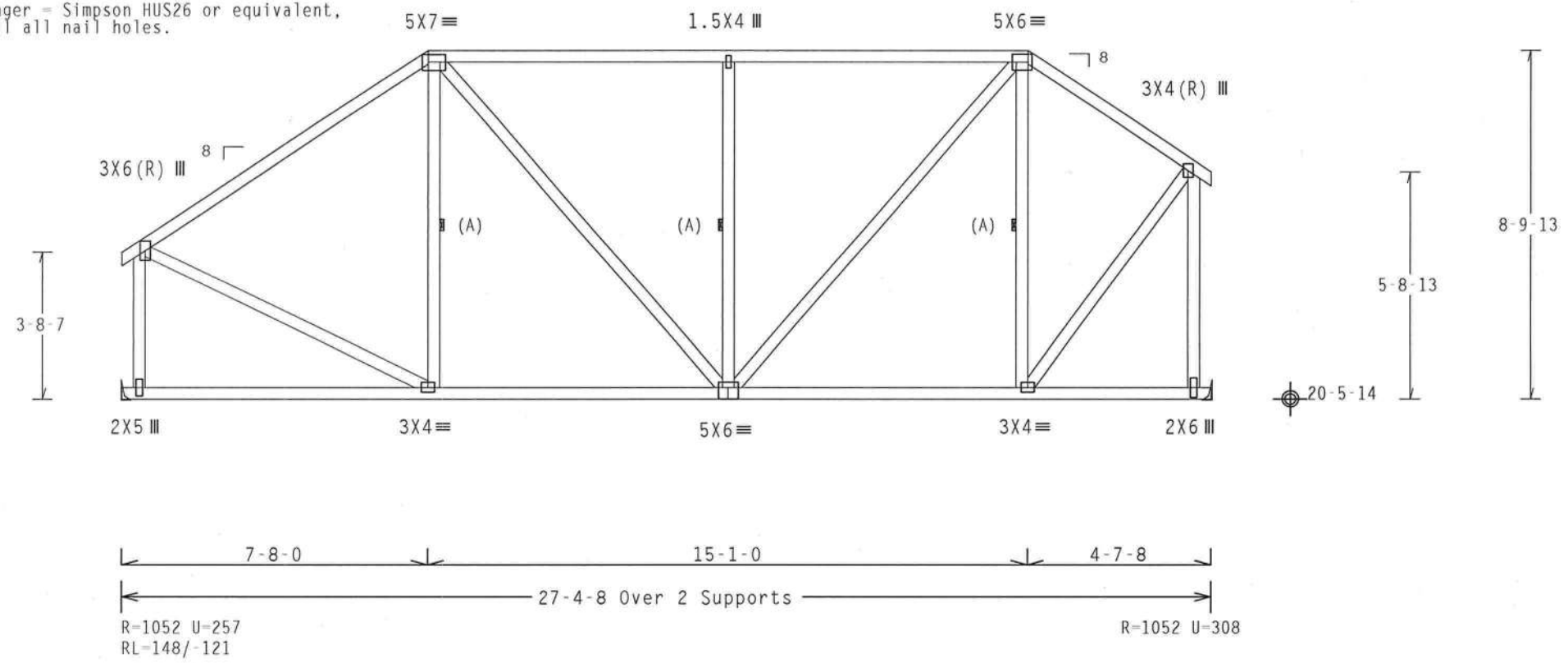
120 mph wind, 26.75 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	67	-0.78	21.97
TC	67	21.97	26.60
BC	120	-0.78	26.60

MWFRS loads based on trusses located at least 13.37 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES E. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94286
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006017
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273524
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B51)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2 :B1 2x8 SP #2:
Webs 2x4 SP #3

120 mph wind, 23.75 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf.
Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	24	-1.92	10.67
TC	41	10.67	32.37
BC	60	0.15	32.37

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

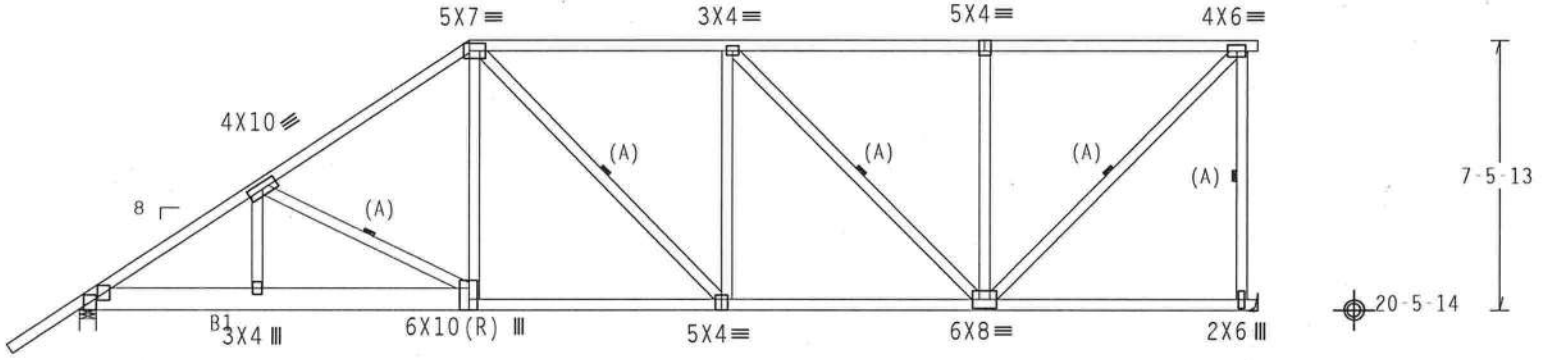
Hanger = Simpson HUS26 or equivalent, fill all nail holes.

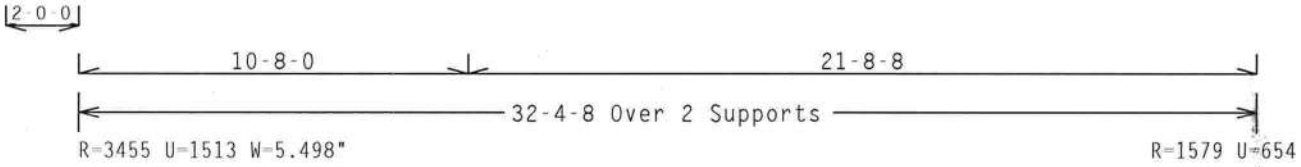
Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 57 plf at -2.00 to 57 plf at 10.67
TC - From 57 plf at 10.67 to 57 plf at 32.37
BC - From 20 plf at 0.00 to 20 plf at 32.37
BC - 40 lb Conc. Load at 0.85
BC - 125 lb Conc. Load at 2.85
BC - 2272 lb Conc. Load at 4.87

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.





Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2) 9.01.00.021

PLT TYP. Wave QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES R. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94287
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006018
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273568
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - B52)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2 :B2 2x4 SP #2 Dense:
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	48	-2.66	8.26
TC	44	8.26	31.63
BC	86	-0.60	31.63

Deflection meets L/240 live and L/180 total load.

Note: Specified hanger(s) based on Southern Pine lumber values. Refer to hanger manufacturer's product catalog for proper applications.

Hanger = Simpson HUS26 or equivalent, fill all nail holes.

120 mph wind, 23.19 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

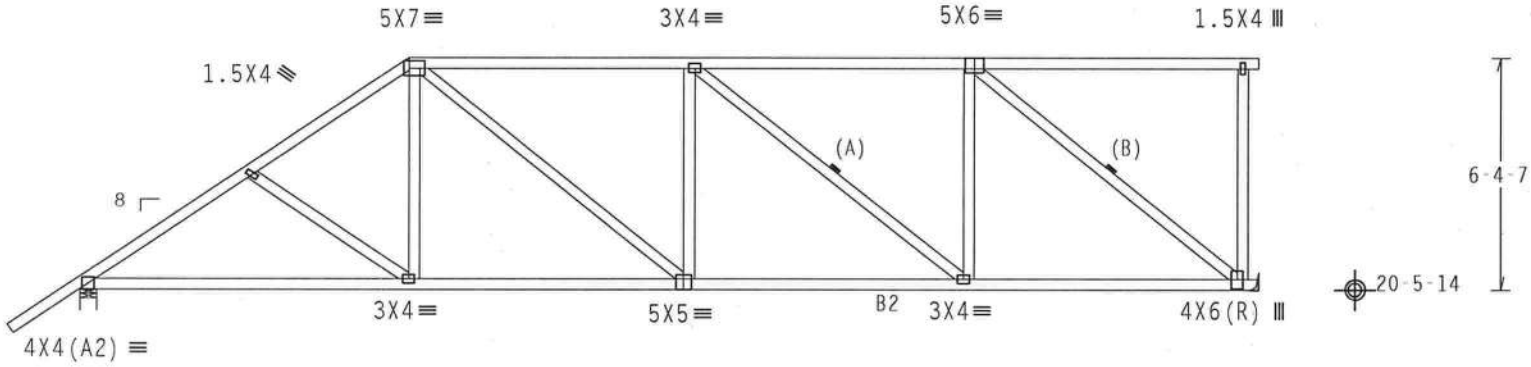
Wind reactions based on MWFRS pressures.

(B) Continuous lateral bracing equally spaced on member. Or 2x6 "T" brace. 80% length of web member. Same species & grade or better, attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.02.10.16 QTY:1 FL/-/4/-/E/-/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES D. COLANS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94288
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006019
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 273498
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ80)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	71	-3.00	1.96
BC	32	-0.74	1.96

***** BEARING ANALOG MODIFIED! *****

MWFRS loads based on trusses located at least 11.10 ft. from roof edge.

120 mph wind, 22.21 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

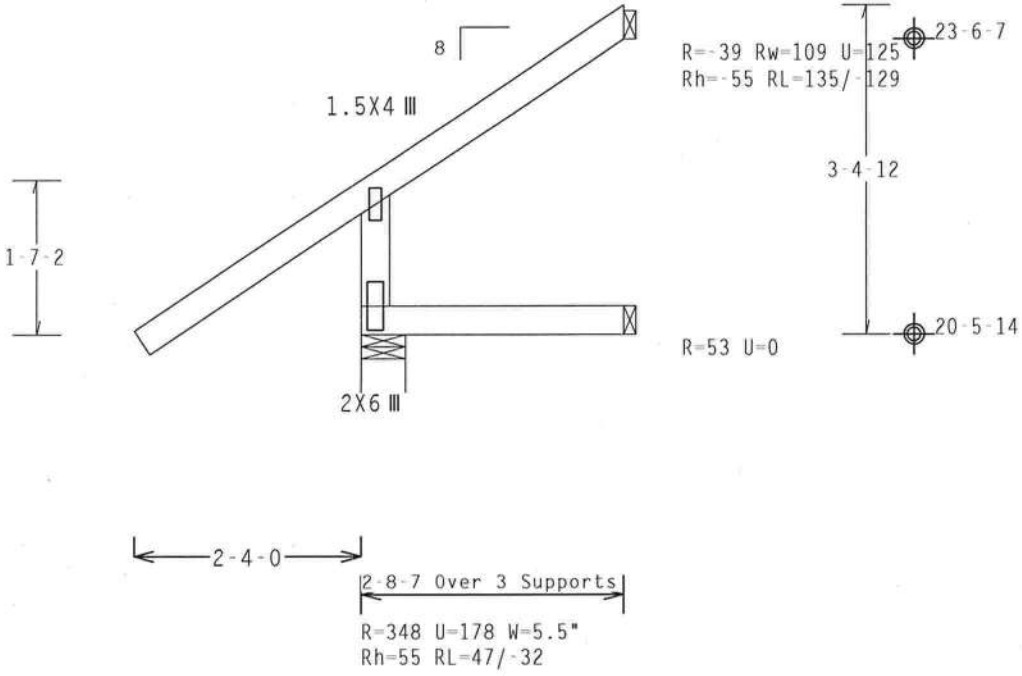
Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=20%(0%)/5(2) | 9.01.00.021 | QTY:1 | FL/-/4/-/E/-/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94314
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006059
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223759
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ81)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (1)2x8 SP #2 supporting member.
(14) 10d Common, 0.148"x3.0" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

120 mph wind, 22.87 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

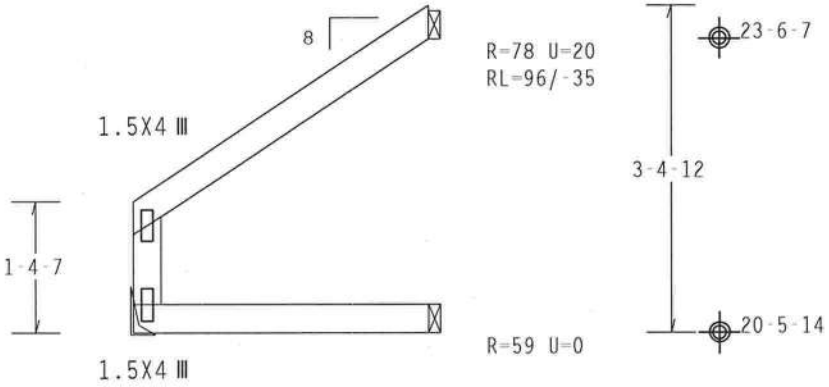
Wind reactions based on MWFRS pressures.

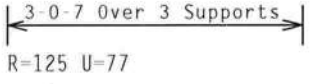
In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	44	-0.13	2.91
BC	36	-0.13	2.91

***** BEARING ANALOG MODIFIED! *****

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 [illegible] QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94315
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006033
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223169
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ82)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

IN LIEU OF STRUCTURAL PANELS OR RIGID CEILING USE PURLINS:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	43	2.25	0.70
BC	8	0.00	0.70

Shim all supports to solid bearing.

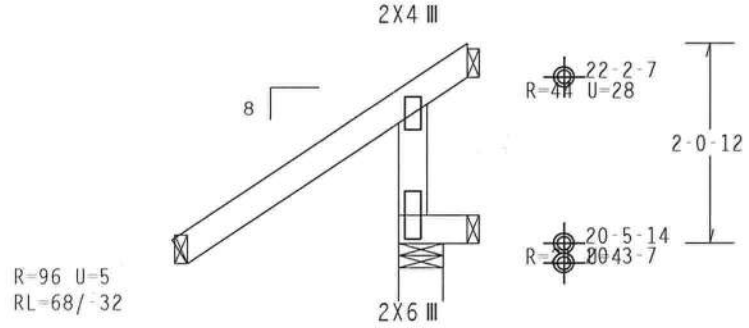
All wind load cases on this truss have a 1.33 duration factor.

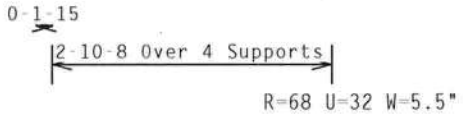
120 mph wind, 21.54 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.02.00.14 01 QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE

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TC LL	20.0 PSF	REF R235-- 94316
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006045
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 11253 REV
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - CJ83)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

These hangers and support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (1)2x8 SP #2 supporting member.
(14) 10d Common, 0.148"x3.0" nails into supporting member,
(4) 10d Common, 0.148"x3.0" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

120 mph wind, 22.21 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

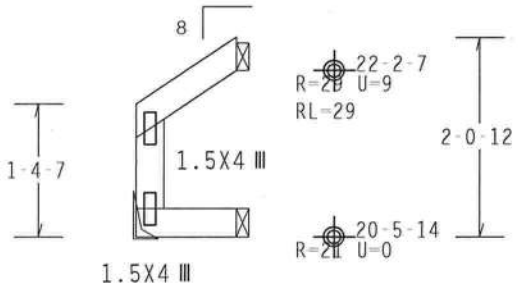
Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	15	0.00	1.04
BC	12	0.00	1.04

***** BEARING ANALOG MODIFIED! *****

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.



1-0-7 Over 3 Supports
R=40 U=21

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF R235-- 94317
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006046
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223172
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - HJ72)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-2.89	9.71
BC	116	0.02	9.71

All wind load cases on this truss have a 1.33 duration factor.

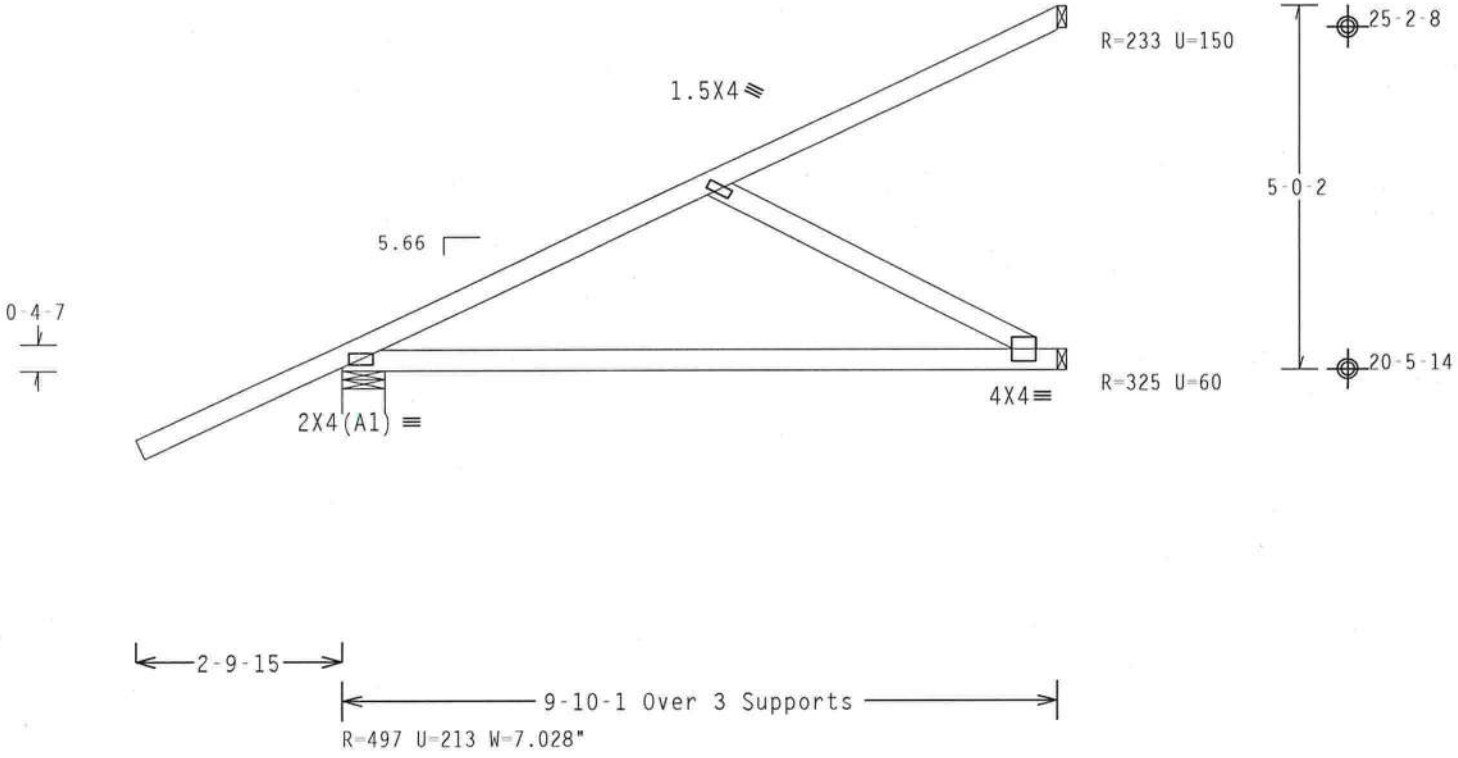
120 mph wind, 22.51 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Hipjack supports 6-11-8 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

Provide (3) 0.131"x3" nails, toe nailed at Top chord.
Provide (3) 0.131"x3" nails, toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:4 FL/-/4/-/E/-/- Scale =.375"/Ft.

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JAMES E. COLLINS JR. LICENSE
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94318
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006034
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223766
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - HJ73)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-2.77	9.84
BC	30	0.15	2.62
BC	87	2.62	9.84

Deflection meets L/240 live and L/180 total load.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (3) 0.162"x3.5" nails, toe nailed at Bot chord

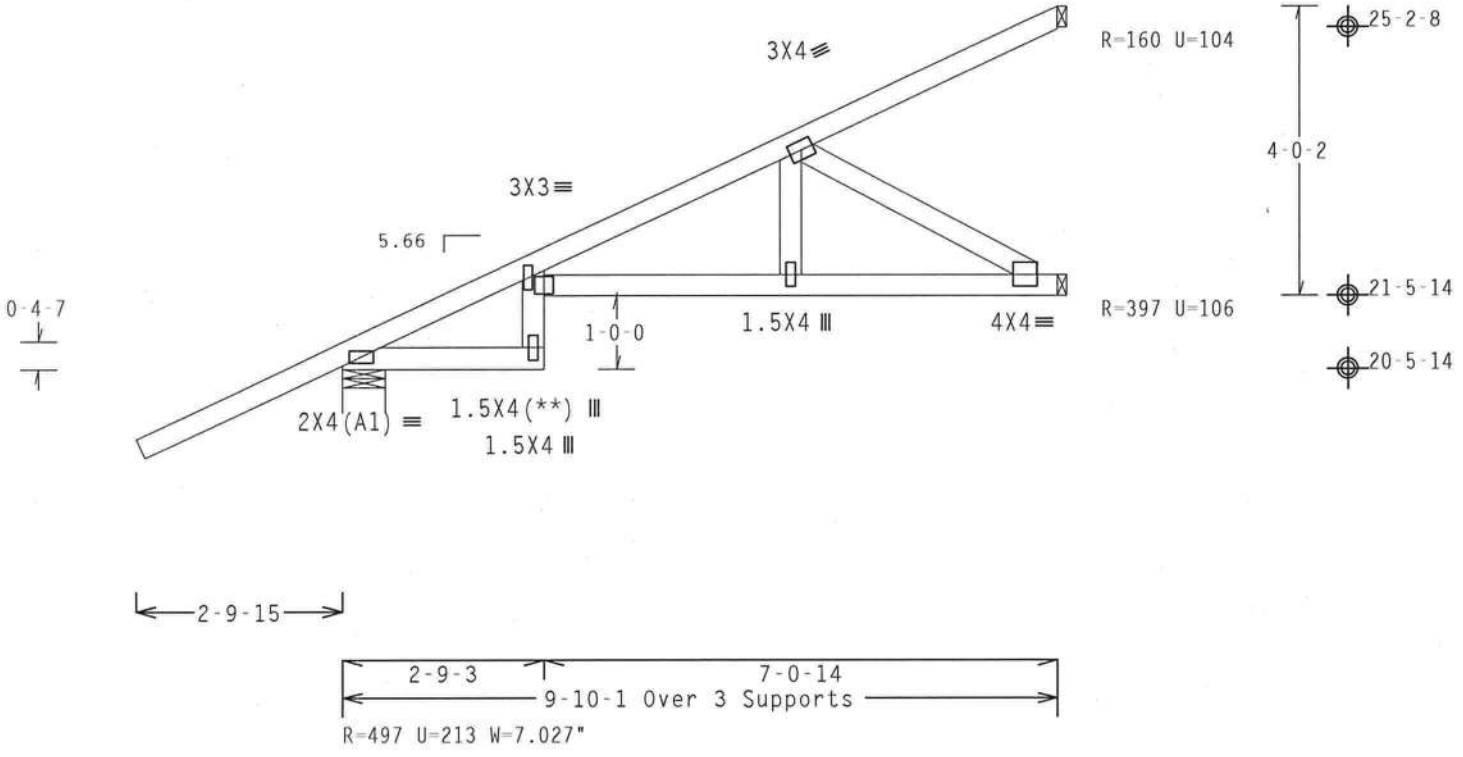
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 22.51 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Hipjack supports 6-11-7 setback jacks with no webs.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2) 9.01.00.4 QTY:1 FL/-/4/-/E/-/- Scale =.375"/Ft.

PLT TYP. Wave

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TC LL	20.0 PSF	REF R235-- 94319
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006061
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223771
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - HJ74)

Top chord 2x6 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	-3.20	6.66
BC	80	0.00	6.66

All wind load cases on this truss have a 1.33 duration factor.

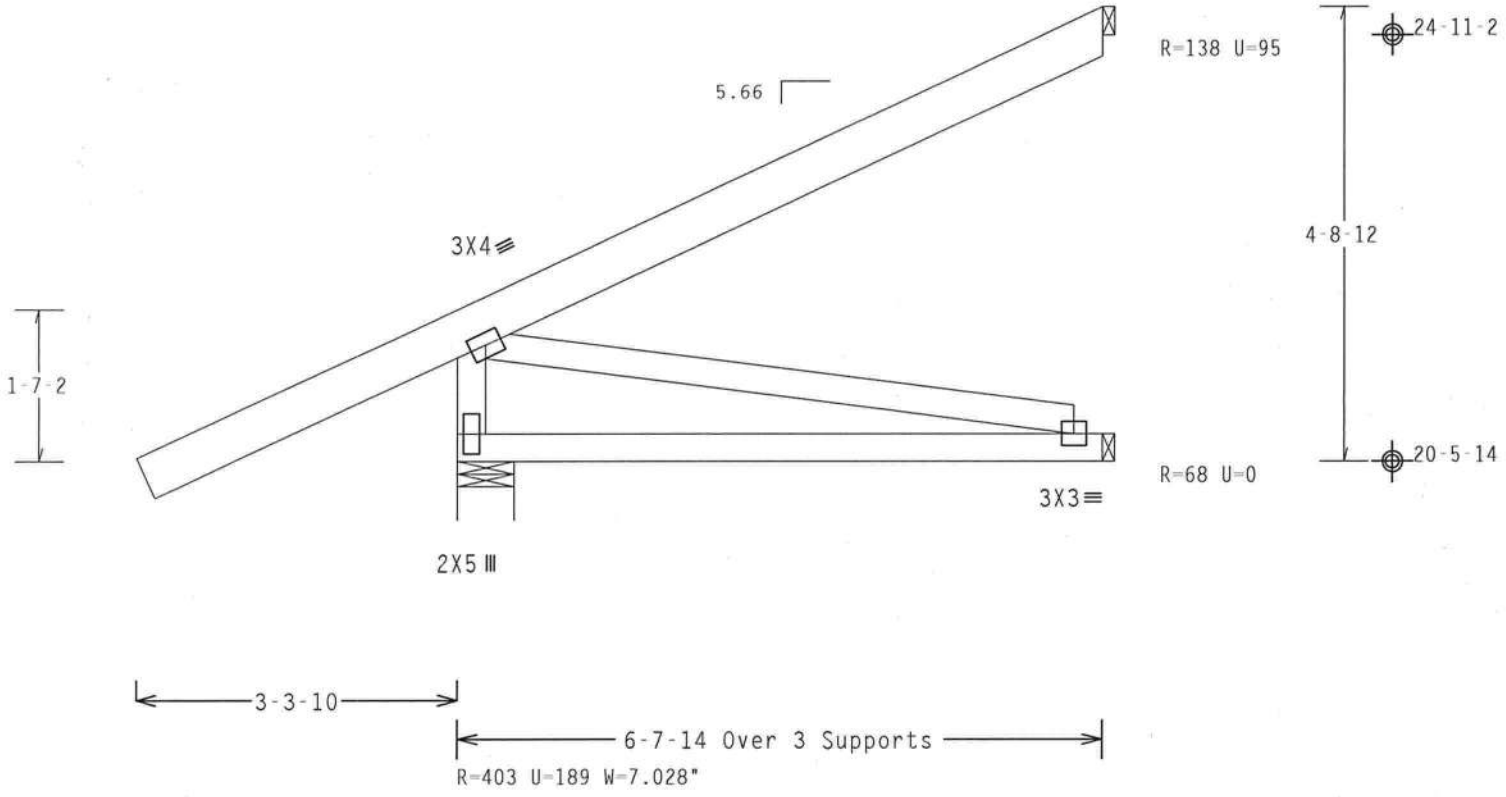
120 mph wind, 22.87 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=3.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Hipjack supports 4-8-8 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

Provide (2) 0.131"x3" nails, toe nailed at Top chord.
Provide (2) 0.131"x3" nails, toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=20%(0%)/5(2) | 9.01.00.12 | QTY:1 | FL/-/4/-/E/-/- | Scale =.5"/Ft.

ALPINE
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JAMES F. TOMLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94320
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006048
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223151
DUR.FAC.	1.25	FROM RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - PB84)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 32.50 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 16.25 ft. from roof edge.

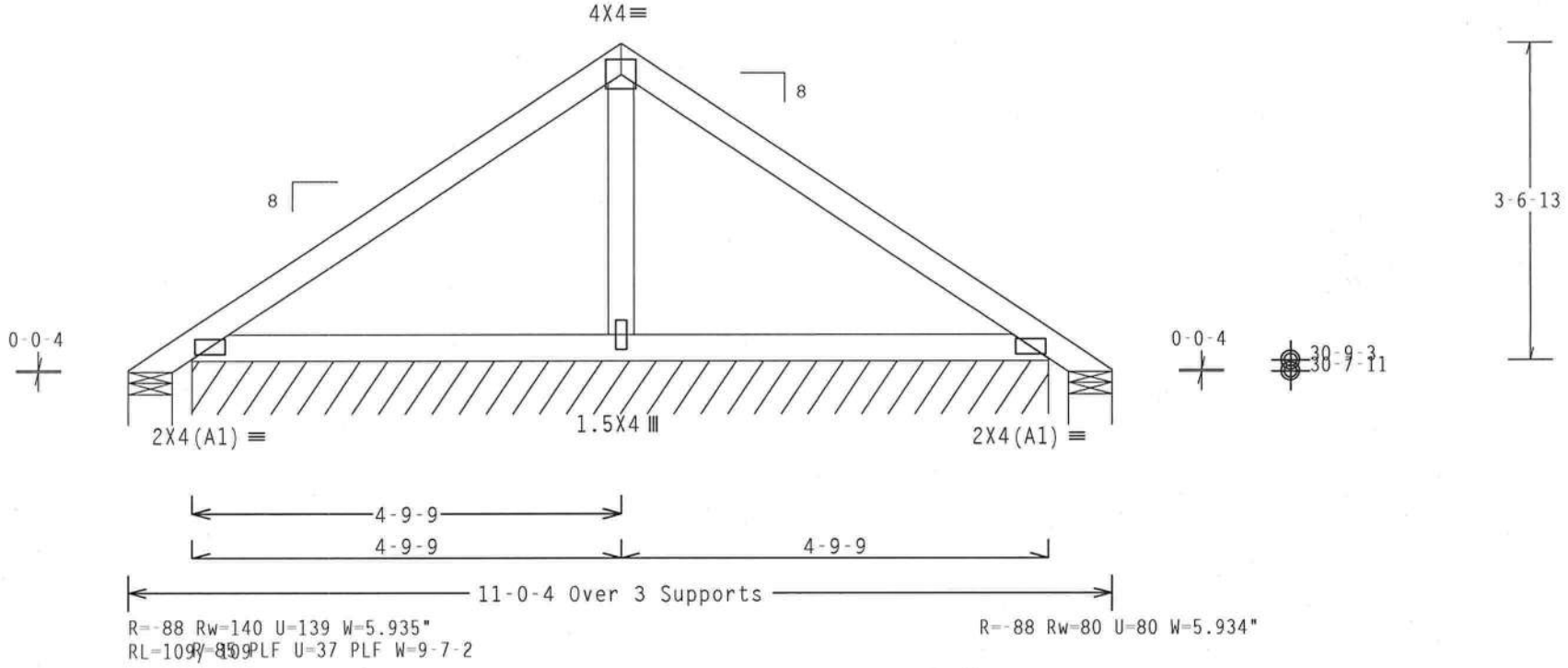
Refer to DWG PB1200109 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 57 plf at -0.71 to 57 plf at 4.80
TC - From 57 plf at 4.80 to 57 plf at 10.31
BC - From 4 plf at -0.71 to 4 plf at 10.31

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	75	0.26	10.82
BC	112	0.89	10.19

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/5(2) 9.01.00.07

PLT TYP. Wave QTY:3 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

JAMES A. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R235-- 94321
TC DL	7.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR235 10006052
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN- 223803
DUR.FAC.	1.25	FROM RCT

(94384B (SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - PB85)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 32.09 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 16.05 ft. from roof edge.

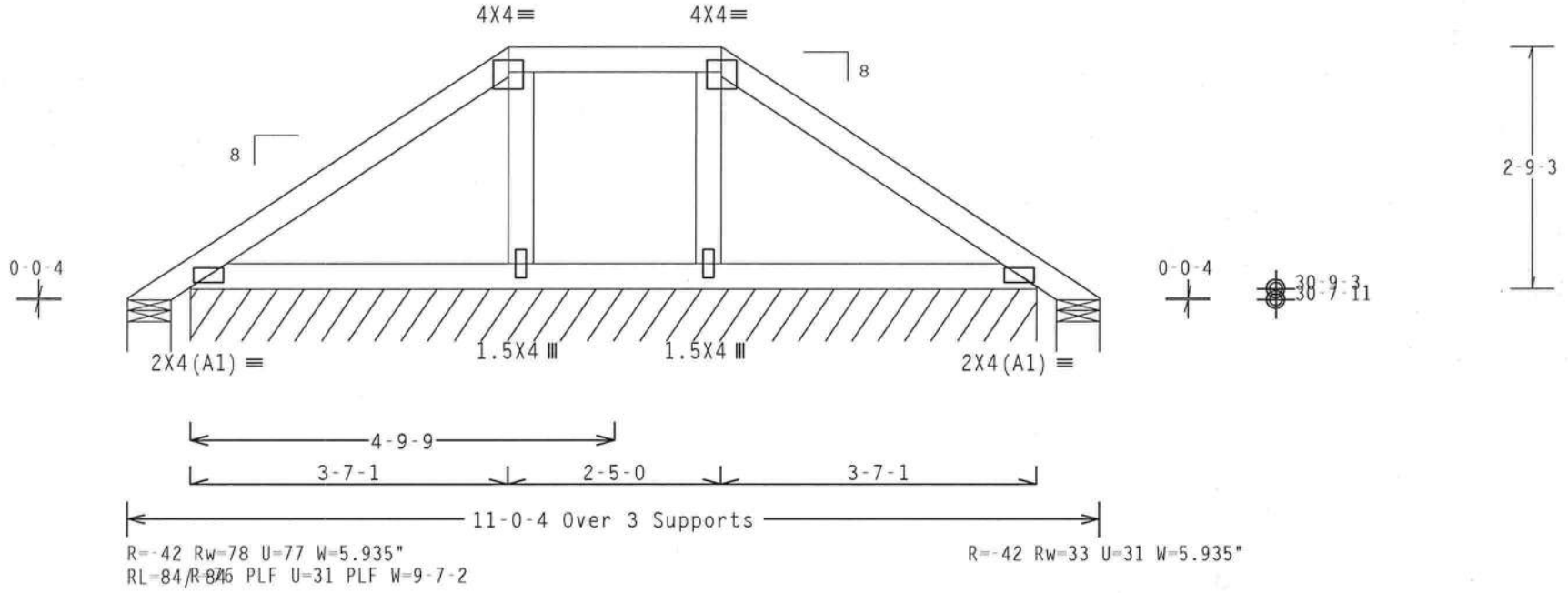
Refer to DWG PB1200109 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC	From	57 plf at	-0.71 to	57 plf at	3.59
TC	From	57 plf at	3.59 to	57 plf at	6.01
TC	From	57 plf at	6.01 to	57 plf at	10.31
BC	From	4 plf at	-0.71 to	4 plf at	10.31

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	59	-0.48	3.59
TC	29	3.59	6.01
TC	59	6.01	10.08
BC	112	0.15	9.45



Design Crit: FBC2007Res/TPI-2002(STD)
PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0	PSF
TC DL	7.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	37.0	PSF
DUR.FAC.	1.25	

REF	R235-- 94322
DATE	01/06/10
DRW	HCUSR235 10006053
HC-ENG	DLJ/DLJ
SEQN-	223808
FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - PB86)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 31.97 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 15.99 ft. from roof edge.

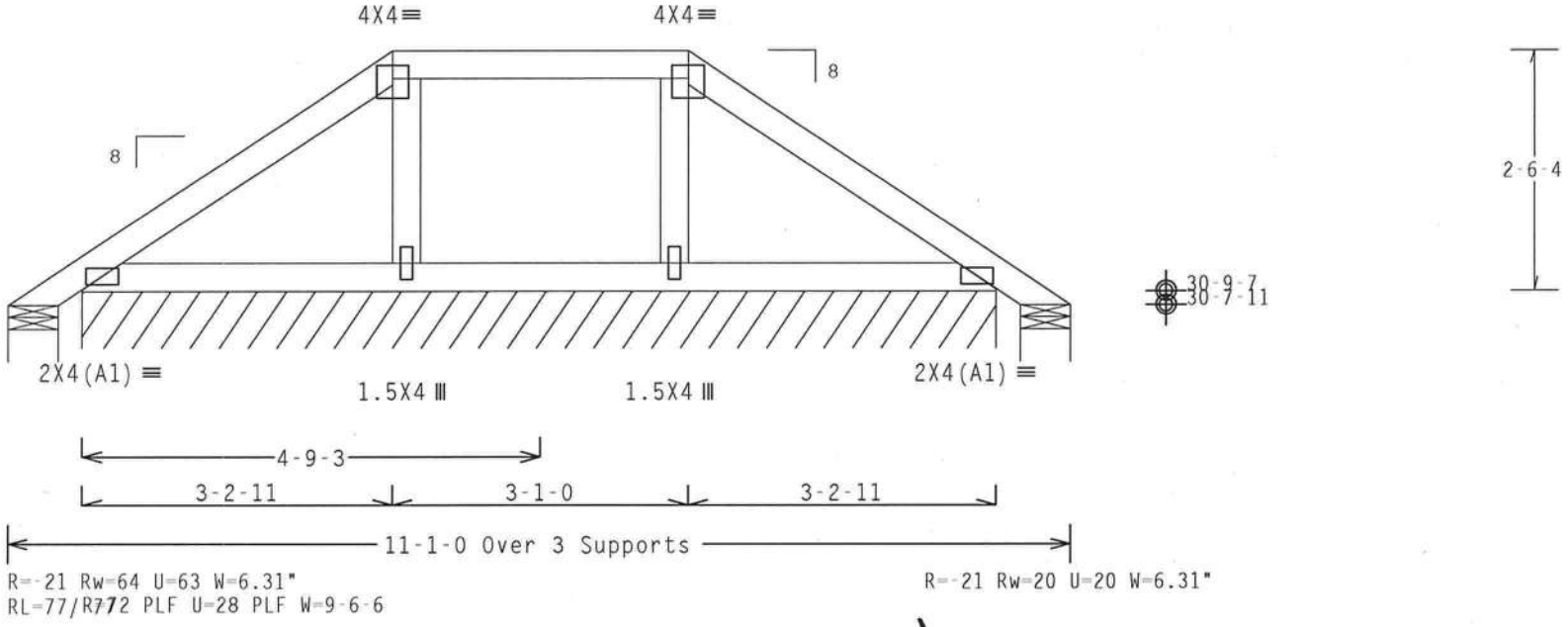
Refer to DWG PB1201009 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC - From	57 plf at	0.00 to	57 plf at	4.00
TC - From	57 plf at	4.00 to	57 plf at	7.08
TC - From	57 plf at	7.08 to	57 plf at	11.08
BC - From	4 plf at	0.00 to	4 plf at	11.08

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	54	0.51	3.22
TC	37	3.22	6.31
TC	54	6.31	10.04
BC	111	0.15	9.39



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01 00.X2i0 QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.

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JAMES F. HOLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94323
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006023
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	273743
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - PB87)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 31.43 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 15.71 ft. from roof edge.

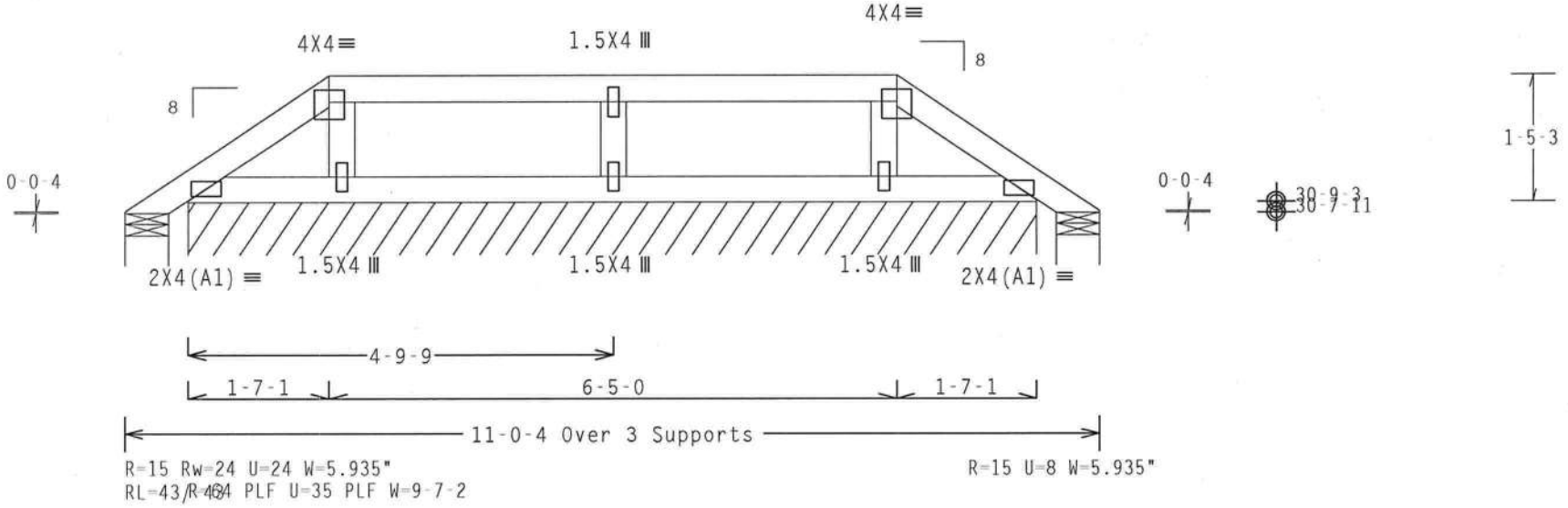
Refer to DWG PB1200109 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC - From	57 plf at	-0.71	to	57 plf at	1.59
TC - From	57 plf at	1.59	to	57 plf at	8.01
TC - From	57 plf at	8.01	to	57 plf at	10.31
BC - From	4 plf at	-0.71	to	4 plf at	10.31

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	30	-0.48	1.59
TC	75	1.59	8.01
TC	30	8.01	10.08
BC	112	0.15	9.45



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00.[illegible] QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R235-- 94324
TC DL	7.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR235 10006056
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	37.0 PSF	SEQN-	223813
DUR.FAC.	1.25	FROM	RCT

(94384B-(SOUTHERN PALMS CONST.)S.W. RIVERLAND COURT/ COL - PB88)

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

120 mph wind, 31.31 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

MWFRS loads based on trusses located at least 15.65 ft. from roof edge.

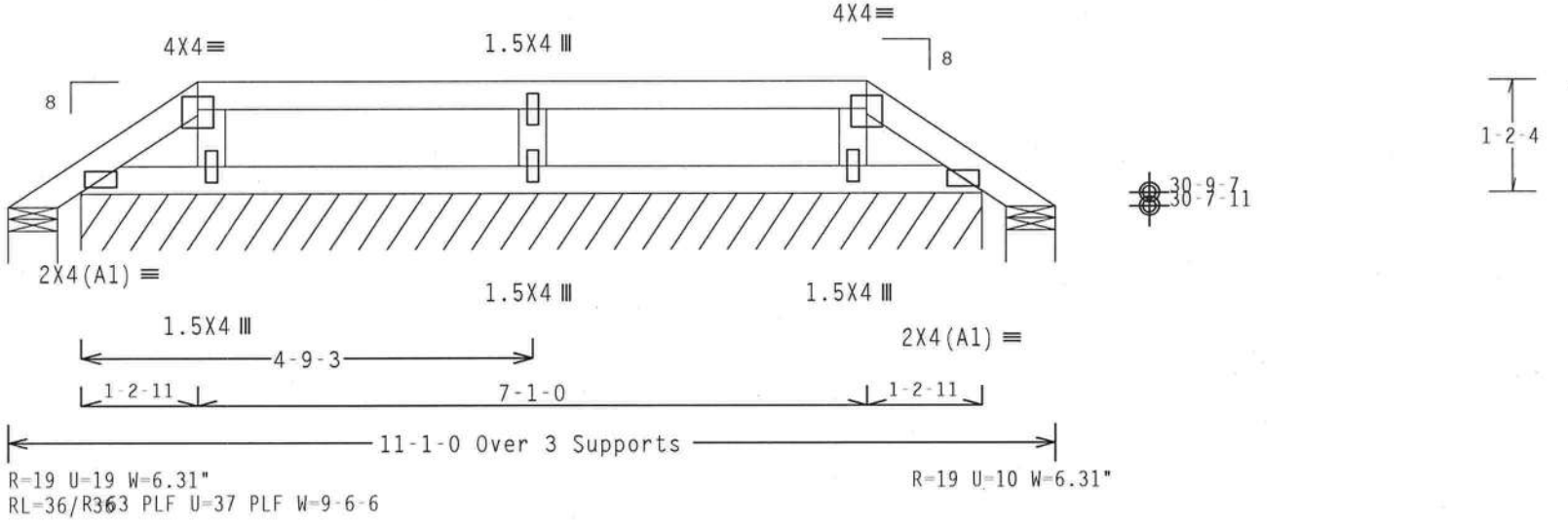
Refer to DWG PB1201009 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC - From	57 plf	at	0.00	to	57 plf	at 2.00
TC - From	57 plf	at	2.00	to	57 plf	at 9.08
TC - From	57 plf	at	9.08	to	57 plf	at 11.08
BC - From	4 plf	at	0.00	to	4 plf	at 11.08

In lieu of structural panels or rigid ceiling use purlins:

CHORD	SPACING(IN OC)	START(FT)	END(FT)
TC	25	0.48	1.26
TC	75	1.26	8.34
TC	25	8.34	10.08
BC	111	0.18	9.42



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/5(2) 9.01.00 [illegible] QTY:1 FL/-/4/-/E/-/- Scale =.5"/Ft.

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JAMES E. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0	PSF	REF R235-- 94325
TC DL	7.0	PSF	DATE 01/06/10
BC DL	10.0	PSF	DRW HCUSR235 10006024
BC LL	0.0	PSF	HC-ENG DLJ/DLJ
TOT.LD.	37.0	PSF	SEQN- 273749
DUR.FAC.	1.25		FROM RCT

120 PIGGYBACK DETAIL

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZT=1.0.

MAXIMUM TRUSS SPACING IS 24" O.C.
DETAIL IS NOT APPLICABLE IF CAP SUPPORTS ADDITIONAL LOADS SUCH AS CUPOLA, STEEPLE, CHIMNEY OR DRAG STRUT LOADS.

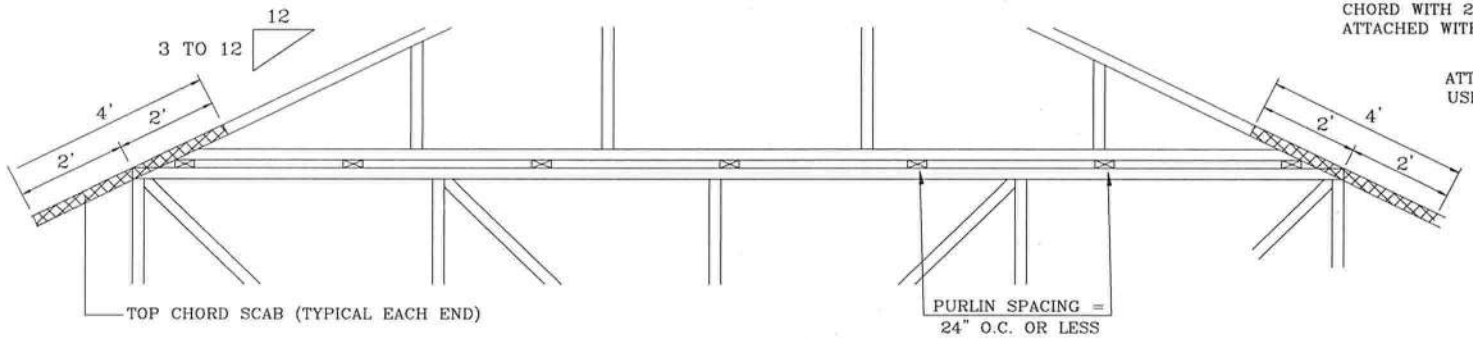
NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING, LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS, OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

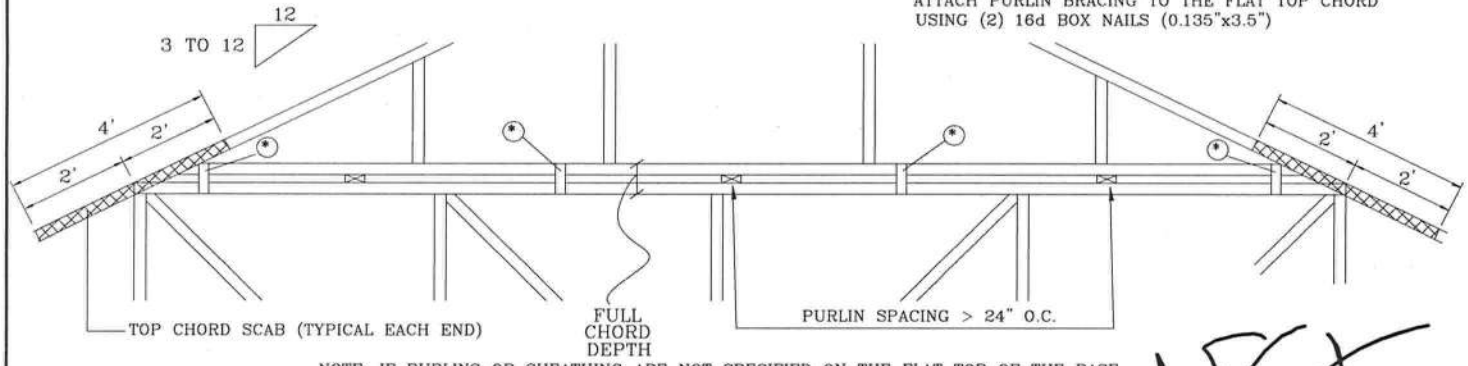
ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")



DETAIL B : PURLIN SPACING > 24" O.C.

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")



NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A

* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

TRULOX
USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8' O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

PLYWOOD GUSSET
8"x8"x1/2" RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8' O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET. (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

2x4 VERTICAL SCABS
2x4 SPF#2, FULL CHORD DEPTH SCABS @ 8' O.C. EACH FACE. STAGGERED 4' O.C. ATTACH WITH (3) 10d BOX NAILS (0.128"x3") INTO BOTH CHORDS (TOTAL OF 6 NAILS PER SCAB).

28PB WAVE PIGGYBACK PLATE
ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8' O.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

ITW
Building Components Group Inc.
Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
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ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

SPACING 24.0"

REF	PIGGYBACK
DATE	10/01/09
DRWG	PB1201009

GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 120 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 5"	5' 11"	6' 0"	7' 0"	7' 2"	8' 4"	8' 6"	10' 11"	11' 3"	13' 1"	13' 5"
		#3	3' 4"	4' 11"	4' 11"	6' 5"	6' 5"	8' 4"	8' 4"	10' 0"	10' 0"	13' 1"	13' 1"
		STUD	3' 4"	4' 10"	4' 10"	6' 5"	6' 5"	8' 4"	8' 4"	10' 0"	10' 0"	13' 1"	13' 1"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 7"	8' 7"	11' 8"	11' 8"
	SP DFL	#1	3' 9"	5' 11"	6' 4"	7' 0"	7' 6"	8' 4"	8' 11"	10' 11"	11' 10"	13' 1"	14' 0"
		#2	3' 8"	5' 11"	6' 4"	7' 0"	7' 6"	8' 4"	8' 11"	10' 11"	11' 10"	13' 1"	14' 0"
		#3	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 4"	8' 9"	10' 3"	10' 3"	13' 1"	13' 9"
		STUD	3' 6"	4' 11"	4' 11"	6' 6"	6' 6"	8' 4"	8' 9"	10' 2"	10' 2"	13' 1"	13' 9"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 7"	7' 7"	8' 9"	8' 9"	11' 11"	11' 11"
16" O.C.	SPF HF	#1 / #2	3' 11"	6' 9"	6' 11"	8' 0"	8' 2"	9' 6"	9' 9"	12' 6"	12' 11"	14' 0"	14' 0"
		#3	3' 10"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 6"	12' 3"	12' 3"	14' 0"	14' 0"
		STUD	3' 10"	5' 11"	5' 11"	7' 10"	7' 10"	9' 6"	9' 6"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 1"	5' 1"	6' 9"	6' 9"	9' 1"	9' 1"	10' 6"	10' 6"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 9"	7' 3"	8' 0"	8' 7"	9' 6"	10' 3"	12' 6"	13' 6"	14' 0"	14' 0"
		#2	4' 2"	6' 9"	7' 3"	8' 0"	8' 7"	9' 6"	10' 3"	12' 6"	13' 6"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	8' 0"	8' 1"	9' 6"	10' 0"	12' 6"	12' 7"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	8' 0"	8' 0"	9' 6"	10' 0"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 3"	5' 3"	6' 11"	6' 11"	9' 3"	9' 3"	10' 9"	10' 9"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 4"	7' 5"	7' 7"	8' 9"	9' 0"	10' 6"	10' 9"	13' 10"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 6"	10' 6"	13' 10"	13' 10"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 6"	10' 6"	13' 10"	13' 10"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 6"	10' 6"	12' 2"	12' 2"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 5"	8' 0"	8' 9"	9' 6"	10' 6"	11' 4"	13' 10"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 5"	8' 0"	8' 9"	9' 6"	10' 6"	11' 4"	13' 10"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 1"	7' 1"	8' 9"	9' 3"	10' 6"	11' 0"	13' 10"	14' 0"	14' 0"	14' 0"
		STUD	4' 5"	7' 0"	7' 0"	8' 9"	9' 3"	10' 6"	11' 0"	13' 10"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 1"	6' 1"	8' 0"	8' 0"	10' 6"	10' 9"	12' 5"	12' 5"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 130 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

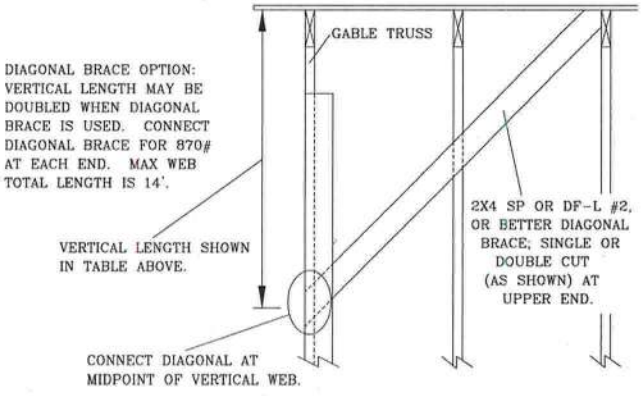
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

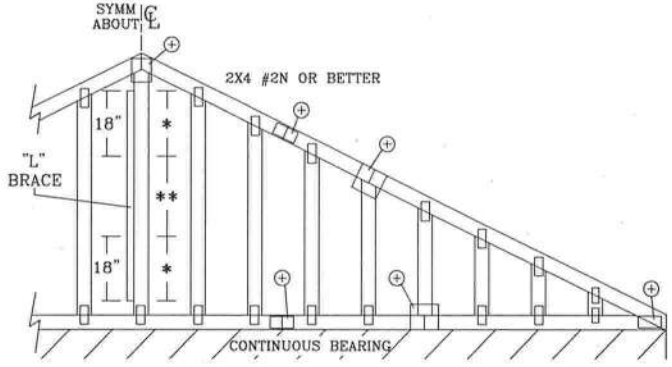
ATTACH EACH "L" BRACE WITH 10d NAILS. (0.128"x3" min)

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	3X4
GREATER THAN 11' 6"	3.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ITW Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

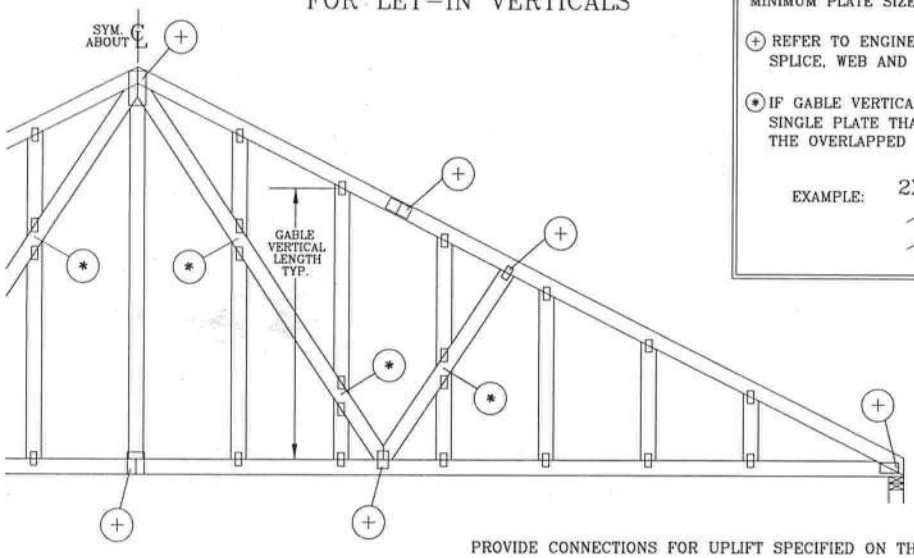
IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/50 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB12030
DATE	1/1/09
DRWG	A12030050109

GABLE DETAIL
FOR LET-IN VERTICALS



GABLE TRUSS PLATE SIZES

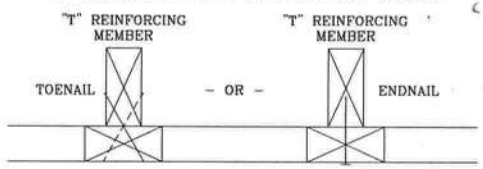
REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

(*) IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE: 2X4 / 2X4 — 2X8

"T" REINFORCEMENT ATTACHMENT DETAIL



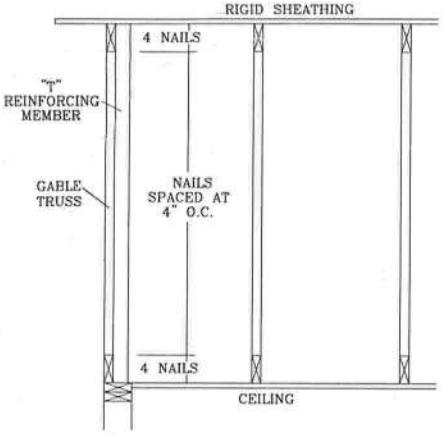
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
END DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) NAILS AT 4" O.C. PLUS
(4) NAILS IN TOP AND BOTTOM CHORD.
TOENAILED NAILS:
10d COMMON (0.148"x3",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

Jan 06 10

JAMES A. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	1/1/09
DRWG	GBLLETIN0109

LUMBER UNLIMITED

2175 West 18th Street
P.O. Box 12267
Jacksonville, FL 32209
904-357-5440

Cover Page

Job #: 94384A

Date: 1/6/2010

Customer Information:

Name: SOUTHERN PALMS CONST.
Contact:
Address:
City, State, Zip:

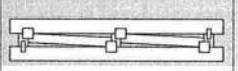
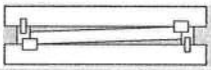
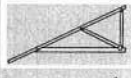
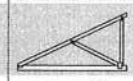
Project Information:

Name: S.W. RIVERLAND COURT/ COLUMBIA CTY.
Address:
City, State, Zip: FT. WHITE, FL

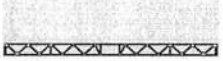
Salesman: HOUSE
Designer: RICHARD

Notes:

WoodTruss

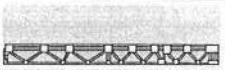
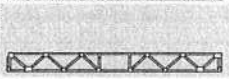
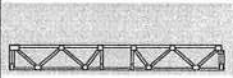
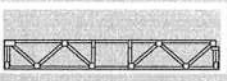
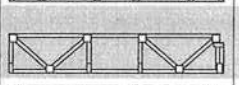
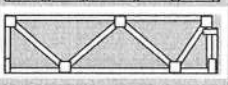
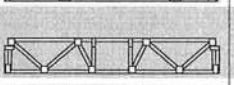
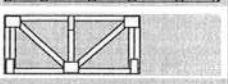
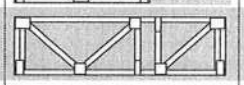
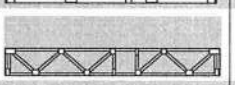
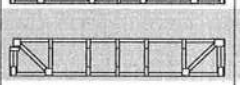
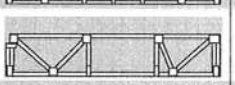
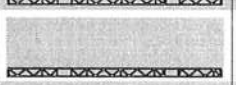
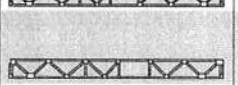
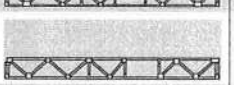
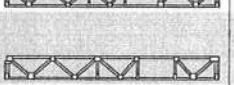
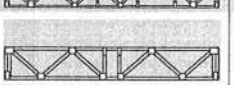
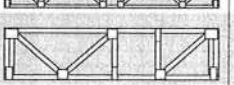
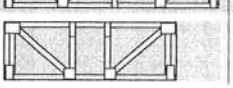
Qty	Span	Description	Truss	Slope TC/BC	OH - L OH - R	PRICE EACH	PRICE QTY
1	06-09-08	FT36 53.19 lbs. each		0.00 2x6 / 2x6	00-00-00 00-00-00		
1	04-07-12	FT37 36.39 lbs. each		0.00 2x6 / 2x6	00-00-00 00-00-00		
33	07-01-00	EJ27 36.39 lbs. each		8.00 2x4 / 2x4	02-00-00 00-00-00		
3	09-11-07	HJ28 60.19 lbs. each		5.66 2x4 / 2x6	02-09-15 00-00-00		
1	09-06-08	HJ29 54.6 lbs. each		5.66 2x4 / 2x6	00-00-00 00-00-00		
7	05-00-07	CJ30 22.39 lbs. each		8.00 2x4 / 2x4	02-00-00 00-00-00		
7	03-00-07	CJ31 16.8 lbs. each		8.00 2x4 / 2x4	02-00-00 00-00-00		
7	01-00-07	CJ32 8.4 lbs. each		8.00 2x4 / 2x4	02-00-00 00-00-00		
1	04-08-15	CJ33 16.8 lbs. each		8.00 2x4 / 2x4	00-00-00 00-00-00		
1	02-08-15	CJ34 11.19 lbs. each		8.00 2x4 / 2x4	00-00-00 00-00-00		
1	00-08-15	CJ35 5.586 lbs. each		8.00 2x4 / 2x4	00-00-00 00-00-00		

WoodFloor

Qty	Span	Description	Truss			PRICE EACH	PRICE QTY
10	24-01-08	F1 142.8 lbs. each					

Or Quote 11,428.20 + taxes

Job#: 94384A **QUOTE**

Qty	Span	Description	Truss			PRICE EACH	PRICE QTY
1	14-06-04	F2 159.6 lbs. each					
2	30-10-00	F3 179.2 lbs. each					
3	30-10-00	F4 176.4 lbs. each					
2	15-01-00	F5 89.59 lbs. each					
1	22-08-04	F6 168 lbs. each					
1	11-04-01	F7 72.79 lbs. each					
1	09-09-08	F8 61.59 lbs. each					
1	07-02-10	F9 47.59 lbs. each					
1	04-10-05	F10 36.39 lbs. each					
1	08-00-12	F11 58.78 lbs. each					
2	03-01-00	F12 30.79 lbs. each					
1	04-09-15	F13 41.98 lbs. each					
2	10-06-00	F14 69.99 lbs. each					
1	07-07-12	F15A 53.19 lbs. each					
4	06-09-00	F15 50.38 lbs. each					
2	32-06-00	F16 196 lbs. each					
1	32-09-00	F17 201.6 lbs. each					
1	31-06-00	F18 193.2 lbs. each					
1	16-02-01	F19 100.8 lbs. each					
1	14-10-08	F20 92.4 lbs. each					
1	13-07-00	F21 84 lbs. each					
1	12-03-07	F22 81.19 lbs. each					
1	08-08-10	F23 61.59 lbs. each					
1	05-05-12	F24 41.98 lbs. each					
1	03-11-01	F25 33.58 lbs. each					

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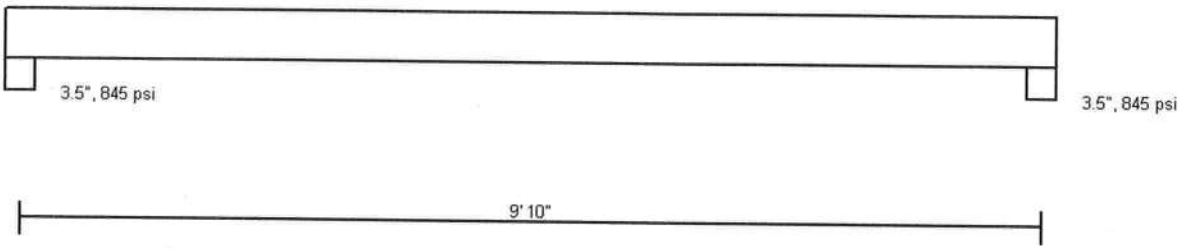
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-5**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Trapezoidal(plf)	330	478	240	348	100%	0	0' 0"	9' 10"	
2	Trapezoidal(plf)	303	200	220	145	100%	0	6' 9"	9' 10"	
	Uniform(plf)	9		0			0	0	9' 10"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	2039	2797	
Min R'n	588	795	
DL R'n	588	795	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	2690	1	9' 8"	21	6318	100%	0.43	
M(ft-lbs)	5670	1	5' 6"	21	13056	100%	0.43	
LtRn(lbs)	2039	0	0' 0"	21	10351		0.20	See Note #4
RtRn(lbs)	2797	0	9' 10"	21	10351		0.27	See Note #4
LLDefl(in.)	0.16	1	4' 11"	21	0.33		L/759	
TLDefl(in.)	0.22	1	4' 11"	21	0.49		L/541	

USE: **GPLAM 2.0E 1.75x 9.50" 2 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
12. *For explanation of GROUP, change to expanded printout.*

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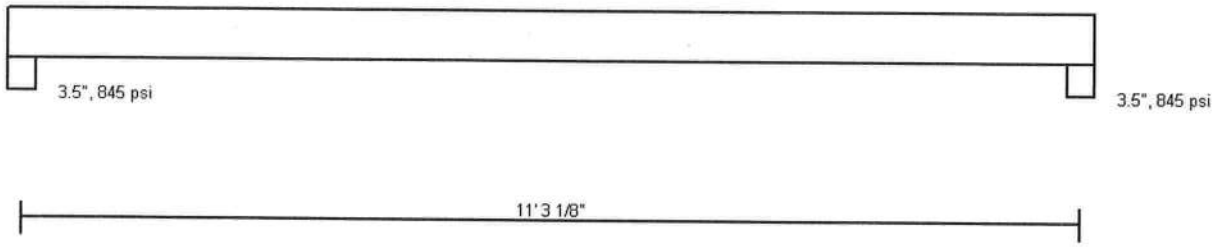
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-6**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Trapezoidal(plf)	0	180	0	131	100%	0	0' 0"	11' 3 1/8"	
2	Trapezoidal(plf)	303	200	220	145	100%	0	7' 9"	11' 3 1/8"	
	Uniform(plf)	9		0			0	0	11' 3 1/8"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	535	1462	
Min R'n	183	436	
DL R'n	183	436	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	1398	1	11' 1"	21	6318	100%	0.22	
M(ft-lbs)	2614	1	7' 0"	21	13056	100%	0.20	
LtRn(lbs)	535	0	0' 0"	21	10351		0.05	See Note #4
RtRn(lbs)	1462	0	11' 3"	21	10351		0.14	See Note #4
LLDefl(in.)	0.08	1	5' 8"	21	0.38		L/1614	
TLDefl(in.)	0.12	1	5' 8"	21	0.56		L/1106	

USE: **GPLAM 2.0E 1.75x 9.50" 2 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
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deleted these after rev. on 1/7/10

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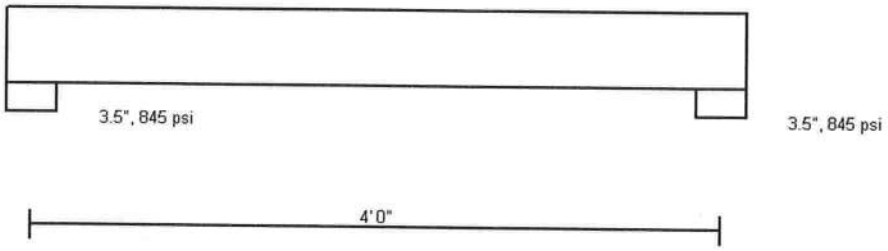
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-10**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Trapezoidal(plf)	284	124	207	90	100%	0	0' 0"	4' 0"	
	Uniform(plf)	15		0			0	0	4' 0"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	492	385	
Min R'n	156	127	
DL R'n	156	127	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	428	1	0' 2"	21	10640	100%	0.04	
M(ft-lbs)	439	1	1' 9"	21	34954	100%	0.01	
LtRn(lbs)	492	0	0' 0"	21	10351		0.05	See Note #4
RtRn(lbs)	385	0	4' 0"	21	10351		0.04	See Note #4
LLDefl(in.)	0.00	1	2' 0"	21	0.13		L/49578	
TLDefl(in.)	0.00	1	2' 0"	21	0.20		L/33563	

USE: GPLAM 2.0E 1.75x16.00" 2 Plies — Grade, Depth, Plies selected by user
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 3 rows of 16d nails @ 12" o/c (one row 2" from top, one row at mid-depth, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
12. *For explanation of GROUP, change to expanded printout.*

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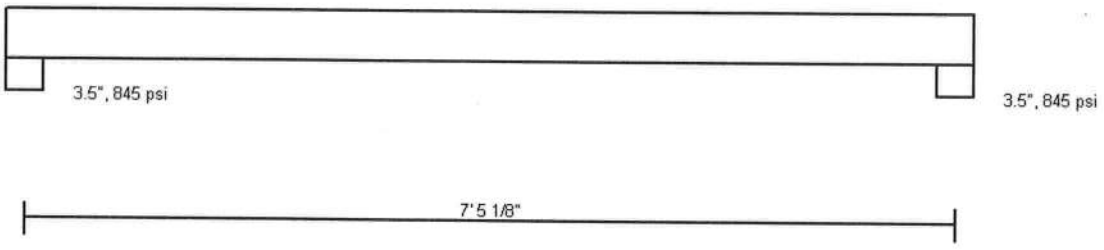
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-11**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Concentrated(lbs)	385		280		100%	0	3' 0 1/2"		
2	Partial(plf)	110		0		100%	0	3' 8"	7' 5 1/8"	
	Uniform(plf)	9		0			0	0	7' 5 1/8"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	365	500	
Min R'n	200	385	
DL R'n	200	385	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	395	1	7' 3"	21	6318	100%	0.06	
M(ft-lbs)	1070	1	3' 1"	21	13056	100%	0.08	
LtRn(lbs)	365	0	0' 0"	21	10351		0.04	See Note #4
RtRn(lbs)	500	0	7' 5"	21	10351		0.05	See Note #4
LLDefl(in.)	0.01	1	3' 9"	21	0.25		L/9530	
TLDefl(in.)	0.02	1	3' 9"	21	0.37		L/3819	

USE: **GPLAM 2.0E 1.75x 9.50" 2 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
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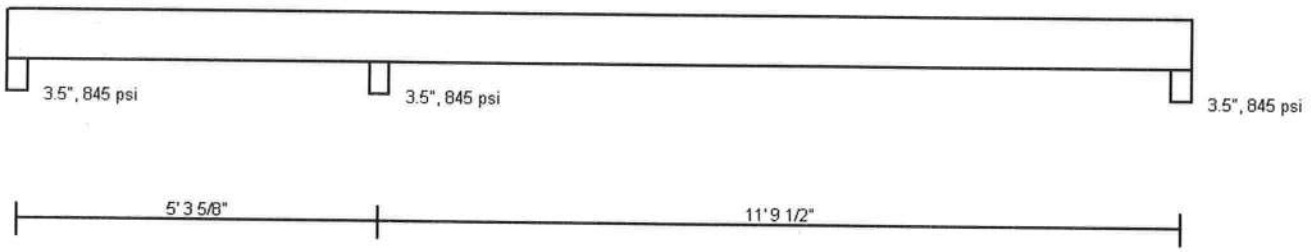
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-12**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Partial(plf)	110		0		100%	0	0' 0"	6' 10 5/8"	
2	Concentrated(lbs)	492		358		100%	0	6' 10"		
3	Concentrated(lbs)	365		265		100%	0	10' 1 7/8"		
4	Span Carried(psf)	55		40		100%	0	10' 1 7/8"	17' 1 1/8"	7' 9 1/2" s.c. -
	Uniform(plf)	9		0			0	0	17' 1 1/8"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	3	
Max R'n	137	2337	1102	
Min R'n	-185	1083	327	
DL R'n	137	1083	327	
Uplift	185	0	0	
Min Brg(in.)	1.50	3.00	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	1398	2	0' 2"	21	6318	100%	0.22	
M(ft-lbs)	2655	2	0' 0"	21	12630	100%	0.21	
LtRn(lbs)	137	0	0' 0"	10	10351		0.01	See Note #5
RtRn(lbs)	1102	0	17' 1"	21	10351		0.11	See Note #5
IntRn(lbs)	2337	0	5' 4"	21	10351		0.23	See Note #5
LLDefl(in.)	0.09	2	5' 11"	21	0.39		L/1559	
TLDefl(in.)	0.13	2	5' 11"	23	0.59		L/1104	

USE: **GPLAM 2.0E 1.75x 9.50" 2 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Allowable negative moment is calculated based on bottom edge laterally unsupported between bearing locations.*
4. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
5. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
6. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
7. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
8. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
9. *Provide approved uplift resistance at supports with negative reactions.*
10. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
11. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
12. *Max/Min reactions are based on the applicable load combinations outlined in the notes. Summation of max/min reactions for various DOL may not match total max/min reaction.*
13. *Analysis valid for dry-use only (less than 16% moisture content).*
14. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
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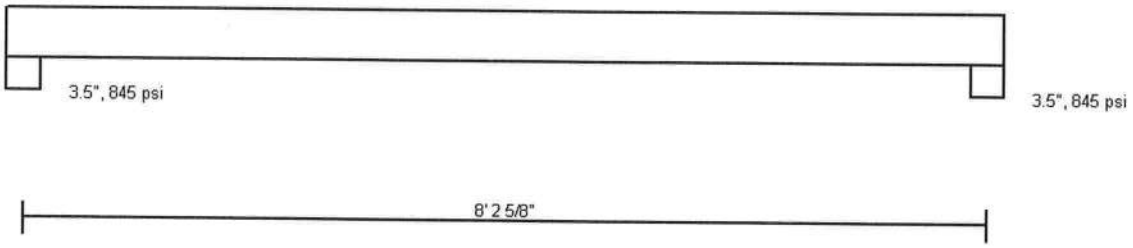
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-13**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Concentrated(lbs)	1102		801		100%	0	0' 7 3/4"		
	Uniform(plf)	9		0			0	0	8' 2 5/8"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	1052	124	
Min R'n	314	61	
DL R'n	314	61	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	728	1	0' 2"	21	6318	100%	0.12	
M(ft-lbs)	678	1	0' 8"	21	13056	100%	0.05	
LtRn(lbs)	1052	0	0' 0"	21	10351		0.10	See Note #4
RtRn(lbs)	124	0	8' 3"	21	10351		0.01	See Note #4
LLDefl(in.)	0.01	1	4' 1"	21	0.27		L/11763	
TLDefl(in.)	0.01	1	4' 1"	21	0.41		L/7231	

USE: **GPLAM 2.0E 1.75x 9.50" 2 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
12. *For explanation of GROUP, change to expanded printout.*

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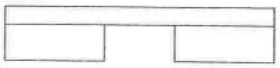
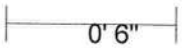
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-16**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



3.5", 845 psi



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF Span#	Location* Starts	Ends	Additional Info
	Uniform(plf)	15		0		0	0	0' 6"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	4	4	
Min R'n	4	4	
DL R'n	4	4	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	0	0	0' 0"	0	10640	100%	0.00	
M(ft-lbs)	11	1	1' 6"	10	31459	90%	0.00	
LtRn(lbs)	4	0	0' 0"	10	10351		0.00	See Note #4
RtRn(lbs)	4	0	0' 6"	10	10351		0.00	See Note #4
LLDefl(in.)	0.00	1	0' 0"	0	0.02		L/60000	
TLDefl(in.)	0.00	1	0' 3"	10	0.03		L/60000	

USE: GPLAM 2.0E 1.75x16.00" 2 Plies — Grade, Depth, Plies selected by user

GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 3 rows of 16d nails @ 12" o/c (one row 2" from top, one row at mid-depth, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
12. *For explanation of GROUP, change to expanded printout.*

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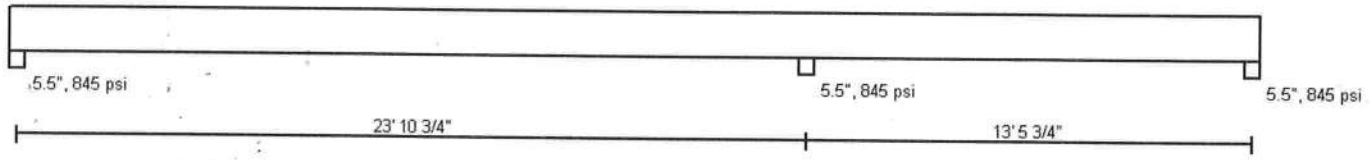
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-1**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf; Roof: Live=20 psf, Dead=17 psf.

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Uniform(plf)	110		0		100%	0	0' 0"	37' 4 1/2"	
2	Span Carried(psf)	37		20		125%	0	0' 0"	37' 4 1/2"	7' 1" s.c. -
3	Trapezoidal(plf)	0	200	0	108	125%	0	0' 0"	7' 0"	
4	Concentrated(lbs)	965		702		125%	0	7' 0"		
5	Concentrated(lbs)	484		262		125%	0	9' 0"		
6	Concentrated(lbs)	448		242		125%	0	11' 0"		
7	Concentrated(lbs)	434		235		125%	0	13' 0"		
8	Concentrated(lbs)	945		511		125%	0	15' 0"		
9	Concentrated(lbs)	945		511		125%	0	17' 0"		
10	Concentrated(lbs)	945		511		125%	0	19' 0"		
11	Concentrated(lbs)	945		511		125%	0	21' 0"		
12	Concentrated(lbs)	6446		3484		125%	0	23' 0"		
13	Partial(plf)	200		108		125%	0	24' 0"	37' 4 1/2"	
	Uniform(plf)	23		0			0	0	37' 4 1/2"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	3	
Max R'n	4952	20288	1611	
Min R'n	2774	11000	-493	
DL R'n	2834	11000	510	
Uplift	0	0	493	
Min Brg(in.)	1.50	4.57	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	11523	1	23' 8"	41	19950	125%	0.58	
M(ft-lbs)	33928	1	23' 11"	41	61867	125%	0.55	
LtRn(lbs)	4952	0	0' 0"	42	24399		0.20	See Note #5
RtRn(lbs)	1611	0	37' 4"	43	24399		0.07	See Note #5
IntRn(lbs)	20288	0	23' 11"	41	24399		0.83	See Note #5
LLDefl(in.)	0.38	1	11' 11"	42	0.80		L/760	
TLDefl(in.)	0.82	1	11' 11"	42	1.19		L/351	

USE: **GPLAM 2.0E 1.75x16.00" 3 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Allowable negative moment is calculated based on bottom edge laterally unsupported between bearing locations.*
4. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
5. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
6. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
7. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
8. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
9. *Provide approved uplift resistance at supports with negative reactions.*
10. *For beams with loads applied equally to the top of all plies, nail plies together with 3 rows of 16d nails @ 12" o/c (one row 2" from top, one row at mid-depth, one row 2" from bottom). Specified attachment is from each side.*

11. For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.
12. Max/Min reactions are based on the applicable load combinations outlined in the notes. Summation of max/min reactions for various DOL may not match total max/min reaction.
13. Analysis valid for dry-use only (less than 16% moisture content).
14. Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.
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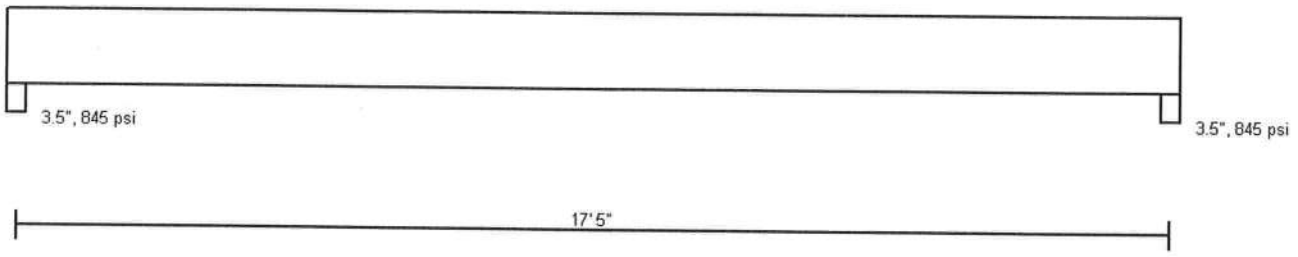
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**

Mark # : **BM-2**

Usage : **Beam (Floor)** Spacing (in.) : **0.0**

Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf; Roof: Live=20 psf, Dead=17 psf.

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Uniform(plf)	200		108		125%	0	0' 0"	17' 5"	
2	Span Carried(psf)	37		20		125%	0	0' 0"	17' 5"	7' 1" s.c. -
	+ Wall(plf)	110		0			0	0' 0"	17' 5"	
	Uniform(plf)	15		0			0	0	17' 5"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	3972	3972	
Min R'n	2413	2413	
DL R'n	2413	2413	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	3298	1	15' 11"	41	13300	125%	0.25	
M(ft-lbs)	17297	1	8' 9"	41	43692	125%	0.40	
LtRn(lbs)	3972	0	0' 0"	41	10351		0.38	See Note #4
RtRn(lbs)	3972	0	17' 5"	41	10351		0.38	See Note #4
LLDefl(in.)	0.17	1	8' 9"	41	0.58		L/1235	
TLDefl(in.)	0.43	1	8' 9"	41	0.87		L/485	

USE: **GPLAM 2.0E 1.75x16.00" 2 Plies** **Grade, Depth, Plies selected by user**

GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 3 rows of 16d nails @ 12" o/c (one row 2" from top, one row at mid-depth, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Max/Min reactions are based on the applicable load combinations outlined in the notes. Summation of max/min reactions for various DOL may not match total max/min reaction.*
11. *Analysis valid for dry-use only (less than 16% moisture content).*
12. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
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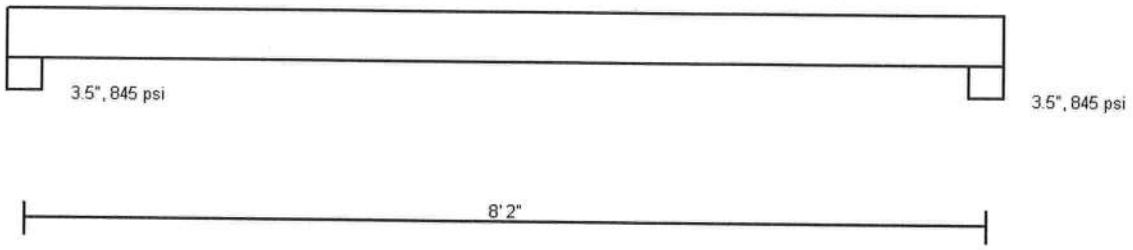
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-3**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**



LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Span Carried(psf)	55		40		100%	0	0' 0"	8' 2"	12' 6" s.c. -
	Uniform(plf)	9		0			0	0	8' 2"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	1441	1441	
Min R'n	421	421	
DL R'n	421	421	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	1110	1	0' 2"	21	6318	100%	0.18	
M(ft-lbs)	2943	1	4' 1"	21	13056	100%	0.23	
LtRn(lbs)	1441	0	0' 0"	21	10351		0.14	See Note #4
RtRn(lbs)	1441	0	8' 2"	21	10351		0.14	See Note #4
LLDefl(in.)	0.06	1	4' 1"	21	0.27		L/1712	
TLDefl(in.)	0.08	1	4' 1"	21	0.41		L/1212	

USE: GPLAM 2.0E 1.75x 9.50" 2 Plies — Grade, Depth, Plies selected by user

GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
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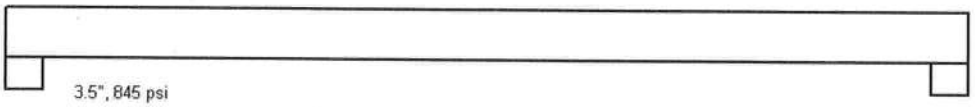
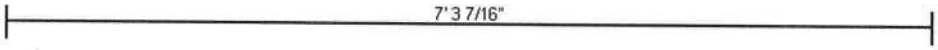
Project : **94384.~FB** **SOUTHERN PLANS CONSTRUCTION MARTIN RESID**
Mark # : **BM-4**
Usage : **Beam (Floor)** Spacing (in.) : **0.0**
Max Defl : **LL = L/360 TL = L/240**





LOADS

Project Design Loads : Floor: Live=40 psf, Dead=15 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Trapezoidal(plf)	55	233	40	169	100%	0	0' 0"	7' 3 7/16"	
	Uniform(plf)	9		0			0	0	7' 3 7/16"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	449	666	
Min R'n	146	205	
DL R'n	146	205	
Min Brg(in.)	1.50	1.50	[Based on bearing stress below]
Brg Str(psi)	845	845	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	623	1	7' 2"	21	6318	100%	0.10	
M(ft-lbs)	1024	1	4' 1"	21	13056	100%	0.08	
LtRn(lbs)	449	0	0' 0"	21	10351		0.04	See Note #4
RtRn(lbs)	666	0	7' 3"	21	10351		0.06	See Note #4
LLDefl(in.)	0.02	1	3' 8"	21	0.24		L/5573	
TLDefl(in.)	0.02	1	3' 8"	21	0.36		L/3815	

USE: **GPLAM 2.0E 1.75x 9.50" 2 Plies** **Grade, Depth, Plies selected by user**
GP LAM tm Georgia-Pacific Wood Products, LLC

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide full depth lateral support at all bearing locations. Allowable positive moment is calculated based on top edge with continuous lateral support.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Wood Products LLC.*
4. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
5. *Bearing length (Min Brg(in.)) based on allowable stress of support material (Brg Str(psi)); support material capacity shall be verified (by others).*
6. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
7. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
8. *For beams with loads applied equally to both plies, either top or side loaded, nail plies together with 2 rows of 16d nails @ 12" o/c (one row 2" from top, one row 2" from bottom).*
9. *For beams with loads not applied equally to all plies, refer to Fastening Recommendations for Side-Loaded, Multiple-Piece Members in the GP Engineered Lumber Residential Floor & Roof Systems Product Guide.*
10. *Analysis valid for dry-use only (less than 16% moisture content).*
11. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
12. *For explanation of GROUP, change to expanded printout.*