

DATE 1/02/2005

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000023803

APPLICANT MIKE HERLONG PHONE 497-3991
 ADDRESS 337 SCATTERED OAKS CT FORT WHITE FL 32038
 OWNER FORT WHITE DEVELOPERS, LLP PHONE 497-3991
 ADDRESS 226 SW PAISLEY CT FORT WHITE FL 32038
 CONTRACTOR MIKE HERLONG PHONE 497-1116
 LOCATION OF PROPERTY 47 S, L 27, L PAISLEY CT, 3D LOT ON LEFT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 82350.00
 HEATED FLOOR AREA 1647.00 TOTAL AREA 2292.00 HEIGHT 18.00 STORIES 1
 FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
 LAND USE & ZONING FORT WHITE MAX. HEIGHT _____
 Minimum Set Back Requirements: STREET-FRONT _____ REAR _____ SIDE _____
 NO. EX.D.U. 0 FLOOD ZONE FW DEVELOPMENT PERMIT NO. _____

PARCEL ID 34-6S-16-04060-131 SUBDIVISION FORT WHITE PARK
 LOT 31 BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES .50

RB0029433
 Culvert Permit No. FORT WHITE Culvert Waiver 05-1006-N Contractor's License Number BK Applicant/Owner/Contractor JH
 Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident N

COMMENTS: TOWN OF FORT WHITE LETTER RECIEVED
 NOC ON FILE _____

Check # or Cash 1104

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by _____ Foundation _____ date/app. by _____ Monolithic _____ date/app. by _____
 Under slab rough-in plumbing _____ date/app. by _____ Slab _____ date/app. by _____ Sheathing/Nailing _____ date/app. by _____
 Framing _____ date/app. by _____ Rough-in plumbing above slab and below wood floor _____ date/app. by _____
 Electrical rough-in _____ date/app. by _____ Heat & Air Duct _____ date/app. by _____ Peri. beam (Lintel) _____ date/app. by _____
 Permanent power _____ date/app. by _____ C.O. Final _____ date/app. by _____ Culvert _____ date/app. by _____
 M/H tie downs, blocking, electricity and plumbing _____ date/app. by _____ Pool _____ date/app. by _____
 Reconnection _____ date/app. by _____ Pump pole _____ date/app. by _____ Utility Pole _____ date/app. by _____
 M/H Pole _____ date/app. by _____ Travel Trailer _____ date/app. by _____ Re-roof _____ date/app. by _____

BUILDING PERMIT FEE \$ 415.00 CERTIFICATION FEE \$ 11.46 SURCHARGE FEE \$ 11.46
 MISC. FEES \$.00 ZONING CERT. FEE \$ _____ FIRE FEE \$.00 WASTE FEE \$ _____
 FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ _____ CULVERT FEE \$ _____ **TOTAL FEE** 437.92
 INSPECTORS OFFICE [Signature] CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-

For Office Use Only Application # 0510-55 Date Received 10/18/05 By JFW Permit # 23803
Application Approved by - Zoning Official Date Plans Examiner OK JFH Date 10-25-05
Flood Zone Development Permit Zoning Land Use Plan Map Category
Comments Town of Fort White letter Received

Applicants Name Fort White Developers, LLP Phone 497-3991
Address Fort White, Fla. 32038 6855 SW Elm Church Rd.
Owners Name Fort White Developers, LLP Phone 497-3991
911 Address 6326 SW Paisley Pt Fort White, Fla 32038
Contractors Name Mike Herlong Phone 497-1116
Address 337 Scattered Oaks Ct. Fort White, Fla 32038
Fee Simple Owner Name & Address Fort White Developers, LLP.
Bonding Co. Name & Address
Architect/Engineer Name & Address Hery R. Hawkins, 6855 SW Elm Church Rd Fort White, Fla
Mortgage Lenders Name & Address Peoples State Bank, 530 SW Main Blvd. Lake City, Fla
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
Property ID Number 34-65-16-04060-131 Estimated Cost of Construction \$110,000
Subdivision Name Fort White Park Lot 31 Block Unit Phase
Driving Directions one mile east of SR 47 on US 27, left on Paisley ct.
Third lot on left.

Type of Construction Brick Veneer Number of Existing Dwellings on Property 0
Total Acreage 0.5Ac. Lot Size 0.5Ac. Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 70 Side 25 Side 25 Rear 100
Total Building Height 18' Number of Stories 1 Heated Floor Area 1647 Roof Pitch 6/12
Porches 225 GARAGE 420 TOTAL 2292

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

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Owner Builder or Agent (including Contractor)
DALE R. BURD
Commission # DD0134600
Expires 7/16/2008
Bonded through
(800-432-4254) Florida Notary Assn., Inc.
Sworn to (or affirmed) and subscribed before me
his 13 day of Oct 2005
personally known or Produced Identification



Mike Herlong
Contractor Signature
Contractors License Number BB-002943
Competency Card Number
NOTARY STAMP/SEAL
Notary Signature

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

*****THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.*****

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 34-05-16-04060-131

1. Description of property: (legal description of the property and street address or 911 address)

Lot 31, Fort White Park
226 SW Paisley Ct.
Fort White, Fla. 32038

Inst:2005025862 Date:10/18/2005 Time:11:29
YMK DC,P.Dewitt Cason,Columbia County B:1062 P:280

2. General description of improvement: New Home, Brick Veneer, 1647 sq.ft.
Heated & Cooled, 2792 Total Sq.Ft.

3. Owner Name & Address Fort White Developers, LLC
6855 SW Elm Church Rd. Interest in Property 100%

4. Name & Address of Fee Simple Owner (if other than owner): Owner

5. Contractor Name Mike Herlong Phone Number 497-3991
Address 397 Scattered Oaks Fort White, Fla 32038

6. Surety Holders Name _____ Phone Number _____
Address _____
Amount of Bond _____

7. Lender Name Peoples State Bank Phone Number _____
Address 530 SW Main Blvd. Lake City, Fla.

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name Heley R. Hawkins Phone Number 497-3991
Address Fort White, Fla 6855 SW Elm Church Rd.

9. In addition to himself/herself the owner designates Mike Herlong of Fort White, Fla. to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) - (a) 7. Phone Number of the designee 497-1116

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) 1 year

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

[Signature]
Signature of Owner
Fort White Developers, LLC

Sworn to (or affirmed) and subscribed before
18th day of October, 2005
DALE R. BURD
Commission # DD0134800
Expires 7/1/2008
Bonded through
(800-432-4264) Florida Notary Assn., Inc.
[Signature]
Signature of Notary

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Amount of Bond _____

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Address 530 SW Main Blvd. Lake City, Fla.

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 713.13 (1)(a) 7; Florida Statutes:

Name Henry R. Hawkins Phone Number 497-3991
Address Fort White, Fla 6855 SW Elm Church Rd

9. In addition to himself/herself the owner designates Mike Herlong of
Fort White, Fla. to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -
(a) 7. Phone Number of the designee 497-1116

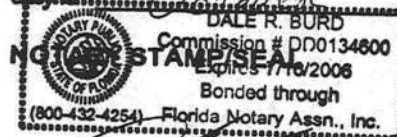
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[Signature]
Signature of Owner
Fort White Developers, LLP

Sworn to (or affirmed) and subscribed before
18th day of October, 2005



[Signature]
Signature of Notary

@ CAM112M01 S CamaUSA Appraisal System
 10/18/2005 12:35 Legal Description Maintenance
 Year T Property * PRIOR YEAR * Sel
 2005 R 34-6S-16-04060-131
 LOT 31 FORT WHITE PARK
 HAWKINS HUEY R & GWENDOLYN P

Columbia County
 10500 Land 001
 AG 000
 Bldg 000
 Xfea 000
 10500 TOTAL B

1	LOT 31 FORT WHITE PARK.	ORB 753-864, 760-997,	2
3	WD 1029-1506.		4
5			6
7			8
9			10
11			12
13			14
15			16
17			18
19			20
21			22
23			24
25			26
27			28

Mnt 11/09/2004 KYLIE

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

0 82 164 246 ft

PARCEL: 34-6S-16-04060-131 - VACANT (000000)

LOT 31 FORT WHITE PARK. ORB 753-864, 760-997, WD 1029-1506.

Name: HAWKINS HUEY R & GWENDOLYN P

Site: FORT WHITE PARK

Mail: 6855 SW ELIM CHURCH RD
FT WHITE, FL 32038

10/11/2004 \$12,500.00 / V

Q

Sales Info 2/11/1992 \$57,100.00 / V

U

11/8/1991 \$60,000.00 / V

U

LandVal	\$10,500.00
BldgVal	\$0.00
ApprVal	\$10,500.00
JustVal	\$10,500.00
Assd	\$10,500.00
Exmpt	\$0.00
Taxable	\$10,500.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County

Town of Fort White

Post Office Box 129 Fort White, Florida 32038-0129
Town Hall - (386) 497-2321 • Public Works - (386) 497-3345
Email: townofftwhite@alltel.com • Web site: Townoffortwhitefl.com

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes:

OWNER'S NAME: Fort White Developers, LLP

ADDRESS: 6855 SW Elim Church Rd. Fort White, FL 32038

PROPERTY DESCRIPTION: Lot #31 Fort White Park
(parcel number if possible) Parcel #4060-131

DEVELOPMENT: Single Family Dwelling

You are hereby authorized to issue the appropriate building permits.

10-17-2005

DATE


LAND DEVELOPMENT REGULATION
ADMINISTRATOR
TOWN OF FORT WHITE

District #1
Donald Cook
497-1086

District #2
Henry Maini
497-2992

District #3
John Gloskowski
497-3999

District #4
Demetric Jackson
497-2078

Mayor
Truett George
497-4741

REVISED

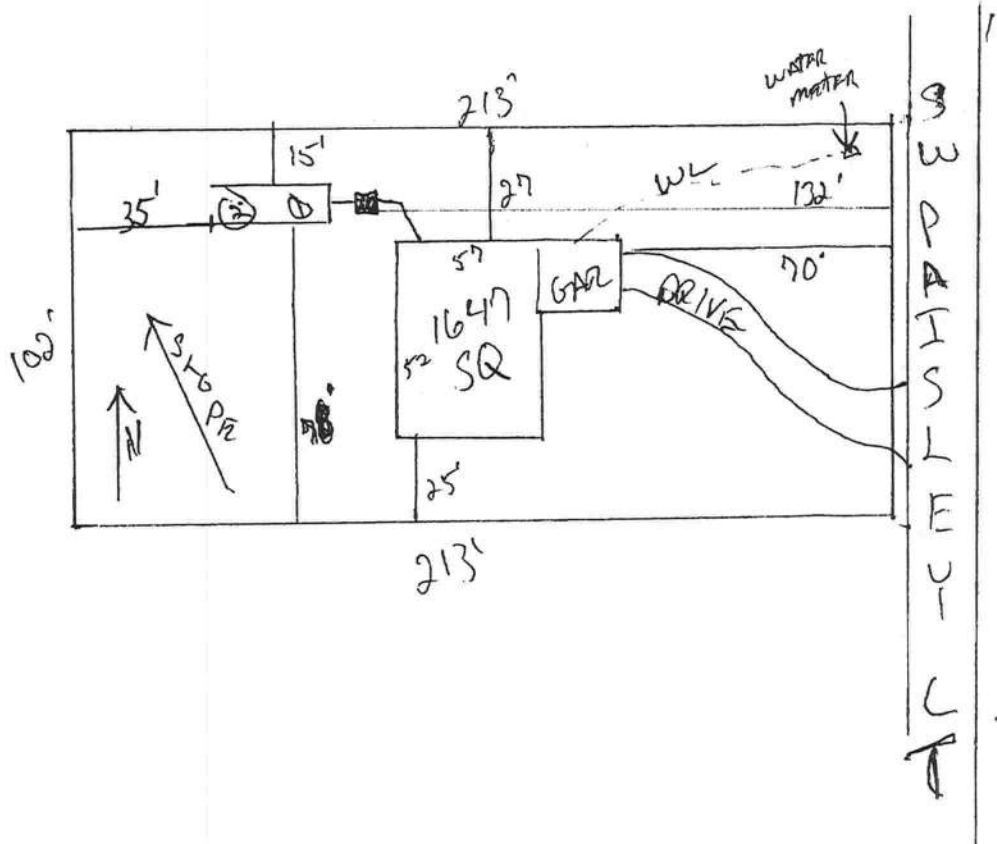
**STATE OF FLORIDA
DEPARTMENT OF HEALTH**

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1006-N

----- PART II - SITEPLAN -----

Scale: 1 inch = 50 feet.



Notes: _____

Site Plan submitted by: *Rock D F* **MASTER CONTRACTOR**
 Plan Approved _____ Not Approved _____ Date _____
 By _____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: Spec House - Lot 31 Address: Lot: 31, Sub: Ft White Park, Plat: 6 page 3 & 3A City, State: Fort White, FL 32046- Owner: Huey Hawkins Climate Zone: North	Builder: Permitting Office: Columbia Co Permit Number: Jurisdiction Number: 221000
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<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 50%;">1. New construction or existing</td> <td style="width: 10%; text-align: center;">New</td> <td style="width: 5%; text-align: center;">___</td> </tr> <tr> <td>2. Single family or multi-family</td> <td style="text-align: center;">Single family</td> <td style="text-align: center;">___</td> </tr> <tr> <td>3. Number of units, if multi-family</td> <td style="text-align: center;">1</td> <td style="text-align: center;">___</td> </tr> <tr> <td>4. Number of Bedrooms</td> <td style="text-align: center;">3</td> <td style="text-align: center;">___</td> </tr> <tr> <td>5. Is this a worst case?</td> <td style="text-align: center;">No</td> <td style="text-align: center;">___</td> </tr> <tr> <td>6. Conditioned floor area (ft²)</td> <td style="text-align: center;">1647 ft²</td> <td style="text-align: center;">___</td> </tr> <tr> <td>7. Glass area & type</td> <td style="text-align: center;">Single Pane Double Pane</td> <td style="text-align: center;">___</td> </tr> <tr> <td> a. Clear glass, default U-factor</td> <td style="text-align: center;">0.0 ft² 245.0 ft²</td> <td style="text-align: center;">___</td> </tr> <tr> <td> b. Default tint</td> <td style="text-align: center;">0.0 ft² 0.0 ft²</td> <td style="text-align: center;">___</td> </tr> <tr> <td> c. Labeled U or SHGC</td> <td style="text-align: center;">0.0 ft² 0.0 ft²</td> <td style="text-align: center;">___</td> </tr> <tr> <td>8. Floor types</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> a. Slab-On-Grade Edge Insulation</td> <td style="text-align: center;">R=0.0, 195.0(p) ft</td> <td style="text-align: center;">___</td> </tr> <tr> <td> b. N/A</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> c. N/A</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td>9. 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Ducts</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> a. Sup: Unc. Ret: Unc. AH: Interior</td> <td style="text-align: center;">Sup. R=6.0, 52.0 ft</td> <td style="text-align: center;">___</td> </tr> <tr> <td> b. N/A</td> <td></td> <td style="text-align: center;">___</td> </tr> </table>	1. New construction or existing	New	___	2. Single family or multi-family	Single family	___	3. Number of units, if multi-family	1	___	4. Number of Bedrooms	3	___	5. Is this a worst case?	No	___	6. Conditioned floor area (ft ²)	1647 ft ²	___	7. Glass area & type	Single Pane Double Pane	___	a. Clear glass, default U-factor	0.0 ft ² 245.0 ft ²	___	b. Default tint	0.0 ft ² 0.0 ft ²	___	c. Labeled U or SHGC	0.0 ft ² 0.0 ft ²	___	8. Floor types		___	a. Slab-On-Grade Edge Insulation	R=0.0, 195.0(p) ft	___	b. N/A		___	c. N/A		___	9. Wall types		___	a. Frame, Wood, Exterior	R=13.0, 1231.0 ft ²	___	b. N/A		___	c. N/A		___	d. N/A		___	e. N/A		___	10. Ceiling types		___	a. 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Electric Heat Pump</td> <td style="text-align: center;">Cap: 35.0 kBtu/hr</td> <td style="text-align: center;">___</td> </tr> <tr> <td></td> <td style="text-align: center;">HSPF: 7.90</td> <td style="text-align: center;">___</td> </tr> <tr> <td> b. N/A</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> c. N/A</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td>14. Hot water systems</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> a. Electric Resistance</td> <td style="text-align: center;">Cap: 30.0 gallons</td> <td style="text-align: center;">___</td> </tr> <tr> <td></td> <td style="text-align: center;">EF: 0.90</td> <td style="text-align: center;">___</td> </tr> <tr> <td> b. N/A</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td>15. HVAC credits</td> <td></td> <td style="text-align: center;">___</td> </tr> <tr> <td> (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)</td> <td style="text-align: center;">PT, CF,</td> <td style="text-align: center;">___</td> </tr> </table>	12. Cooling systems			a. Central Unit	Cap: 35.0 kBtu/hr	___		SEER: 10.00	___	b. N/A		___	c. N/A		___	13. Heating systems		___	a. Electric Heat Pump	Cap: 35.0 kBtu/hr	___		HSPF: 7.90	___	b. N/A		___	c. N/A		___	14. Hot water systems		___	a. Electric Resistance	Cap: 30.0 gallons	___		EF: 0.90	___	b. N/A		___	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		___	15. HVAC credits		___	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	___
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	SEER: 10.00	___																																																																																																																																			
b. N/A		___																																																																																																																																			
c. N/A		___																																																																																																																																			
13. Heating systems		___																																																																																																																																			
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	___																																																																																																																																			
	HSPF: 7.90	___																																																																																																																																			
b. N/A		___																																																																																																																																			
c. N/A		___																																																																																																																																			
14. Hot water systems		___																																																																																																																																			
a. Electric Resistance	Cap: 30.0 gallons	___																																																																																																																																			
	EF: 0.90	___																																																																																																																																			
b. N/A		___																																																																																																																																			
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		___																																																																																																																																			
15. HVAC credits		___																																																																																																																																			
(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	___																																																																																																																																			

Glass/Floor Area: 0.15	Total as-built points: 23037 Total base points: 25624	PASS
------------------------	--	------

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene


DATE: 10/24/05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 31, Sub: Ft White Park, Plat: 6 page 3 & 3A, Fort White, FL, 32045 PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area	X	BSPM = Points							
	1647.0		10.21			16815.9				
Summer Base Points: 20918.6				Summer As-Built Points: 20693.8						
Total Summer Points	X	System Multiplier	= Cooling Points	Total Component	X	Cap Ratio	X Duct Multiplier	X System Multiplier	X Credit Multiplier	= Cooling Points
						(DM x DSM x AHU)				
20918.6		0.4266	8923.9	20693.8		1.000	(1.090 x 1.147 x 0.91)	0.341	0.902	7252.0
				20693.8		1.00	1.138	0.341	0.902	7252.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 31, Sub: Ft White Park, Plat: 6 page 3 & 3A, Fort White, FL, 32046 PERMIT #:

BASE				AS-BUILT											
INFILTRATION	Area	X	BWPM = Points												
	1647.0		-0.59	-971.7											
Winter Base Points: 13488.1				Winter As-Built Points: 16220.7											
Total Winter	X	System	=	Heating	Total	X	Cap	X	Duct	X	System	X	Credit	=	Heating
Points		Multiplier		Points	Component		Ratio		Multiplier		Multiplier		Multiplier		Points
					(DM x DSM x AHU)										
13488.1		0.6274		8462.4	16220.7		1.000		(1.069 x 1.169 x 0.93)		0.432		0.950		7730.3
					16220.7		1.00		1.162		0.432		0.950		7730.3

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 31, Sub: Ft White Park, Plat: 6 page 3 & 3A, Fort White, FL, 32046 PERMIT #:

BASE				AS-BUILT								
WATER HEATING												
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X	Credit = Total Multiplier		
3		2746.00	8238.0	30.0	0.90	3		1.00	2684.98	1.00	8054.9	
											As-Built Total:	8054.9

CODE COMPLIANCE STATUS											
BASE				AS-BUILT							
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
8924		8462		8238	25624	7252		7730		8055	23037

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 31, Sub: Ft White Park, Plat: 6 page 3 & 3A, Fort White, FL, 32046 PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

0510-55

Reference to: Build permit application Number:

Mike Herlong Owners Fort White Developers for Lot 31 Fort White Park

On the date of October 21, 2005 application 0510-55 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0510-55 when making reference to this application.

- ✓ 1. Application 0510-55 which was filed with the building department on the date of October 18, 2005 will be reviewed under the Florida Building Code 2004. The Wind Load design by Mr. Huey Hawkins was design under the Florida Building Code 2001. The wind Load design should reflect the code sections of the Florida Building Code 2004 that relate to wind Load design code requirements.

- ✓ 2. The area summary condition living area (page A-1) states 1647 square feet, the Florida energy efficiency code for building construction form 600-A-2001 line six (condition floor area sq. ft.) 1638 square feet. Please correct this discrepancy on the Florida energy efficiency code for building construction form 600-A-2001 resubmit this form 600-A-2001.

✓ 3. Please verify if the window over the tub area in bathroom two will comply with R308.4 Hazardous locations of the Florida Residential Code 2004. Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

October 24, 2005

Columbia County Building Department
"Hand Delivered"

Subject: Fort White Developers, LLP / Mike Herlong
Lot 31, Fort White Park
Fort White, Florida

Dear Mr. Haltiwanger:

Please accept this letter as an addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004. The plan was drawn prior to the effective date of FBC 2004, October 1, 2005. Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.

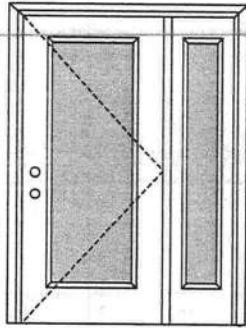
Thanks

A handwritten signature in blue ink, appearing to read "Huey R. Hawkins", with the date "10-24-05" written below it.

Huey R. Hawkins, PE
Florida P.E. No 33665

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etsemko.com), the Masonite website (www.masonite.com) or the Masonite Technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 1 Sidelite
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0003-02 or MAD-WL-MA0006-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0003-02.

APPROVED DOOR STYLES:

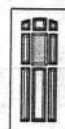
1/4 GLASS:



100 Series



133, 135 Series



136 Series

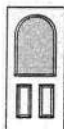


680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

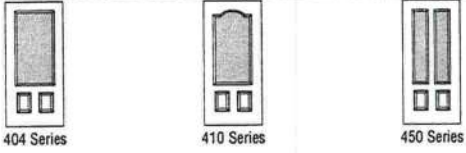
Johnson
EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

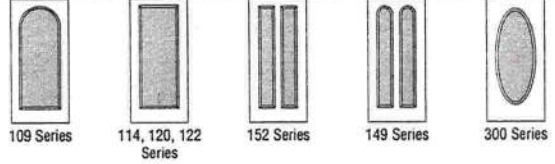


WOOD-EDGE STEEL DOORS

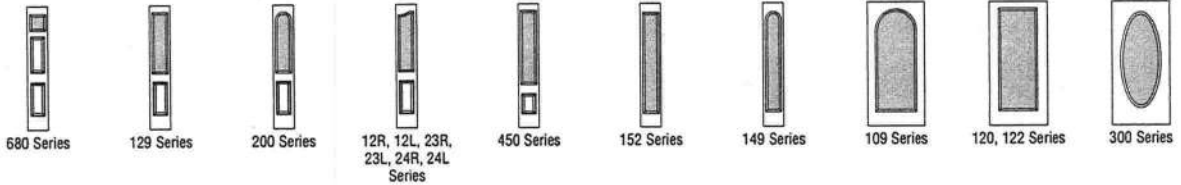
**APPROVED DOOR STYLES:
3/4 GLASS:**



FULL GLASS:



APPROVED SIDELITE STYLES:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthaz

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etssemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

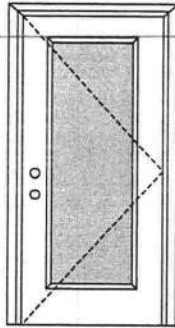


June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etssemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™
EntrySystems

June 17, 2002
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Exclusively from
Masonite®
Masonite International Corporation

X

Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



404 Series



410 Series



450 Series

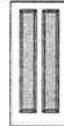
FULL GLASS:



109 Series



114, 120, 122 Series



152 Series



149 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

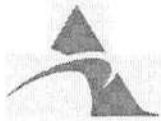
2

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June 17, 2002
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Exclusively from
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Masonite International Corporation



**AAMA/NWDA 101/I.S.2-97
TEST REPORT**

Rendered to:

MI HOME PRODUCTS, INC.

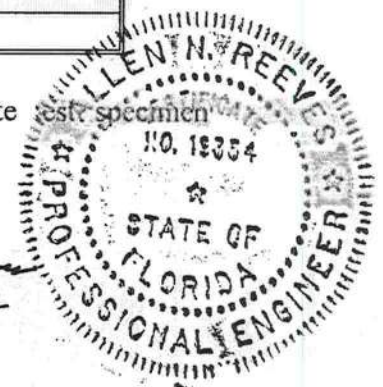
SERIES/MODEL: 650

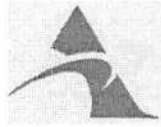
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft ²
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen H. Reeves
7 JUNE 2002





Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com





Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

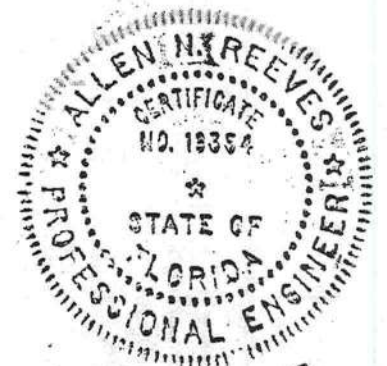
Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

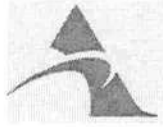
Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



Allen N. Reeves
7 JUNE 2002



Test Specimen Description: (Continued)

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft ²	0.3 cfm/ft ² max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)
(with and without screen)
WTP = 2.86 psf

No leakage



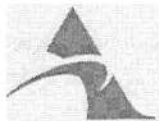


Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max. 0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs Right sash, meeting rail Right sash, bottom rail Middle sash, meeting rail Middle sash, bottom rail Left sash, meeting rail Left sash, bottom rail In remaining direction at 50 lbs Right sash, right stile Right sash, left stile Middle sash, right stile Middle sash, left stile Left sash, right stile Left sash, left stile	0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97) Type: A Grade: 10 Lock Manipulation Test Test A1 through A5 Test A7 Lock Manipulation Test	No entry No entry No entry No entry	No entry No entry No entry No entry

Allen N. Reeves
7 JUNE 2002






Test Results: (Continued)

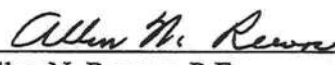
<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	<i>*Exceeds L/175 for deflection, but meets all other test requirements.</i>		
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

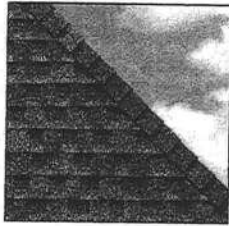
For ARCHITECTURAL TESTING, INC.


Mark A. Hess
Technician

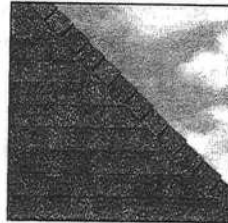
MAH:nlb
01-41641.01


Allen N. Reeves, P.E.
Director - Engineering Services
7 JUNE 2002





**PRESTIQUE®
HIGH DEFINITION®**



RAISED PROFILE®

**Prestique Plus High Definition
and Prestique Gallery Collection****

Product size _____ 13 1/4" x 39 1/4"	50-year limited warranty period:
Exposure _____ 5 1/2"	5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph***
Pieces/Bundle _____ 16	
Bundles/Square _____ 4/98.5 sq.ft.	
Squares/Pallet _____ 11	

Raised Profile

Product size _____ 13 1/4" x 38 1/4"	30-year limited warranty period:
Exposure _____ 5 1/2"	5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.
Pieces/Bundle _____ 22	
Bundles/Square _____ 3/100 sq.ft.	
Squares/Pallet _____ 16	

Prestique I High Definition

Product size _____ 13 1/4" x 39 1/4"	40-year limited warranty period:
Exposure _____ 5 1/2"	5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***
Pieces/Bundle _____ 16	
Bundles/Square _____ 4/98.5 sq.ft.	
Squares/Pallet _____ 14	

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/FLX™	Vented RidgeCrest™ w/FLX™
Size: 12" x 12"	Size: 13" x 13 1/4"
Exposure: 6 1/2"	Exposure: 9 1/4"
Pieces/Bundle: 45	Pieces/Box: 26
Coverage: 4 Bundles = 100 linear feet	Coverage: 5 boxes = 100 linear feet

Prestique High Definition

Product size _____ 13 1/4" x 38 1/4"	30-year limited warranty period:
Exposure _____ 5 1/2"	5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.
Pieces/Bundle _____ 22	
Bundles/Square _____ 3/100 sq.ft.	
Squares/Pallet _____ 16	

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakedown, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood, Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard™ treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

*See actual limited warranty for conditions and limitations.

** Effective January 1, 2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.

***For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

PREPARATION OF ROOF DECK: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low slopes [4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

**SOUTHEAST &
ATLANTIC OFFICE:**
800.945.5551

CORPORATE HEADQUARTERS:
800.354.7732

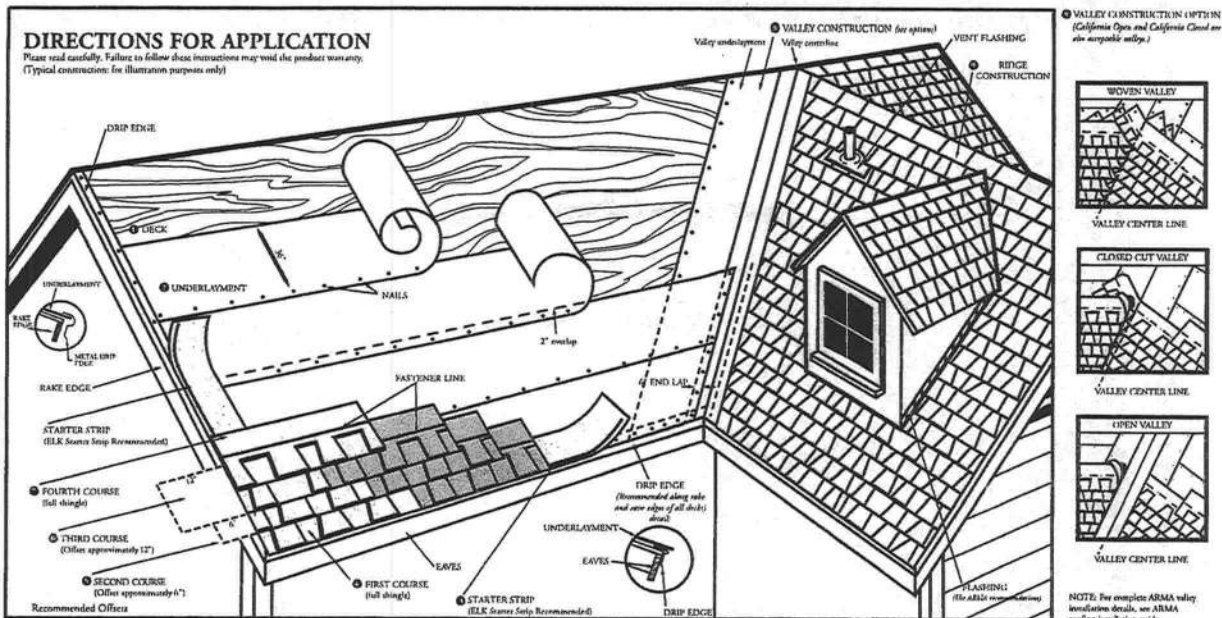
PLANT LOCATION:
800.945.5545



SS00T 06/04

DIRECTIONS FOR APPLICATION

Please read carefully. Failure to follow these instructions may void the product warranty. (Typical constructions for illustration purposes only)



DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

1 DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

2 UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self-adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 15". Begin by fastening a 15" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

3 STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

4 FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

5 SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

6 THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

7 FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

8 VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

9 RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" 2"Ridge or Seal-A-Ridge® with formula FLX™ or RidgeCrest™ with FLX (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-roofs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

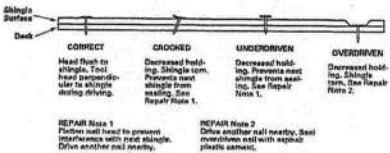
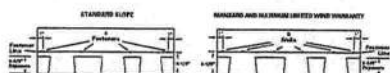
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 80 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4" of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



REPAIR Note 1: Fastener well back to prevent interference with next shingle. Drive another nail nearby.

REPAIR Note 2: Drive another nail nearby. Seal penetration with repair plastic cement.

Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified. All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.



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Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 567

Florida Certificate of Product Approval # FL1999

Page 1 of 1 Document ID:1SR1487-Z0305145238

Truss Fabricator: Anderson Truss Company
Job Identification: 5-432-FORT WHITE DEVELOPERS/SPEC LOT 31
Truss Count: 33
Model Code: Florida Building Code
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.04.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-98 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11015EC-GBLLETIN

Seal Date: 10/05/2005

-Truss Design Engineer-
Harry Goodson

Florida License Number: 26454

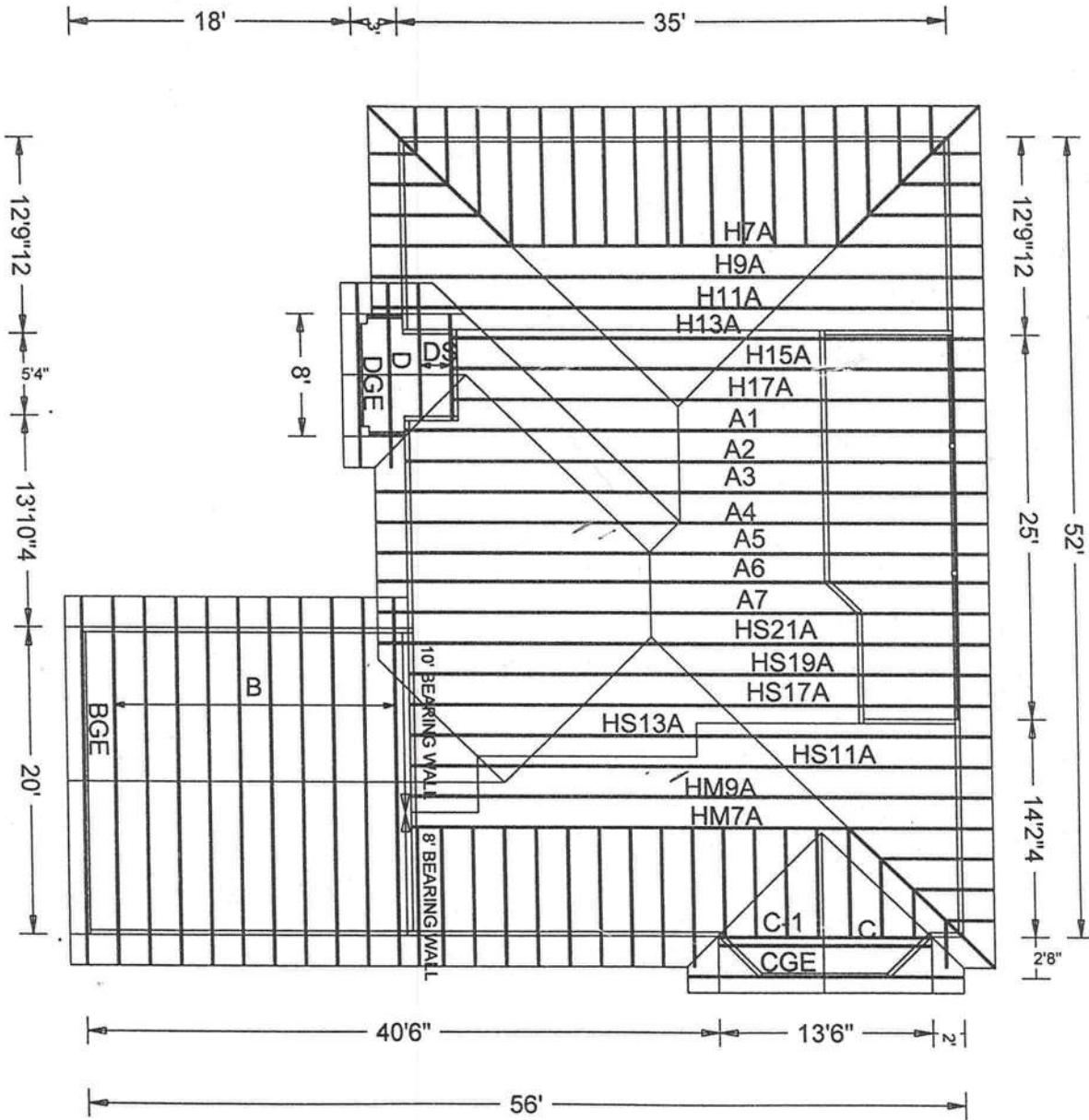
1950 Marley Drive

Haines City, FL 33844

#	Ref	Description	Drawing#	Date
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2	00328--	H7A	05278020	10/05/05
3	00329--	H9A	05278012	10/05/05
4	00330--	H11A	05278013	10/05/05
5	00331--	HM9A	05278014	10/05/05
6	00332--	HS11A	05278015	10/05/05
7	00333--	H13A	05278022	10/05/05
8	00334--	H15A	05278025	10/05/05
9	00335--	H17A	05278028	10/05/05
10	00336--	A1	05278029	10/05/05
11	00337--	A2	05278030	10/05/05
12	00338--	A3	05278031	10/05/05
13	00339--	A4	05278033	10/05/05
14	00340--	A6	05278034	10/05/05
15	00341--	A5	05278036	10/05/05
16	00342--	A7	05278038	10/05/05
17	00343--	HS13A	05278016	10/05/05
18	00344--	HS19A	05278040	10/05/05
19	00345--	HS21A	05278041	10/05/05
20	00346--	HS17A	05278042	10/05/05
21	00347--	BGE	05278043	10/05/05
22	00348--	B	05278017	10/05/05
23	00349--	C-1	05278045	10/05/05
24	00350--	CGE	05278046	10/05/05
25	00351--	C	05278047	10/05/05
26	00352--	DGE	05278048	10/05/05
27	00353--	D	05278018	10/05/05
28	00354--	DS	05278049	10/05/05
29	00355--	CJ1	05278050	10/05/05
30	00356--	HJ7	05278051	10/05/05
31	00357--	CJ3	05278052	10/05/05
32	00358--	CJ5	05278054	10/05/05
33	00359--	EJ7	05278055	10/05/05



FORT WHITE DEVELOPERS/LOT 31 SPEC



Roof Plane Sheathing Area = 3060 sq. ft
 Fascia Material = 256 linear ft
 Valley Flashing Material = 100 linear ft
 Ridge Cap Material = 59 linear ft
 Hip Ridge Material = 125 linear ft

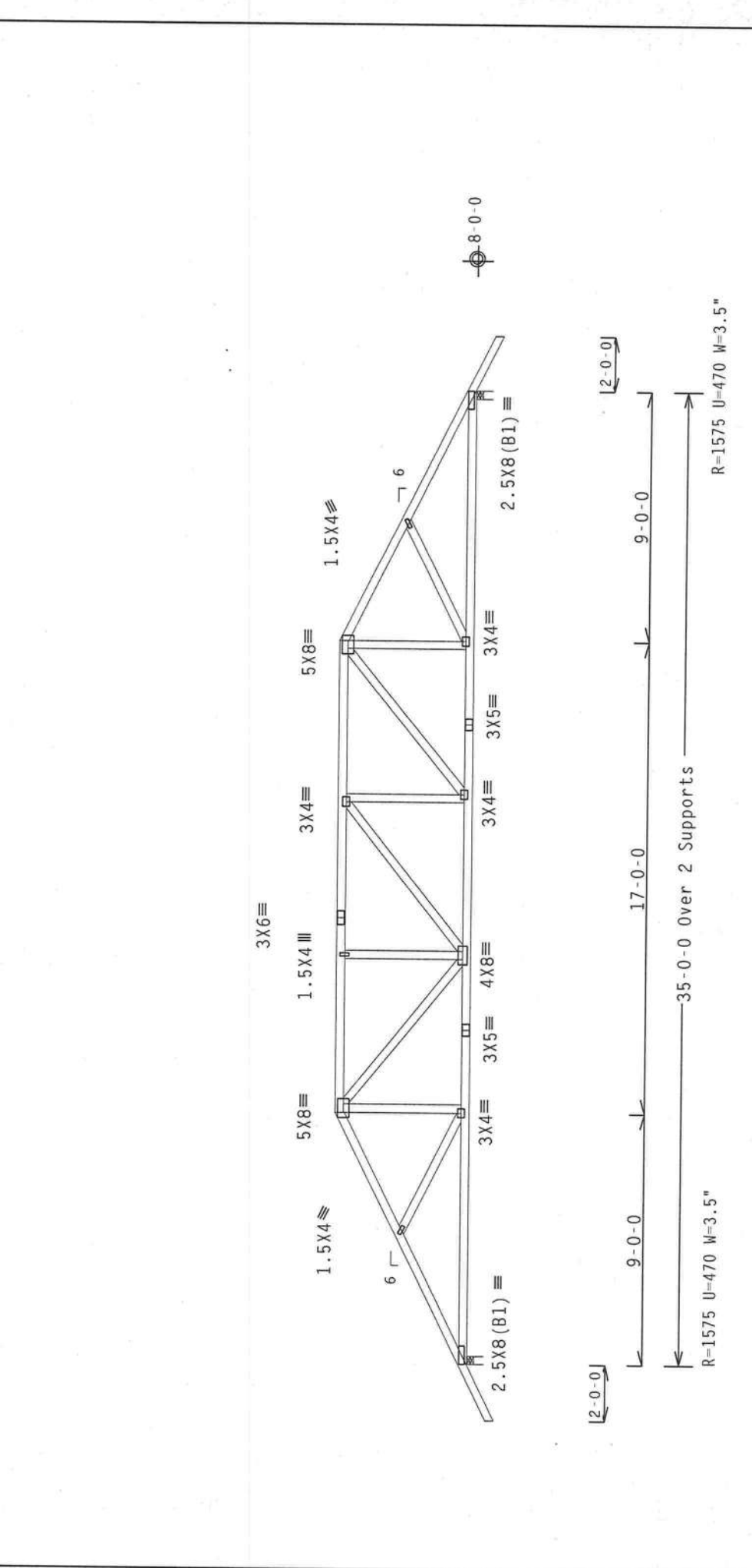
961-1727

JOB NO: 5-432 PAGE NO: 1 OF 1	DESIGNED BY:	JOB DESCRIPTION: FORT WHITE DEVELOPERS/SPEC LO	JOB LOCATION:
--	--------------	---	---------------

110 mph wind, 10.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge. CAT II, Exp B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave
 Design Crit: TPI-2002 (STD) /FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805 QTY:1 FL/-/4/-/R/- Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487 -- 329
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278012
BC LL	0.0 PSF	HC-ENG	SL/HBG3 *
TOT.LD.	40.0 PSF	SEQN-	125209
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03



Oct 05 '05

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 1000 W. MADISON, WI 53719 AND WIGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

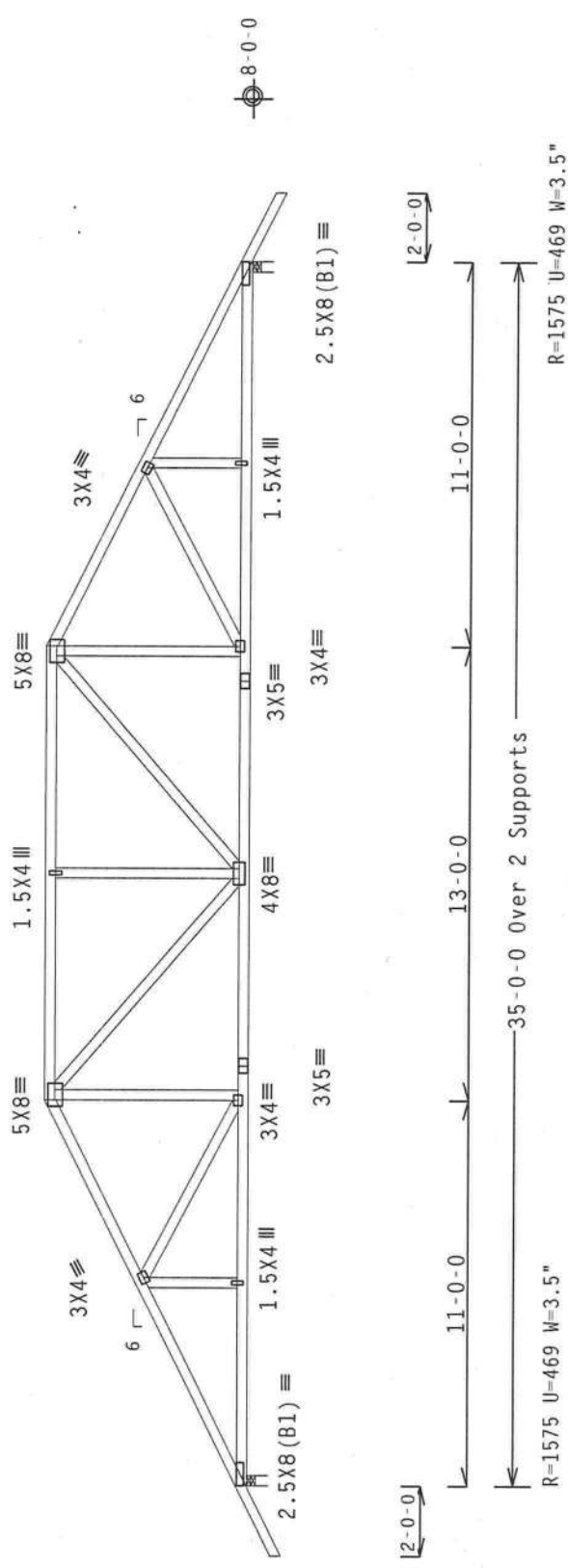
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGNER SHALL BE RESPONSIBLE. PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE TO THE TPI/AF&PA/ASTM A653 GRADE 40/60 (4, K/H-S) GALV. STEEL. ALPINE PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE SPECIFIED, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE OR (2) SHALL BE PER TPI. SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE PROFESSIONAL BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.
 1950 Nanley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567

110 mph wind, 10.60 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.04.0805

QTY: 1

Scale = .1875" / Ft.
REF R487 -- 330
DATE 10/05/05
DRW HCUSR487 05278013
HC-ENG SL/HBG3
SEQN- 125215
DUR.FAC. 1.25
SPACING 24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS MANUFACTURING AND BRACING. THIS DESIGN IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/T1606 (M.M./S/P) ASTM A653 GRADE 40/60 (4. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. BRACING OF TRUSSES SHALL BE PER AISC 308 OF TPI-2002 SEC. 3. A SEAL ON THIS DESIGN SHOWS THE DESIGNER'S RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT TYP. Wave

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

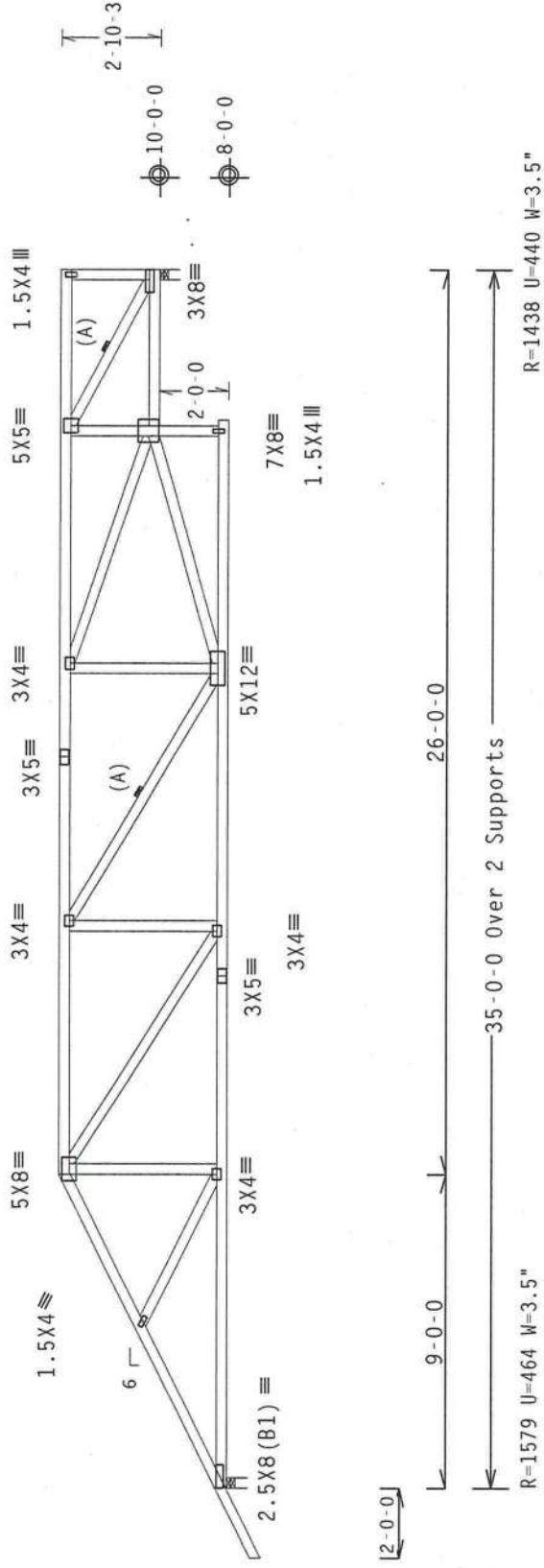
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 10.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2'-0-0

9'-0-0

26'-0-0

35'-0-0 Over 2 Supports

R=1579 U=464 W=3.5"

R=1438 U=440 W=3.5"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0805

QTY:1

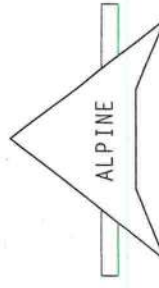
FL/-/4/-/R/-

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDORIO DR., SUITE 200, MADISON, WI 53719), AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR AN INSTALLATION FROM THIS DESIGN. ANY FACTORS OF SAFETY OR DESIGN IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY A789A) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M-H/S/K) ASTM A653 GRADE 40/60 (M, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT TYP. Wave



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Oct 05 '05

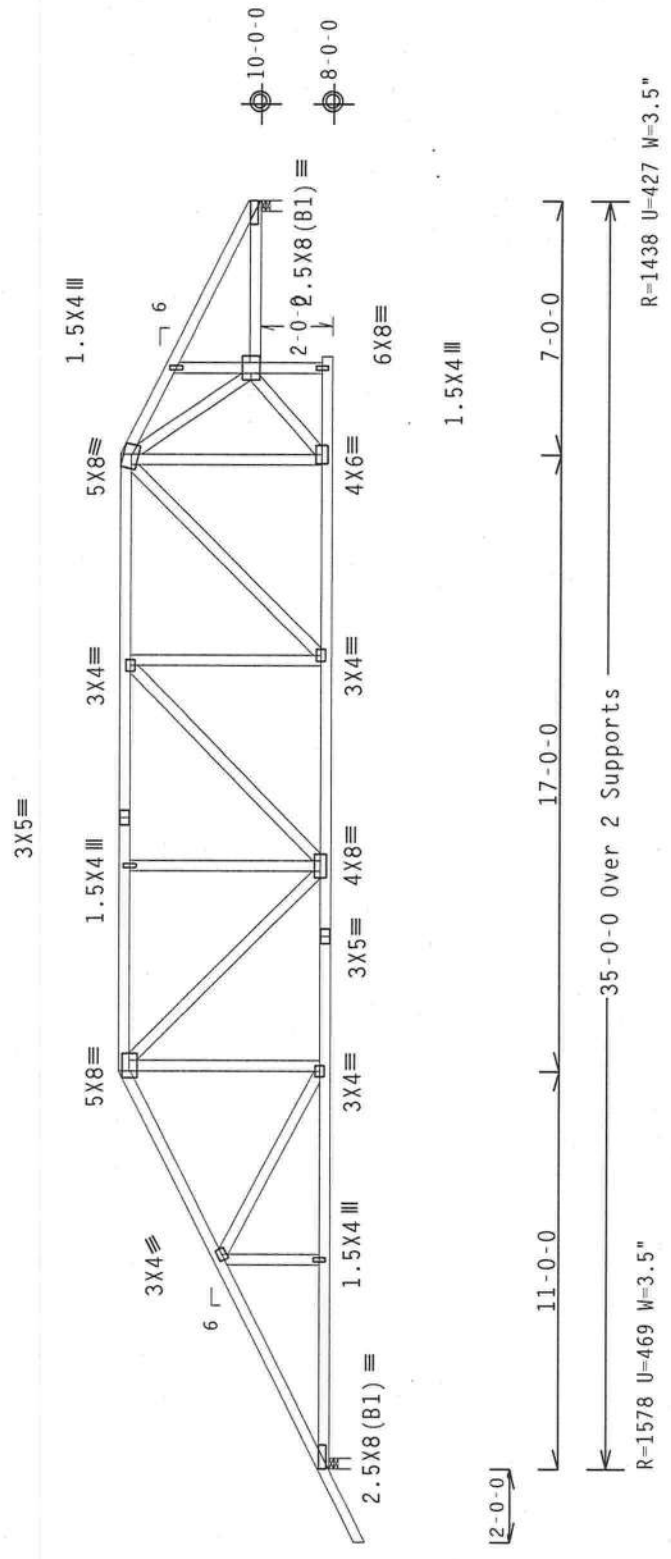
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24'-0"

REF	R487--331
DATE	10/05/05
DRW	HCUSR487 05278014
HC-ENG	SL/HBG3
SEQN-	125231
JREF-	1SR1487_Z03

110 mph wind, 10.60 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave
Design Crit: TPI-2002 (STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.04.0805 QTY:1 FL/-/4/-/ -/R/- Scale = .1875" / Ft.

TC LL	20.0 PSF	REF	R487 - - 332
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278015
BC LL	0.0 PSF	HC-ENG	SL/HBG3 *
TOT.LD.	40.0 PSF	SEON-	125253
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

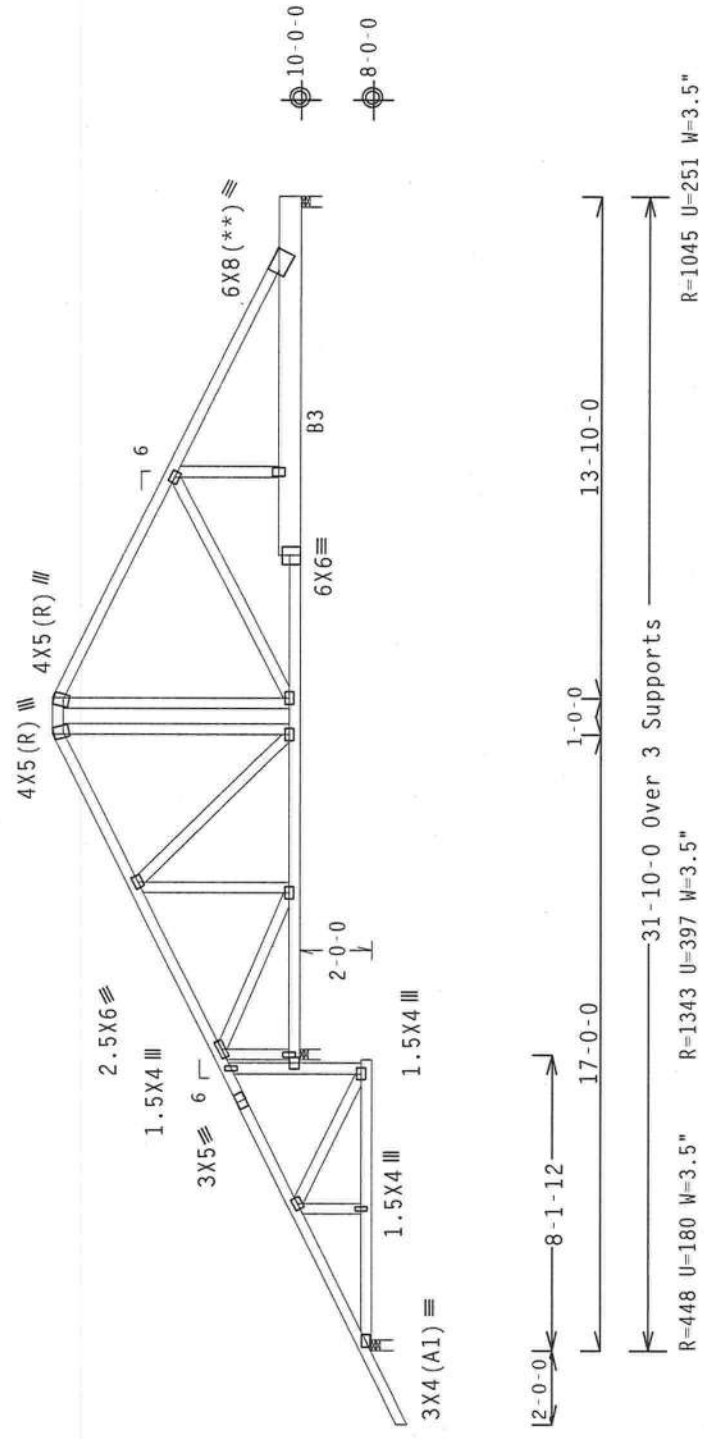
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (H/R/S) ASTM A653 GRADE 40/50 (H, K/M-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ALL TRUSS COMPONENTS SHALL BE PERMANENTLY IDENTIFIED BY A SEAL ON THIS DESIGN. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEER'S DESIGN FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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110 mph wind, 12.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense :B3 2x8 SP SS:
 Webs 2x4 SP #3
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3x4 Except As Shown.
 Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04-.0805

R=448 U=180 W=3.5" R=1343 U=397 W=3.5" R=1045 U=251 W=3.5"

PLT TYP. Wave

TC LL	20.0 PSF	FL/-/4/-/-/R/-	QTY:1	Scale = .1875" / Ft.
TC DL	10.0 PSF	REF R487-- 335		
BC DL	10.0 PSF	DATE 10/05/05		
BC LL	0.0 PSF	DRW HCUSR487 05278028		
TOT.LD.	40.0 PSF	HC-ENG SL/HBG3		
DUR.FAC.	1.25	SEQN- 158705		
SPACING	24.0"	JREF- 1SR1487_Z03		



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE DESIGNER SHALL BE RESPONSIBLE FOR THE PROPER DESIGN OF THE TRUSS AND THE PROPER CONNECTIONS TO THE CEILING. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FULL WEIGHT OF THE CEILING AND THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FULL WEIGHT OF THE CEILING. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FULL WEIGHT OF THE CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M/M/S/K) ASTM A653 GRADE 40/60 (M. K/M-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN SHOWS ACCEPTANCE BY THE DESIGNER. THE DESIGNER SHALL BE RESPONSIBLE FOR THE PROPER COMPONENT BUILDING DESIGNER PER ANS17/PP1 SEC. 2.

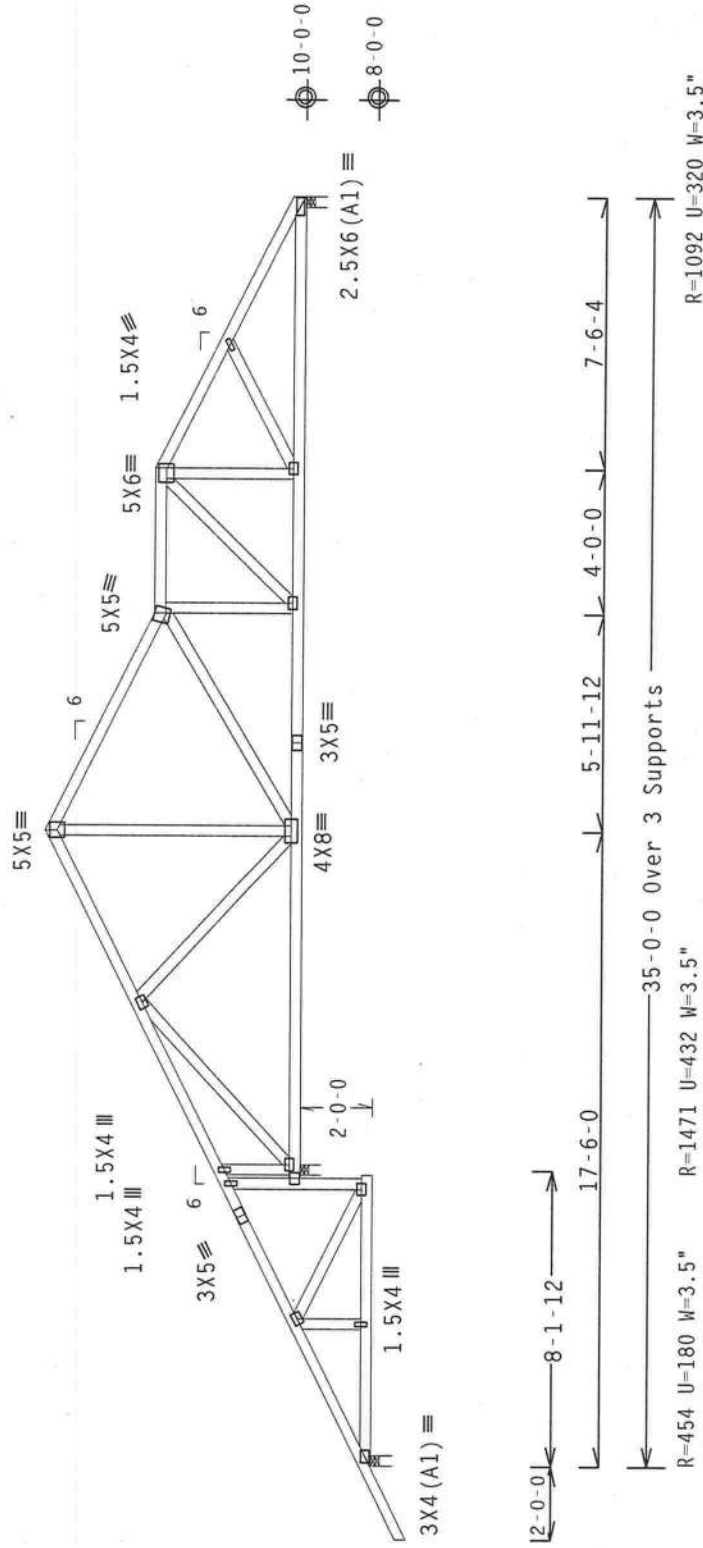
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 1950 Marley Drive
 Haines City, FL 33844
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Oct 05 '05

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 12.22 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

QTY: 1

FL/-4/-/-R/-

Scale = .1875" / Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC61 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 D'ARNOFF DR., SUITE 200, MADISON, WI 53719) AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, WOODBRIDGE, VA 22191) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** *TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN PROVISIONS OF NOS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/S/K) ASTM A653 GRADE 40/60 (W, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-7. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.

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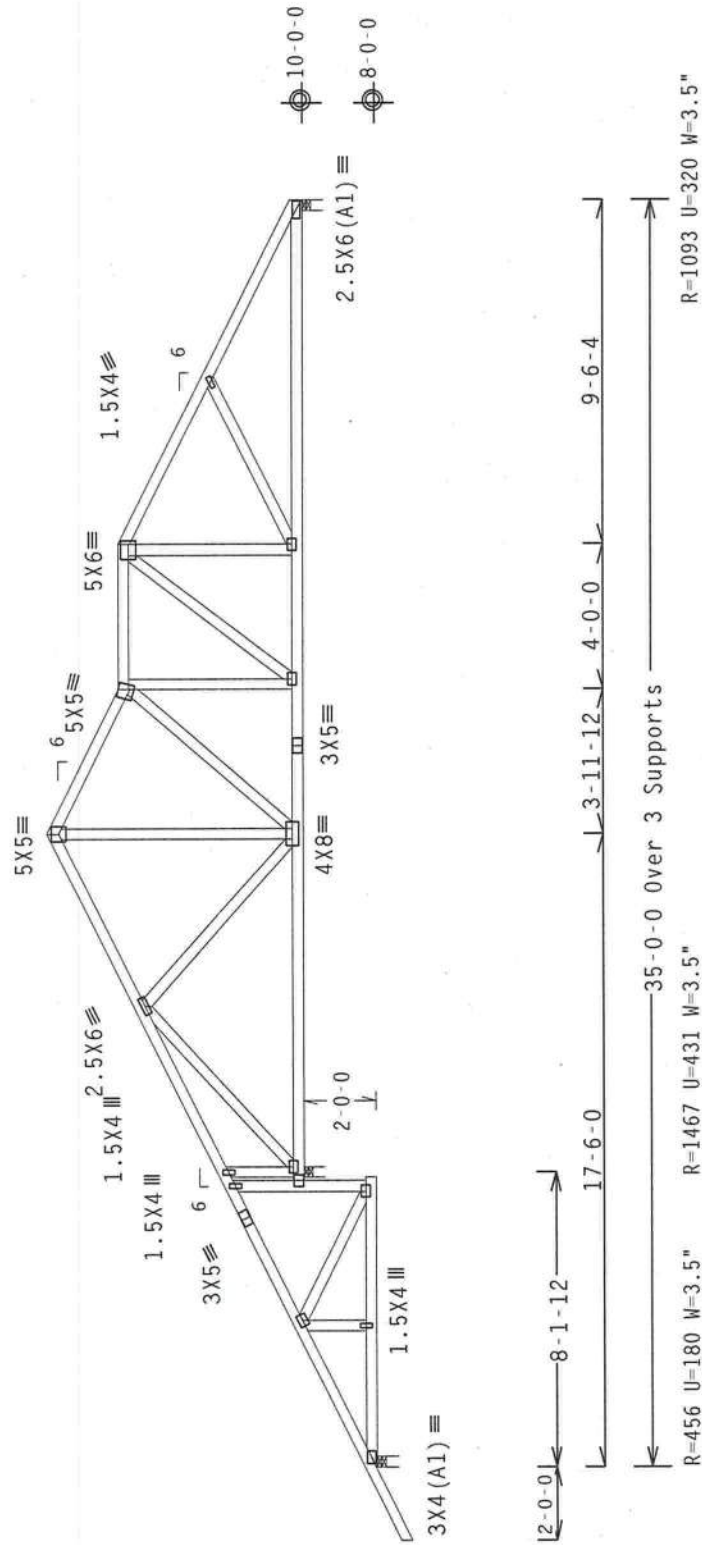
Oct 05 '05

JREF - 1SR1487 Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 12.22 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.



35-0-0 Over 3 Supports

R=456 U=180 W=3.5

R=1093 U=320 W=3.5

Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.04.0805

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 337
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278030
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	158787
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719), AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6000 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS ASSUMES NO RESPONSIBILITY FOR THE QUALITY OF THE INSTALLATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (40/30/20) ASH A653 GRADE 40/60 (40/30/20) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.

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Haines City, FL 33844

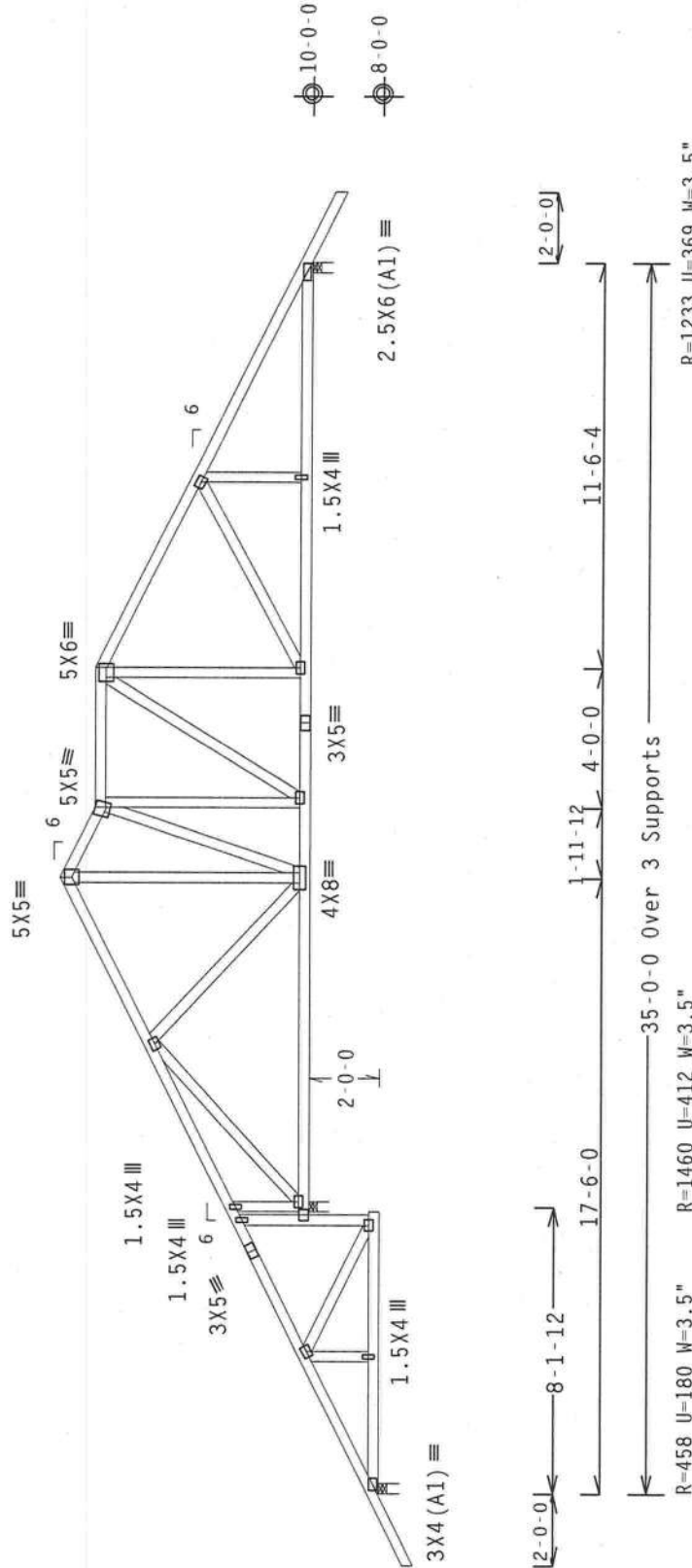
FL Certificate of Authorization # 567

Oct 05 '05

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 12.22 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave	QTY: 1	FL/-/4/-/1-/R/-	Scale = .1875"/Ft.
TC LL	20.0	PSF	REF R487 -- 338
TC DL	10.0	PSF	DATE 10/05/05
BC DL	10.0	PSF	DRW HCUSR487 05278031
BC LL	0.0	PSF	HC-ENG SL/HBG3
TOT.LD.	40.0	PSF	SEON- 158781
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1SR1487 703



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NFCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

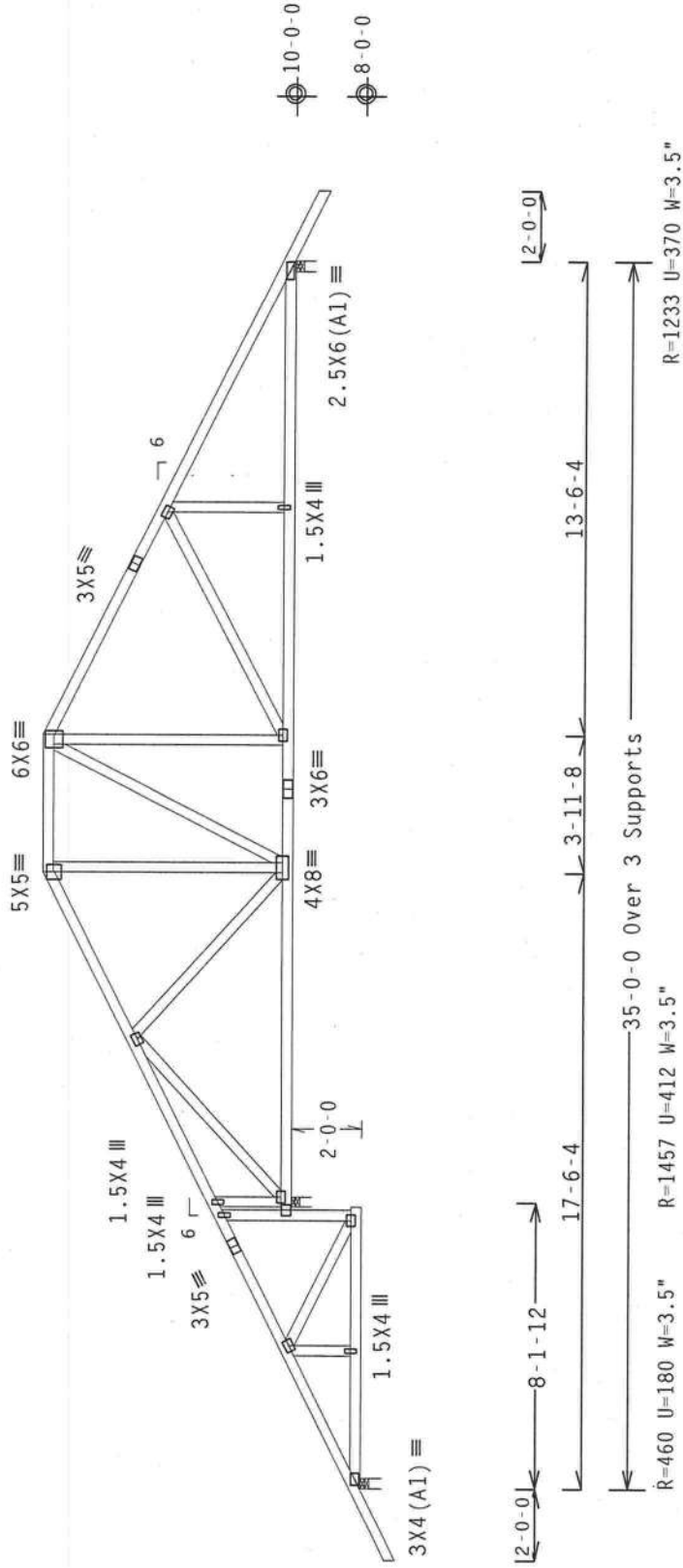
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE DESIGNER'S LIABILITY IS LIMITED TO THE DESIGN. DESIGN CONFORMS WITH THE FOLLOWING PROVISIONS: (1) ALL TRUSSES SHALL BE FABRICATED AND SHIPPED TO THE JOB SITE BY AIR OR TRUCK. (2) ALL TRUSSES SHALL BE MADE OF 2018/21664 (M U/S) ASTM A575 GRADE 60, 60,000 PSI YIELD STRENGTH ALPINE CONNECTOR PLATES ARE MADE OF 2018/21664 (M U/S) ASTM A575 GRADE 60, 60,000 PSI YIELD STRENGTH ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 12.23 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002 (STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0805

QTY: 1 FL/-/4/-/R/-

Scale = .1875" / Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC61 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGNER SHALL BE RESPONSIBLE FOR PROVIDING ALL NECESSARY CONNECTIONS AND BRACING. ALL TRUSS MEMBER CONNECTION PLATES ARE MADE OF 304/18/1/8 STEEL UNLESS OTHERWISE INDICATED. ALL STEEL SHALL BE GALVALUM APPLIED TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER A SEAL OR THIS DESIGN SHALL BE PER ANCH A3 OF TPI-2002 SEC. 3. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANCH A3 OF TPI-2002 SEC. 3. BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

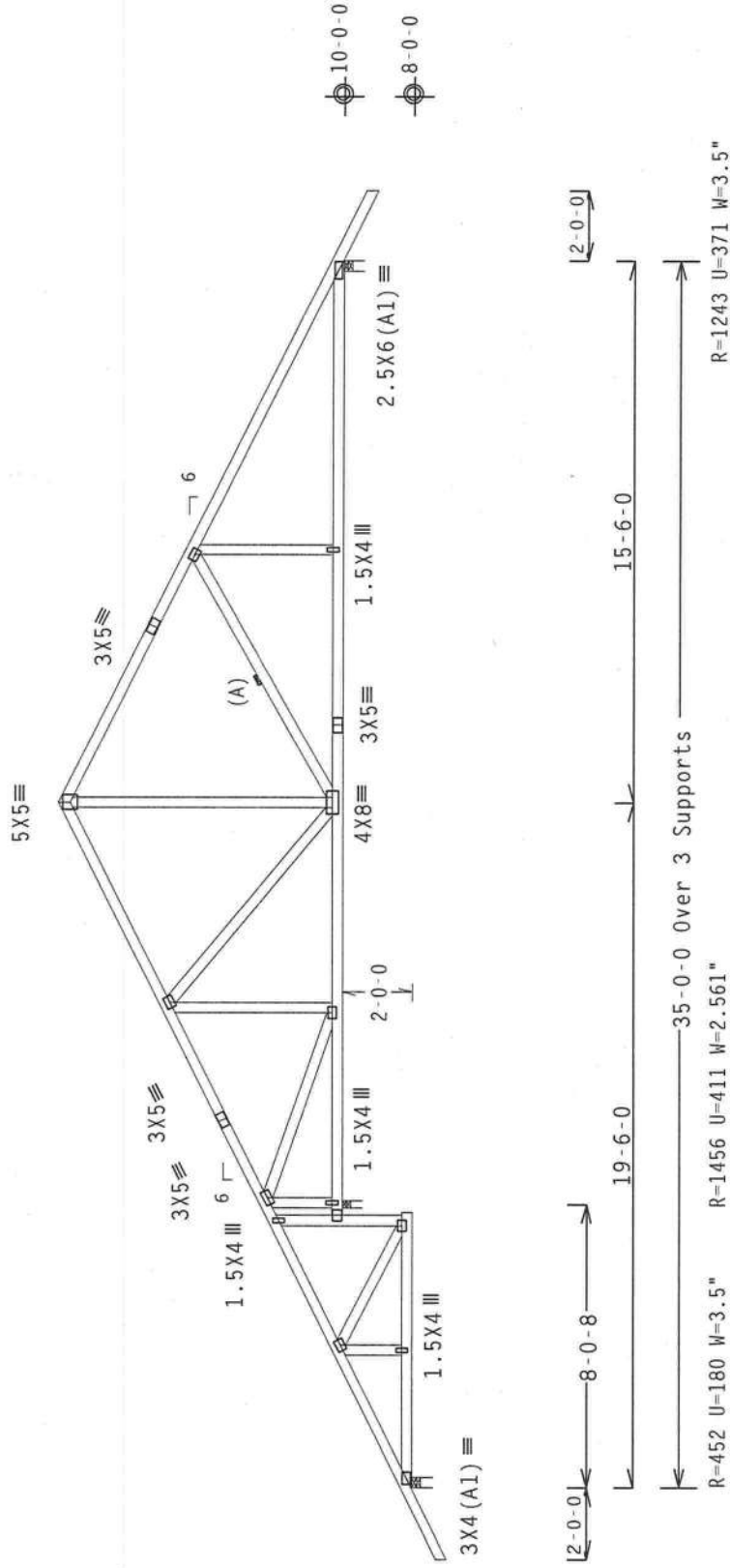
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1950 Narley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 12.72 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3x4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0805

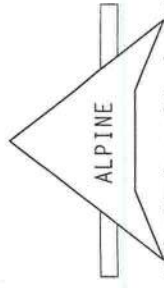
QTY:1

FL/-/4/-/R/-

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719), AND METCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR ALL DESIGN AND CONSTRUCTION OF THE TRUSS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS IN CONFORMANCE WITH THE NATIONAL DESIGN SPEC. BY AREA) AND TPI DESIGN SPEC. CONNECTOR PLATES ARE MADE OF 2018/186A (A/R/S) ASTM A653 GRADE 40/60 (M, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANHX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1050 N. Orlando Drive
Haines City, FL 33844
FL Certificate of Authorization # 567



Oct 05 '05

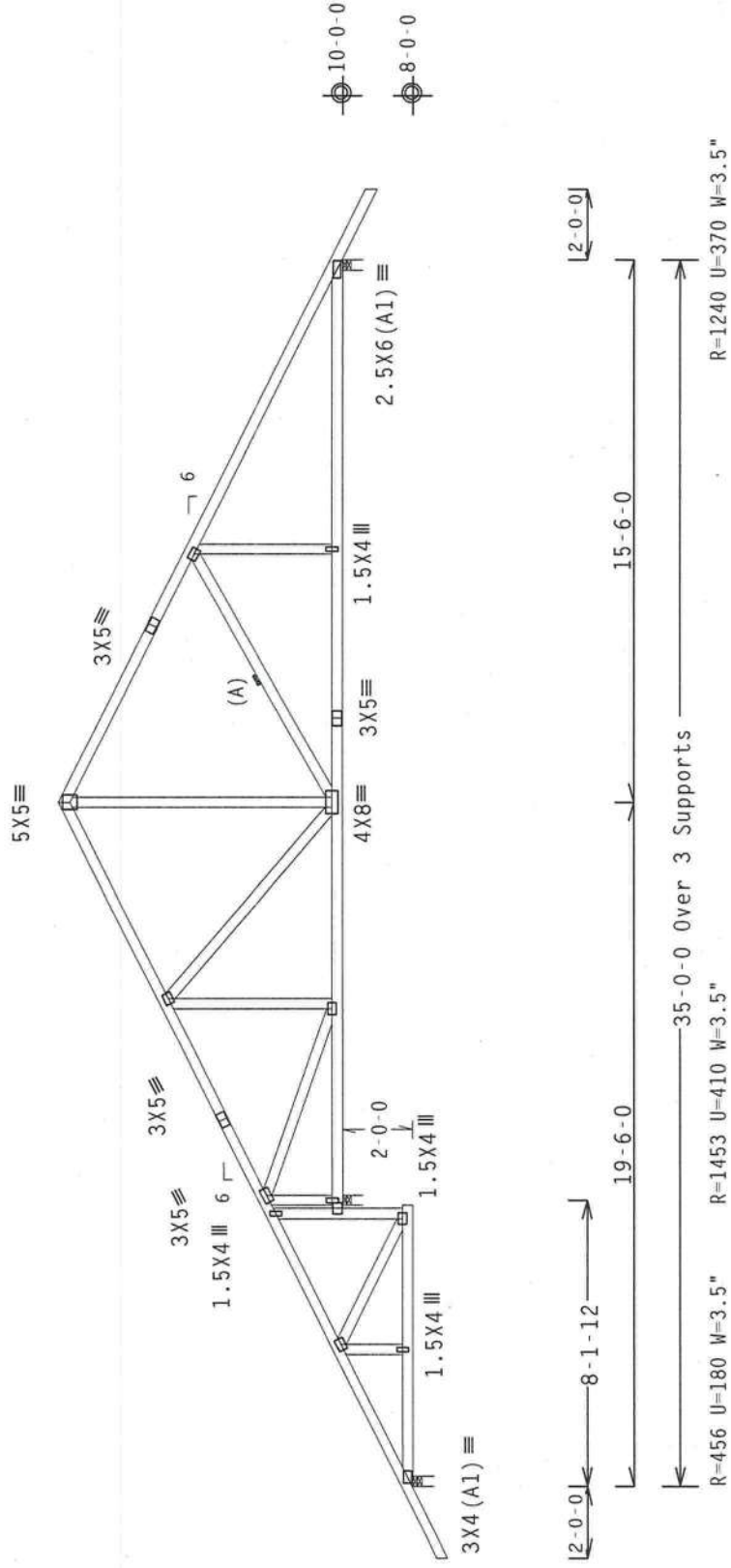
TC LL	20.0 PSF	REF	R487 -- 340
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278034
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	158824
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 12.72 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04 .0805

QTY: 1

FL/-/4/-/-/R/-

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487 -- 341
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278036
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	158815
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03

HARRY B. GOODSON
 LICENSE # 26454
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER

Oct 05 2005

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (MANUAL DESIGN SPEC. BY AISC) AND AISC STEEL CONSTRUCTION MANUAL (MANUAL OF STEEL CONSTRUCTION). ALL TRUSS MEMBERS TO BE FABRICATED AND SHIPPED TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AHS1/TPI 1 SEC. 2.

PLT TYP. Wave

ALPINE

Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844

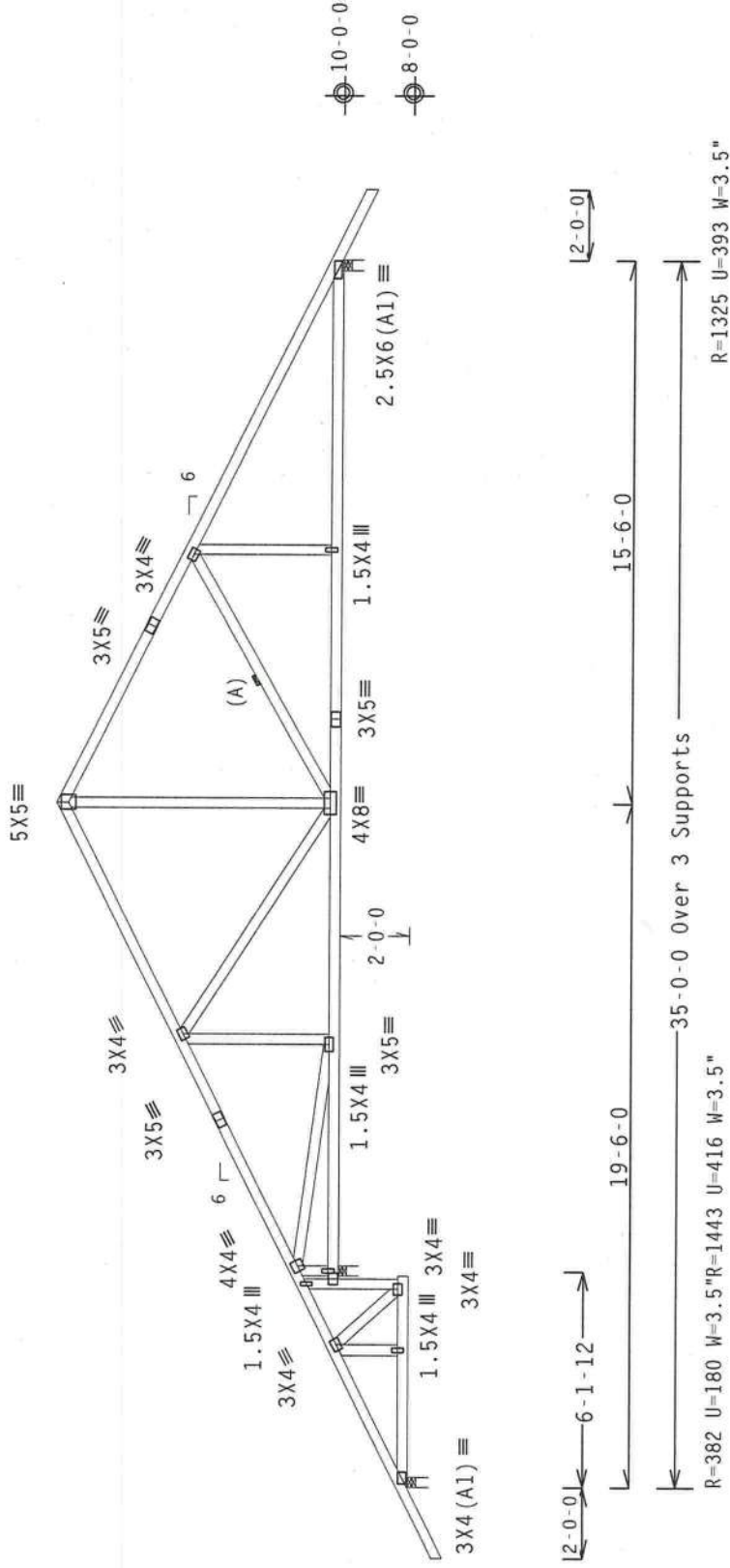
PI Certificate of Authorization # 567

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 12.72 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



6-1-12 →
 19-6-0 →
 15-6-0 →
 2-0-0 →
 10-0-0 →
 8-0-0 →

35-0-0 Over 3 Supports
 R=382 U=180 W=3.5 R=1443 U=416 W=3.5
 R=1325 U=393 W=3.5

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

ALPINE
 Alpine Engineered Products, Inc.
 1950 Mayday Drive
 Haines City, FL 33844

FL Certificate of Authorization # 567

Scale = .1875" / Ft.

TC LL	20.0 PSF	REF	R487 -- 342
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278038
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN	158835
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1SR1487 Z03

QTY: 1 FL/-/4/-/-/R/-

Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRILO DR., SUITE 200, MADISON, WI 53719), AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THIS DESIGN. FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING NATIONAL DESIGN SPEC. BY AISC(A3) AND TPI DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. FOR WOOD). APPLY CONNECTOR PLATES ARE MADE OF 20/10/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (H, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

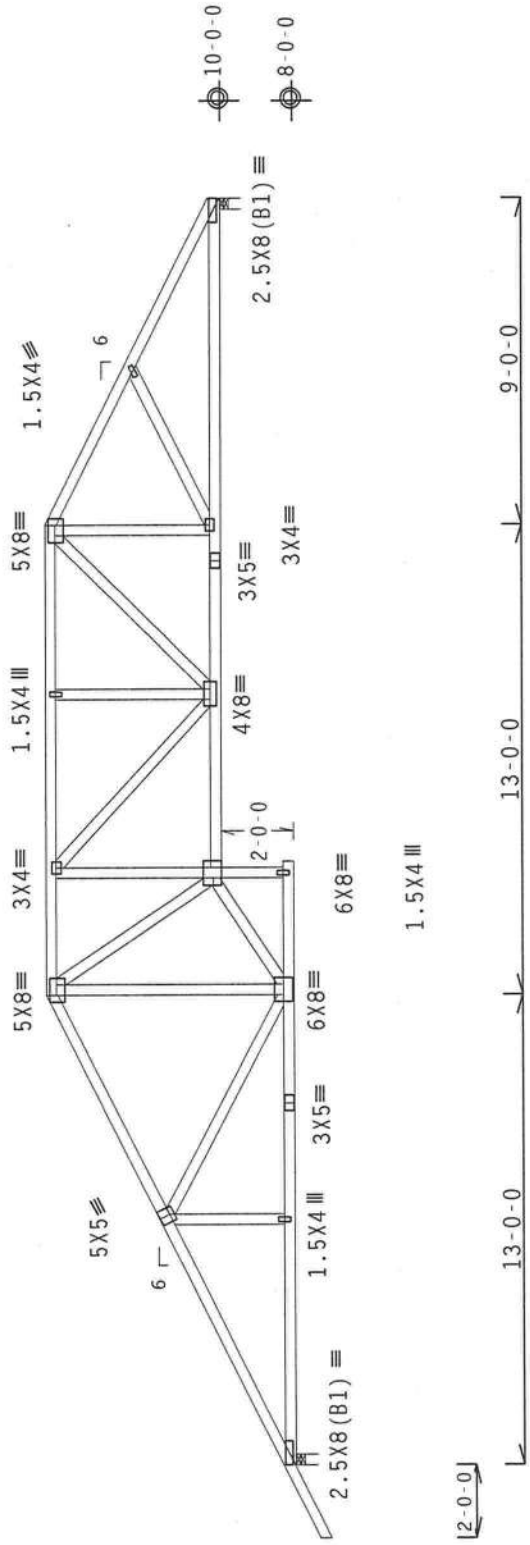
HARRY B. GOODSON
 LICENSE
 No. 26454
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER

Oct 05 '05

110 mph wind, 11.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1580 U=469 W=3.5"
35'-0-0 Over 2 Supports
R=1436 U=425 W=3.5"

Design Crit: TPI-2002 (STD)/FBC

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487 -- 343
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278016
BC LL	0.0 PSF	HC-ENG	SL/HBG3 *
TOT.LD.	40.0 PSF	SEQN-	158851
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03



PLT TYP. Wave

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

Oct 05 05

QTY: 1 FL/-/4/-/-/R/-

Design Crit: TPI-2002 (STD)/FBC
Cg/RT=1.00(1.25)/10(0) 7.04.0805 16

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. SEE TPI-2002 (STD) FOR BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 563 D'ORFELLO DR., SUITE 200, MADISON, WI 53719) AND MCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

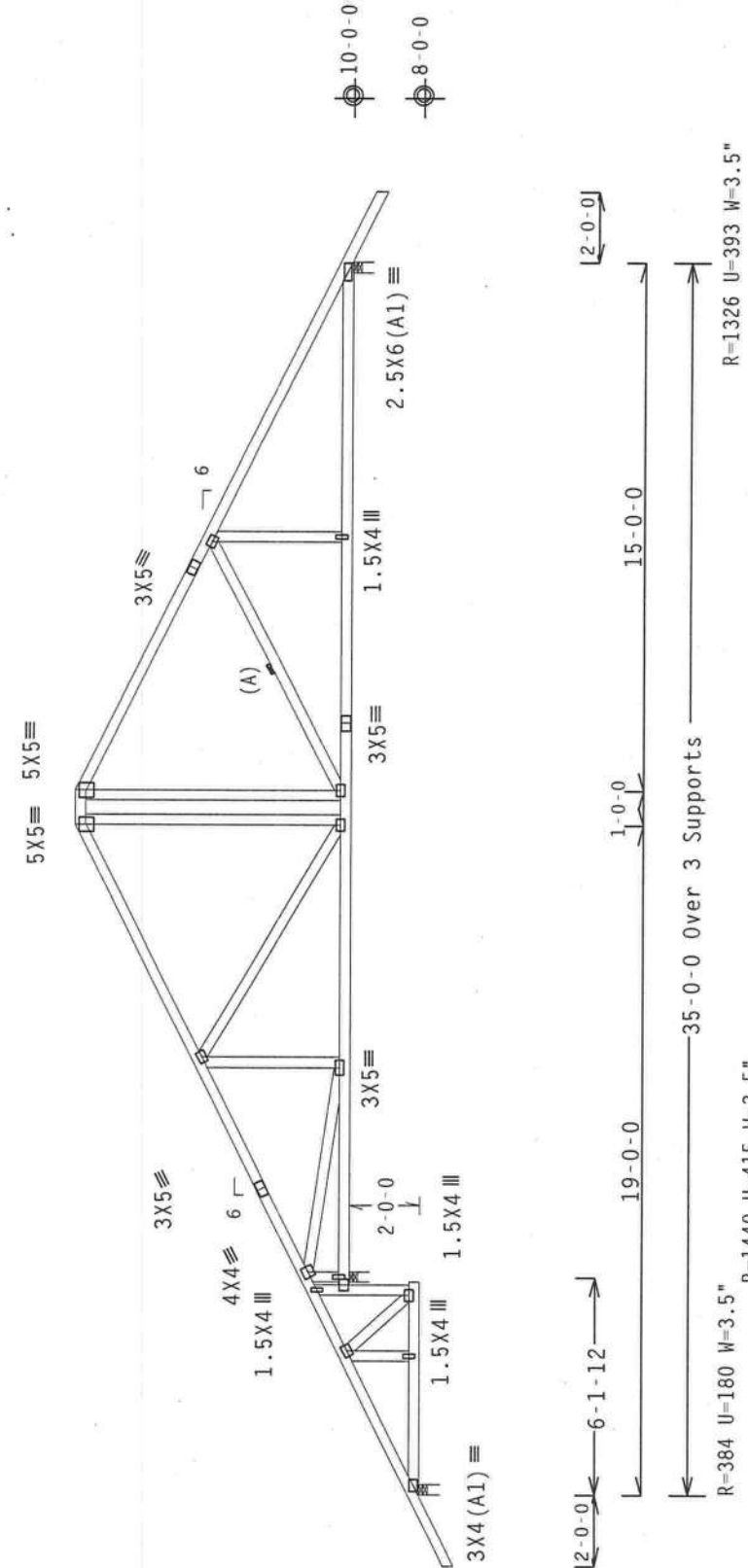
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2019/1606A (A/N/S/K) ASTM A553 GRAD. 40/60 (4. 6/PL5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, LOCATED ON THE 11-2082 SEC. SEAL OR THIS SEAL. ANY TRUSSES FOR WHICH ALPINE ENGINEERED PRODUCTS, INC. HAS BEEN DESIGNATED AS THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHALL INDICATE THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 12.60 ft mean ht, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002 (STD) /FBC

PLT TYP. Wave

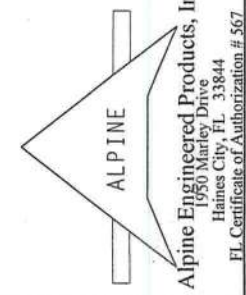
QTY: 1 FL / - / 4 / - / - / R / -

Scale = .1875" / Ft.	REF R487 -- 345
TC LL 20.0 PSF	DATE 10/05/05
TC DL 10.0 PSF	DRW HCUSR487 05278041
BC DL 10.0 PSF	HC-ENG SL/HBG3
BC LL 0.0 PSF	SEQN- 158883
TOT.LD. 40.0 PSF	DUR.FAC. 1.25
SPACING 24.0"	JREF- 1SR1487_Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), (PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 903 D'ONDRILO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

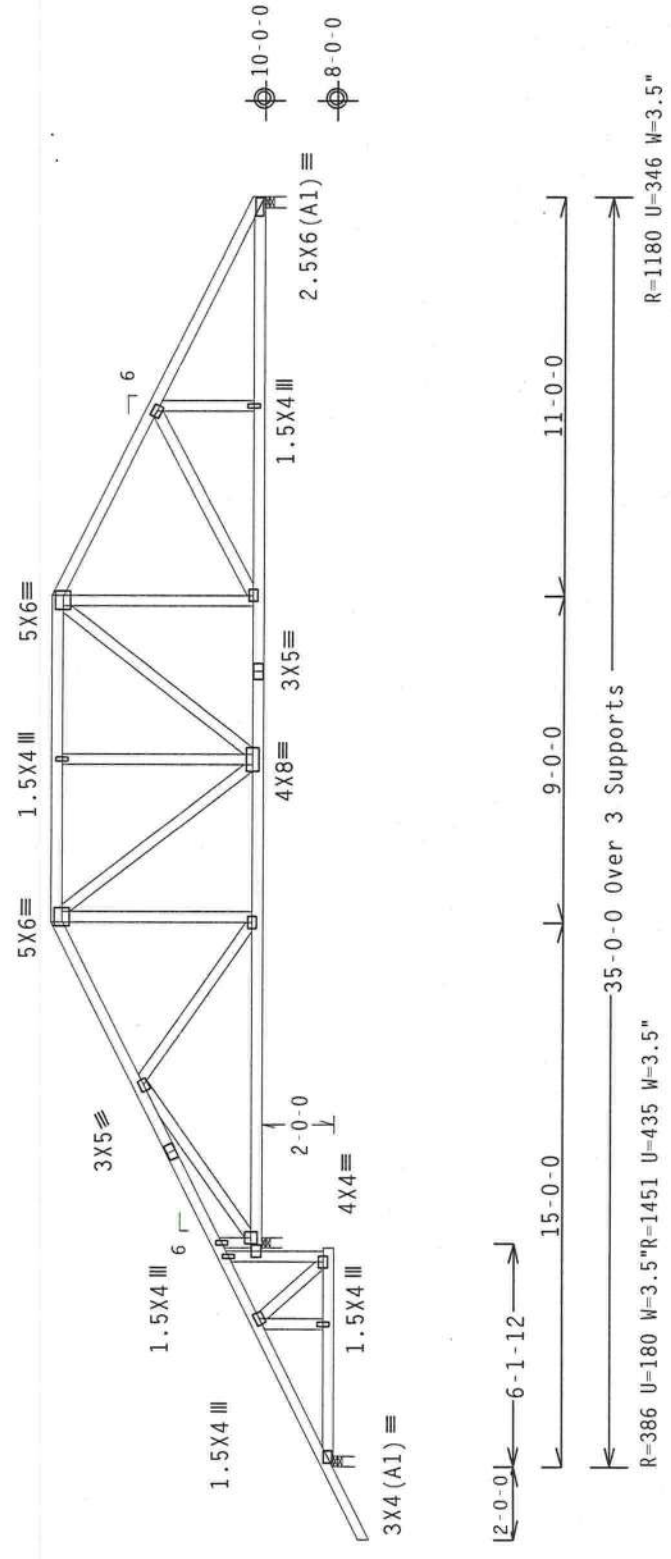
****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. OTHER CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ALPINE CORP. SHALL BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. UNLESS OTHERWISE INDICATED, ALL TRUSS PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED, SHALL BE 3/8" THICK GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANSI A308 OR TPI-2002 SEC. 2. UNLESS OTHERWISE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



OCL 05 '05

110 mph wind, 11.60 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002 (STD)/FBC
 Cg/RT=1.00(1.25)/10(0) 7.04.0805

PLT TYP. Wave

TC LL	20.0 PSF	FL/-/4/-/R/-	QTY:1	Scale = .1875"/Ft.
TC DL	10.0 PSF			REF R487-- 346
BC DL	10.0 PSF			DATE 10/05/05
BC LL	0.0 PSF			DRW HCUSR487 05278042
TOT.LD.	40.0 PSF			HC-ENG SL/HBG3
DUR.FAC.	1.25			SEQN- 158861
SPACING	24.0"			JREF- 1SR1487_Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 MADISON ST., SUITE 200, MADISON, WI 53719), AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MERRILL ST., PITTSBURGH, PA 15222) FOR MORE INFORMATION. THESE FABRICATORS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (4-H/S/A) ASTM A653 GRADE 40/60 (4. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SOLE LIABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

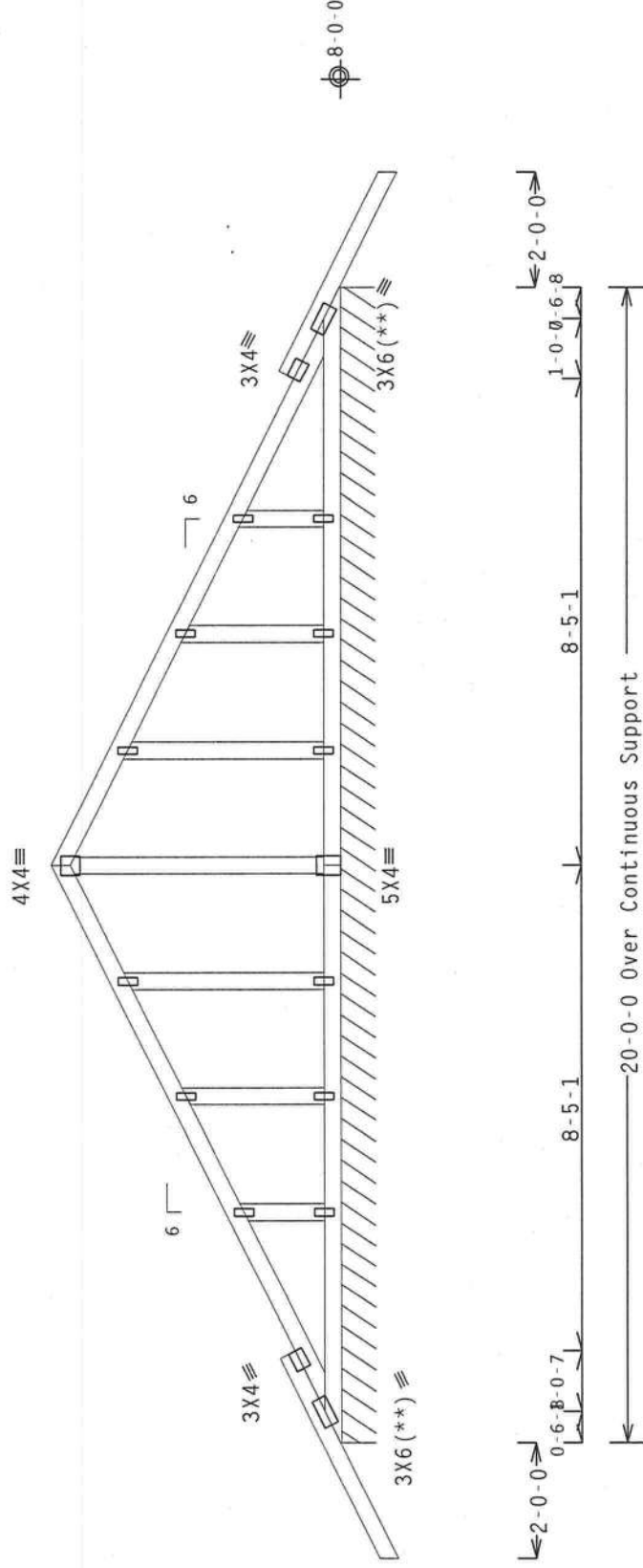
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
 TC - From 83 PLF at -2.00 to 83 PLF at 22.00
 BC - From 4 PLF at -2.00 to 4 PLF at 0.00
 BC - From 20 PLF at 0.00 to 20 PLF at 20.00
 BC - From 4 PLF at 20.00 to 4 PLF at 22.00

110 mph wind, 10.18 ft mean hgt, ASCE 7-98, CLOSED bldg,
 Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf,
 wind BC DL=2.2 psf.

See DWGS AL1015EC1103 & 68LLETIN0405 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep
 increase factor for dead load is 1.50.



R=112 PLF U=43 PLF W=20-0-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0805

QTY:1

Scale = .3125"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D-080R10 DL., SUITE 200, MADISON, WI 53719), AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., PROSPER, TX 75078). ALL SAFETY PRACTICES PROPORTIONAL TO THE TRUSS DESIGN SHALL BE USED TO MAINTAIN RIGID CEILING.
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (M/M/S/K) ASTM A653 GRADE 40/60 (IN. K/H/S). STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE
 Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567



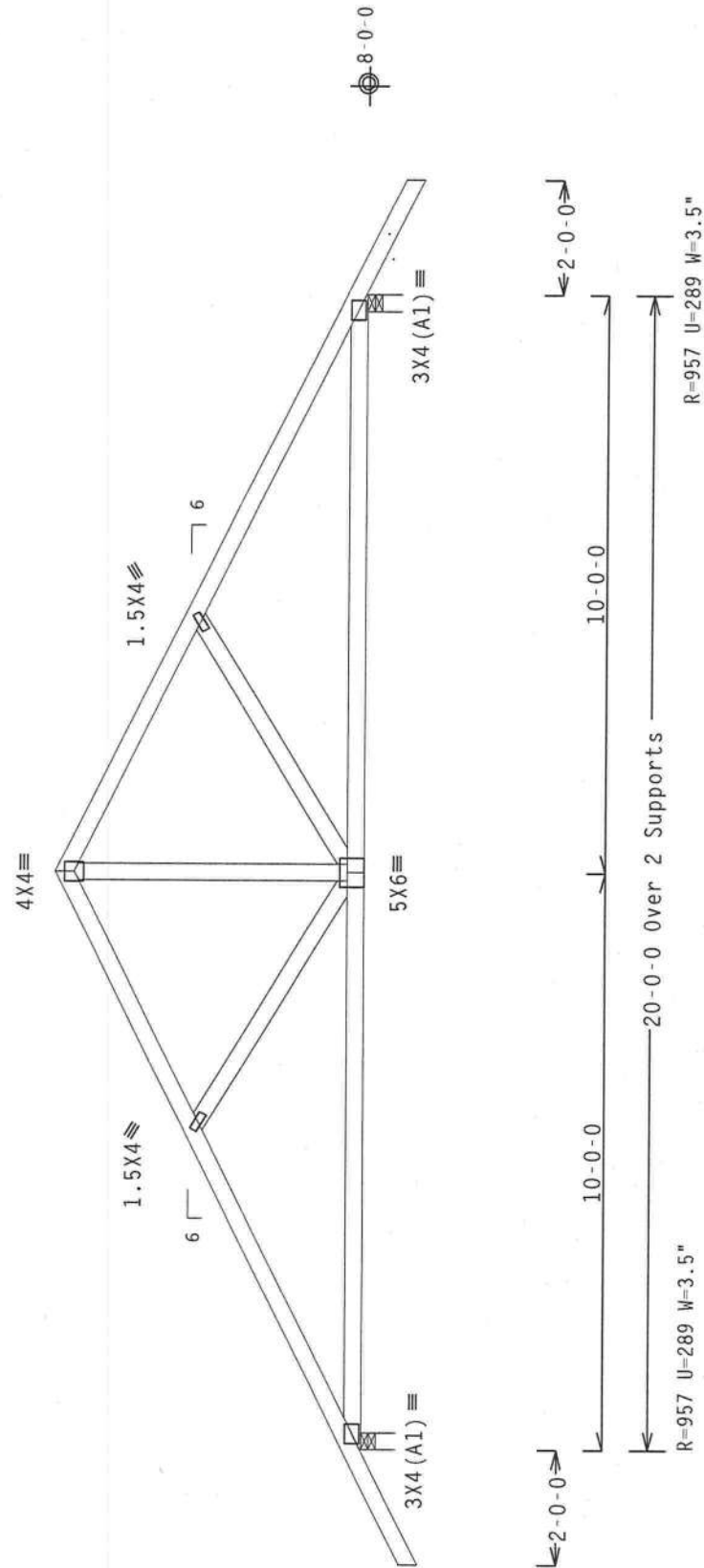
Oct 05 '05

TC LL	20.0 PSF	FL / - / 4 / - / - / R / -	REF	R487 - - 347
TC DL	10.0 PSF		DATE	10/05/05
BC DL	10.0 PSF		DRW	HCUSR487 05278043
BC LL	0.0 PSF		HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF		SEQN-	125188
DUR.FAC.	1.25		JREF-	1SR1487_Z03
SPACING	24.0"			

110 mph wind, 10.35 ft mean hgt, ASCE 7-98, CLOSED bldg.
 Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf,
 wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805 QTY:10 FL/-/4/-/R/- Scale = .3125"/Ft.

TC LL	20.0 PSF	REF	R487 -- 348
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278017
BC LL	0.0 PSF	HC-ENG	SL/HBG3 *
TOT.LD.	40.0 PSF	SEQN	125195
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1SR1487_Z03

ALPINE
 Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567

HARRY B. GOODSON
 LICENSE
 No. 26454
 PROFESSIONAL ENGINEER
 STATE OF FLORIDA
 Oct 05 '05

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS I-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI, P.O. BOX 1000, MADISON, WI 53719 OR, SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 630 ENTERPRISE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/186GA (H-M/57A) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY ANY INSPECTION OF PLATES AND JOINTS TO THE PRESSURE SIDE OF THE TRUSS. POSITION PER DRAWINGS 160A-2. A SEAL ON THIS DESIGN SHOWS ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. FOR THE TRUSSES COMPANY BUILDING DESIGNER PER AMSI/TPI 1 SEC. 2.

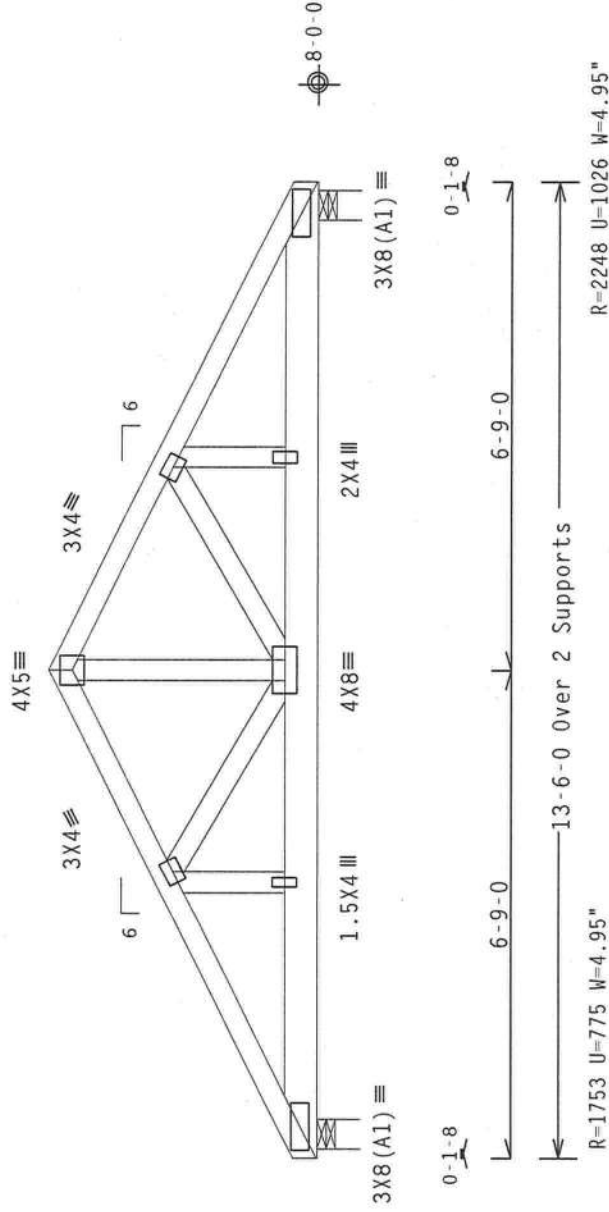
Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg,
Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf,
wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep
increase factor for dead load is 1.50.

SPECIAL LOADS

TC	From 62 PLF at 0.00 to 20 PLF at 13.50	DUR.FAC.=1.25	PLATE	DUR.FAC.=1.25
BC	From 20 PLF at 0.00 to 49 LB Conc. Load at 1.09			
TC	120 LB Conc. Load at 3.09			
TC	182 LB Conc. Load at 5.06			
BC	129 LB Conc. Load at 1.09			
BC	211 LB Conc. Load at 3.09			
BC	294 LB Conc. Load at 5.06			



Design Crit: TPI-2002(STD)/FBC

<p>PLT TYP. Wave</p> <p>Alpine Engineered Products, Inc. 1050 Marley Drive Haines City, FL 33844 FL Certificate of Authorization # 567</p>	<p>QTY: 1</p> <p>FL/14/-/R/-</p> <p>Scale = .375"/Ft.</p>	<p>TC LL 20.0 PSF</p> <p>TC DL 10.0 PSF</p> <p>BC DL 10.0 PSF</p> <p>BC LL 0.0 PSF</p> <p>TOT.LD. 40.0 PSF</p> <p>DUR.FAC. 1.25</p> <p>SPACING 24.0"</p>	<p>REF R487 -- 349</p> <p>DATE 10/05/05</p> <p>DRW HCUSR487 05278045</p> <p>HC-ENG SL/HBG3</p> <p>SEQN- 125180</p> <p>JREF- 1SR1487_Z03</p>
		<p>Design Crit: TPI-2002(STD)/FBC</p> <p>Cq/RT=1.00(1.25)/10(0) 7.04.0805 16</p> <p>**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719), AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.</p> <p>**IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE OF THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AWS (NATIONAL DESIGN SPEC, BY AFPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (4-11/8) ASTM A653 GRADE 40/60 (4, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AWS/TPI 1 SEC. 2.</p>	

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

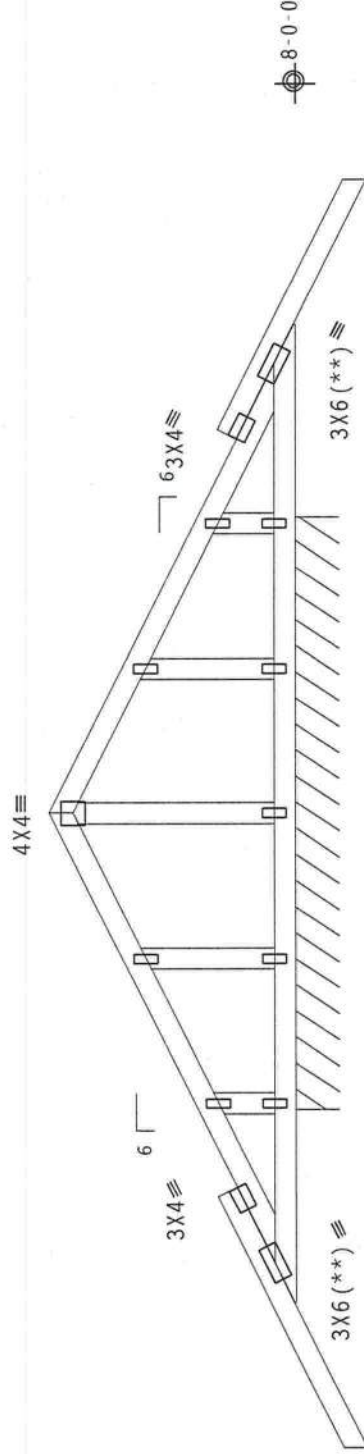
SPECIAL LOADS

---(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 83 PLF at -2.00 to 83 PLF at 15.50
BC - From 4 PLF at -2.00 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 13.50
BC - From 4 PLF at 13.50 to 4 PLF at 15.50

110 mph wind, 9.37 ft mean ht., ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

See DWGS A11015EC1103 & 68LLETIN0405 for more requirements.

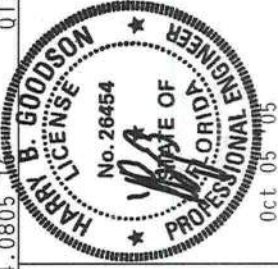
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.04.0805

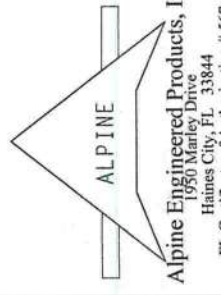
QTY: 1 FL/- /4/- /- /R/- Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 -- 350
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278046
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN	125165
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1SR1487 Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DECSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NITA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ANPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018T166A (4-1/2X8) ASH 5053 GRADE 40700 (4. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, SUBSTITUTION PER DRAWING 2-6001Z. ALL TRUSS CHORDS AND WEBS SHALL BE FULLY BRACED TO THE TRUSS CHORDS AND WEBS TO MAINTAIN THE STABILITY OF THE TRUSS SYSTEM. THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

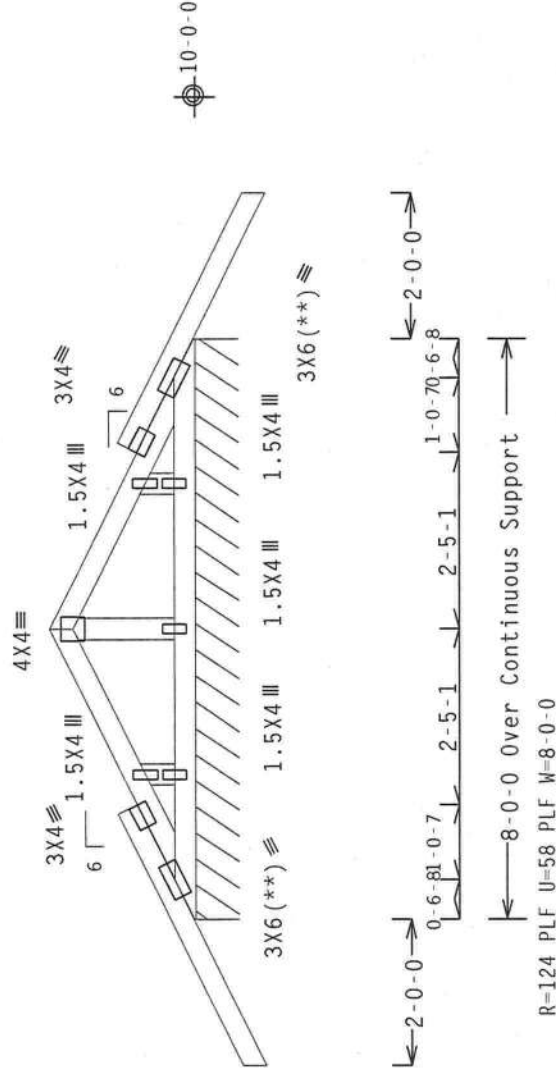
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 83 PLF at -2.00 to 83 PLF at 10.00
BC - From 4 PLF at -2.00 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 8.00
BC - From 4 PLF at 8.00 to 4 PLF at 10.00

110 mph wind, 10.68 ft mean hgt, ASCE 7-98, CLOSED bldg,
Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf,
wind BC DL=2.2 psf.

See DWGS A11015EC1103 & GBLLETTIN0405 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep
increase factor for dead load is 1.50.



Design Crit: TPI-2002 (STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0805

QTY:1

FL/-/4/-/-/R/-

Scale = .375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT MANUFACTURER'S, TPI, TPI-2002 (STD), TPI-2002 (STD)/FBC, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 (STD) OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M-H/S/K) ASTM A653 GRADE 40/50 (M, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC 330 OF TPI-2002, SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SOLE RESPONSIBILITY FOR THE USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.

PLT TYP. Wave

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

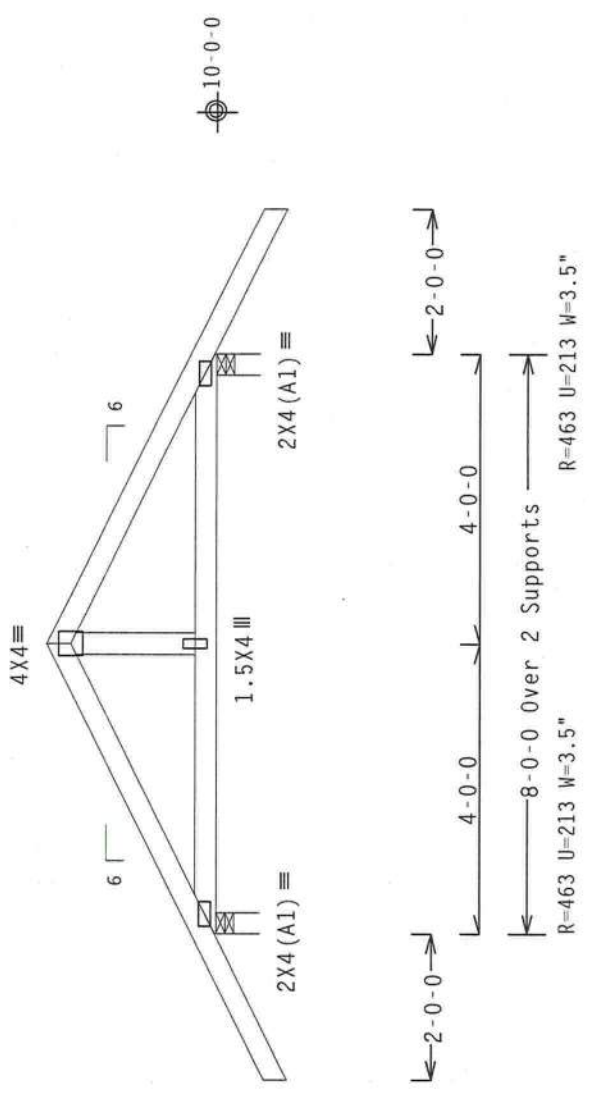
Oct 05 '05

TC LL	20.0 PSF	REF	R487-- 352
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278048
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	125150
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 10.85 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.



Design Crit: TPI-2002 (STD) /FBC

PLT TYP. Wave	QTY:1	FL/-/4/-/-/R/-	Scale = .375" /Ft.
REF R487--	353	TC LL	20.0 PSF
DATE 10/05/05		TC DL	10.0 PSF
DRW HCUSR487 05278018		BC DL	10.0 PSF
HC-ENG SL/HBG3		BC LL	0.0 PSF
SEQN- 125154		TOT.LD.	40.0 PSF
JREF- 1SR1487_Z03		DUR.FAC.	1.25
		SPACING	24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING PROPER BRACING AND BRACING OF TRUSSES. CORNER PLATES ARE MADE TO ORDER. THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING CORNER PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER BRACKINGS TABLE 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

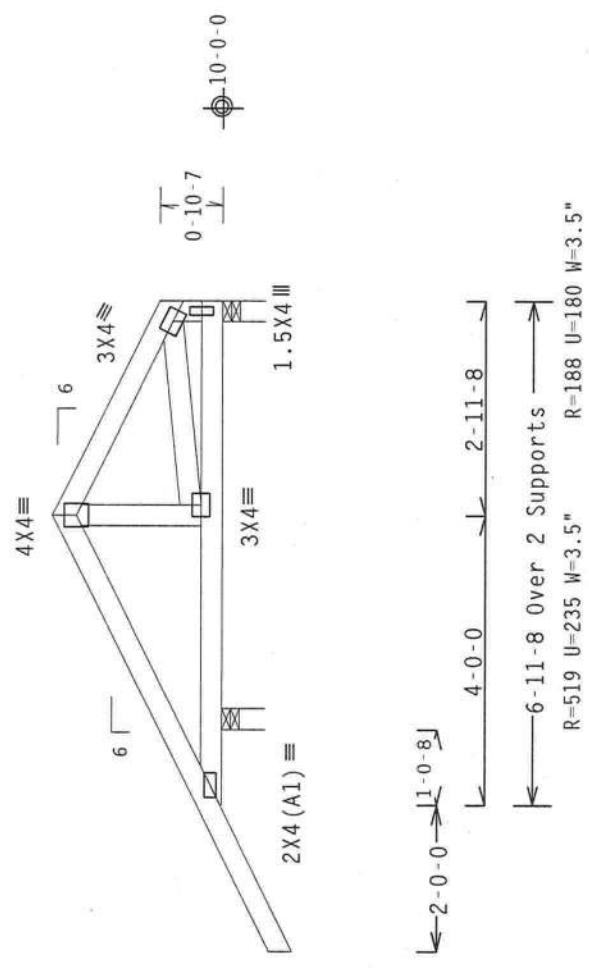
ALPINE

Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 10.85 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.



PLT TYP. Wave
 Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04-.0805 QTY:2 FL/-/4/-/ -/R/- Scale = .375"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS DESIGN AND CONSTRUCTION. TPI CROSS PLATE INSTITUTE, 593 DUNDORF DR., SUITE 200, MADISON, WI 53715) AND UTILIZE THE FOLLOWING BEST PRACTICES FOR TRUSS DESIGN AND CONSTRUCTION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/K) ASTM A653 GRADE 40/60 (H, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ALL DIMENSIONS OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002, SEC.3. A SEAL ON THIS DRAWING INDICATES THE SUITABILITY AND USE OF THIS COMPONENT FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.

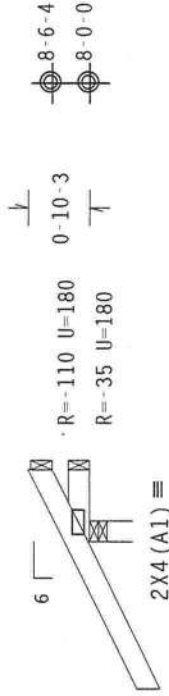
ALPINE
 Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 8.10 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



←-2-0-0→

0-60-5-8
1-0-0 over 3 Supports

R=361 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0805

QTY:6

FL/-/4/-/-/R/-

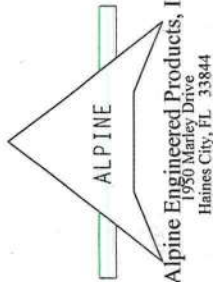
Scale = .375"/Ft.

TC LL	20.0 PSF	REF	R487 -- 355
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278050
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	125106
DUR.FAC.	1.25		
CDAPTMC	24 0"		
TDEC	1001A07 700		



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 563 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719), AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS OR THE BUILDING. THE DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC, BY AISC AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1866 (A-N/S) ASTM A653 GRADE 40/60 (M, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

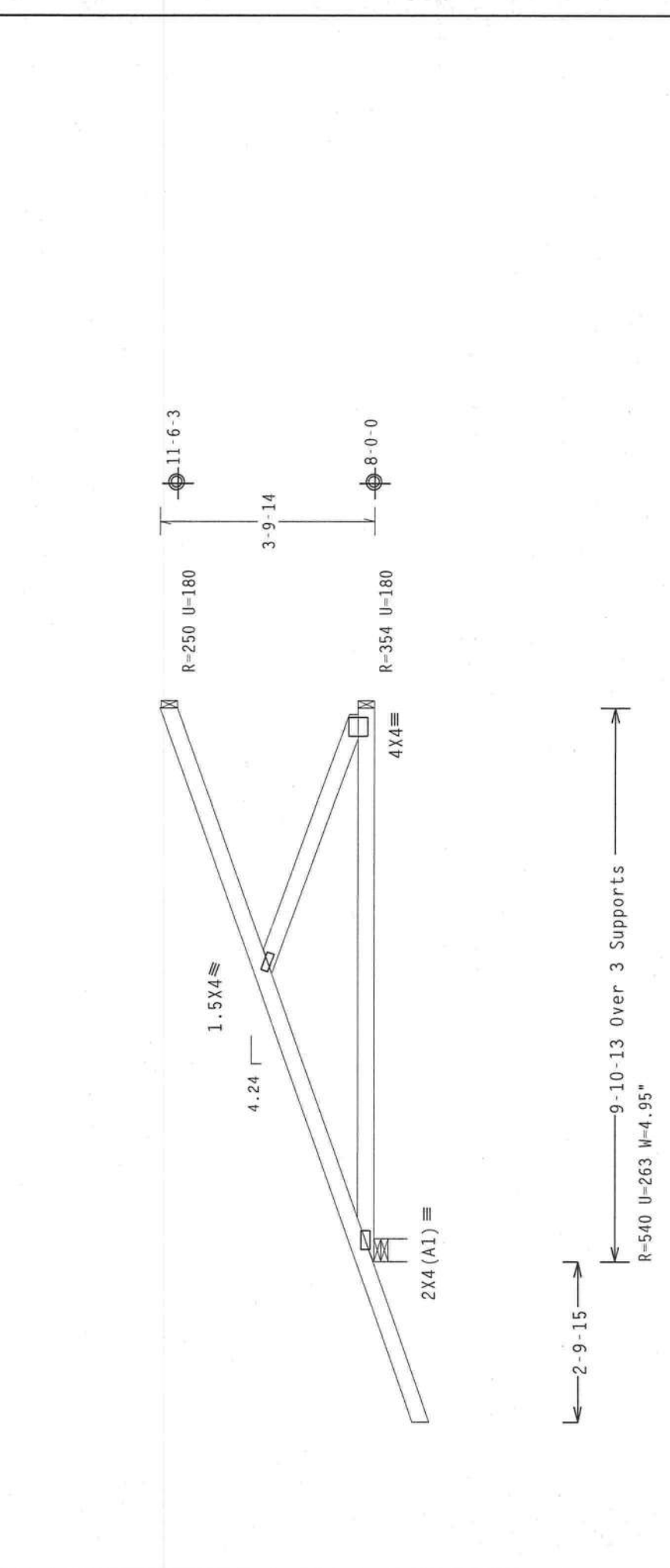


Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Hipjack supports 7-0-0 setback jacks with no webs.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
 Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805.16 QTY:3 FL/-/4/-/ -/R/-

Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 - - 356
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278051
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	125143
DUR.FAC.	1.25		
SPACING	24 0"		



Oct 05 05

Oct 05 05

ALPINE
 Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805.16 QTY:3 FL/-/4/-/ -/R/-

Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 - - 356
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278051
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEQN-	125143
DUR.FAC.	1.25		
SPACING	24 0"		

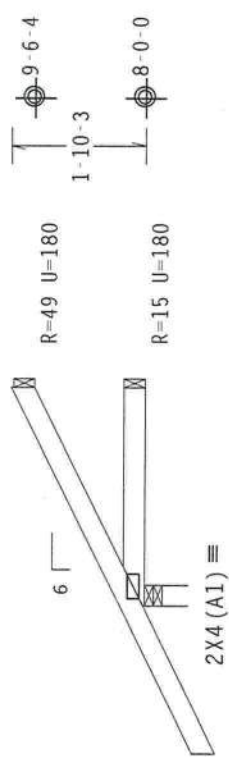
Oct 05 05

Oct 05 05

ALPINE
 Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844

110 mph wind, 8.60 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



← 2-0-0 →
 3-0-0 Over 3 Supports
 R=317 U=180 W=3.5"

Design Crit: TPI-2002 (STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805

PLT TYP. Wave	QTY: 6	FL / - / 4 / - / - / R / -	Scale = .375" / Ft.
REF R487-- 357	TC LL	20.0 PSF	
DATE 10/05/05	TC DL	10.0 PSF	
DRW HCUSR487 05278052	BC DL	10.0 PSF	
HC-ENG SL/HBG3	BC LL	0.0 PSF	
SEQN- 125128	TOT.LD.	40.0 PSF	
	DUR.FAC.	1.25	
JREF - 1SR1487_Z03	SPACING	24.0"	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRETO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, TOPSHAM, ME 04086) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

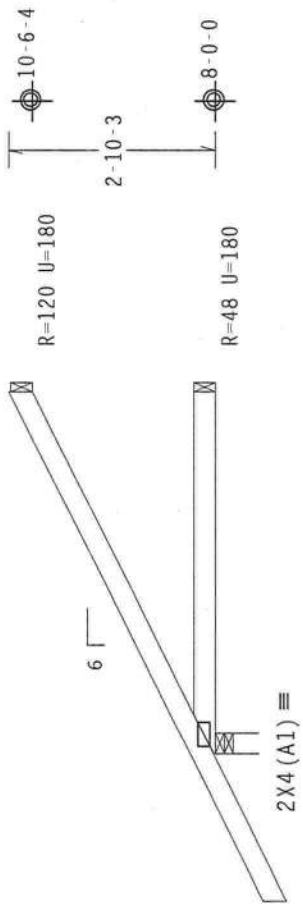
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFECTS OR DAMAGE TO THE TRUSS OR TO THE BUILDING. THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING HANDLING, SHIPPING, INSTALLING & BRACING OF THE TRUSS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/R) ASTM A653 GRADE 40/60 (P, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.
 1950 Manley Drive
 Gaines City, FL 33844
 FL Certificate of Authorization # 567

Oct 05 '05

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 9.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



←-2-0-0→

←-5-0-0 Over 3 Supports →
 R=377 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805

QTY: 6 FL/-/4/-/ -/R/- Scale = .375"/Ft.

TC LL	20.0 PSF	REF	R487-- 358
TC DL	10.0 PSF	DATE	10/05/05
BC DL	10.0 PSF	DRW	HCUSR487 05278054
BC LL	0.0 PSF	HC-ENG	SL/HBG3
TOT.LD.	40.0 PSF	SEON-	125132
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SR1487_Z03



Oct 05 '05

PLT TYP. Wave

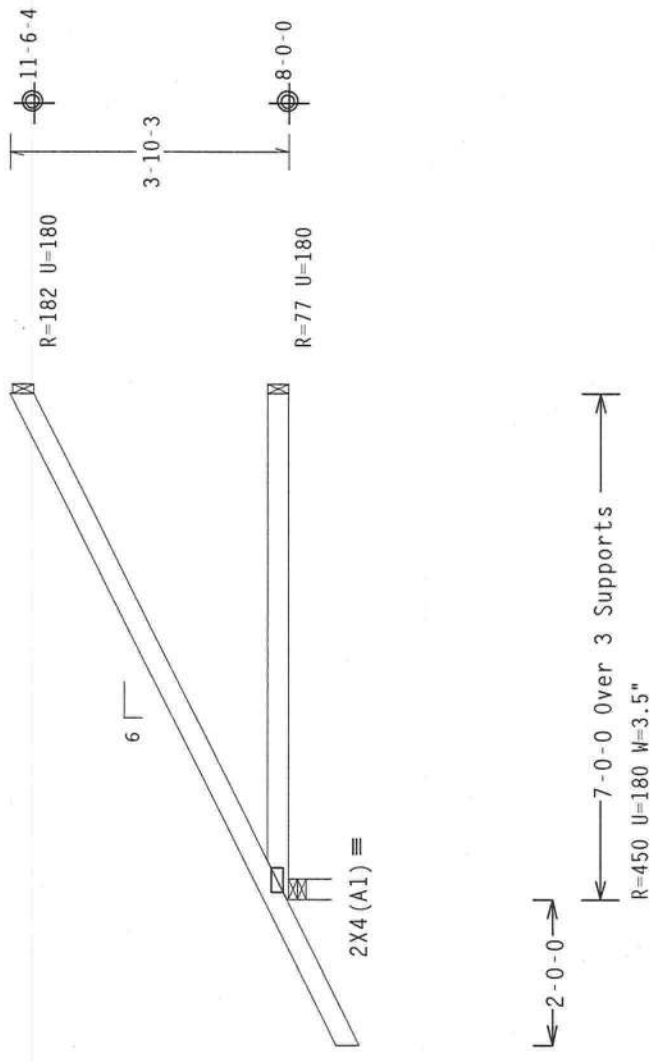
ALPINE
 Alpine Engineering Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 PE Certificate of Authorization # 567

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPIE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE USER OF THIS DESIGN SHALL BE RESPONSIBLE FOR PROVIDING THE PROPER BRACING FOR ALL TRUSSES. ALL TRUSS CONNECTOR PLATES ARE MADE OF 60,10/16GALV IN HV5/ASTM A563 GRADE 40/60. BY WELDED OR GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

110 mph wind, 9.60 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805 QTY:27 FL/-/4/-/1/R/- Scale = .375"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R487 -- 359
 DATE 10/05/05
 DRW HCUSR487 05278055
 HC-ENG SL/HBG3
 SEON- 125136
 JREF- 1SR1487_Z03

ALPINE
 Alpine Engineered Products, Inc.
 1950 Marley Drive
 Haines City, FL 33844
 FL Certificate of Authorization # 567

HARRY B. GOUDSON
 LICENSE No. 26454
 PROFESSIONAL ENGINEER STATE OF FLORIDA
 Oct 05 '05

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 963 W. WISCONSIN ST., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (M/M/S/K) ASTM A653 GRADE 40/60 (M, K/M-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002-SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGNER'S USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT.TYP. Wave
 Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/10(0) 7.04.0805 QTY:27 FL/-/4/-/1/R/- Scale = .375"/Ft.

ASCE 7-98: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH	GABLE SPACING	2X4 VERTICAL SPECIES	BRACE GRADE	NO BRACES		(1) 1X4 "L" BRACE *		(2) 2X4 "L" BRACE *		(1) 2X6 "L" BRACE **		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	STUD	3' 10"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
				3' 9"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
24" O.C.	HF	STANDARD	#1	3' 9"	6' 0"	7' 11"	6' 9"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
				4' 3"	6' 8"	7' 11"	6' 9"	9' 5"	9' 5"	10' 7"	10' 7"	14' 0"	14' 0"
24" O.C.	SP	#1	#2	4' 2"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
				4' 0"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
24" O.C.	DFL	STANDARD	#3	3' 10"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
				4' 5"	7' 8"	9' 1"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	STUD	4' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	HF	STANDARD	#3	4' 4"	7' 4"	9' 1"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
				4' 10"	7' 8"	9' 1"	9' 9"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SP	#1	#2	4' 9"	7' 8"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 6"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	DFL	STANDARD	#3	4' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
				4' 9"	8' 5"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	STUD	4' 9"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 9"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	HF	STANDARD	#1	5' 4"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
				5' 3"	8' 5"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SP	#1	#2	5' 0"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
				5' 0"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	DFL	STANDARD	#3	4' 11"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
				4' 11"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#2
STUD	STUD
#3	STANDARD

DOUGLAS FIR-LARCH

STUD	SOUTHERN PINE
#3	#3
STANDARD	STANDARD

GROUP B:

HEM-FIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
#1	#2

SOUTHERN PINE

STUD	DOUGLAS FIR-LARCH
#1	#1
#2	#2

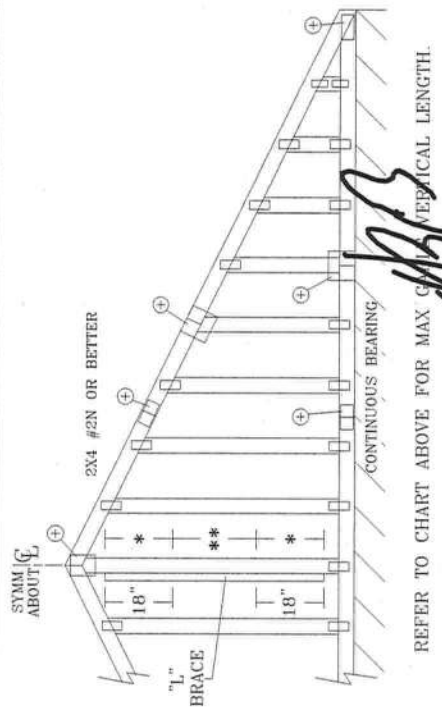
GABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS L/240.
- PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
- GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH "L" BRACE WITH 10d NAILS.
- * FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
- ** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
- "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2-5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

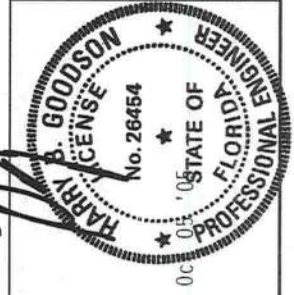
VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDDPOINT OF VERTICAL WEB.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 983 BONDURID DR., SUITE 200, MADISON, WI 53719, AND VICA (WOOD TRUSS COUNCIL) 1000 W. 10TH ST., SUITE 100, DENVER, CO 80202 FOR MORE INFORMATION ON THE PROPER USE OF THESE FUNCTIONAL UNLESS OTHERWISE INDICATED. TPI CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. FOR WOOD CONSTRUCTION) SHALL BE RESPONSIBLE FOR VERIFYING THAT ALL TRUSSES ARE FABRICATED TO THE DESIGN AND THAT ALL TRUSSES ARE PROPERLY BRACED AND SHIPPED. ALL TRUSSES SHALL BE PER ANEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANS1/TPI 1 SEC. 2.



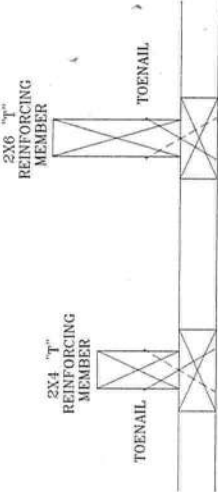
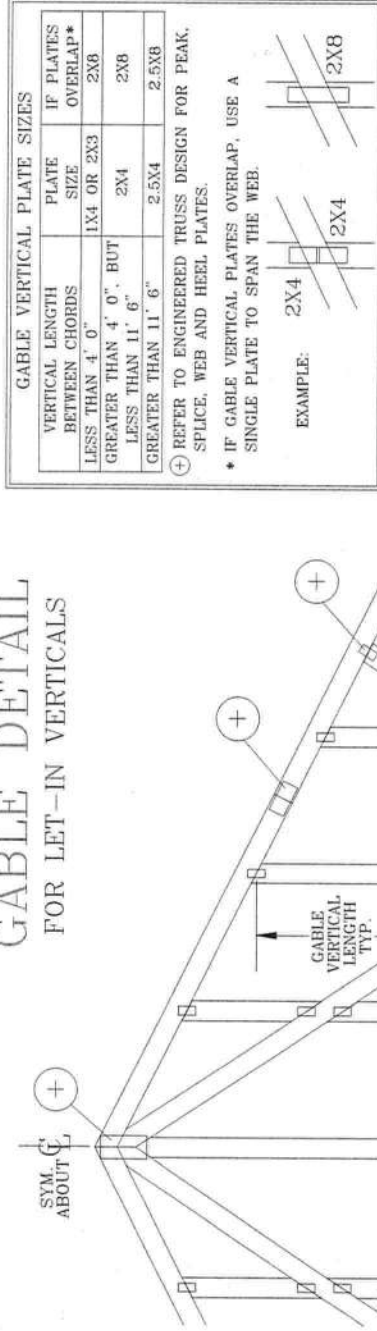
REF	ASCE7-98-GABI1015
DATE	11/26/03
DRWG	A11015EC1103
	-ENG

MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

GABLE DETAIL FOR LET-IN VERTICALS



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

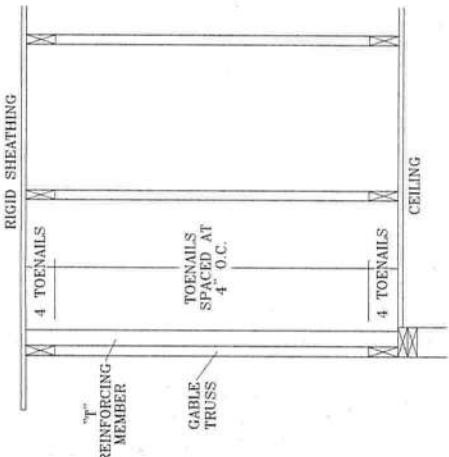
WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
 ASCE WIND SPEED = 100 MPH
 MEAN ROOF HEIGHT = 30 FT
 GABLE VERTICAL = 24' O.C. SP #3
 "T" REINFORCING MEMBER SIZE = 2X4
 "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
 (1) 2X4 "L" BRACE LENGTH = 6' 7"
 MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

- ATTACH EACH "T" REINFORCING MEMBER WITH HAND DRIVEN NAILS:
 - 10d COMMON TOENAILS AT 4" O.C. PLUS (4) 16d COMMON TOENAILS IN TOP AND BOTTOM CHORD.
 - GUN DRIVEN NAILS - 0.131" X 3"
 - TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.
- THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
- ASCE 7-93 GABLE DETAIL DRAWINGS:
 - A11015EN1103, A10015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
 - A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
- ASCE 7-98 GABLE DETAIL DRAWINGS:
 - A13015EC1103, A12015EC1103, A11015EC1103, A10015EC1103, A08515EC1103
 - A13030EC1103, A12030EC1103, A11030EC1103, A10030EC1103, A08530EC1103
- SBCCI GABLE DETAIL DRAWINGS:
 - S11015EN1103, S10015EN1103, S09015EN1103, S08015EN1103, S07015EN1103
 - S11030EN1103, S10030EN1103, S09030EN1103, S08030EN1103, S07030EN1103



THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE COMPANY'S SAFETY INSTRUCTIONS, HANDLING, SHIPPING, INSTALLING AND BRACING MANUAL, PUBLISHED BY TPI CROSS PRODUCTS, INC., 583 DODD RD., SUITE 200, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BRACE OR BRACING OF TRUSSES, DESIGN CONNECTIONS WITH APPROPRIATE TOENAILS, OR FAILURE TO BRACE BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1656 (A/1/2) ASTM A572M GRADE, 40/60 (W/K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI J-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035

REF LET-IN VERT

DATE 01/16/04

DRWG GBLLETIN1103

-ENG DLJ/KAR

MAX TOT. LD. 60 PSF

DUR. FAC. ANY

MAX SPACING 24.0"



Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ~~d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth~~
- ~~e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails~~
- f) Must show and identify accessibility requirements (accessible bathroom)

set
NFR
3

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- ~~c) Any special support required by soil analysis such as piling~~
- ~~d) Location of any vertical steel~~

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and sealed by FI. Pro. Eng.
 - 2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
 - 1. All materials making up wall
 - 2. Block size and mortar type with size and spacing of reinforcement
 - 3. Lintel, tie-beam sizes and reinforcement
 - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
 - 6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - 7. Fire resistant construction (if required)
 - 8. Fireproofing requirements
 - 9. Shoe type of termite treatment (termicide or alternative method)
 - 10. Slab on grade
 - a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
 - a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

~~c) Metal frame wall and roof~~ (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) ~~Floor truss package~~ including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) ~~Floor joist size~~ and spacing
- c) ~~Girder size~~ and spacing
- d) ~~Attachment of joist to girder~~
- e) ~~Wind load requirements~~ where applicable

Plumbing Fixture layout

Electrical layout including:

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
 - d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms

HVAC Information

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

*****Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water (city water)

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

See
NOTE
2

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#23803

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 252-3611
FHAVA Case No. (if any) _____

Section 2: Builder Information

Company Name: Ft. White Developments Company Phone No. _____

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) Ft. White Park Lot 31
Ft. White Z1

Type of Construction (More than one box may be checked) Slab Basement Crawl Other _____
Approximate Depth of Footing: Outside 12 Inside 24 Type of Fill Dirk

Section 4: Treatment Information

Date(s) of Treatment(s) 2-9-06
Brand Name of Product(s) Used Prebuild T.C
EPA Registration No. 100-1006
Approximate Final Mix Solution % 0.25%
Approximate Size of Treatment Area: Sq. ft. 2292 Linear ft. 216 Linear ft. of Masonry Voids 216
Approximate Total Gallons of Solution Applied 445
Was treatment completed on exterior? Yes No
Service Agreement Available? Yes No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments _____

Name of Applicator(s) Steve Brannon Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Steve Brannon Date 2-9-06

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)

Reorder Product #2581 • from CROWNMAX • 1-800-252-4011

August 5, 2006

Columbia County Building and Zoning
North Hernando Street
Lake City, Florida 32056

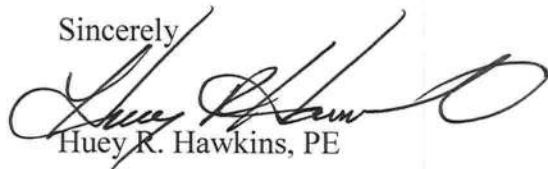
Attention: Mr. Joe Haltiwanger

Reference: Permit Number 23803
Permit Issued 11/02/06
Contractor Mike Herong
Home Owner Fort White Developers, LLP

The referenced permit was issued using a roof truss design submitted by Anderson Truss Company since the issuance we have decided to use Mayo Truss Company for the subject house. The truss design for the front of the house looks much better using Mayo Truss design.

Attached are two copies of the new truss design prepared by Mayo Truss Company. Please accept this revision and substitute this truss design for the one submitted with the permit application.

Sincerely



Huey R. Hawkins, PE

Permit Number: _____ Lot Number: _____
Miscellaneous: _____ Address: _____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Standard Loading:

T.C. Live 20 psf
T.C. Dead 10 psf
B.C. Live 0 psf
B.C. Dead 10 psf
Total 40 psf

Job Reference: FWD-HAWK1 - FWD-HAWKINS1

ANSI/ASCE 7-02
Wind Speed - 120 mph
Mean Roof Ht. - 15 ft.
Exposure Category - B
Occupancy Factor - 1.00
MWFRS
Enclosed

ROBBINS ENGINEERING, INC.	P.O. Box 280055 Tampa, FL 33682-0055 Phone: (813) 972-1135	Engineering Index Sheet	
		Index Page 1 of 1	
Job Number T06031832	Date 03/17/2006	FBC - 2004 Chapter 16 and 23	Specification Quantity 5
<p>A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Software - Online Plus)</p>			

Notes: Refer to individual truss design drawings for special loading conditions.

Date Mark

Date Mark

Date Mark

Date Mark

1	03/17/06	CJ1
5	03/17/06	J4

2	03/17/06	J1
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3	03/17/06	J2
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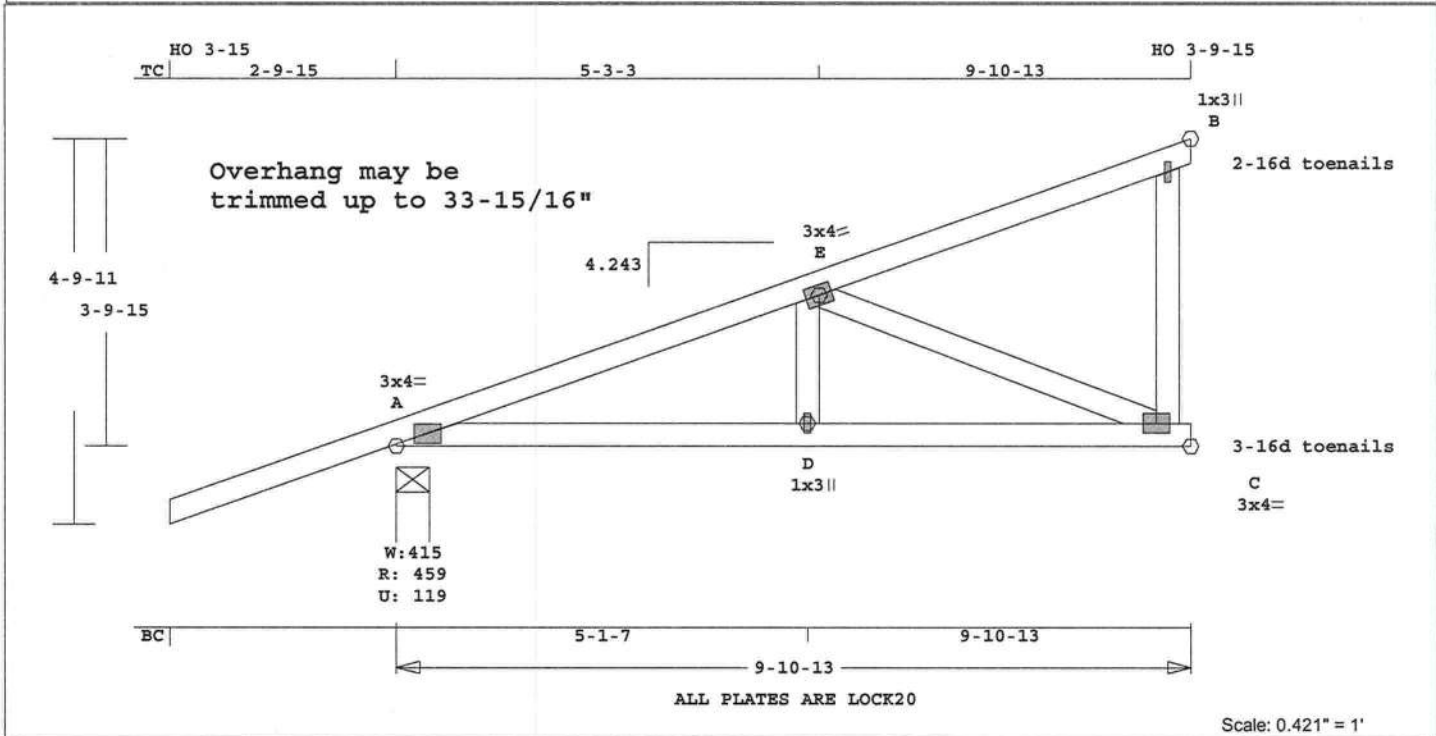
4	03/17/06	J3
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Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 3/17/2006

FWD-HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.9 LBS

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

CSI	-Size-	----Lumber----
TC	0.36	2x 4 SP-#2
BC	0.27	2x 4 SP-#2
WB	0.19	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9-10-13
BC Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Girder Loading	Lumber Duration Factor	Plate Duration Factor	
	1.25	1.25	
plf - Live	Dead	From	To
TC V	40	20	0.0' 9.9'
BC V	0	20	0.0' 9.9'
TC V	-40	-20	0.0' 9.9'
BC V	0	-20	0.0' 9.9'
BC V	0	22	0.0' 9.9'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	459	119	4-15	1- 8
			Hz =	-51
C	336	32	3- 8	1- 8
B	220	97	3- 8	1- 8
			Hz =	148

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -E	0.30	634	C	0.00	0.30
E -B	0.36	92	T	0.00	0.36
-----Bottom Chords-----					
A -D	0.23	614	T	0.07	0.16
D -C	0.27	614	T	0.07	0.20
-----Webs-----					
D -E	0.03	235	T		
E -C	0.19	663	C		
C -B	0.02	0	T	WindLd	

TL Defl -0.05" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in E -B 0.27

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 4.0 Ctr Ctr 0.64
E LOCK 3.0x 4.0 Ctr Ctr 0.43
B LOCK 1.0x 3.0 Ctr Ctr 0.75
D LOCK 1.0x 3.0 Ctr Ctr 0.75
C LOCK 3.0x 4.0 Ctr Ctr 0.54

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

Girder King Jack
Loading TC and BC
Setback 7- 0- 0

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.

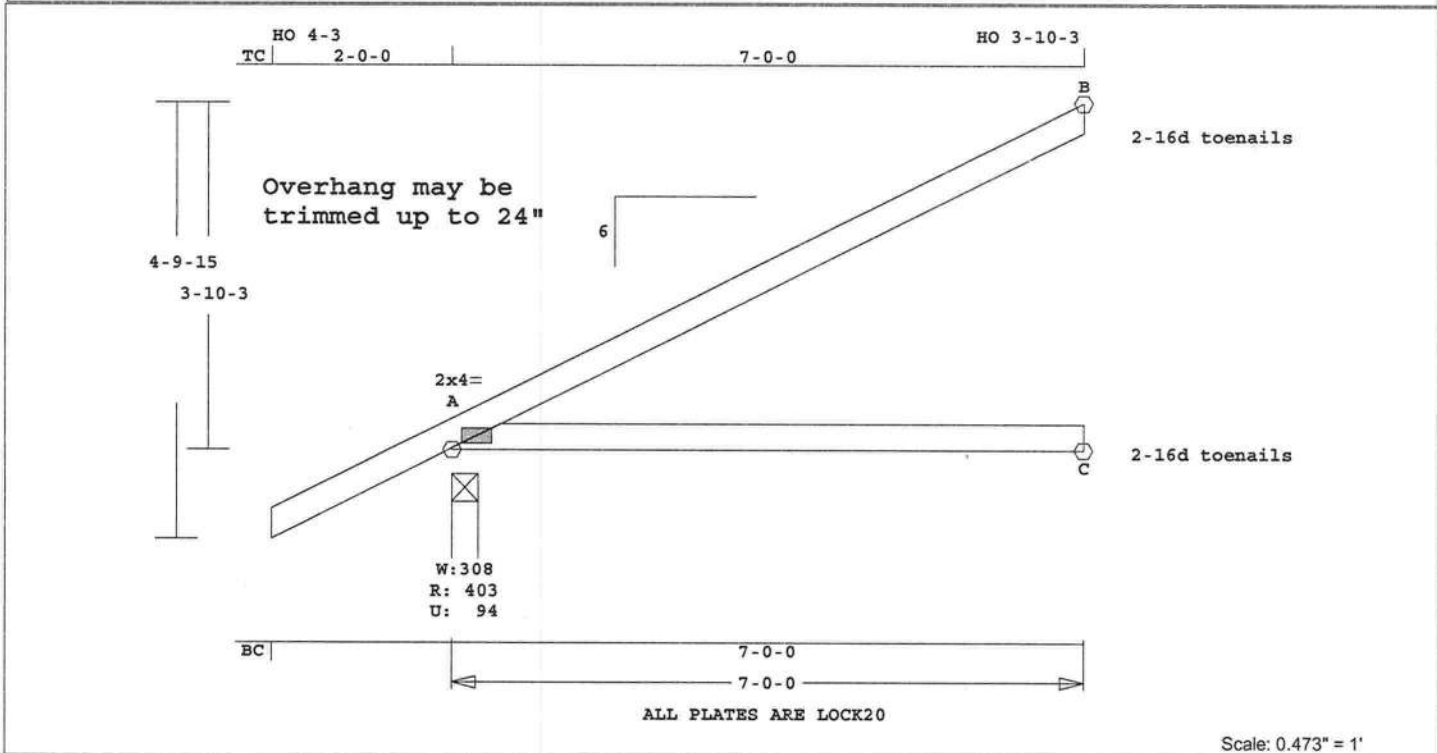
Use properly rated hangers for
loads framing into girder
truss.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 663 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



FWD-HAWKINS1



Scale: 0.473" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.5 LBS
 A -C 0.34 0 T 0.00 0.34 concurrent LL on BC.

Online Plus -- Version 19.0.009
 RUN DATE: 16-MAR-06

CSI -Size- ----Lumber----
 TC 0.47 2x 4 SP-#2
 BC 0.34 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 7- 0- 0
 BC Cont. 0- 0- 0 7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 5 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	404	95	3- 8	1- 8
			Hz =	126
C	132	0	3- 8	1- 8
B	196	108	3- 8	1- 8
			Hz =	86

Membr CSI P Lbs Axl-C SI-Bnd
 -----Top Chords-----
 A -B 0.47 63 C 0.00 0.47
 -----Bottom Chords-----

TL Defl -0.18" in A -C L/427
 LL Defl -0.07" in A -C L/999
 Shear // Grain in A -B 0.22
 Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORT: NER 691
 ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 2.0x 4.0 Ctr Ctr 0.69

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

For proper installation of
 toe-nails, refer to the 2001
 National Design Specification
 (NDS) for Wood Construction

NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-

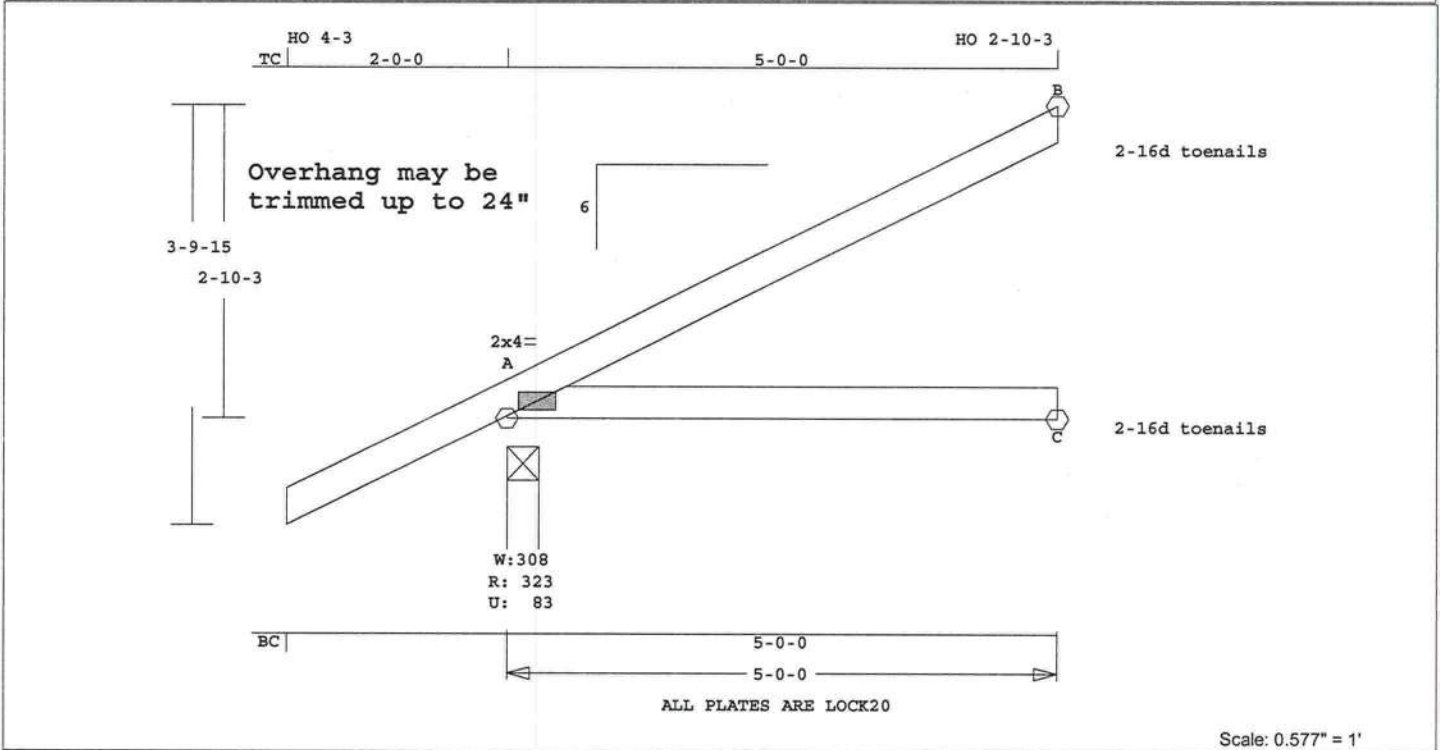
Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 120 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Max comp. force 63 Lbs
 Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
 License #: 62111
 Address: P.O. Box 280055, Tampa, FL 33682



Job FWD-HAWK1	Mark J2	Quan 4	Type JCA2	Span 50000	P1-H1 6	Left OH 2- 0- 0	Right OH 0	Engineering T06031832
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FWD-HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.5 LBS
A -C 0.16 0 T 0.00 0.16

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

CSI -Size- ----Lumber----
TC 0.22 2x 4 SP-#2
BC 0.16 2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 5- 0- 0
BC Cont. 0- 0- 0 5- 0- 0

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	324	83	3- 8	1- 8
			Hz =	90
C	94	0	3- 8	1- 8
B	142	78	3- 8	1- 8
			Hz =	61

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
A -B 0.22 45 C 0.00 0.22
-----Bottom Chords-----

TL Defl -0.04" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.15
Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.65

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

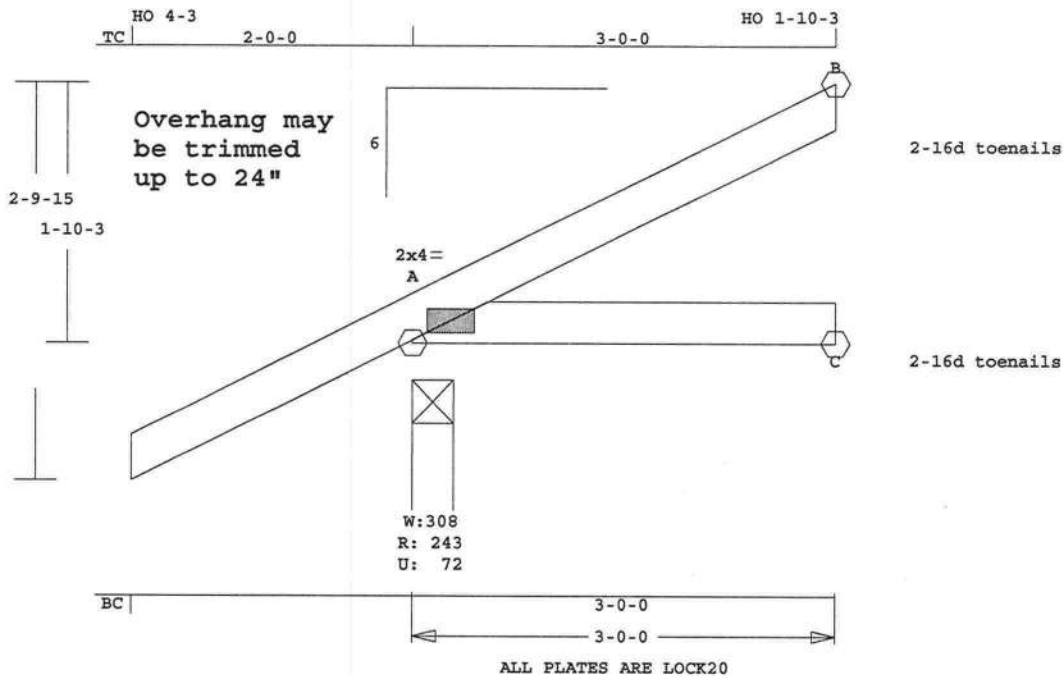
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 45 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Job FWD-HAWK1	Mark J3	Quan 4	Type JCA2	Span 30000	P1-H1 6	Left OH 2- 0- 0	Right OH 0	Engineering T06031832
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FWD-HAWKINS1



Scale: 0.739" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 16.5 LBS
A -C 0.05 0 T 0.00 0.05

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

CSI -Size- ----Lumber----
TC 0.06 2x 4 SP-#2
BC 0.05 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	243	72	3- 8	1- 8
			Hz =	53
C	56	0	3- 8	1- 8
B	88	49	3- 8	1- 8
			Hz =	36

Membr CSI P Lbs Axl-CSt-Bnd
-----Top Chords-----
A -B 0.06 28 C 0.00 0.06
-----Bottom Chords-----

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.08
Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.65

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

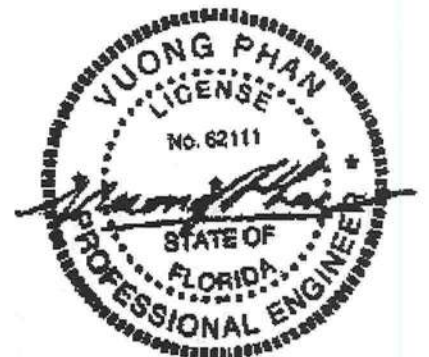
REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

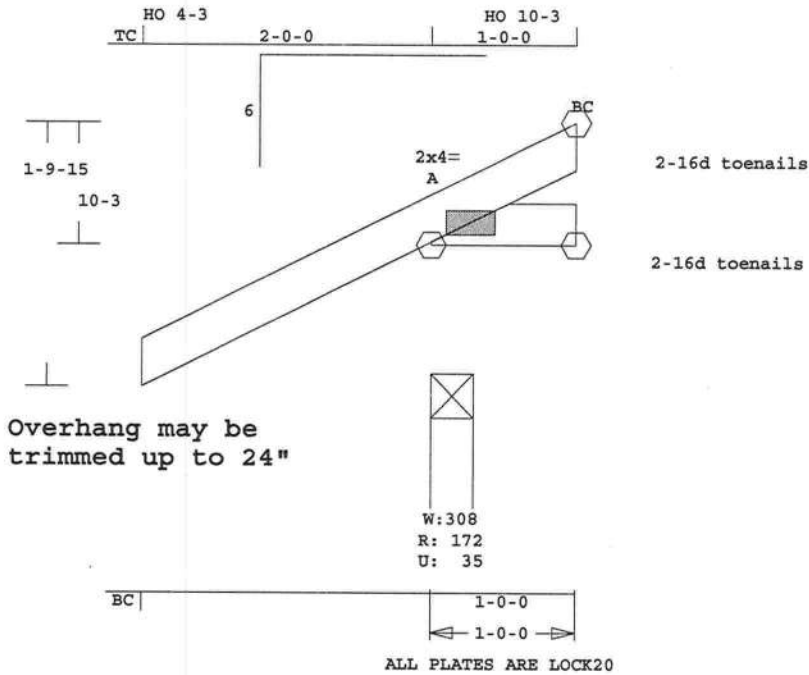
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 28 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



FWD-HAWKINS1



Scale: 0.759" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 8.5 LBS
A -C 0.00 10 T

Online Plus -- Version 19.0.009
RUN DATE: 16-MAR-06

CSI -Size- ----Lumber----
TC 0.00 2x 4 SP-#2
BC 0.00 2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 1- 0- 0
BC Cont. 0- 0- 0 1- 0- 0

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	172	35	3- 8	1- 8
			Hz =	16
B	20	11	1- 8	1- 8
C	14	1	1- 8	1- 8
			Hz =	11

Membr CSI P Lbs Axl-Csi-Bnd
-----Top Chords-----
A -B 0.00 6 C
-----Bottom Chords-----

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.02
Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.65

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

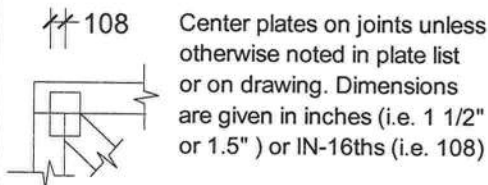
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 6 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

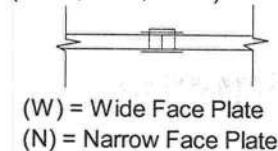


ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

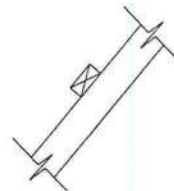
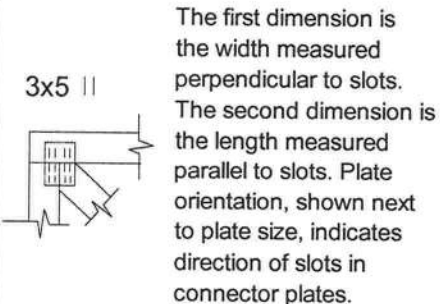
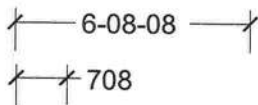


PLATE SIZE AND ORIENTATION



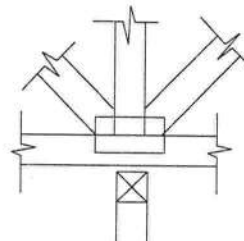
DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.



W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

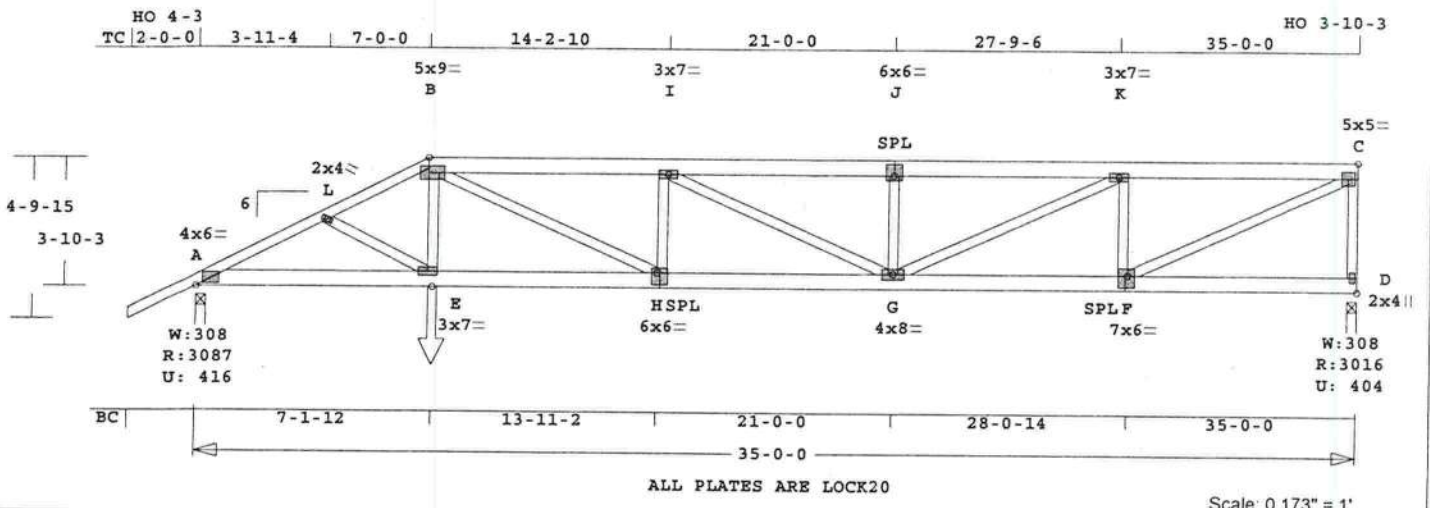


6904 Parke East Blvd.
Tampa, FL 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbseng.com

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
FWDEV-HAWKINS1	A9	1*2P	HHIP	350000	6	2- 0- 0	0	T06080392

U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 287.0 LBS

Online Plus -- Version 19.0.034
 RUN DATE: 03-AUG-06

 * 2-Ply Truss *

CSI	-Size-	-----Lumber----
TC	0.37	2x 6 SP-#2
EX A -B	2x 4	SP-#2
BC	0.63	2x 6 SP-#2
WB	0.55	2x 4 SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 35- 0- 0
 BC Cont. 0- 0- 0 35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Girder Loading					
Lumber Duration Factor 1.25					
Plate Duration Factor 1.25					
plf - Live Dead From To					
TC V	40	20	0.0'	35.0'	
BC V	0	20	0.0'	35.0'	
TC V	50	25	7.0'	34.0'	
TC V	-40	-20	34.0'	35.0'	
BC V	0	25	7.1'	34.0'	
BC V	0	-20	34.0'	35.0'	
BC V	280	280	7.1'	CL-LB	

Plus 6 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	3088	417	3- 8	1-13
			Hz =	-54
D	3016	404	3- 8	1-12
			Hz =	118

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A -L	0.22		6035	C	0.08 0.14
L -B	0.35		5963	C	0.07 0.28
B -I	0.37		8140	C	0.07 0.30
I -J	0.32		7959	C	0.05 0.27
J -K	0.33		7959	C	0.05 0.28

Bottom Chords			
A -E	0.42	5370	T 0.35 0.07
E -H	0.46	5355	T 0.35 0.11
H -G	0.63	8140	T 0.54 0.09
G -F	0.45	5385	T 0.36 0.09
F -D	0.15	92	T 0.00 0.15
-----Webs-----			
L -E	0.00	90	T
E -B	0.07	826	T
B -H	0.28	3067	T
H -I	0.04	946	C
I -G	0.02	201	C
G -J	0.04	881	C
J -K	0.26	2866	T
F -K	0.11	2297	C
D -C	0.55	5996	T
D -C	0.14	2914	C WindLd

TL Defl -0.42" in H -G L/975
 LL Defl -0.21" in H -G L/999
 Shear // Grain in B -I 0.22

Plates for each ply each face.
 PLATING CONFORMS TO TPI.

REPORT: NER 691
 ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 4.0x 6.0 Ctr Ctr 0.72
 L LOCK 2.0x 4.0 Ctr Ctr 0.44
 B LOCK 5.0x 9.0-0.5 Ctr 1.00
 I LOCK 3.0x 7.0 Ctr Ctr 0.45
 J LOCK 6.0x 6.0 Ctr 1.2 0.59
 K LOCK 3.0x 7.0 Ctr Ctr 0.47
 C LOCK 5.0x 5.0 Ctr Ctr 0.89
 E LOCK 3.0x 7.0 Ctr Ctr 0.44
 H LOCK 6.0x 6.0 Ctr-1.2 0.71
 G LOCK 4.0x 8.0 Ctr Ctr 0.55
 F LOCK 7.0x 6.0 0.5-0.8 0.81
 D LOCK 2.0x 4.0 Ctr Ctr 0.80

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

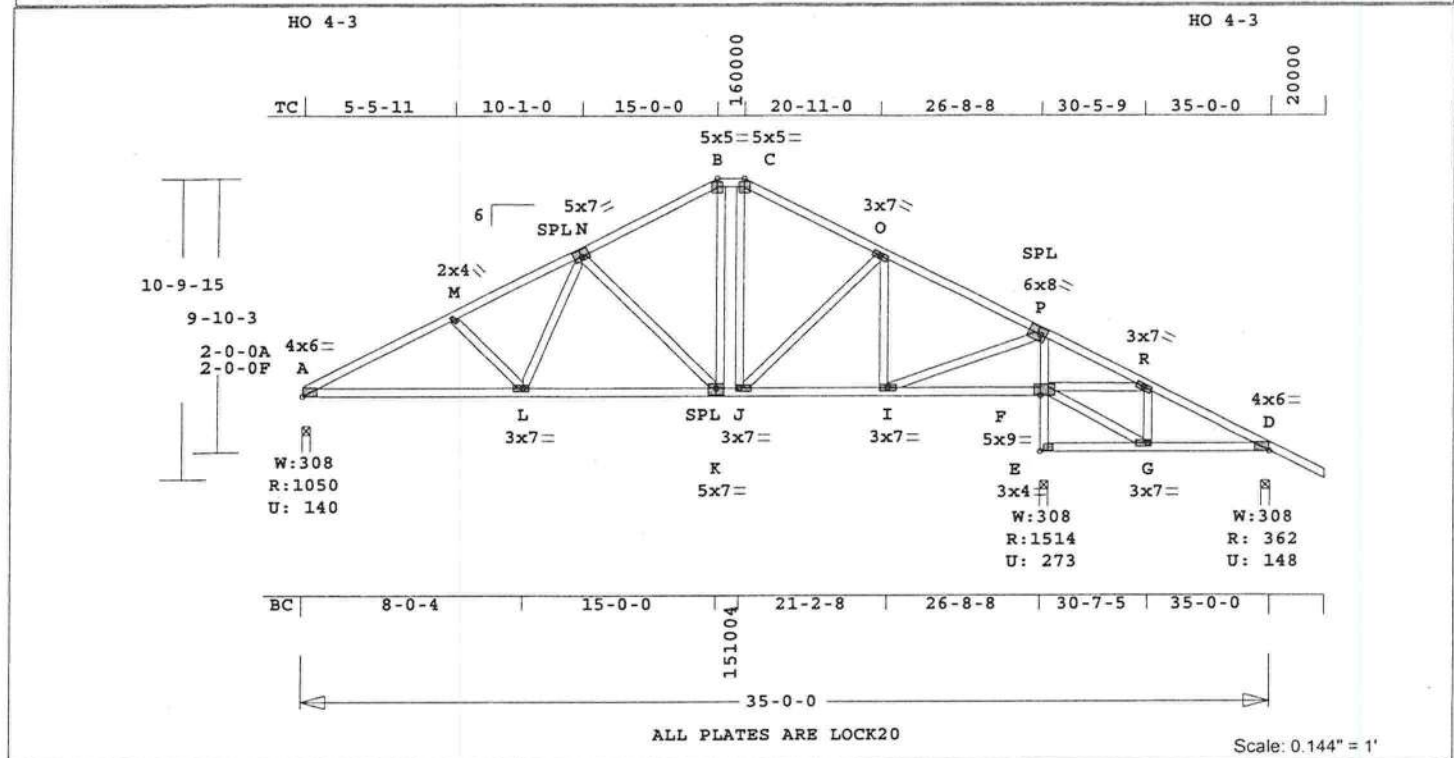
NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:

FBC2004
 Girder Half Hip
 Framing King Jacks
 Jack Open Faced
 Setback 7- 0- 0
 2 COMPLETE TRUSSES REQUIRED.
 Fasten together in staggered
 pattern. (1/2" bolts -OR-
 SDS3 screws -OR- 10d nails
 as each layer is applied.)
 ----Spacing (In)----
 Rows Nails Screws Bolts
 TC 1 12 24 0
 BC 2 12 24 0
 WB 1 8 8
 Plus clusters of nails where
 shown.
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Prevent truss rotation at all
 bearing locations.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Max comp. force 8140 Lbs
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
 License #: 58126
 Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 259.7 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	Size	Lumber
TC	0.31 2x 4	SP-#2
BC	0.49 2x 4	SP-#2
CW	0.13 2x 4	SP-#2
WB	0.36 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	35- 0- 0
BC Cont.	0- 0- 0	35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1051	141	3- 8	1- 8
			Hz =	-177
E	1514	274	3- 8	1- 8
D	363	149	3- 8	1- 8
			Hz =	180

Membr	CSI	P Lbs	Axl	CSI	Bnd
-----Top Chords-----					
A -M	0.22	1799	C	0.03	0.19
M -N	0.24	1585	C	0.01	0.23
N -B	0.23	988	C	0.00	0.23
B -C	0.09	875	C	0.00	0.09
C -O	0.28	986	C	0.00	0.28
O -P	0.28	985	C	0.00	0.28
P -R	0.31	318	T	0.06	0.25
R -D	0.13	164	C	0.00	0.13
-----Bottom Chords-----					
A -L	0.49	1613	T	0.16	0.33
L -K	0.46	1277	T	0.13	0.33
K -J	0.33	875	T	0.14	0.19
J -I	0.33	882	T	0.14	0.19
I -F	0.18	262	C	0.00	0.18

Chord-Webs	Webs
E -G 0.11 32 C 0.00 0.11	M -L 0.05 273 C
G -D 0.12 152 T 0.01 0.11	L -N 0.07 431 T
E -F 0.13 1488 C 0.13 0.00	N -K 0.36 567 C
F -P 0.12 1300 C 0.12 0.00	K -B 0.06 345 T
	J -C 0.03 201 T
	J -O 0.05 95 T
	I -O 0.10 331 C
	I -P 0.22 1219 T
	F -R 0.08 430 C
	F -G 0.03 199 T
	G -R 0.01 111 T

TL Defl	-0.16"	in L -K	L/999
LL Defl	-0.07"	in A -L	L/999
Shear // Grain		in O -P	0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	4.0x 6.0	Ctr	0.1	0.69
M LOCK	2.0x 4.0	Ctr	0.44	
N LOCK	5.0x 7.0	0.2	0.5	0.73
B LOCK	5.0x 5.0	Ctr	0.2	0.99
C LOCK	5.0x 5.0	Ctr	0.2	0.99
O LOCK	3.0x 7.0	Ctr	Ctr	0.43
P LOCK	6.0x 8.0	Ctr	Ctr	0.96
R LOCK	3.0x 7.0	Ctr	Ctr	0.40
D LOCK	4.0x 6.0	Ctr	0.1	0.69
L LOCK	3.0x 7.0	0.9	Ctr	0.54
K LOCK	5.0x 7.0	Ctr	0.5	0.74
J LOCK	3.0x 7.0	Ctr	Ctr	0.45
I LOCK	3.0x 7.0	Ctr	Ctr	0.50
F LOCK	5.0x 9.0	Ctr	0.8	0.63
E LOCK	3.0x 4.0	Ctr	Ctr	0.70
G LOCK	3.0x 7.0	Ctr	Ctr	0.43

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

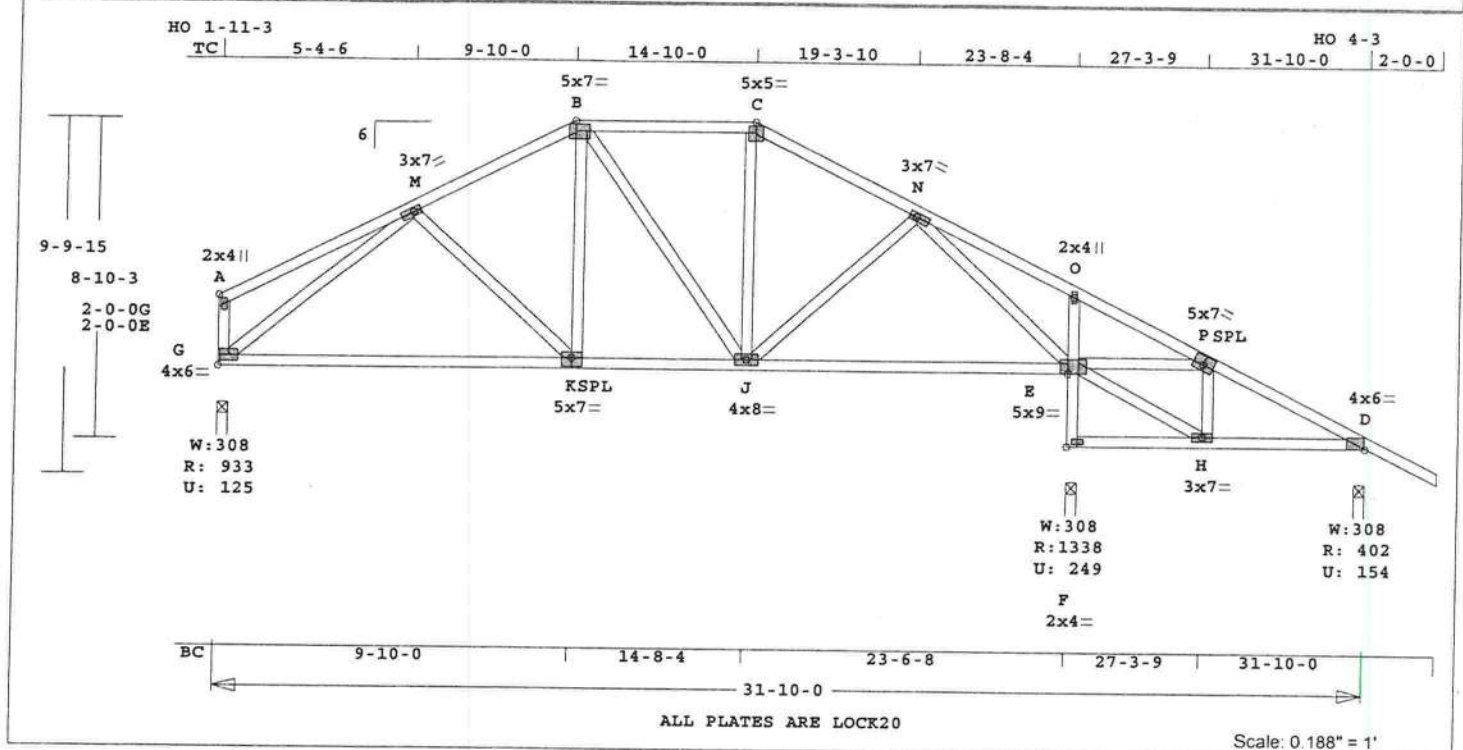
REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC
regions --From-- --To--
26-10- 4 35- 0- 0
Max comp. force 1799 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 243.0 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

TC	CS	Size	Lumber
0.27	2x 4	SP-#2	
0.60	2x 4	SP-#2	
0.26	2x 4	SP-#2	
0.63	2x 4	SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	31-10- 0
BC Cont.	0- 0- 0	31-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
G	934	126	3- 8	1- 8
			Hz =	-176
F	1338	249	3- 8	1- 8
D	402	154	3- 8	1- 8
			Hz =	149

Membr	CSI	P	Lbs	Axl	CSI-Bnd
---Top Chords---					
A -M	0.27		59 T	0.01	0.26
M -B	0.26		931 C	0.00	0.26
B -C	0.21		775 C	0.00	0.21
C -N	0.16		869 C	0.00	0.16
N -O	0.18		109 T	0.02	0.16
O -P	0.20		151 T	0.03	0.17
P -D	0.14		250 C	0.00	0.14
---Bottom Chords---					
G -K	0.60		862 T	0.08	0.52
K -J	0.60		824 T	0.08	0.52
J -E	0.53		694 T	0.06	0.47

F -H	0.10	76 C	0.00	0.10
H -D	0.12	229 T	0.02	0.10
-----Chord-Webs-----				
F -E	0.26	1311 C	0.00	0.26
E -O	0.25	267 C	0.00	0.25
-----Webs-----				
G -A	0.01	138 C	WindLd	
G -M	0.63	1117 C		
M -K	0.02	119 T		
K -B	0.04	305 T		
B -J	0.07	100 C		
J -C	0.02	185 T		
J -N	0.02	163 T		
N -E	0.52	1111 C		
E -P	0.06	364 C		
E -H	0.05	305 T		
H -P	0.01	84 C		

TL Defl	-0.60"	in G -K	L/461
LL Defl	-0.30"	in G -K	L/942
Shear // Grain		in G -K	0.25

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate	LOCK	20 Ga,	Gross Area	
Plate	RHS	20 Ga,	Gross Area	
Jt Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr Ctr	0.45
M	LOCK	3.0x 7.0	0.9-0.5	0.55
B	LOCK	5.0x 7.0	0.5-0.1	0.89
C	LOCK	5.0x 5.0	Ctr-0.2	0.94
N	LOCK	3.0x 7.0	0.9-0.5	0.49
O	LOCK	2.0x 4.0	Ctr Ctr	0.24
P	LOCK	5.0x 7.0	0.2 0.5	0.69
D	LOCK	4.0x 6.0	Ctr 0.1	0.66
G	LOCK	4.0x 6.0	Ctr Ctr	0.67
K	LOCK	5.0x 7.0	Ctr-0.5	0.71
J	LOCK	4.0x 8.0	Ctr Ctr	0.43
E	LOCK	5.0x 9.0	Ctr 0.8	0.61
F	LOCK	2.0x 4.0	Ctr Ctr	0.92
H	LOCK	3.0x 7.0	Ctr Ctr	0.42

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

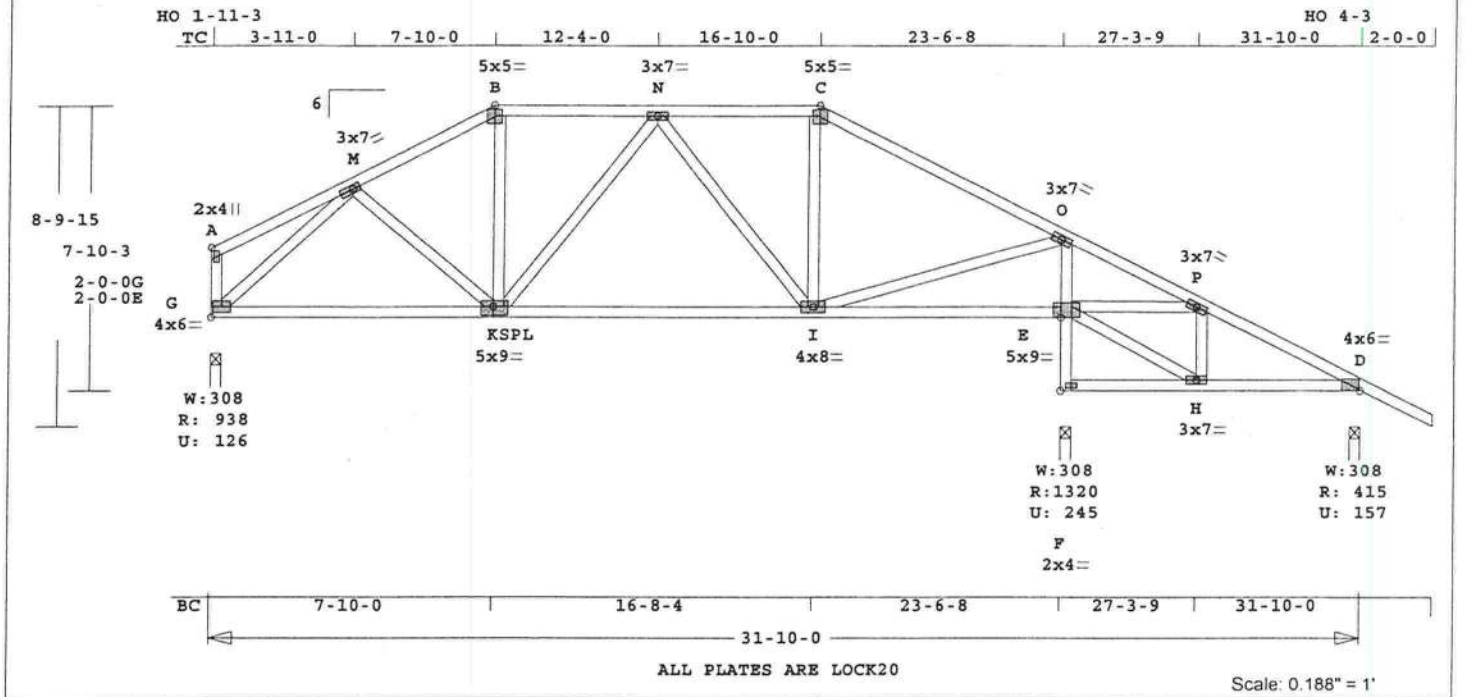
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC
regions --From-- --To--
23- 8- 4 31-10- 0
Max comp. force 1311 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HAWKINS1	A12	1	SP	311000	6	0	2-0-0	T06080392

U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 235.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

	CSI	Size	Lumber
TC	0.41	2x 4	SP-#2
BC	0.54	2x 4	SP-#2
CW	0.11	2x 4	SP-#2
WB	0.37	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	31-10-0
BC Cont.	0-0-0	31-10-0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
G	938	127	3-8	1-8
			Hz =	-155
F	1320	245	3-8	1-8
D	416	157	3-8	1-8
			Hz =	128

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A	-M	0.15	47	T	0.01	0.14
M	-B	0.14	1004	C	0.00	0.14
B	-N	0.17	896	C	0.00	0.17
N	-C	0.17	825	C	0.00	0.17
C	-O	0.40	931	C	0.00	0.40
O	-P	0.41	92	T	0.01	0.40
P	-D	0.14	269	C	0.00	0.14
-----Bottom Chords-----						
G	-K	0.52	815	T	0.07	0.45
K	-I	0.54	968	T	0.09	0.45
I	-E	0.32	63	T	0.00	0.32

	F-H	0.10	34	C	0.00	0.10
H-D	0.13	243	T	0.04	0.09	
-----Chord-Webs-----						
F-E	0.11	1294	C	0.11	0.00	
E-O	0.11	1069	C	0.00	0.11	
-----Webs-----						
G-A	0.01	101	C	WindLd		
G-M	0.37	1123	C			
M-K	0.02	173	T			
K-B	0.04	263	T			
K-N	0.07	115	C			
N-I	0.14	231	C			
I-C	0.03	203	T			
I-O	0.16	890	T			
E-P	0.05	287	C			
E-H	0.05	298	T			
H-P	0.01	74	T			

TL Defl -0.20" in G-K L/999
LL Defl -0.10" in G-K L/999
Shear // Grain in C-O 0.25

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691

ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.45
M	LOCK	3.0x 7.0-0.8-0.4	0.51		
B	LOCK	5.0x 5.0	Ctr-0.2	0.94	
N	LOCK	3.0x 7.0	Ctr	Ctr	0.46
C	LOCK	5.0x 5.0	Ctr-0.2	0.94	
O	LOCK	3.0x 7.0	Ctr	Ctr	0.47
P	LOCK	3.0x 7.0	Ctr	Ctr	0.39
D	LOCK	4.0x 6.0	Ctr	0.1	0.66
G	LOCK	4.0x 6.0	Ctr	Ctr	0.67
K	LOCK	5.0x 9.0	0.5-0.5	0.71	
I	LOCK	4.0x 8.0	Ctr	Ctr	0.52
E	LOCK	5.0x 9.0	Ctr	0.8	0.61
F	LOCK	2.0x 4.0	Ctr	Ctr	0.91
H	LOCK	3.0x 7.0	Ctr	Ctr	0.42

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

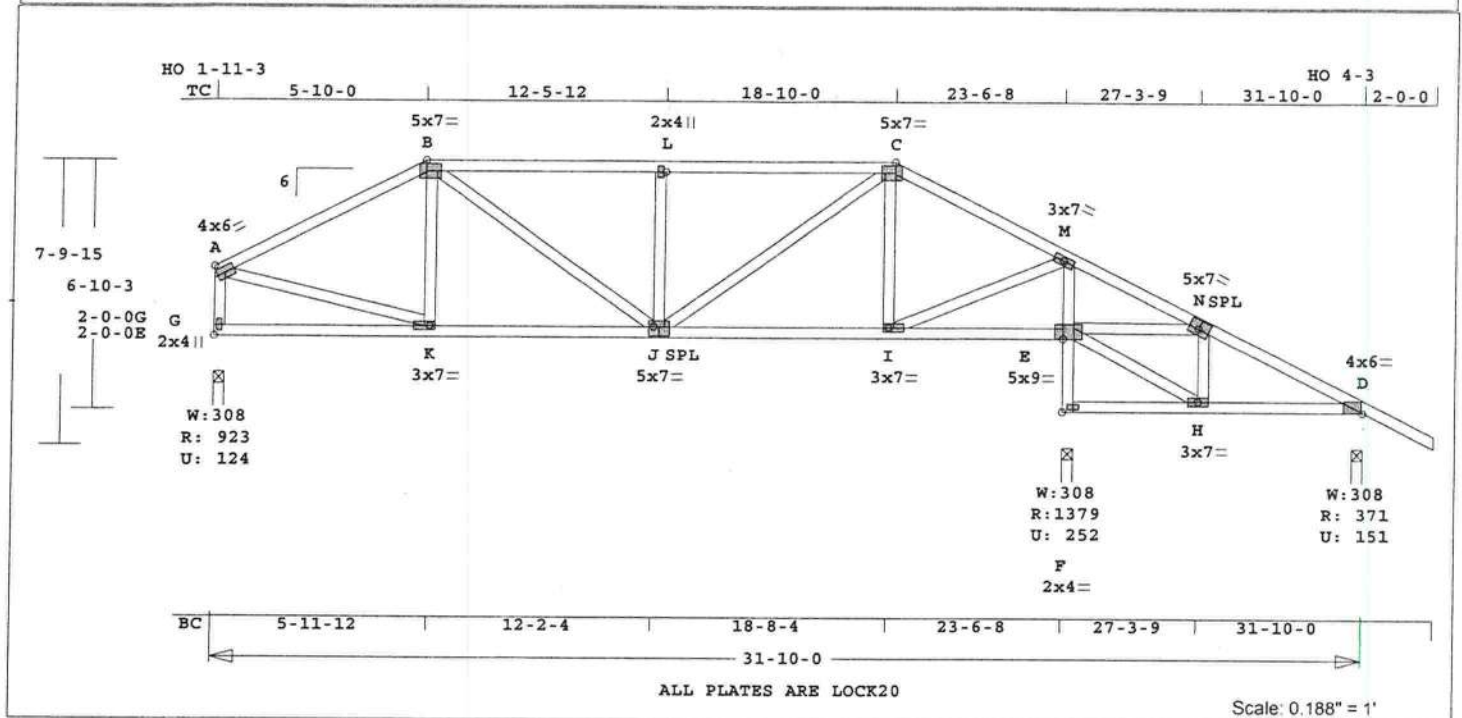
REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
User-defined wind-exposed BC
regions --From-- --To--
23- 8- 4 31-10- 0
Max comp. force 1294 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 229.8 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	-Size-	-----Lumber-----
TC	0.41	2x 4 SP-#2
BC	0.32	2x 4 SP-#2
CW	0.12	2x 4 SP-#2
WB	0.19	2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 31-10- 0
BC Cont. 0- 0- 0 31-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
G	923	125	3- 8	1- 8
			Hz =	-135
F	1380	253	3- 8	1- 8
D	371	152	3- 8	1- 8
			Hz =	107

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.37	1005	C	0.01 0.36
B -L	0.41	1195	C	0.01 0.40
L -C	0.41	1195	C	0.01 0.40
C -M	0.21	804	C	0.00 0.21
M -N	0.26	300	T	0.05 0.21
N -D	0.14	184	C	0.00 0.14
-----Bottom Chords-----				
G -K	0.23	114	T	0.00 0.23
K -J	0.32	909	T	0.09 0.23
J -I	0.27	721	T	0.07 0.20
I -E	0.17	257	C	0.00 0.17

F	-H	0.10	20	C	0.00	0.10
H	-D	0.11	171	T	0.02	0.09
-----Chord-Webs-----						
F	-E	0.12	1353	C	0.12	0.00
E	-M	0.10	1177	C	0.10	0.00
-----Webs-----						
G	-A	0.08	874	C	WindLd	
A	-K	0.17	944	T		
K	-B	0.03	127	C		
B	-J	0.06	349	T		
J	-L	0.10	407	C		
J	-C	0.10	579	T		
I	-C	0.07	297	C		
I	-M	0.19	1055	T		
E	-N	0.08	437	C		
E	-H	0.03	211	T		
H	-N	0.01	112	T		

TL Defl -0.09" in J -I L/999
LL Defl -0.04" in J -I L/999
Shear // Grain in B -L 0.26

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	0.1	Ctr	0.67
B	LOCK	5.0x	7.0	0.5-0.1	0.89	
L	LOCK	2.0x	4.0	Ctr	Ctr	0.42
C	LOCK	5.0x	7.0	0.5-0.1	0.89	
M	LOCK	3.0x	7.0	Ctr	Ctr	0.52
N	LOCK	5.0x	7.0	0.2	0.5	0.69
D	LOCK	4.0x	6.0	Ctr	0.1	0.66
G	LOCK	2.0x	4.0	Ctr	Ctr	0.45
K	LOCK	3.0x	7.0	Ctr	Ctr	0.85
J	LOCK	5.0x	7.0	Ctr	-0.5	0.71
I	LOCK	3.0x	7.0	Ctr	Ctr	0.46
E	LOCK	5.0x	9.0	Ctr	0.8	0.61
F	LOCK	2.0x	4.0	Ctr	Ctr	0.95
H	LOCK	3.0x	7.0	Ctr	Ctr	0.42

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

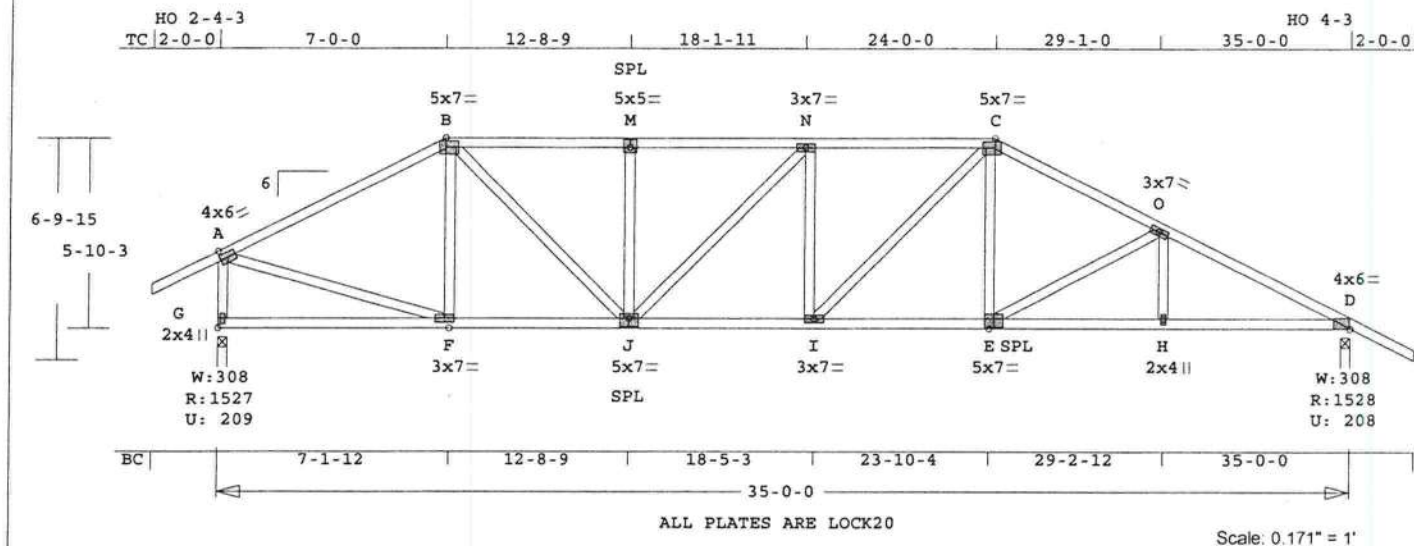
REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
User-defined wind-exposed BC
regions --From-- ---To---
23- 8- 4 31-10- 0
Max comp. force 1353 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 259.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	-Size-	----	Lumber----
TC	0.65	2x 4	SP-#2
BC	0.47	2x 4	SP-#2
WB	0.27	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	35- 0- 0
BC Cont.	0- 0- 0	35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
G	1528	209	3- 8	1-13
			Hz =	-144
D	1528	208	3- 8	1-13
			Hz =	107

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----					
A -B	0.65	1593	C	0.05	0.60
B -M	0.29	2031	C	0.02	0.27
M -N	0.33	2031	C	0.02	0.31
N -C	0.34	2203	C	0.03	0.31
C -O	0.25	2120	C	0.02	0.23
O -D	0.26	2550	C	0.07	0.19
-----Bottom Chords-----					
G -F	0.28	117	T	0.00	0.28
F -J	0.43	1438	T	0.15	0.28
J -I	0.45	2203	T	0.36	0.09
I -E	0.40	1892	T	0.31	0.09
E -H	0.47	2278	T	0.38	0.09

-----Webs-----			
G -A	0.14	1339	C WindLd
A -F	0.27	1497	T
F -B	0.10	283	C
B -J	0.15	831	T
J -M	0.12	314	C
J -N	0.18	242	C
I -N	0.07	191	C
I -C	0.08	435	T
E -C	0.05	373	T
E -O	0.19	433	C
H -O	0.03	206	T

TL Defl -0.24" in I -E L/999
LL Defl -0.11" in I -E L/999
Shear // Grain in A -B 0.25

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	- LOCK	20 Ga,	Gross Area
Plate	- RHS	20 Ga,	Gross Area
Jt	Type	Plt Size	X Y JSI
A	LOCK	4.0x 6.0	0.1 Ctr 0.73
B	LOCK	5.0x 7.0	0.5-0.1 0.93
M	LOCK	5.0x 5.0	Ctr 0.5 0.74
N	LOCK	3.0x 7.0	Ctr Ctr 0.45
C	LOCK	5.0x 7.0	0.5-0.1 0.93
O	LOCK	3.0x 7.0	Ctr Ctr 0.40
D	LOCK	4.0x 6.0	Ctr 0.1 0.69
G	LOCK	2.0x 4.0	Ctr Ctr 0.49
F	LOCK	3.0x 7.0	Ctr Ctr 0.53
J	LOCK	5.0x 7.0	Ctr-0.5 0.74
I	LOCK	3.0x 7.0	Ctr Ctr 0.45
E	LOCK	5.0x 7.0	Ctr-0.5 0.74
H	LOCK	2.0x 4.0	Ctr Ctr 0.44

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

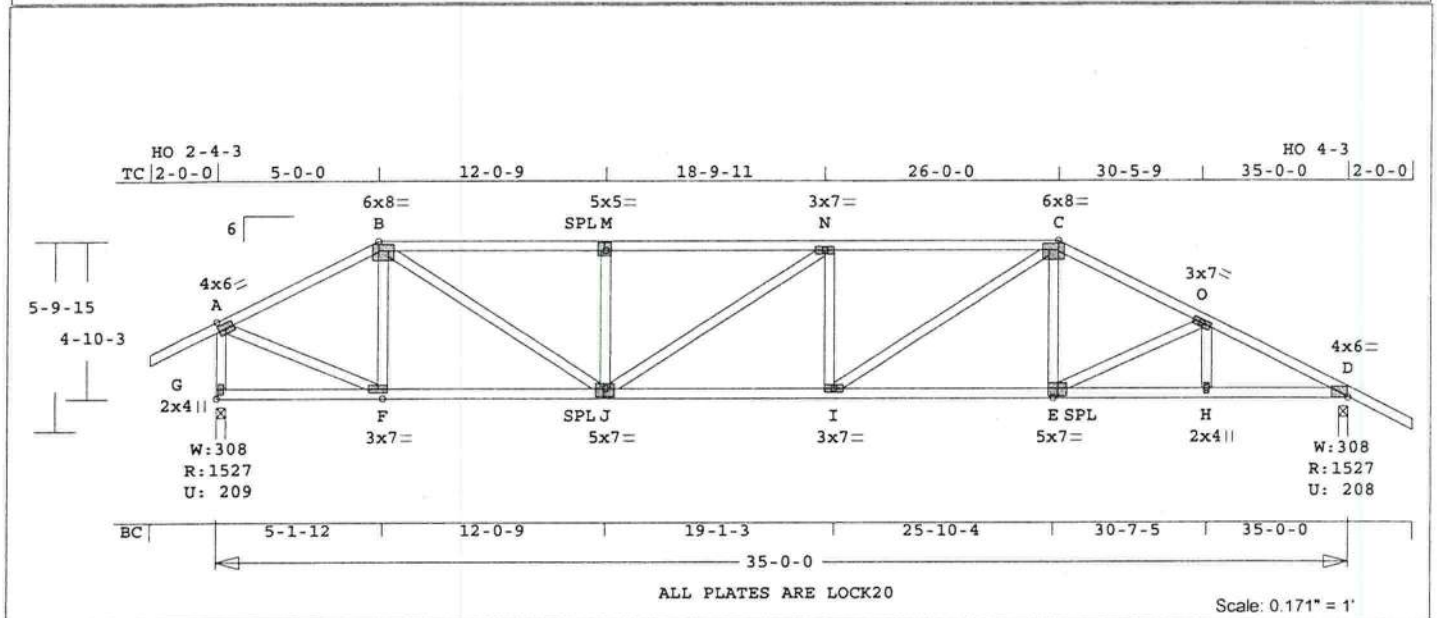
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 2550 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
FWDEV-HAWKINS1	A15	1	HIPP	350000	6	2- 0- 0	2- 0- 0	T06080392

U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 247.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	Size	Lumber
TC	0.52 2x 4	SP-#2
BC	0.58 2x 4	SP-#2
WB	0.26 2x 4	SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 35- 0- 0
BC Cont. 0- 0- 0 35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
G	1528	209	3- 8	1-13
			Hz =	-123
D	1528	208	3- 8	1-13
			Hz =	85

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----					
A -B	0.28	1478	C	0.02	0.26
B -M	0.45	2446	C	0.04	0.41
M -N	0.50	2446	C	0.04	0.46
N -C	0.52	2696	C	0.10	0.42
C -O	0.29	2292	C	0.03	0.26
O -D	0.20	2597	C	0.04	0.16
-----Bottom Chords-----					
G -F	0.22	96	T	0.00	0.22
F -J	0.38	1338	T	0.14	0.24
J -I	0.58	2696	T	0.45	0.13
I -E	0.47	2055	T	0.34	0.13
E -H	0.50	2313	T	0.38	0.12
H -D	0.49	2313	T	0.38	0.11

-----Webs-----			
G -A	0.14	1360	C WindLd
A -F	0.26	1441	T
F -B	0.11	428	C
B -J	0.24	1319	T
J -M	0.10	413	C
J -N	0.25	299	C
I -N	0.07	274	C
I -C	0.14	763	T
E -C	0.05	335	T
E -O	0.09	278	C
H -O	0.02	132	T

TL Defl -0.32" in J -I L/999
LL Defl -0.15" in J -I L/999
Shear // Grain in N -C 0.29

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	LOCK	20 Ga, Gross Area			
Plate	RHS	20 Ga, Gross Area			
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.1	Ctr	0.73
B	LOCK	6.0x 8.0	Ctr	-0.6	0.93
M	LOCK	5.0x 5.0	Ctr	0.5	0.74
N	LOCK	3.0x 7.0	Ctr	Ctr	0.42
C	LOCK	6.0x 8.0	Ctr	-0.6	0.93
O	LOCK	3.0x 7.0	Ctr	Ctr	0.40
D	LOCK	4.0x 6.0	Ctr	0.1	0.69
G	LOCK	2.0x 4.0	Ctr	Ctr	0.49
F	LOCK	3.0x 7.0	Ctr	Ctr	0.47
J	LOCK	5.0x 7.0	0.5-0.5	Ctr	0.74
I	LOCK	3.0x 7.0	Ctr	Ctr	0.42
E	LOCK	5.0x 7.0	Ctr	-0.5	0.74
H	LOCK	2.0x 4.0	Ctr	Ctr	0.44

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

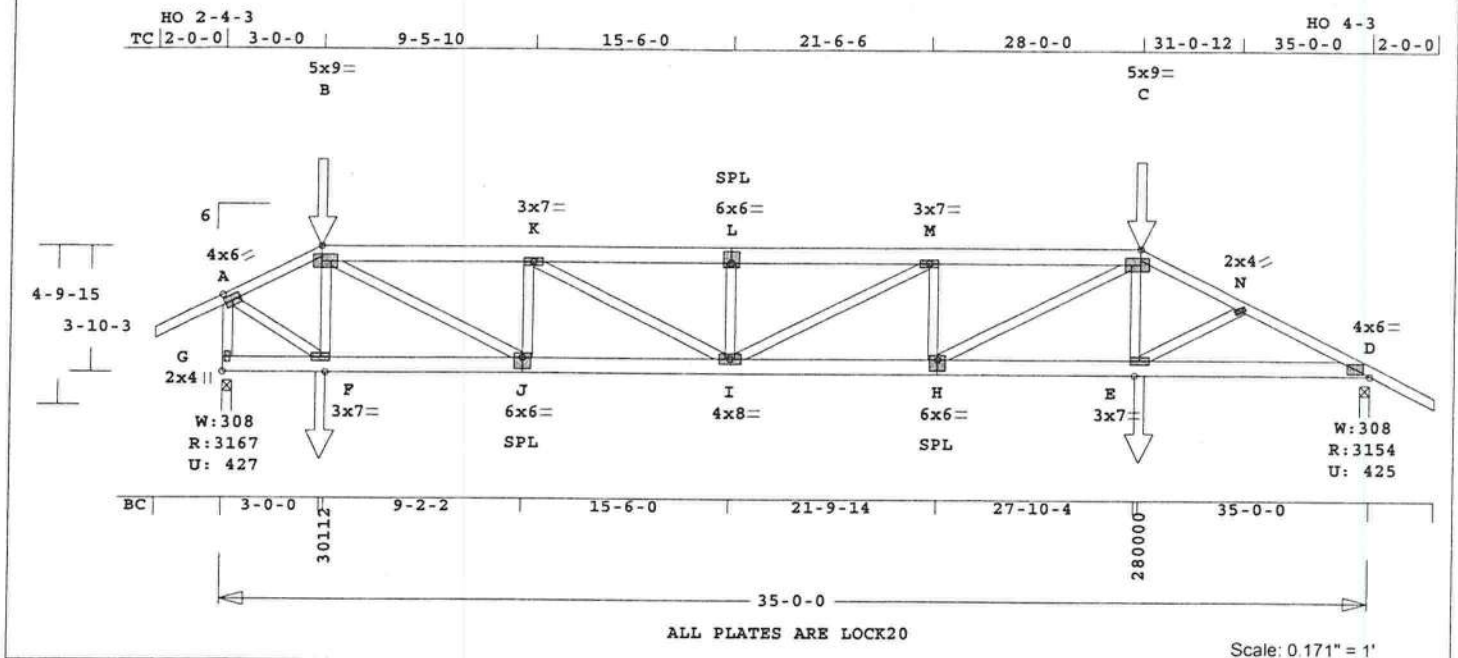
NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 2696 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 293.4 LBS

Online Plus -- Version 19.0.034
 RUN DATE: 03-AUG-06

 * 2-Ply Truss *

CSI	Size	Lumber
TC	0.32 2x 4	SP-#2
EX B	-L 2x 6	SP-#2
EX L	-C 2x 6	SP-#2
BC	0.61 2x 6	SP-#2
WB	0.40 2x 4	SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 35- 0- 0
 BC Cont. 0- 0- 0 35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Standard Loading

Lumber Duration Factor	1.25
Plate Duration Factor	1.25
plf - Live Dead From To	
TC V	40 20 0.0' 35.0'
BC V	0 20 0.0' 35.0'
TC V	50 25 3.1' 27.9'
BC V	0 25 3.1' 27.9'
TC V	43 43 3.0' CL-LB
BC V	29 29 3.0' CL-LB
TC V	129 129 28.0' CL-LB
BC V	196 196 28.0' CL-LB

Plus 6 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
G	3167	428	3- 8	1-14
			Hz =	-98
D	3155	425	3- 8	1-14
			Hz =	63

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.14		2813	C	0.01 0.13
B -K	0.22		6442	C	0.03 0.19
K -L	0.27		8245	C	0.07 0.20
L -M	0.25		8245	C	0.05 0.20
M -C	0.30		8039	C	0.06 0.24

Member	CSI	Size	Material	Area	Length	Weight
-----Bottom Chords-----						
G -F	0.10	72	T	0.00	0.10	
F -J	0.27	2550	T	0.17	0.10	
J -I	0.53	6442	T	0.43	0.10	
I -H	0.61	8038	T	0.53	0.08	
H -E	0.45	5477	T	0.36	0.09	
E -D	0.43	5502	T	0.36	0.07	
-----Webs-----						
G -A	0.14	3072	C	WindLd		
A -F	0.27	3019	T			
F -B	0.06	1366	C			
B -J	0.40	4383	T			
J -K	0.08	1777	C			
K -I	0.18	2056	T			
I -L	0.04	827	C			
L -M	0.02	235	T			
M -H	0.05	1024	C			
H -C	0.26	2884	T			
E -C	0.05	691	T			
E -N	0.00	69	T			

TL Defl -0.41" in I -H L/999
 LL Defl -0.20" in I -H L/999
 Shear // Grain in M -C 0.19

Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORT: NER 691
 ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 4.0x 6.0 0.1 Ctr 0.73
 B LOCK 5.0x 9.0-0.5 Ctr 1.00
 K LOCK 3.0x 7.0 Ctr Ctr 0.44
 L LOCK 6.0x 6.0 Ctr 1.2 0.59
 M LOCK 3.0x 7.0 Ctr Ctr 0.44
 C LOCK 5.0x 9.0 0.5 Ctr 1.00
 N LOCK 2.0x 4.0 Ctr Ctr 0.44
 D LOCK 4.0x 6.0 Ctr Ctr 0.72
 G LOCK 2.0x 4.0 Ctr Ctr 0.80
 F LOCK 3.0x 7.0 Ctr Ctr 0.52
 J LOCK 6.0x 6.0 Ctr-1.2 0.73
 I LOCK 4.0x 8.0 Ctr Ctr 0.46
 H LOCK 6.0x 6.0 Ctr-1.2 0.70
 E LOCK 3.0x 7.0 Ctr Ctr 0.44

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

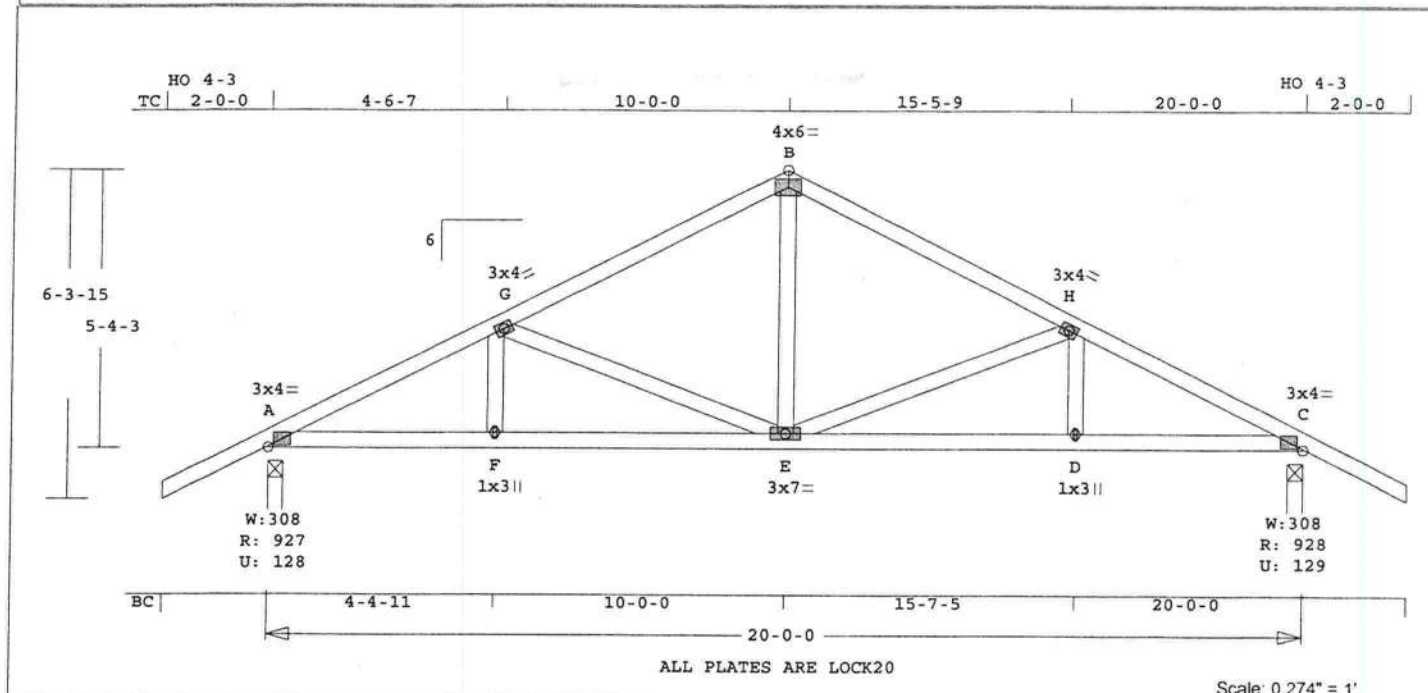
REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004
 2 COMPLETE TRUSSES REQUIRED.
 Fasten together in staggered
 pattern. (1/2" bolts -OR-
 SDS3 screws -OR- 10d nails
 as each layer is applied.)
 ----Spacing (In)----
 Rows Nails Screws Bolts
 TC 1 12 24 0
 BC 2 12 24 0
 WB 1 8 8
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Prevent truss rotation at all
 bearing locations.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load: 5.0 psf
 BC Dead Load: 5.0 psf
 Max comp. force 8245 Lbs
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
 License #: 58126
 Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 127.0 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	Size	Lumber
TC	0.23 2x 4	SP-#2
BC	0.32 2x 4	SP-#2
WB	0.20 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 0- 0
BC Cont.	0- 0- 0	20- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	928	129	3- 8	1- 8
			Hz =	-91
C	928	129	3- 8	1- 8
			Hz =	92

Membr CSI P Lbs Axl-Csi-Bnd

-----Top Chords-----					
A -G	0.22	1370	C	0.01	0.21
G -B	0.23	905	C	0.01	0.22
B -H	0.23	905	C	0.01	0.22
H -C	0.22	1370	C	0.01	0.21

-----Bottom Chords-----					
A -F	0.24	1232	T	0.20	0.04
F -E	0.32	1232	T	0.20	0.12
E -D	0.32	1232	T	0.20	0.12
D -C	0.24	1232	T	0.20	0.04

-----Webs-----					
F -G	0.02	184	T		
G -E	0.20	455	C		
E -B	0.08	492	T		
E -H	0.20	455	C		
D -H	0.02	184	T		

TL Defl -0.09" in E -D L/999
LL Defl -0.04" in E -D L/999
Shear // Grain in G -B 0.19

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 4.0 Ctr Ctr 0.76
G LOCK 3.0x 4.0 Ctr Ctr 0.57
B LOCK 4.0x 6.0 Ctr Ctr 0.55
H LOCK 3.0x 4.0 Ctr Ctr 0.57
C LOCK 3.0x 4.0 Ctr Ctr 0.76
F LOCK 1.0x 3.0 Ctr Ctr 0.81
E LOCK 3.0x 7.0 Ctr Ctr 0.51
D LOCK 1.0x 3.0 Ctr Ctr 0.81

REVIEWED BY:
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PO Box 280055
Tampa, FL 33682

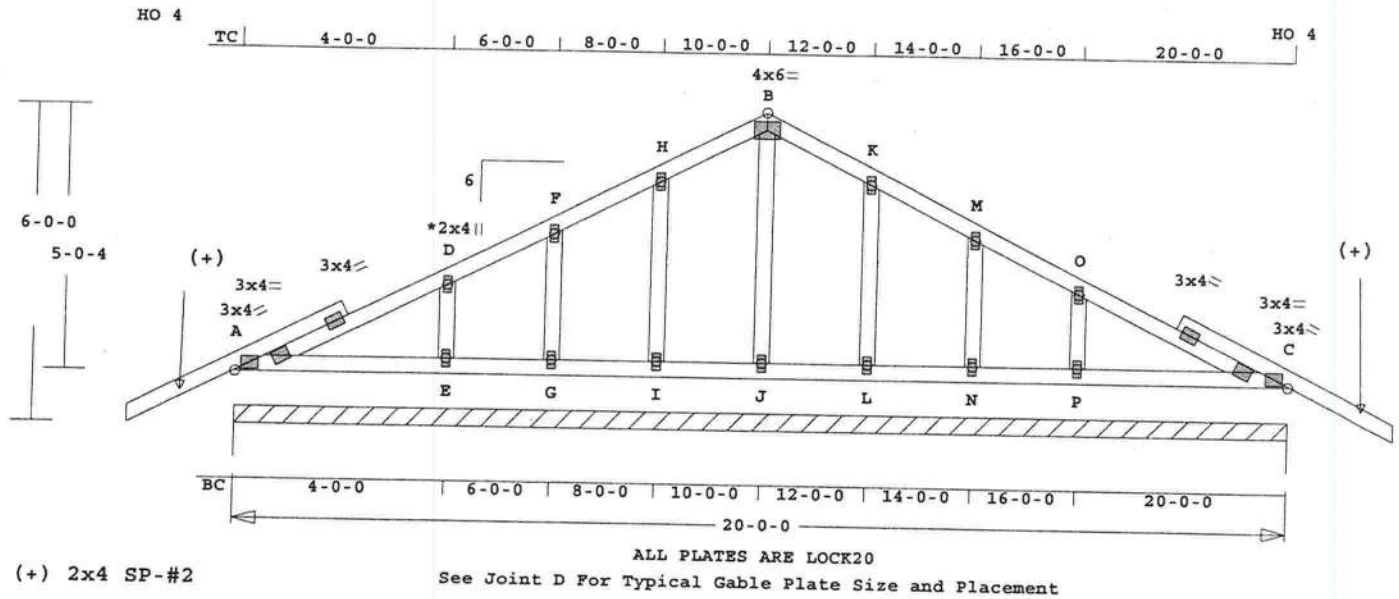
REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1370 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 131.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

	CSI	Size	Lumber
TC	0.09	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
GW	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 0- 0
BC Cont.	0- 0- 0	20- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 20-	0- 0	0- 0
	1600	213	Hz =	86

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -D	0.09	122 C	0.00	0.09
D -F	0.09	144 C	0.00	0.09
F -H	0.03	135 C	0.00	0.03
H -B	0.03	139 C	0.00	0.03
B -K	0.03	139 C	0.00	0.03
K -M	0.03	135 C	0.00	0.03
M -O	0.09	144 C	0.00	0.09
O -C	0.09	122 C	0.00	0.09
-----Bottom Chords-----				
A -E	0.07	4 T	0.00	0.07
E -G	0.06	0 T	0.00	0.06
G -I	0.02	0 T	0.00	0.02
I -J	0.02	0 T	0.00	0.02
J -L	0.02	0 T	0.00	0.02

L -N	0.02	0 T	0.00	0.02	
N -P	0.06	0 T	0.00	0.06	
P -C	0.07	4 T	0.00	0.07	
-----Gable Webs-----					
E -D	0.02	201 C			
G -F	0.01	98 C			
I -H	0.02	125 C			
J -B	0.01	44 T			
L -K	0.02	125 C			
N -M	0.01	98 C			
P -O	0.02	201 C			
TL Defl	-0.01"	in A -E	L/999		
LL Defl	0.00"	in A -E	L/999		
Shear // Grain	in A -D	0.13			

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 4.0 Ctr Ctr 0.76
D LOCK 2.0x 4.0 Ctr Ctr 0.00
F LOCK 2.0x 4.0 Ctr Ctr 0.00
H LOCK 2.0x 4.0 Ctr Ctr 0.00
B LOCK 4.0x 6.0 Ctr Ctr 0.55
K LOCK 2.0x 4.0 Ctr Ctr 0.00
M LOCK 2.0x 4.0 Ctr Ctr 0.00
O LOCK 2.0x 4.0 Ctr Ctr 0.00
C LOCK 3.0x 4.0 Ctr Ctr 0.76
E LOCK 2.0x 4.0 Ctr Ctr 0.00
G LOCK 2.0x 4.0 Ctr Ctr 0.00
I LOCK 2.0x 4.0 Ctr Ctr 0.00
J LOCK 2.0x 4.0 Ctr Ctr 0.00
L LOCK 2.0x 4.0 Ctr Ctr 0.00
N LOCK 2.0x 4.0 Ctr Ctr 0.00
P LOCK 2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR

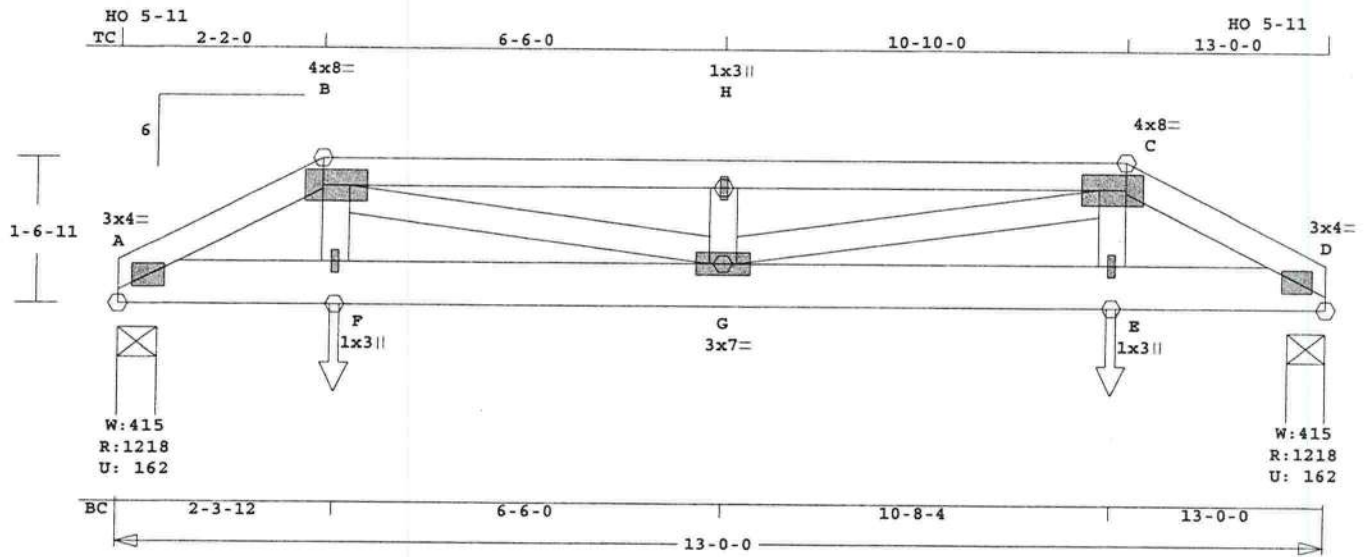
ADDITIONAL SPECIFICATIONS.
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe
member between outside of
truss and first tie-plate
to inside of heel plate.
Design checked for 10 psf non-
concurrent LL on BC.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 201 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/3/2006

U# J#FWDEV-HAWKINS1 HAWKINS1



Scale: 0.485" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 82.5 LBS

Online Plus -- Version 19.0.034
 RUN DATE: 03-AUG-06

 * 2-Ply Truss *

CSI	Size	Lumber
TC	0.12 2x 4	SP-#2
BC	0.26 2x 6	SP-#2
WB	0.11 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	13- 0- 0
BC Cont.	0- 0- 0	13- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading

Lumber Duration Factor	1.25			
Plate Duration Factor	1.25			
plf - Live	Dead	From	To	
TC V	40	20	0.0'	13.0'
BC V	0	20	0.0'	13.0'
BC V	50	50	0.0'	13.0'
TC V	2	1	2.2'	10.8'
BC V	0	1	2.3'	10.7'
BC V	17	17	2.3'	CL-LB
BC V	17	17	10.7'	CL-LB

Plus 6 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1218	162	4-15	1- 8
			Hz =	-16
D	1218	162	4-15	1- 8
			Hz =	17

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----				
A -B	0.03	2069	C	0.00 0.03
B -H	0.12	3068	C	0.02 0.10
H -C	0.12	3068	C	0.02 0.10
C -D	0.03	2069	C	0.00 0.03
-----Bottom Chords-----				
A -F	0.17	1838	T	0.12 0.05
F -G	0.26	1806	T	0.12 0.14
G -E	0.26	1806	T	0.12 0.14
E -D	0.17	1838	T	0.12 0.05
-----Webs-----				
F -B	0.03	490	T	
B -G	0.11	1296	T	
G -H	0.00	195	C	
G -C	0.11	1296	T	
E -C	0.03	490	T	

TL Defl -0.08" in F -G L/999
 LL Defl -0.04" in F -G L/999
 Shear // Grain in F -G 0.12

Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORT: NER 691
 ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 3.0x 4.0 Ctr Ctr 0.73
 B LOCK 4.0x 8.0 Ctr Ctr 0.73
 H LOCK 1.0x 3.0 Ctr Ctr 0.75
 C LOCK 4.0x 8.0 Ctr Ctr 0.73
 D LOCK 3.0x 4.0 Ctr Ctr 0.73
 F LOCK 1.0x 3.0 Ctr Ctr 0.75
 G LOCK 3.0x 7.0 Ctr Ctr 0.59
 E LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.

Analysis Conforms To:
 FBC2004
 Girder Step Down Hip
 Framing King Jacks
 Jack Open Faced
 Setback 2- 2- 0
 2 COMPLETE TRUSSES REQUIRED.
 Fasten together in staggered
 pattern. (1/2" bolts -OR-
 SDS3 screws -OR- 10d nails
 as each layer is applied.)
 -----Spacing (In)-----

Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 2	12	24	0
WB 1	8	8	

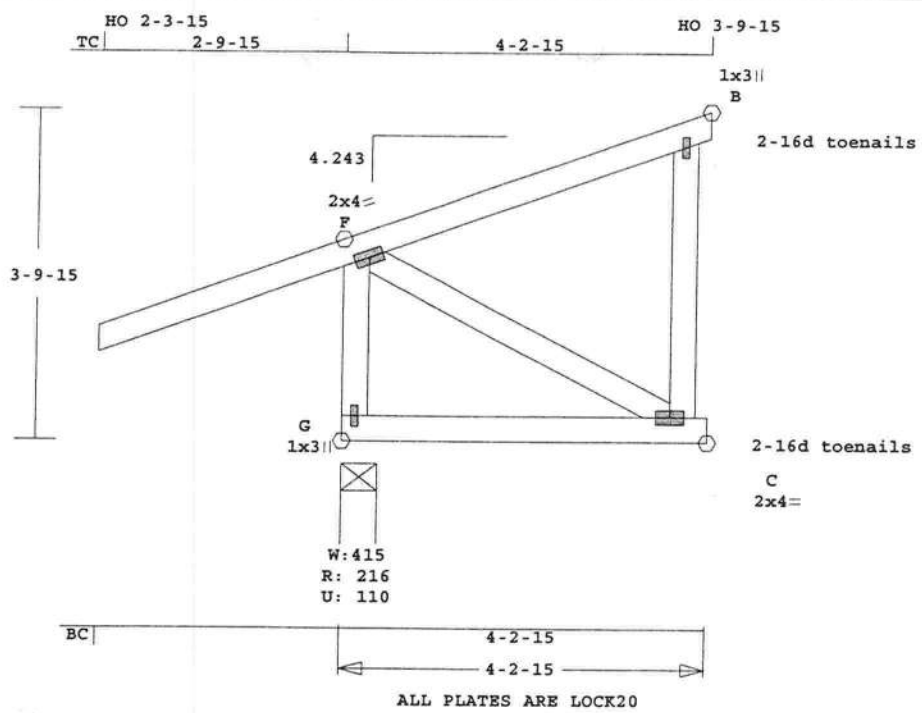
Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Max comp. force 3068 Lbs
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
 License # 58126
 Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
FWDEV-HAWKINS1	CJ2	1	MONO	40215	4.243	2- 9-15	0	T06080392

U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 39.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	Size	Lumber
TC	0.07 2x 4	SP-#2
BC	0.04 2x 4	SP-#2
WB	0.02 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 2-15
BC Cont.	0- 0- 0	4- 2-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading

Lumber Duration Factor	1.25			
Plate Duration Factor	1.25			
plf - Live	Dead	From	To	
TC V	40	20	0.0'	4.2'
BC V	0	20	0.0'	4.2'
TC V	-40	-20	0.0'	4.2'
	-12	-6		
BC V	0	-20	0.0'	4.2'
	0	-6		

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
G	217	110	4-15	1- 8
			Hz =	-76
C	38	0	3- 8	1- 8
B	57	10	3- 8	1- 8
			Hz =	96

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
F -B	0.07		69 T	0.00	0.07
-----Bottom Chords-----					
G -C	0.04		61 T	0.00	0.04
-----Webs-----					
G -F	0.01		52 T	WindLd	
F -C	0.01		51 C		
C -B	0.02		0 T	WindLd	

TL Defl -0.01" in G -C L/999
LL Defl 0.00" in G -C L/999
Shear // Grain in F -B 0.06

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
F LOCK 2.0x 4.0 Ctr Ctr 0.75
B LOCK 1.0x 3.0 Ctr Ctr 0.75
G LOCK 1.0x 3.0 Ctr Ctr 0.75
C LOCK 2.0x 4.0 Ctr Ctr 0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

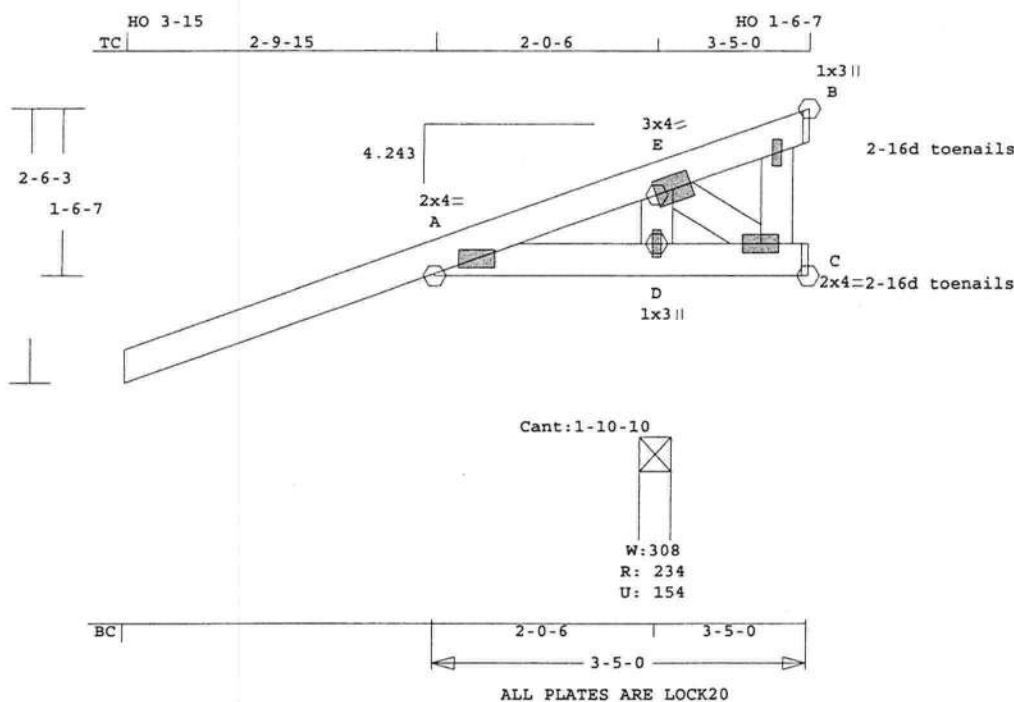
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:

FBC2004
Girder King Jack
Loading TC and BC
Setback 3- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Use properly rated hangers for
loads framing into girder
truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 51 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	-Size-	---	Lumber----
TC	0.00	2x 4	SP-#2
BC	0.01	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 5- 0
BC Cont.	0- 0- 0	3- 5- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading

Lumber Duration Factor	1.25			
Plate Duration Factor	1.25			
plf - Live	Dead	From	To	
TC V	40	20	0.0'	3.4'
BC V	0	20	0.0'	3.4'
TC V	-40	-20	0.0'	3.4'
BC V	0	-20	0.0'	3.4'
	0	-10		3.4'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
D	234	155	3- 8	1- 8
			Hz =	-12
C	41	4G	1- 8	1- 8
B	19	1	1- 8	1- 8
			Hz =	21

G = Gravity Uplift

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A - E	0.00		48 C		
E - B	0.00		12 T		
-----Bottom Chords-----					
A - D	0.01		46 T	0.00	0.01
D - C	0.01		59 T	0.00	0.01
-----Webs-----					
D - E	0.00		67 T		
E - C	0.00		55 C		
C - B	0.00		0 T	WindLd	

TL Defl	0.00"	in C - C	L/999
LL Defl	0.00"	in D - C	L/999
Shear // Grain		in A - E	0.01

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.72
E LOCK 3.0x 4.0 Ctr Ctr 0.40
B LOCK 1.0x 3.0 Ctr Ctr 0.75
D LOCK 1.0x 3.0 Ctr Ctr 0.75
C LOCK 2.0x 4.0 Ctr Ctr 0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

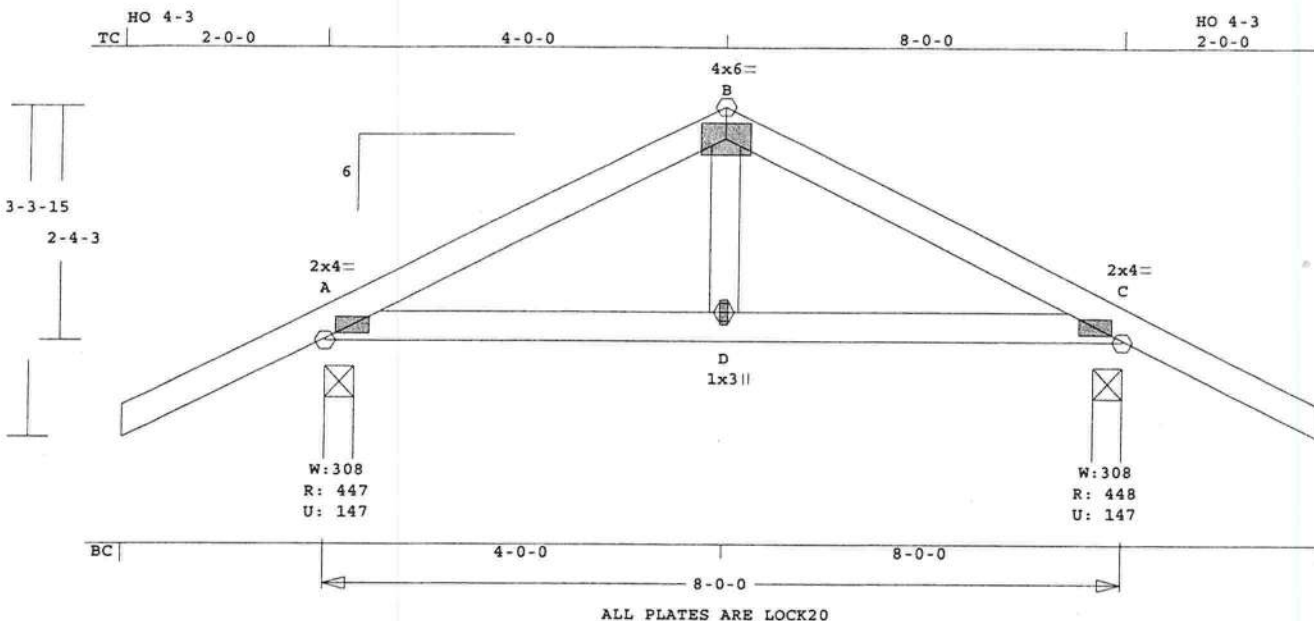
NOTES:
Trusses Manufactured by:

Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 2- 5- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Use properly rated hangers for
loads framing into girder
truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 55 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Scale: 0.528" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.3 LBS

Online Plus -- Version 19.0.034
 RUN DATE: 03-AUG-06

CSI	-Size-	---	Lumber	----
TC	0.10	2x 4	SP-#2	
BC	0.12	2x 4	SP-#2	
WB	0.02	2x 4	SP-#2	

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	8- 0- 0
BC Cont.		0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

A - D	0.12	335 T	0.05	0.07
D - C	0.12	335 T	0.05	0.07
D - B	0.02	168 T		

TL Defl -0.01" in D - C L/999
 LL Defl 0.00" in D - C L/999
 Shear // Grain in A - B 0.12

Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORT: NER 691

ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 2.0x 4.0 Ctr Ctr 0.71
 B LOCK 4.0x 6.0 Ctr Ctr 0.41
 C LOCK 2.0x 4.0 Ctr Ctr 0.71
 D LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:

Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-

concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--
 0- 0- 0 8- 0- 0
 Max comp. force 372 Lbs
 Quality Control Factor 1.25

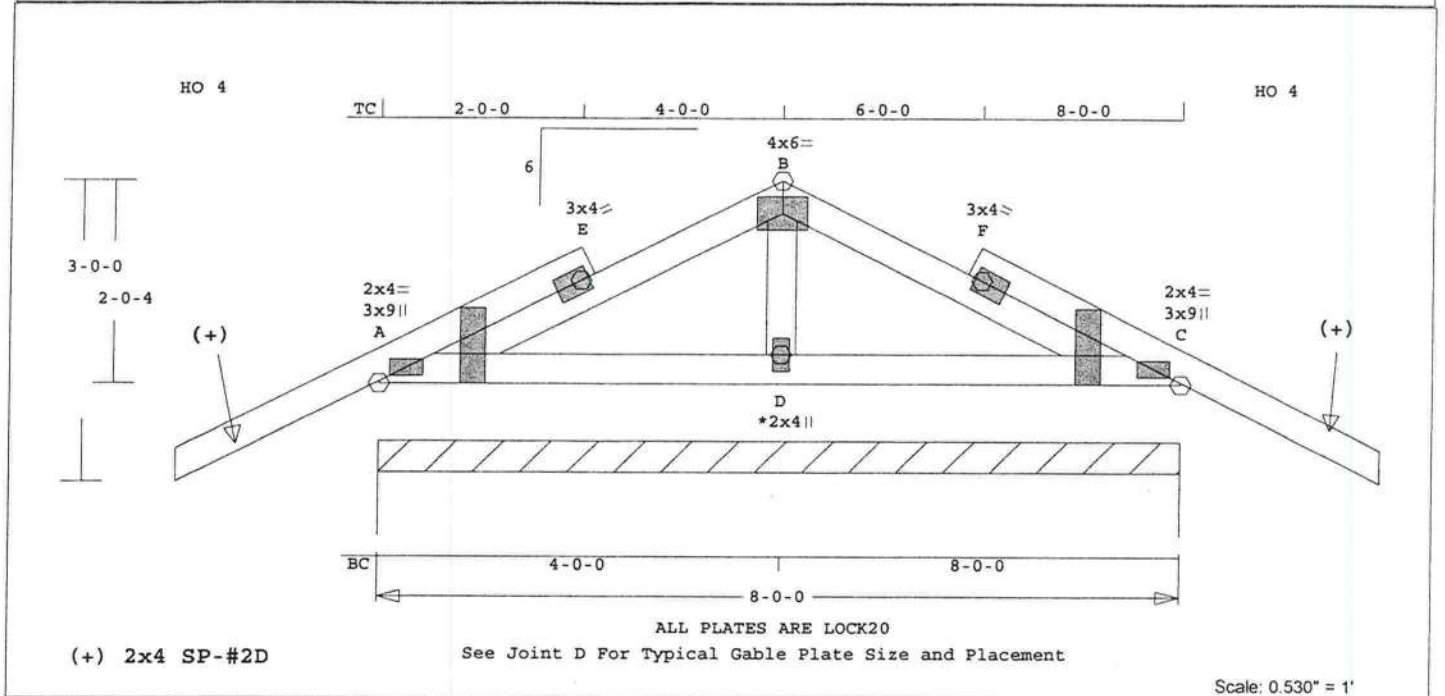
Truss Design Engineer: Philip J. O'Regan
 License #: 58126
 Address: P.O. Box 280055, Tampa, FL 33682



Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	448	147	3- 8	1- 8
			Hz =	-31
C	448	147	3- 8	1- 8
			Hz =	32

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A - B	0.10		372 C	0.00	0.10	
B - C	0.10		372 C	0.00	0.10	
-----Bottom Chords-----						

U# J#FWDEV-HAWKINS1 HAWKINS1



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.8 LBS

Online Plus -- Version 19.0.034
 RUN DATE: 03-AUG-06

CSI -Size-	Lumber	TL Defl	LL Defl	Shear // Grain
TC 0.11	2x 4 SP-#2	-0.01" in D -C L/999	0.00" in D -C L/999	0.11
BC 0.10	2x 4 SP-#2			
GW 0.03	2x 4 SP-#2			

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 0- 0
BC Cont.	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0	8- 0- 0	0
	896	130	Hz =	27

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -E	0.10		106 T	0.01		0.09
E -B	0.11		144 T	0.02		0.09
B -F	0.11		144 T	0.02		0.09
F -C	0.10		106 T	0.01		0.09
-----Bottom Chords-----						

Member	Length	Area	Weight
A -D	0.10	19 C	0.10
D -C	0.10	19 C	0.10
D -B	0.03	330 C	

TL Defl -0.01" in D -C L/999
 LL Defl 0.00" in D -C L/999
 Shear // Grain in E -B 0.11

Plates for each ply each face.
 PLATING CONFORMS TO TPI.

REPORT: NER 691
 ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
Plate - LOCK 20 Ga, Gross Area					
Plate - RHS 20 Ga, Gross Area					
Jt Type	Plt Size	X	Y	JSI	
A LOCK	3.0x 9.0	7.9	2.6	0.46	
A LOCK	2.0x 4.0	Ctr	Ctr	0.00	
E LOCK	3.0x 4.0	Ctr	Ctr	0.77	
B LOCK	4.0x 6.0	Ctr	Ctr	0.41	
F LOCK	3.0x 4.0	Ctr	Ctr	0.77	
C LOCK	3.0x 9.0	7.9	2.6	0.46	
C LOCK	2.0x 4.0	Ctr	Ctr	0.00	
D LOCK	2.0x 4.0	Ctr	Ctr	0.00	

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

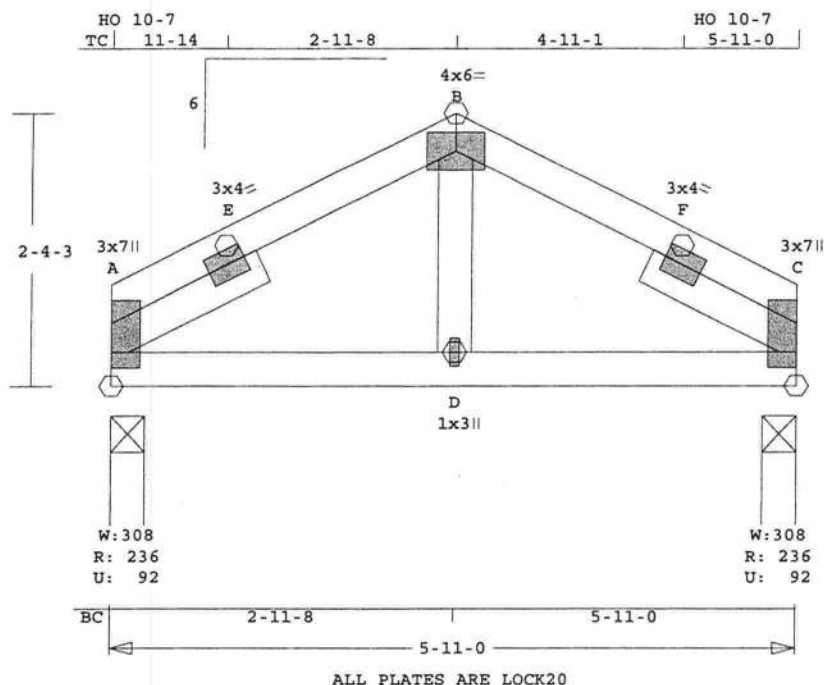
NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:

FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Refer to Gen Det 3 series for web bracing and plating.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Max comp. force 330 Lbs
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
 License #: 58126
 Address: P.O. Box 280055, Tampa, FL 33682



U# J#FWDEV-HAWKINS1 HAWKINS1



Scale: 0.612" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 03-AUG-06

CSI	Size	Lumber
TC	0.03 2x 4	SP-#2
BC	0.06 2x 4	SP-#2
WB	0.01 2x 4	SP-#2
SL	0.01 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5-11- 0
BC Cont.	0- 0- 0	5-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	237	92	3- 8	1- 8
			Hz =	-23
C	237	92	3- 8	1- 8
			Hz =	24

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -E	0.01		79 C	0.00	0.01	
E -B	0.03		200 C	0.00	0.03	
B -F	0.03		200 C	0.00	0.03	
F -C	0.01		79 C	0.00	0.01	
-----Bottom Chords-----						
A -D	0.06		178 T	0.01	0.05	

Member	Q	T	W	H
D -C	0.06	178 T	0.01	0.05
-----Webs-----				
D -B	0.01	104 T		
-----Sliders-----				
A -E	0.01	186 C		
F -C	0.01	186 C		

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in E -B 0.06

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 7.0 1.5 0.3 0.73
E LOCK 3.0x 4.0 Ctr Ctr 0.50
B LOCK 4.0x 6.0 Ctr Ctr 0.38
F LOCK 3.0x 4.0 Ctr Ctr 0.50
C LOCK 3.0x 7.0-1.5 0.3 0.73
D LOCK 1.0x 3.0 Ctr Ctr 0.75

Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC
regions --From-- ---To---
0- 0- 0 5-11- 0
Max comp. force 200 Lbs
Quality Control Factor 1.25

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

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